

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

PTR SECTION STAFF GUIDANCE

SELECTION OF BAFFLING FACTORS AND OPERATING CONDITIONS FOR T10 CALCULATIONS

Rules Affected: Title 30 Texas Administrative Code §290.42(c)(1), §290.42(d)(1), and §290.110(b)(1)

Background

The Texas Commission on Environmental Quality (TCEQ) regulations require utilities which use surface water sources, such as spring water, surface water or ground water under the influence of surface water (GUI), to achieve a 2.0-log removal of *Cryptosporidium parvum*, a 3.0-log removal/inactivation of *Giardia lamblia*, and a 4.0-log removal/inactivation of viruses. Physical removal is achieved by coagulation, sedimentation, and/or filtration processes while inactivation is achieved through disinfection processes.

To determine the inactivation level achieved, it is necessary to calculate the “Concentration-Time”, or “CT”, for the disinfection process. While it is possible to directly measure the disinfectant concentration, "C", the contact time, "T", must be derived. The theoretical detention time, ($T_{theoretical}$) depends on the volume of a basin and the rate that water flows through it. The actual contact time depends on these two factors and the hydraulic characteristics of the basin.

To compensate for the fact that individual drops of water do not always remain in the basin for exactly the same amount of time, the contact time used in CT calculations is T10, which is the time it takes for 10% of the water to pass through the basin. In basins with hydraulic conditions that cause water to move in a “plug-flow” fashion, T10 is about the same as the theoretical detention time. However, in basins that have a lot of short-circuiting, T10 can be much less than the theoretical detention time. The term “baffling factor” is used to describe the degree of short-circuiting that occurs within a basin. The relationship between the baffling factor and T10 is defined by the following relationship:

$$\begin{aligned} T_{10} &= \text{Baffling factor} \times T_{theoretical} \\ &= \text{Baffling factor} \times \text{Basin Volume} / \text{Flow Rate} \end{aligned}$$

The purpose of this document is to describe the method that the TCEQ staff will use to determine the value of the baffling factor, basin volume, and flow rate for the equation above in the absence of empirical data.

Guidance

Baffling Factor

Preferably, the baffling factor for a particular disinfection zone is determined by conducting a tracer study and collecting empirical data. However, the baffling factor for a particular basin can also be estimated based on the configuration of the basin and the degree of short-circuiting assigned. In basins where significant short-circuiting occurs, low baffling factors are assigned, while in well-baffled basins where there is less chance of short-circuiting, higher factors are expected. In the absence of empirical (tracer study) data, the baffling factors shown in Table 1 below may be used. It should be noted that these suggested baffling factors should be adjusted based on site-specific conditions. For example, it may be appropriate to assign two unbaffled basins that operate in series a baffling factor of 0.3 instead of 0.2 if they both fill from the top and discharge from the bottom.

Table 1: Suggested Baffling Factors for Various Basin Configurations.

| Basin Description | Examples | Baffling Factor |
|---|---|-----------------|
| Single chamber with a mechanical mixer ^(a) | Mechanical rapid mix basin Single-stage flocculator in its own basin | 0.1 |
| Tank with a single submerged inlet or a single submerged outlet and no intra-basin baffles ^(a) | Typical clearwell | 0.1 |
| Two unbaffled basins or tanks in series ^(a) | Two-stage rapid mix basin with mechanical mixers Two unbaffled clearwells operating in series | 0.2 |
| Three unbaffled tanks in series ^(a) | | 0.3 |
| Tank with a 2.0 foot air gap between the inlet and the water surface ^(c) | Clearwell with an internal or external riser that discharges above the maximum water level | 0.3 |
| Tank with 1 – 2 baffles ^(1,2) ^(a) | Clearwell with a baffled inlet and one baffle wall Sedimentation basin with an external flocculator, perforated inlet channel and straight-edge weir Flocculators and sedimentation basins in a single basin with no intra-basin baffles in the sedimentation basin | 0.3 |
| Tank with a length:width ratio greater than 10:1 ^(a) | Short, narrow, and shallow flow channel | 0.3 |

Table 1 continued.

| Basin Description | Examples | Baffling Factor |
|--|--|------------------------|
| Walker Claricone® Clarifier (b) | | 0.42 |
| Tank with 3 – 4 baffles ^(1,2) (a) | <p>Clearwell with baffled inlet and three internal baffle walls</p> <p>Three-stage flocculators with a perforated inlet channel, two internal baffling walls, and outlet baffling</p> <p>Typical flocculator/sedimentation basin with multi-stage flocculator, an intra-basin baffle in the sedimentation basin, and settled water launderers.</p> | 0.5 |
| Tank with a length:width ratio greater than 20:1 (a) | Long, narrow, and shallow flow channels | 0.5 |
| Tank with a length:width ratio greater than 50:1 (a) | Extremely long, narrow, and shallow flow channels | 0.7 |
| Filters (including inlet gullet, water above media, media bed, and plenum) (b) | | 0.7 |
| Trident®, Pacer II®, and Actiflow® (b) package plants | | 0.7 |
| Piping (a) | <p>Typical piping</p> <p>Narrow, shallow flow channels with a length: width ratio above 100:1</p> | 1.0 |
| Other package plant technologies (b) | Aquarius®, Tri-Mite®, etc | case-by-case |

(a) Crozes, Hagstrom, Clark, Ducoste, & Burns, 1999

(b) Tracer Studies

(c) Water Quality Technology Conference, 1988

Notes: **(1)** Assumes that:

- inlet baffles are used to dissipate the hydraulic energy of the water entering the basin; and
- intrabasin baffles are distributed fairly uniformly across the length and width of the basin in a manner that will maximize the length: width ratio of the flow path and achieve a uniform flow pattern across the entire basin.

(2) Baffles can include:

- an inlet diffusion plate;
- a perforated inlet header;
- a perforated inlet or outlet trough;
- perforated walls in a cross-flow or radial flow basin;
- solid walls in a basin with serpentine flow;
- weirs or launderers that span the entire width of the basin; or
- any similar device that is designed to dissipate the energy of the water entering or leaving a basin.

Basin Volume

To assure that disinfection requirements will be met under all operating conditions, the TCEQ staff will base their T_{10} calculations on the minimum volume that will be available in the basin. It is a relatively simple matter to determine the minimum water volume in a basin where the water level remains fairly constant. This is typically the case in rapid mix, flocculation, sedimentation, and clarification basins. It is also the case in many basins that have effluent weirs.

However, certain assumptions must be made when determining the minimum volume of a basin where the water level changes. These types of basins include clearwells and other basins where the outlet is located at the bottom of the basin instead of the top. In the absence of historical site-specific data, the TCEQ staff will use the following assumptions when establishing the minimum water level in variable-level basins:

- The minimum operating level in transfer wells and pump sumps will be based on the elevation of the low level pump cut-off switch.
- The minimum operating level in most clearwells will be based on:
 - 50% of the nominal tank capacity when the plant meets the TCEQ's minimum total storage capacity requirements; **OR**
 - 30% of the nominal tank capacity when the plant fails to meet the TCEQ's minimum total storage capacity requirements.

- A minimum operating level greater than 80% of the nominal clearwell capacity will not be approved in order to reduce the incentive to operate the plant for short, intermittent cycles. This mode of operation frequently results in poor particle removal in the clarification and filtration processes and thereby compromises the ability of the plant to remove *Cryptosporidium*.

Flow Rate

To ensure that disinfection requirements will be met under all operating conditions, TCEQ staff will base their T₁₀ calculations on the maximum anticipated flow rate through the basin.

Since the flow rate through an individual basin depends on the total flow rate through the plant and the number of parallel treatment units in the plant, the TCEQ staff member will make the following assumptions:

- The total flow rate through the plant will usually be based on the greater of the following two values:
 - the historical maximum daily raw water flow rate reported by the plant, **OR**
 - the rated capacity of the water treatment plant. (see Endnote 1)
- The flow rate through an individual basin will usually be based on one of the following values:
 - the percentage of the total plant flow that passes through the basin, **OR**
 - the design capacity of the particular treatment unit.
- If the plant has parallel treatment trains, the TCEQ will usually assume an equal flow distribution to each of the parallel basins. For example, if there are four identical clarifiers that are operating in parallel, the TCEQ will assume that each clarifier is treating 25% of the total flow through the plant.

The TCEQ is aware that flow is not always distributed evenly amongst all the treatment trains and that these assumptions do not reflect the conditions that exist under most actual operating conditions. However, the impact of the assumptions will be eliminated by the macros that are incorporated into the Surface Water Monthly Operating Report (SWMOR) spreadsheet.

While the influent flow rate and the effluent flow rate will always be identical in fixed-level basins, this is not the case for variable-level basins. In variable-level basins, the water level rises when the influent flow rate exceeds the effluent flow rate and the water level falls when the opposite conditions exist. On the other hand, by assuming that the variable level basin is operating at a minimum water level, we establish a condition where the overall influent and effluent flow rates will be equal.

For example, assume that the raw water pumps have a capacity of 1,000-gallons per minute (gpm) but the service pumps have a capacity of 5,000-gpm. Although the service pumps are capable of discharging 5 times as much water as the raw water pumps produce, they will only be operating 1/5 of the time when the water level reaches a minimum; otherwise, the water level would not be at a minimum. If they were operating more than 20% of the time, the water level would continue to fall; if they were operating less than 20% of the time, the water level in the clearwell would rise.

Endnotes:

- 1) As implied in the preceding flow rate discussion, the flow rate used in the CT Study approval letter may be different from the “approved” capacity of the plant.
- 2) The CT Study approval letter DOES set the minimum inactivation levels that must be met by a treatment plant and establish the flow rate and T10 time that should be used as a reference when completing the SWMOR.
- 3) The CT Study approval letter **DOES NOT**:
 - approve an exception to any of the TCEQ design requirements,
 - approve the construction of new facilities,
 - approve any Alternative Capacity Requirements, or
 - change the TCEQ-approved rated plant capacity, unless the letter explicitly and specifically addresses the issue.

Bibliography:

Crozes, G. F., Hagstrom, J. P., Clark, M. M., Ducoste, J., & Burns, C. (1999). *Improving Clearwell Design for CT Compliance*. AWWA Research Foundation and American Water Works Association.
Water Quality Technology Conference. (1988). AWWA. San Antonio.

Finalized and Approved by:

Joel Klumpp, Plan and Technical Review Section Manager: 04/04/2016

If no formal expiration date has been established for this staff guidance, it will remain in effect until superseded or canceled.

Revision History:

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| 11/29/2012 | Revised | Cindy Haynie |
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