Texas Commission on Environmental Quality

Edwards Aquifer Application Cover Page

Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with 30 TAC 213.

Administrative Review

- Edwards Aquifer applications must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.
 - To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: http://www.tceq.texas.gov/field/eapp.
- 2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
- 3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
- 4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.
 - An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.
- 5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
- 6. If the geologic assessment was completed before October 1, 2004 and the site contains "possibly sensitive" features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

Technical Review

- When an application is deemed administratively complete, the technical review period begins. The regional
 office will distribute copies of the application to the identified affected city, county, and groundwater
 conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days
 to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
- 2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

- clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.
- 3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited**.
- 4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

Mid-Review Modifications

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a "Mid-Review Modification". Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ's Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ's San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: Berry Creek Business Park			2. Regulated Entity No.: Not yet assigned				
3. Customer Name: Fidelis Realty Partners		4. Customer No.: CN603034521					
5. Project Type: (Please circle/check one)	New	Modif	Modification Extension		Exception		
6. Plan Type: (Please circle/check one)	WPAP CZP	SCS	SCS TUSTIASTIEATIEAT I		Technical Clarification	Optional Enhanced Measures	
7. Land Use: (Please circle/check one)	Residential	Non-residential			8. Sit	e (acres):	56.73 ac
9. Application Fee:	\$8,000	10. Permanent F		BMP(s):		Batch Detention	
11. SCS (Linear Ft.):	N/A	12. AST/UST (No		UST (No. Tanks):		N/A	
13. County:	Williamson County	14. Watershed:				Berry Creek	

Application Distribution

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the "Texas Groundwater Conservation Districts within the EAPP Boundaries" map found at:

http://www.tceq.texas.gov/assets/public/compliance/field_ops/eapp/EAPP%2oGWCD%2omap.pdf

For more detailed boundaries, please contact the conservation district directly.

Austin Region				
County:	Hays	Travis	Williamson	
Original (1 req.)	_	_	_X_	
Region (1 req.)	_	_	_X_	
County(ies)	_	_	_x_	
Groundwater Conservation District(s)	Edwards Aquifer AuthorityBarton Springs/ Edwards AquiferHays TrinityPlum Creek	Barton Springs/ Edwards Aquifer	NA	
City(ies) Jurisdiction	AustinBudaDripping SpringsKyleMountain CitySan MarcosWimberleyWoodcreek	AustinBee CavePflugervilleRollingwoodRound RockSunset ValleyWest Lake Hills	AustinCedar ParkFlorenceGeorgetownJerrellLeanderLiberty HillPflugerville x Round Rock	

	San Antonio Region					
County:	Bexar	Comal	Kinney	Medina	Uvalde	
Original (1 req.)	_	_	_	_	_	
Region (1 req.)	_				_	
County(ies)	_		_		_	
Groundwater Conservation District(s)	Edwards Aquifer Authority Trinity-Glen Rose	Edwards Aquifer Authority	Kinney	EAA Medina	EAA Uvalde	
City(ies) Jurisdiction	Castle HillsFair Oaks RanchHelotesHill Country VillageHollywood ParkSan Antonio (SAWS)Shavano Park	BulverdeFair Oaks RanchGarden RidgeNew BraunfelsSchertz	NA	San Antonio ETJ (SAWS)	NA	

I certify that to the best of my knowledge, that the ap application is hereby submitted to TCEQ for adminis	
Hollis Scheffler, P.E.	
Print Name of Customer/Authorized Agent	
	12/20/2022
Signature of Customer/Authorized Agent	Date

FOR TCEQ INTERNAL USE ONLY				
Date(s)Reviewed:	Date Ad	ate Administratively Complete:		
Received From:	Correct	Number of Copies:		
Received By:	Distribu	ution Date:		
EAPP File Number:	Comple	Complex:		
Admin. Review(s) (No.):	No. AR	No. AR Rounds:		
Delinquent Fees (Y/N):	Review	Review Time Spent:		
Lat./Long. Verified:	SOS Cu	stomer Verification:		
Agent Authorization Complete/Notarized (Y/N):				
Core Data Form Complete (Y/N):	Check:	Signed (Y/N):		
Core Data Form Incomplete Nos.:		Less than 90 days old (Y/N):		

General Information Form

Texas Commission on Environmental Quality

Print Name of Customer/Agent: Hollis Scheffler, P.E.

For Regulated Activities on the Edwards Aquifer Recharge and Transition Zones and Relating to 30 TAC §213.4(b) & §213.5(b)(2)(A), (B) Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

Date: 12/20/22

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **General Information Form** is hereby submitted for TCEQ review. The application was prepared by:

υa	Date. 12/20/22	
Sig	Signature of Customer/Agent:	
	A S	
Pi	Project Information	
1.	 Regulated Entity Name: <u>Berry Creek Business Park</u> 	
2.	2. County: Williamson	
3.	3. Stream Basin: <u>Berry Creek</u>	
4.	4. Groundwater Conservation District (If applicable): <u>n/a</u>	
5.	5. Edwards Aquifer Zone:	
	Recharge Zone Transition Zone	
6.	6. Plan Type:	
	✓ WPAP✓ SCS✓ Modification✓ AST✓ UST✓ Exception Request	est

7.	Customer (Applicant):	
	Contact Person: Steven Kimosh Entity: Fidelis Realty Partners Mailing Address: 8140 Walnut Hill Lane 400 City, State: Dallas, Texas Telephone: (512)554-6191 Email Address: skimosh@frpltd.com	Zip: <u>75231</u> FAX: <u>n/a</u>
8.	Agent/Representative (If any):	
	Contact Person: Hollis Scheffler, P.E. Entity: Westwood Professional Services Mailing Address: 8701 N. Mopac Expw. Ste. 320 City, State: Austin, Texas Telephone: 512-485-0831 Email Address: hollis.scheffler@westwoodps.com	Zip: <u>78759</u> FAX: <u>n/a</u>
9.	Project Location:	
	 ☑ The project site is located inside the city limits ☑ The project site is located outside the city limit jurisdiction) of ☑ The project site is not located within any city's 	s but inside the ETJ (extra-territorial
10.	The location of the project site is described bel detail and clarity so that the TCEQ's Regional st boundaries for a field investigation.	
	The property is located off the I 35 Service Roa Street.	d and at the end of William Scotsman
11.	Attachment A – Road Map. A road map showing project site is attached. The project location and the map.	_
12.	Attachment B - USGS / Edwards Recharge Zon USGS Quadrangle Map (Scale: 1" = 2000') of th The map(s) clearly show:	
	 ☑ Project site boundaries. ☑ USGS Quadrangle Name(s). ☑ Boundaries of the Recharge Zone (and Tran ☑ Drainage path from the project site to the boundaries. 	• • • •
13.	The TCEQ must be able to inspect the project sufficient survey staking is provided on the protect the boundaries and alignment of the regulated features noted in the Geologic Assessment.	ject to allow TCEQ regional staff to locate

\boxtimes	Survey staking will be completed by this date: 1/1/2023
14. 🔀	Attachment C – Project Description . Attached at the end of this form is a detailed narrative description of the proposed project. The project description is consistent throughout the application and contains, at a minimum, the following details:
	 ✓ Area of the site ✓ Offsite areas ✓ Impervious cover ✓ Permanent BMP(s) ✓ Proposed site use ✓ Site history ✓ Previous development ✓ Area(s) to be demolished
15. Exis	sting project site conditions are noted below:
	Existing commercial site Existing industrial site Existing residential site Existing paved and/or unpaved roads Undeveloped (Cleared) Undeveloped (Undisturbed/Uncleared) Other:
Proh	nibited Activities
16. 🔀	I am aware that the following activities are prohibited on the Recharge Zone and are not proposed for this project:
	(1) Waste disposal wells regulated under 30 TAC Chapter 331 of this title (relating to Underground Injection Control);
	(2) New feedlot/concentrated animal feeding operations, as defined in 30 TAC §213.3;
	(3) Land disposal of Class I wastes, as defined in 30 TAC §335.1;
	(4) The use of sewage holding tanks as parts of organized collection systems; and
	(5) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41(b), (c), and (d) of this title (relating to Types of Municipal Solid Waste Facilities).
	(6) New municipal and industrial wastewater discharges into or adjacent to water in the state that would create additional pollutant loading.
17.	I am aware that the following activities are prohibited on the Transition Zone and are not proposed for this project:
	(1) Waste disposal wells regulated under 30 TAC Chapter 331 (relating to Underground Injection Control);

- (2) Land disposal of Class I wastes, as defined in 30 TAC §335.1; and
- (3) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41 (b), (c), and (d) of this title.

Administrative Information

18.	The fee	e for the plan(s) is based on:
	who foo foo nur A re	ra Water Pollution Abatement Plan or Modification, the total acreage of the site ere regulated activities will occur. If an Organized Sewage Collection System Plan or Modification, the total linear otage of all collection system lines. If a UST Facility Plan or Modification or an AST Facility Plan or Modification, the total mber of tanks or piping systems. If a use of tanks or piping systems is equest for an exception to any substantive portion of the regulations related to the otection of water quality. If a use of tanks or piping systems is equest for an extension to a previously approved plan.
19.	fee cor	plication fees are due and payable at the time the application is filed. If the correct is not submitted, the TCEQ is not required to consider the application until the rect fee is submitted. Both the fee and the Edwards Aquifer Fee Form have been at to the Commission's:
	\boxtimes	TCEQ cashier Austin Regional Office (for projects in Hays, Travis, and Williamson Counties) San Antonio Regional Office (for projects in Bexar, Comal, Kinney, Medina, and Uvalde Counties)
20.	nee	omit one (1) original and one (1) copy of the application, plus additional copies as eded for each affected incorporated city, groundwater conservation district, and unty in which the project will be located. The TCEQ will distribute the additional pies to these jurisdictions. The copies must be submitted to the appropriate regional ice.
21.		person shall commence any regulated activity until the Edwards Aquifer Protection n(s) for the activity has been filed with and approved by the Executive Director.

Attachment A – Road Map



Attachment B – USGS / Edwards Recharge Zone Map



CHECKED BY

HAS

SCALE

1"=2000'

DATE

12/2/2022

DRAWN BY

JPQ

130

INDIAN-MOUND RD

TONKOWA

MA DR

BERRY CREEK BUSINESS PARK

JOB NUMBER

4670-22.125

4000

GROVE LN

1000

Berry Springs Cemetery 2000

GRAPHIC SCALE IN FEET

Attachment C - Project Description

The proposed development includes the construction of three industrial buildings with all associated grading, drainage, utility, detention, parking, vehicular conveyance, and water quality improvements on 56.73 acres of a combination of developed and undeveloped land. The proposed site is located at 3000 N IH 35 NB in Georgetown, Texas 78626. The existing site consists of light paving, gravel, and Class D soil classification. According to FEMA Maps 48491C0285F (dated 12/20/2019) and 48491C0292F (dated 12/20/2019), the subject site does not fall within a regulatory floodplain.

The site generally slopes at $\pm 0.5\%$ from the middle of the north side of the site to the south of the site. Around the site there is only undeveloped land.

The project will consist of three industrial buildings, associated grading, utilities, parking, public roadways, and tie into existing sewer and water lines down the South Interstate 35 Service Road. The total impervious cover on the site is 28.12 acres. All proposed impervious cover is to be treated with the proposed batch detention pond.

As included in our demolition plans, we will first be removing the existing fences throughout the property in order to have the space open for construction. The existing buildings and pavement throughout the site will be demolished for the design of the three new proposed industrial buildings. We will also be removing any existing signs and gravel throughout the property.



Geologic Assessment

Texas Commission on Environmental Quality

For Regulated Activities on The Edwards Aquifer Recharge/transition Zones and Relating to 30 TAC §213.5(b)(3), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

P.G. Fax: 512-493-9693 Date: 3/28/2023 Representing: Braun Intertec Corporation, TBPG Registration No. 50151 (Name of Company)	/ and
Representing: <u>Braun Intertec Corporation</u> , <u>TBPG Registration No. 50151</u> (Name of Company	/ and
•	/ anc
TBPG or TBPE registration number)	
Signature of Geologist:	
MELISSA L. WANN	
Regulated Entity Name: Berry Creek Business Park Project Information GEOLOGY 3484 FOR CENSED SCHOOL STATE OF	
Project Information Project Information	
1. Date(s) Geologic Assessment was performed: <u>10/28/2022</u> , <u>11/2/2022</u> , and <u>3/21/2023</u>	
2. Type of Project:	
SCS	
3. Location of Project:	
Recharge Zone Transition Zone	

Contributing Zone within the Transition Zone

- 4. Attachment A Geologic Assessment Table. Completed Geologic Assessment Table (Form TCEQ-0585-Table) is attached.
- 5. Soil cover on the project site is summarized in the table below and uses the SCS Hydrologic Soil Groups* (Urban Hydrology for Small Watersheds, Technical Release No. 55, Appendix A, Soil Conservation Service, 1986). If there is more than one soil type on the project site, show each soil type on the site Geologic Map or a separate soils map.

Table 1 - Soil Units, Infiltration Characteristics and Thickness

Soil Name	Group *	Thickness(fee t)
SvA - Sunev silty loam	В	6.67
KrB - Krum silty clay	С	6
OkA - Oakalla silty clay loam	В	6.67
BkrG - Brackett- Rock outcrop/Gravell y clay loam	D	5

Soil Name	Group *	Thickness(fee t)

- * Soil Group Definitions (Abbreviated)
 - A. Soils having a high infiltration rate when thoroughly wetted.
 - B. Soils having a moderate infiltration rate when thoroughly wetted.
 - C. Soils having a slow infiltration rate when thoroughly wetted.
 - D. Soils having a very slow infiltration rate when thoroughly wetted.
- 6. Attachment B Stratigraphic Column. A stratigraphic column showing formations, members, and thicknesses is attached. The outcropping unit, if present, should be at the top of the stratigraphic column. Otherwise, the uppermost unit should be at the top of the stratigraphic column.
- 7. Attachment C Site Geology. A narrative description of the site specific geology including any features identified in the Geologic Assessment Table, a discussion of the potential for fluid movement to the Edwards Aquifer, stratigraphy, structure(s), and karst characteristics is attached.
- 8. Attachment D Site Geologic Map(s). The Site Geologic Map must be the same scale as the applicant's Site Plan. The minimum scale is 1": 400'

Applicant's Site Plan Scale: 1'' = 40'Site Geologic Map Scale: 1'' = 40'

Site Soils Map Scale (if more than 1 soil type): 1" = 40'

9. Method of collecting positional data:

☐ Global Positioning System (GPS) technology.☐ Other method(s). Please describe method of data collection:	
10. $igotimes$ The project site and boundaries are clearly shown and labeled on the Site Geologic Ma	ър.
11. $igotimes$ Surface geologic units are shown and labeled on the Site Geologic Map.	
12. Geologic or manmade features were discovered on the project site during the field investigation. They are shown and labeled on the Site Geologic Map and are described in the attached Geologic Assessment Table.	d
Geologic or manmade features were not discovered on the project site during the field investigation.	t
13. The Recharge Zone boundary is shown and labeled, if appropriate.	
14. All known wells (test holes, water, oil, unplugged, capped and/or abandoned, etc.): If applicable, the information must agree with Item No. 20 of the WPAP Application Section.	
 ☐ There are 1 (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply.) ☐ The wells are not in use and have been properly abandoned. ☐ The wells are not in use and will be properly abandoned. ☐ The wells are in use and comply with 16 TAC Chapter 76. ☐ There are no wells or test holes of any kind known to exist on the project site. 	
Administrative Information	
15. Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and	

county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

LIST OF ATTACHMENTS

Attachment A Geologic Assessment Table

Attachment B Stratigraphic Column

Attachment C Site Geology & Photographic Log

Attachment D Site Maps

Attachment E Physical Setting Report



Attachment A Geologic Assessment Table



GEOLO	GIC ASSE	SSMENT TA	BLE				PRO	JECT	NAME	:		Fidelis l	ndustrial -	58 Acre Pro	perty, G	eorge	town,	Texas		
LOCATION						FEATURE CHARACTERISTICS								EVALUATION		PHYSICAL SETTING				
1A	1B *	1C*	2A	2B	3		4		5	5A	6	7	8A	86	9	1	10	1	11	12
FEATURE ID	LATITUDE	LONGITUDÉ	FEATURE TYPE	POINTS	FORMATION	DIN	IENSIONS (F	EET)	TREND (DEGREES)	DOM	DENSITY (NO/FT)	APERTURE (FEET)	INFILL	RELATIVE INFILTRATION RATE	TOTAL	SENS	ITIVITY	GATCHME (ACE	ENT AREA RES)	TOPOGRAPHY
						Х	Y	Z		10						<40	<u>>40</u>	<1.6	<u>>1.6</u>	
Berry Creek	30.689250	-97.652162	Z	30_	Kgt	2,575	varies	varies	S40°E		_	`	С,О	34	64		х		х	Streambed
Dry Creek Bed	30.687209	-97.653381	Z	30_	Kgt	1,126	varies	varies	N40°E		_		N,C,O	45	75		x		х	Streambe
Water Well	30.688260	-97.653479	МВ	30	Ked	0.8	0.8_	225		_		0.5	х	45	75		х	х		Hilltop
				•																-
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																			_	
			<u> </u>		1-					_						├	├	一		

* DATUM:_	WGS 84	
2A TYPE	TYPE	2B POINTS
С	Cave	30
sc	Solution cavity	20
SF	Solution-enlarged fracture(s)	20
F	Fault	20
0	Other natural bedrock features	5
MB	Manmade feature in bedrock	30
sw	Swallow hole	30
SH	Sinkhole	20
CD	Non-karst closed depression	5
z	Zone, clustered or aligned features	30

	8A INFILLING	
N	None, exposed bedrock	
С	Coarse - cobbles, breakdown, sand, gravel	
0	Loose or soft mud or soil, organics, leaves, sticks, dark colors	
F	Fines, compacted clay-rich sediment, soil profile, gray or red colors	
V	Vegetation. Give details in narrative description	
FS	Flowstone, cements, cave deposits	
Х	Other materials - Water well was completed using 60 feet of 6-inch diameter plastic pipe, grouted to a depth of 18 feet.	

12 TOPOGRAPHY

I have read, I understood, and I have followed the Texas Commission on Environmental Quality's Instructions to Geologists. The information presented here complies with that document and is a true representation of the conditions observed in the field.

My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

Molinsa Worm P.G.

ate 3/28/2023

Sheet ___1__ of __1___ (Attachment)

TCEQ-0585-Table (Rev. 10-01-04)

MELISSA L. WANN

MELISSA L. WANN

GEOLOGY
3484

CSONAL TO GEOSCIE

S. 31-202-3

Geologic Assessment Table - Attachment 1

GPS Requirements

DATUM: WGS 84

METHOD: GPS Unit - Garmin eTrex 30x

DATE: October 28, 2022 USER: Melissa Wann

DATUM: WGS 84

METHOD: GPS Unit - Trimble R1 99133

DATE: March 21, 2023 USER: Melissa Wann

Attachment B Stratigraphic Column



SYSTEM	GROUP	FORMATION or MEMBER	APPROX. THICKNESS (ft)
QUATERNARY	N/A	Fluviatile Terrace Deposits, undivided (Qu)	Up to 50 ft
0	WASHITA	Georgetown Limestone, (Kgt)	30 to 80 ft
LOWER CRETACEOUS	FREDRICKSBURG	Edwards and Commanche Peak Limestones, undivided (Kec)	60 to 350 ft

Attachment B

Stratigraphic Column
Fidelis Industrial - 58-Acre Property
Georgetown, Texas



11/3/2022 Braun Project No.: B2210531

Drawn by: WW

Checked by: PF

Revised by:

Attachment C Site Geology & Photographic Log



The approximately 58-acre property is located east of the intersection of the northbound North IH-35 Frontage Road and William Scotsman Road in Georgetown, Williamson County, Texas (Site). The Site consists of approximately 25 acres of agriculture land and an approximately 8.5-acre area developed as a gravel, fenced commercial mobile trailer storage facility. The remaining approximately 24.5 acres are riparian forest that includes Berry Creek along the north and east of the Site and a dry creek bed along the southern Site boundary.

The Site is located in central Williamson County on the western edge and uplifted portion of the Balcones Fault Zone. The Balcones Fault Zone divides two distinct physiographic provinces: the structurally uplifted Hill Country of the Edwards Plateau to the west and the Blackland Prairie of the Gulf Coast Plain to the east. The uplifted portion of the Balcones Fault Zone contains generally flat-lying beds and few faults. The nearest mapped fault is located approximately 1 mile southwest of the Site. The Site is within the mapped Edwards Aquifer Recharge Zone.

The Site is relatively flat and elevation ranges from approximately 670 to 680 feet above mean sea level (AMSL). Surface water runoff appears to occur as overland flow across the Site with a general radial drainage pattern to Berry Creek on the north and east, and toward the dry creek bed channel along the southern Site boundary; several rills were observed emanating from the fenced off developed area toward the dry creek bed, with the most significant located just outside the fence at the southwestern corner of the fenced facility.

Based on a review of the Bureau of Economic Geology (BEG) Geologic Atlas of Texas, Austin Sheet (1974), Fluviatile Terrace Deposits (undivided, Qu) underlay the Site. The Georgetown Limestone (Kgt) underlays the Fluviatile Terrace Deposits. The Fluviatile Terrace Deposits consist of various proportions of gravel, sand, silt and clay with a thickness that ranges up to 50 feet. The Georgetown Limestone in this area consists of fine-grained limestone and marl with a thickness that ranges from 30–80 feet and outcrops in a small area along the dry creek bed on the southern property boundary, as well as throughout the region to the west of the Site. The Edwards and Comanche Peak Limestones (undivided, Kec) underlay the Georgetown Limestone. The Edwards Limestone formation is comprised of limestone, dolostone, and chert that is fine-grained, massive- to thin-bedded, hard, and brittle. The Comanche Peak Limestone formation is comprised of limestone that is fine to very fine grained, fairly hard, and nodular.

A field survey of the riparian forest area and agricultural portion of the Site was conducted by Patrick Fortson, Melissa Wann, and Wesmond Williams on October 28, 2022; Ms. Wann returned to conduct a survey of the developed (fenced) portion of the Site on November 2, 2022. The initial field survey included reconnaissance of the property performed on foot on approximately 30-foot or smaller transects. The forested portion of the Site was vegetated with herbaceous cover, woody vines, understory shrub, and trees ranging from approximately 1-inch diameter to large live oak and pecan trees with up to 2-foot diameter trucks. The agricultural portion of the Site was vegetated and contained recently mowed hay. Minimal areas were bare soil cover with no vegetation. Soil cover was observed in the agricultural and fenced portion of the Site. According to the well report for the water well at the Site, limestone was encountered at 14 ft below ground surface. Rock outcrops indicative of the Georgetown Limestone were observed in the dry creek bed along the southern Site boundary and in cut banks of Berry Creek along the northern and eastern Site boundary during the field survey. Soil cover in the dry creek bed, was estimated to be 0-10 ft thick. No evidence of sinkholes or faults was observed during the field survey; however, karst features such as solution cavities were observed in portions of the dry creek bed. Berry Creek and the dry creek bed are interpreted to be recharge features because they are assumed to be losing streams with direct recharge into the Georgetown Limestone and then to the Edwards Limestone. Additionally, a review

of published records did not reveal any caves or other mapped karst features in the general Site vicinity. An additional field survey was conducted on March 21, 2023 by Patrick Fortson, P.G., and Melissa Wann, P.G., from Braun Intertec, and James "Bo" Sloan, P.G., and Colin Gearing from the Texas Commission of Environmental Quality (TCEQ). The additional field survey was conducted to further assess Berry Creek along the northern Site boundary and the dry creek bed along the southern Site boundary. Both features are discussed in more detail below. Photographs from the initial field survey and follow up field survey are provided in **Attachment C**.

The following sensitive features were observed during Site reconnaissance and are listed in the Geologic Assessment Table. The sensitive features and associated buffer, where applicable, are presented on the Attachment D Geologic, Soils, and Sensitive Feature Location Maps (Attachment D Maps).

Berry Creek: Berry Creek is present along the northern and eastern Site boundary. Water was flowing during each field survey. Berry Creek flows from west to east before curving to the south/southeast. The creek cuts through fluvial terrace deposits and mostly low-cut banks were observed. However, a steep cut bank, approximately 18 to 20 feet high, forms the southern bank along the western portion of the creek. Bedrock outcrops were observed along some of the cut banks and limestone cobbles were observed along creek banks. Along the northern Site boundary, limestone bedrock was observed in the creek bed. Along the northeastern and eastern Site boundary, the creek bed was not observable but it is assumed bedrock is present beneath sediment.

Berry Creek is listed as a feature on the Geologic Assessment Table as it was observed to be shallow and is assumed to be a losing stream with recharge into the Georgetown Limestone and then to the Edwards Limestone. Based on observations along the western reach of Berry Creek, and given the extent of Berry Creek, Mr. Sloan and Mr. Gearing of the TCEQ indicated that a minimum 50-foot buffer would be required adjacent to Berry Creek. The buffer is shown on the **Attachment D Maps**.

Dry Creek bed: A dry creek bed is present along the southern Site boundary extending from the North IH-35 Frontage Road at the western Site boundary to Berry Creek at the southeastern portion of the Site, as shown on the **Attachment D Maps**. The creek bed was dry during the initial field survey, but small pools of ponded water were observed along a portion of the central to western reach during the March 21, 2023 field survey, which occurred following precipitation events a week prior. Based on topography, observed vegetation patterns, and aerial imagery, when water is present it flows from west to east and discharges to Berry Creek.

Outcrop of the Georgetown Limestone (approximately 20 feet high) was observed along the southern bank in the eastern portion of the dry creek bed during the field surveys. The outcrop is deeply weathered, several small slides, and some solution cavities were observed.

The thickness of soil cover in the dry creek bed varies. In the western reach, soil cover up to approximately 1 foot thick was observed; however, soil cover decreases to the east as the dry creek bed cuts into exposed bedrock. The western reach of the dry creek bed is not considered a sensitive feature as the thickness of soil cover in this area appears to prevent rapid infiltration to the Edwards Aquifer. Additionally, the pools of water in this portion of the creek bed appeared stagnant during the March 21, 2023 field survey indicating the potential for rapid infiltration was low. Mr. Sloan and Mr. Gearing of the TCEQ agreed with this interpretation. Although no significant direct recharge features were observed within and along the creek bed, solution cavities in the bedrock outcrop area and limestone cobbles with signs of karst weathering were observed. As a result, the eastern portion of the dry creek bed is considered a sensitive

feature and assumed to be a losing stream (following precipitation events) with direct recharge into the Georgetown Limestone and then to the Edwards Limestone.

Based on conditions observed within the sensitive portion of the dry creek bed and minimal soil cover, Mr. Sloan and Mr. Gearing indicated that a minimum 50-foot buffer would be required around the eastern portion of the dry creek bed identified as a sensitive feature.

Water Well #5819633: A water well (State Well number 5819633) is located in a small pump house on the southwestern portion of the fenced area. As indicated on the well log obtained from the TCEQ well data base, the well was installed in 1998 and consists of plastic casing to a depth of 60 feet, grouted to a depth of 18 feet; the well was drilled to a total depth of 225 feet below ground surface and produces from the Edwards Aquifer. The location of the well is shown on the Attachment D Maps. Based on review of the well log obtained from TCEQ, information from the Texas Water Development Board (TWDB), and observations of the wellhead during Site reconnaissance, the wellhead is secure and not exposed; however, the well is considered a rapid infiltration feature due to the nature of the well being a direct conduit into the Edwards Limestone should a release occur and impact the well, or if the wellhead was damaged. The water well is planned to be plugged and abandoned prior to development of the Site, which would remove the potential for a release to the well affecting the Edwards Aquifer.

No other manmade features, natural features, or documented features that would be considered sensitive features were observed in the area during the field survey. Berry Springs, located approximately 2,500 feet southeast of the Site was identified on aerial images reviewed as part of this assessment.

Other notable features observed during the field survey include:

- One small (10-inch diameter) animal burrow was observed in soil (no bedrock observed) on the
 north side of the fenced developed area, near the eastern corner of the fence. Because bedrock
 was not observed and there were no indications of infiltration or a recharge feature, it was not
 considered a sensitive feature at the Site.
- A small depression, approximately 5 feet across, was observed in the southwestern corner of the developed area. This depression is adjacent to a low (less than 4 feet) manmade berm along the southern fence and adjacent to an observed drainage feature outside of the southwest corner of the fence that drains to the dry creek bed. Based on observations of this area, from upslope of the creek on the developed property and from the dry creek bed, the drainage feature and the small depression appear to be the result of headward erosion from surface drainage. Bedrock was not observed in this area of the developed property and no evidence of rapid infiltration was observed. The depression is not considered a sensitive feature at the Site.



Photo 1: View of northwest field, viewed facing northwest.



Photo 2: View of southwest field, viewed facing southwest.





Photo 3: View of dry creek bed along the southwest Site boundary, viewed facing southeast.



Photo 4: View of dry creek bed along the south Site boundary, viewed facing east.





Photo 5: Common root collapse found in the dry creek bed along the southern Site boundary.



Photo 6: Common soil depression found in the dry creek bed .





Photo 7: View of bedrock outcrop found at the southern Site boundary, viewed facing southeast.



Photo 8: View of bedrock outcrop found at the southern Site boundary, viewed facing south.





Photo 9: View of Berry Creek along the east Site boundary, viewed facing east.



Photo 10: View of Berry Creek along the north Site boundary, viewed facing south.





Photo 11: View of solution cavity in bedrock in dry creek bed at southern Site boundary.



Photo 12: View of on-site water well (State Well ID 5819633).





Photo 13: View of central field, viewed facing southeast.



Photo 14: View of burrow opening at north exterior fencing, viewed facing northeast.





Photo 15: View of southwest corner berm and depression, viewed facing southeast.



Photo 16: View of southern boundary fencing and berm, viewed facing west.





Photo 17: View of drainage at southwestern fence corner, viewed facing northwest.



Photo 18: View of drainage at southwestern fence corner, viewed facing southeast.





Photo 18: View of cliff along Berry Creek on northwestern portion of the site, viewed facing west.



Photo 19: View of western portion of dry creek bed, viewed facing east.





Photo 20: View of dry creek bed at bedrock outcrop, viewed facing east.



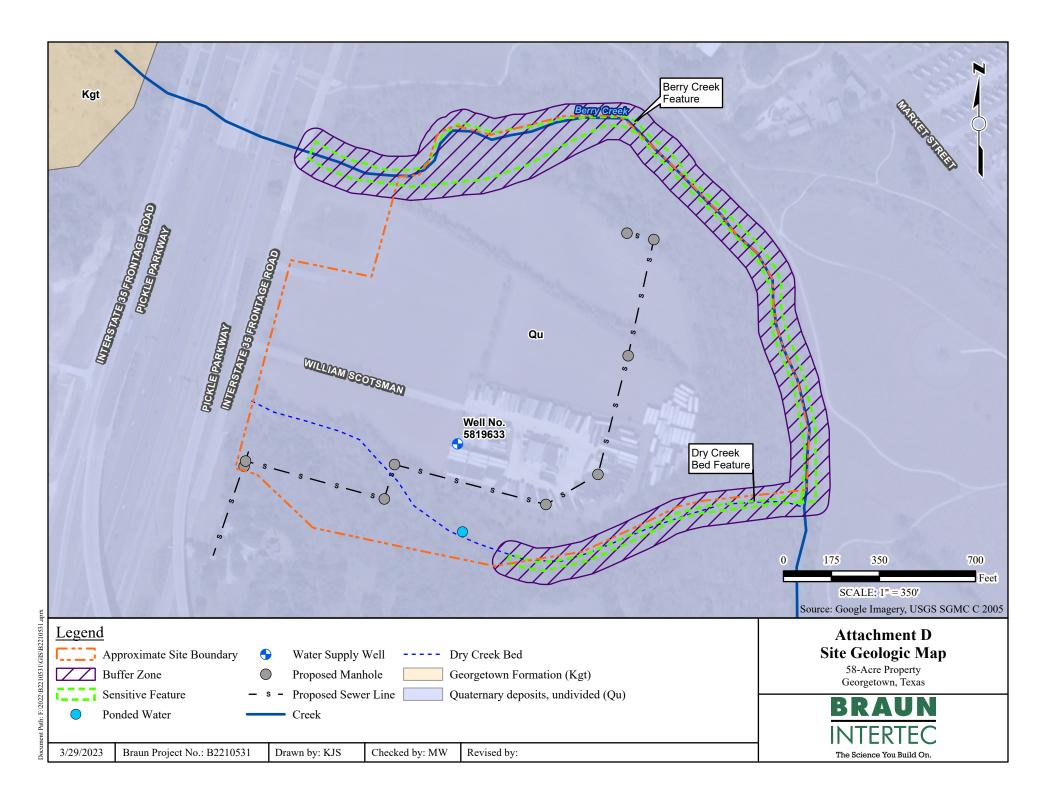
Photo 21: View of western portion of dry creek bed showing ponded water and soil cover supporting vegetation, viewed facing east.

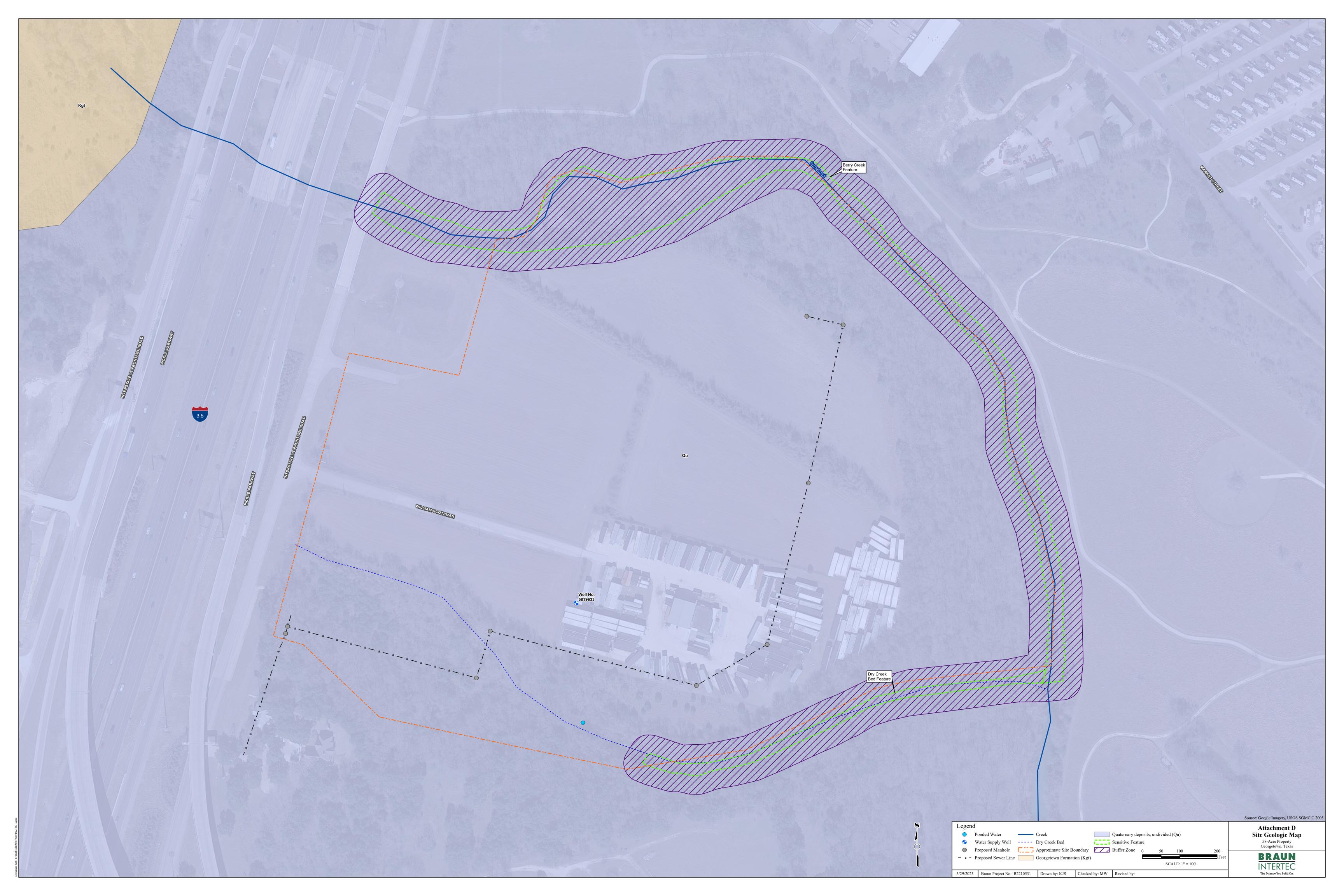


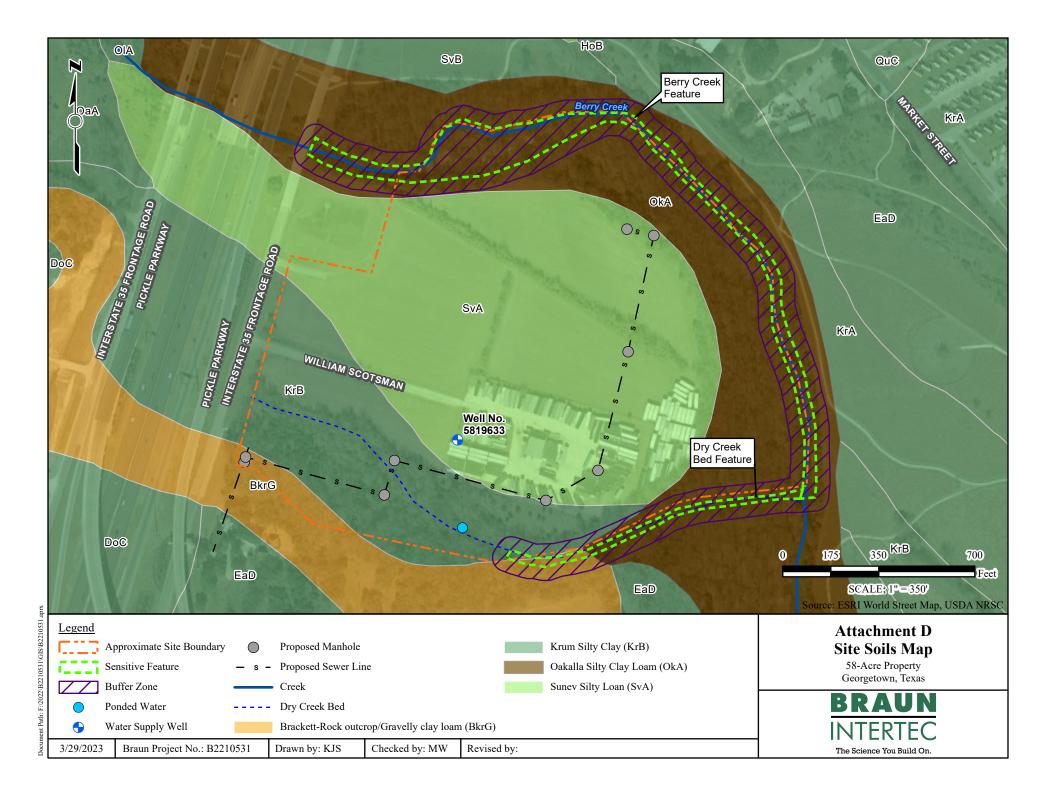
Attachment D

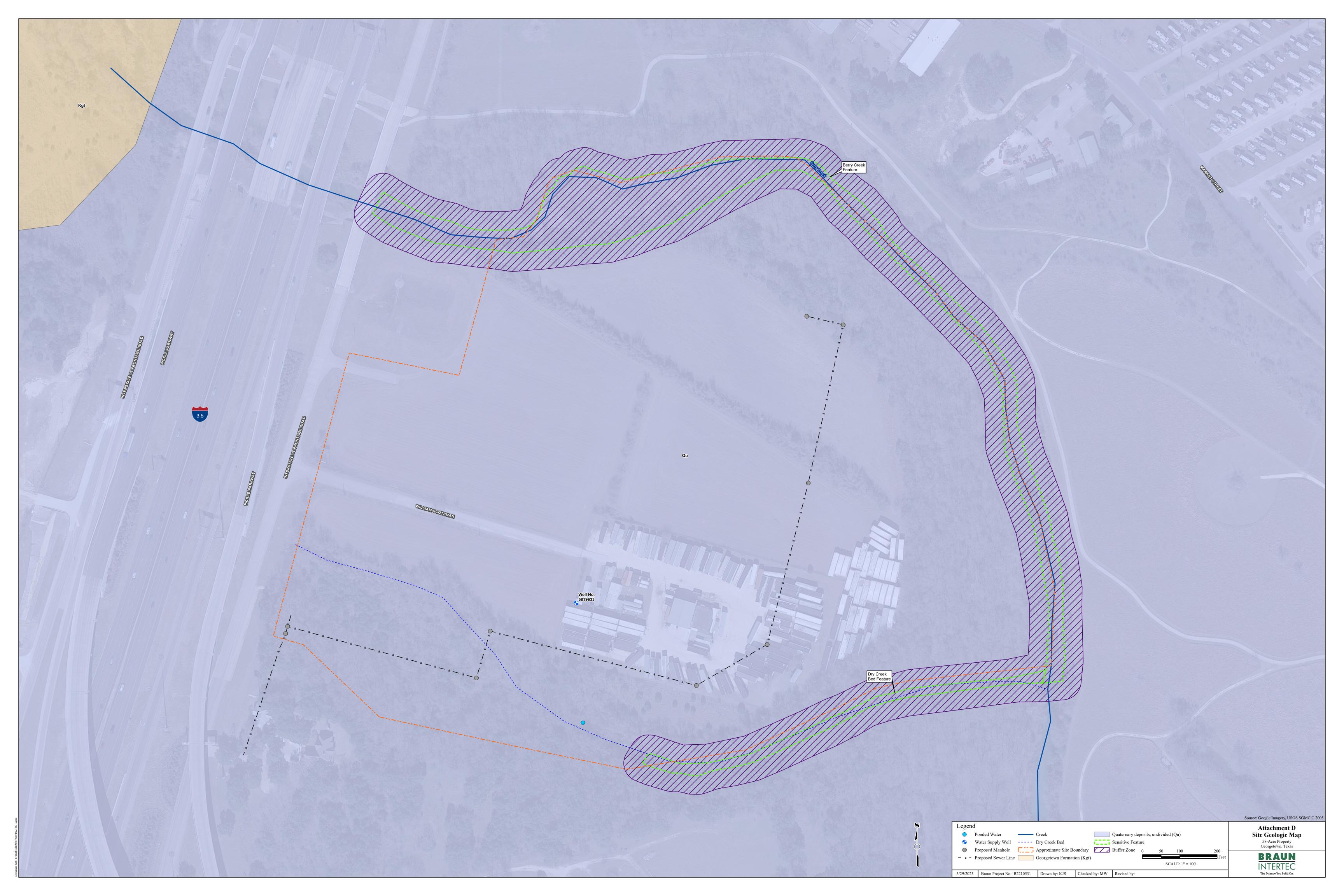
Site Maps

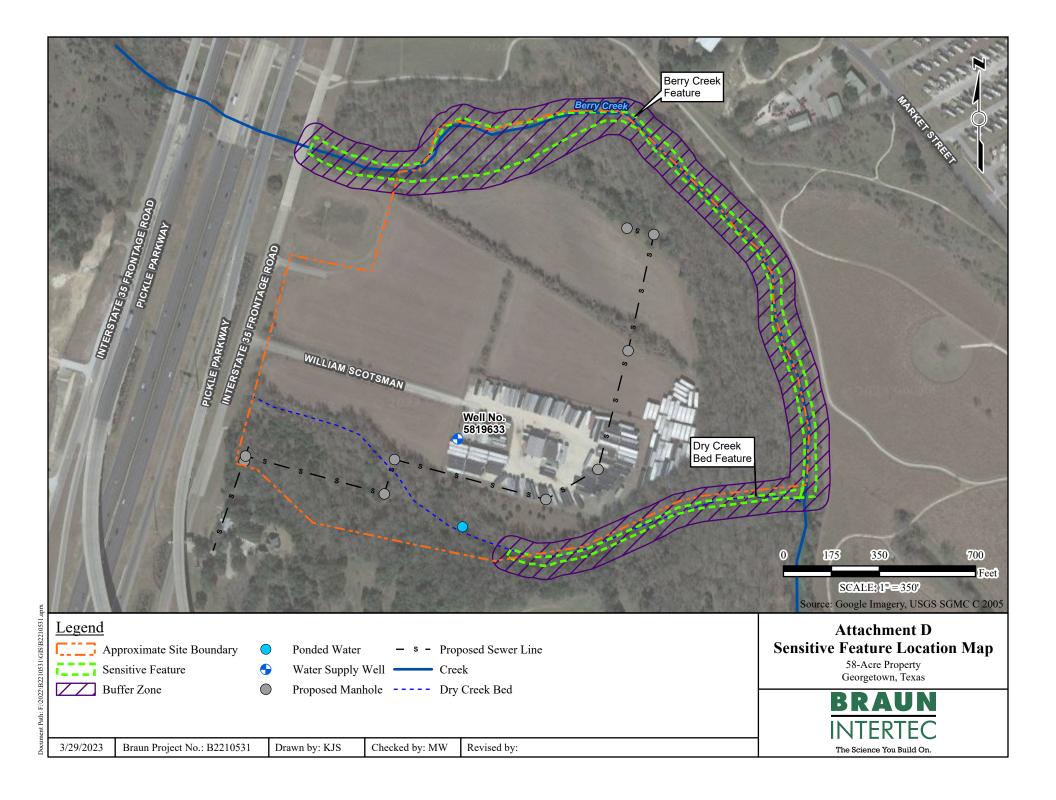












Water Pollution Abatement Plan Application

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Water Pollution Abatement Plan Application Form** is hereby submitted for TCEQ review and Executive Director approval. The form was prepared by:

Print Name of Customer/Agent: <u>Hollis Scheffler, P.E.</u>

Date: <u>12/20/2022</u>

Signature of Customer/Agent:

Regulated Entity Name: Berry Creek Business Park

Regulated Entity Information

1. The type of project is:

Residential: Number of Lots:____
Residential: Number of Living Unit Equivalents:____
Commercial
Industrial
Other:____

- 2. Total site acreage (size of property):56.73
- 3. Estimated projected population:n/a
- 4. The amount and type of impervious cover expected after construction are shown below:

Table 1 - Impervious Cover Table

Impervious Cover of Proposed Project	Sq. Ft.	Sq. Ft./Acre	Acres					
Structures/Rooftops	518,100	÷ 43,560 =	11.89					
Parking	368,750	÷ 43,560 =	8.47					
Other paved surfaces	520542	÷ 43,560 =	7.76					
Total Impervious Cover	1,407,420	÷ 43,560 =	28.12					

Total Impervious Cover $\underline{28.12}$ ÷ Total Acreage $\underline{56.73}$ X 100 = $\underline{49.57}$ % Impervious Cover

5.	Attachment A - Factors Affecting Surface Water Quality. A detailed description of all
	factors that could affect surface water and groundwater quality that addresses ultimate
	land use is attached.

6. Only inert materials as defined by 30 TAC §330.2 will be used as fill material.

For Road Projects Only

Complete questions 7 - 12 if this application is exclusively for a road project.

7.	Type of project:
	 TXDOT road project. County road or roads built to county specifications. City thoroughfare or roads to be dedicated to a municipality. Street or road providing access to private driveways.
8.	Type of pavement or road surface to be used:
	Concrete Asphaltic concrete pavement Other:
9.	Length of Right of Way (R.O.W.): feet.
	Width of R.O.W.: feet. $L \times W = Ft^2 \div 43,560 Ft^2/Acre = acres.$
10.	Length of pavement area: feet.
	Width of pavement area: feet. L x W = $Ft^2 \div 43,560 Ft^2/Acre = acres$. Pavement area acres \div R.O.W. area acres x $100 = \%$ impervious cover.
11.	A rest stop will be included in this project.
	A rest stop will not be included in this project.

TCEQ Executive Director. Modificat	roadways that do not require approval from the tions to existing roadways such as widening ore than one-half (1/2) the width of one (1) existing the TCEQ.
Stormwater to be generat	ed by the Proposed Project
volume (quantity) and character (quantity) occur from the proposed project is quality and quantity are based on t	cter of Stormwater. A detailed description of the quality) of the stormwater runoff which is expected to attached. The estimates of stormwater runoff the area and type of impervious cover. Include the the pre-construction and post-construction conditions
Wastewater to be generat	red by the Proposed Project
14. The character and volume of wastewa	ter is shown below:
100% Domestic% Industrial% Commingled TOTAL gallons/day	138,001 Gallons/day Gallons/day Gallons/day
15. Wastewater will be disposed of by:	
On-Site Sewage Facility (OSSF/Sept	ic Tank):
will be used to treat and dispossing authority's (authorize the land is suitable for the use the requirements for on-site second Each lot in this project/develop size. The system will be design	er from Authorized Agent. An on-site sewage facility se of the wastewater from this site. The appropriate d agent) written approval is attached. It states that of private sewage facilities and will meet or exceed ewage facilities as specified under 30 TAC Chapter 28 lities. Inment is at least one (1) acre (43,560 square feet) in ed by a licensed professional engineer or registered ensed installer in compliance with 30 TAC Chapter
Sewage Collection System (Sewer L	ines):
to an existing SCS.	e wastewater generating facilities will be connected e wastewater generating facilities will be connected
☐ The SCS was previously submitted☐ The SCS was submitted with the☐ The SCS will be submitted at a look be installed prior to Executive I	is application. ater date. The owner is aware that the SCS may not

The sewage collection system will convey the wastewater to the <u>Pecan Branch WWTP</u> (name) Treatment Plant. The treatment facility is:
☑ Existing.☐ Proposed.
16. All private service laterals will be inspected as required in 30 TAC §213.5.
Site Plan Requirements
Items 17 – 28 must be included on the Site Plan.
17. \square The Site Plan must have a minimum scale of 1" = 400'.
Site Plan Scale: $1'' = 40'$.
18. 100-year floodplain boundaries:
 Some part(s) of the project site is located within the 100-year floodplain. The floodplais shown and labeled. No part of the project site is located within the 100-year floodplain. The 100-year floodplain boundaries are based on the following specific (including date of material) sources(s): FEMA Maps 48491C0285F and 48491C0292F (dated 12/20/2019)
19. The layout of the development is shown with existing and finished contours at appropriate, but not greater than ten-foot contour intervals. Lots, recreation centers, buildings, roads, open space, etc. are shown on the plan.
The layout of the development is shown with existing contours at appropriate, but not greater than ten-foot intervals. Finished topographic contours will not differ from the existing topographic configuration and are not shown. Lots, recreation centers, buildings, roads, open space, etc. are shown on the site plan.
20. All known wells (oil, water, unplugged, capped and/or abandoned, test holes, etc.):
\boxtimes There are <u>1</u> (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply)
 ☐ The wells are not in use and have been properly abandoned. ☐ The wells are not in use and will be properly abandoned. ☐ The wells are in use and comply with 16 TAC §76.
There are no wells or test holes of any kind known to exist on the project site.
21. Geologic or manmade features which are on the site:
 ✓ All sensitive geologic or manmade features identified in the Geologic Assessment a shown and labeled. ✓ No sensitive geologic or manmade features were identified in the Geologic Assessment.
Attachment D - Exception to the Required Geologic Assessment. A request and justification for an exception to a portion of the Geologic Assessment is attached.

22. 🔀	The drainage patterns and approximate slopes anticipated after major grading activities
23. 🔀	Areas of soil disturbance and areas which will not be disturbed.
24. 🔀	Locations of major structural and nonstructural controls. These are the temporary and permanent best management practices.
25. 🔀	Locations where soil stabilization practices are expected to occur.
26. 🗌	Surface waters (including wetlands).
\boxtimes	N/A
27.	Locations where stormwater discharges to surface water or sensitive features are to occur.
\boxtimes	There will be no discharges to surface water or sensitive features.
28. 🔀	Legal boundaries of the site are shown.
Adn	ninistrative Information
29. 🔀	Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
30. 🔀	Any modification of this WPAP will require Executive Director approval, prior to construction, and may require submission of a revised application, with appropriate fees.

Attachment A – Factors Affecting Surface Water Quality

The following are potential sources of surface and groundwater contamination from construction activities:

- Clearing and grubbing
- Grading and site excavation
- Vehicle tracking
- Topsoil stripping and stockpiling
- Landscaping operations
- Staging and storage area
- Paving (including curb and gutter)
- Building Construction
- Concrete washout area



Attachment B - Volume and Character of Stormwater

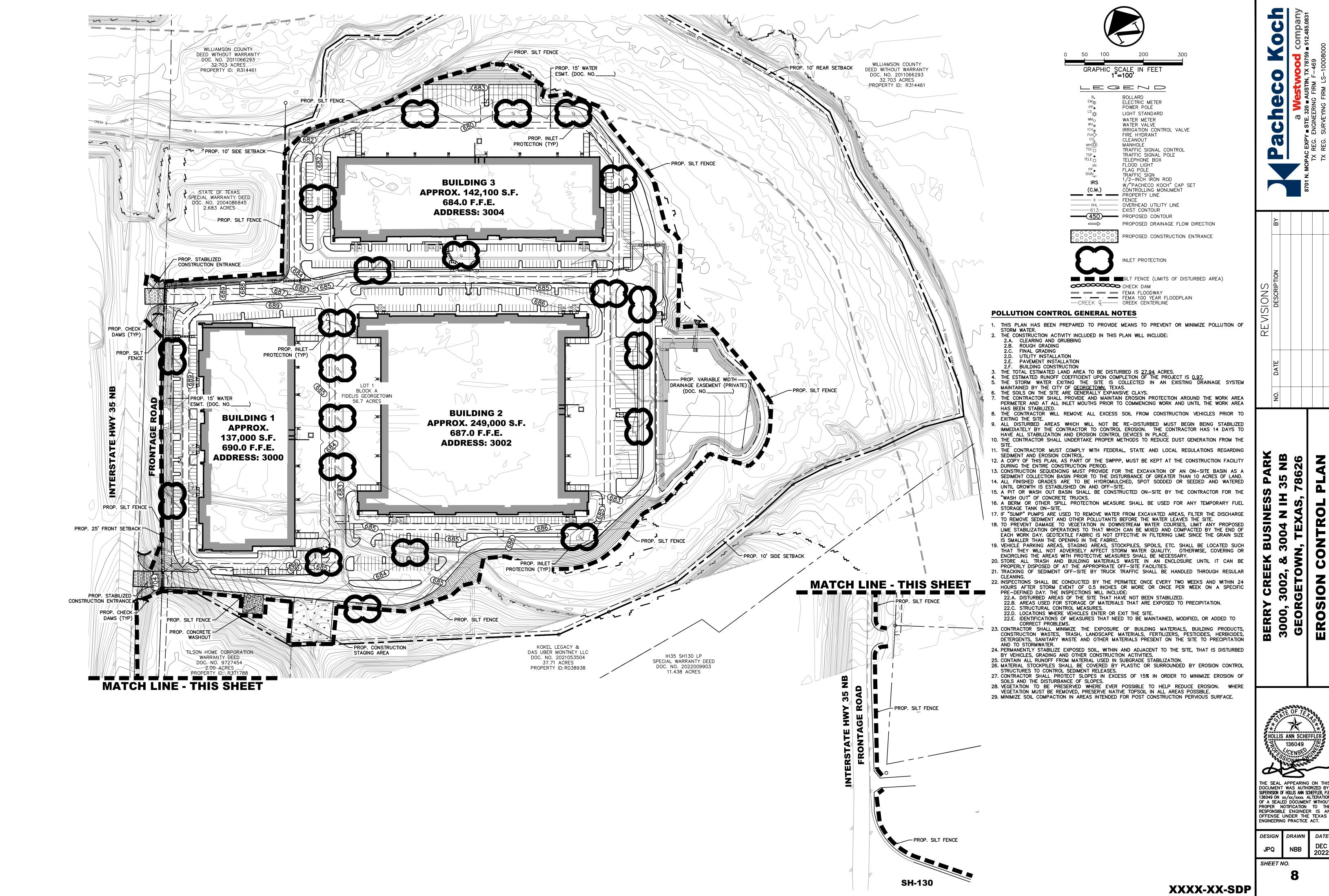
The existing site generates approximately 152.0 cfs flowing over 30.33 acres primarily from the northwest to southeast over 75% grass cover at roughly 1.0% slope. The runoff coefficient utilized for the existing site is 0.40.

The proposed development generates an approximate 304.61 cfs and has a required TSS removal of 85%. The runoff from the site is generated from the streets, building roof, driveways, parking, and other paved and impervious surfaces. The runoff coefficient utilized for the proposed site is 0.97. Flow is directed from the previously listed impervious structures and sent into catch basins to then be piped into the proposed batch detention pond, WQ Pond A.



Site Plan





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DEC 2022

DRAWN

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Temporary Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: <u>Hollis Scheffler, P.E.</u>

Date: <u>12/20/22</u>

Signature of Customer/Agent:

Regulated Entity Name: Berry Creek Business Park

Project Information

Potential Sources of Contamination

Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.

1.	Fuels for construction equipment and hazardous substances which will be used during construction:
	The following fuels and/or hazardous substances will be stored on the site:
	These fuels and/or hazardous substances will be stored in:
	Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

	 Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year. Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
	Evels and hazardous substances will not be stored on the site.
2.	Attachment A - Spill Response Actions. A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
3.	Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
4.	Attachment B - Potential Sources of Contamination. A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.
Se	equence of Construction
5.	Attachment C - Sequence of Major Activities. A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
	 For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given. For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
6.	Name the receiving water(s) at or near the site which will be disturbed or which will

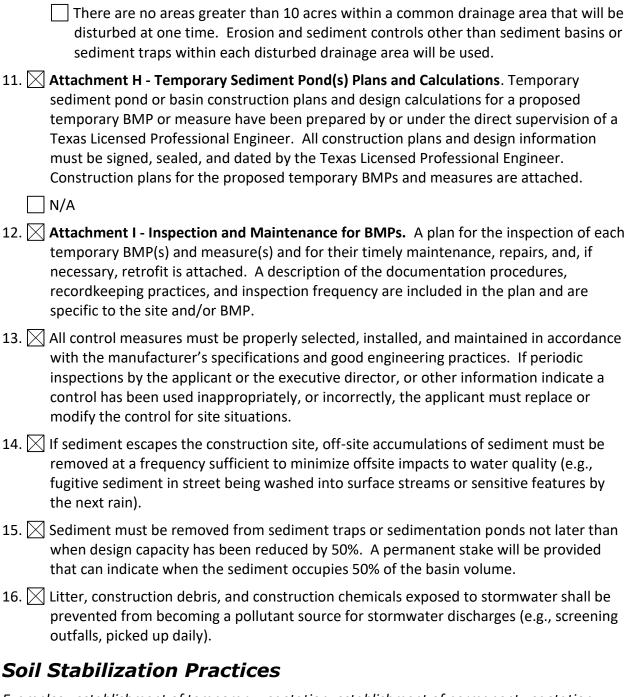
Temporary Best Management Practices (TBMPs)

receive discharges from disturbed areas of the project: Berry Creek

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

7. Attachment D – Temporary Best Management Practices and Measures. TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

	A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.
	A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.
	A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer.
	A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.
8. 🔀	The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
	Attachment E - Request to Temporarily Seal a Feature. A request to temporarily seal a feature is attached. The request includes justification as to why no reasonable and practicable alternative exists for each feature.
	There will be no temporary sealing of naturally-occurring sensitive features on the site.
9.	Attachment F - Structural Practices. A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.
10.	Attachment G - Drainage Area Map. A drainage area map supporting the following requirements is attached:
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided.
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used.
	For areas that will have more than 10 acres within a common drainage area
	disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area.
	There are no areas greater than 10 acres within a common drainage area that will be
	disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.



Examples: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

17. Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices. A schedule of the interim and permanent soil stabilization practices for the site is attached.

- 18. Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 19. Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

Administrative Information

- 20. All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
- 21. If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
- 22. Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

Attachment A – Spill Response Actions

In accordance with the Edwards Aquifer Technical Guidance on Best Management Practices Operators, the following actions will be followed to ensure appropriate measures are taken in the case of a spill:

Education

- Be aware that different materials pollute in different amounts. Make sure that each employee knows what a "significant spill" is for each material they use, and what is the appropriate response for "significant" and "insignificant" spills. Employees should also be aware of when spill must be reported to the TCEQ. Information available in 30 TAC 327.4 and 40 CFR 302.4.
- Educate employees and subcontractors on potential dangers to humans and the environment from spills and leaks.
- Hold regular meetings to discuss and reinforce appropriate disposal procedures (incorporate into regular safety meetings).
- Establish a continuing education program to indoctrinate new employees.
- Have contractor's superintendent or representative oversee and enforce proper spill prevention and control measures.

General Measures

- To the extent that the work can be accomplished safely, spills of oil, petroleum products, substances listed under 40 CFR parts 110,117, and 302, and sanitary and septic wastes should be contained and cleaned up immediately.
- Store hazardous materials and wastes in covered containers and protect from vandalism.
- Place a stockpile of spill cleanup materials where it will be readily accessible.
- Train employees in spill prevention and cleanup.
- Designate responsible individuals to oversee and enforce control measures.
- Spills should be covered and protected from stormwater run on during rainfall to the extent that it doesn't compromise cleanup activities.
- Do not bury or wash spills with water.
- Store and dispose of used clean up materials, contaminated materials, and recovered spill material that is no longer suitable for the intended purpose in conformance with the provisions in applicable BMPs.
- Do not allow water used for cleaning and decontamination to enter storm drains or watercourses. Collect and dispose of contaminated water in accordance with applicable regulations.
- Contain water overflow or minor water spillage and do not allow it to discharge into drainage facilities or watercourses.
- Place Material Safety Data Sheets (MSDS), as well as proper storage, cleanup, and spill reporting instructions for hazardous materials stored or used on the project site in an open, conspicuous, and accessible location.
- Keep waste storage areas clean, well-organized, and equipped with ample cleanup supplies as appropriate for the materials being stored. Perimeter controls, containment structures, covers, and liners should be repaired or replaced as needed to maintain proper function.



Cleanup

- Clean up leaks and spills immediately.
- Use a rag for small spills on paved surfaces, a damp mop for general cleanup, and absorbent material for larger spills. If the spilled material is hazardous, then the used cleanup materials are also hazardous and must be disposed of as hazardous waste.
- Never hose down or bury dry material spills. Clean up as much of the material as possible and dispose of properly. See the waste management BMPs in this section for specific information.

Minor Spills

- Minor spills typically involve small quantities of oil, gasoline, paint, etc. which can be controlled by the first responder at the discovery of the spill.
- Use absorbent materials on small spills rather than hosing down or burying the spill.
- Absorbent materials should be promptly removed and disposed of properly.
- Follow the practice below for a minor spill:
- Contain the spread of the spill.
- Recover spilled materials.
- Clean the contaminated area and properly dispose of contaminated materials.

Semi-Significant Spills

- Contain spread of the spill.
- Notify the project foreman immediately.
- If the spill occurs on paved or impermeable surfaces, clean up using "dry" methods (absorbent materials, cat litter and/or rags). Contain the spill by encircling with absorbent materials and do not let the spill spread widely.
- If the spill occurs in dirt areas, immediately contain the spill by constructing an earthen dike. Dig up and properly dispose of contaminated soil.
- If the spill occurs during rain, cover spill with tarps or other material to prevent contaminating runoff.

Significant/Hazardous Spills

- Notify the TCEQ by telephone as soon as possible and within 24 hours at 512-339-2929 (Austin) or 210-490-3096 (San Antonio) between 8 AM and 5 PM. After hours, contact the Environmental Release Hotline at 1-800-832-8224. It is the contractor's responsibility to have all emergency phone numbers at the construction site.
- For spills of federal reportable quantities, in conformance with the requirements in 40 CFR parts 110,119, and 302, the contractor should notify the National Response Center at (800) 424-8802
- Notification should first be made by telephone and followed up with a written report.
- The services of a spills contractor or a Haz-Mat team should be obtained immediately.
 Construction personnel should not attempt to clean up until the appropriate and qualified staffs have arrived at the job site.
- Other agencies which may need to be consulted include, but are not limited to, the City Police Department, County Sheriff Office, Fire Departments, etc.



Spills, Discharges, and Releases

- Report an environmental emergency, discharge, spill, or air release. Links to rules, law, technical assistance, waste management, State Emergency Response Commission.
- Please contact TCEQ emergencies for reportable quantities using the link below: https://www.tceq.texas.gov/response/spills/spill rq.html

To report and environmental emergency, discharge, spill, or air release, control:

<u>State</u>

- State of Texas Spill-Reporting Hotline and the SERC: 1-800-832-8224 --- 24 hours a day
- TCEQ Regional Office, Monday-Friday, 8 a.m. 5 p.m.

Federal

 National Response Center: 1-800-424-8802 (notifying the NRC does not constitute to the state)



Attachment B - Potential Sources of Contamination

The following are potential sources of surface and groundwater contamination from construction activities:

- Clearing and grubbing
- Grading and site excavation
- Vehicle tracking
- Topsoil stripping and stockpiling
- Landscaping operations
- Staging and storage area
- Paving (including curb and gutter)
- Building Construction
- Concrete washout area



Attachment C - Sequence of Major Activities

The following sequence of construction is included in the construction plans:

- 1. Temporary erosion controls, silt fencing and tree protection fencing to be installed. Estimated area disturbed =56.73 ac
 - Estimated timing = 1 week
- 2. Pre-construction meeting to be held on-site.

Estimated area disturbed = n/a ac

Estimated timing = 1 day

3. Demolition of existing materials.

Estimated area disturbed = 56.73 ac

Estimated timing = 6 weeks

4. Site staking and rough grading.

Estimated area disturbed = 56.73 ac

Estimated timing = 6 weeks

5. Storm sewers to be installed.

Estimated area disturbed = 56.73 ac

Estimated timing = 8 weeks

6. Water, wastewater and paving improvements to begin.

Estimated area disturbed = 56.73 ac

Estimated timing = 8 weeks

7. Temporary erosion control measures to be inspected on a regular basis; any sediment buildup to be removed.

Estimated area disturbed = n/a

Estimated timing = 1 week

8. Site to be cleaned up and revegetated.

Estimated area disturbed = 56.73 ac

Estimated timing = 6 weeks

9. Temporary erosion controls to be removed after permanent restoration of site is established.

Estimated area disturbed = n/a

Estimated timing = 1 week



Attachment D – Temporary Best Management Practices and Measures

The following temporary best management practices will be conducted to prevent pollution of surface water, groundwater, and stormwater in accordance with the Edwards Aquifer Technical Guidance on Best Management Practices.

<u>Temporary Vegetation</u>

Vegetation will be used as a temporary stabilization technique for areas disturbed by construction, but not covered by pavement, buildings, or other structures. As a temporary control, vegetation will be used to stabilize stockpiles and barren areas that are inactive for long periods of time.

Dust Control

Dust control will prevent blowing and movement of dust from exposed soil surfaces, reduce on and off-site damage, health hazards and improve traffic safety. This practice is applicable to areas subject to dust blowing and movement where on and off-site damage is likely without treatment.

Temporary Construction Entrance/Exit

The temporary gravel construction entrance will provide a stable entrance/exit condition from the construction site and keep mud and sediment off public roads. A stabilized construction entrance is a stabilized pad of crushed stone located at any point traffic will be entering or leaving the construction site from a public right-of way, street, alley, sidewalk or parking area. The stabilized construction entrance will reduce or eliminate the tracking or flowing of sediment onto public rights of-way. This practice should be used at all points of construction ingress and egress.

<u>Silt Fence</u>

A silt fence is a barrier consisting of geotextile fabric supported by metal posts to prevent soil and sediment loss from a site. Proposed silt fences will be highly effective at controlling sediment from disturbed areas. They cause runoff to pond, allowing heavier solids to settle out.

Inlet Protection

All proposed inlets that may receive storm runoff from disturbed areas should be protected. Temporary inlet protection is a series of different measures that provide protection against silt transport or accumulation in storm sewer systems. This clogging can greatly reduce or completely stop the flow in the pipes. The different measures are used for different site conditions and inlet types. Filter barrier protection using silt fence is appropriate when the drainage area is less than one acre, and the basin slope is less than five percent. This type of protection is not applicable in paved areas. Block and gravel protection is used when flows exceed 0.5 cubic feet per second, and it is necessary to allow for overtopping to prevent flooding. This form of protection is also useful for curb type inlets as it works well in paved areas. Wire mesh and gravel protection is used when flows exceed 0.5 cubic feet per second and construction traffic may occur over the inlet. This form of protection may be used with both curb and drop inlets.



Concrete Washout Area

The purpose of concrete washout areas is to prevent or reduce the discharge of pollutants to stormwater from concrete waste by conducting washout offsite, performing onsite washout in a designated area, and training employees and subcontractors.

The following steps will help reduce stormwater pollution from concrete wastes: • Incorporate requirements for concrete waste management into material supplier and subcontractor agreements.

- Avoid mixing excess amounts of fresh concrete.
- Perform washout of concrete trucks in designated areas only.
- Do not wash out concrete trucks into storm drains, open ditches, streets, or streams.
- Do not allow excess concrete to be dumped onsite, except in designated areas.

For onsite washout:

- Locate washout area at least 50 feet from sensitive features, storm drains, open ditches, or water bodies. Do not allow runoff from this area by constructing a temporary pit or bermed area large enough for liquid and solid waste.
- Wash out wastes into the temporary pit where the concrete can set, be broken up, and then disposed properly.



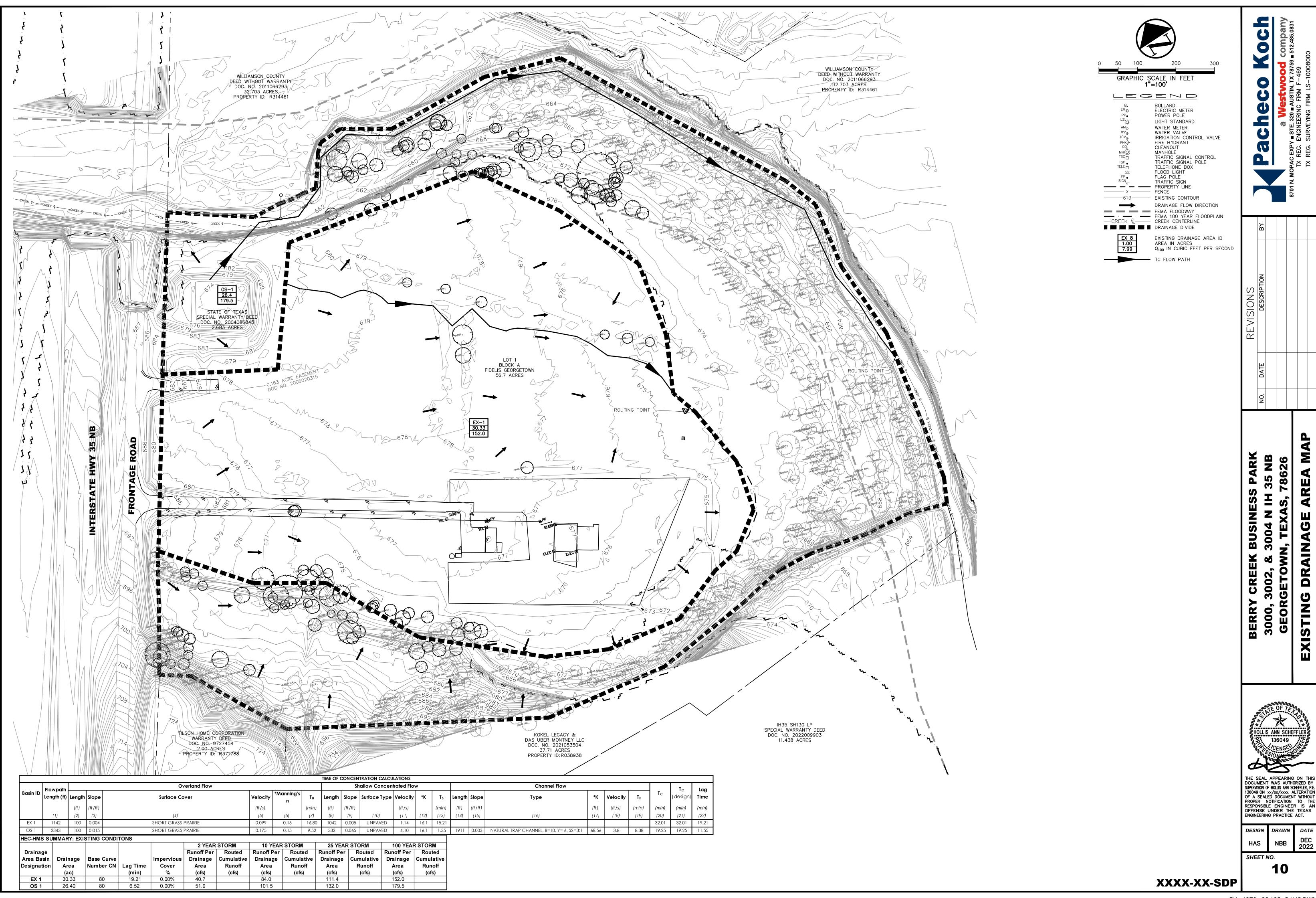
Attachment F - Structural Practices

Stormwater will be routed through the proposed silt fence and inlet protection for pollutant removal. The proposed permanent BMPs are to be constructed as to intercept stormwater flowing from the parking lots, streets, building roofs, and other impervious areas. The silt fence will provide temporary sedimentation control during construction prior to the permanent BMPs being finalized. No part of the site or placement of the structural practices will be encumbered by floodplain as shown on FEMA #48491C0285F and 48491C0292F.

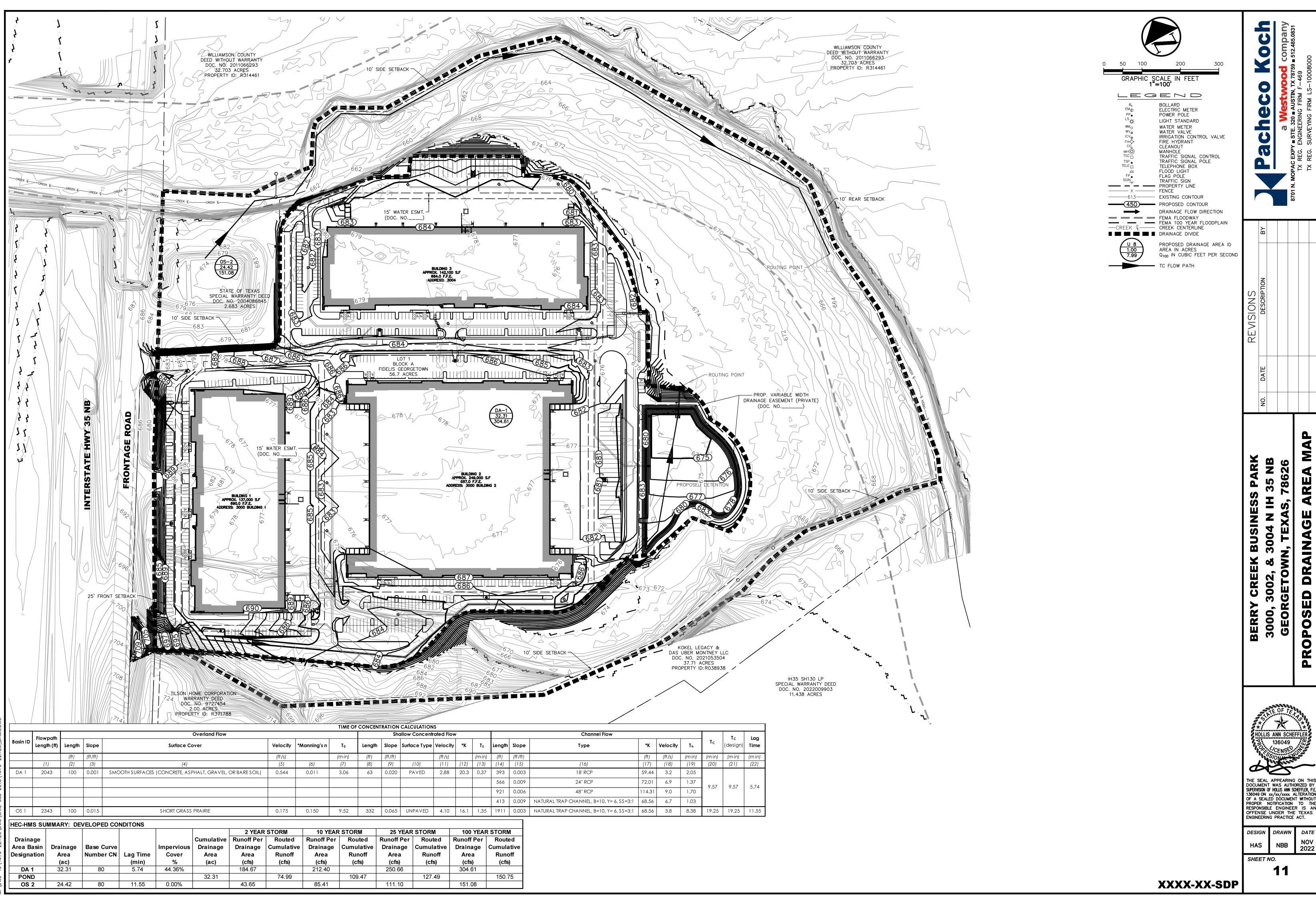


Attachment G – Drainage Area Map





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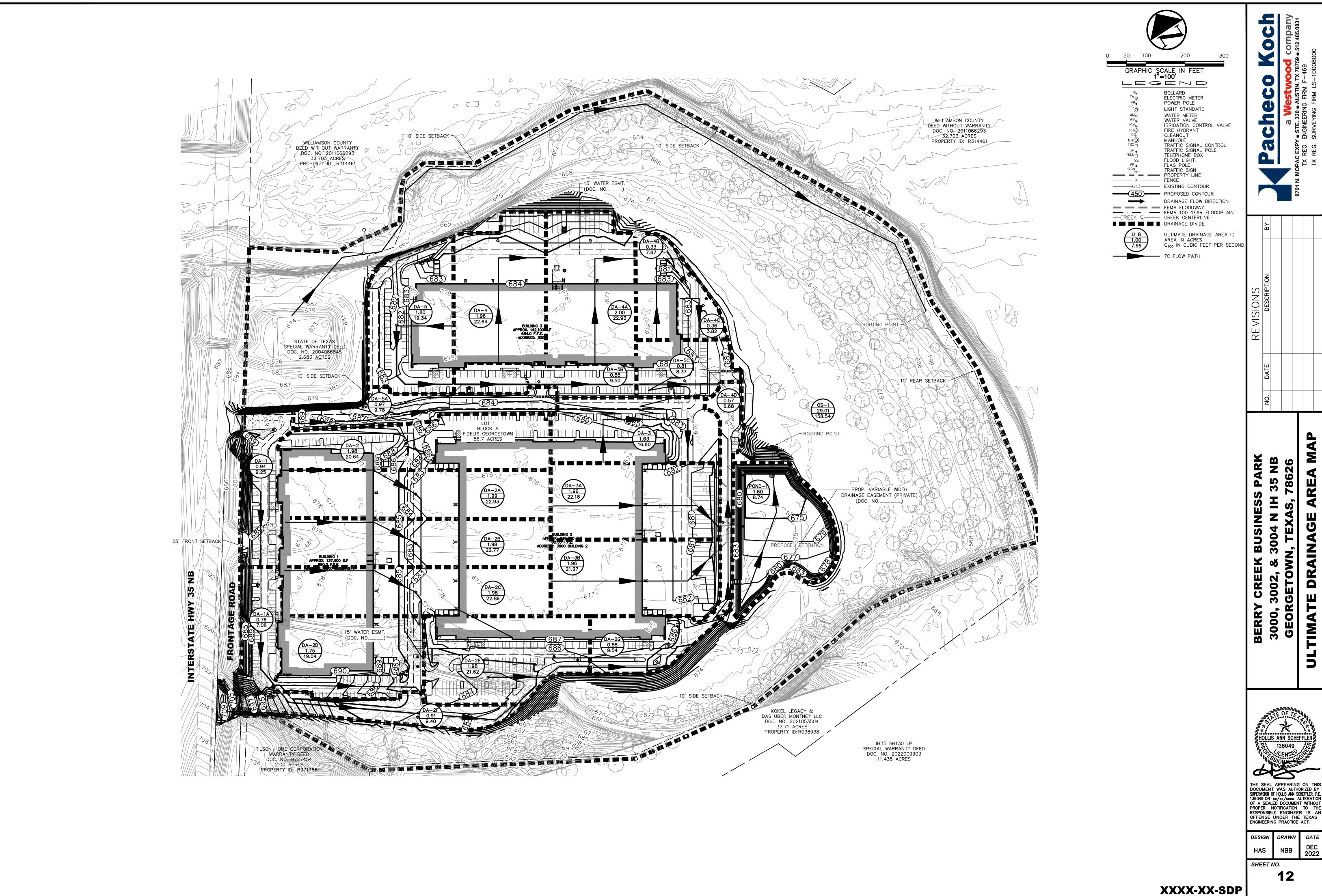
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DEC 2022

AREA

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ULTIMATE

GEORGETOWN,

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2022

SHEET NO.
13

	MASTER I	DRAINAGI					2 year					10 year				25 year						100 year							
Drainage Number	AREA	IC	PC	%ІС	pe	rv C	im	p C	weighted C	рє	rv C	im	p C	weighted C	pei	v C	im	p C	weighted C		perv C		imp) C	weighted (
ULTIMATE CONDITION	N DRAINAGE	AREAS																						 	+				
DA 1	0.94	0.694	0.2477	73.71%	0.31	0.08	0.97	0.67	0.80	0.36	0.09	0.97	0.67	0.81	0.39	0.10	0.97	0.67	0.82	0.46	6 0).11	0.97	0.67	0.84				
DA1A	0.76	0.516	0.2492	67.42%	0.31	0.08	0.97	0.50	0.75	0.36	0.09	0.97	0.50	0.77	0.39	0.10	0.97	0.50	0.78	0.40	0).11	0.97	0.50	0.80				
DA 2	1.98	1.782	0.2010	89.86%	0.31	0.06	0.97	1.73	0.90	0.36	0.07	0.97	1.73	0.91	0.39	0.08	0.97	1.73	0.91	0.46	0	0.09	0.97	1.73	0.92				
DA 2A	1.99	1.990	0.0000	100.00%	0.31	0.00	0.97	1.93	0.97	0.36	0.00	0.97	1.93	0.97	0.39	0.00	0.97	1.93	0.97	0.40	6 0	0.00	0.97	1.93	0.97				
DA 2B	1.98	1.976	0.0000	100.00%	0.31	0.00	0.97	1.92	0.97	0.36	0.00	0.97	1.92	0.97	0.39	0.00	0.97	1.92	0.97	0.40	6 0	0.00	0.97	1.92	0.97				
DA 2C	1.98	1.984	0.0000	100.00%	0.31	0.00	0.97	1.92	0.97	0.36	0.00	0.97	1.92	0.97	0.39	0.00	0.97	1.92	0.97	0.40	6 0	0.00	0.97	1.92	0.97				
DA 2D	1.75	1.638	0.1155	93.41%	0.31	0.04	0.97	1.59	0.93	0.36	0.04	0.97	1.59	0.93	0.39	0.05	0.97	1.59	0.93	0.40	6 0	0.05	0.97	1.59	0.94				
DA 2E	1.98	1.782	0.1979	90.00%	0.31	0.06	0.97	1.73	0.90	0.36	0.07	0.97	1.73	0.91	0.39	0.08	0.97	1.73	0.91	0.40	6 0	0.09	0.97	1.73	0.92				
DA 2F	0.91	0.728	0.1838	79.85%	0.31	0.06	0.97	0.71	0.84	0.36	0.07	0.97	0.71	0.85	0.39	0.07	0.97	0.71	0.85	0.40	6 0	0.08	0.97	0.71	0.87				
DA 2G	0.88	0.780	0.1006	88.58%	0.31	0.03	0.97	0.76	0.89	0.36	0.04	0.97	0.76	0.90	0.39	0.04	0.97	0.76	0.90	0.40	6 0	0.05	0.97	0.76	0.91				
DA 3	1.63	1.269	0.3605	77.88%	0.31	0.11	0.97	1.23	0.82	0.36	0.13	0.97	1.23	0.84	0.39	0.14	0.97	1.23	0.84	0.40	6 0).17	0.97	1.23	0.86				
DA 3A	1.96	1.893	0.0683	96.52%	0.31	0.02	0.97	1.84	0.95	0.36	0.02	0.97	1.84	0.95	0.39	0.03	0.97	1.84	0.95	0.46	6 0	0.03	0.97	1.84	0.95				
DA 3B	1.96	1.856	0.1071	94.54%	0.31	0.03	0.97	1.80	0.93	0.36	0.04	0.97	1.80	0.94	0.39	0.04	0.97	1.80	0.94	0.40	6 0	0.05	0.97	1.80	0.94				
DA 4	1.96	1.964	0.0000	100.00%	0.31	0.00	0.97	1.91	0.97	0.36	0.00	0.97	1.91	0.97	0.39	0.00	0.97	1.91	0.97	0.46	6 0	0.00	0.97	1.91	0.97				
DA 4A	2.00	1.983	0.0138	99.31%	0.31	0.00	0.97	1.92	0.97	0.36	0.00	0.97	1.92	0.97	0.39	0.01	0.97	1.92	0.97	0.40	6 0).01	0.97	1.92	0.97				
DA 4B	0.68	0.649	0.0338	95.04%	0.31	0.01	0.97	0.63	0.94	0.36	0.01	0.97	0.63	0.94	0.39	0.01	0.97	0.63	0.94	0.40	6 0	0.02	0.97	0.63	0.94				
DA 4C	0.36	0.305	0.0561	84.45%	0.31	0.02	0.97	0.30	0.87	0.36	0.02	0.97	0.30	0.88	0.39	0.02	0.97	0.30	0.88	0.46	6 0	0.03	0.97	0.30	0.89				
DA 4D	0.57	0.557	0.0148	97.41%	1.31	0.02	0.00	0.00	0.03	1.36	0.02	0.97	0.54	0.98	1.39	0.02	0.97	0.54	0.98	1.46	6 0	0.02	0.97	0.54	0.98				
DA 5	1.80	1.572	0.2237	87.54%	0.31	0.07	0.97	1.52	0.89	0.36	0.08	0.97	1.52	0.89	0.39	0.09	0.97	1.52	0.90	0.46	6 0).10	0.97	1.52	0.91				
DA 5A	0.97	0.741	0.2268	76.57%	0.31	0.07	0.97	0.72	0.82	0.36	0.08	0.97	0.72	0.83	0.39	0.09	0.97	0.72	0.83	0.46).10	0.97	0.72	0.85				
DA 5B	0.85	0.802	0.0453	94.65%	0.31	0.01	0.97	0.78	0.93	0.36	0.02	0.97	0.78	0.94	0.39	0.02	0.97	0.78	0.94	0.46	0	0.02	0.97	0.78	0.94				
DA 5C	0.81	0.654	0.1530	81.04%	0.31	0.05	0.97	0.63	0.84	0.36	0.06	0.97	0.63	0.85	0.39	0.06	0.97	0.63	0.86	0.46			0.97	0.63	0.87				
POND-1	1.60	0.000	1.5996	0.00%	0.31	0.50	0.97	0.00	0.31	0.36	0.58	0.97	0.00	0.36	0.39	0.62	0.97	0.00	0.39	0.46			0.97	0.00	0.46				
OS-1	29.01	0.000	29.0100	0.00%	0.31	8.99	0.97	0.00	0.31	0.36	10.44	0.97	0.00	0.36	0.39	11.31	0.97	0.00	0.39	0.40	6 <u>13</u>	3.34	0.97	0.00	0.46				

									SHEET FLOV				<u> </u>	SH	ALLOW CONCE	NTRATED FLO	W		1		CHANN	IEL FLOW					INTE	NSITY		<u> </u>	DISC	HARGE	
DRAINAGE	INLET	AREA	C ₂	C ₁₀	C ₂₅	C ₁₀₀	Length	Slope	Surface Cover	Velocity	Manning's	T _{sheet}	Length	Slope	Surface Type	Velocity	K	T _{shallow}	Length	Slope	Туре	K (f†)	Velocity	T _{channel}	Tc (min)	l 2yr	l 10yr	l 25yr	l 100yr	Q 2	Q 10	Q 25	Q 100
NUMBER	NUMBER	(acres)					(ft)	(ft /ft)		(ft /s)	Manning's n	(min)	(ft)	(ft /ft)		(ft /s)		(min)	(ft)	(ft /ft)		$K = \frac{1.486 \cdot R^{\frac{2}{3}}}{n}$	(ft/s)	(min)		(in/hr)	(in/hr)	(in/hr)	(in/hr)	(cfs)	(cfs)	(cfs)	(cfs)
DA 1	DA 1	0.94	0.80	0.81	0.82	0.84	100.00	0.001	CONCRETE	0.40	0.02	4.13	63.13	0.020	PAVED	2.88	20.33	0.36	225.66	0.006	18" RCP	59.44	4.42	0.85	5.35	6.39	8.50	9.72	11.74	4.79	6.48	7.49	9.25
DA1A	DATA	0.76	0.75	0.77	0.78	0.80	100.00	0.001	CONCRETE	0.38	0.02	4.40	55.58	0.002	PAVED	0.98	20.33	0.94	168.43	0.004	24" RCP	72.01	4.51	0.62	5.96	6.23	8.31	9.52	11.51	3.60	4.90	5.69	7.08
DA 2	DA 2	1.98	0.90	0.91	0.91	0.92	100.00	0.001	CONCRETE	0.35	0.02	4.73	259.10	0.017	PAVED	2.65	20.33	1.63	60.67	0.030	18" RCP	59.44	10.38	0.10	6.46	6.11	8.16	9.36	11.33	10.94	14.71	16.92	20.64
DA 2A	DA 2A	1.99	0.97	0.97	0.97	0.97	100.00	0.065	CONCRETE	1.87	0.02	0.89	25.28	0.061	PAVED	5.00	20.33	0.08	68.74	0.218	24" RCP	72.01	33.64	0.03	5.00	6.48	8.62	9.84	11.88	12.51	16.63	18.99	22.93
DA 2B	DA 2B	1.98	0.97	0.97	0.97	0.97	100.00	0.065	CONCRETE	1.87	0.02	0.89	25.28	0.061	PAVED	5.00	20.33	0.08	68.74	0.218	24" RCP	72.01	33.64	0.03	5.00	6.48	8.62	9.84	11.88	12.42	16.51	18.86	22.77
DA 2C	DA 2C	1.98	0.97	0.97	0.97	0.97	100.00	0.065	CONCRETE	1.87	0.02	0.89	25.28	0.061	PAVED	5.00	20.33	0.08	68.74	0.218	30" RCP	83.56	39.03	0.03	5.00	6.48	8.62	9.84	11.88	12.47	16.58	18.93	22.86
DA 2D	DA 2D	1.75	0.93	0.93	0.93	0.94	100.00	0.002	CONCRETE	0.44	0.02	3.83	229.75	0.024	PAVED	3.13	20.33	1.22	56.03	0.001	18" RCP	59.44	1.38	0.68	5.73	6.29	8.38	9.59	11.60	10.21	13.67	15.67	19.04
DA 2E	DA 2E	1.98	0.90	0.91	0.91	0.92	100.00	0.004	CONCRETE	0.63	0.02	2.64	172.57	0.015	PAVED	2.46	20.33	1.17	270.58	0.011	48" RCP	114.31	11.83	0.38	5.00	6.48	8.62	9.84	11.88	11.60	15.51	17.77	21.62
DA 2F	DA 2F	0.91	0.84	0.85	0.85	0.87	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	288.00	0.010	24" RCP	72.01	7.20	0.67	5.00	6.48	8.62	9.84	11.88	4.95	6.66	7.65	9.40
DA 2G	DA 2G	0.88	0.89	0.90	0.90	0.91	50.00	0.010	CONCRETE	0.77	0.02	1.08	100.00	0.010	PAVED	2.03	20.33	0.82	100.00	0.011	48" RCP	114.31	11.99	0.14	5.00	6.48	8.62	9.84	11.88	5.11	6.83	7.83	9.54
DA 3	DA 3	1.63	0.82	0.84	0.84	0.86	100.00	0.010	CONCRETE	0.88	0.02	1.88	217.00	0.010	PAVED	2.03	20.33	1.78	107.00	0.010	24" RCP	72.01	7.20	0.25	5.00	6.48	8.62	9.84	11.88	8.70	11.73	13.50	16.60
DA 3A	DA 3A	1.96	0.95	0.95	0.95	0.95	100.00	0.010	CONCRETE	0.88	0.02	1.88	150.00	0.010	PAVED	2.03	20.33	1.23	201.00	0.010	24" RCP	72.01	7.20	0.47	5.00	6.48	8.62	9.84	11.88	12.03	16.03	18.33	22.18
DA 3B	DA 3B	1.96	0.93	0.94	0.94	0.94	100.00	0.010	CONCRETE	0.88	0.02	1.88	200.00	0.010	PAVED	2.03	20.33	1.64	149.00	0.010	18" RCP	59.44	5.94	0.42	5.00	6.48	8.62	9.84	11.88	11.88	15.84	18.12	21.97
DA 4	DA 4	1.96	0.97	0.97	0.97	0.97	50.00	0.010	CONCRETE	0.77	0.02	1.08	150.00	0.010	PAVED	2.03	20.33	1.23	189.00	0.010	18" RCP	59.44	5.94	0.53	5.00	6.48	8.62	9.84	11.88	12.34	16.41	18.75	22.64
DA 4A	DA 4A	2.00	0.97	0.97	0.97	0.97	50.00	0.010	CONCRETE	0.77	0.02	1.08	150.00	0.010	PAVED	2.03	20.33	1.23	1/3.00	0.010	24" RCP	/2.01	7.20	0.40	5.00	6.48	8.62	9.84	11.88	12.49	16.61	18.98	22.93
DA 4B	DA 4B	0.68	0.94	0.94	0.94	0.94	50.00	0.010	CONCRETE	0.77	0.02	1.08	150.00	0.010	PAVED	2.03	20.33	1.23	108.00	0.010	24" RCP	/2.01	7.20	0.25	5.00	6.48	8.62	9.84	11.88	4.15	5.53	6.33	7.67
DA 4C	DA 4C	0.36	0.87	0.88	0.88	0.89	50.00	0.010	CONCRETE	0.77	0.02	1.08	50.00	0.010	PAVED	2.03	20.33	0.41	50.00	0.010	30" RCP	83.56	8.36	0.10	5.00	6.48	8.62	9.84	11.88	2.03	2.72	3.13	3.82
DA 4D	DA 4D	0.57	0.03	0.95	0.98	0.98	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	240.00	0.010	42" RCP	104.57	10.46	0.38	5.00	6.48	8.62	9.84	11.88	0.13	4.68	5.52	6.68
DA 5	DA 5	1.80	0.89	0.89	0.90	0.91	50.00	0.010	CONCRETE	0.77	0.02	1.08	100.00	0.010	PAVED	2.03	20.33	0.82	280.00	0.010	18" RCP	59.44	5.94	0.79	5.00	6.48	8.62	9.84	11.88	10.33	13.83	15.86	19.34
DA 5A	DA 5A	0.97	0.82	0.83	0.83	0.85	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	1/5.00	0.010	18" RCP	59.44	5.94	0.49	5.00	6.48	8.62	9.84	11.88	5.12	6.90	7.95	9.78
DA 5B	DA 50	0.85	0.93	0.94	0.94	0.94	50.00	0.010	CONCRETE CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.04	54.00	0.010	30" RCP	83.56	8.36	0.42	5.00	6.48	8.62	9.84	11.00	5.13	6.85	7.83	9.50
DA 5C	DA 3C	1.60	0.84	0.83	0.86	0.87	50.00	0.010		0.77	0.02	1.08	100.00	0.010	PAVED	2.03	20.33	0.82	0.4.00	0.010	1011 5 05	83.36	0.36	0.11	5.00	6.48	8.62	7.01	11.00	4.42	5.94	4.1.4	8.37
POND-1	POND-1	1.60 29.01	0.31	0.36	0.39	0.46	50.00	0.010	CONCRETE CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.04	94.00 288.00	0.010	P CHANNEL, B=	114.31	11.43	0.14	5.00	6.48 6.48	8.62	9.84	11.88	3.21 58.27	4.96	6.14	8./4 158.54
OS-1	U3-1	29.01	0.51	0.30	0.37	J 0.40	50.00	0.010	CONCREIE	0.77	0.02	1.08	200.00	0.010	PAVED	∠.∪ა	20.33	1.04	288.00	1 0.010	I CHAMMEL, B	/7./	/.7/	0.00	3.00	0.40	0.02	7.04	11.00	JO.2/	89.98	111.32 '	100.04

GEORGETOWN IDF CURVE TABLE

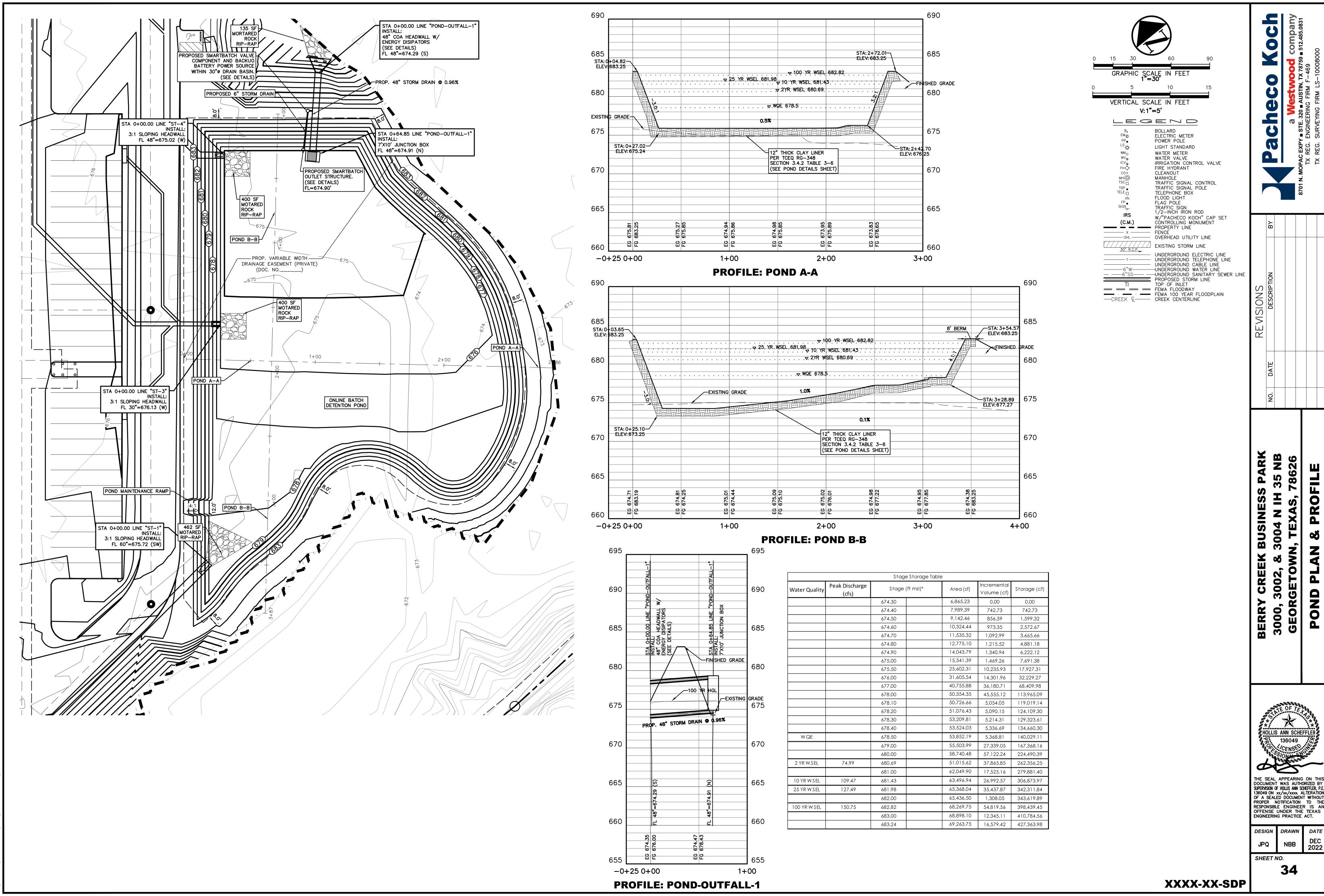
TABLE 3-3 INTENSITY-DURATION-FREQUENCY EQUATION COEFFICIENTS²

Storm Frequency	а	b	С				
2-year	106.29	16.81	0.9076				
5-year	99.75	16.74	0.8327				
10-year	96.84	15.88	0.7952				
25-year	111.07	17.23	0.7815				
50-year	119.51	17.32	0.7705				
100-year	129.03	17.83	0.7625				
500-year	160.57	19.64	0.7449				

Attachment H – Temporary Sediment Pond Plans and Calculations

A rough-cut water quality batch detention pond will be utilized for the temporary sedimentation removal on-site and is to be graded in accordance with the following plan sheet provided. Revegetation or placement of underdrain piping shall not be carried out until the site construction phase is complete.





PK-4670-22.125_POND.DWG

Attachment I – Inspection and Maintenance for BMPs

The following inspection and maintenance guidelines for the temporary best management practices will be followed in accordance with the Edwards Aquifer Technical Guidance on Best Management Practices. Inspections of the Temporary BMPs will be documented in an inspection report. Inspection reports will document maintenance activities, sediment removal and modifications to the sediment and erosion controls.

Temporary Vegetation

- 1. Temporary vegetation should be inspected weekly and after each rain event to locate and repair any erosion.
- 2. Erosion from storms or other damage should be repaired as soon as practical by regrading the area and applying new seed.
- 3. If the vegetated cover is less than 80%, the area should be reseeded.

Dust Control

1. When dust is evident during dry weather, reapply dust control BMPs.

Temporary Construction Entrance/Exit

- 1. Inspect all fencing weekly, and after any rainfall.
- 2. Remove sediment when buildup reaches 6 inches.
- 3. Replace any torn fabric or install a second line of fencing parallel to the torn section. (4) Replace or repair any sections crushed or collapsed in the course of construction activity. If a section of fence is obstructing vehicular access, consider relocating it to a spot where it will provide equal protection, but will not obstruct vehicles. A triangular filter dike may be preferable to a silt fence at common vehicle access points.
- 4. When construction is complete, the sediment should be disposed of in a manner that will not cause additional siltation and the prior location of the silt fence should be revegetated. The fence itself should be disposed of in an approved landfill.

Silt Fence

- 1. Inspect all fencing weekly, and after any rainfall.
- 2. Remove sediment when buildup reaches 6 inches.
- 3. Replace any torn fabric or install a second line of fencing parallel to the torn section.
- 4. Replace or repair any sections crushed or collapsed in the course of construction activity. If a section of fence is obstructing vehicular access, consider relocating it to a spot where it will provide equal protection, but will not obstruct vehicles. A triangular filter dike may be preferable to a silt fence at common vehicle access points.
- 5. When construction is complete, the sediment should be disposed of in a manner that will not cause additional siltation and the prior location of the silt fence should be revegetated. The fence itself should be disposed of in an approved landfill.

Inlet Protection

- 1. Inspection should be made weekly and after each rainfall. Repair or replacement should be made promptly as needed by the contractor.
- 2. Remove sediment when buildup reaches a depth of 3 inches. Removed sediment should be deposited in a suitable area and in such a manner that it will not erode.
- 3. Check placement of device to prevent gaps between device and curb.
- 4. Inspect filter fabric and patch or replace if torn or missing.
- 5. Structures should be removed and the area stabilized only after the remaining drainage area has been properly stabilized.



Concrete Washout Area

When temporary concrete washout facilities are no longer required for the work, the hardened concrete should be removed and disposed of. Materials used to construct temporary concrete washout facilities should be removed from the site of the work and disposed of. Holes, depressions or other ground disturbance caused by the removal of the temporary concrete washout facilities should be backfilled and repaired.



Company Na	me:									_		
Company Ad	dress:_									_		
City/State/Zip:								_				
Phone:	Phone:								_			
Engineer:										_		
Engineers Ado	dress:_									_		
City/State/Zip	:									_		
Phone:										_		
Property Own	er:									_		
Batch Detenti	ion Po	nd										
Monitoring / I	Mainte	enance	e Table									
	Jan	Feb	Mar	Apr	May	Jun	July	Aug	Sep	Oct	Nov	Dec
Structural Repairs and Replacement					,		,		·			
Seasonal Mowing and Lawn Care (in)												
Debris and Litter Removal												
Sediment Removal												
Logic Controller												
Erosion Control												
Nuisance Control												
Completed By												
Date												
I hereby certif completed in maintenance	acco											was

(Signed by property owner or designee)



Attachment J – Schedule of Interim and Permanent Soil Stabilization Practices

Seeding of the disturbed areas will be on-going after completion of the rough grading process. Temporary seeding will be utilized until permanent landscaping is installed. Seeding will occur on any areas that are undisturbed for a period of 14 days. If construction progress is stopped for a period of 14 days, soil stabilization practices must be initiated by the contractor. Permanent landscaping will be provided as soon as final grades are achieved and the final paving and building operations are completed. Bare soils should be seeded or otherwise stabilized within 14 calendar days after final grading or where construction activity has temporarily ceased for more than 21 days.



Permanent Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(C), (D)(Ii), (E), and (5), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Permanent Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: <u>Hollis Scheffler, P.E.</u>

Date: <u>12/20/2022</u>

Signature of Customer/Agent

Regulated Entity Name: Berry Creek Business Park

Permanent Best Management Practices (BMPs)

Permanent best management practices and measures that will be used during and after construction is completed.

1.	Permanent BMPs and measures must be implemented to control the discharge of pollution from regulated activities after the completion of construction.
	□ N/A
2.	These practices and measures have been designed, and will be constructed, operated, and maintained to insure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity is removed. These quantities have been calculated in accordance with technical guidance prepared or accepted by the executive director.
	The TCEQ Technical Guidance Manual (TGM) was used to design permanent BMPs and measures for this site.

	A technical guidance other than the TCEQ TGM was used to design permanent BMPs and measures for this site. The complete citation for the technical guidance that was used is:
	□ N/A
3.	Owners must insure that permanent BMPs and measures are constructed and function as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the appropriate regional office within 30 days of site completion.
	□ N/A
4.	Where a site is used for low density single-family residential development and has 20 % or less impervious cover, other permanent BMPs are not required. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
	 □ The site will be used for low density single-family residential development and has 20% or less impervious cover. □ The site will be used for low density single-family residential development but has more than 20% impervious cover. □ The site will not be used for low density single-family residential development.
5.	The executive director may waive the requirement for other permanent BMPs for multifamily residential developments, schools, or small business sites where 20% or less impervious cover is used at the site. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
	 □ Attachment A - 20% or Less Impervious Cover Waiver. The site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is attached. □ The site will be used for multi-family residential developments, schools, or small business sites but has more than 20% impervious cover. □ The site will not be used for multi-family residential developments, schools, or small
6.	business sites. Attachment B - BMPs for Upgradient Stormwater.

	 A description of the BMPs and measures that will be used to prevent poll surface water, groundwater, or stormwater that originates upgradient from and flows across the site is attached. No surface water, groundwater or stormwater originates upgradient from and flows across the site, and an explanation is attached. ✓ Permanent BMPs or measures are not required to prevent pollution of surface, groundwater, or stormwater that originates upgradient from the stormwater, groundwater, and an explanation is attached. 	om the site I the site rface
7.	Attachment C - BMPs for On-site Stormwater.	
	A description of the BMPs and measures that will be used to prevent poll surface water or groundwater that originates on-site or flows off the site pollution caused by contaminated stormwater runoff from the site is atta Permanent BMPs or measures are not required to prevent pollution of su or groundwater that originates on-site or flows off the site, including poll caused by contaminated stormwater runoff, and an explanation is attach	including ched. rface water ution
8.	Attachment D - BMPs for Surface Streams. A description of the BMPs and m that prevent pollutants from entering surface streams, sensitive features, or is attached. Each feature identified in the Geologic Assessment as sensitive haddressed.	the aquifer
	□ N/A	
9.	The applicant understands that to the extent practicable, BMPs and measure maintain flow to naturally occurring sensitive features identified in either the assessment, executive director review, or during excavation, blasting, or con-	geologic
	 The permanent sealing of or diversion of flow from a naturally-occurring feature that accepts recharge to the Edwards Aquifer as a permanent polabatement measure has not been proposed. Attachment E - Request to Seal Features. A request to seal a naturally-o sensitive feature, that includes, for each feature, a justification as to why reasonable and practicable alternative exists, is attached. 	lution
10.	Attachment F - Construction Plans. All construction plans and design calcula the proposed permanent BMP(s) and measures have been prepared by or ur direct supervision of a Texas Licensed Professional Engineer, and are signed, dated. The plans are attached and, if applicable include:	der the
	 ✓ Design calculations (TSS removal calculations) ✓ TCEQ construction notes ✓ All geologic features ✓ All proposed structural BMP(s) plans and specifications 	
	N/A	

ir	Attachment G - Inspection, Maintenance, Repair and Retrofit Plan. A plan for the nspection, maintenance, repairs, and, if necessary, retrofit of the permanent BMPs and neasures is attached. The plan includes all of the following:
	Prepared and certified by the engineer designing the permanent BMPs and measures
	✓ Signed by the owner or responsible party✓ Procedures for documenting inspections, maintenance, repairs, and, if necessary retrofit
	A discussion of record keeping procedures
	I/A
r	Attachment H - Pilot-Scale Field Testing Plan . Pilot studies for BMPs that are not ecognized by the Executive Director require prior approval from the TCEQ. A plan for bilot-scale field testing is attached.
\boxtimes N	I/A
o a a c b	Attachment I -Measures for Minimizing Surface Stream Contamination. A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity, which increase erosion that results in water quality degradation.
\boxtimes N	I/A
Resp	onsibility for Maintenance of Permanent BMP(s)
-	ibility for maintenance of best management practices and measures after tion is complete.
u e o o r	The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be esponsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
	N/A
a n	A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development, or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.
\boxtimes N	I/A

Attachment B – BMPs for Upgradient Stormwater

There will be no upstream surface waters running onto the site and will not be treated with the proposed pond. The proposed batch detention basin and all associated ESC practices are designed for the subject site. The proposed primary batch detention WQ Pond will be used to receive onsite flows from stormwater coming from the proposed site.



Attachment C - BMPs for On-site Stormwater

The Berry Creek Business Park Entity is proposing one primary batch detention basin based on 56.730 acres of contributing area, encompassing 49.57% of impervious cover across the site. The stormwater is diverted through impervious structures and piped into a batch detention basin. The batch detention basin has adequate water quality storage to account for the proposed development and acts as the primary treatment for TSS removal.



Attachment D - BMPs for Surface Streams

The Berry Creek Business Park Entity is proposing one primary batch detention basin based on 56.730 acres of contributing area, encompassing 49.57% of impervious cover across the site. The stormwater is diverted through impervious structures and piped into a batch detention basin. The batch detention basin has adequate water quality storage to account for the proposed development and acts as the primary treatment for TSS removal. The aforementioned BMP will provide adequate measures to prevent pollutant removal from entering the aquifer. No surface streams or sensitive features are located on the site.



Attachment F - Construction Plans



DEVELOPER



FIDELIS REALTY PARTNERS STEVEN KIMOSH 8140 WALNUT HILL LANE SUITE 400 SKIMOSH@FRPLTD.COM

ARCHITECT

WWW.FRPLTD.COM



GSR ANDRADE ARCHITECTS IGNACIO ANGUIANO 4121 COMMERCE STREET, #1 DALLAS, TX, 75226 PHONE: 214.824.7040 IANGUIANO@GSR-ANDRADE.COM WWW.GSR-ANDRADE.COM

ENGINEER

1 Pacheco Koch |

HOLLIS SCHEFFLER, P.E. 8701 N. MOPAC EXPY. SUITE 320 AUSTIN, TEXAS 78759 PHONE: 512.485.0831 HOLLIS.SCHEFFLER@WESTWOODPS.COM WWW.PKCE.COM

LANDSCAPE ARCHITECT

1⊿Pacheco Koch

AMBER DAVIS, LPA 7557 RAMBLER RD, SUITE 1400 DALLAS, TEXAS 75231 PHONE: 872.235.3031 AMBER.DAVIS@WESTWOODPS.COM

LEGAL DESCRIPTION:

LOT 1, BLOCK A, FIDELIS, GEORGETOWN

FLOODPLAIN INFORMATION:

PER FEMA FIRM PANEL NO. 48491C0285F & 48491C0292F, DATED DECEMBER 20, 2019, THE PROPOSED DEVELOPMENT IS IN THE AREA OF SPECIAL FLOOD HAZARD, ZONE A, AND IS IN THE 100 YEAR FLOODPLAIN THERE ARE SPRINGS. STREAMS OR BUFFER ZONES LOCATED ON THE SUBJECT SITE.

PROPOSED USE:

WAREHOUSING AND DISTRIBUTION. GENERAL

ACREAGE: 56.73 AC (2,471,138 SF)

AVERAGE DAILY TRIPS:

DAILY: 754 VPD AM PEAK: 92 (43 IN / 33 OUT) VPH PM PEAK: 54 (15 IN / 39 OUT) VPH

TOTAL IMPERVIOUS COVER: 0 ACRES (00.0%)

PROPOSED:

PEC.COOP

FIRE DEPARTMENT: **GEORGETOWN FIRE DEPARTMENT** 3500 DB WOOD RD. **GEORGETOWN, TEXAS 78628** (512) 930-3473

ELECTRICITY, WATER & WASTEWATER:

PREPARED BY

Pacheco Koch

T: 512.485.0831

27.94 ACRES (54.19%)

GEORGETOWN UTILITY SYSTEMS 300-1 INDUSTRIAL AVENUE GEORGETOWN, TX 78626 (512) 930-3555 **GUS.GEORGETOWN.ORG**

PEDERNALES ELECTRIC COOPERATIVE, INC. 201 S. AVENUE F **JOHNSON CITY, TEXAS 78636** 830-868-7155

8701 N. MOPAC EXPY, SUITE 320

TX REG. ENGINEERING FIRM F-469

TX REG. SURVEYING FIRM LS-10008000

AUSTIN, TX 78759

SITE DEVELOPMENT PERMIT PLANS **FOR**

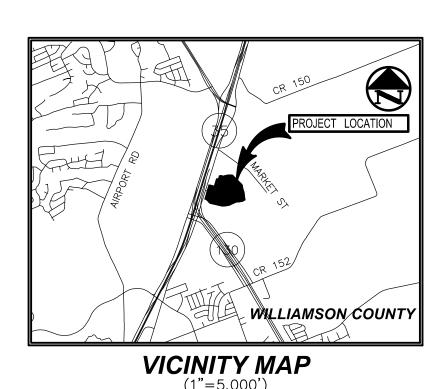
BERRY CREEK BUSINESS PARK 3000, 3002, & 3004 N-IH 35 NB GEORGETOWN, TEXAS, 78626

GENERAL NOTES:

REVISIONS/CORRECTIONS

- IT IS THE RESPONSIBILITY OF THE PROPERTY OWNER, AND SUCCESSORS TO THE CURREN

- SIDEWALKS SHALL BE PROVIDED IN ACCORDANCE WITH THE UDC.
- OUTDOOR LIGHTING SHALL COMPLY WITH SECTION 7.04 OF THE UDC.
- SCREENING OF MECHANICAL EQUIPMENT, DUMPSTERS AND PARKING SHALL COMPLY WITH CHAPTER 8 TO MEET ALL REQUIREMENTS OF THE UDC.
- ALL MAINTENANCE OF REQUIRED LANDSCAPE SHALL COMPLY WITH THE MAINTENANCE STANDARDS OF CHAPTER 8 OF THE UDC.
- 11. A SEPARATE IRRIGATION PLAN SHALL BE REQUIRED AT THE TIME OF BUILDING PERMIT APPLICATION
- 12. FIRE FLOW REQUIREMENTS OF 2,000 GPM ARE BEING MET BY THIS PLAN. 13. ANY HERITAGE TREE NOTED ON THIS SITE DEVELOPMENT PLAN IS SUBJECT, IN PERPETUITY, TO THE
- MAINTENANCE, CARE, PRUNING AND REMOVAL REQUIREMENTS OF THE UNIFIED DEVELOPMENT CODE. 14. THE CONSTRUCTION PORTION OF THESE PLANS WERE PREPARED, SEALED, SIGNED AND DATED BY A TEXAS LICENSED PROFESSIONAL ENGINEER. THEREFORE, BASED ON THE ENGINEER'S CONCURRENCE OF COMPLIANCE, THE CONSTRUCTION PLANS FOR CONSTRUCTION OF THE PROPOSED PROJECT ARE HEREBY APPROVED SUBJECT TO THE STANDARD CONSTRUCTION SPECIFICATIONS AND DETAILS MANUAL AND ALL OTHER APPLICABLE CITY, STATE AND FEDERAL REQUIREMENTS AND CODES.
- 15. THIS PROJECT IS SUBJECT TO ALL CITY STANDARD CONSTRUCTION SPECIFICATIONS AND DETAILS IN EFFECT AT THE TIME OF SUBMITTAL OF THE PROJECT TO THE CITY.
- 16. WHERE NO EXISTING OVERHEAD INFRASTRUCTURE EXISTS, UNDERGROUND ELECTRIC UTILITY LINES SHALL BE LOCATED ALONG THE STREET AND WITHIN THE SITE. WHERE EXISTING OVERHEAD INFRASTRUCTURE IS TO BE RELOCATED, IT SHALL BE RE-INSTALLED UNDERGROUND AND THE EXISTING FACILITIES SHALL BE REMOVED AT THE DISCRETION OF THE DEVELOPMENT ENGINEER.
- 17. ALL ELECTRIC AND COMMUNICATION INFRASTRUCTURE SHALL COMPLY WITH UDC SECTION 13.06. 18. THE PROPERTY SUBJECT TO THIS APPLICATION IS SUBJECT TO THE WATER QUALITY REGULATIONS OF THE CITY OF GEORGETOWN
- 19. A GEOLOGIC ASSESSMENT, IN ACCORDANCE WITH THE CITY OF GEORGETOWN WATER QUALITY REGULATIONS, WAS COMPLETED ON 11/04/2022. ANY SPRINGS AND STREAMS AS IDENTIFIED IN THE GEOLOGIC ASSESSMENT ARE SHOWN HEREIN.
- 20. SCREENING AND LOCATION OF OUTDOOR STORAGE SHALL COMPLY WITH SECTION 5.09 OF THE UDC.



PROJECT ZONING:

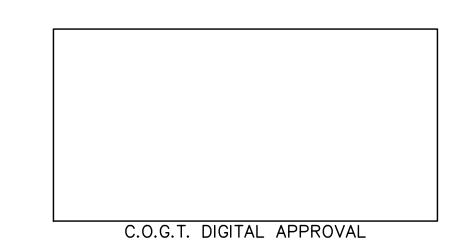
BUSINESS PARK (PUD)

PROJECT ADDRESS:

3000, 3002, 3004 N IH 35 NB

SUBMITTAL DATE:

DECEMBER 19, 2022



FINAL PLAT SHEET 3 OF 4 FINAL PLAT SHEET 4 OF 4 GENERAL NOTES EXIST. CONDITIONS & DEMO PLAN **EROSION CONTROL PLAN** EROSION CONTROL DETAILS & NOTES PROPOSED DRAINAGE AREA MAP ULTIMATE DRAINAGE AREA MAP ULTIMATE DRAINAGE AREA CALCULATIONS IMENSIONAL CONTROL PLAN SHEET 2 OF 4 DECEL LANE PLAN SHEET OVERALL GRADING PLAN GRADING PLAN SHEET 1 OF 4 GRADING PLAN SHEET 2 OF 4 GRADING PLAN SHEET 3 OF 4 GRADING PLAN SHEET 4 OF 4 UTILITY PROFILE SHEET 1 OF 6 UTILITY PROFILE SHEET 6 OF 6 UTILITY DETAIL SHEET 1 OF 3 UTILITY DETAIL SHEET 2 OF 3 UTILITY DETAIL SHEET 3 OF 3 PAVING PLAN PAVING DETAILS ELECTRICAL SERVICE PLAN OVERALL TREE MITIGATION PLAN L1.05 TREE MITIGATION PLAN TREE MITIGATION PLAN GENERAL NOTES & SCHEDULES OVERALL PLANTING PLAN PLANTING PLAN PLANTING PLAN PLANTING PLAN PLANTING PLAN PLANTING DETAILS BUILDING A ELEVATION BUILDING B ELEVATION BUILDING C ELEVATION **BUILDING A PHOTOMETRICS**

BUILDING B PHOTOMETRICS

BUILDING C PHOTOMETRICS

DRAWING SHEET INDEX

DESCRIPTION

FINAL PLAT SHEET 1 OF 4

FINAL PLAT SHEET 2 OF 4

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HOLLIS ANN SCHEFFLI 136049

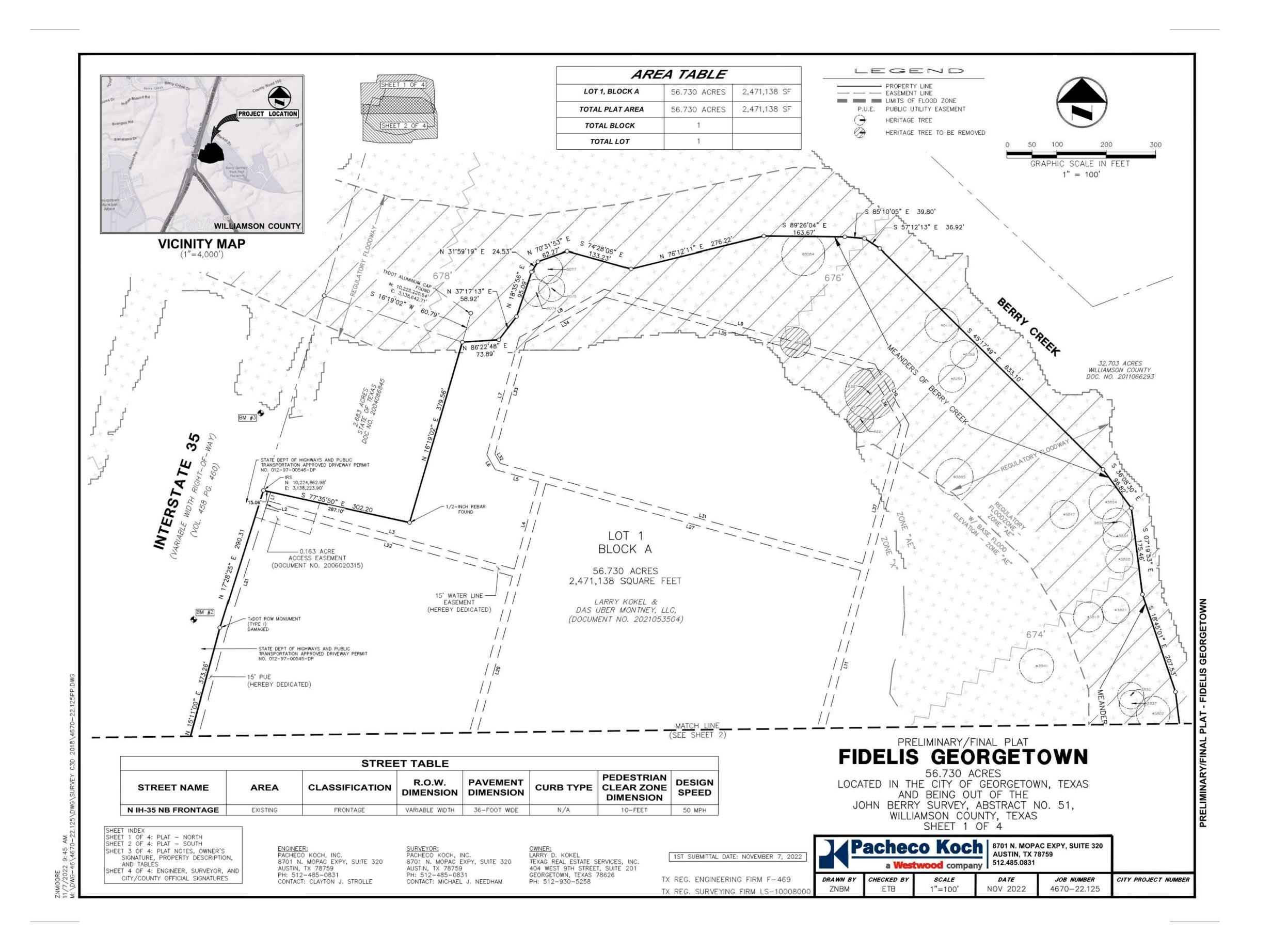
136049 ON xx/xx/xxxx ALTERATIO OF A SEALED DOCUMENT WITHOU PROPER NOTIFICATION TO THE RESPONSIBLE ENGINEER IS A OFFENSE UNDER THE TEXAS ENGINEERING PRACTICE ACT.

DATE **DESIGN** DRAWN DEC 2022 JPQ NBB SHEET NO.

NET CHANGE TOTAL SITE REVISE (R) SHEETS IN TO IMP. COVER IMP. COVER GEORGETOWN IMAGED ADD (A) DESCRIPTION PLAN SET APPROVAL/DATE (sq. ft.) (%) (sq. ft.) VOID (V) SHEET NO.S

BENCHMARK LIST

BM #2 - "X" CUT IN SOUTHEASTERLY EDGE OF CONCRETE OF THE NORTHBOUND IH-35 FRONTAGE ROAD, ±23.5' NORTHWEST OF A MAILBOX. E: 3,138,083.91' ELEV: 688.94' BM #3 - " \boxtimes " CUT IN A CONCRETE MOW STRIP BEHIND A GUARDRAIL AT THE SOUTH END OF A BRIDGE ON THE NORTHBOUND IH-35 FRONTAGE ROAD, ±156.0' NORTH OF THE WESTERLY-MOST NORTHWEST PROPERTY CORNER OF E: 3,138,218.52' ELEV: 686.78'



BY

a Westwood company

8701 N. MOPAC EXPY = STE. 320 = AUSTIN, TX 78759 = 512.485.0831

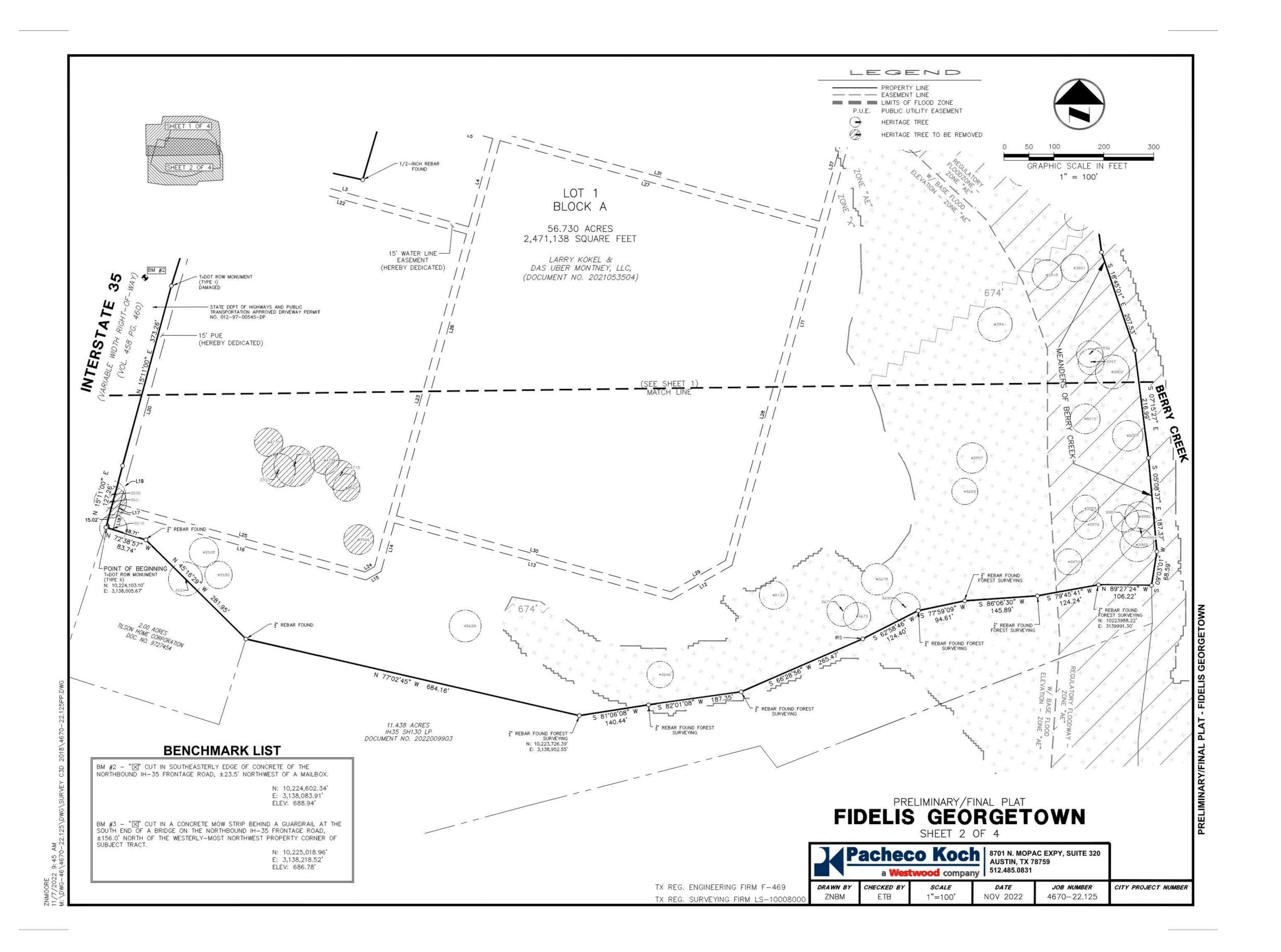
TX REG. ENGINEERING FIRM F-469

TX REG. SURVEYING FIRM LS-10008000

	ВҮ			
REVISIONS	DESCRIPTION			
	DATE			
	NO.			

BERRY CREEK BUSINESS PARK 3000, 3002, & 3004 N IH 35 NB GEORGETOWN, TEXAS, 78626 FINAL PLAT SHEET 1 OF 4

DESIGNDRAWNDATEJPQNBBDEC
2022



REVISIONS DESCRIPT

NO. DATE				
	2, & 3004 N IH 35 NB	OWN. TEXAS. 78626	AT SHEET 2 OF 4	

BERRY CREEK BUSINESS P 3000, 3002, & 3004 N IH 3 GEORGETOWN, TEXAS, 78

DRAWN DATE JPQ NBB

GENERAL NOTES

- 1. Bearing system for this survey is based on the State Plane Coordinate System, North American Datum of 1983 (2011), Texas Central Zone 4203. Distances and areas reported have been scaled by applying the surface adjustment factor of 1.0001200000. To obtain grid values, apply a scale factor of 0.999880014398.
- 2. Coordinates shown hereon are grid values.
- 3. The water, wastewater/septic, and electric utility provider for this development is the City of Georgetown.
- All structures/obstructions are prohibited in drainage easements.
- 5. There are areas within the boundaries of this subdivision in the 100-year floodplain as defined by FIRM Map Number 48491C0285F, effective date of December 19, 2019 and 48491C0292, effective date of December 20, 2019
- 6. In order to promote drainage away from a structure, the slab elevation should be built at least one-foot above the surrounding ground, and the ground should be graded away from the structure at a slope of 1/2" per foot for a distance of at least 10 feet.
- 7. The monuments of this plat have been rotated to the NAD 83/93 HARN Texas Central Zone and NAVD 88.
- 8. All sedimentation, filtration, detention, and/or retention basins and related appurtenances shown shall be situated within a drainage easement or drainage lot. The owners or assignees of the tracts upon which are located such easements, appurtenances, and detention facilities shall maintain same and be responsible for their maintenance, routine inspection, and upkeep.
- 9. The landowner assumes all risks associated with improvements located in the right-of-way, or road widening easements. By placing anything in the right-of-way or road widening easements, the landowner indemnifies and holds the City of Georgetown, Williamson County, their officers, agents and employees harmless from any liability owing to property defects or negligence not attributable to them and acknowledges that the improvements may be removed by the City and/or County and that the owner of the improvements will be responsible for the relocation and/or replacement of the improvements.
- 10. The building of all streets, roads, and other public thoroughfares and any bridges or culverts necessary to be constructed or placed is the responsibility of the owners of the tract of land covered by this plat in accordance with the plans and specifications prescribed by the City of Georgetown and/or Williamson County, Texas. Neither the City of Georgetown nor Williamson County assumes any obligation to build any of the streets, roads, or other public thoroughfares shown on this plat or of constructing any of the bridges or drainage improvements in connection therewith. Neither the City of Georgetown nor Williamson County assumes any responsibility for drainage ways or easements in the subdivision, other than those draining or protecting the road system and streets in their respective jurisdictions.
- 11. Right-of-way easements for widening roadways or improving drainage shall be maintained by the landowner until road or drainage improvements are actually constructed on the property. The City and/or County have the right at any time to take possession of any road widening easement for construction, improvement, or maintenance of the adjacent road.
- 12. Neither the City of Georgetown nor Williamson County assumes any responsibility for the accuracy of representations by other parties in this plat. Floodplain data, in particular, may change depending on subsequent development. It is further understood that the owners of the tract of land covered by this plat must install at their own expense all traffic control devices and signage that may be required before the streets in the subdivision have finally been accepted for maintenance by the City and / or County.
- 13. This plat is subject to the provisions of the City of Georgetown Water Conservation Ordinance.
- 14. The subdivision subject to this application is subject to the Water Quality Regulations of the City of Georgetown.
- 15. A geologic assessment, in accordance with the City of Georgetown water quality regulations, was completed on November 4, 2022. Any springs and streams as identified in the geologic
- 16. All individual lots containing Heritage Trees are configured and designed so that the lot is developable for the intended purpose without requiring removal of the Heritage Trees or exceeding the percentage of allowable disturbance within the Heritage Trees CRZ.
- 17. No development shall begin prior to the issuance of a Floodplain Development Permit for Lot 1, Block A.
- 18. Any heritage tree as noted on this plat is subject, in perpetuity, to the maintenance, care, pruning and removal requirements of the City of Georgetown. Approved removal does not require modification of the plat.
- 19. A 15-foot Public Utility Easement is dedicated along all street frontages within this plat.
- Impervious Coverage Plat Notes Non-Residential Lots:

TREE TABLE

- . The maximum impervious coverage per non-residential lot shall be pursuant to the UDC at the time of Site Plan application based on the zoning designation of the property.
- 21. Unless otherwise noted herein, all easements dedicated to the City of Georgetown by this plat shall be EXCLUSIVE to the City of Georgetown, and Grantor covenants that Grantor and Grantor's heirs, successors, and assigns shall not convey any other easement, license, or conflicting right to use in any manner, the area covered by this gran
- 22. All easements dedicated to the City of Georgetown by this plat additionally include the following rights:
- (1) the right of the City to change the size of any facilities installed, maintained, or operated within the easement area, (2) the right of the City to relocate any facilities within the easement area; and

TREE TABLE

(3) the right of the City to remove from the easement area all trees and parts thereof, or other obstructions, which endanger or interfere with the efficiency and maintenance of any

POINT NO.	DESCRIPTION	POINT NO.	DESCRIPTION
3510	27.5" LIVE OAK	3854	34.5" ASH
3520	27.5" LIVE OAK	3885	37.5" LIVE OAK
3521	32.5" LIVE OAK	3936	26.5" PECAN
3528	29.5" PECAN	3937	26" PECAN
3530	27.5" PECAN	3941	34.5" CEDAR ELM
3531	34" CEDAR ELM	3957	30.5" ELM
3555	33.5" ELM	3972	26" PECAN
3563	27" PECAN	3976	29.5" PECAN
3566	30" CEDAR ELM	3983	34" ASH
3628	32" LIVE OAK	3984	37.5" ASH
3646	26.5" CEDAR ELM	3985	30" ASH
3672	31.5" CEDAR ELM	3987	27.5" PECAN
3673	27.5" LIVE OAK	3993	26.5" PECAN
3678	31.5" OAK	4773	30" CEDAR ELM
3690	29" ASH	4775	28.5" CEDAR ELM
3693	26" PECAN	4794	28.5" ELM
3802	34" PECAN	4795	40.5" ELM
3819	30.5" PECAN	8007	30.5" PECAN
3821	27.5" PECAN	8015	30" PECAN
3828	27.5" PECAN	8074	27.5" ASH
3834	29.5" PECAN	8075	26.5" ASH
3836	29" PECAN	8077	28.5" ASH
3847	28.5" PECAN	8084	43.5" ASH

POINT NO.	DESCRIPTION
8116	34.5" ASH
8132	28.5" COTTONWOOD
8221	27" LIVE OAK
8227	51.5" LIVE OAK
8253	28" ELM
8254	27.5" PECAN
8258	30.5" LIVE OAK

	LINE TAB	LE	LINE TABLE				
LINE	BEARING	LENGTH	LINE	BEARING	LENGTH		
L1	S 17*28'25" W	18.68'	L14	S 15'27'52" W	100.69'		
L2	S 17*28'25" W	15.01'	L15	S 60°27'52" W	34.50'		
L3	S 74*32'29" E	516.37'	L16	N 74'32'37" W	514.34		
L4	N 15*27'52" E	182.99'	L17	S 15"10'51" W	15.00'		
L5	N 74'32'08" W	90.61'	L18	S 15'10'51" W	37.63'		
L6	N 29*32'08" W	34.50'	L19	S 15"10'51" W	75.18'		
L7	N 15'27'52" E	245.75'	L20	S 15"10'59" W	372.80'		
L8	N 60°27'52" E	197.13'	L21	S 17*28'25" W	257.74		
L9	S 74°32'08" E	566.21	L22	S 74°32'29" E	516.90*		
L10	S 29'32'08" E	139.15'	L23	S 15'27'52" W	689.93'		
L11	S 15*27'52" W	1006.20'	L24	S 60°27'52" W	22.07'		
L12	S 60°27'52" W	139.15'	L25	N 74'32'37" W	508.13'		
L13	N 74'32'08" W	575.61'	L26	N 15*27'52" E	778.45'		

OWNER'S SIGNATURE BLOCK

KNOW ALL MEN BY THESE PRESENTS COUNTY OF ____

That, TEXAG REAL ESTATE SERVICES, acting by LARRY D. KOKEL, Manager, owner of the certain 56.730 acres tract of land shown hereon and described in deed recorded in Document No. 2021053504 the Official Public Records of Williamson County, Texas, do hereby state that there are no lien holders of the certain tract of land, do hereby certify there are no easement holders except as shown hereon and do hereby subdivide, said tract as shown hereon; do hereby covenant to all restrictions listed herein; which shall run with the land; and do hereby dedicate to the City of Georgetown the streets, alleys, rights-of-way, easements and public places shown hereon for such public purposes as the City of Georgetown may deem appropriate. I hereby bind my heirs, successors, and assigns to warrant and forever defend such dedications, all and singular, to the City of Georgetown against every person whomsoever claiming or to claim the same or any part thereof. This subdivision is to be known as FIDELIS GEORGETOWN.

TO CERTIFY WHICH, WITNESS by my hand this ____ day of _____ 2022. TEXAG REAL ESTATE SERVICES 404 WEST 9TH STREET, SUITE 201 GEORGETOWN, TEXAS 78626 BY: LARRY D. KOKEL 404 WEST 9TH STREET, SUITE 201 GEORGETOWN, TEXAS 78626

LARRY D. KOKEL, Manager

Its Manager

foregoing instrument.

STATE OF TEXAS KNOW ALL MEN BY THESE PRESENTS COUNTY OF ____

Before me, the undersigned, a notary public in and for said county and state, on this day personally appeared ______, known to me to be the person whose name is subscribed to the

GIVEN UNDER MY HAND AND SEAL of office this ____ day of _____ 2022.

Notary Public in and for the State of Texas

My Commission expires on: _

DESCRIPTION OF PROPERTY SURVEYED

56.730 ACRES of land in Georgetown, Williamson County, Texas, being out of the John Berry Survey 3, Abstract 51, said 56.730 acres being all of that tract of land described in the deed from Larry D. Kokel to Das Uber Montney, LLC dated March 31, 2021, and recorded in Williamson County Official Public Records as document number 2021053504, save and except the tract described as 11.438 acres in the deed to IH35 SH130, LP dated January 19, 2022, and recorded in Official Public Records as document number 2022009903.

BEGINNING at the northwest corner of the Tilson Subdivision (Plat Cabinet O Slide 263), said corner being marked with a rebar stake with a TXDOT aluminum cap, said stake being located in the southeast right-of-way line of Interstate Highway 35 (Volume 458, Page 460).

THENCE along the southeast right-of-way line of said Interstate 35 as follows:

- N 15°11'00" E 500.52' to a bend in said right-of-way line, said point found to be on the southerly side of the remains of a concrete right-of-way marker; and
- N 17°28'25" E 290.31' to the southwest corner of a 2.683 acre tract described in the Official Public Records as document number 2004086845.

THENCE along the south and east boundaries of said 2.683 acre tract as follows:

- \$ 77°35'50" E 302.20' to the southeast corner of said 2.683 acre tract, said corner being marked with a found ½" rebar; and
- N 16°19'02" E 379.56' to the northeast corner of said 2.683 acre tract, said corner being located S 16°19'02" W 60.79' of a rebar stake found with a TXDOT aluminum cap.

THENCE along the meanders of Berry Creek approximately as follows:

- N 86°22'48" E 73.89'; N 37°17'13" E 58.92'; N 18°35'56" E 95.09'; N 31°59'19" E 24.53'; N 70°31'53" E 62.27'; S 74°28'06" E 133.23'; N 76°12'11" E 276.22'; S 89°26'04" E 163.67'; S 85°10'05" E 39.80'; S 57°12'13" E 36.92'; S 45°17'49" E 633.10'; S 36°08'30" E 96.82';
- \$ 07°19'53" E 175.46'; \$ 18°45'01" E 207.53'; \$ 07°15'27" E 216.99'; \$ 05°08'37" E 187.37'; \$ 09°03'01" W 68.59' to the northeast corner of a 11.438 acre tract described in the Official Public Records as document number 2022009903.

THENCE along the north line of said 11.438 acre tract as follows:

- N 89°27'24" W 106.22' to a corner of said tract marked with a 1/2" rebar found with a Forest Survey plastic identifier cap;
- \$ 79°45'41" W 124.24' to a corner of said tract marked with a ½" rebar found with a Forest Survey plastic identifier cap; S 86°06'30" W 145.89' to a corner of said tract marked with a ½" rebar found with a Forest Survey plastic identifier cap;
- \$ 77"59'09" W 94.61' to a corner of said tract marked with a ½" rebar found with a Forest Survey plastic identifier cap;
- \$ 62°58'46" W 124.40' to a corner of said tract marked with a \%" rebar found with a Forest Survey plastic identifier cap; • \$ 66°28'56" W 265.47' to a corner of said tract marked with a ½" rebar found with a Forest Survey plastic identifier cap;
- \$ 82°01'08" W 187.35' to a corner of said tract marked with a 1/2" rebar found with a Forest Survey plastic identifier cap; • \$ 81°06'08" W 140.44' to a corner of said tract marked with a 1/2" rebar found with a Forest Survey plastic identifier cap; and

• N 77°02'45" W 684.16' to the northeast corner of said Tilson Subdivision, said corner being marked with a found ½" rebar stake.

THENCE along the northerly boundary of said Tilson Subdivision as follows:

- N 45°16'29" W 281.95' to a corner of said subdivision marked with a found ½" rebar; and
- N 72°38'57" W 83.74' to the point of beginning, this tract having an area of 56.730 Acres (2,471,138 Square Feet).

LINE	BEARING	LENGTH
L27	S 74*32'08" E	659.00'
L28	S 15°27'52" W	688.85
L29	S 60°27′52" W	126.72
L30	N 74*32'08" W	569.39'
L31	N 74'32'08" W	758.39'
L32	N 29*32'08" W	22.07
L33	N 15'27'52" E	233.32'
L34	N 60*27'52" E	184.71
L35	S 74*32'08" E	553.79'
L36	S 29*32'08" E	126.72
L37	S 15'27'52" W	289.93'

PRELIMINARY/FINAL PLAT FIDELIS GEORGETOWN

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L35	S 74'32'08" E	553.79'		a a k a a	V-	l ozos v uon	A EVDY CUITE 222
L36	S 29*32'08" E	126.72'		acnec	O NOC	AUSTIN, TX 7	AC EXPY, SUITE 320 8759
L37	S 15'27'52" W	289.93'		a West	twood compa	ny 512.485.0831	estatura traen
TX REG.	ENGINEERING FIF	RM F-469	DRAWN BY	CHECKED BY	92	DATE	JOB NUMBER
TX REG.	SURVEYING FIRM	1 LS-10008000	ZNBM	ETB	N/A	NOV 2022	4670-22.125

CITY PROJECT NUMBER

JPQ SHEET NO.

XXXX-XX-SDP

œ

BUSINESS

CREEK

BERRY

SURVEYOR'S CERTIFICATE	PLANNING AND ZONING COMMISSION
STATE OF TEXAS	This subdivision to be known as FIDELIS GEORGETOWN has been accepted and approved for filing of record with the County Clerk of Williamson County, Texas, according to the minutes of the meeting of the Georgetown Planning and Zoning Commission of the
COUNTY TRAVIS	rexas, according to the minutes of the meeting of the George town Flamming and Zoming Commission of the day of
I, MICHAEL J. NEEDHAM, A REGISTERED PROFESSIONAL LAND SURVEYOR, LICENSED BY THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS PLAT IS TRUE AND CORRECTLY MADE FROM AN ACTUAL SURVEY MADE ON THE GROUND OF THE PROPERTY LEGALLY DESCRIBED HEREON, AND THAT THERE ARE NO APPARENT DISCREPANCIES, CONFLICTS, OVERLAPPING OF IMPROVEMENTS, VISIBLE UTILITY LINES OR ROADS IN PLACE, EXCEPT AS SHOWN ON THE ACCOMPANYING PLAT, AND THAT THE CORNER MONUMENTS SHOWN THEREON WERE PROPERLY PLACED UNDER MY SUPERVISION IN ACCORDANCE WITH THE SUBDIVISION REGULATIONS OF THE CITY OF GEORGETOWN, TEXAS.	R. Travis Perthuis, Chairman Date
TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT AUSTIN, TEXAS THIS 7 TH DAY OF NOVEMBER, 2022	
PRELIMINARY THIS DOCUMENT SHALL NOT BE RECORDED FOR ANY PURPOSE AND SHALL NOT BE USED OR VIEWED OR RELIED UPON AS A FINAL SURVEY DOCUMENT. RELEASED 11/4/22.	Kaylah McCord, Secretary Date
MICHAEL J. NEEDHAM REGISTERED PROFESSIONAL LAND SURVEYOR NO. 5183 STATE OF TEXAS	
	PLANNING DIRECTOR
	I, Sofia Nelson, Planning Director of the City of Georgetown, Texas, do hereby certify this plat is approved for filing of record with the County Clerk of Williamson County, Texas.
STATE OF TEXAS ENGINEER'S CERTIFICATE	
COUNTY TRAVIS	
I, HOLLIS A. SCHEFFLER, REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS SUBDIVISION IS IN THE EDWARDS AQUIFER RECHARGE ZONE AND IS ENCROACHED BY ZONE A FLOOD AREA, AS DENOTED HEREIN AND AS DEFINED BY FEDERAL EMERGENCY MANAGEMENT ADMINISTRATION FLOOD HAZARD BOUNDARY MAP, COMMUNITY PANEL NUMBER 48491C0291F, EFFECTIVE DATE DECEMBER 20, 2019 AND THAT EACH LOT CONFORMS TO THE CITY OF GEORGETOWN REGULATIONS.	Sofia Nelson, Planning Director Date
THE FULLY DEVELOPED, CONCENTRATED STORMWATER RUNOFF RESULTING FROM THE ONE HUNDRED (100) YEAR FREQUENCY STORM IS CONTAINED WITHIN THE DRAINAGE EASEMENT SHOWN AND/OR PUBLIC RIGHTS-OF-WAY DEDICATED BY THIS PLAT.	COUNTY CLERK SIGNATURE BLOCK
TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT AUSTIN, TEXAS THIS DAY OF NOVEMBER, 2022	STATE OF TEXAS { KNOW ALL MEN BY THESE PRESENTS COUNTY OF WILLIAMSON {
PRELIMINARY THIS DOCUMENT SHALL NOT BE RECORDED FOR ANY PURPOSE AND SHALL NOT BE USED OR VIEWED OR RELIED UPON AS A FINAL SURVEY DOCUMENT. RELEASED 11/4/22.	I, Nancy Rister, Clerk of the County Court of said County, do hereby certify that the foregoing instrument in writing, with its certificate of authentication was filed for record in my office on the day of, 2022, A.D., at o'clock,M., and duly recorded this the day of, 2022, A.D., at o'clock,M., in the Official Public Records of said County in Document No
HOLLIS A. SCHEFFLER REGISTERED PROFESSIONAL ENGINEER NO. 136049 STATE OF TEXAS FOR REVIEW. THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF REVIEW UNDER THE AUTHORITY OF HOLLIS A. SCHEFFLER, REGISTERED PROFESSIONAL ENGINEER NO. 136049, ON NOVEMBER 7, 2022. IT IS NOT TO BE USED FOR BIDDING, PERMIT OR CONSTRUCTION.	TO CERTIFY WHICH, WITNESS my hand and seal at the County Court of said County, at my office in Georgetown, Texas, the date last shown above written.
	Nancy Rister, Clerk County Court of Williamson County, Texas
	Nancy Rister, Clerk County Court of Williamson County, Texas By:, Deputy
CITY BUILDING OFFICIAL	
Based upon the above representations of the Engineer or Surveyor whose seal is affixed hereto, and after a review of the plat as represented by the said Engineer or Surveyor, I find that this plat complies with the requirements of Chapter 15.44, Flood Damage Prevention, of the Georgetown Municipal Code. This certification is made solely upon such representations and should not be relied upon for verifications of the facts alleged. The City of Georgetown disclaims any responsibility to any member of the public or independent verifications of the representation, factual or otherwise, contained in this plat and the documents associated with it.	
Glen Holcomb, Building Official Date City of Georgetown	PRELIMINARY/FINAL PLAT FIDELIS GEORGETOWN SHEET 4 OF 4

TX REG. ENGINEERING FIRM F-469

TX REG. SURVEYING FIRM LS-1000800

DRAWN BY ZNBM

CHECKED BY ETB

Pacheco Koch

a Westwood company

8701 N. MOPAC EXPY, SUITE 320
AUSTIN, TX 78759
512.485.0831

DATE NOV 2022 JOB NUMBER 4670-21.337 CITY PROJECT NUMBER

BERRY CREEK BUSINESS PARK 3000, 3002, & 3004 N IH 35 NB GEORGETOWN, TEXAS, 78626 FINAL PLAT SHEET 4 OF 4

DESIGNDRAWNDATEJPQNBBDEC 2022

THE NAME OF THE APPROVED PROJECT;

THE ACTIVITY START DATE; AND THE CONTACT INFORMATION OF THE PRIME CONTRACTOR.

ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROJECT MUST BE PROVIDED WITH COMPLETE COPIES OF THE APPROVED WATER POLLUTION ABATEMENT PLAN (WPAP) AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS ARE REQUIRED TO KEEP ON-SITE COPIES OF THE APPROVED PLAN AND APPROVAL LETTER.

IF ANY SENSITIVE FEATURE(S) (CAVES, SOLUTION CAVITY, SINK HOLE, ETC.) IS DISCOVERED DURING CONSTRUCTION, ALL REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE SUSPENDED IMMEDIATELY. THE APPROPRIATE TCEQ REGIONAL OFFICE MUST BE IMMEDIATELY NOTIFIED OF ANY SENSITIVE FEATURES ENCOUNTERED DURING CONSTRUCTION. CONSTRUCTION ACTIVITIES MAY NOT BE RESUMED UNTIL THE TCEQ HAS REVIEWED AND APPROVED THE APPROPRIATE PROTECTIVE MEASURES IN ORDER TO PROTECT ANY SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM POTENTIALLY ADVERSE IMPACTS TO WATER QUALITY.

4. NO TEMPORARY OR PERMANENT HAZARDOUS SUBSTANCE STORAGE TANK SHALL BE INSTALLED WITHIN 150 FEET OF A WATER SUPPLY SOURCE, DISTRIBUTION SYSTEM, WELL, OR SENSITIVE FEATURE FRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITY, ALL TEMPORARY EROSION AND SEDIMENTATION (E&S)

CONTROL MEASURES MUST BE PROPERLY INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE APPROVED PLANS AND MANUFACTURERS SPECIFICATIONS. IF INSPECTIONS INDICATE A CONTROL HAS BEEN USED INAPPROPRIATELY, OR INCORRECTLY, THE APPLICANT MUST REPLACE OR MODIFY THE CONTROL FOR SITE

SITUATIONS. THESE CONTROLS MUST REMAIN IN PLACE UNTIL THE DISTURBED AREAS HAVE BEEN PERMANENTLY STABILIZED.

. ANY SEDIMENT THAT ESCAPES THE CONSTRUCTION SITE MUST BE COLLECTED AND PROPERLY DISPOSED OF BEFORE THE NEXT RAIN EVENT TO ENSURE IT IS NOT WASHED INTO SURFACE STREAMS, SENSITIVE FEATURES.

SEDIMENT MUST BE REMOVED FROM THE SEDIMENT TRAPS OR SEDIMENTATION BASINS NOT LATER THAN TCEQ-0592 (REV. JULY 15, 2015) PAGE 2 OF 2 WHEN IT OCCUPIES 50% OF THE BASIN'S DESIGN CAPACITY.

B. LITTER, CONSTRUCTION DEBRIS, AND CONSTRUCTION CHEMICALS EXPOSED TO STORMWATER SHALL BE PREVENTED FROM BEING DISCHARGED OFFSITE. . ALL SPOILS (EXCAVATED MATERIAL) GENERATED FROM THE PROJECT SITE MUST BE STORED ON-SITE WITH

PROPER E&S CONTROLS. FOR STORAGE OR DISPOSAL OF SPOILS AT ANOTHER SITE ON THE EDWARDS AQUIFER RECHARGE ZONE, THE OWNER OF THE SITE MUST RECEIVE APPROVAL OF A WATER POLLUTION ABATEMENT PLAN FOR THE PLACEMENT OF FILL MATERIAL OR MASS GRADING PRIOR TO THE PLACEMENT OF SPOILS AT THE

10. IF PORTIONS OF THE SITE WILL HAVE A TEMPORARY OR PERMANENT CEASE IN CONSTRUCTION ACTIVITY

LONGER THAN 14 DAYS, SOIL STABILIZATION IN THOSE AREAS SHALL BE INITIATED AS SOON AS POSSIBLE PRIOR TO THE 14TH DAY OF INACTIVITY. IF ACTIVITY WILL RESUME PRIOR TO THE 21ST DAY, STABILIZATION MEASURES ARE NOT REQUIRED. IF DROUGHT CONDITIONS OR INCLEMENT WEATHER PREVENT ACTION BY THE 14TH DAY, STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS POSSIBLE.

11. THE FOLLOWING RECORDS SHALL BE MAINTAINED AND MADE AVAILABLE TO THE TCEQ UPON REQUEST: THE DATES WHEN MAJOR GRADING ACTIVITIES OCCUR

THE DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION

THE DATES WHEN STABILIZATION MEASURES ARE INITIATED. 12. THE HOLDER OF ANY APPROVED EDWARD AQUIFER PROTECTION PLAN MUST NOTIFY THE APPROPRIATE REGIONAL OFFICE IN WRITING AND OBTAIN APPROVAL FROM THE EXECUTIVE DIRECTOR PRIOR TO INITIATING ANY

A. ANY PHYSICAL OR OPERATIONAL MODIFICATION OF ANY WATER POLLUTION ABATEMENT STRUCTURE(S), INCLUDING BUT NOT LIMITED TO PONDS, DAMS, BERMS, SEWAGE TREATMENT PLANTS, AND DIVERSIONARY STRUCTURES;

B. ANY CHANGE IN THE NATURE OR CHARACTER OF THE REGULATED ACTIVITY FROM THAT WHICH WAS ORIGINALLY APPROVED OR A CHANGE WHICH WOULD SIGNIFICANTLY IMPACT THE ABILITY OF THE PLAN TO PREVENT POLLUTION OF THE EDWARDS AQUIFER;

C. ANY DEVELOPMENT OF LAND PREVIOUSLY IDENTIFIED AS UNDEVELOPED IN THE ORIGINAL WATER POLLUTION ABATEMENT PLAN.

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY ORGANIZED SEWAGE COLLECTION SYSTEM **GENERAL CONSTRUCTION NOTES**

THIS ORGANIZED SEWAGE COLLECTION SYSTEM (SCS) MUST BE CONSTRUCTED IN ACCORDANCE WITH 30 TEXAS ADMINISTRATIVE CODE (TAC) §213.5(C), THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY'S (TCEQ) EDWARDS AQUIFER RULES AND ANY LOCAL GOVERNMENT STANDARD SPECIFICATIONS. ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROPOSED REGULATED

PROJECT MUST BE PROVIDED WITH COPIES OF THE SCS PLAN AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS MUST BE REQUIRED TO KEEP ON-SITE COPIES OF THE PLAN AND THE APPROVAL LETTER

A WRITTEN NOTICE OF CONSTRUCTION MUST BE SUBMITTED TO THE PRESIDING TCEQ REGIONAL OFFICE AT LEAST 48 HOURS PRIOR TO THE START OF ANY REGULATED ACTIVITIES. THIS NOTICE MUST INCLUDE:

THE NAME OF THE APPROVED PROJECT;

MATERIAL FOR ANY PORTION OF THE MANHOLE.

THE ACTIVITY START DATE; AND THE CONTACT INFORMATION OF THE PRIME CONTRACTOR

ANY MODIFICATION TO THE ACTIVITIES DESCRIBED IN THE REFERENCED SCS APPLICATION FOLLOWING THE DATE OF APPROVAL MAY REQUIRE THE SUBMITTAL OF AN SCS APPLICATION TO MODIFY THIS APPROVAL. INCLUDING THE PAYMENT OF APPROPRIATE FEES AND ALL INFORMATION NECESSARY FOR ITS REVIEW AND

PRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITY, ALL TEMPORARY EROSION AND SEDIMENTATION (E&S) CONTROL MEASURES MUST BE PROPERLY INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS. THESE CONTROLS MUST REMAIN IN PLACE UNTIL THE DISTURBED AREAS HAVE BEEN PERMANENTLY STABILIZED.

REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE SUSPENDED IMMEDIATELY. THE APPLICANT MUST IMMEDIATELY NOTIFY THE APPROPRIATE REGIONAL OFFICE OF THE TCEQ OF THE FEATURE DISCOVERED A GEOLOGIST'S ASSESSMENT OF THE LOCATION AND EXTENT OF THE FEATURE DISCOVERED MUST BE REPORTED TO THAT REGIONAL OFFICE IN WRITING AND THE APPLICANT MUST SUBMIT A PLAN FOR ENSURING THE STRUCTURAL INTEGRITY OF THE SEWER LINE OR FOR MODIFYING THE PROPOSED COLLECTION SYSTEM ALIGNMENT AROUND THE FEATURE. THE REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MAY NOT PROCEED UNTIL THE TCEQ-0596 (REV. JULY 15, 2015) PAGE 2 OF 6

EXECUTIVE DIRECTOR HAS REVIEWED AND APPROVED THE METHODS PROPOSED TO PROTECT THE SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM ANY POTENTIALLY ADVERSE IMPACTS TO WATER QUALITY WHILE MAINTAINING THE STRUCTURAL INTEGRITY OF THE LINE.

SEWER LINES LOCATED WITHIN OR CROSSING THE 5-YEAR FLOODPLAIN OF A DRAINAGE WAY WILL BE

FROM INUNDATION AND STREAM VELOCITIES WHICH COULD CAUSE EROSION AND SCOURING OF BACKFILL. THE TRENCH MUST BE CAPPED WITH CONCRETE TO PREVENT SCOURING OF BACKFILL, OR THE SEWER LINES MUST BE ENCASED IN CONCRETE. ALL CONCRETE SHALL HAVE A MINIMUM THICKNESS OF 6 INCHES.

BLASTING PROCEDURES FOR PROTECTION OF EXISTING SEWER LINES AND OTHER UTILITIES WILL BE IN ACCORDANCE WITH THE NATIONAL FIRE PROTECTION ASSOCIATION CRITERIA. SAND IS NOT ALLOWED AS BEDDING OR BACKFILL IN TRENCHES THAT HAVE BEEN BLASTED. IF ANY EXISTING SEWER LINES ARE DAMAGED, THE LINES MUST BE REPAIRED AND RETESTED.

ALL MANHOLES CONSTRUCTED OR REHABILITATED ON THIS PROJECT MUST HAVE WATERTIGHT SIZE ON SIZE RESILIENT CONNECTORS ALLOWING FOR DIFFERENTIAL SETTLEMENT. IF MANHOLES ARE CONSTRUCTED WITHIN THE 100-YEAR FLOODPLAIN, THE COVER MUST HAVE A GASKET AND BE BOLTED TO THE RING. WHERE GASKETED MANHOLE COVERS ARE REQUIRED FOR MORE THAN THREE MANHOLES IN SEQUENCE OR FOR MORE THAN 1500 FEET, ALTERNATE MEANS OF VENTING WILL BE PROVIDED. BRICKS ARE NOT AN ACCEPTABLE CONSTRUCTION

THE DIAMETER OF THE MANHOLES MUST BE A MINIMUM OF FOUR FEET AND THE MANHOLE FOR ENTRY MUST HAVE A MINIMUM CLEAR OPENING DIAMETER OF 30 INCHES. THESE DIMENSIONS AND OTHER DETAILS SHOWING COMPLIANCE WITH THE COMMISSION'S RULES CONCERNING MANHOLES AND SEWER LINE/MANHOLE INVERTS DESCRIBED IN 30 TAC $\S 217.55$ ARE INCLUDED ON PLAN SHEET $__$ OF $_$ IT IS SUGGESTED THAT ENTRANCE INTO MANHOLES IN EXCESS OF FOUR FEET DEEP BE ACCOMPLISHED BY

MEANS OF A PORTABLE LADDER. THE INCLUSION OF STEPS IN A MANHOLE IS PROHIBITED. 10. WHERE WATER LINES AND NEW SEWER LINE ARE INSTALLED WITH A SEPARATION DISTANCE CLOSER THAN

FEET (I.E., WATER LINES CROSSING WASTEWATER LINES, WATER LINES PARALLELING WASTEWATER LINES, OR WATER LINES NEXT TO MANHOLES) THE INSTALLATION MUST MEET THE REQUIREMENTS OF 30 TAC §217.53(D) (PIPE DESIGN) AND 30 TAC §290.44(E) (WATER DISTRIBUTION)

I1. WHERE SEWERS LINES DEVIATE FROM STRAIGHT ALIGNMENT AND UNIFORM GRADE ALL CURVATURE OF

SEWER PIPE MUST BE ACHIEVED BY THE FOLLOWING PROCEDURE WHICH IS RECOMMENDED BY THE PIPE

MANUFACTURER: N/A

IF PIPE FLEXURE IS PROPOSED, THE FOLLOWING METHOD OF PREVENTING DEFLECTION OF THE JOINT MUST BE N/A SPECIFIC CARE MUST BE TAKEN TO ENSURE THAT THE JOINT IS PLACED IN THE CENTER OF THE TRENCH AND PROPERLY BEDDED IN ACCORDANCE WITH 30 TAC §217.54.

12. NEW SEWAGE COLLECTION SYSTEM LINES MUST BE CONSTRUCTED WITH STUB OUTS FOR THE CONNECTION OF ANTICIPATED EXTENSIONS. THE LOCATION OF SUCH STUB OUTS MUST BE MARKED ON THE GROUND SUCH THAT THEIR LOCATION CAN BE EASILY DETERMINED AT THE TIME OF CONNECTION OF THE EXTENSIONS. SUCH STUB OUTS MUST BE MANUFACTURED WYES OR TEES THAT ARE COMPATIBLE IN SIZE AND MATERIAL WITH BOTH THE SEWER LINE AND THE EXTENSION. AT THE TIME OF ORIGINAL CONSTRUCTION. NEW STUB-OUTS MUST BE CONSTRUCTED SUFFICIENTLY TO EXTEND BEYOND THE END OF THE STREET PAVEMENT. ALL STUB-OUTS MUST BE SEALED WITH A MANUFACTURED CAP TO PREVENT LEAKAGE. EXTENSIONS THAT WERE NOT ANTICIPATED AT THE TIME OF ORIGINAL CONSTRUCTION OR THAT ARE TO BE CONNECTED TO AN EXISTING SEWER LINE NOT FURNISHED WITH STUB OUTS MUST BE CONNECTED USING A MANUFACTURED SADDLE AND IN ACCORDANCE WITH ACCEPTED PLUMBING TECHNIQUES.

TCEQ-0596 (REV. JULY 15, 2015) PAGE 3 OF 6 IF NO STUB-OUT IS PRESENT AN ALTERNATE METHOD OF JOINING LATERALS IS SHOWN IN THE DETAIL ON PLAN SHEET OF . (FOR POTENTIAL FUTURE LATERALS).

THE PRIVATE SERVICE LATERAL STUB-OUTS MUST BE INSTALLED AS SHOWN ON THE PLAN AND PROFILE SHEETS ON PLAN SHEET __ OF __ AND MARKED AFTER BACKFILLING AS SHOWN IN THE DETAIL ON PLAN

SHEET OF 13. TRENCHING, BEDDING AND BACKFILL MUST CONFORM WITH 30 TAC §217.54. THE BEDDING AND BACKFILL FOR FLEXIBLE PIPE MUST COMPLY WITH THE STANDARDS OF ASTM D-2321, CLASSES IA, IB, II OR III. RIGID PIPE BEDDING MUST COMPLY WITH THE REQUIREMENTS OF ASTM C 12 (ANSI A 106.2) CLASSES

14. SEWER LINES MUST BE TESTED FROM MANHOLE TO MANHOLE. WHEN A NEW SEWER LINE IS CONNECTED TO AN EXISTING STUB OR CLEAN-OUT, IT MUST BE TESTED FROM EXISTING MANHOLE TO NEW MANHOLE. IF A STUB OR CLEAN-OUT IS USED AT THE END OF THE PROPOSED SEWER LINE, NO PRIVATE SERVICE ATTACHMENTS MAY BE CONNECTED BETWEEN THE LAST MANHOLE AND THE CLEANOUT UNLESS IT CAN BE CERTIFIED AS CONFORMING WITH THE PROVISIONS OF 30 TAC §213.5(C)(3)(E).

15. ALL SEWER LINES MUST BE TESTED IN ACCORDANCE WITH 30 TAC §217.57. THE ENGINEER MUST RETAIN COPIES OF ALL TEST RESULTS WHICH MUST BE MADE AVAILABLE TO THE EXECUTIVE DIRECTOR UPON REQUEST. THE ENGINEER MUST CERTIFY IN WRITING THAT ALL WASTEWATER LINES HAVE PASSED ALL REQUIRED TESTING TO THE APPROPRIATE REGIONAL OFFICE WITHIN 30 DAYS OF TEST COMPLETION AND PRIOR TO USE OF THE NEW

COLLECTION SYSTEM. TESTING METHOD WILL BE: (A) FOR A COLLECTION SYSTEM PIPE THAT WILL TRANSPORT WASTEWATER BY GRAVITY FLOW, THE DESIGN MUST SPECIFY AN INFILTRATION AND EXFILTRATION TEST OR A LOW-PRESSURE AIR TEST. A TEST MUST CONFORM TO THE FOLLOWING REQUIREMENTS:

(A) A LOW PRESSURE AIR TEST MUST FOLLOW THE PROCEDURES DESCRIBED IN AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) C-828, ASTM C924, OR ASTM F-1417 OR OTHER PROCEDURE RECOMMENDATIONS. APPROVED BY THE EXECUTIVE

DIRECTOR, EXCEPT AS TO TESTING TIMES AS REQUIRED IN TABLE C.3 IN SUBPARAGRAPH (C) OF THIS PARAGRAPH OR EQUATION C.3 IN SUBPARAGRAPH (B)(II) OF THIS PARAGRAPH.

(B) FOR SECTIONS OF COLLECTION SYSTEM PIPE LESS THAN 36 INCH AVERAGE INSIDE DIAMETER, THE FOLLOWING PROCEDURE MUST APPLY, UNLESS A PIPE IS TO BE TESTED AS REQUIRED BY PARAGRAPH (2) OF THIS SUBSECTION. (I) A PIPE MUST BE PRESSURIZED TO 3.5 POUNDS PER SQUARE INCH (PSI) GREATER THAN THE PRESSURE EXERTED BY GROUNDWATER ABOVE THE

(II) ONCE THE PRESSURE IS STABILIZED, THE MINIMUM TIME ALLOWABLE FOR THE PRESSURE TO DROP FROM 3.5 PSI GAUGE TO 2.5 PSI GAUGE IS COMPUTED FROM THE FOLLOWING EQUATION: **EQUATION C.3**

T = TIME FOR PRESSURE TO DROP 1.0 POUND PER SQUARE INCH GAUGE IN

K = 0.000419 X D X L, BUT NOT LESS THAN 1.0 D = AVERAGE INSIDE PIPE DIAMETER IN INCHES L = LENGTH OF LINE OF SAME SIZE BEING TESTED, IN FEET Q = RATE OF LOSS, 0.0015 CUBIC FEET PER MINUTE PER SQUARE FOOT INTERNAL

(C) SINCE A K VALUE OF LESS THAN 1.0 MAY NOT BE USED, THE MINIMUM TESTING

TIME FOR EACH PIPE DIAMETER IS SHOWN IN THE FOLLOWING TABLE C.3:

PIPE	MINIMUM TIME	MAXIMUM LENGTH	TIME FOR LONGER
DIAMETER(INCHES)	(SECONDS)	FOR MINIMUM TIME	LENGTH
, ,		(FEET)	(SECONDS/FOOT)
6	340	398	0.855
8	454	298	1.520
10	467	239	2.374
10	600	100	7 /10

		(FEEI)	(SECONDS/FOOT)
6	340	398	0.855
8	454	298	1.520
10	467	239	2.374
12	680	199	3.419
15	850	159	5.342
19	1020	133	7.693
21	1190	114	10.471
14	1360	100	13.676
17	1530	88	17.309
30	1700	80	21.369
33	1870	72	25.856

AN OWNER MAY STOP A TEST IF NO PRESSURE LOSS HAS OCCURRED DURING THE FIRST 25% OF THE CALCULATED TESTING TIME.

(B) IF ANY PRESSURE LOSS OR LEAKAGE HAS OCCURRED DURING THE FIRST 25% OF A TESTING PERIOD, THEN THE TEST MUST CONTINUE FOR THE ENTIRE TEST DURATION AS OUTLINED ABOVE OR UNTIL FAILURE.

(C) WASTEWATER COLLECTION SYSTEM PIPES WITH A 27 INCH OR LARGER AVERAGE INSIDE DIAMETER MAY BE AIR TESTED AT EACH JOINT INSTEAD OF FOLLOWING THE PROCEDURE OUTLINED IN THIS SECTION.

(D) A TESTING PROCEDURE FOR PIPE WITH AN INSIDE DIAMETER GREATER THAN 33 INCHES MUST BE APPROVED BY THE EXECUTIVE DIRECTOR. INFILTRATION/EXFILTRATION TEST

THE TOTAL EXFILTRATION, AS DETERMINED BY A HYDROSTATIC HEAD TEST, MUST NOT EXCEED 50 GALLONS PER INCH OF DIAMETER PER MILE OF PIPE PER 24 HOURS AT A MINIMUM TEST HEAD OF 2.0 FEET ABOVE THE CROWN OF A PIPE AT AN UPSTREAM MANHOLE.

AN OWNER SHALL USE AN INFILTRATION TEST IN LIEU OF AN EXFILTRATION TEST WHEN PIPES ARE INSTALLED BELOW THE GROUNDWATER LEVEL. (C) THE TOTAL EXFILTRATION, AS DETERMINED BY A HYDROSTATIC HEAD TEST, MUST NOT EXCEED 50 GALLONS PER INCH DIAMETER PER MILE OF PIPE PER 24 HOURS AT A MINIMUM TEST HEAD OF TWO FEET ABOVE THE CROWN OF A PIPE AT AN UPSTREAM

MANHOLE, OR AT LEAST TWO FEET ABOVE EXISTING GROUNDWATER LEVEL, (D) FOR CONSTRUCTION WITHIN A 25-YEAR FLOOD PLAIN, THE INFILTRATION OR EXFILTRATION MUST NOT EXCEED 10 GALLONS PER INCH DIAMETER PER MILE OF PIPE PER 24

HOURS AT THE SAME MINIMUM TEST HEAD AS IN SUBPARAGRAPH (C) OF THIS PARAGRAPH. (E) IF THE QUANTITY OF INFILTRATION OR EXFILTRATION EXCEEDS THE MAXIMUM QUANTITY SPECIFIED, AN OWNER SHALL UNDERTAKE REMEDIAL ACTION IN ORDER TO REDUCE

TCEQ-0596 (REV. JULY 15, 2015) PAGE 5 OF 6 THE INFILTRATION OR EXFILTRATION TO AN AMOUNT WITHIN THE LIMITS SPECIFIED. AN OWNER SHALL RETEST A PIPE FOLLOWING A REMEDIATION ACTION. (b) IF A GRAVITY COLLECTION PIPE IS COMPOSED OF FLEXIBLE PIPE, DEFLECTION TESTING IS ALSO¹².

REQUIRED. THE FOLLOWING PROCEDURES MUST BE FOLLOWED: FOR A COLLECTION PIPE WITH INSIDE DIAMETER LESS THAN 27 INCHES, DEFLECTION

MEASUREMENT REQUIRES A RIGID MANDREL. MANDREL SIZING.

A RIGID MANDREL MUST HAVE AN OUTSIDE DIAMETER (OD) NOT LESS THAN 95% OF THE BASE INSIDE DIAMETER (ID) OR AVERAGE ID OF A PIPE, AS SPECIFIED IN THE APPROPRIATE STANDARD BY THE ASTMS. AMERICAN WATER WORKS ASSOCIATION, UNI-BELL, OR AMERICAN

NATIONAL STANDARDS INSTITUTE, OR ANY RELATED APPENDIX. (ii) IF A MANDREL SIZING DIAMETER IS NOT SPECIFIED IN THE APPROPRIATE STANDARD, THE MANDREL MUST HAVE AN OD EQUAL TO 95% OF THE ID OF A PIPE. IN THIS CASE, THE ID OF THE PIPE, FOR THE PURPOSE OF DETERMINING THE OD OF THE MANDREL, MUST EQUAL BE THE AVERAGE OUTSIDE DIAMETER MINUS TWO MINIMUM WALL THICKNESSES FOR OD

CONTROLLED PIPE AND THE AVERAGE INSIDE DIAMETER FOR ID CONTROLLED PIPE. ALL DIMENSIONS MUST MEET THE APPROPRIATE STANDARD. MANDREL DESIGN.

A RIGID MANDREL MUST BE CONSTRUCTED OF A METAL OR A RIGID PLASTIC MATERIAL THAT CAN WITHSTAND 200 PSI WITHOUT BEING DEFORMED. A MANDREL MUST HAVE NINE OR MORE ODD NUMBER OF RUNNERS OR

(iii) A BARREL SECTION LENGTH MUST EQUAL AT LEAST 75% OF THE INSIDE

DIAMETER OF A PIPE (iv) EACH SIZE MANDREL MUST USE A SEPARATE PROVING RING

CASE-BY-CASE BASIS

BACKFILL.

METHOD OPTIONS. AN ADJUSTABLE OR FLEXIBLE MANDREL IS PROHIBITED. A TEST MAY NOT USE TELEVISION INSPECTION AS A SUBSTITUTE FOR A

(iii) IF REQUESTED, THE EXECUTIVE DIRECTOR MAY APPROVE THE USE OF A DEFLECTOMETER OR A MANDREL WITH REMOVABLE LEGS OR RUNNERS ON A

(2) FOR A GRAVITY COLLECTION SYSTEM PIPE WITH AN INSIDE DIAMETER 27 INCHES AND GREATER, OTHER TEST METHODS MAY BE USED TO DETERMINE VERTICAL DEFLECTION. (3) A DEFLECTION TEST METHOD MUST BE ACCURATE TO WITHIN PLUS OR MINUS 0.2% DEFLECTION. (4) AN OWNER SHALL NOT CONDUCT A DEFLECTION TEST UNTIL AT LEAST 30 DAYS AFTER THE FINAL

(5) GRAVITY COLLECTION SYSTEM PIPE DEFLECTION MUST NOT EXCEED FIVE PERCENT (5%) (6) IF A PIPE SECTION FAILS A DEFLECTION TEST, AN OWNER SHALL CORRECT THE PROBLEM AND CONDUCT A SECOND TEST AFTER THE FINAL BACKFILL HAS BEEN IN PLACE AT LEAST 30 DAYS. ALL MANHOLES MUST BE TESTED TO MEET OR EXCEED THE REQUIREMENTS OF 30 TAC §217.58.

(a) ALL MANHOLES MUST PASS A LEAKAGE TEST. (b) AN OWNER SHALL TEST EACH MANHOLE (AFTER ASSEMBLY AND BACKFILLING) FOR LEAKAGE SEPARATE AND INDEPENDENT OF THE COLLECTION SYSTEM PIPES, BY HYDROSTATIC EXFILTRATION

TESTING, VACUUM TESTING, OR OTHER METHOD APPROVED BY THE EXECUTIVE DIRECTOR. (1) HYDROSTATIC TESTING. (A) THE MAXIMUM LEAKAGE FOR HYDROSTATIC TESTING OR ANY ALTERNATIVE TEST

METHODS IS 0.025 GALLONS PER FOOT DIAMETER PER FOOT OF MANHOLE DEPTH PER HOUR. (B) TO PERFORM A HYDROSTATIC EXFILTRATION TEST, AN OWNER SHALL SEAL ALL WASTEWATER PIPES COMING INTO A MANHOLE WITH AN INTERNAL PIPE PLUG, FILL THE MANHOLE WITH WATER, AND MAINTAIN THE TEST FOR AT LEAST ONE HOUR. (C) A TEST FOR CONCRETE MANHOLES MAY USE A 24-HOUR WETTING PERIOD BEFORE TESTING TO ALLOW SATURATION OF THE CONCRETE.

(2) VACUUM TESTING. (A) TO PERFORM A VACUUM TEST, AN OWNER SHALL PLUG ALL LIFT HOLES AND EXTERIOR JOINTS WITH A NON-SHRINK GROUT AND PLUG ALL PIPES ENTERING A MANHOLE. (B) NO GROUT MUST BE PLACED IN HORIZONTAL JOINTS BEFORE TESTING.

(C) STUB-OUTS, MANHOLE BOOTS, AND PIPE PLUGS MUST BE SECURED TO PREVENT

MOVEMENT WHILE A VACUUM IS DRAWN. (D) AN OWNER SHALL USE A MINIMUM 60 INCH/LB TORQUE WRENCH TO TIGHTEN THE EXTERNAL CLAMPS THAT SECURE A TEST COVER TO THE TOP OF A MANHOLE. (E) A TEST HEAD MUST BE PLACED AT THE INSIDE OF THE TOP OF A CONE SECTION, AND THE SEAL INFLATED IN ACCORDANCE WITH THE MANUFACTURER'S

(F) THERE MUST BE A VACUUM OF 10 INCHES OF MERCURY INSIDE A MANHOLE TO PERFORM A VALID TEST

(G) A TEST DOES NOT BEGIN UNTIL AFTER THE VACUUM PUMP IS OFF. (H) A MANHOLE PASSES THE TEST IF AFTER 2.0 MINUTES AND WITH ALL VALVES CLOSED, THE VACUUM IS AT LEAST 9.0 INCHES OF MERCURY.

17. ALL PRIVATE SERVICE LATERALS MUST BE INSPECTED AND CERTIFIED IN ACCORDANCE WITH 30 TAC §213.5(C)(3)(I). AFTER INSTALLATION OF AND, PRIOR TO COVERING AND CONNECTING A PRIVATE SERVICE LATERAL TO AN EXISTING ORGANIZED SEWAGE COLLECTION SYSTEM, A TEXAS LICENSED PROFESSIONAL ENGINEER, TEXAS REGISTERED SANITARIAN, OR APPROPRIATE CITY INSPECTOR MUST VISUALLY INSPECT THE CONSTRUCTED IN CONFORMITY WITH THE APPLICABLE PROVISIONS OF THIS SECTION. THE OWNER OF THE COLLECTION SYSTEM MUST MAINTAIN SUCH CERTIFICATIONS FOR FIVE YEARS AND FORWARD COPIES TO THE APPROPRIATE REGIONAL OFFICE UPON REQUEST. CONNECTIONS MAY ONLY BE MADE TO AN APPROVED SEWAGE COLLECTION SYSTEM.

ALL WORK, UNLESS OTHERWISE NOTED, SHALL CONFORM TO TEXAS DEPARTMENT OF TRANSPORTATION STANDARD CONSTRUCTION SPECIFICATIONS OR THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION ISSUED BY THE CITY OF GEORGETOWN STANDARD CONSTRUCTION SPECIFICATIONS.

PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR SHALL BE FAMILIAR WITH THE PLANS, ALL NOTES, THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION ISSUED BY THE NORTH CENTRAL TEXAS COUNCIL OF GOVERNMENTS, THE CITY STANDARDS FOR CONSTRUCTION, AND ANY OTHER APPLICABLE STANDARDS AND SPECIFICATIONS RELEVANT TO THE PROPER COMPLETION OF THE WORK SPECIFIED. FAILURE ON THE PART OF THE CONTRACTOR TO BE FAMILIAR WITH RESPONSIBILITY OF PERFORMING THE WORK IN ACCORDANCE WITH ALL SUCH APPLICABLE STANDARDS AND

THE HORIZONTAL AND VERTICAL LOCATIONS OF EXISTING SUBSURFACE UTILITIES HAVE BEEN DETERMINED FROM DATA CLEARANCES BETWEEN EXISTING AND PROPOSED UTILITIES EXIST PRIOR TO CONSTRUCTION OF ANY SUCH CROSSINGS. IT WILL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROTECT ALL UTILITIES IN THE CONSTRUCTION OF THIS PROJECT. CONTRACTOR TO VERIFY SIZE AND LOCATION OF ALL UTILITIES PRIOR TO CONSTRUCTION AND NOTIFY ENGINEER OF ANY DISCREPANCIES

IT WILL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROTECT ALL MANHOLES, CLEANOUTS, VALVE BOXES, AND FIRE HYDRANTS, ETC. CONTRACTOR TO ADJUST TO PROPER LINE AND GRADE PRIOR TO AND AFTER THE PLACING OF PERMANENT PAVING AND GRADING. UTILITIES MUST BE MAINTAINED TO PROPER LINE AND GRADE DURING THE CONSTRUCTION OF THE PAVING FOR THIS DEVELOPMENT.

OPEN IN EACH DIRECTION: 5.2. PROVIDE AND MAINTAIN INTERIM ACCESS FROM ROADWAYS CURRENTLY IN USE TO ALL DRIVEWAYS INTERSECTING STREETS OR ALLEYS;

5.1. PROTECT AND MAINTAIN ROADWAY TRAFFIC THROUGHOUT THE PROJECT, PROVIDING A

5.3. MAINTAIN NORMAL PROJECT DRAINAGE UNTIL NEW DRAINAGE FACILITIES ARE FUNCTIONAL, INCLUDING, WHERE NECESSARY, INTERIM REPLACEMENT OF EXISTING DRAINAGE STRUCTURES REMOVED FOR CONSTRUCTION OF NEW 5.4. MAINTAIN ALL WORK AND MATERIAL STORAGE AREAS IN ORDERLY CONDITION, FREE OF DEBRIS AND WASTE. ON

CONDITION, ALL AS PROVIDED IN THE GENERAL CONDITIONS. PRIOR TO COMMENCEMENT OF CONSTRUCTION, BONDS AND THREE-WAY CONTRACTS SHALL BE SUBMITTED TO THE CITY AS REQUIRED

7. THE CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL FEDERAL, STATE, AND LOCAL REGULATIONS REGARDING TRENCH SAFETY REFER TO ARCHITECTURAL AND STRUCTURAL PLANS TO VERIFY ALL BUILDING DIMENSIONS

COMPLETION OF CONSTRUCTION, CLEAN UP THE PROJECT AND ADJACENT AFFECTED AREAS TO ACCEPTABLE

REFER TO ARCHITECTURAL PLANS FOR DETAILED BUILDING ENTRANCE LAYOUTS, RAMPS, LANDSCAPE, AND SIDEWALKS. 10. BARRICADING AND PROJECT SIGNS SHALL CONFORM TO TEXAS DEPARTMENT OF TRANSPORTATION MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES AND LATEST UPDATES 11. EXACT SAWCUT PAVEMENT REMOVAL AND REPLACEMENT LIMITS WITHIN THE PUBLIC RIGHT-OF-WAY IS TO BE IN

ACCORDANCE WITH THE CITY PAVEMENT REPAIR MANUAL AND INCLUDED IN THE BASE BID. DEMOLITION GENERAL NOTES

CONTRACTOR IS TO REVIEW ALL GENERAL NOTES PRIOR TO BEGINNING WORK. REMOVE ALL EXISTING PAVEMENT AND STRUCTURES WITHIN THE LIMITS OF DEMOLITION UNLESS OTHERWISE

FROM BACK OF CURB. SIDEWALKS, PAVEMENT, AND UTILITIES WITHIN THE PUBLIC RIGHT-OF-WAY ARE TO REMAIN UNLESS OTHERWISE NOTED. CONSULT THE DIMENSIONAL CONTROL PLAN. VERIFY THE PORTION OF EXISTING CONCRETE CURBS AND PAVEMENT WHICH ARE TO REMAIN.

SAWCUT AND REMOVE ALL EXISTING DRIVE APPROACHES (WITHIN THE LIMITS OF DEMOLITION) TWO FEET

COORDINATE WITH LOCAL POWER, TELEPHONE, CABLE, AND GAS COMPANIES PRIOR TO THE REMOVAL AND/OR RELOCATION OF EXISTING UTILITIES. ALL UTILITIES SHOULD BE CUT AND PLUGGED IN ACCORDANCE WITH THEIR RESPECTIVE UTILITY COMPANY REQUIREMENTS AND PRIOR TO DEMOLITION OF THE EXISTING BUILDINGS.

CONTRACTOR TO PLUG ALL EXISTING EXPOSED ENDS OF ABANDONED UTILITIES. CONTRACTOR TO DETERMINE SOURCE OF ALL EXPOSED UTILITIES AND, IF REQUIRED, RECONNECT TO PROPOSED UTILITIES CONTRACTOR IS RESPONSIBLE FOR THE REMOVAL AND LEGAL DISPOSAL OF ALL THE UNSUITABLE MATERIALS 7.

FROM THE PROJECT SITE. CONTRACTOR SHALL CONTACT ALL LOCAL AUTHORITIES TO DETERMINE DISPOSAL 10. ALL TREES ON THE PROPERTY SHALL BE PROTECTED AGAINST DAMAGE DURING DEMOLITION OPERATIONS 8 UNLESS OTHERWISE NOTED. THE TREE PROTECTION SHALL BE PLACED AROUND TREES PRIOR TO ANY DEMOLITION OR GRADING. TREE PROTECTION SHALL REMAIN UNTIL ALL WORK IS COMPLETED. REFER TO $^{\,9}$ LANDSCAPE PLANS FOR TREE REMOVAL AND PROTECTION DETAILS.

11. ANY DAMAGE DONE TO EXISTING TREE CROWNS OR ROOT SYSTEMS SHALL BE REPAIRED IMMEDIATELY BY AN APPROVED TREE SURGEON AT THE OWNER'S DIRECTION. ROOTS EXPOSED AND/OR DAMAGED DURING DEMOLITION AND/OR GRADING OPERATIONS SHALL BE CUT OFF CLEANLY INSIDE THE EXPOSED OR DAMAGED AREA, CUT SURFACES PAINTED WITH AN APPROVED TREE PAINT, AND TOPSOIL AND MULCH PLACED OVER THE EXPOSED ROOT AREA IMMEDIATELY

CONTRACTOR IS RESPONSIBLE FOR ESTABLISHING AND MAINTAINING EROSION CONTROL MEASURES ON THE SITE IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL REGULATIONS UNTIL THE SITE HAS BEEN STABILIZED. 13. CONTRACTOR IS RESPONSIBLE FOR GRADING ALL DISTURBED AREAS TO ALLOW FOR POSITIVE DRAINAGE. GRADING SLOPES ARE NOT TO EXCEED 3:1. AREAS EXCAVATED FOR FOUNDATION OR UNDERGROUND STRUCTURE REMOVAL SHALL BE BACK-FILLED AND

15. CONTRACTOR IS RESPONSIBLE FOR SECURITY OF THE SITE DURING DEMOLITION ACTIVITIES AND UNTIL 16. ALL WORK, UNLESS OTHERWISE NOTED, SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR PUBLIC 8. WORKS CONSTRUCTION ISSUED BY THE NORTH CENTRAL TEXAS COUNCIL OF GOVERNMENTS AND CITY STANDARD CONSTRUCTION SPECIFICATIONS

COMPACTED TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY.

REGARDING TRENCH SAFETY.

WITHIN THE PUBLIC RIGHT-OF-WAY.

COMPLETE THE SCOPE OF WORK.

17. THE HORIZONTAL AND VERTICAL LOCATIONS OF EXISTING SUBSURFACE UTILITIES HAVE BEEN DETERMINED FROM DATA RECORDED BY OTHERS. IT WILL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROTECT ALL UTILITY MAINS, MANHOLES, CLEANOUTS, VALVE BOXES, AND FIRE HYDRANTS, ETC. IN THE AREA OF DEMOLITION. 18. THE CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL FEDERAL, STATE, AND LOCAL REGULATIONS

BARRICADING AND PROJECT SIGNS SHALL CONFORM TO TEXAS DEPARTMENT OF TRANSPORTATION MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES AND LATEST UPDATES. 20. CONTRACTOR SHALL MAINTAIN EXISTING PAVEMENT AND ACCESS TO FIRE HYDRANTS ON SITE UNTIL THE BUILDINGS AND STRUCTURES IN THAT AREA HAVE BEEN DEMOLISHED AND REMOVED.

CONTRACTOR WILL BE RESPONSIBLE FOR IMPLEMENTING AND MAINTAINING ADEQUATE DUST CONTROL MEASURES DURING DEMOLITION ACTIVITIES. 23. CONTRACTOR IS TO COORDINATE DEMOLITION ACTIVITIES WITH THE HAZARDOUS MATERIAL ABATEMENT CONTRACTORS' ACTIVITIES, IF APPLICABLE.

24. THE CONTRACTOR WILL BE RESPONSIBLE FOR OBTAINING ALL TEMPORARY UTILITY SERVICES REQUIRED TO

GRADING & DRAINAGE GENERAL NOTES

6.D. CONTECH ALUMINIZED ULTRA FLOW

6.E. LANE ENTERPRISES HDPE OR APPROVED EQUAL

1. REFER TO GEOTECHNICAL REPORT 19106100.094 BY MLA GEOTECHNICAL FOR REQUIREMENTS REGARDING FILL COMPACTION AND MOISTURE

2. UNLESS NOTED, ALL FILL IS TO BE COMPACTED TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY WITHIN 3% OF OPTIMUM MOISTURE CONTENT. FILL TO BE PLACED IN MAXIMUM LIFTS OF 6 INCHES.

3. SIDEWALKS AND ACCESSIBLE ROUTES SHALL HAVE A RUNNING SLOPE NO GREATER THAN 5% (UNLESS OTHERWISE NOTED) AND A CROSS SLOPE NO GREATER THAN 2%.

4. GRADING OF ALL HANDICAPPED SPACES AND ROUTES TO CONFORM TO FEDERAL, STATE, AND LOCAL GUIDELINES. 5. ALL PROPOSED AND EXISTING GRADES IN NON-PAVED AREAS ARE "FINISHED GRADE" (i.e. IN LANDSCAPE BEDS, TOP OF MULCH/BEDDING MATERIAL).

6. UNLESS NOTED, STORM DRAIN LINES SHALL BE OF THE FOLLOWING MATERIALS AND INSTALLED IN ACCORDANCE WITH MANUFACTURER'S

SPECIFICATIONS: 6.A. RCP C-76, CLASS II 6.B. ADS N-12 6.C. HANCOR HI-C

FINAL PAVING, CURB, AND SIDEWALK ELEVATIONS WILL BE PLACED AT PLUS OR MINUS 0.03 FOOT 9. REFER TO LANDSCAPE SPECIFICATIONS FOR SEEDING AND SODDING REQUIREMENTS. 10. ANY CONCRETE, ROCK, OR MATERIAL DEEMED BY THE ENGINEER TO BE UNSUITABLE FOR SUBGRADE SHALL BE DISPOSED OF OFFSITE AT

11. TRENCH BACKFILL MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF CITY OF GEORGETOWN STANDARDS AND SHALL BE MECHANICALLY COMPACTED IN 6-INCH LIFTS TO THE TOP OF SUBGRADE TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY IN ACCORDANCE WITH CITY OF GEORGETOWN STANDARDS UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN THE STANDARD CITY SPECIFICATIONS. 12. EMBEDMENT SHALL CONFORM TO THE REQUIREMENTS OF CITY OF AUSTIN ITEM 510 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN

THE STANDARD CITY SPECIFICATIONS 13. A ROUND MANHOLE COVER MEETING CITY SPECIFICATIONS SHALL BE PLACED IN ALL INLET TOPS NEAR THE OUTLET PIPE. 14. ALL CONCRETE FOR INLETS AND DRAINAGE STRUCTURES SHALL CONFORM TO CITY OF GEORGETOWN, CLASS "A" (3000 PSI) UNLESS OTHERWISE

SHOWN ON THESE PLANS OR STATED IN STANDARD CITY SPECIFICATIONS. 15. CRUSHED STONE BEDDING OR APPROVED EQUAL SHALL BE PROVIDED BY THE CONTRACTOR WHEN ROCK IS ENCOUNTERED IN TRENCHES. THERE SHALL BE NO ADDITIONAL PAY ITEM FOR CRUSHED STONE BEDDING.

16. IF REQUIRED DUE TO CONSTRUCTION, POWER POLES TO BE BRACED OR RELOCATED AT CONTRACTOR'S EXPENSE.

UNLESS NOTED, STORM STRUCTURES TO BE "FORTERRA PIPE AND PRECAST" SIZED AS SHOWN, OR APPROVED EQUAL.

WATER & SANITARY SEWER GENERAL NOTES

11. ALL METER BOXES SHALL BE LOCATED IN NON-TRAFFIC AREAS.

ORANGE

 ALL CONCRETE SHALL BE CLASS "A" (3000 PSI), UNLESS OTHERWISE NOTED 2. ALL WATER MAINS SHALL BE PVC C900, DR 18, CLASS 235, FIRE PROTECTION SERVICES SHALL BE PVC C900, DR 14, CLASS 305 AND INSTALLED IN ACCORDANCE WITH THE DESIGN AND SPECIFICATIONS OF THE FIRE PROTECTION PLANS TO BE PREPARED BY A LICENSED FIRE PROTECTION

3. WATER AND SANITARY SEWER SERVICES SHALL MEET PLUMBING CODE REQUIREMENTS. ALL WATER MAINS SHALL HAVE A MINIMUM COVER OF 48 INCHES BELOW IMPROVED FINISHED GRADE, UNLESS OTHERWISE NOTED.

6. WHEN WATER AND SANITARY SEWER MAINS, SERVICES, AND LATERALS ARE INSTALLED, THEY SHALL BE INSTALLED NO CLOSER TO EACH OTHER THAN NINE FEET IN ALL DIRECTIONS AND PARALLEL LINES MUST BE INSTALLED IN SEPARATE TRENCHES. WHERE THE NINE FOOT SEPARATION DISTANCE CANNOT BE ACHIEVED, THE FOLLOWING TCEQ CHAPTERS SHALL APPLY: 6.A. TCEQ CHAPTER 217.53 PIPE DESIGN, SECTION (d) SEPARATION DISTANCES.

6.B. TCEQ CHAPTER 290.44 WATER DISTRIBUTION, SECTION (e) LOCATION OF WATERLINES 7. CONTRACTOR TO VERIFY ALL EXISTING SEWER FLOW LINES BEFORE BEGINNING CONSTRUCTION 8. CONTRACTOR SHALL TIE A ONE INCH WIDE PIECE OF RED PLASTIC FLAGGING TO THE END OF SEWER SERVICE AND SHALL LEAVE A MINIMUM OF 36 INCHES OF FLAGGING EXPOSED AFTER BACKFILL. AFTER CURB AND PAVING IS COMPLETED, CONTRACTOR SHALL MARK THE LOCATION OF

THE SEWER SERVICE ON THE CURB OR ALLEY IN ACCORDANCE WITH THE STANDARD CITY SPECIFICATIONS 9. ALL SANITARY SEWER LINES SHALL BE TESTED IN ACCORDANCE WITH THE STANDARD CITY SPECIFICATIONS. THE UTILITY CONTRACTOR SHALL INSTALL THE WATER SERVICES TO A POINT TWO FEET BACK OF THE CURB LINE AT A DEPTH OF 12 INCHES. THE METER BOX SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR AFTER THE PAVING CONTRACTOR HAS COMPLETED THE FINE PRIVATE SERVICE LATERAL AND THE CONNECTION TO THE SEWAGE COLLECTION SYSTEM, AND CERTIFY THAT IT IS GRADING BEHIND THE BACK OF THE CURB. EACH SERVICE LOCATION SHALL BE MARKED ON THE CURB WITH A BLUE LETTER "W" BY THE UTILITY CONTRACTOR AND TIED TO PROPERTY CORNERS ON THE "RECORD DRAWINGS."

> 12. TRENCH BACKFILL MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF NCTCOG ITEM 504.2 AND SHALL BE MECHANICALLY COMPACTED IN 6-INCH LIFTS TO THE TOP OF SUBGRADE TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY IN ACCORDANCE WITH NCTCOG ITEM 504.5 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN THE STANDARD CITY SPECIFICATIONS 13. EMBEDMENT SHALL CONFORM TO THE REQUIREMENTS OF NCTCOG ITEM 504.5 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN

THE STANDARD CITY SPECIFICATIONS. 14. VALVE BOXES SHALL BE FURNISHED AND SET ON EACH GATE VALVE. AFTER THE FINAL CLEAN-UP AND ALIGNMENT HAS BEEN COMPLETED, THE UTILITY CONTRACTOR SHALL POUR A 24"X24"X6" CONCRETE BLOCK AROUND ALL VALVE BOX TOPS LEVEL WITH THE FINISHED GRADE.

15. CONTRACTOR SHALL RECONNECT ALL EXISTING SERVICES AND MAINTAIN EXISTING SERVICES THROUGHOUT CONSTRUCTION. 16. IF REQUIRED DUE TO CONSTRUCTION, POWER POLES TO BE BRACED OR RELOCATED AT CONTRACTOR'S EXPENSE.

AT THE CONCLUSION OF CONSTRUCTION AND AS PART OF THE PROCESS FOR THE CITY TO ACCEPT THIS PHASE, THE FIRE HYDRANTS SHALL BE FLOWED ALL STANDARDS AND SPECIFICATIONS PERTAINING TO THIS WORK SHALL IN NO WAY RELIEVE THE CONTRACTOR OF AND TESTED AND A COPY OF THE REPORT SHALL BE EMAILED INTO THE FIRE DEPARTMENT AND THE HYDRANTS SHALL BE PAINTED AND COLOR CODED. CAUTION: IF PRESSURE REDUCING VALVES WERE INSTALLED IN THIS PHASING THEY MUST BE SET PRIOR TO FIRE HYDRANT FLOW TESTING.

A. ALL PRIVATE HYDRANT BARRELS WILL BE PAINTED RED WITH THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN PARAGRAPH C OF THIS RECORDED BY OTHERS. CONTRACTOR SHALL VERIFY ELEVATIONS SHOWN AND ENSURE THAT NECESSARY CROSSING SECTION TO INDICATE FLOW. IT WILL BE THE CUSTOMER'S RESPONSIBILITY TO TEST AND MAINTAIN THEIR PRIVATE FIRE HYDRANT(S). B. ALL PRIVATE FIRE HYDRANTS SHOULD BE TESTED ANNUALLY AND SHALL BE COLOR CODED TO INDICATE THE EXPECTED FIRE FLOW FROM THE HYDRANT DURING NORMAL OPERATION. SUCH COLOR APPLIED TO THE FIRE HYDRANT BY PAINTING THE BONNET THE APPROPRIATE COLOR FOR THE EXPECTED FLOW CONDITION.

> C. HYDRANT FLOW CODING STANDARDS. BLUE 1000 TO 1500 GPM

> > PAVING GENERAL NOTES

500 TO 999 GPM

LESS THAN 500 GPM

TCEQ REQUIREMENT.

LONG FIRE HYDRANT LEADS SHALL BE RESTRAINED

ALL DIMENSIONS ARE FROM BACK OF CURB UNLESS OTHERWISE NOTED. ALL CONCRETE SHALL CONFORM TO CITY OF GEORGETOWN STANDARDS, UNLESS OTHERWISE SHOWN ON THESE PLANS, STATED IN

STANDARD CITY SPECIFICATIONS OR STATED IN TXDOT STANDARD SPECIFICATIONS. SUBGRADE PREPARATION IN RIGHT OF WAY SHALL CONFORM TO STANDARD CITY SPECIFICATIONS OR TXDOT STANDARD SPECIFICATIONS. ALL FILL PLACED UNDER PAVING SHALL BE COMPACTED TO 95% STANDARD PROCTOR DENSITY IN 6 INCH LIFTS, UNLESS OTHERWISE NOTED. OR STATED IN GEOTECH REPORT. REFER TO STRUCTURAL SPECIFICATIONS FOR FILL PLACED BENEATH BUILDING AREAS. ALL OTHER FILL AREAS TO BE COMPACTED TO 90% STANDARD PROCTOR.

THE CONTRACTOR SHALL SUBMIT A JOINT SPACING PLAN TO THE ENGINEER FOR APPROVAL. EXPANSION JOINT SPACING SHALL BE 90' MAXIMUM EACH WAY WITH NO KEYWAYS AND SAWED DUMMY JOINTS SHALL BE 15' EACH WAY, UNLESS OTHERWISE NOTED. TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED AT THE END OF EACH DAYS PAVING AND WHERE INTERRUPTIONS SUSPEND **OPERATIONS FOR 30 MINUTES OR MORE**

ALL PAVING TO BE REMOVED SHALL BE SAWCUT TO A NEAT LINE, MINIMUM 1-1/2" DEEP, AND THE PAVEMENT REMOVED IN SUCH A MANNER AS TO PRESERVE THE EXISTING TRANSVERSE REINFORCING STEEL TO THE MAXIMUM EXTENT POSSIBLE. ALL CURB AND GUTTER SHALL BE INTEGRAL WITH THE PAVEMENT AND HAVE THE SAME COMPRESSIVE STRENGTH PAVEMENT REINFORCEMENT SHALL BE #3 BARS. SPACED AT 18 INCHES CENTER TO CENTER EACH WAY EXCEPT WHERE OTHERWISE NOTED

IN THE PLANS OR GEOTECH REPORT. BAR LAPS SHALL BE 30 DIAMETERS IN LENGTH

ALL STRIPES SHALL BE 4 INCHES WIDE, UNLESS OTHERWISE NOTED. INSTALLATION AND PLACEMENT OF IRRIGATION SLEEVES AND UTILITY CONDUITS SHALL BE IN ACCORDANCE WITH LANDSCAPE ARCHITECT AND MEP PLANS. CONTRACTOR TO VERIFY ALL SLEEVES HAVE BEEN PLACED PRIOR TO PAVING BEING PLACED.

SIDEWALKS AND ACCESSIBLE ROUTES SHALL HAVE A RUNNING SLOPE NO GREATER THAN 5% (UNLESS OTHERWISE NOTED) AND A CROSS

SLOPE NO GREATER THAN 2%.

APPROVAL OF THIS SITE PLAN DOES NOT IMPLY APPROVAL TO INSTALL UNDERGROUND FIRE LINES, PRIOR TO INSTALLATION OF UNDERGROUND FIRE LINES, A SEPARATE PERMIT SHALL BE SUBMITTED, UNDER GROUND FIRE LINE SUPPLY. BACKFLOW PROTECTION WILL BE PROVIDED IN ACCORDANCE WITH THE CITY OF GEORGETOWN REQUIREMENTS WHEN REQUIRED. BACKFLOW PROTECTION WILL BE NSTALLED IN ACCORDANCE WITH THE DETAIL PROVIDED IN THE UTILITY DRAWINGS.

ALL PRIVATE FIRE LINES AND WHAT THEY PROVIDE SERVICE TO WILL BE INSTALLED IN ACCORDANCE WITH NFPA 24 INSTULLATION OF PRIVATE SERVICE MAINS AND THEIR APPURTENANCES. 4. ALL TEES, PLUGS, CAPS, BENDS, REDUCERS, VALVES SHALL BE RESTRAINED AGAINST MOVEMENT, THRUST BLOCKING WILL BE INSTALLED IN ACCORDANCE WITH NFPA 24 ALL UNDERGROUND SHALL REMAIN UNCOVERED UNTIL A VISUAL INSPECTION IS CONDUCTED BY THE GEORGETOWN FIRE MARSHAL'S OFFICE (FMO). ALL JOINTS AND THRUST BLOCKING SHALL BE UNCOVERED FOR VISUAL INSPECTION

ALL UNDERGROUND SHALL BE FLUSHED PER THE REQUIREMENTS OF NFPA STANDARD 24 AND WITNESSED BY GEORGETOWN FMC ALL UNDERGROUND SHALL PASS A HYDROSTATIC TEST WITNESSED BY GEORGETOWN FMO. ALL JOINTS SHALL BE UNCOVERED FOR HYDROSTATIC TESTING. ALL PIPING AND ATTACHMENTS SUBJECTED TO SYSTEM WORKING PRESSURE SHALL BE TESTED AT 200 PSI. OR 50 PSI IN EXCESS OF THE SYSTEM WORKING PRESSURE, WHICHEVER IS GREATER. AND SHALL MAINTAIN THAT PRESSURE \pm OR \pm 5 PSI FOR 2 HOURS. FENCES, LANDSCAPING AND OTHER ITEMS WILL NOT BE INSTALLED WITHIN 3 FT, AND WHERE THEY WILL OBSTRUCT THE VISIBILITY OR ACCESS TO HYDRANTS, OR REMOTE

LICENSE REQUIREMENTS OF EITHER RME-U OR G. WHEN CONNECTING BY UNDERGROUND TO THE WATER PURVEYOR'S MAIN FROM THE POINT OF CONNECTION OR VALVE

CITY OF GEORGETOWN GENERAL NOTES

1. THESE CONSTRUCTION PLANS WERE PREPARED, SEALED, SIGNED, AND DATED BY A TEXAS LICENSED PROFFESIONAL ENGINEER. THEREFORE BASED ON THE ENGINEER'S CONCURRAENCE OF COMPLIANCE, THE CONSTRUCTION PLANS FOR CONSTRUCTION OF THE PROPOSED PROJECT ARE HEREBY APPROVED SUBJECT TO THE STANDARD CONSTRUCTION SPECIFICATIONS AND DETAILS MANUAL AND ALL OTHER APPLICABLE CITY, STATE, AND FEDERAL

REQUIREMENTS AND CODES THIS PROJECT IS SUBJECT TO ALL CITY STANDARD SPECIFICATIONS AND DETAILS IN EFFECT AT THE TIME OF SUBMITTAL OF THE PROJECT TO THE CITY. THE SITE CONSTRUCTION PLANS SHALL MEET ALL REQUIREMENTS OF THE APPROVED SITE PLAN. WASTEWATER MAINS AND SERVICE LINES SHALL BE SDR 26 PVC

WASTEWATER MAINS SHALL BE INSTALLED WITHOUT HORIZONTAL OR VERTICAL BENDS. MAXIMUM DISTANCE BETWEEN WASTEWATER MANHOLES IS 500 FEET. WASTEWATER MAINS SHALL BE LOW PRESSURE AIR TESTED AND MANDREL TESTED BY THE CONTRACTOR ACCORDING TO CITY OF GEORGETOWN AN

WASTEWATER MANHOLES SHALL BE VACUUM TESTED AND COATED BY THE CONTRACTOR ACCORDING TO CITY OF GEORGETOWN AND TCEQ REQUIREMENTS. WASTEWATER MAINS SHALL BE CAMERA TESTED BY THE CONTRACTOR AND SUBMITTED TO THE CITY ON DVD FORMAT PRIOR TO PAVING THE STREETS PRIVATE WATER SYSTEM FIRE LINES SHALL BE TESTED BY THE CONTRACTOR TO 200 PSI FOR 2 HOURS.

PRIVATE WATER SYSTEM FIRE LINES SHALL BE DUCTILE IRON PIPING FROM THE WATER MAIN TO THE BUILDING SPRINKLER SYSTEM, AND 200 PSI C900 PV PUBLIC WATER SYSTEM MAINS SHALL BE 150 PSI C900 PVC AND TESTED BY THE CONTRACTOR AT 150 PSI FOR 4 HOURS.

ALL BENDS AND CHANGES IN DIRECTION ON WATER MAINS SHALL BE RESTRAINED AND THRUST BLOCKED.

ALL WATER LINES ARE TO BE BACTERIA TESTED BY THE CONTRACTOR ACCORDING TO CITY STANDARDS AND SPECIFICATIONS WATER AND SEWER MAIN CROSSINGS SHALL MEET ALL REQUIREMENTS OF THE TCEQ AND THE CITY. FLEXIBLE BASE MATERIAL FOR PUBLIC STREETS SHALL BE TXDOT TYPE A GRADE 1 HOT MIX ASPHALTIC CONCRETE PAVEMENT SHALL BE TYPE D UNLESS OTHERWISE SPECIFIED AND SHALL BE A MINIMUM OF 2 INCHES THICK ON PUBLIC

RECORD DRAWINGS OF PUBLIC IMPROVEMENTS SHALL BE SUBMITTED TO THE CITY BY THE DESIGN ENGINEER PRIOR TO ACCEPTANCE OF THE PROJEC

THESE DRAWINGS SHALL BE ON MYLAR OR ON TIFF OR PDF DISK (300DPI). IF A DISK IS SUBMITTED. A BOND SET SHALL BE INCLUDED WITH THE DISK

STREETS AND ROADWAYS. 21. CONTRACTOR WILL PROVIDE ON-SITE PARKING FOR WORKERS. VEHICLE PARKING WILL NOT BE ALLOWED ALL SIDEWALK RAMPS ARE TO BE INSTALLED WITH THE PUBLIC INFRASTRUCTURE. A MAINTENANCE BOND IS REQUIRED TO BE SUBMITTED TO THE CITY PRIOR TO ACCEPTANCE OF THE PUBLIC IMPROVEMENTS. THIS BOND SHALL BE ESTABLISHED FOR 2 YEARS IN THE AMOUNT OF 10% OF THE COST OF THE PUBLIC IMPROVEMENTS AND SHALL FOLLOW THE CITY FORMAT

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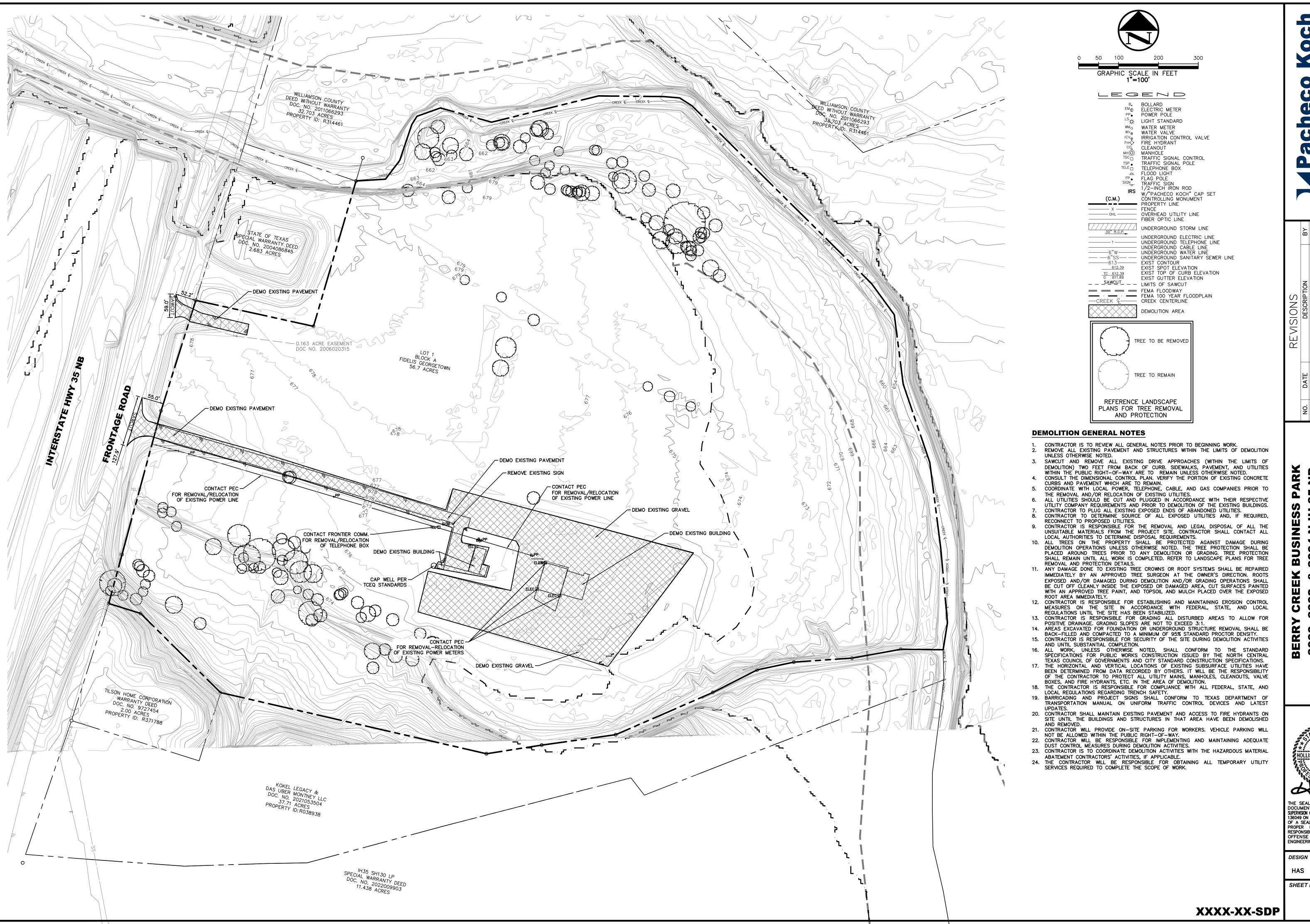
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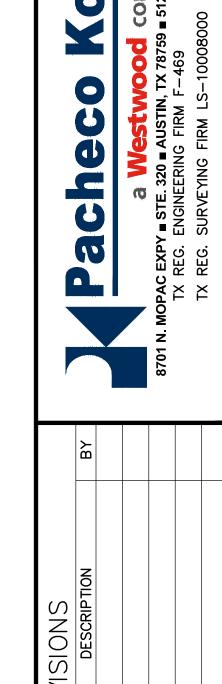
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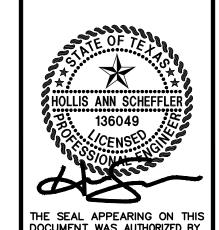
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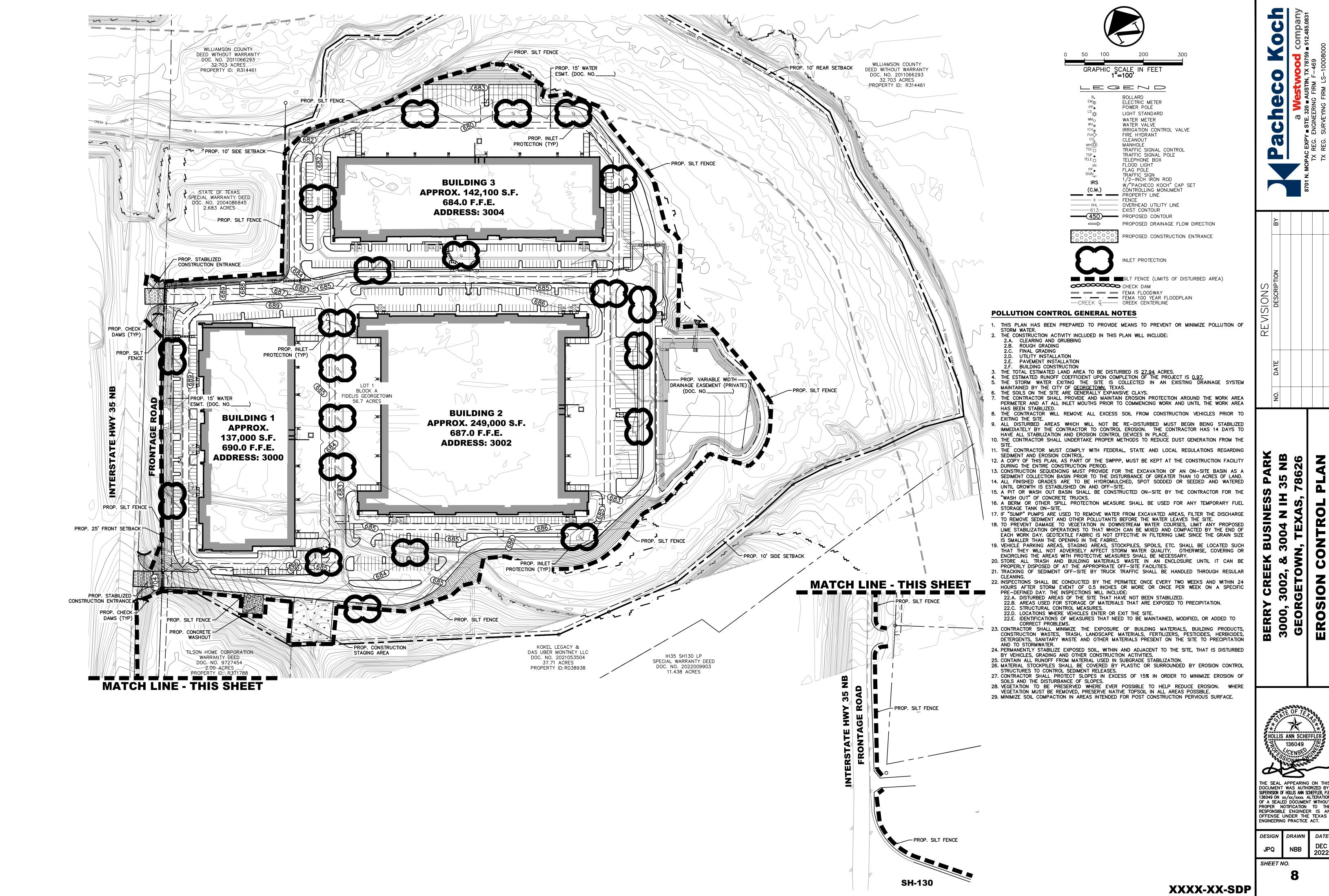
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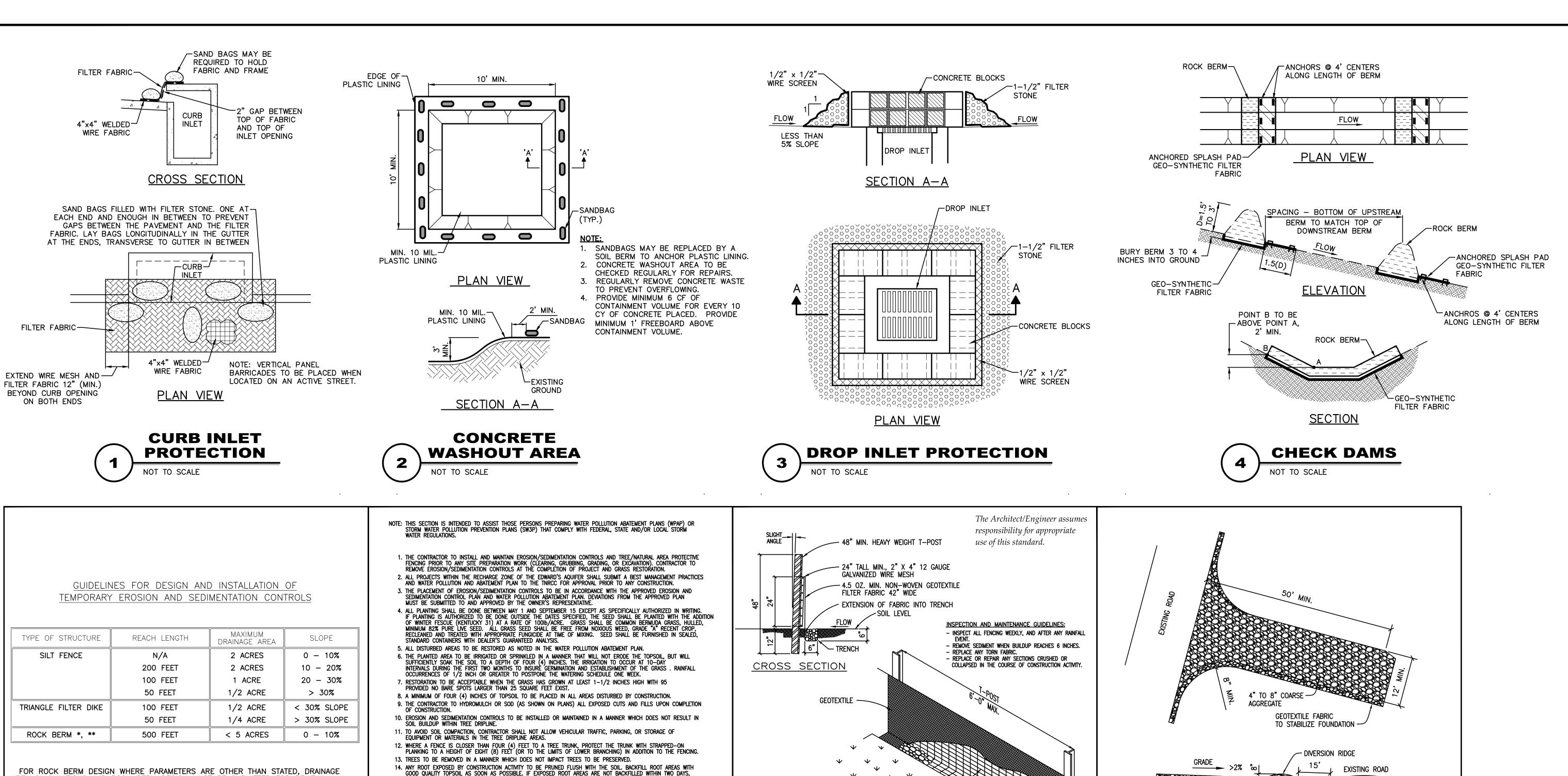
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GEORGETOWN TEXAS

AREA CALCULATIONS AND ROCK BERM DESIGN MUST BE SUBMITTED FOR REVIEW. HIGH SERVICE ROCK BERMS MAY BE REQUIRED IN AREAS OF ENVIRONMENTAL SIGNIFICANCE AS DETERMINED BY THE CITY OF GEORGETOWN.

CITY OF GEORGETOWN

CONSTRUCTION STANDARDS AND DETAILS

TEMPORARY EROSION AND

SEDIMENTATION CONTROL GUIDELINES NTS 1/2003

ADOPTED 6/21/2006

MRS TRB

GEORGETOWN

14. ANY ROOT EXPOSED BY CONSTRUCTION ACTIVITY TO BE PRUNED FLUSH WITH THE SOIL. BACKFILL ROOT AREAS WITH GOOD QUALITY TOPSOIL AS SOON AS POSSIBLE. IF EXPOSED ROOT AREAS ARE NOT BACKFILLED WITHIN TWO DAYS, COVER THEM WITH ORGANIC MATERIAL IN A MANNER WHICH REDUCES SOIL TEMPERATURE AND MINIMIZES WATER LÓSS 15. CONTRACTOR TO PRUNE VEGETATION TO PROVIDE CLEARANCE FOR STRUCTURES, VEHICULAR TRAFFIC, AND EQUIPMENT BEFORE DAMAGE OCCURS (RIPPING OF BRANCHES, ETC.). ALL FINISHED PRUNING TO BE DONE ACCORDING TO RECOGNIZED, APPROVED STANDARDS OF THE INDUSTRY (REFERENCE THE "NATIONAL ARBORIST ASSOCIATION PRUNING STANDARDS FOR SHADE TREES"). 16. THE CONTRACTOR IS TO INSPECT THE CONTROLS AT WEEKLY INTERVALS AND AFTER EVERY RAINFALL EXCEEDING 1/4 INCH TO VERIFY THAT THEY HAVE NOT BEEN SIGNIFICANTLY DISTURBED. ANY ACCUMULATED SEDIMENT AFTER A SIGNIFICANT RAINFALL TO BE REMOVED AND PLACED IN THE OWNER DESIGNATED SPOIL DISPOSAL SITE. THE CONTRACTOR TO CONDUCT PERIODIC INSPECTIONS OF ALL EROSION/SEDIMENTATION CONTROLS AND TO MAKE ANY REPAIRS OR MODIFICATIONS NECESSARY TO ASSURE CONTINUED EFFECTIVE OPERATION OF EACH DEVICE. 17. WHERE THERE IS TO BE AN APPROVED GRADE CHANGE, IMPERMEABLE PAVING SURFACE, TREE WELL, OR OTHER SUCH SITE DEVELOPMENT IMMEDIATELY ADJACENT TO A PROTECTED TREE, ERECT THE FENCE APPROXIMATELY TWO TO FOUR FEET (2'-4') BEHIND THE AREA IN QUESTION. 18. NO ABOVE AND/OR BELOW GROUND TEMPORARY FUEL STORAGE FACILITIES TO BE STORED ON THE PROJECT SITE. 19. IF EROSION AND SEDIMENTATION CONTROL SYSTEMS ARE EXISTING FROM PRIOR CONTRACTS, OWNER'S REPRESENTATIVE AND THE CONTRACTOR TO EXAMINE THE EXISTING EROSION AND SEDIMENTATION CONTROL SYSTEMS FOR DAMAGE PRIOR TO CONSTRUCTION. ANY DAMAGE TO PREEXISTING EROSION AND SEDIMENTATION CONTROLS NOTED TO BE REPAIRED AT OWNERS EXPENSE. 20. INTENTIONAL RELEASE OF VEHICLE OR EQUIPMENT FLUIDS ONTO THE GROUND IS NOT ALLOWED. CONTAMINATED SOIL RESULTING FROM ACCIDENTAL SPILL TO BE REMOVED AND DISPOSED OF PROPERLY. The Architect/Engineer assume responsibility for appropriate use of this standard. REVISION NOTE: ADOPTED 6/21/2006 CITY OF GEORGETOWN CONSTRUCTION STANDARDS AND DETAILS EC01A

EROSION AND SEDIMENTATION AND

TREE PROTECTION NOTES

DIRWIN BY: APPROVED BY:
MRS TRB

2" X 4" WIRE MESH **INSTALLATION:** LAYOUT THE SILT FENCE FOLLOWING AS CLOSELY AS POSSIBLE TO THE CONTOUR. CLEAR THE GROUND OF DEBRIS, ROCKS, PLANTS (INCLUDING GRASSES TALLER THAN 2") TO PROVIDE A SMOOTH FLOW APPROACH SURFACE. EXCAVATE 6" DEEP X 6" WIDE TRENCH ON UPSTREAM SIDE OF FACE PER PLANS. DRIVE THE HEAVY DUTY T-POST AT LEAST 12 INCHES INTO THE GROUND AND AT A SLIGHT ANGLE TOWARDS THE FLOW. ATTACH THE 2" X 4" 12 GAUGE WELDED WIRE MESH TO THE T-POST WITH 11 1/2 GAUGE GALVANIZED T-POST CLIPS. THE TOP OF THE WIRE TO BE 24" ABOVE GROUND LEVEL. THE WELDED WIRE MESH TO BE OVERLAPPED 6" AND TIED AT LEAST 6 TIMES WITH HOG RINGS. THE SILT FENCE TO BE INSTALLED WITH A SKIRT A MINIMUM OF 6" WIDE PLACED ON THE UPHILL SIDE OF THE FENCE INSIDE EXCAVATED TRENCH. THE FABRIC TO OVERLAP THE TOP OF THE WIRE BY 1". ANCHOR THE SILT FENCE BY BACKFILLING WITH EXCAVATED DIRT AND ROCKS (NOT LARGER THAN 2"). GEOTEXTILE SPLICES SHOULD BE A MINIMUM OF 18" WIDE ATTACHED IN AT LEAST 6 PLACES. SPLICES IN CONCENTRATED FLOW AREAS WILL NOT BE ACCEPTED. SILT FENCE SHALL BE REMOVED WHEN THE SITE IS COMPLETELY STABILIZED SO AS NOT TO BLOCK OR IMPEDE STORM FLOW OR DRAINAGE.

CITY OF GEORGETOWN

CONSTRUCTION STANDARDS AND DETAILS

SILT FENCE DETAIL

A PARTICIPATION OF THE PARTICI GEOTEXTILE FABRIC AS APPROVED BY THE CITY - CLEAR THE AREA OF DEBRIS, ROCKS OR PLANTS THAT WILL INTERFERE WITH INSTALLATION. - PLACE GEOTEXTILE FABRIC AS APPROVED BY THE CITY. - PLACE ROCK AS APPROVED BY THE CITY. NSPECTIONS AND MAINTENANCE GUIDELINES:

- GRADE THE AREA FOR THE ENTRANCE TO FLOW BACK ON TO THE CONSTRUCTION SITE. RUNOFF FROM THE STABILIZED CONSTRUCTION

- THE ENTRANCE SHOULD BE MAINTAINED IN A CONDITION, WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. - ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ON TO PUBLIC RIGHTS-OF-WAY SHOULD BE REMOVED IMMEDIATELY BY

- WHEN NECESSARY, WHEELS SHOULD BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO PUBLIC RIGHTS-OF-WAY. - When washing is required, it should be done on an area stabilized with crushed stone that drains into an approved SEDIMENT TRAP OR SEDIMENT BASIN.

- ALL SEDIMENT SHOULD BE PREVENTED FROM ENTERING ANY STORM DRAIN, DITCH OR WATER COURSE BY USING APPROVED METHODS.

The Architect/Engineer assumes

responsibility for appropriate

use of this standard.

ADOPTED 6/21/2006

NTS 1/2003

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CITY OF GEORGETOWN CONSTRUCTION STANDARDS AND DETAILS 94 STABILIZED CONSTRUCTION ENTRANCE Georgetown

HOLLIS ANN SCHEFFLE 136049 ADOPTED 6/21/2006

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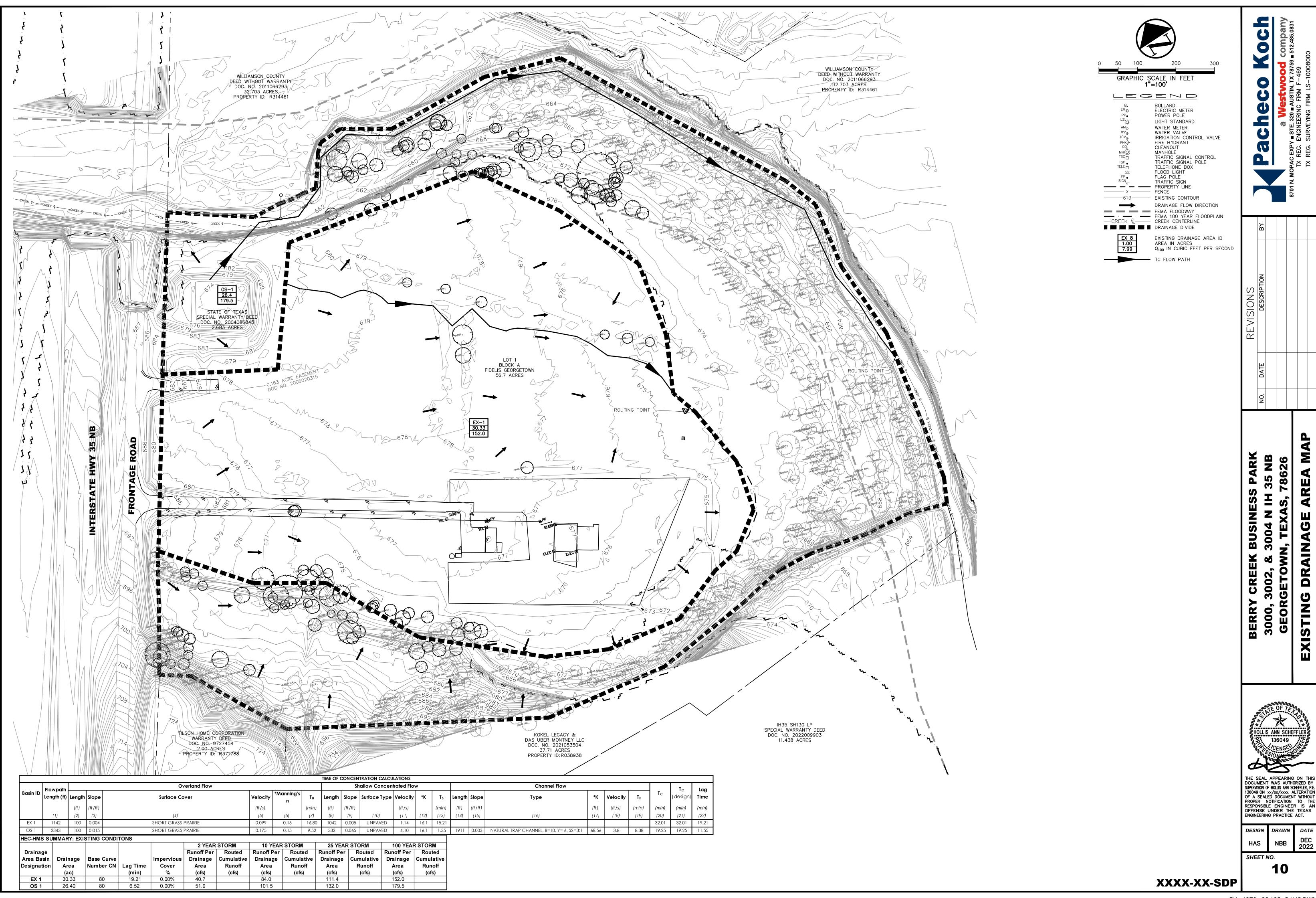
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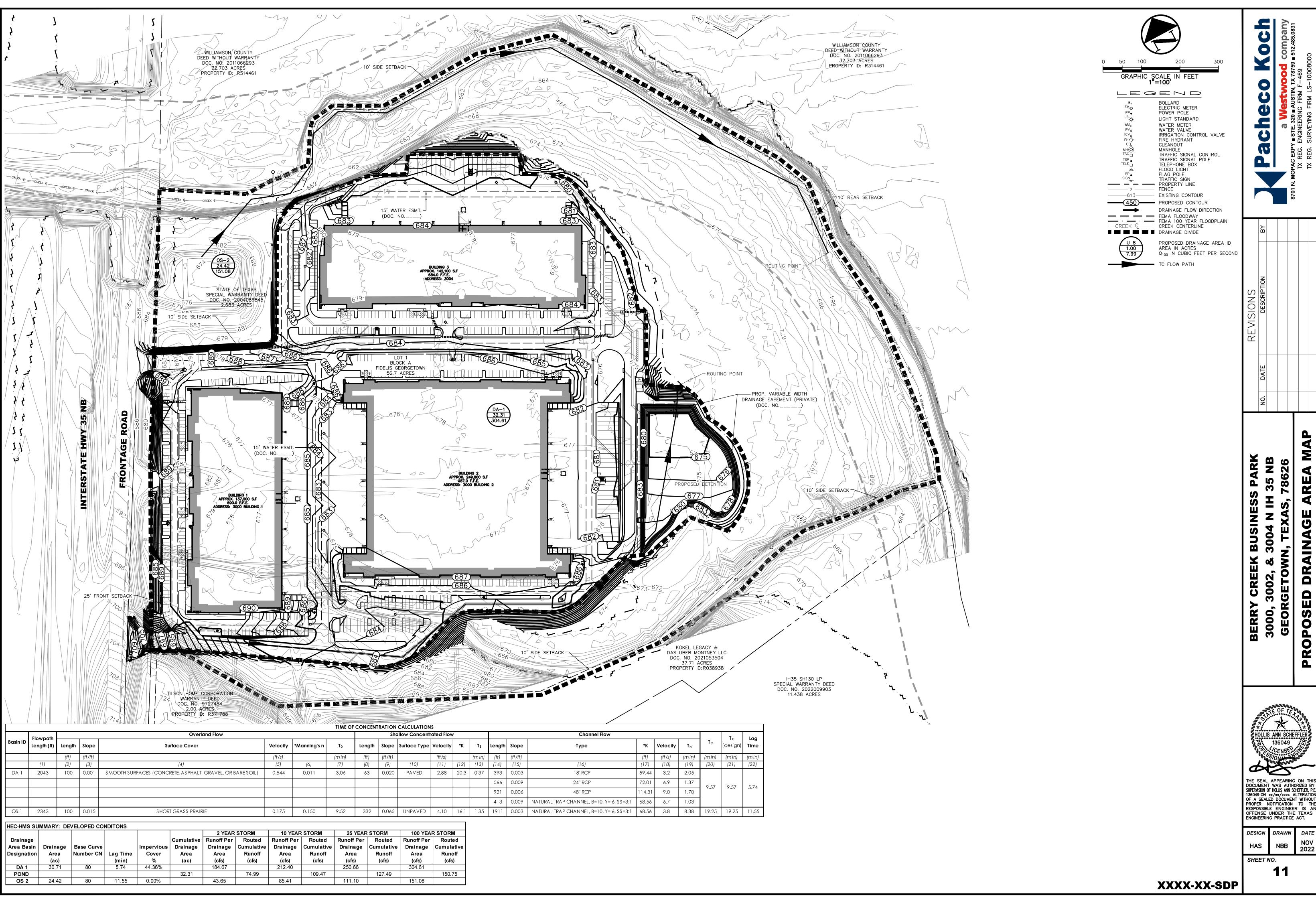
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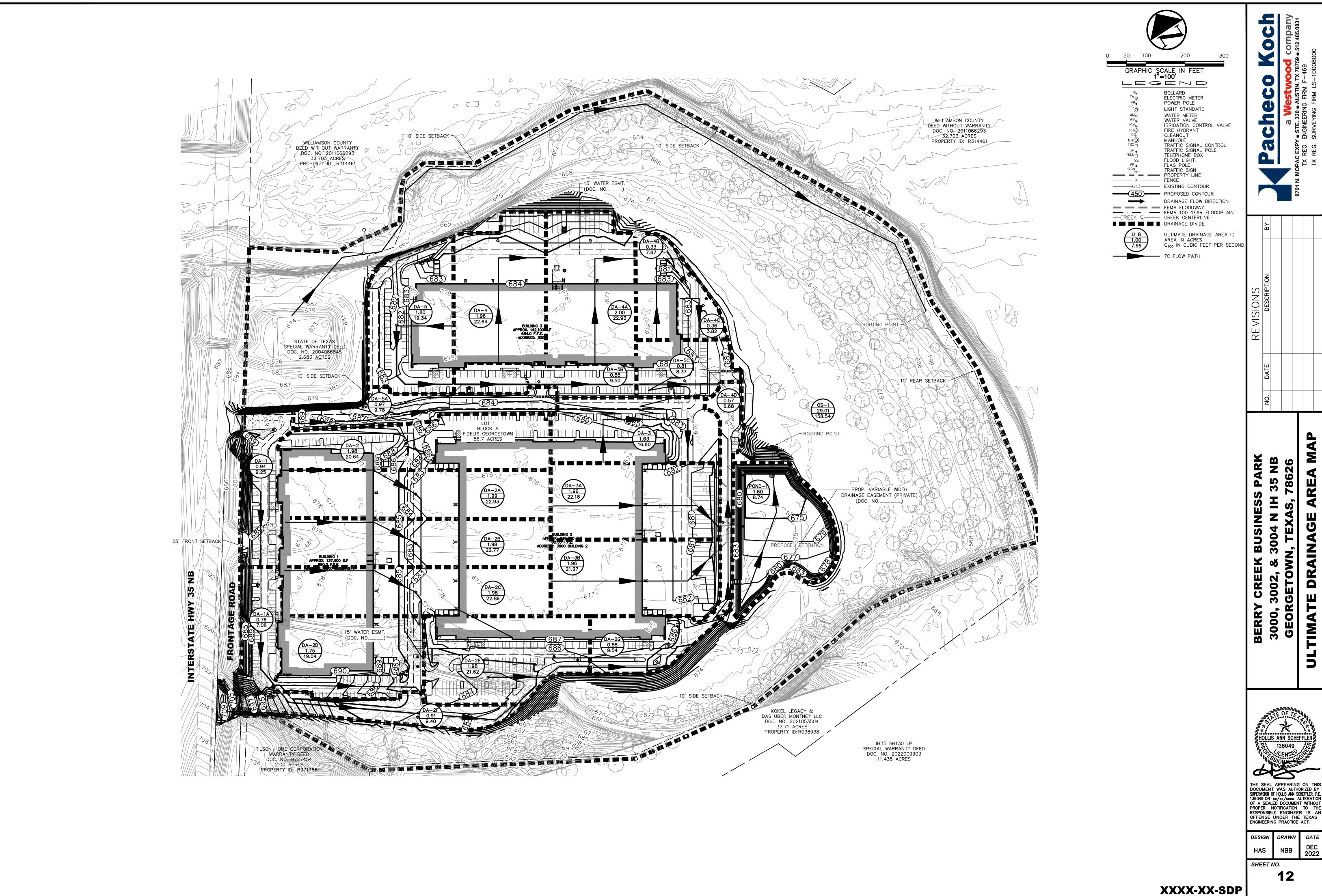
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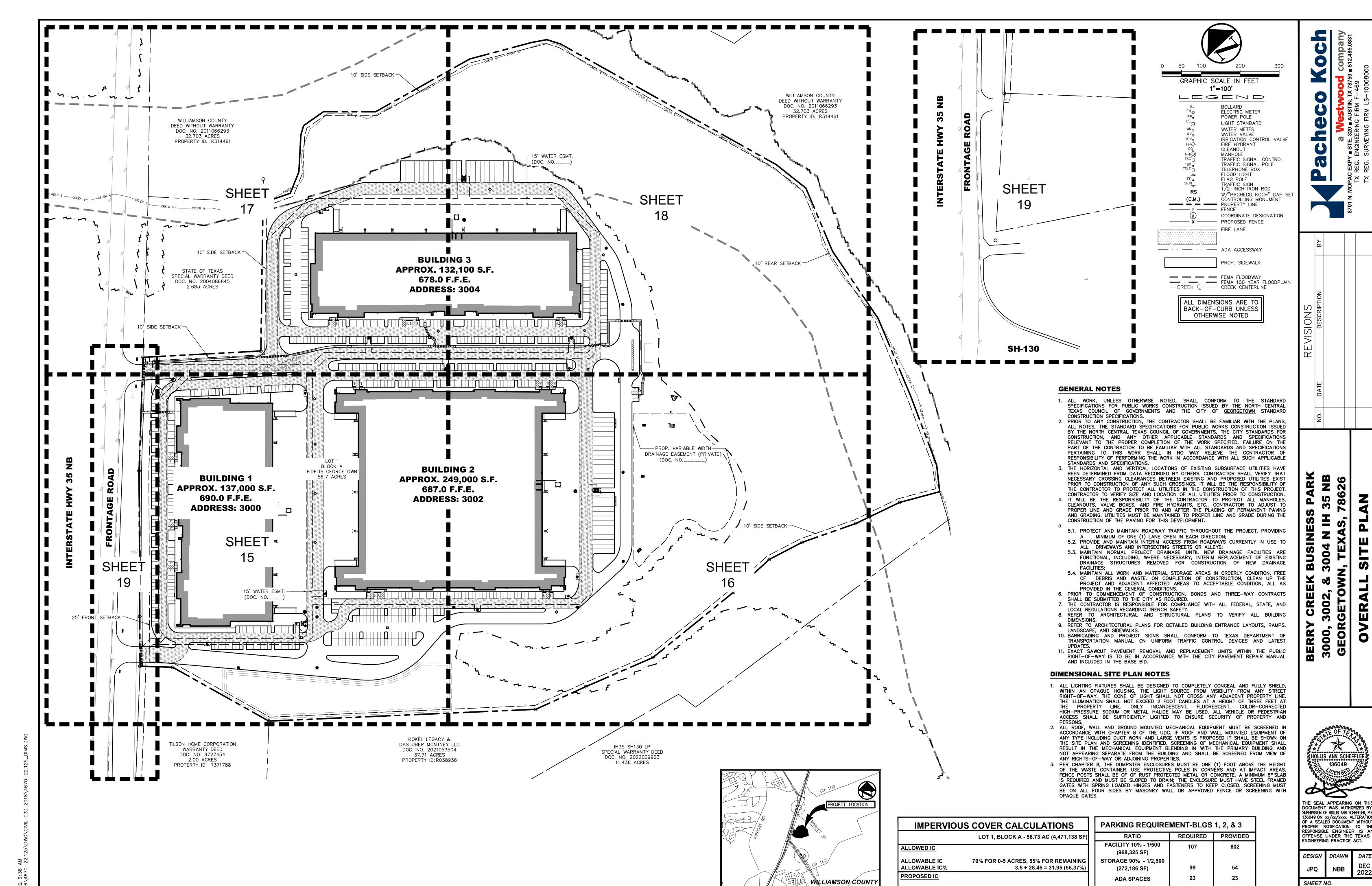
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Drainage Number	AREA	IC	PC	%IC	pe	rv C	im	p C	weighted C	рє	perv C imp C weighted C		pei	v C	imp C		weighted C		perv C		np C	weighted C							
ULTIMATE CONDITION	N DRAINAGE	AREAS																											
DA 1	0.94	0.694	0.2477	73.71%	0.31	0.08	0.97	0.67	0.80	0.36	0.09	0.97	0.67	0.81	0.39	0.10	0.97	0.67	0.82	0.46	0.11	0.97	0.67	0.84					
DA1A	0.76	0.516	0.2492	67.42%	0.31	0.08	0.97	0.50	0.75	0.36	0.09	0.97	0.50	0.77	0.39	0.10	0.97	0.50	0.78	0.46	0.11	0.97	0.50	0.80					
DA 2	1.98	1.782	0.2010	89.86%	0.31	0.06	0.97	1.73	0.90	0.36	0.07	0.97	1.73	0.91	0.39	0.08	0.97	1.73	0.91	0.46	0.09	0.97	1.73	0.92					
DA 2A	1.99	1.990	0.0000	100.00%	0.31	0.00	0.97	1.93	0.97	0.36	0.00	0.97	1.93	0.97	0.39	0.00	0.97	1.93	0.97	0.46	0.00	0.97	1.93	0.97					
DA 2B	1.98	1.976	0.0000	100.00%	0.31	0.00	0.97	1.92	0.97	0.36	0.00	0.97	1.92	0.97	0.39	0.00	0.97	1.92	0.97	0.46	0.00	0.97	1.92	0.97					
DA 2C	1.98	1.984	0.0000	100.00%	0.31	0.00	0.97	1.92	0.97	0.36	0.00	0.97	1.92	0.97	0.39	0.00	0.97	1.92	0.97	0.46	0.00	0.97	1.92	0.97					
DA 2D	1.75	1.638	0.1155	93.41%	0.31	0.04	0.97	1.59	0.93	0.36	0.04	0.97	1.59	0.93	0.39	0.05	0.97	1.59	0.93	0.46	0.05	0.97	1.59	0.94					
DA 2E	1.98	1.782	0.1979	90.00%	0.31	0.06	0.97	1.73	0.90	0.36	0.07	0.97	1.73	0.91	0.39	0.08	0.97	1.73	0.91	0.46	0.09	0.97	1.73	0.92					
DA 2F	0.91	0.728	0.1838	79.85%	0.31	0.06	0.97	0.71	0.84	0.36	0.07	0.97	0.71	0.85	0.39	0.07	0.97	0.71	0.85	0.46	0.08	0.97	0.71	0.87					
DA 2G	0.88	0.780	0.1006	88.58%	0.31	0.03	0.97	0.76	0.89	0.36	0.04	0.97	0.76	0.90	0.39	0.04	0.97	0.76	0.90	0.46	0.05	0.97	0.76	0.91					
DA 3	1.63	1.269	0.3605	77.88%	0.31	0.11	0.97	1.23	0.82	0.36	0.13	0.97	1.23	0.84	0.39	0.14	0.97	1.23	0.84	0.46	0.17	0.97	1.23	0.86					
DA 3A	1.96	1.893	0.0683	96.52%	0.31	0.02	0.97	1.84	0.95	0.36	0.02	0.97	1.84	0.95	0.39	0.03	0.97	1.84	0.95	0.46	0.03	0.97	1.84	0.95					
DA 3B	1.96	1.856	0.1071	94.54%	0.31	0.03	0.97	1.80	0.93	0.36	0.04	0.97	1.80	0.94	0.39	0.04	0.97	1.80	0.94	0.46	0.05	0.97	1.80	0.94					
DA 4	1.96	1.964	0.0000	100.00%	0.31	0.00	0.97	1.91	0.97	0.36	0.00	0.97	1.91	0.97	0.39	0.00	0.97	1.91	0.97	0.46	0.00	0.97	1.91	0.97					
DA 4A	2.00	1.983	0.0138	99.31%	0.31	0.00	0.97	1.92	0.97	0.36	0.00	0.97	1.92	0.97	0.39	0.01	0.97	1.92	0.97	0.46	0.01	0.97	1.92	0.97					
DA 4B	0.68	0.649	0.0338	95.04%	0.31	0.01	0.97	0.63	0.94	0.36	0.01	0.97	0.63	0.94	0.39	0.01	0.97	0.63	0.94	0.46	0.02	0.97	0.63	0.94					
DA 4C	0.36	0.305	0.0561	84.45%	0.31	0.02	0.97	0.30	0.87	0.36	0.02	0.97	0.30	0.88	0.39	0.02	0.97	0.30	0.88	0.46	0.03	0.97	0.30	0.89					
DA 4D	0.57	0.557	0.0148	97.41%	1.31	0.02	0.00	0.00	0.03	1.36	0.02	0.97	0.54	0.98	1.39	0.02	0.97	0.54	0.98	1.46	0.02	0.97	0.54	0.98					
DA 5	1.80	1.572	0.2237	87.54%	0.31	0.07	0.97	1.52	0.89	0.36	0.08	0.97	1.52	0.89	0.39	0.09	0.97	1.52	0.90	0.46	0.10	0.97	1.52	0.91					
DA 5A	0.97	0.741	0.2268	76.57%	0.31	0.07	0.97	0.72	0.82	0.36	0.08	0.97	0.72	0.83	0.39	0.09	0.97	0.72	0.83	0.46	0.10	0.97	0.72	0.85					
DA 5B	0.85	0.802	0.0453	94.65%	0.31	0.01	0.97	0.78	0.93	0.36	0.02	0.97	0.78	0.94	0.39	0.02	0.97	0.78	0.94	0.46	0.02	0.97	0.78	0.94					
DA 5C	0.81	0.654	0.1530	81.04%	0.31	0.05	0.97	0.63	0.84	0.36	0.06	0.97	0.63	0.85	0.39	0.06	0.97	0.63	0.86	0.46		0.97	0.63	0.87					
POND-1	1.60	0.000	1.5996	0.00%	0.31	0.50	0.97	0.00	0.31	0.36	0.58	0.97	0.00	0.36	0.39	0.62	0.97	0.00	0.39	0.46		0.97	0.00	0.46					
O\$-1	29.01	0.000	29.0100	0.00%	0.31	8.99	0.97	0.00	0.31	0.36	10.44	0.97	0.00	0.36	0.39	11.31	0.97	0.00	0.39	0.46	13.34	0.97	0.00	0.46					

									SHEET FLOW	V			<u> </u>	SH	ALLOW CONCE	NTRATED FLO	W		1		CHANI	NEL FLOW			1		INTE	NSITY		<u> </u>	DISC!	HARGE	
DRAINAGE	INLET	AREA	C ₂	C ₁₀	C ₂₅	C ₁₀₀	Length	Slope	Surface Cover	Velocity	Manning's	T _{sheet}	Length	Slope	Surface Type	Velocity	K	T _{shallow}	Length	Slope	Туре	K (ft)	Velocity	T _{channel}	Tc (min)	l 2yr	l 10yr	l 25yr	l 100yr	Q 2	Q 10	Q 25	Q 100
NUMBER	NUMBER	(acres)					(ft)	(ft /ft)		(ft /s)	n	(min)	(ft)	(ft /ft)		(ft/s)		(min)	(π)	(ft /ft)		$K = \frac{1.486 \cdot R^{\frac{2}{3}}}{n}$	(π/s)	(min)		(in/hr)	(in/hr)	(in/hr)	(in/hr)	(cfs)	(cfs)	(cfs)	(cfs)
DA 1	DA 1	0.94	0.80	0.81	0.82	0.84	100.00	0.001	CONCRETE	0.40	0.02	4.13	63.13	0.020	PAVED	2.88	20.33	0.36	225.66	0.006	18" RCP	59.44	4.42	0.85	5.35	6.39	8.50	9.72	11.74	4.79	6.48	7.49	9.25
DA1A	DAIA	0.76	0.75	0.77	0.78	0.80	100.00	0.001	CONCRETE	0.38	0.02	4.40	55.58	0.002	PAVED	0.98	20.33	0.94	168.43	0.004	24" RCP	72.01	4.51	0.62	5.96	6.23	8.31	9.52	11.51	3.60	4.90	5.69	7.08
DA 2	DA 2	1.98	0.90	0.91	0.91	0.92	100.00	0.001	CONCRETE	0.35	0.02	4.73	259.10	0.017	PAVED	2.65	20.33	1.63	60.67	0.030	18" RCP	59.44	10.38	0.10	6.46	6.11	8.16	9.36	11.33	10.94	14.71	16.92	20.64
DA 2A	DA 2A	1.99	0.97	0.97	0.97	0.97	100.00	0.065	CONCRETE	1.87	0.02	0.89	25.28	0.061	PAVED	5.00	20.33	0.08	68.74	0.218	24" RCP	72.01	33.64	0.03	5.00	6.48	8.62	9.84	11.88	12.51	16.63	18.99	22.93
DA 2B	DA 2B	1.98	0.97	0.97	0.97	0.97	100.00	0.065	CONCRETE	1.87	0.02	0.89	25.28	0.061	PAVED	5.00	20.33	0.08	68.74	0.218	24" RCP	72.01	33.64	0.03	5.00	6.48	8.62	9.84	11.88	12.42	16.51	18.86	22.77
DA 2C	DA 2C	1.98	0.97	0.97	0.97	0.97	100.00	0.065	CONCRETE	1.87	0.02	0.89	25.28	0.061	PAVED	5.00	20.33	0.08	68.74	0.218	30" RCP	83.56	39.03	0.03	5.00	6.48	8.62	9.84	11.88	12.47	16.58	18.93	22.86
DA 2D	DA 2D	1.75	0.93	0.93	0.93	0.94	100.00	0.002	CONCRETE	0.44	0.02	3.83	229.75	0.024	PAVED	3.13	20.33	1.22	56.03	0.001	18" RCP	59.44	1.38	0.68	5.73	6.29	8.38	9.59	11.60	10.21	13.67	15.67	19.04
DA 2E	DA 2E	1.98	0.90	0.91	0.91	0.92	100.00	0.004	CONCRETE	0.63	0.02	2.64	172.57	0.015	PAVED	2.46	20.33	1.17	270.58	0.011	48" RCP	114.31	11.83	0.38	5.00	6.48	8.62	9.84	11.88	11.60	15.51	17.77	21.62
DA 2F	DA 2F	0.91	0.84	0.85	0.85	0.87	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	288.00	0.010	24" RCP	72.01	7.20	0.67	5.00	6.48	8.62	9.84	11.88	4.95	6.66	7.65	9.40
DA 2G	DA 2G	0.88	0.89	0.90	0.90	0.91	50.00	0.010	CONCRETE	0.77	0.02	1.08	100.00	0.010	PAVED	2.03	20.33	0.82	100.00	0.011	48" RCP	114.31	11.99	0.14	5.00	6.48	8.62	9.84	11.88	5.11	6.83	7.83	9.54
DA 3	DA 3	1.63	0.82	0.84	0.84	0.86	100.00	0.010	CONCRETE	0.88	0.02	1.88	217.00	0.010	PAVED	2.03	20.33	1.78	107.00	0.010	24" RCP	72.01	7.20	0.25	5.00	6.48	8.62	9.84	11.88	8.70	11.73	13.50	16.60
DA 3A	DA 3A	1.96	0.95	0.95	0.95	0.95	100.00	0.010	CONCRETE	0.88	0.02	1.88	150.00	0.010	PAVED	2.03	20.33	1.23	201.00	0.010	24" RCP	72.01	7.20	0.47	5.00	6.48	8.62	9.84	11.88	12.03	16.03	18.33	22.18
DA 3B	DA 3B	1.96	0.93	0.94	0.94	0.94	100.00	0.010	CONCRETE	0.88	0.02	1.88	200.00	0.010	PAVED	2.03	20.33	1.64	149.00	0.010	18" RCP	59.44	5.94	0.42	5.00	6.48	8.62	9.84	11.88	11.88	15.84	18.12	21.97
DA 4	DA 4	1.96	0.97	0.97	0.97	0.97	50.00	0.010	CONCRETE	0.77	0.02	1.08	150.00	0.010	PAVED	2.03	20.33	1.23	189.00	0.010	18" RCP	59.44	5.94	0.53	5.00	6.48	8.62	9.84	11.88	12.34	16.41	18.75	22.64
DA 4A	DA 4A	2.00	0.97	0.97	0.97	0.97	50.00	0.010	CONCRETE	0.77	0.02	1.08	150.00	0.010	PAVED	2.03	20.33	1.23	173.00	0.010	24" RCP	72.01	7.20	0.40	5.00	6.48	8.62	9.84	11.88	12.49	16.61	18.98	22.93
DA 4B	DA 4B	0.68	0.94	0.94	0.94	0.94	50.00	0.010	CONCRETE	0.77	0.02	1.08	150.00	0.010	PAVED	2.03	20.33	1.23	108.00	0.010	24" RCP	72.01	7.20	0.25	5.00	6.48	8.62	9.84	11.88	4.15	5.53	6.33	7.67
DA 4C	DA 4C	0.36	0.87	0.88	0.88	0.89	50.00	0.010	CONCRETE	0.77	0.02	1.08	50.00	0.010	PAVED	2.03	20.33	0.41	50.00	0.010	30" RCP	83.56	8.36	0.10	5.00	6.48	8.62	9.84	11.88	2.03	2.72	3.13	3.82
DA 4D	DA 4D	0.57	0.03	0.95	0.98	0.98	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	240.00	0.010	42" RCP	104.57	10.46	0.38	5.00	6.48	8.62	9.84	11.88	0.13	4.68	5.52	6.68
DA 5	DA 5	1.80	0.89	0.89	0.90	0.91	50.00	0.010	CONCRETE	0.77	0.02	1.08	100.00	0.010	PAVED	2.03	20.33	0.82	280.00	0.010	18" RCP	59.44	5.94	0.79	5.00	6.48	8.62	9.84	11.88	10.33	13.83	15.86	19.34
DA 5A	DA 5A	0.97	0.82	0.83	0.83	0.85	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	175.00	0.010	18" RCP	59.44	5.94	0.49	5.00	6.48	8.62	9.84	11.88	5.12	6.90	7.95	9.78
DA 5B	DA 5B	0.85	0.93	0.94	0.94	0.94	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	212.00	0.010	30" RCP	83.56	8.36	0.42	5.00	6.48	8.62	9.84	11.88	5.13	6.85	7.83	9.50
DA 5C	DA 5C	0.81	0.84	0.85	0.86	0.87	50.00	0.010	CONCRETE	0.77	0.02	1.08	100.00	0.010	PAVED	2.03	20.33	0.82	54.00	0.010	30" RCP	83.56	8.36	0.11	5.00	6.48	8.62	9.84	11.88	4.42	5.94	6.83	8.37
POND-1	POND-1	1.60	0.31	0.36	0.39	0.46	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	94.00	0.010	48" RCP	114.31	11.43	0.14	5.00	6.48	8.62	9.84	11.88	3.21	4.96	6.14	8.74
OS-1	OS-1	29.01	0.31	0.36	0.39	0.46	50.00	0.010	CONCRETE	0.77	0.02	1.08	200.00	0.010	PAVED	2.03	20.33	1.64	288.00	0.010	P CHANNEL, B=	FJ 79.71	7.97	0.60	5.00	6.48	8.62	9.84	11.88	58.27	89.98	111.32	158.54

GEORGETOWN IDF CURVE TABLE

TABLE 3-3 INTENSITY-DURATION-FREQUENCY EQUATION COEFFICIENTS²

Storm Frequency	а	b	С
2-year	106.29	16.81	0.9076
5-year	99.75	16.74	0.8327
10-year	96.84	15.88	0.7952
25-year	111.07	17.23	0.7815
50-year	119.51	17.32	0.7705
100-year	129.03	17.83	0.7625
500-year	160.57	19.64	0.7449



XXXX-XX-SDP

12.12 AC (528,100 SF) 21.36%

15.82 AC (689,078 SF) 32.83%

27.94 AC (1,217,178 SF) 54.19%

TOTAL

BUILDINGS 1, 2, & 3

VICINITY MAP

PAVEMENT (Includes sidewalks)

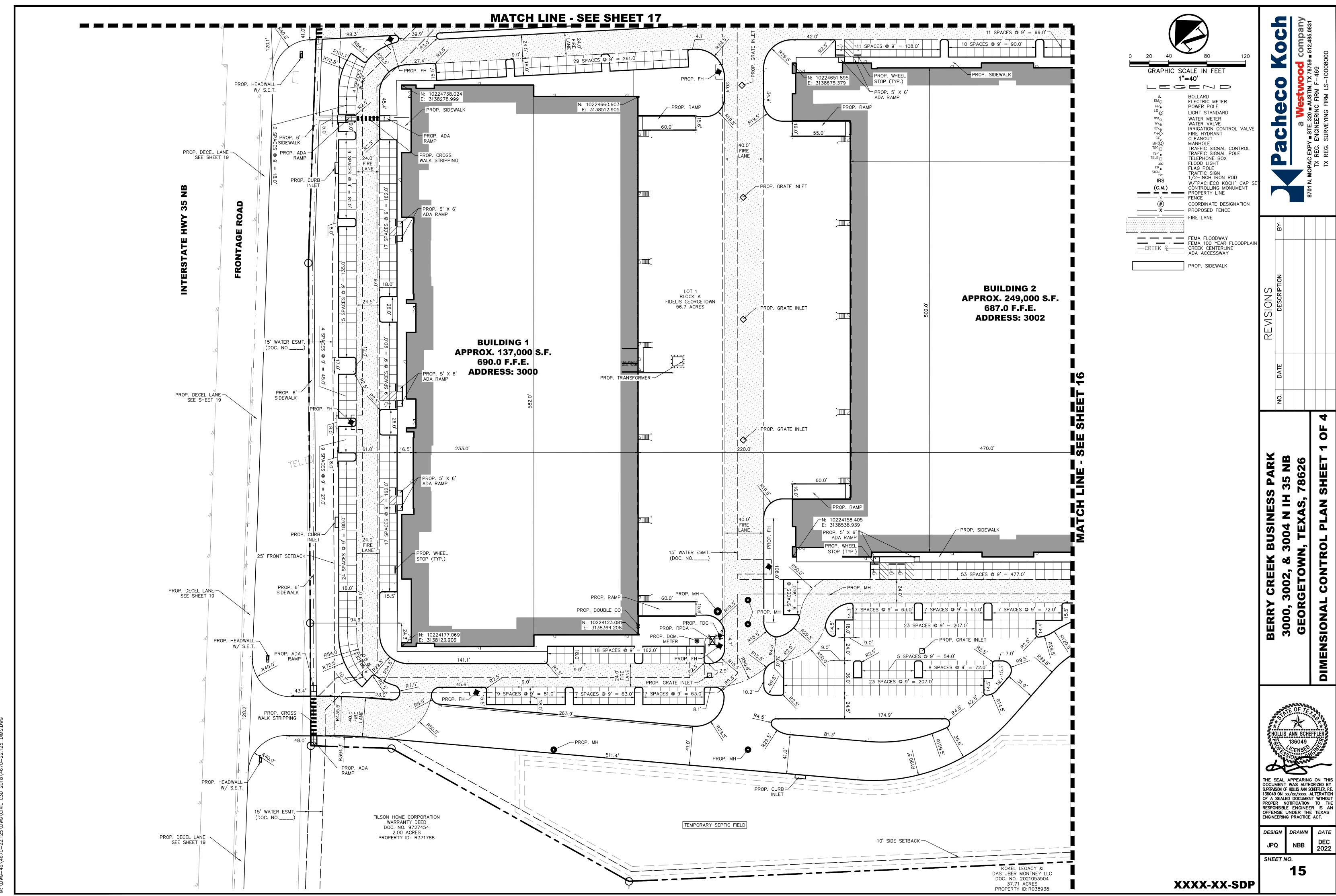
DEC 2022

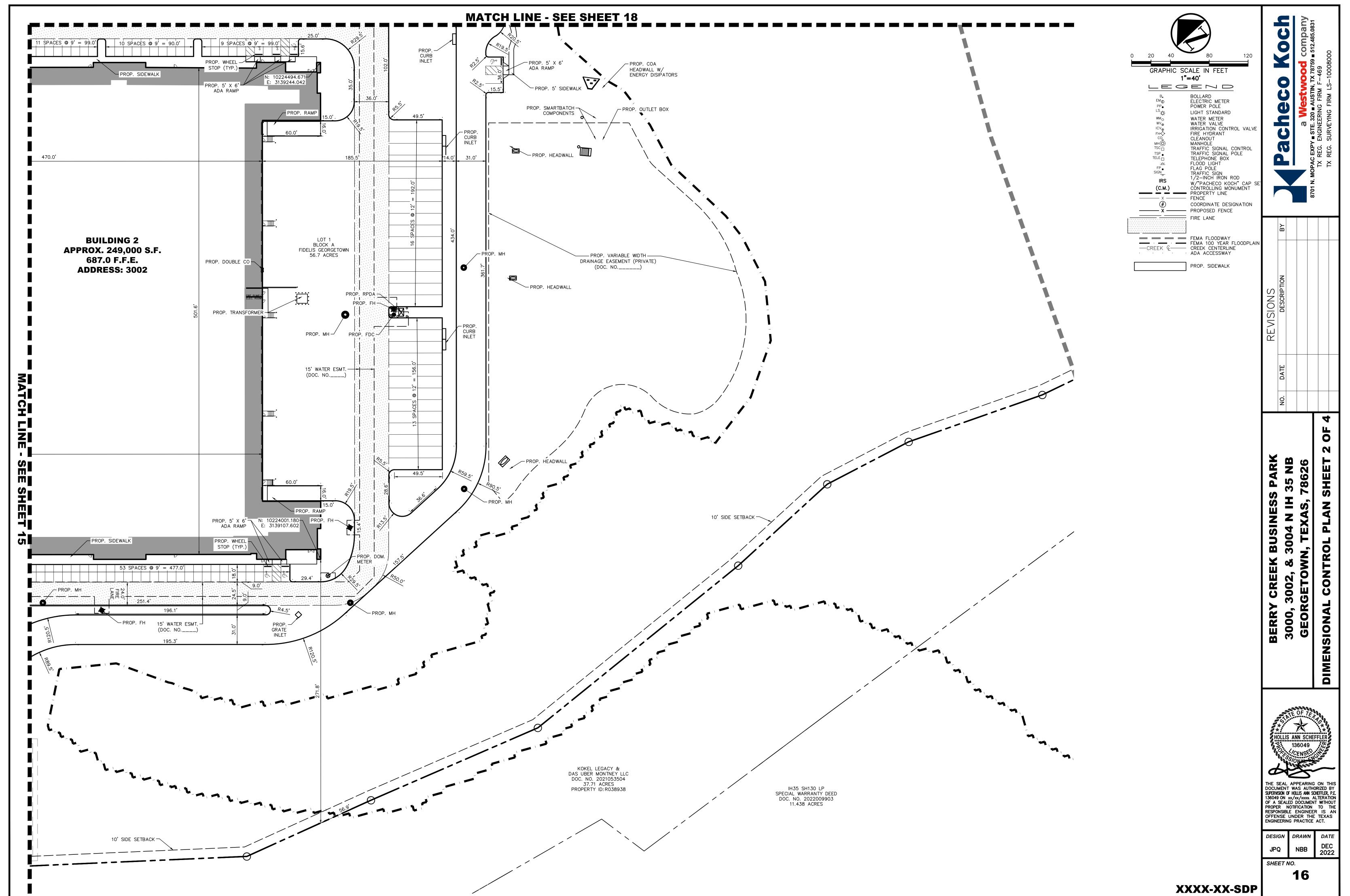
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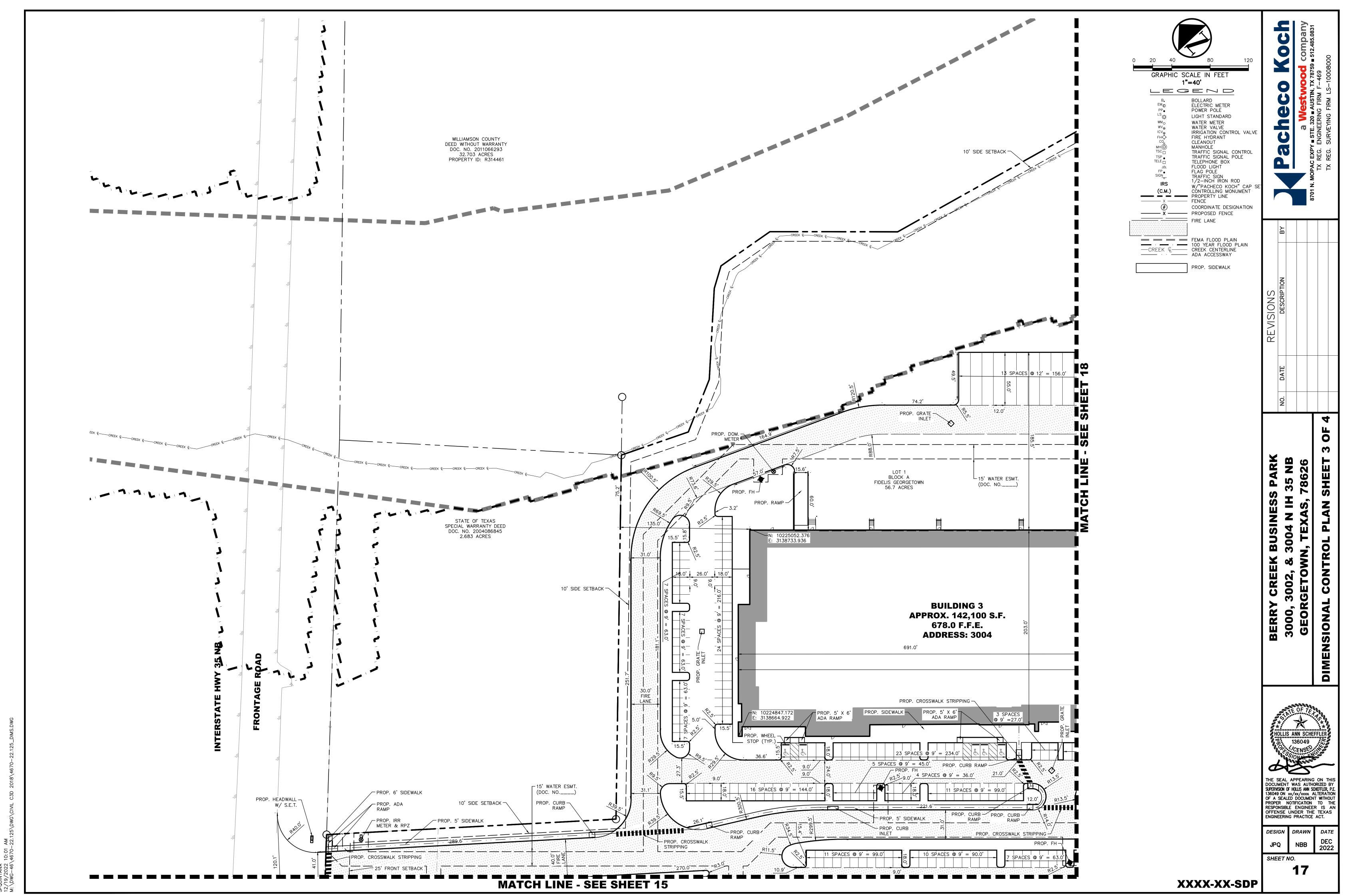
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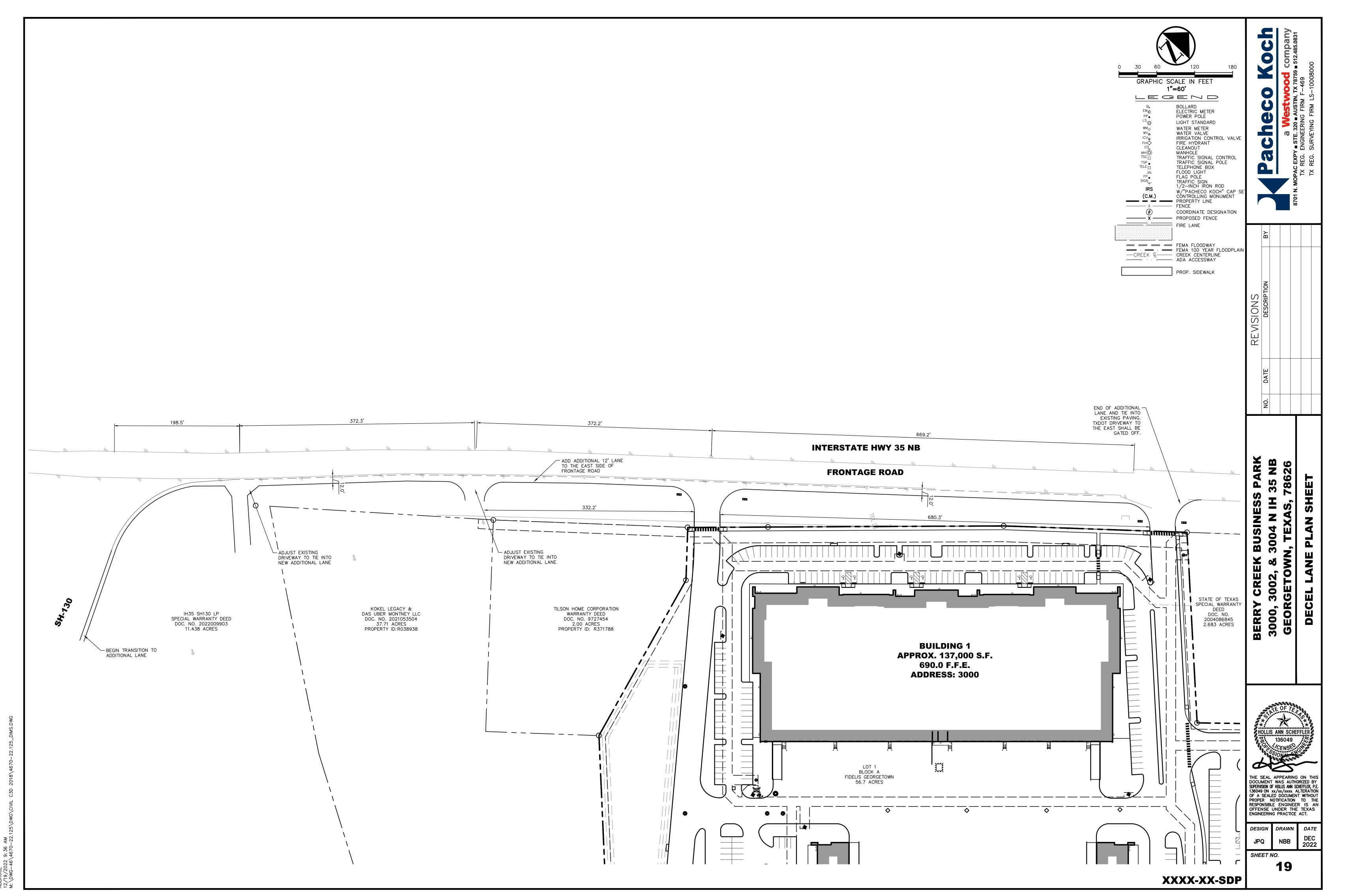
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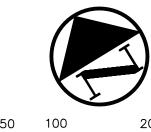


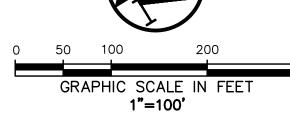




JPQUINTANA 12/19/2022 10:01 AM M·\DWG-46\4670-22 125\DWG\CIVII C3D 2018\4670-







LEGEZD

B. EM. PPP	BOLLARD ELECTRIC METER POWER POLE LIGHT STANDARD WATER METER WATER VALVE IRRIGATION CONTROL VALVE FIRE HYDRANT CLEANOUT MANHOLE TRAFFIC SIGNAL CONTROL TRAFFIC SIGNAL POLE TELEPHONE BOX FLOOD LIGHT FLAG POLE TRAFFIC SIGN 1/2-INCH IRON ROD W/"PACHECO KOCH" CAP SET CONTROLLING MONUMENT PROPERTY LINE FENCE OVERHEAD UTILITY LINE EXIST CONTOUR EXIST SPOT ELEVATION EXIST TOP OF CURB ELEVATION EXIST GUTTER ELEVATION
TC 614.50 G 614.00	PROPOSED CONTOUR PROPOSED TOP OF CURB ELEVATION PROPOSED GUTTER ELEVATION
EL 614.25 TW 620.50 EL 614.00	PROPOSED SPOT ELEVATION PROPOSED TOP OF WALL ELEVATION PROPOSED GROUND ELEVATION AT BOTTOM OF WALL
M.G. * CREEK Q	MATCH EXISTING GRADE PROPOSED SWALE PROPOSED GRADE BREAK PROPOSED DRAINAGE FLOW DIRECTION FEMA FLOODWAY FEMA 100 YEAR FLOODPLAIN CREEK CENTERLINE

GRADING & DRAINAGE GENERAL NOTES

- REFER TO GEOTECHNICAL REPORT FOR REQUIREMENTS REGARDING FILL COMPACTION AND MOISTURE CONTENT.
 UNLESS NOTED, ALL FILL IS TO BE COMPACTED TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY WITHIN 3% OF OPTIMUM MOISTURE CONTENT. FILL TO BE PLACED IN MAXIMUM LIFTS OF 6 INCHES.
 SIDEWALKS AND ACCESSIBLE ROUTES SHALL HAVE A RUNNING SLOPE NO
- GREATER THAN 5% (UNLESS OTHERWISE NOTED) AND A CROSS SLOPE NO GREATER THAN 2%.

 4. GRADING OF ALL HANDICAPPED SPACES AND ROUTES TO CONFORM TO FEDERAL, STATE, AND LOCAL GUIDELINES.

 5. ALL PROPOSED AND EXISTING GRADES IN NON-PAVED AREAS ARE "FINISHED
- GRADE" (i.e. IN LANDSCAPE BEDS, TOP OF MULCH/BEDDING MATERIAL). 6. UNLESS NOTED, STORM DRAIN LINES SHALL BE OF THE FOLLOWING MATERIALS AND INSTALLED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS:
 6.A. RCP C-76, CLASS III
 6.B. ADS N-12
- 6.C. HANCOR HI-Q 6.D. CONTECH ALUMINIZED ULTRA FLOW
- 7. UNLESS NOTED, GRATE INLETS TO BE "FORTERRA PIPE AND PRECAST" CATCH BASIN SIZED AS SHOWN, OR APPROVED EQUAL. 8. FINAL PAVING, CURB, AND SIDEWALK ELEVATIONS WILL BE PLACED AT PLUS OR
- MINUS 0.03 FOOT.

 9. REFER TO LANDSCAPE SPECIFICATIONS FOR SEEDING AND SODDING
- REQUIREMENTS.

 10. ANY CONCRETE, ROCK, OR MATERIAL DEEMED BY THE ENGINEER TO BE
- UNSUITABLE FOR SUBGRADE SHALL BE DISPOSED OF OFFSITE AT CONTRACTOR'S EXPENSE.

 11. TRENCH BACKFILL MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF NCTCOG ITEM 504.2 AND SHALL BE MECHANICALLY COMPACTED IN 6—INCH LIFTS TO THE TOP OF SUBGRADE TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY IN ACCORDANCE WITH NCTCOG ITEM 504.5 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN THE STANDARD CITY SPECIFICATIONS.

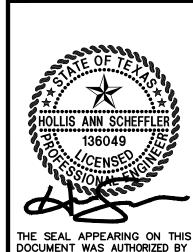
 12. EMBEDMENT SHALL CONFORM TO THE REQUIREMENTS OF NCTCOG ITEM 504.5 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN THE STANDARD CITY SPECIFICATIONS.
- CITY SPECIFICATIONS. 13. A ROUND MANHOLE COVER MEETING CITY SPECIFICATIONS SHALL BE PLACED ALL INLET TOPS NEAR THE OUTLET PIPE.

 14. ALL CONCRETE FOR INLETS AND DRAINAGE STRUCTURES SHALL CONFORM TO A SHALL
- 14. ALL CONCRETE FOR INLETS AND DRAINAGE STRUCTURES SHALL CONFORM TO NCTCOG ITEM 702.2.4, CLASS "A" (3000 PSI) UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN STANDARD CITY SPECIFICATIONS.
 15. CRUSHED STONE BEDDING OR APPROVED EQUAL SHALL BE PROVIDED BY THE CONTRACTOR WHEN ROCK IS ENCOUNTERED IN TRENCHES. THERE SHALL BE NO ADDITIONAL PAY ITEM FOR CRUSHED STONE BEDDING.
 16. IF REQUIRED DUE TO CONSTRUCTION, POWER POLES TO BE BRACED OR RELOCATED AT CONTRACTOR'S EXPENSE.

GRADIN GEORGETOW OVERALL

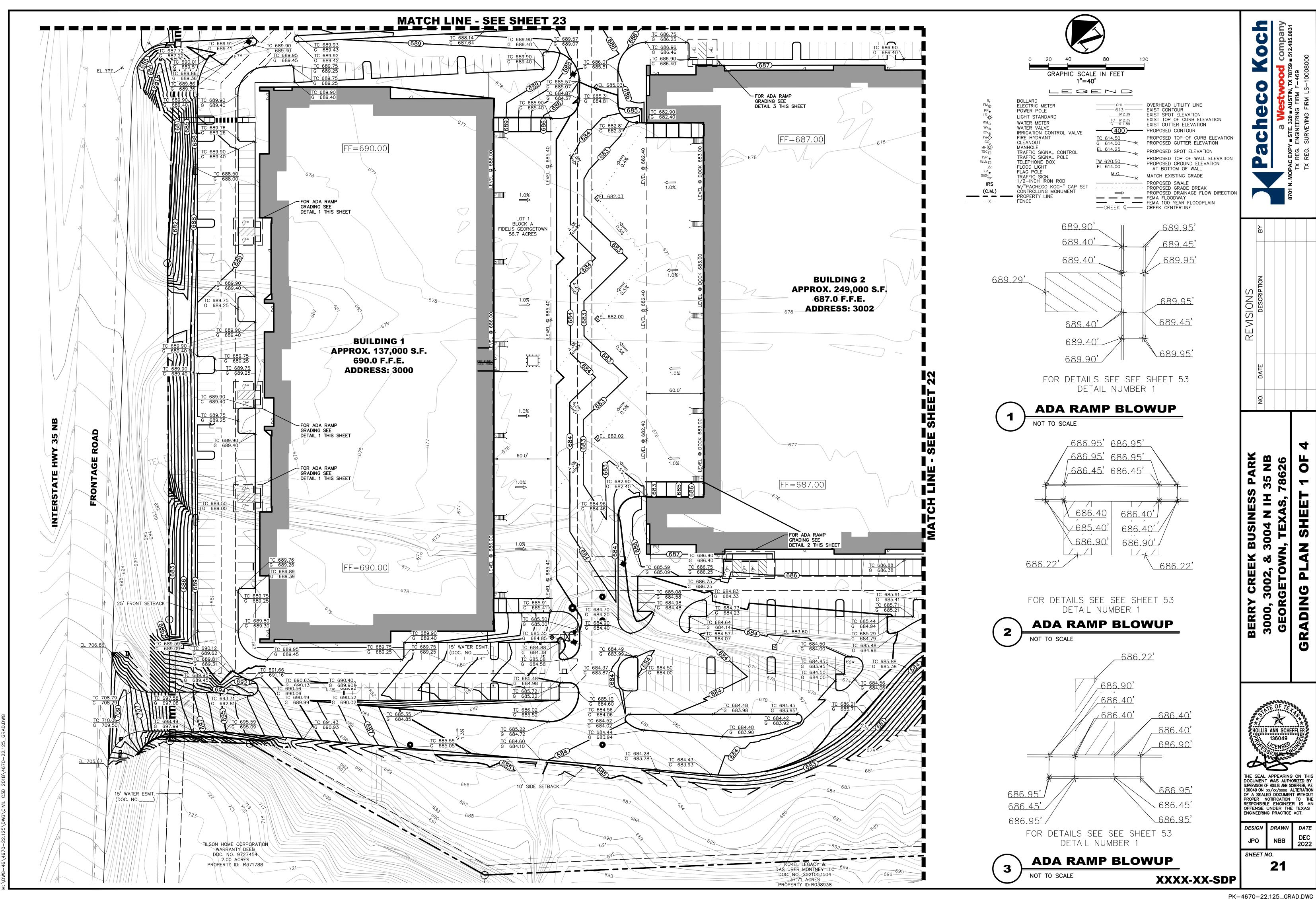
BUSINESS

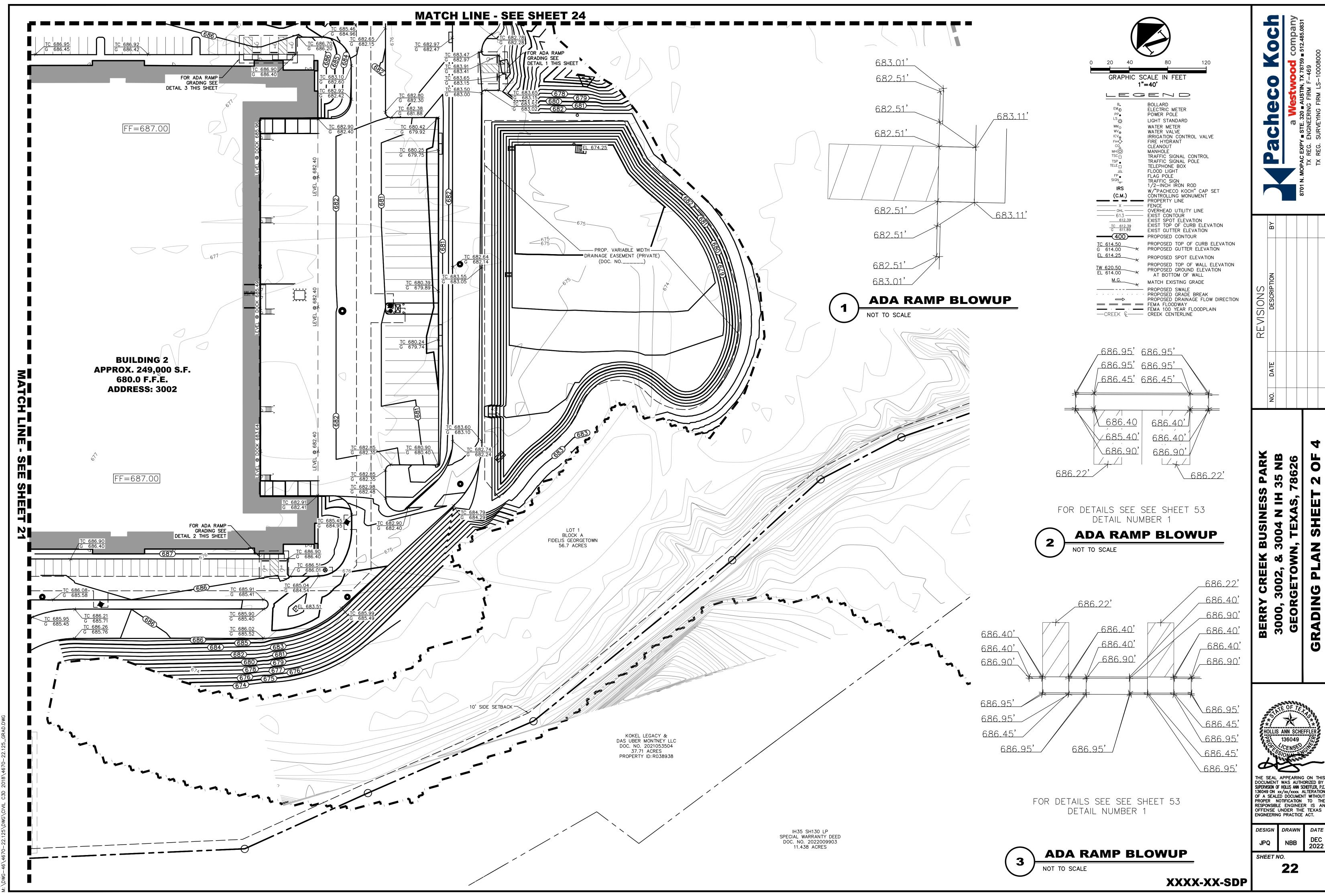
BERRY CREEK

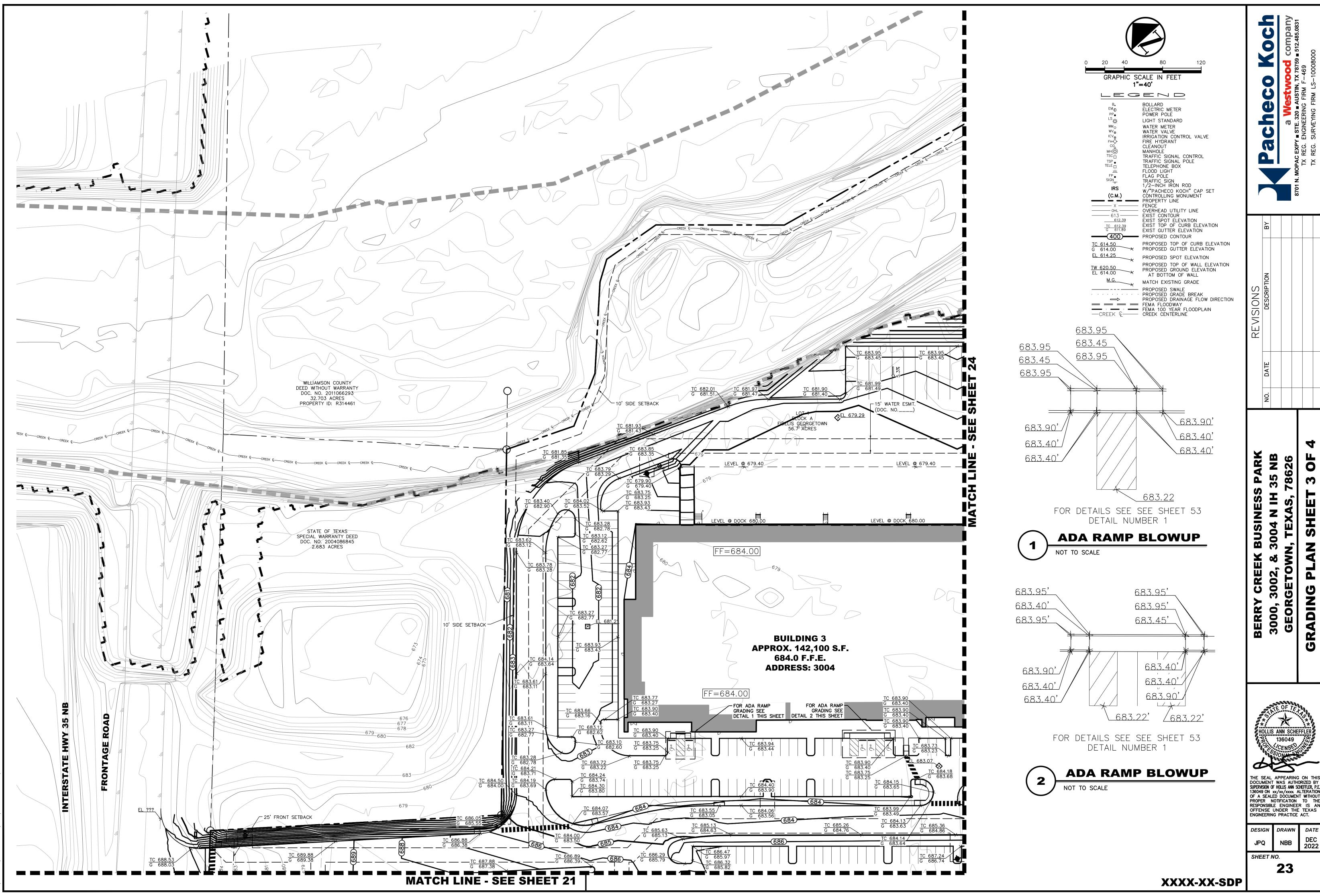


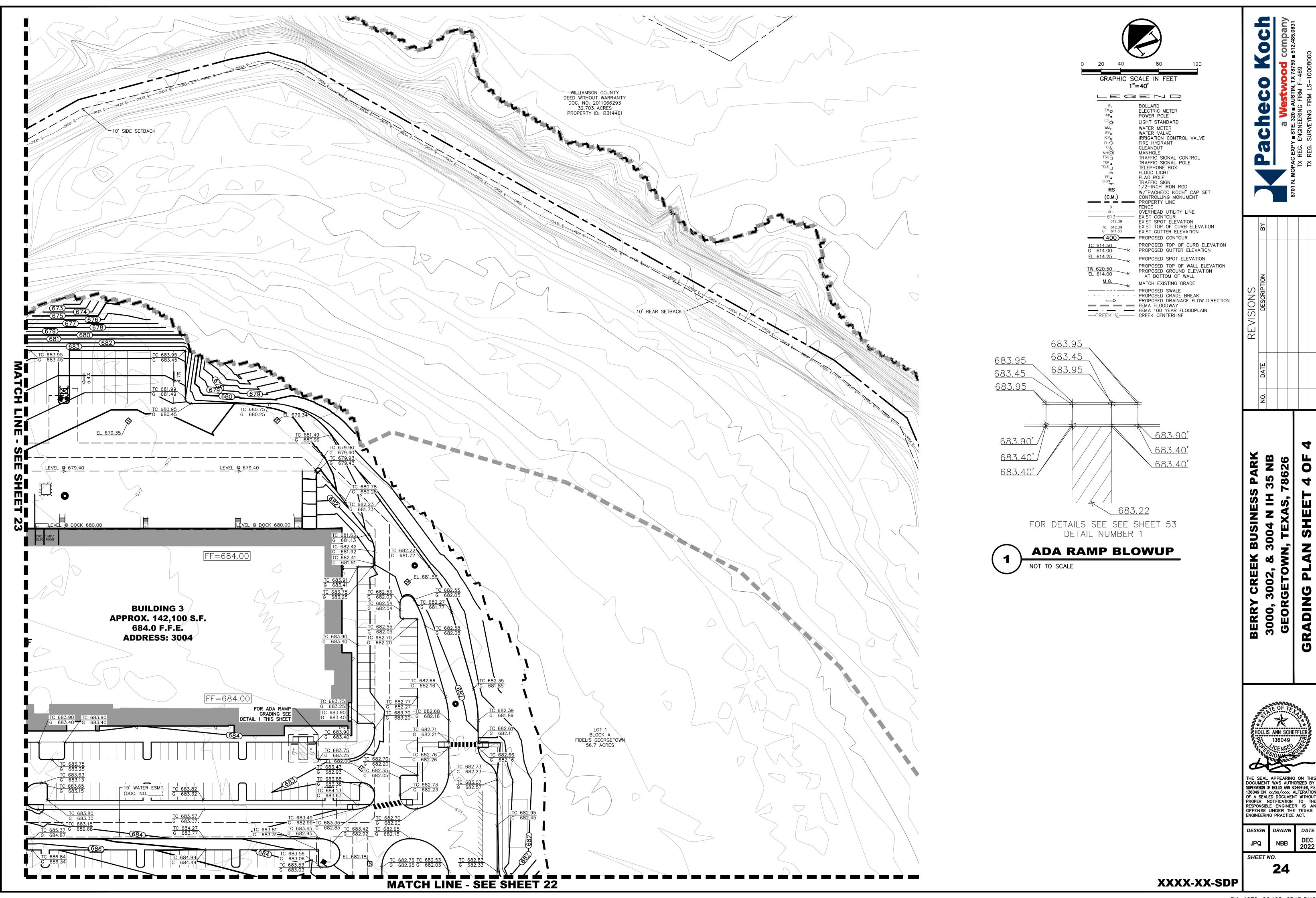
THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY SUPERVISION OF HOLLIS ANN SCHEFFLER, P.E. 136049 ON xx/xx/xxxx ALTERATION OF A SEALED DOCUMENT WITHOUT PROPER NOTIFICATION TO THE RESPONSIBLE ENGINEER IS AN OFFENSE UNDER THE TEXAS ENGINEERING PRACTICE ACT.

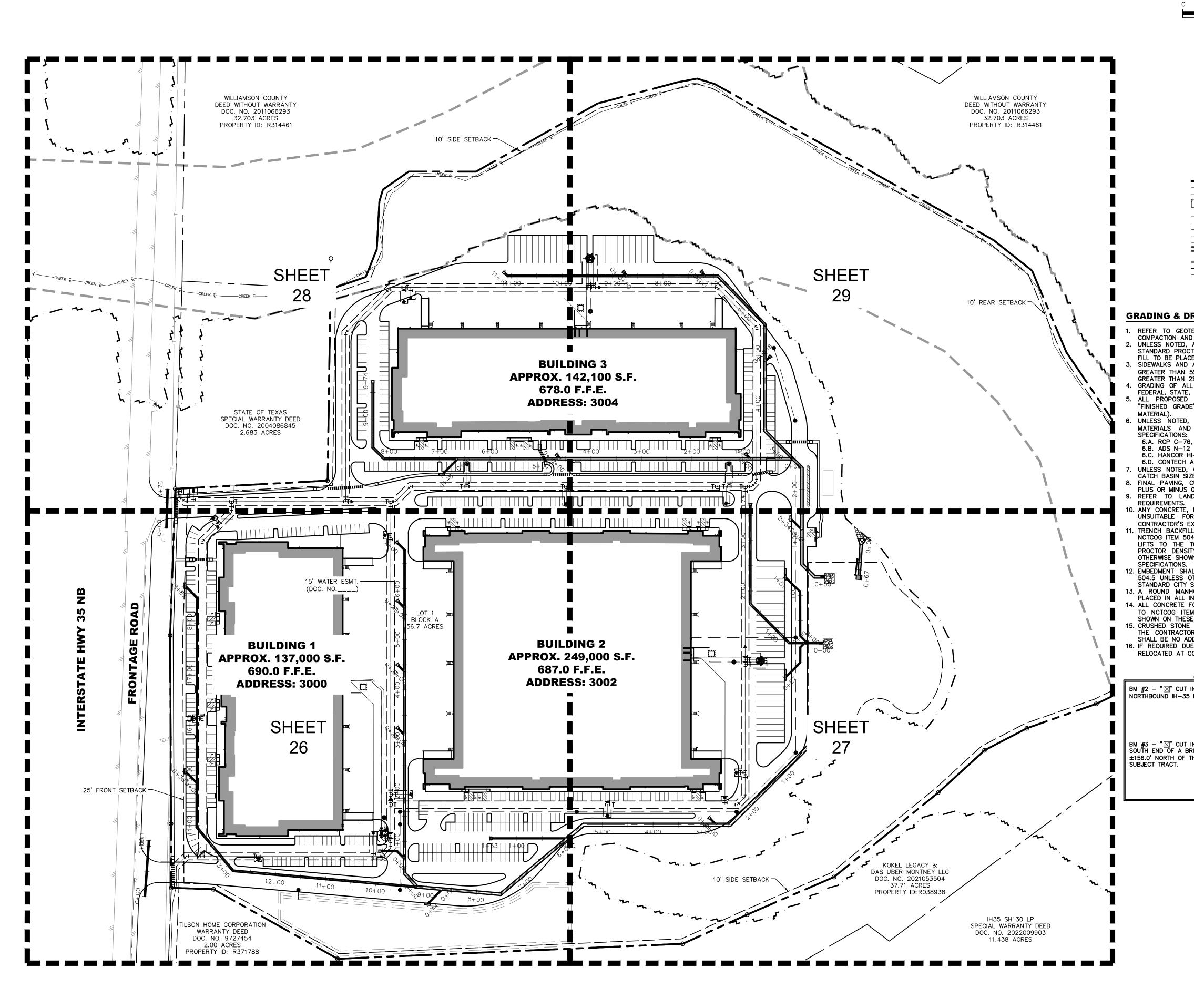
DESIGN DRAWN DEC 2022 JPQ NBB











FIRE HYDRANT CLEANOUT MANHOLE TRAFFIC SIGNAL CONTROL
TRAFFIC SIGNAL POLE TELEPHONE BOX FLOOD LIGHT FLAG POLE TRAFFIC SIGN 1/2-INCH IRON ROD W/"PACHECO KOCH" CAP SET CÓNTROLLING MONUMENT PROPERTY LINE OVERHEAD UTILITY LINE ZOT R.C.P. EXISTING STORM LINE UNDERGROUND ELECTRIC LINE UNDERGROUND TELEPHONE LINE UNDERGROUND CABLE LINE ----UNDERGROUND WATER LINE - — 6"SS — — UNDERGROUND SANITARY SEWER LINE PROPOSED STORM LINE
TOP OF INLET FEMA FLOODWAY
FEMA 100 YEAR FLOODPLAIN

GRADING & DRAINAGE GENERAL NOTES

REFER TO GEOTECHNICAL REPORT FOR REQUIREMENTS REGARDING FILL COMPACTION AND MOISTURE CONTENT. UNLESS NOTED, ALL FILL IS TO BE COMPACTED TO A MINIMUM OF 95%

GRAPHIC SCALE IN FEET 1"=100"

BOLLARD ELECTRIC METER POWER POLE LIGHT STANDARD WATER METER WATER VALVE

IRRIGATION CONTROL VALVE

- STANDARD PROCTOR DENSITY WITHIN 3% OF OPTIMUM MOISTURE CONTENT. FILL TO BE PLACED IN MAXIMUM LIFTS OF 6 INCHES. SIDEWALKS AND ACCESSIBLE ROUTES SHALL HAVE A RUNNING SLOPE NO
- GREATER THAN 5% (UNLESS OTHERWISE NOTED) AND A CROSS SLOPE NO GREATER THAN 2%.
- GRADING OF ALL HANDICAPPED SPACES AND ROUTES TO CONFORM TO FEDERAL, STATE, AND LOCAL GUIDELINES. ALL PROPOSED AND EXISTING GRADES IN NON-PAVED AREAS ARE "FINISHED GRADE" (i.e. IN LANDSCAPE BEDS, TOP OF MULCH/BEDDING
- UNLESS NOTED, STORM DRAIN LINES SHALL BE OF THE FOLLOWING MATERIALS AND INSTALLED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS: 6.A. RCP C-76, CLASS III
 - 6.C. HANCOR HI-Q
 - 6.D. CONTECH ALUMINIZED ULTRA FLOW
- UNLESS NOTED, GRATE INLETS TO BE "FORTERRA PIPE AND PRECAST" CATCH BASIN SIZED AS SHOWN, OR APPROVED EQUAL. FINAL PAVING, CURB, AND SIDEWALK ELEVATIONS WILL BE PLACED AT PLUS OR MINUS 0.03 FOOT.
- REFER TO LANDSCAPE SPECIFICATIONS FOR SEEDING AND SODDING
- 10. ANY CONCRETE, ROCK, OR MATERIAL DEEMED BY THE ENGINEER TO BE UNSUITABLE FOR SUBGRADE SHALL BE DISPOSED OF OFFSITE AT CONTRACTOR'S EXPENSE. I. TRENCH BACKFILL MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF
- NCTCOG ITEM 504.2 AND SHALL BE MECHANICALLY COMPACTED IN 6-INCH LIFTS TO THE TOP OF SUBGRADE TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY IN ACCORDANCE WITH NCTCOG ITEM 504.5 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN THE STANDARD CITY
- 12. EMBEDMENT SHALL CONFORM TO THE REQUIREMENTS OF NCTCOG ITEM 504.5 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN THE STANDARD CITY SPECIFICATIONS.
- 13. A ROUND MANHOLE COVER MEETING CITY SPECIFICATIONS PLACED IN ALL INLET TOPS NEAR THE OUTLET PIPE.

 14. ALL CONCRETE FOR INLETS AND DRAINAGE STRUCTURES SHALL CONFORM
 TO NCTCOG ITEM 702.2.4, CLASS "A" (3000 PSI) UNLESS OTHERWISE
- BUSIN SHOWN ON THESE PLANS OR STATED IN STANDARD CITY SPECIFICATIONS. 15. CRUSHED STONE BEDDING OR APPROVED EQUAL SHALL BE PROVIDED BY THE CONTRACTOR WHEN ROCK IS ENCOUNTERED IN TRENCHES. THERE SHALL BE NO ADDITIONAL PAY ITEM FOR CRUSHED STONE BEDDING.
- 16. IF REQUIRED DUE TO CONSTRUCTION, POWER POLES TO BE BRACED OR RELOCATED AT CONTRACTOR'S EXPENSE.

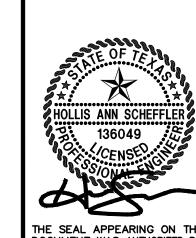
BENCHMARK LIST

BM #2 - "\sum CUT IN SOUTHEASTERLY EDGE OF CONCRETE OF THE NORTHBOUND IH-35 FRONTAGE ROAD, ±23.5' NORTHWEST OF A MAILBOX.

N: 10,224,602.34' E: 3,138,083.91 ELEV: 688.94'

BM #3 - "\sum " CUT IN A CONCRETE MOW STRIP BEHIND A GUARDRAIL AT THE SOUTH OF A BRIDGE ON THE NORTHBOUND IH-35 FRONTAGE ROAD, ±156.0' NORTH OF THE WESTERLY-MOST NORTHWEST PROPERTY CORNER OF

N: 10,225,018.96' E: 3,138,218.52' ELEV: 686.78'



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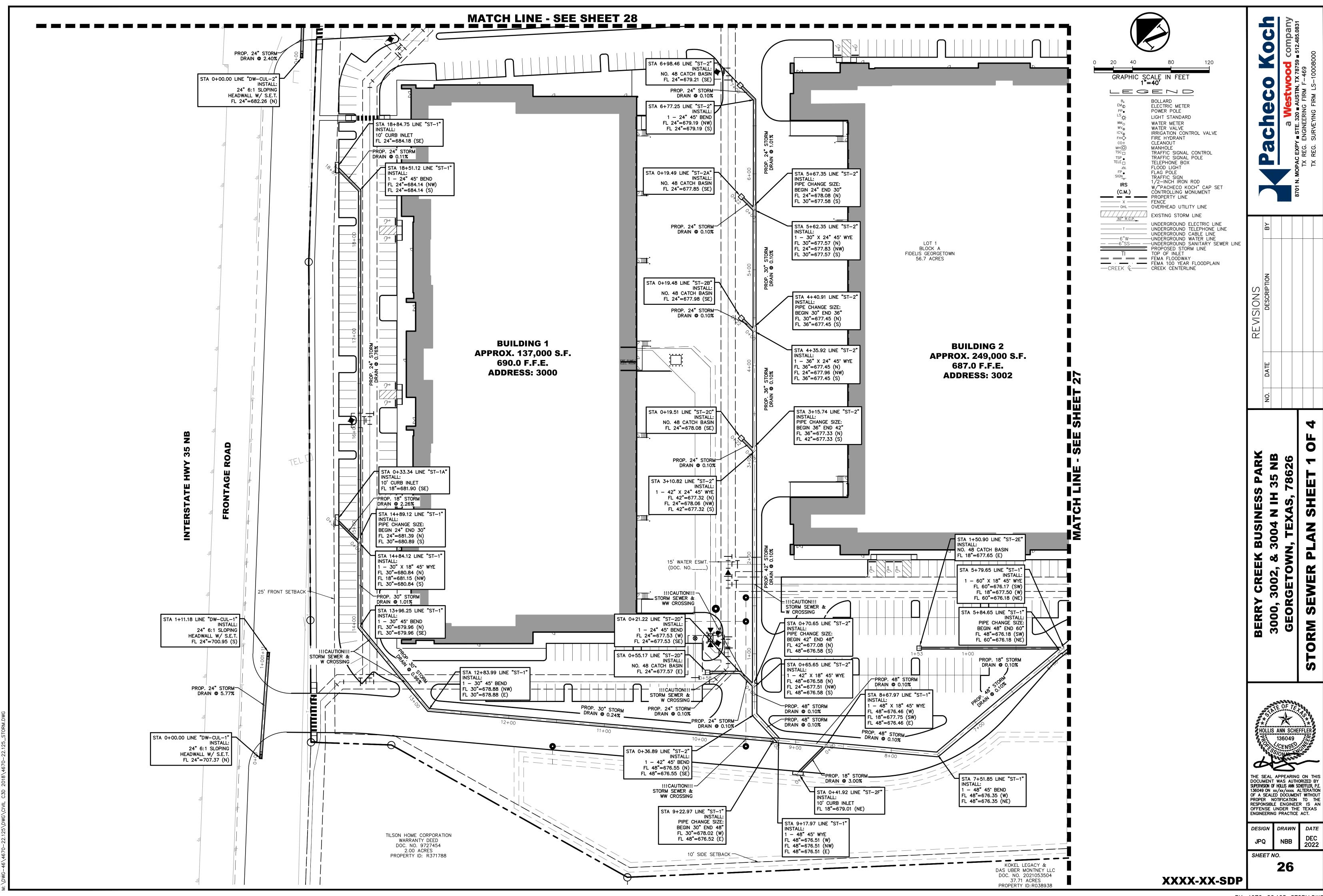
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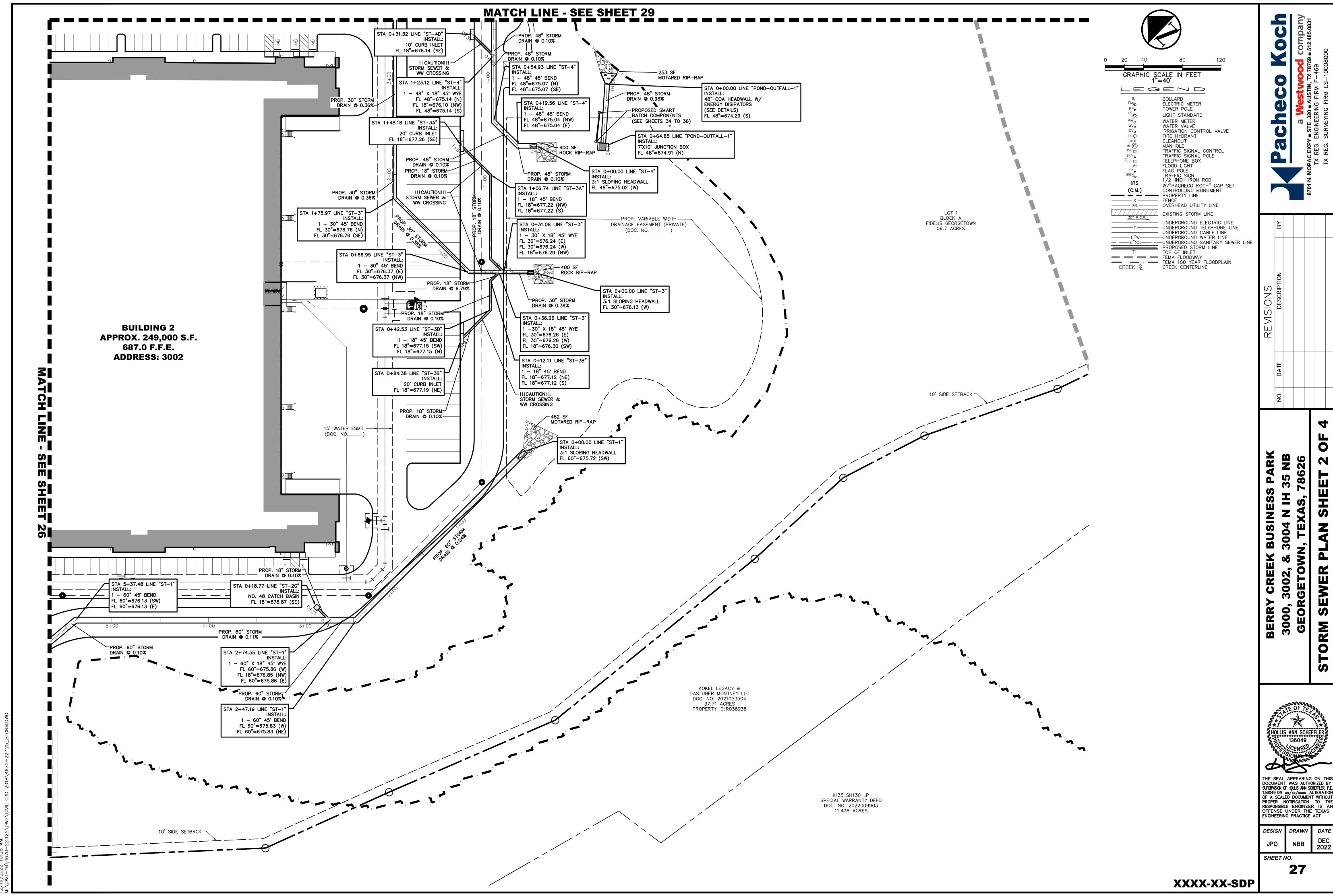
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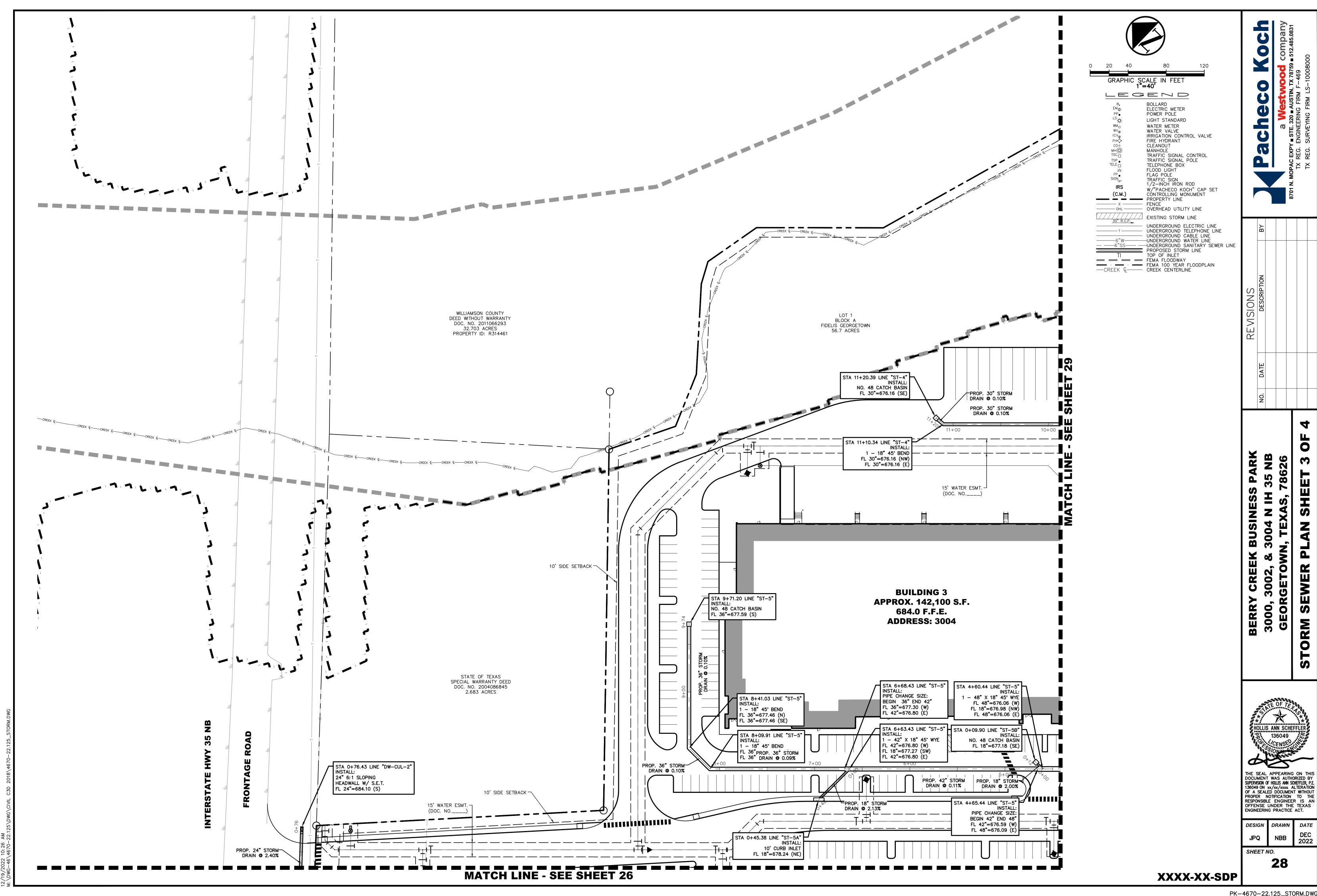
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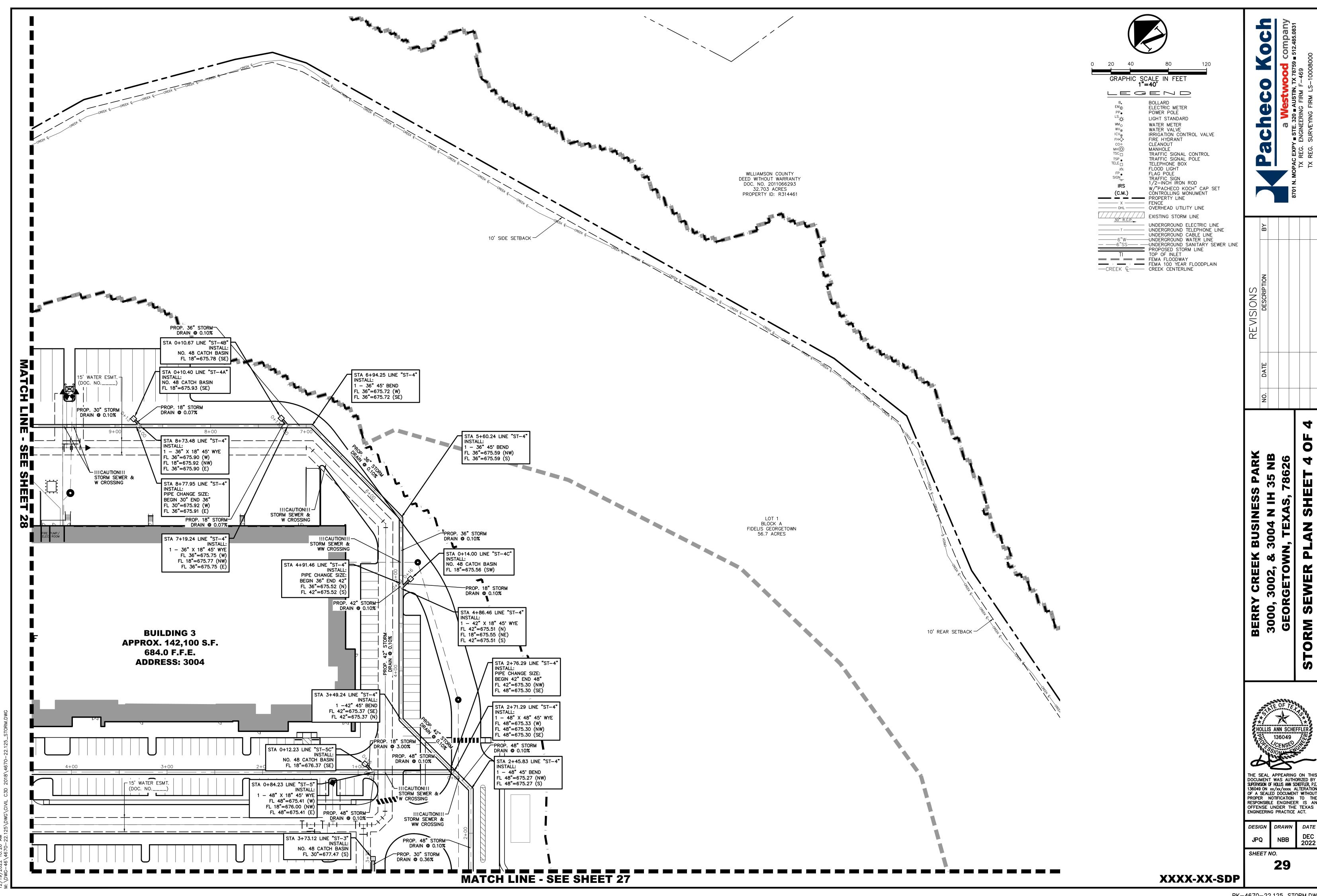
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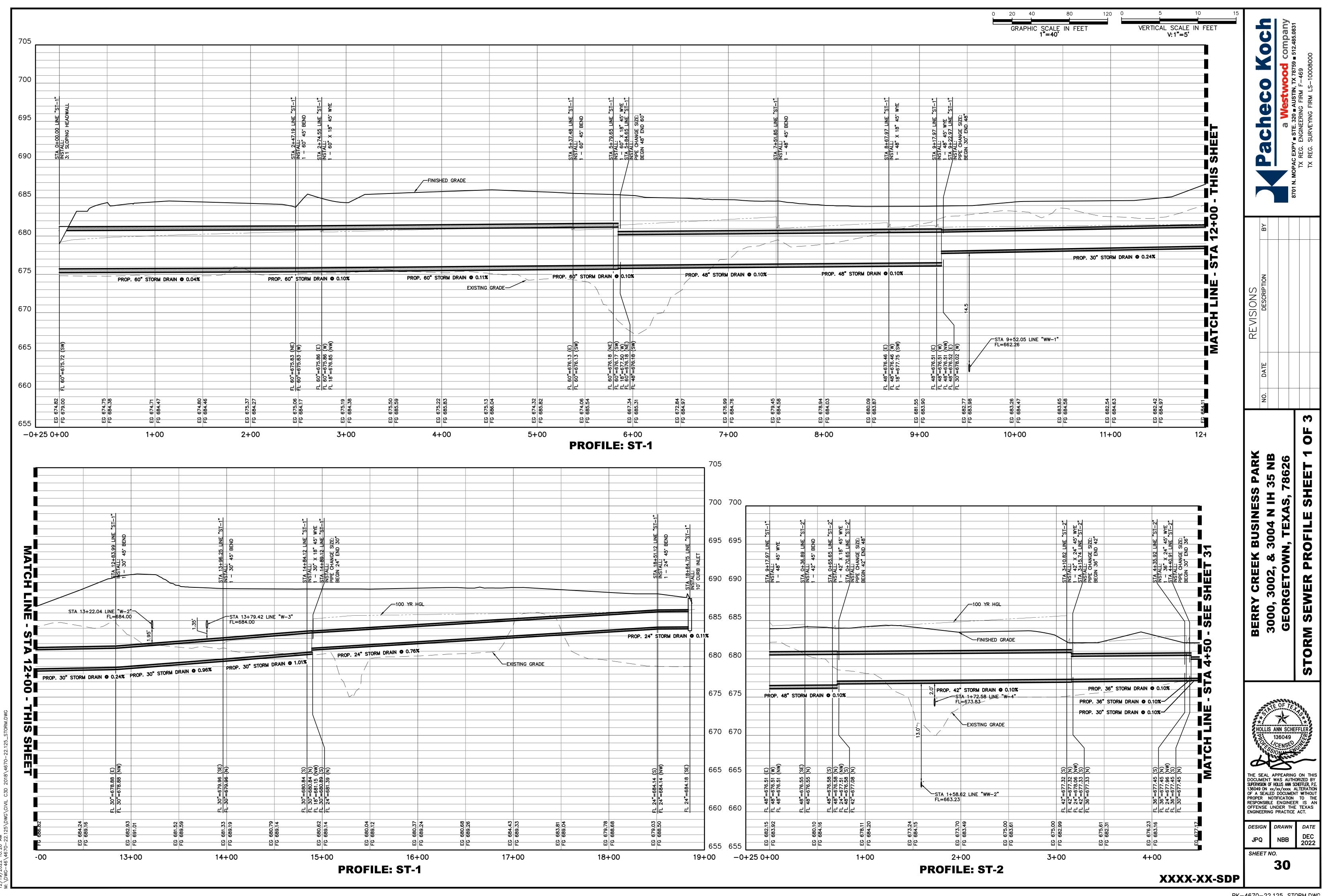
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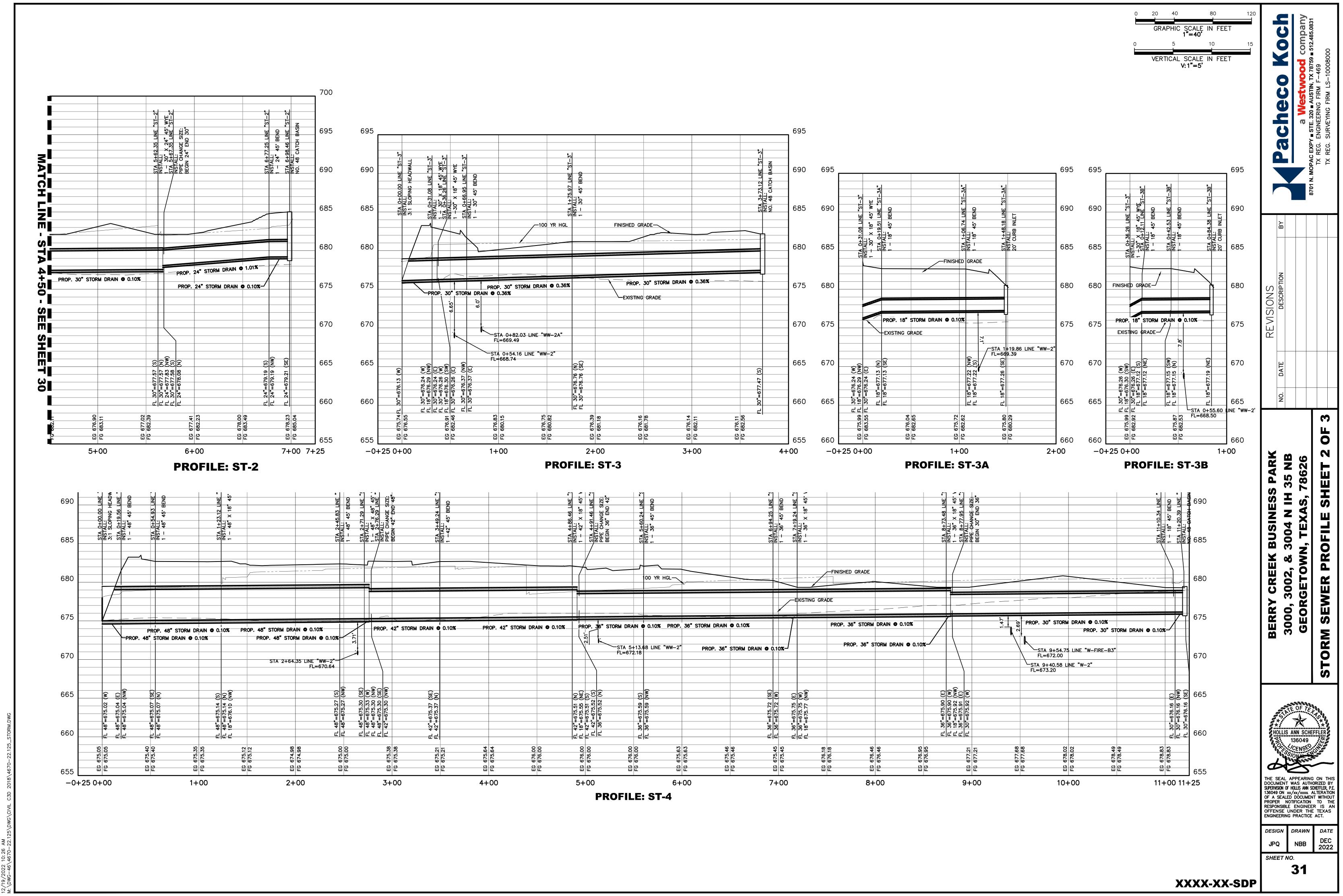


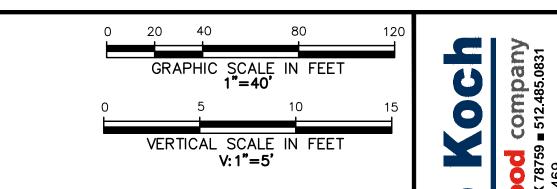










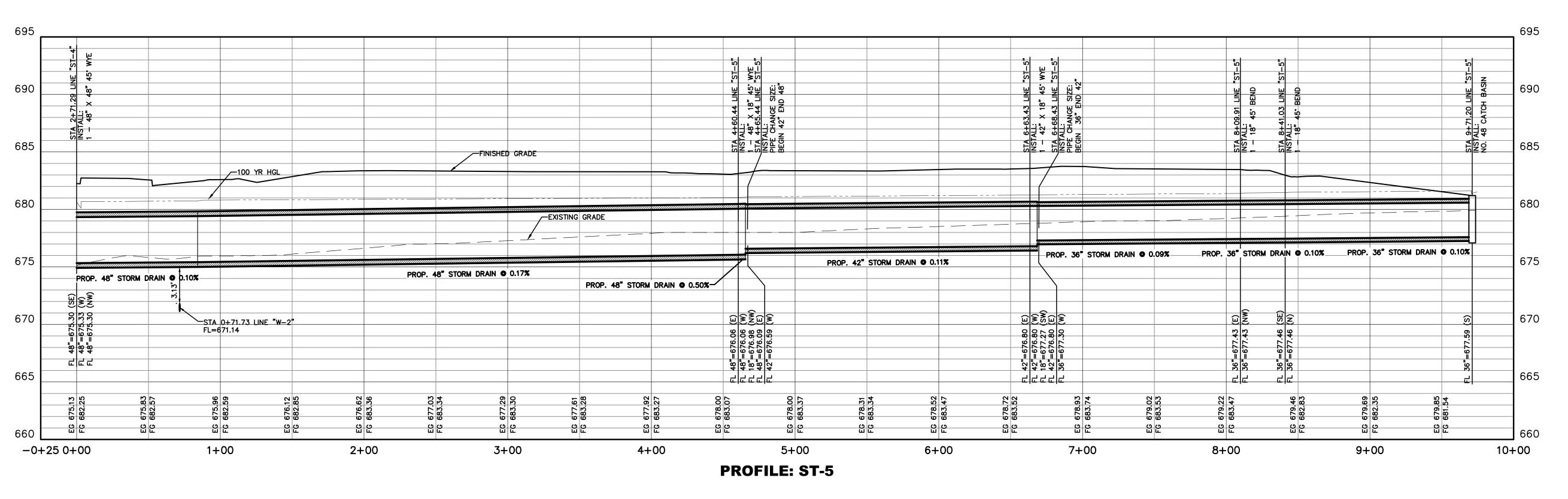


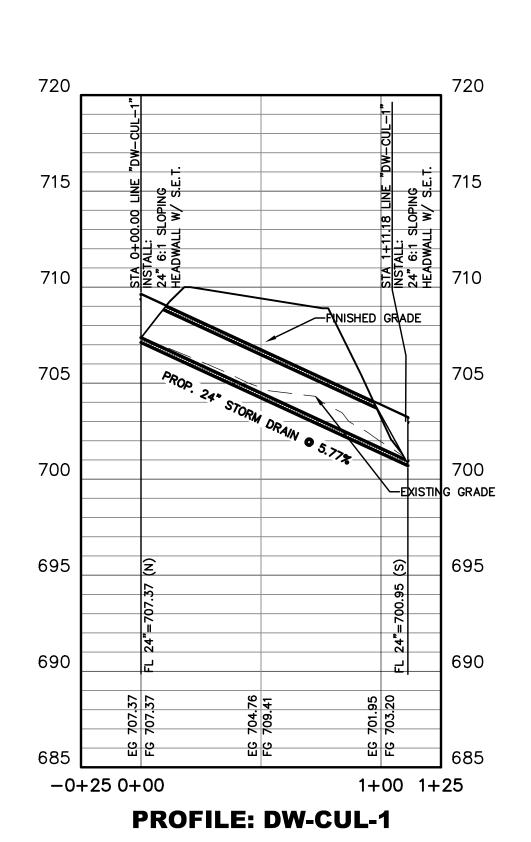
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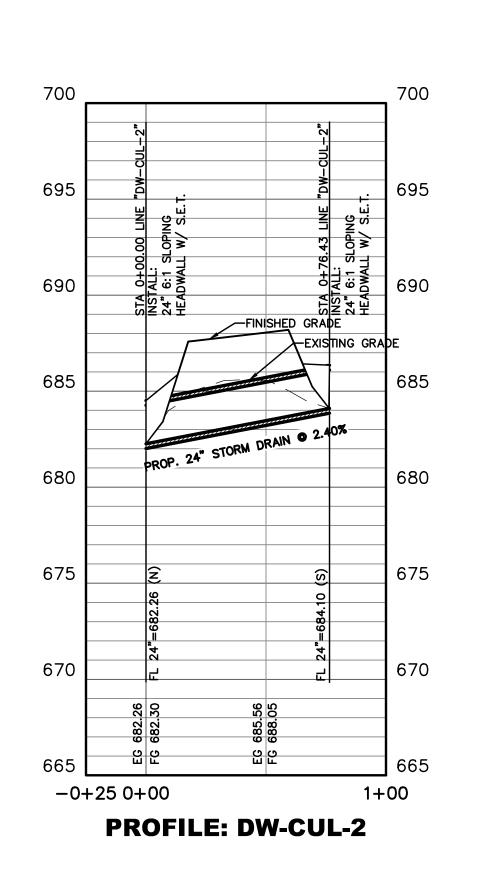
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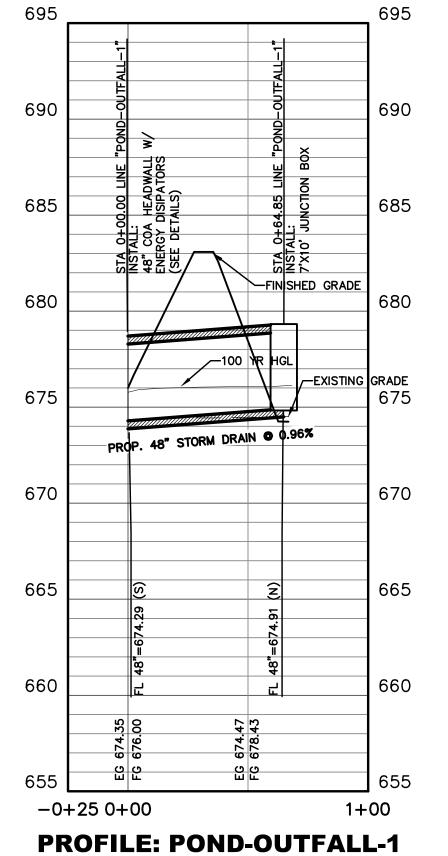
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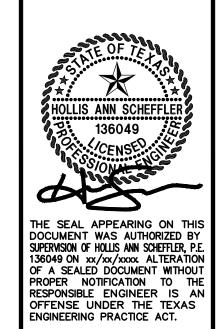


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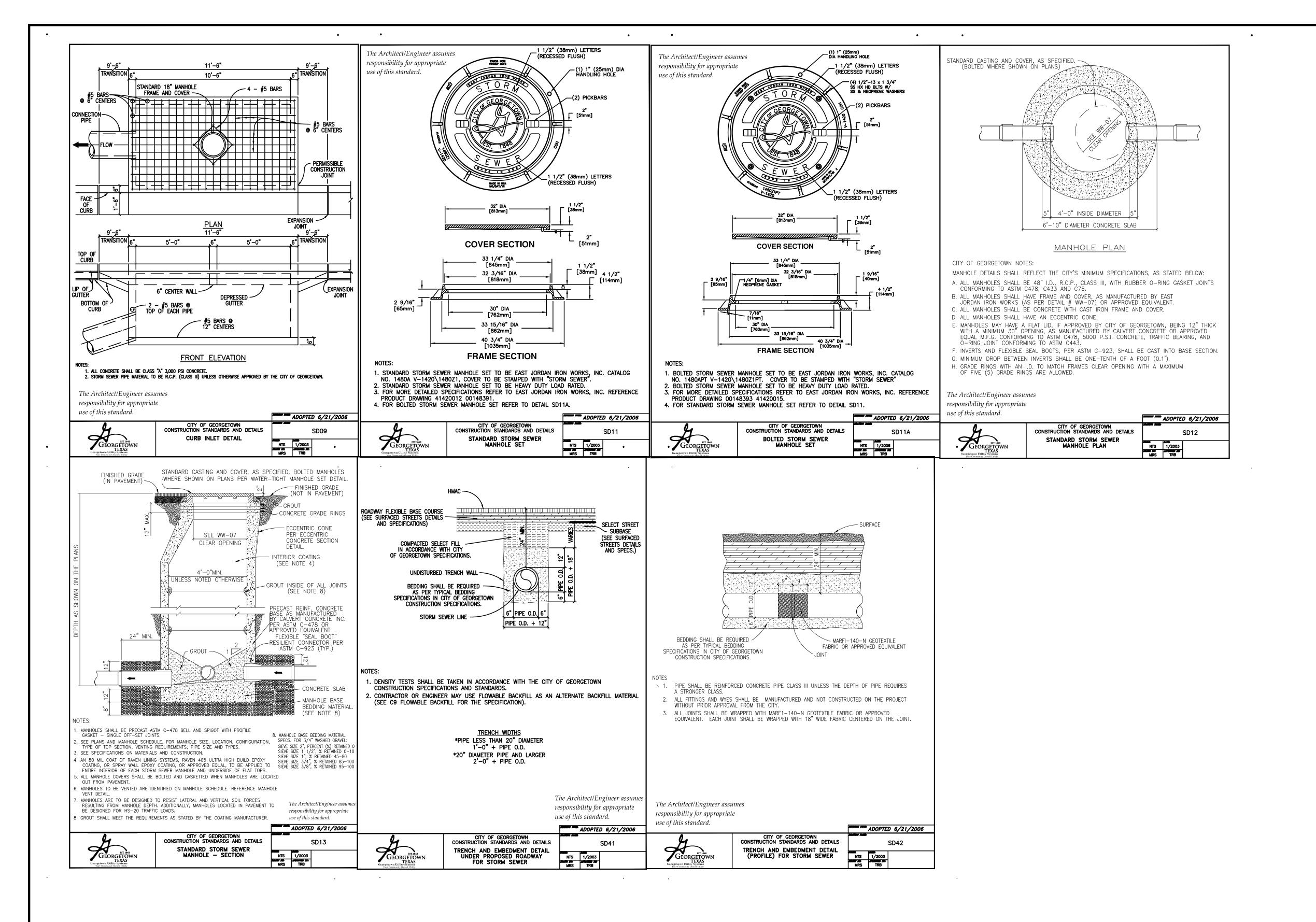
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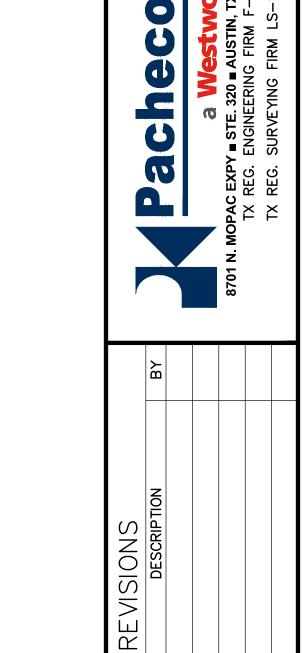
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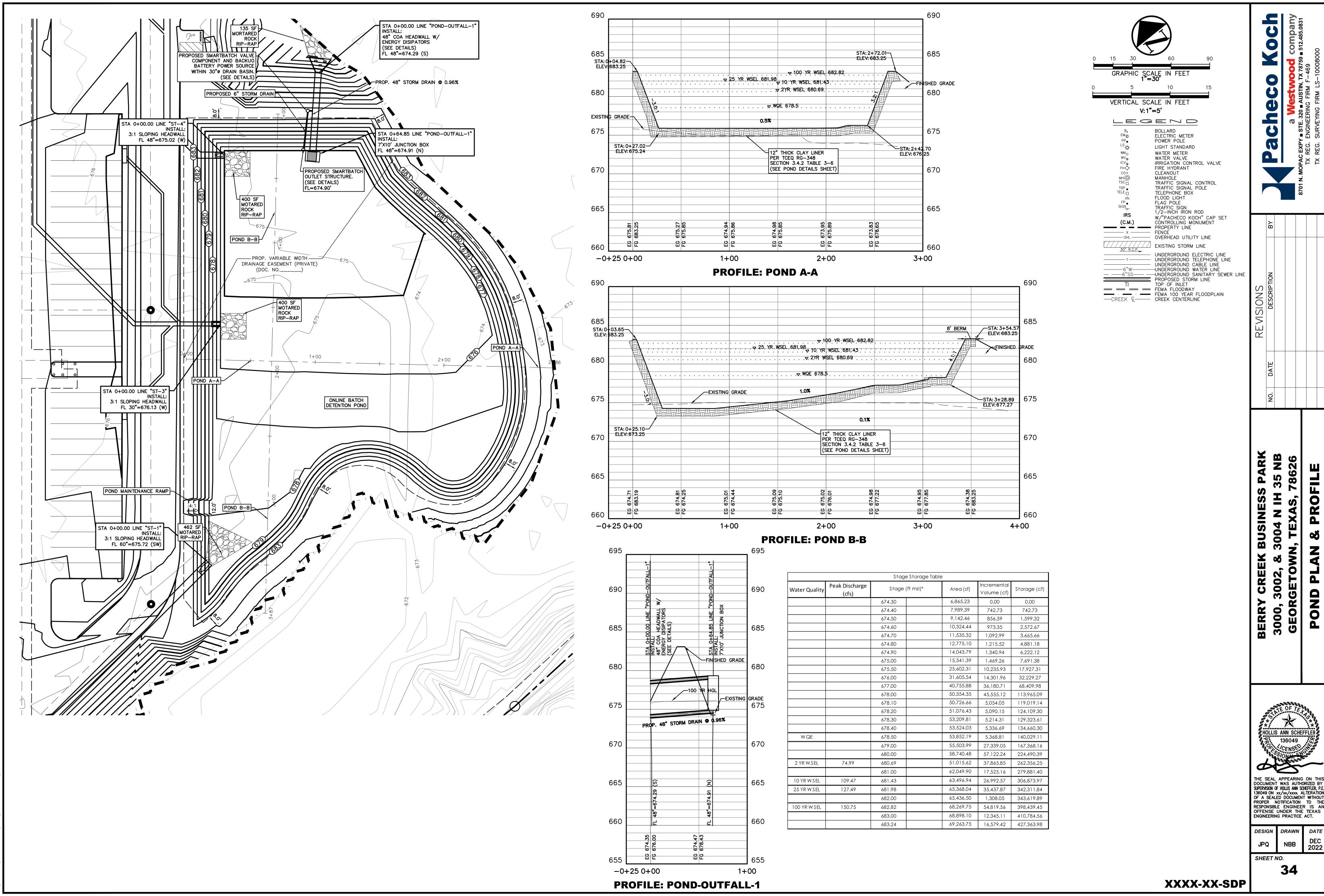
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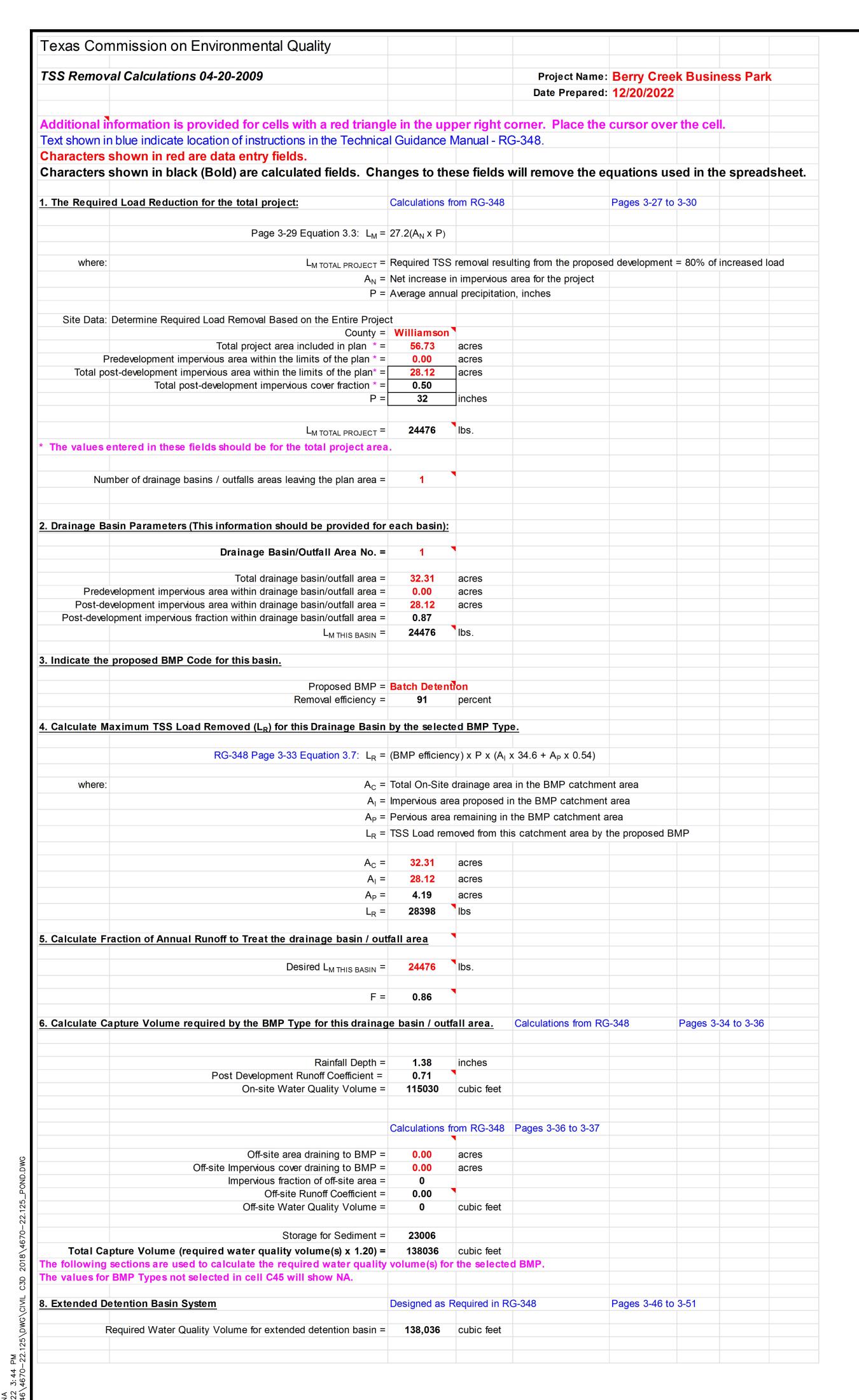
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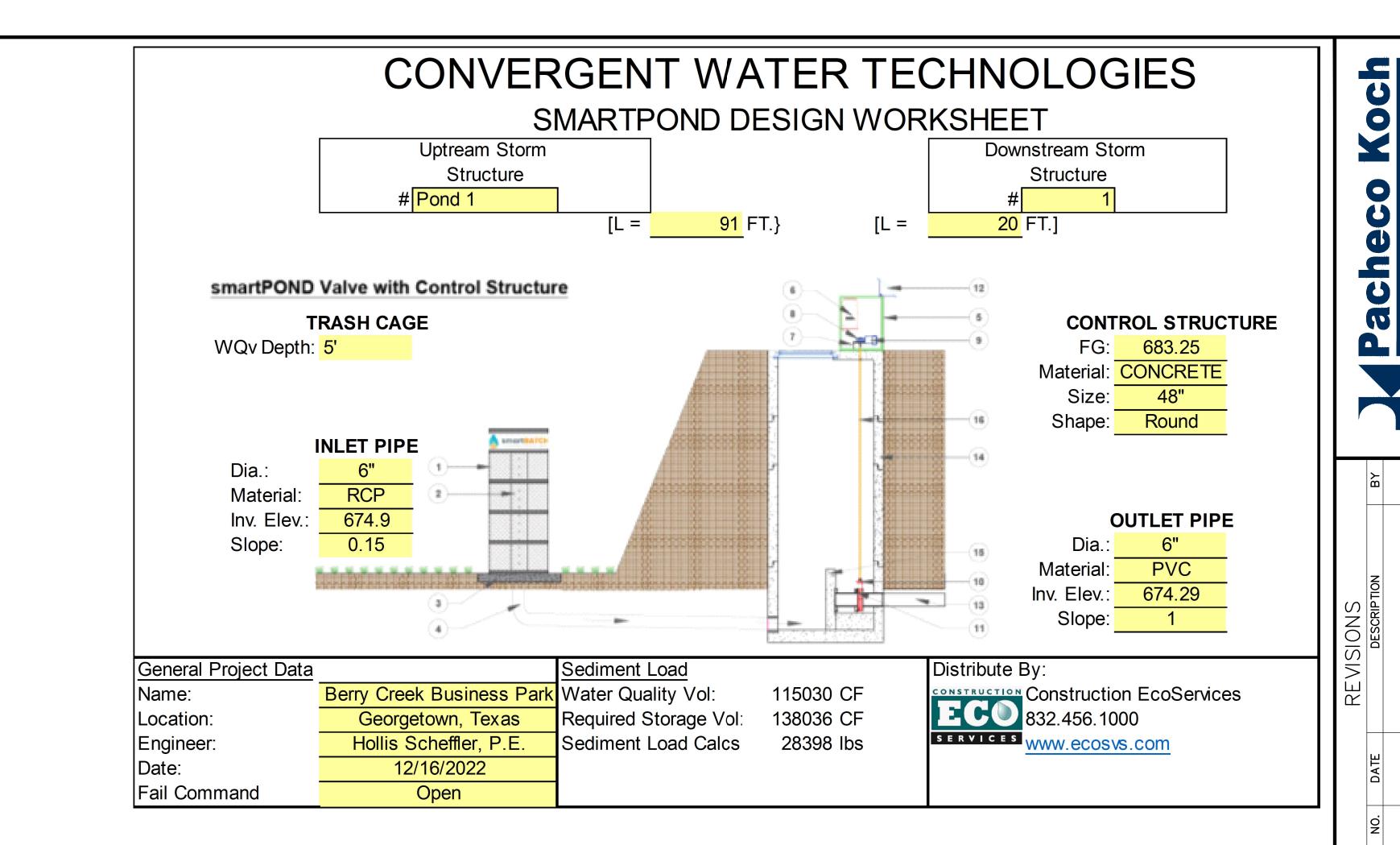
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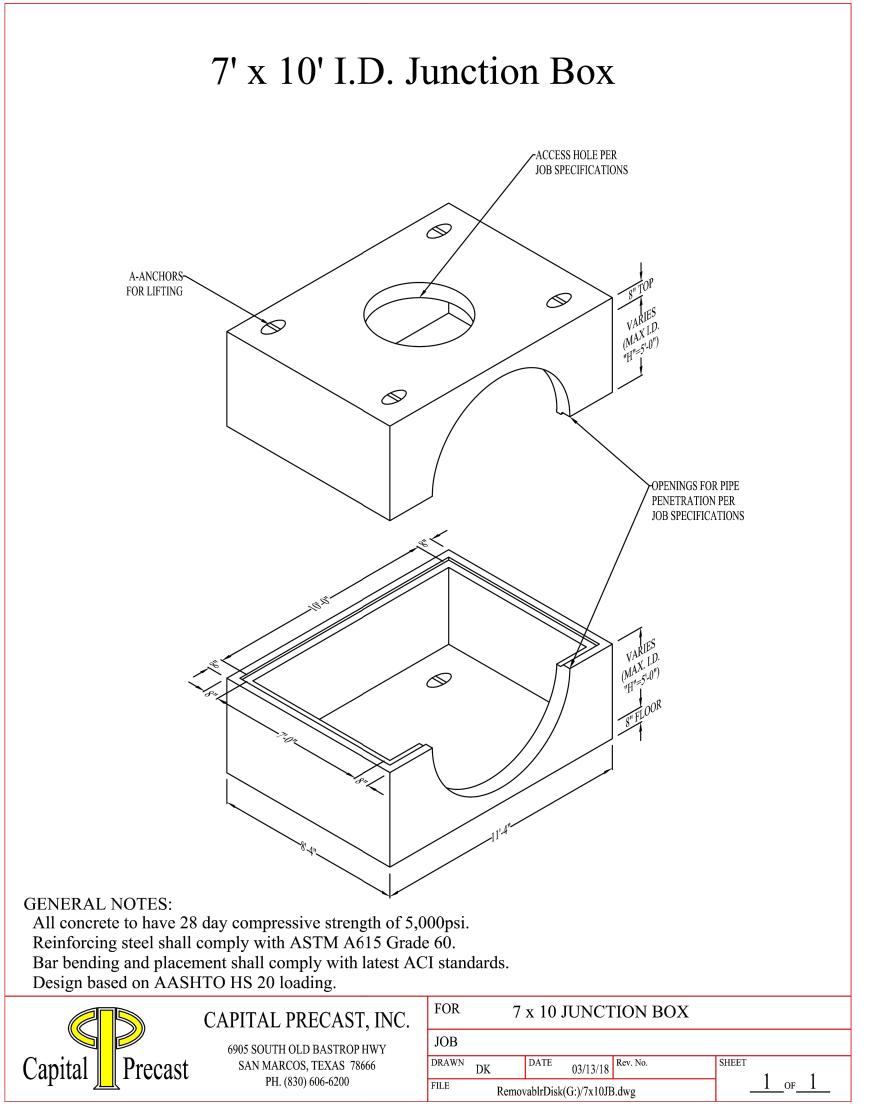
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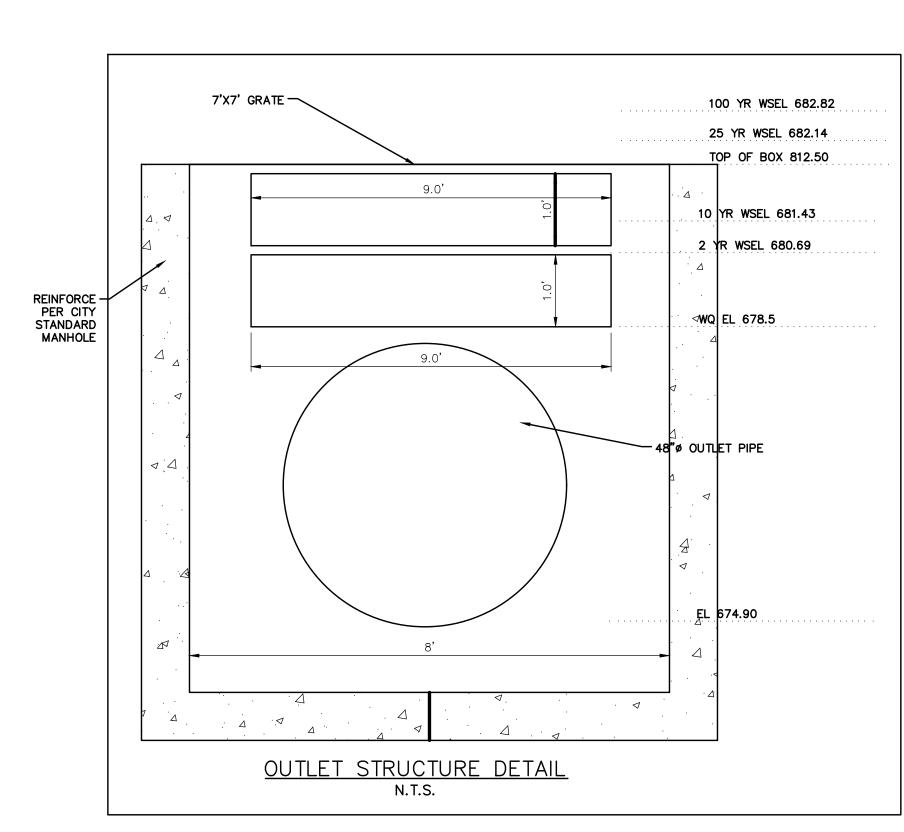


PK-4670-22.125_POND.DWG









CLAY LINER SPECIFICATION (CAO, 2004)							
PROPERTY	TEST METHOD	UNIT	SPECIFICATION				
PERMEABILITY	ASTM D-2434	CM/SEC	1 x 10 ⁻⁶				
PLASTICITY INDEX OF CLAY	ASTM D-423 & D-424	%	NOT LESS THAN 15				
LIQUID LIMIT OF CLAY	ASTM D-2216	%	NOT LESS THAN 30				
CLAY PARTICLES PASSING	ASTM D-422	%	NOT LESS THAN 30				
CLAY COMPACTION	ASTM D-2216	%	95% OF STANDARD PROCTOR DENSITY				

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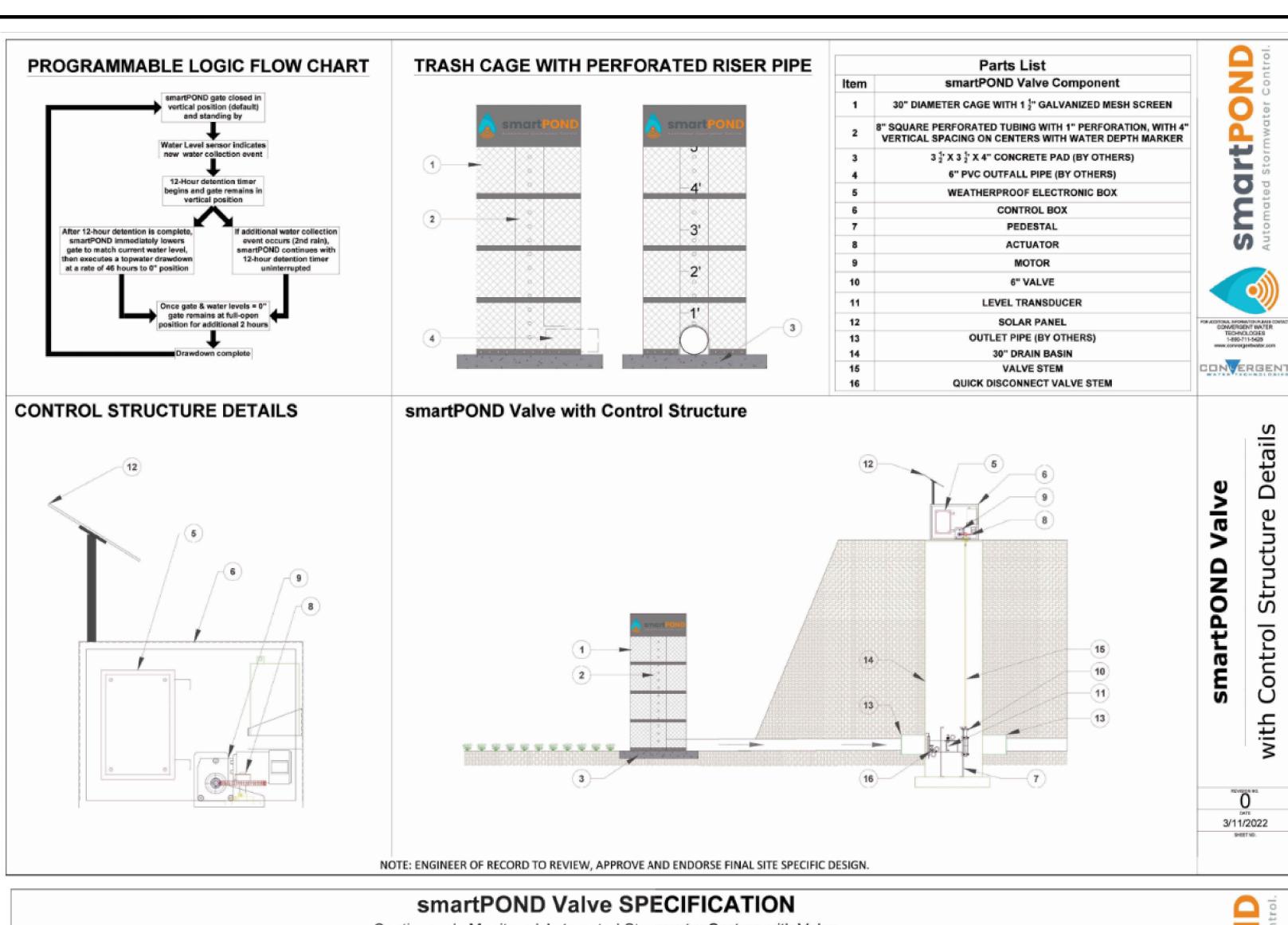
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size and quality.

NOTE: ENGINEER OF RECORD TO REVIEW, APPROVE AND ENDORSE FINAL SITE SPECIFIC DESIGN.

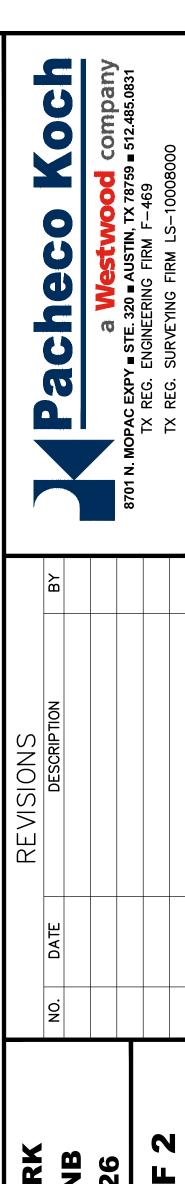
Loss of function

Valve malfunction

If the telemetry option is selected, the unit will upload the above alerts to the Autoflow app and notify the operator via text or email.

is to be serformed only by skilled work people with satisfactory record of performance on earthworks, pipe, welding, chamber, or pond/landfill construction projects of comparable

3/11/2022



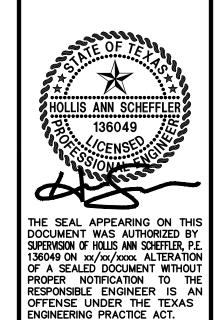
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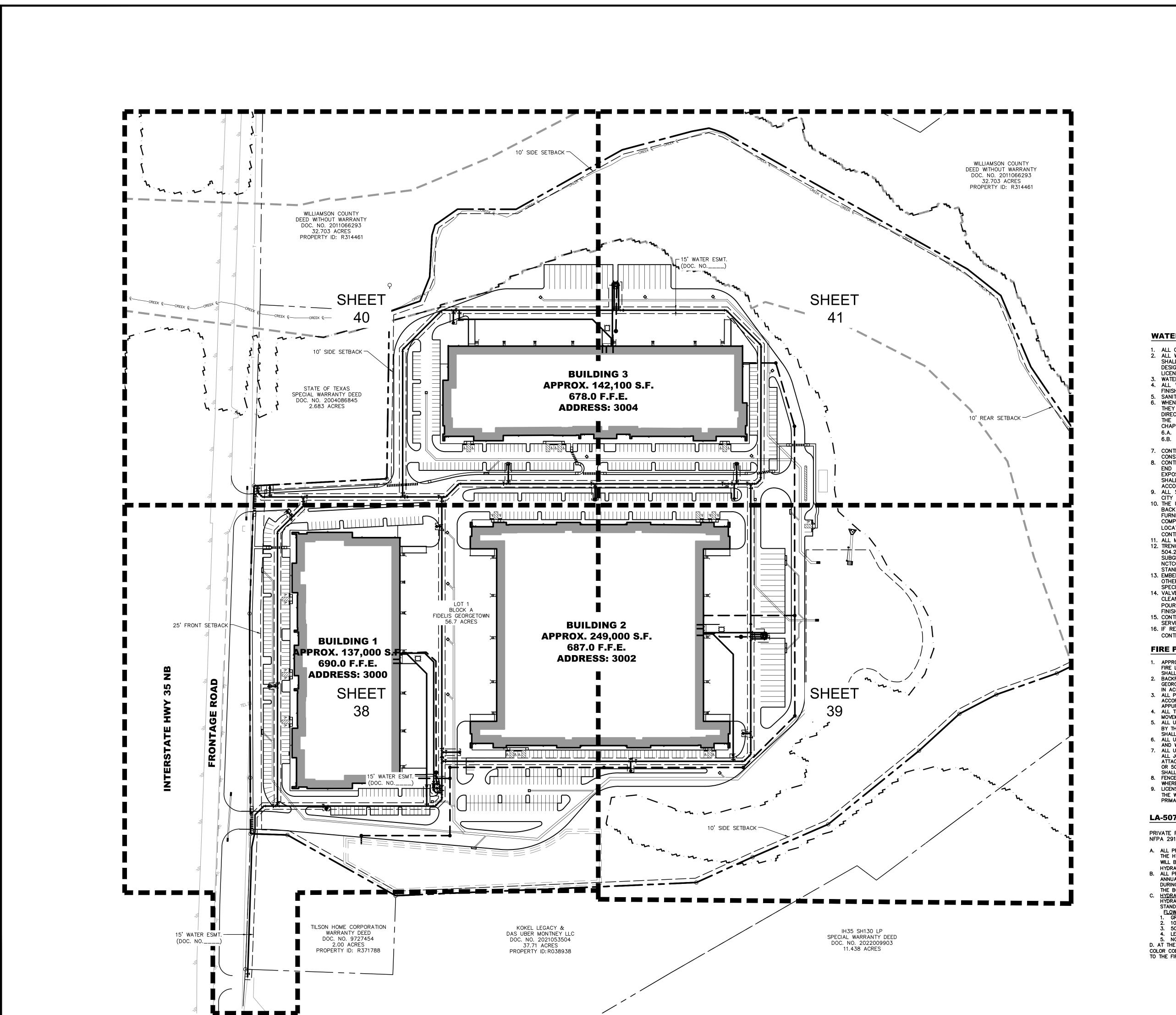
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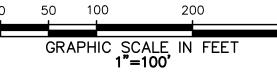


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B. EM® PP SIGN FRS IRS	E F L V V III F C N T T T T T T T	BOLLARD ELECTRIC METER POWER POLE LIGHT STAND, WATER METER WATER VALVE RRIGATION COMMENTER HYDRAN CLEANOUT MANHOLE TRAFFIC SIGN TRAFFIC SIGN TELEPHONE BETLOOD LIGHT FLAG POLE TRAFFIC SIGN	ARD R E ONTROL VA T AL CONTRO AL POLE BOX ON ROD	DL
(C.M.)	CONTROLLING PROPERTY LIFENCE OVERHEAD UT JNDERGROUN JNDERGROUN JNDERGROUN JNDERGROUN JNDERGROUN PROP FDC LO PROP FIRE HT PROP WATER PROP SANITA	MONUMEN' NE TILITY LINE D ELECTRIC D TELEPHO D CABLE L D WATER L D SANITAR' OCATION VALVE YDRANT LINE W/ E RY SEWER RY SEWER RY SEWER RY SEWER	T C LINE INE LINE INE INE Y SEWER BEND LINE MANHOLE CLEANOU

WATER & SANITARY SEWER GENERAL NOTES

- ALL CONCRETE SHALL BE CLASS "A" (3000 PSI), UNLESS OTHERWISE NOTED. 2. ALL WATER MAINS SHALL BE PVC C900, DR 18, CLASS 235. FIRE PROTECTION SERVICES SHALL BE PVC C900, DR 14, CLASS 305 AND INSTALLED IN ACCORDANCE WITH THE DESIGN AND SPECIFICATIONS OF THE FIRE PROTECTION PLANS TO BE PREPARED BY A LICENSED FIRE PROTECTION CONTRACTOR.
- 3. WATER AND SANITARY SEWER SERVICES SHALL MEET PLUMBING CODE REQUIREMENTS. 4. ALL WATER MAINS SHALL HAVE A MINIMUM COVER OF 48 INCHES BELOW IMPROVED
- FINISHED GRADE, UNLESS OTHERWISE NOTED. 5. SANITARY SEWER PIPE SHALL BE PVC SDR-35.
- WHEN WATER AND SANITARY SEWER MAINS, SERVICES, AND LATERALS ARE INSTALLED, THEY SHALL BE INSTALLED NO CLOSER TO EACH OTHER THAN NINE FEET IN ALL DIRECTIONS AND PARALLEL LINES MUST BE INSTALLED IN SEPARATE TRENCHES. WHERE THE NINE FOOT SEPARATION DISTANCE CANNOT BE ACHIEVED, THE FOLLOWING TCEQ
- 6.A. TCEQ CHAPTER 217.53 PIPE DESIGN, SECTION (d) SEPARATION DISTANCES. 6.B. TCEQ CHAPTER 290.44 WATER DISTRIBUTION, SECTION (e) LOCATION OF
- 7. CONTRACTOR TO VERIFY ALL EXISTING SEWER FLOW LINES BEFORE BEGINNING
- 8. CONTRACTOR SHALL TIE A ONE INCH WIDE PIECE OF RED PLASTIC FLAGGING TO THE END OF SEWER SERVICE AND SHALL LEAVE A MINIMUM OF 36 INCHES OF FLAGGING EXPOSED AFTER BACKFILL. AFTER CURB AND PAVING IS COMPLETED, CONTRACTOR SHALL MARK THE LOCATION OF THE SEWER SERVICE ON THE CURB OR ALLEY IN ACCORDANCE WITH THE STANDARD CITY SPECIFICATIONS.
- 9. ALL SANITARY SEWER LINES SHALL BE TESTED IN ACCORDANCE WITH THE STANDARD CITY SPECIFICATIONS. 10. THE UTILITY CONTRACTOR SHALL INSTALL THE WATER SERVICES TO A POINT TWO FEET BACK OF THE CURB LINE AT A DEPTH OF 12 INCHES. THE METER BOX SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR AFTER THE PAVING CONTRACTOR HAS COMPLETED THE FINE GRADING BEHIND THE BACK OF THE CURB. EACH SERVICE LOCATION SHALL BE MARKED ON THE CURB WITH A BLUE LETTER "W" BY THE UTILITY
- CONTRACTOR AND TIED TO PROPERTY CORNERS ON THE "RECORD DRAWINGS.

 11. ALL METER BOXES SHALL BE LOCATED IN NON-TRAFFIC AREAS. 12. TRENCH BACKFILL MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF NCTCOG ITEM 504.2 AND SHALL BE MECHANICALLY COMPACTED IN 6-INCH LIFTS TO THE TOP OF SUBGRADE TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY IN ACCORDANCE WITH NCTCOG ITEM 504.5 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN THE
- STANDARD CITY SPECIFICATIONS. 13. EMBEDMENT SHALL CONFORM TO THE REQUIREMENTS OF NCTCOG ITEM 504.5 UNLESS OTHERWISE SHOWN ON THESE PLANS OR STATED IN THE STANDARD CITY
- 14. VALVE BOXES SHALL BE FURNISHED AND SET ON EACH GATE VALVE. AFTER THE FINAL CLEAN-UP AND ALIGNMENT HAS BEEN COMPLETED, THE UTILITY CONTRACTOR SHALL POUR A 24"X24"X6" CONCRETE BLOCK AROUND ALL VALVE BOX TOPS LEVEL WITH THE
- 15. CONTRACTOR SHALL RECONNECT ALL EXISTING SERVICES AND MAINTAIN EXISTING SERVICES THROUGHOUT CONSTRUCTION. 16. IF REQUIRED DUE TO CONSTRUCTION, POWER POLES TO BE BRACED OR RELOCATED AT CONTRACTOR'S EXPENSE.

FIRE PROTECTION NOTES

- APPROVAL OF THIS SITE PLAN DOES NOT IMPLY APPROVAL TO INSTALL UNDERGROUND FIRE LINES. PRIOR TO INSTALLATION OF UNDERGROUND FIRE LINES, A SEPARATE PERMIT SHALL BE SUBMITTED, UNDER GROUND FIRE LINE SUPPLY.
- 2. BACKFLOW PROTECTION WILL BE PROVIDED IN ACCORDANCE WITH THE CITY OF GEORGETOWN REQUIREMENTS WHEN REQUIRED. BACKFLOW PROTECTION WILL BE INSTALLED IN ACCORDANCE WITH THE DETAIL PROVIDED IN THE UTILITY DRAWINGS.
- 3. ALL PRIVATE FIRE LINES AND WHAT THEY PROVIDE SERVICE TO WILL BE INSTALLED IN ACCORDANCE WITH NFPA 24 INSTULLATION OF PRIVATE SERVICE MAINS AND THEIR
- 4. ALL TEES, PLUGS, CAPS, BENDS, REDUCERS, VALVES SHALL BE RESTRAINED AGAINST MOVEMENT. THRUST BLOCKING WILL BE INSTALLED IN ACCORDANCE WITH NFPA 24.
- 5. ALL UNDERGROUND SHALL REMAIN UNCOVERED UNTIL A VISUAL INSPECTION IS CONDUCTED BY THE GEORGETOWN FIRE MARSHAL'S OFFICE (FMO). ALL JOINTS AND THRUST BLOCKING SHALL BE UNCOVERED FOR VISUAL INSPECTION.
- 6. ALL UNDERGROUND SHALL BE FLUSHED PER THE REQUIREMENTS OF NFPA STANDARD 24 AND WITNESSED BY GEORGETOWN FMO. 7. ALL UNDERGROUND SHALL PASS A HYDROSTATIC TEST WITNESSED BY GEORGETOWN FMO. ALL JOINTS SHALL BE UNCOVERED FOR HYDROSTATIC TESTING. ALL PIPING AND ATTACHMENTS SUBJECTED TO SYSTEM WORKING PRESSURE SHALL BE TESTED AT 200 PSI. OR 50 PSI IN EXCESS OF THE SYSTEM WORKING PRESSURE, WHICHEVER IS GREATER, AND
- SHALL MAINTAIN THAT PRESSURE + OR 5 PSI FOR 2 HOURS.

 8. FENCES, LANDSCAPING AND OTHER ITEMS WILL NOT BE INSTALLED WITHIN 3 FT, AND WHERE THEY WILL OBSTRUCT THE VISIBILITY OR ACCESS TO HYDRANTS, OR REMOTE FDCS. 9. LICENSE REQUIREMENTS OF EITHER RME-U OR G. WHEN CONNECTING BY UNDERGROUND TO THE WATER PURVEYOR'S MAIN FROM THE POINT OF CONNECTION OR VALVE WHERE THE PRIMARY PURPOSE OF WATER IS FOR FIRE PROTECTION SPRINKLER SYSTEM.

LA-507.5.7 FIRE HYDRANT COLOR CODE SYSTEM

PRIVATE FIRE HYDRANT MAINTENANCE SHALL BE IN ACCORDANCE WITH

- A. ALL PRIVATE HYDRANT BARRELS WILL BE PAINTED RED WITH THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN PARAGRAPH C OF THIS SECTION TO INDICATE FLOW. IT WILL BE THE CUSTOMER'S RESPONSIBILITY TO TEST AND MAINTAIN THEIR PRIVATE FIRE
- B. ALL PRIVATE FIRE HYDRANTS SHOULD BE INSPECTED, MAINTAINED AND FLOW TESTED ANNUALLY AND COLOR CODED TO INDICATE THE EXPECTED FIRE FLOW FROM THE HYDRANT DURING NORMAL OPERATION. SUCH COLOR APPLIED TO THE FIRE HYDRANT BY PAINTING
- THE BONNET THE APPROPRIATE COLOR FOR THE EXPECTED FLOW CONDITION.

 C. HYDRANT FLOW CODING STANDARDS, PUBLIC HYDRANTS WILL BE PAINTED SILVER, THE HYDRANTS WILL BE FLOW TESTED, AND THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN AS FOLLOWS: AT 20 PSI RESIDUAL.
 - FLOW COLOR:

 1. GREATER THAN 1500 GPM BLUE

 2. 1000 TO 1500 GPM GREEN
 - 3. 500 TO 999 GPM ORANGE
 - 4. LESS THAN 500 GPM RED 5. NOT WORKING BLACK OR BAGGED
- D. AT THE CONCLUSION OF CONSTRUCTION FIRE HYDRANTS SHALL BE FLOW TESTED AND COLOR CODED IN ACCORDANCE WITH CITY'S STANDARDS, AND RESULTS SHALL BE EMAILED TO THE FIRE DEPARTMENT. IFC-LA-507.5.7 FIRE HYDRANT SYSTEMS.

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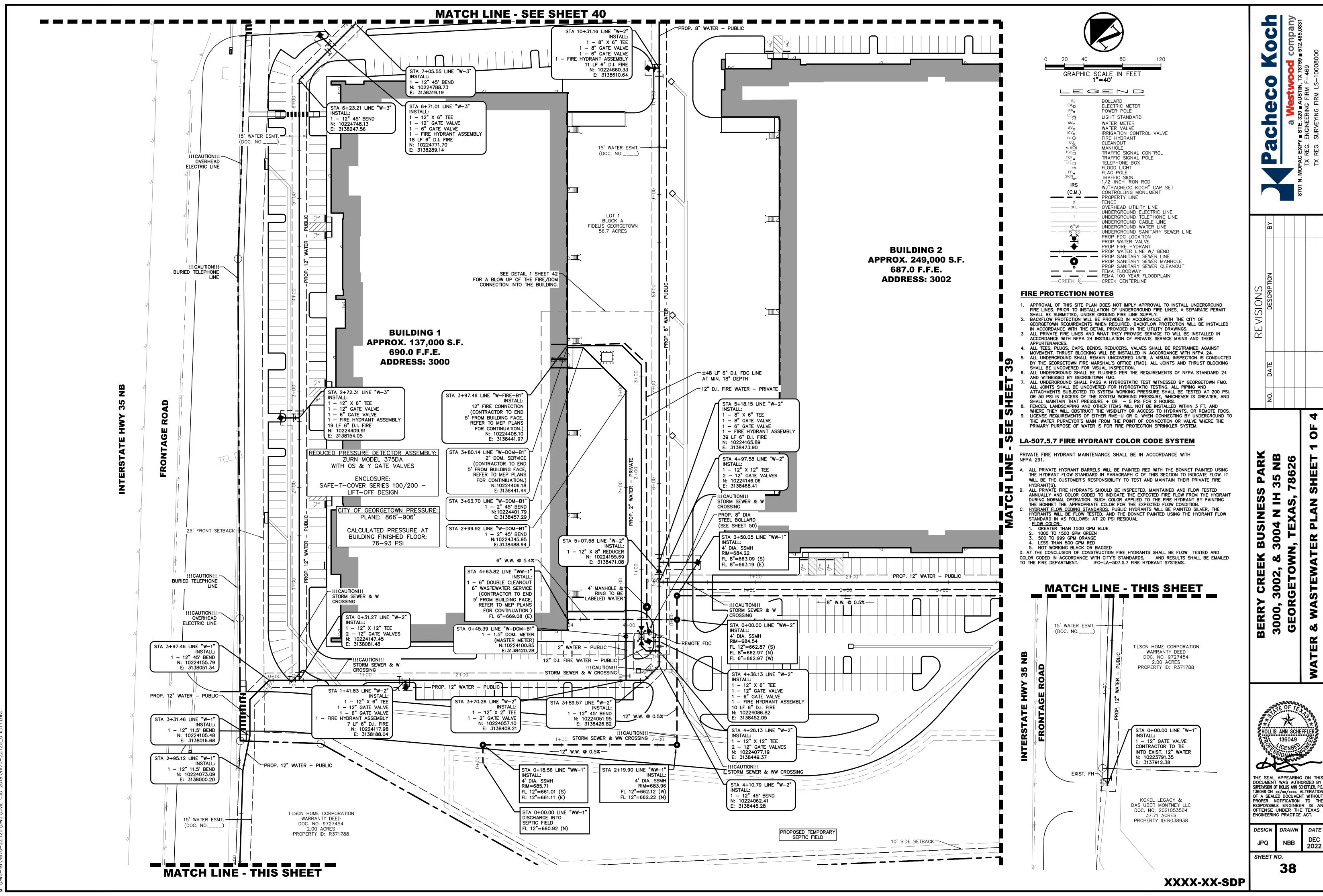
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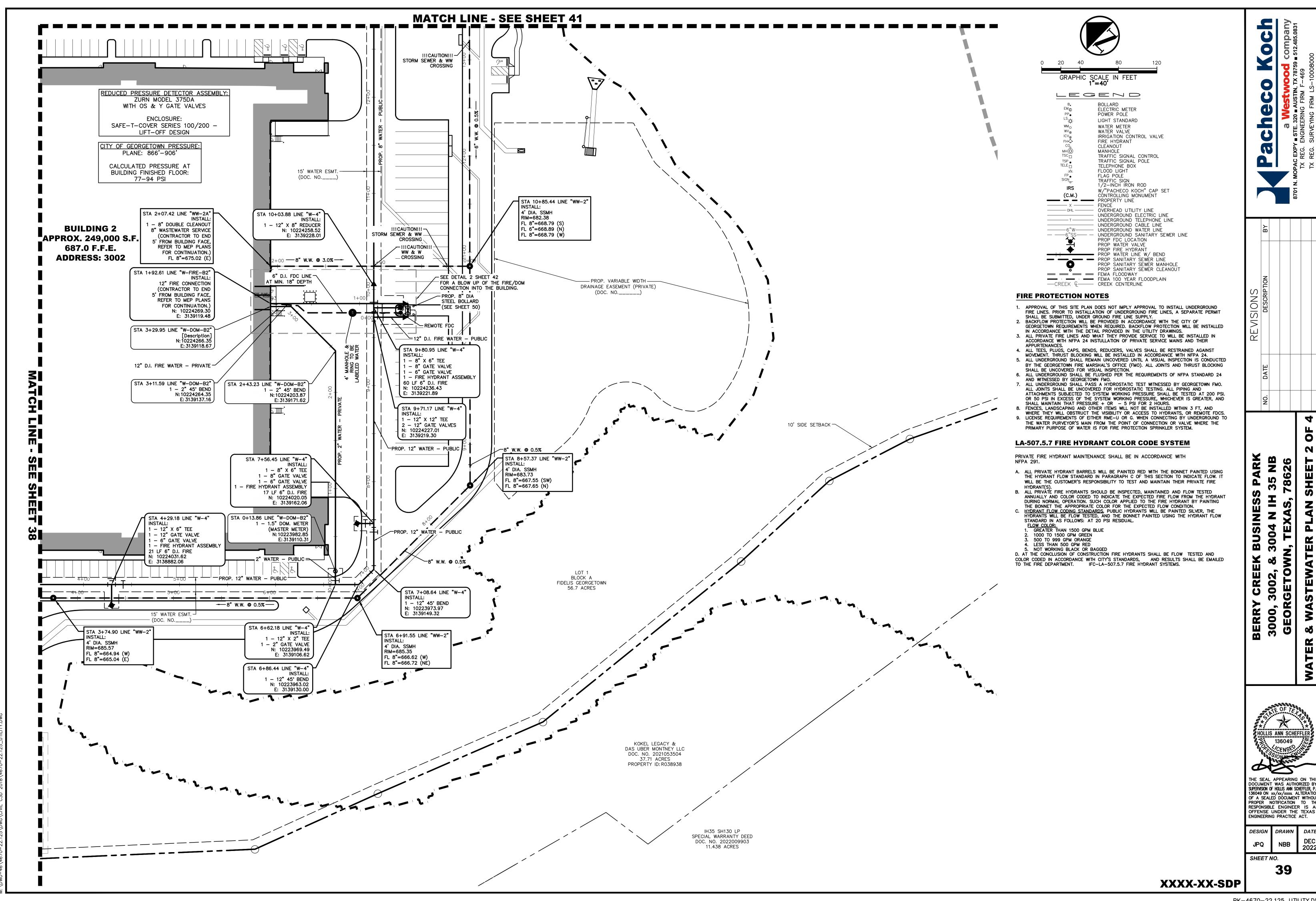
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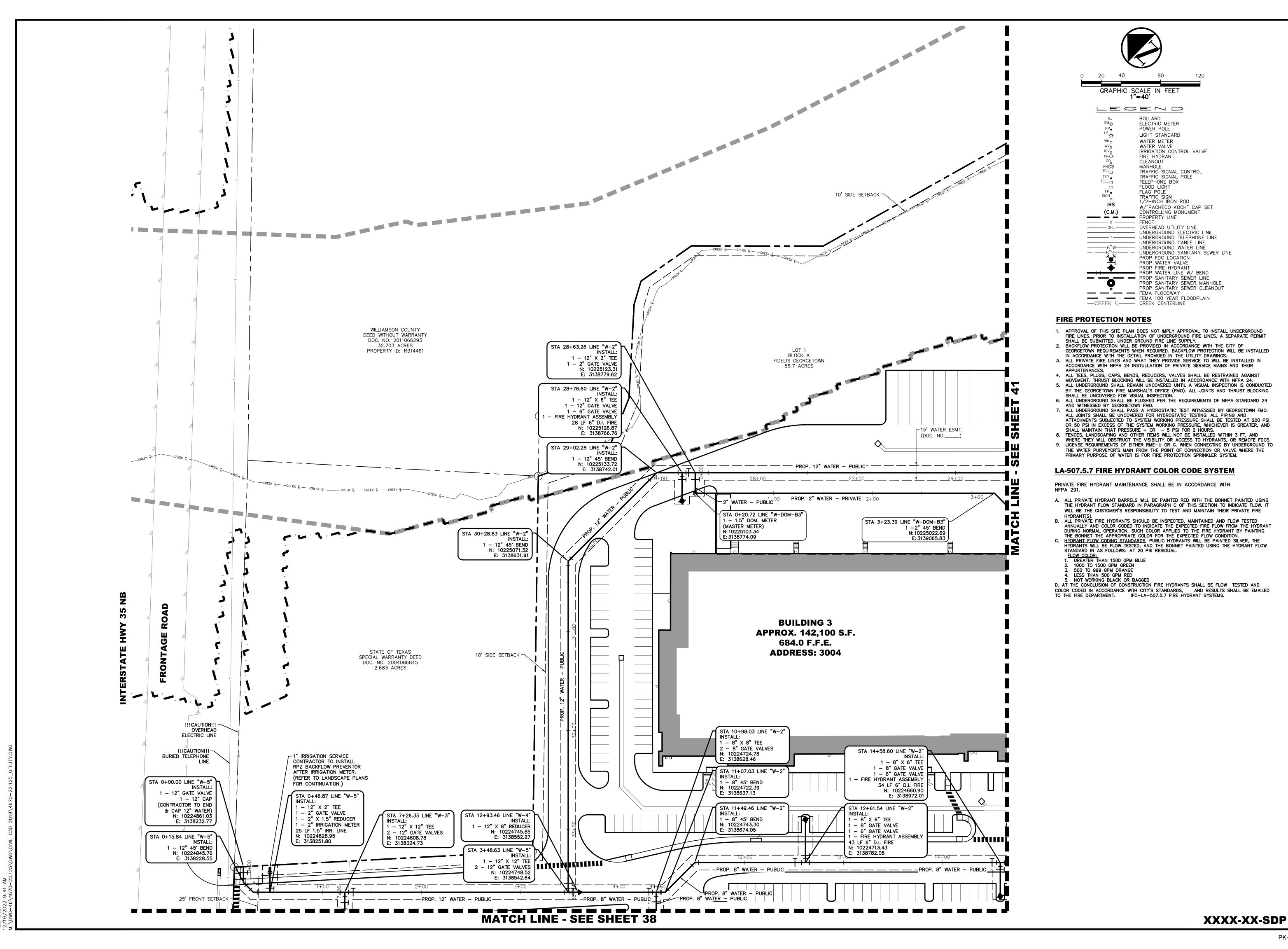
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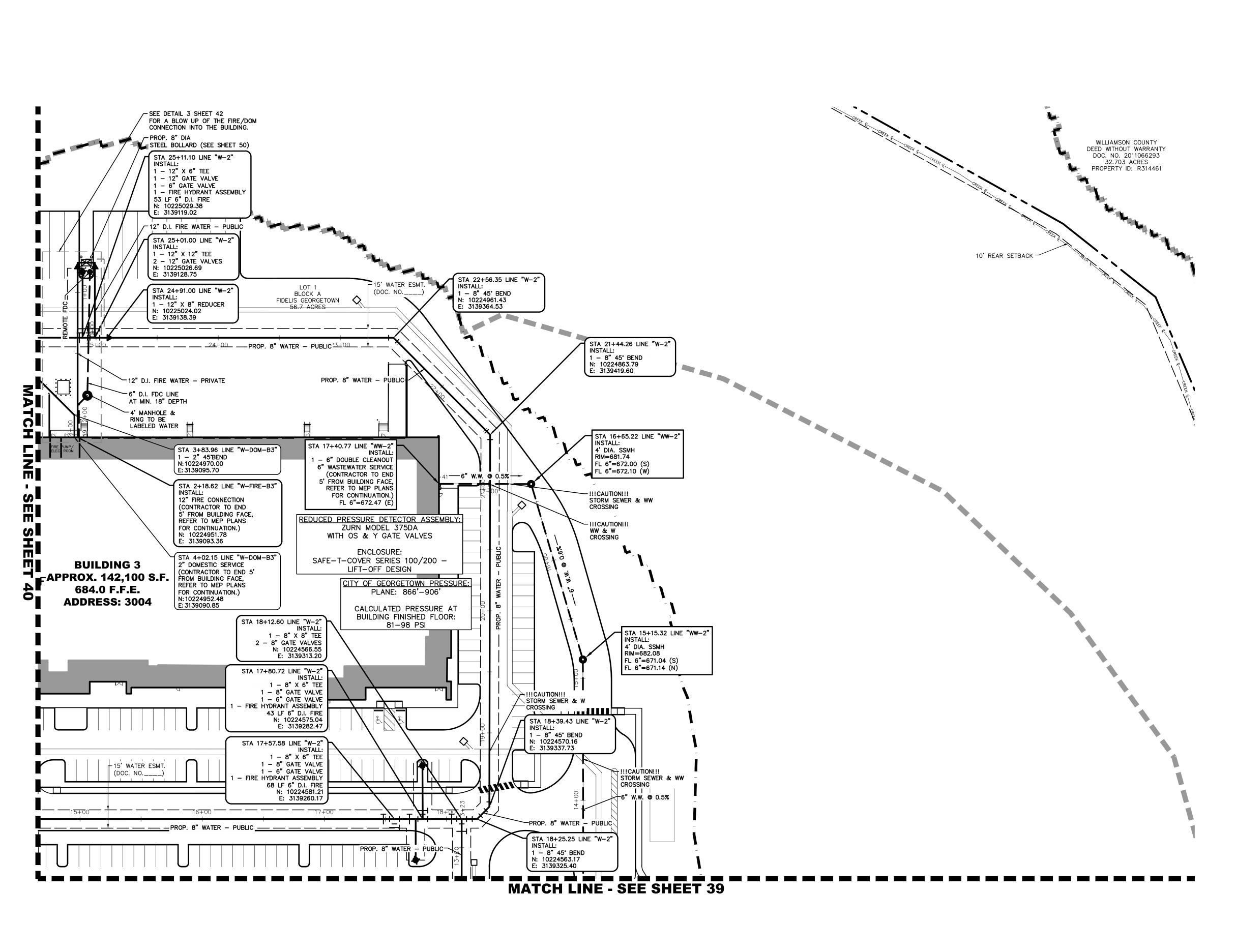
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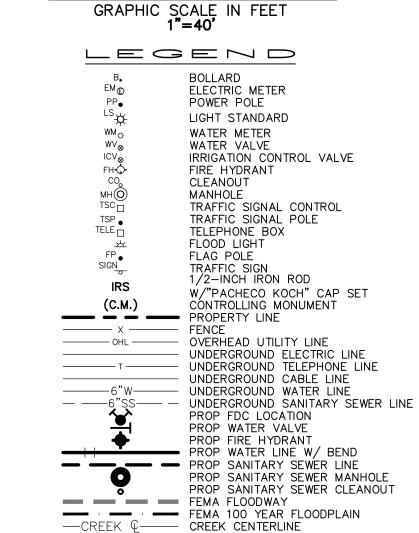
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FIRE PROTECTION NOTES

- 1. APPROVAL OF THIS SITE PLAN DOES NOT IMPLY APPROVAL TO INSTALL UNDERGROUND FIRE LINES. PRIOR TO INSTALLATION OF UNDERGROUND FIRE LINES, A SEPARATE PERMIT
- SHALL BE SUBMITTED, UNDER GROUND FIRE LINE SUPPLY. 2. BACKFLOW PROTECTION WILL BE PROVIDED IN ACCORDANCE WITH THE CITY OF
- GEORGETOWN REQUIREMENTS WHEN REQUIRED. BACKFLOW PROTECTION WILL BE INSTALLED IN ACCORDANCE WITH THE DETAIL PROVIDED IN THE UTILITY DRAWINGS. 3. ALL PRIVATE FIRE LINES AND WHAT THEY PROVIDE SERVICE TO WILL BE INSTALLED IN
- ACCORDANCE WITH NFPA 24 INSTULLATION OF PRIVATE SERVICE MAINS AND THEIR APPURTENANCES.
- 4. ALL TEES, PLUGS, CAPS, BENDS, REDUCERS, VALVES SHALL BE RESTRAINED AGAINST MOVEMENT. THRUST BLOCKING WILL BE INSTALLED IN ACCORDANCE WITH NFPA 24. 5. ALL UNDERGROUND SHALL REMAIN UNCOVERED UNTIL A VISUAL INSPECTION IS CONDUCTED
- BY THE GEORGETOWN FIRE MARSHAL'S OFFICE (FMO). ALL JOINTS AND THRUST BLOCKING SHALL BE UNCOVERED FOR VISUAL INSPECTION. 6. ALL UNDERGROUND SHALL BE FLUSHED PER THE REQUIREMENTS OF NFPA STANDARD 24
- AND WITNESSED BY GEORGETOWN FMO. 7. ALL UNDERGROUND SHALL PASS A HYDROSTATIC TEST WITNESSED BY GEORGETOWN FMO. ALL JOINTS SHALL BE UNCOVERED FOR HYDROSTATIC TESTING. ALL PIPING AND ATTACHMENTS SUBJECTED TO SYSTEM WORKING PRESSURE SHALL BE TESTED AT 200 PSI.
- OR 50 PSI IN EXCESS OF THE SYSTEM WORKING PRESSURE, WHICHEVER IS GREATER, AND SHALL MAINTAIN THAT PRESSURE + OR 5 PSI FOR 2 HOURS.

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LA-507.5.7 FIRE HYDRANT COLOR CODE SYSTEM

PRIVATE FIRE HYDRANT MAINTENANCE SHALL BE IN ACCORDANCE WITH

- NFPA 291. A. ALL PRIVATE HYDRANT BARRELS WILL BE PAINTED RED WITH THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN PARAGRAPH C OF THIS SECTION TO INDICATE FLOW. IT WILL BE THE CUSTOMER'S RESPONSIBILITY TO TEST AND MAINTAIN THEIR PRIVATE FIRE
- HYDRANT(S).

 B. ALL PRIVATE FIRE HYDRANTS SHOULD BE INSPECTED, MAINTAINED AND FLOW TESTED ANNUALLY AND COLOR CODED TO INDICATE THE EXPECTED FIRE FLOW FROM THE HYDRANT DURING NORMAL OPERATION. SUCH COLOR APPLIED TO THE FIRE HYDRANT BY PAINTING THE BONNET THE APPROPRIATE COLOR FOR THE EXPECTED FLOW CONDITION.

 C. HYDRANT FLOW CODING STANDARDS, PUBLIC HYDRANTS WILL BE PAINTED SILVER, THE HYDRANTS WILL BE FLOW TESTED, AND THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN AS FOLLOWS: AT 20 PSI RESIDUAL.
 - FLOW COLOR:

 1. GREATER THAN 1500 GPM BLUE
 2. 1000 TO 1500 GPM GREEN
 3. 500 TO 999 GPM ORANGE
 4. LESS THAN 500 GPM RED
 - 5. NOT WORKING BLACK OR BAGGED
- D. AT THE CONCLUSION OF CONSTRUCTION FIRE HYDRANTS SHALL BE FLOW TESTED AND COLOR CODED IN ACCORDANCE WITH CITY'S STANDARDS, AND RESULTS SHALL BE EMAILED TO THE FIRE DEPARTMENT. IFC-LA-507.5.7 FIRE HYDRANT SYSTEMS.

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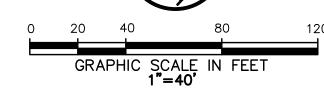
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ELECTRIC METER POWER POLE LIGHT STANDARD WATER METER WATER VALVE IRRIGATION CONTROL VALVE FIRE HYDRANT CLEANOUT TRAFFIC SIGNAL POLE TELEPHONE BOX FLOOD LIGHT FLAG POLE 1/2-INCH IRON ROD IRS W/"PACHECO KOCH" CAP SET (C.M.) CÓNTROLLING MONUMENT PROPERTY LINE OVERHEAD UTILITY LINE UNDERGROUND ELECTRIC LINE UNDERGROUND TELEPHONE LINE UNDERGROUND CABLE LINE UNDERGROUND WATER LINE UNDERGROUND SANITARY SEWER LINE PROP FDC LOCATION PROP WATER VALVE PROP FIRE HYDRANT - PROP WATER LINE W/ BEND - PROP SANITARY SEWER LINE 0 PROP SANITARY SEWER MANHOLE PROP SANITARY SEWER CLEANOUT

FIRE PROTECTION NOTES

1. APPROVAL OF THIS SITE PLAN DOES NOT IMPLY APPROVAL TO INSTALL UNDERGROUND FIRE LINES. PRIOR TO INSTALLATION OF UNDERGROUND FIRE LINES, A SEPARATE PERMIT SHALL BE SUBMITTED, UNDER GROUND FIRE LINE SUPPLY.

FEMA 100 YEAR FLOODPLAIN

FEMA FLOODWAY

- 2. BACKFLOW PROTECTION WILL BE PROVIDED IN ACCORDANCE WITH THE CITY OF GEORGETOWN REQUIREMENTS WHEN REQUIRED. BACKFLOW PROTECTION WILL BE INSTALLED IN ACCORDANCE WITH THE DETAIL PROVIDED IN THE UTILITY DRAWINGS.
- 3. ALL PRIVATE FIRE LINES AND WHAT THEY PROVIDE SERVICE TO WILL BE INSTALLED IN ACCORDANCE WITH NFPA 24 INSTULLATION OF PRIVATE SERVICE MAINS AND THEIR APPURTENANCES.
- 4. ALL TEES, PLUGS, CAPS, BENDS, REDUCERS, VALVES SHALL BE RESTRAINED AGAINST MOVEMENT. THRUST BLOCKING WILL BE INSTALLED IN ACCORDANCE WITH NFPA 24. 5. ALL UNDERGROUND SHALL REMAIN UNCOVERED UNTIL A VISUAL INSPECTION IS CONDUCTED
- BY THE GEORGETOWN FIRE MARSHAL'S OFFICE (FMO). ALL JOINTS AND THRUST BLOCKING SHALL BE UNCOVERED FOR VISUAL INSPECTION.

 6. ALL UNDERGROUND SHALL BE FLUSHED PER THE REQUIREMENTS OF NFPA STANDARD 24
- AND WITNESSED BY GEORGETOWN FMO. 7. ALL UNDERGROUND SHALL PASS A HYDROSTATIC TEST WITNESSED BY GEORGETOWN FMO. ALL JOINTS SHALL BE UNCOVERED FOR HYDROSTATIC TESTING. ALL PIPING AND ATTACHMENTS SUBJECTED TO SYSTEM WORKING PRESSURE SHALL BE TESTED AT 200 PSI.
- OR 50 PSI IN EXCESS OF THE SYSTEM WORKING PRESSURE, WHICHEVER IS GREATER, AND SHALL MAINTAIN THAT PRESSURE + OR 5 PSI FOR 2 HOURS.

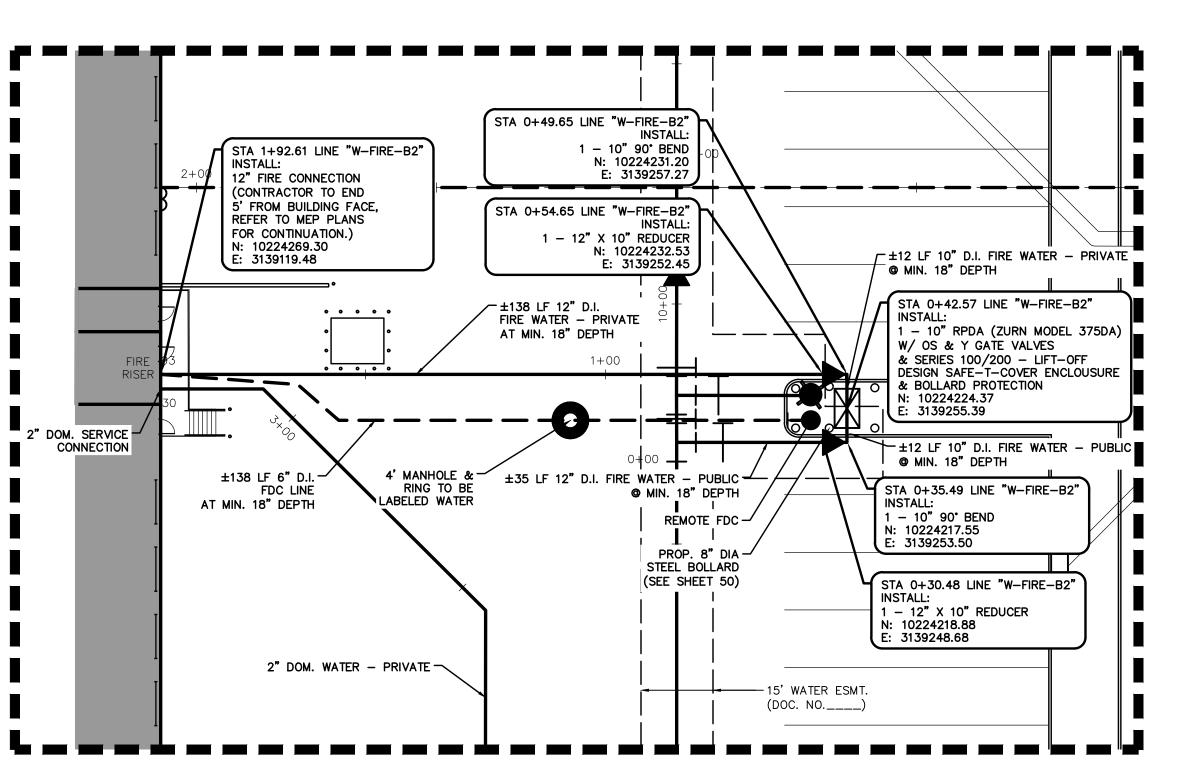
 8. FENCES, LANDSCAPING AND OTHER ITEMS WILL NOT BE INSTALLED WITHIN 3 FT, AND WHERE THEY WILL OBSTRUCT THE VISIBILITY OR ACCESS TO HYDRANTS, OR REMOTE FDCS.
- 9. LICENSE REQUIREMENTS OF EITHER RME-U OR G. WHEN CONNECTING BY UNDERGROUND TO THE WATER PURVEYOR'S MAIN FROM THE POINT OF CONNECTION OR VALVE WHERE THE PRIMARY PURPOSE OF WATER IS FOR FIRE PROTECTION SPRINKLER SYSTEM.

LA-507.5.7 FIRE HYDRANT COLOR CODE SYSTEM

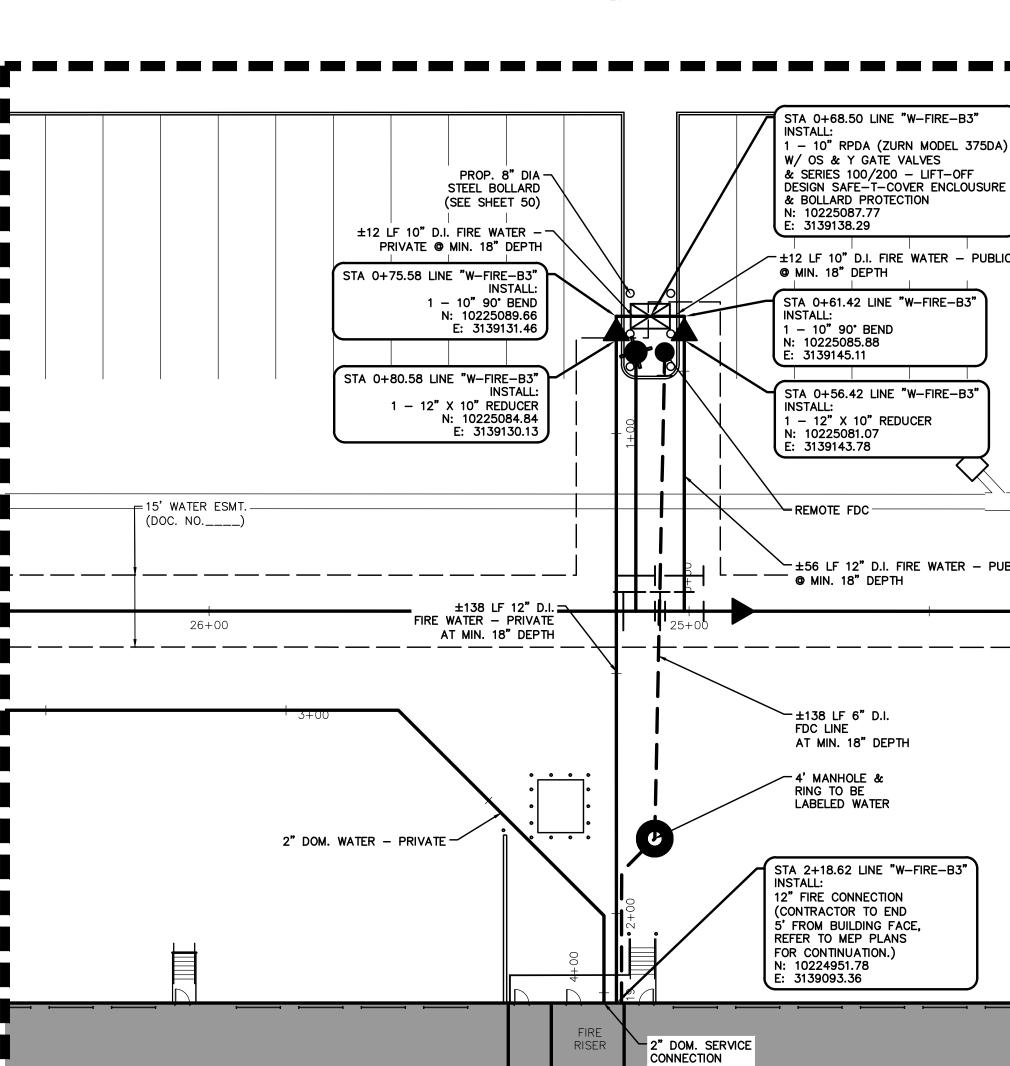
PRIVATE FIRE HYDRANT MAINTENANCE SHALL BE IN ACCORDANCE WITH NFPA 291.

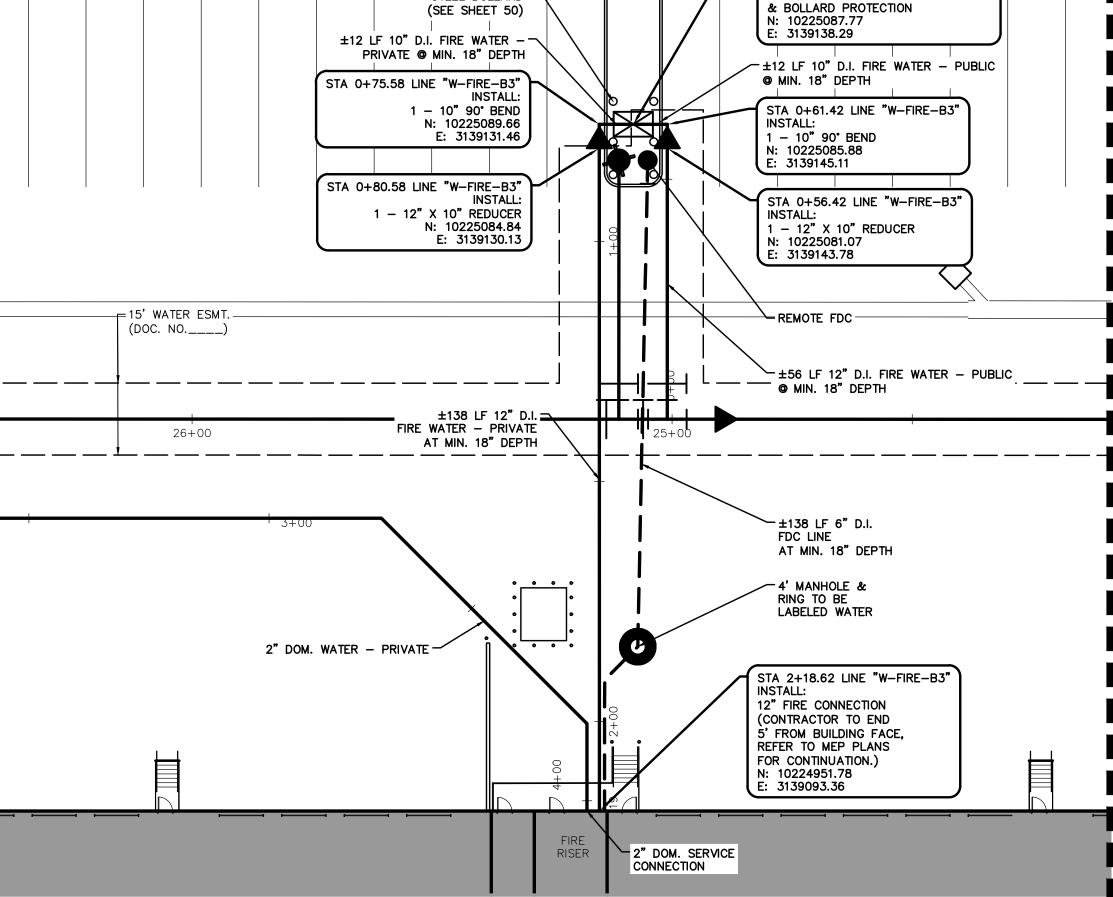
- A. ALL PRIVATE HYDRANT BARRELS WILL BE PAINTED RED WITH THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN PARAGRAPH C OF THIS SECTION TO INDICATE FLOW. IT WILL BE THE CUSTOMER'S RESPONSIBILITY TO TEST AND MAINTAIN THEIR PRIVATE FIRE
- B. ALL PRIVATE FIRE HYDRANTS SHOULD BE INSPECTED, MAINTAINED AND FLOW TESTED ANNUALLY AND COLOR CODED TO INDICATE THE EXPECTED FIRE FLOW FROM THE HYDRANT DURING NORMAL OPERATION. SUCH COLOR APPLIED TO THE FIRE HYDRANT BY PAINTING THE BONNET THE APPROPRIATE COLOR FOR THE EXPECTED FLOW CONDITION.
- C. HYDRANT FLOW CODING STANDARDS, PUBLIC HYDRANTS WILL BE PAINTED SILVER, THE HYDRANTS WILL BE FLOW TESTED, AND THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN AS FOLLOWS: AT 20 PSI RESIDUAL.
- FLOW COLOR:

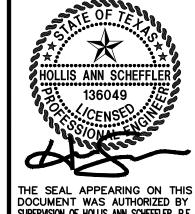
 1. GREATER THAN 1500 GPM BLUE
 2. 1000 TO 1500 GPM GREEN 3. 500 TO 999 GPM ORANGE LESS THAN 500 GPM RED
- 5. NOT WORKING BLACK OR BAGGED D. AT THE CONCLUSION OF CONSTRUCTION FIRE HYDRANTS SHALL BE FLOW TESTED AND COLOR CODED IN ACCORDANCE WITH CITY'S STANDARDS, AND RESULTS SHALL BE EMAILED TO THE FIRE DEPARTMENT. IFC-LA-507.5.7 FIRE HYDRANT SYSTEMS.



2.) BUILDING 2: FIRE & DOMESTIC CONNECTION







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DESIGN JPQ NBB

SHEET NO. **42**

3.) BUILDING 3: FIRE & DOMESTIC CONNECTION

PK-4670-22.125_UTILITY.DWG

XXXX-XX-SDP

DRAWN DATE DEC 2022

STA 3+97.46 LINE "W-FIRE-B1"

2" DOM. SERVICE

±355 LF 12" D.I.

FIRE WATER - PRIVATE

2" DOM. WATER - PRIVATE

STA 4+51.80 LINE "W-2"

W/ OS & Y GATE VALVES

& BOLLARD PROTECTION

1 - 12" X 10" REDUCER

N: 10224097.18

E: 3138435.88

@ MIN. 18" DEPTH

N: 10224088.08 E: 3138433.37

1 - 12" 45° BEND

N: 10224086.89

E: 3138433.04

@ MIN. 18" DEPTH

1.) BUILDING 1: FIRE & DOMESTIC CONNECTION

1"=20

INSTALL:

INSTALL

STA 0+31.07 LINE "W-FIRE-B1"

- 10" RPDA (ZURN MODEL 375DA)

& SERIES 100/200 - LIFT-OFF DESIGN SAFE-T-COVER ENCLOUSURE

±10 LF 10" D.I. FIRE WATER - PUBLIC=

STA 0+21.63 LINE "W-FIRE-B1"

STA 0+20.40 LINE "W-FIRE-B1"

±21 LF 12" D.I. FIRE WATER - PUBLIC

1 - 12" X 10" REDUCER

N: 10224106.82

E: 3138438.54

INSTALL

AT MIN. 18" DEPTH

CONNECTION

12" FIRE CONNECTION

(CONTRACTOR TO END

REFER TO MEP PLANS

FOR CONTINUATION.)

N: 10224408.10

: 3138441.97

RISER

5' FROM BUILDING FACE,

STA 3+40.66 LINE "W-FIRE-B1"

STA 3+12.36 LINE "W-FIRE-B1"

- 15' WATER ESMT

' MANHOLE & RING TO BE

- PROP. 8" DIA STEEL BOLLARD

STA 0+13.33 LINE "W-FIRE-B1"

±10 LF 10" D.I. FIRE WATER - PRIVATE

LABELED WATER

W & WW CROSSING

@ MIN. 18" DEPTH

(SEE SHEET 50)

-REMOTE FDC

INSTALL:

1 - 12" 45° BEND

STA 4+26.13 LINE "W-2"

- 12" X 12" TEE

N: 10224077.19

E: 3138449.37

2 - 12" GATE VALVES

N: 10224080.74

E: 3138436.52

!!!CAUTION!!!

(DOC. NO.___

1 - 12" 45° BEND

N: 10224368.34

E: 3138510.67

1/-±368 LF 6" D.I.

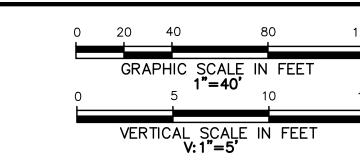
AT MIN. 18" DEPTH

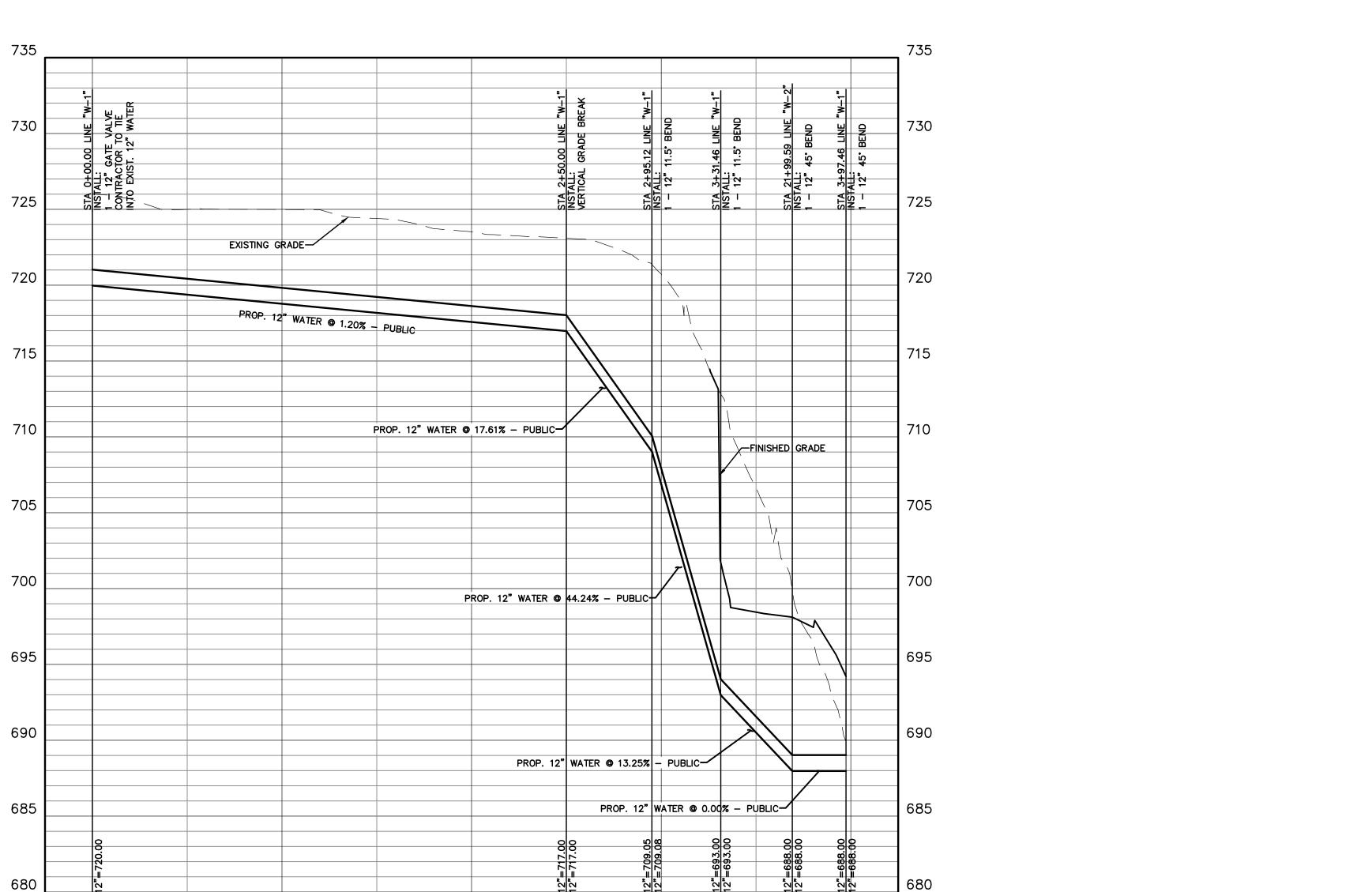
FDC LINE

1 - 12" 45° BEND

N: 10224392.96

E: 3138496.72





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3+00

-0+25 0+00

1+00

2+00

PROFILE: W-1 (PUBLIC) 5 S

4+00 4+25

BERRY CREEK BUSINESS PARK 3000, 3002, & 3004 N IH 35 NB GEORGETOWN, TEXAS, 78626 UTILITY PROFILE SHEET 1 OF

9

REVISIONS DESCRIPTION

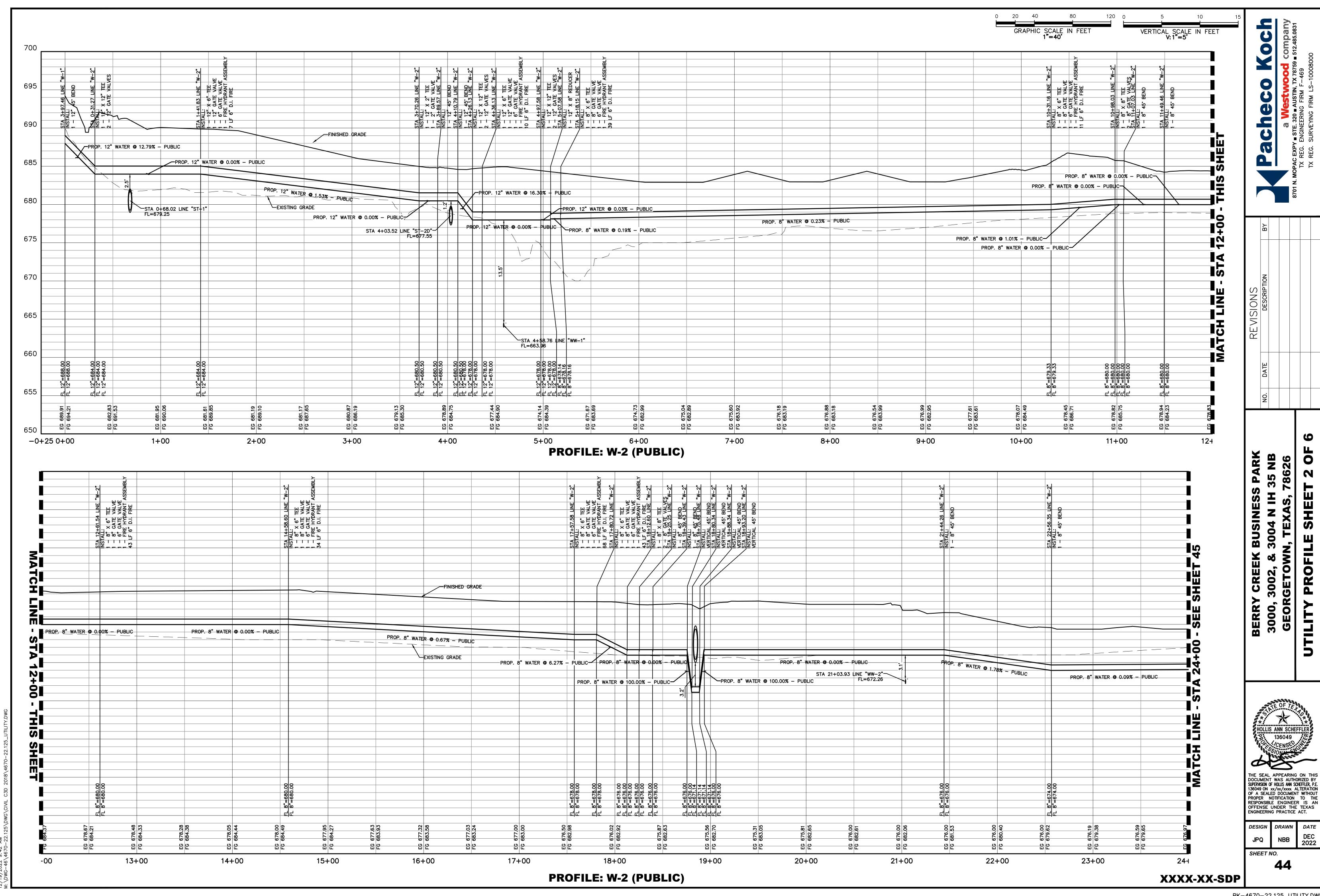
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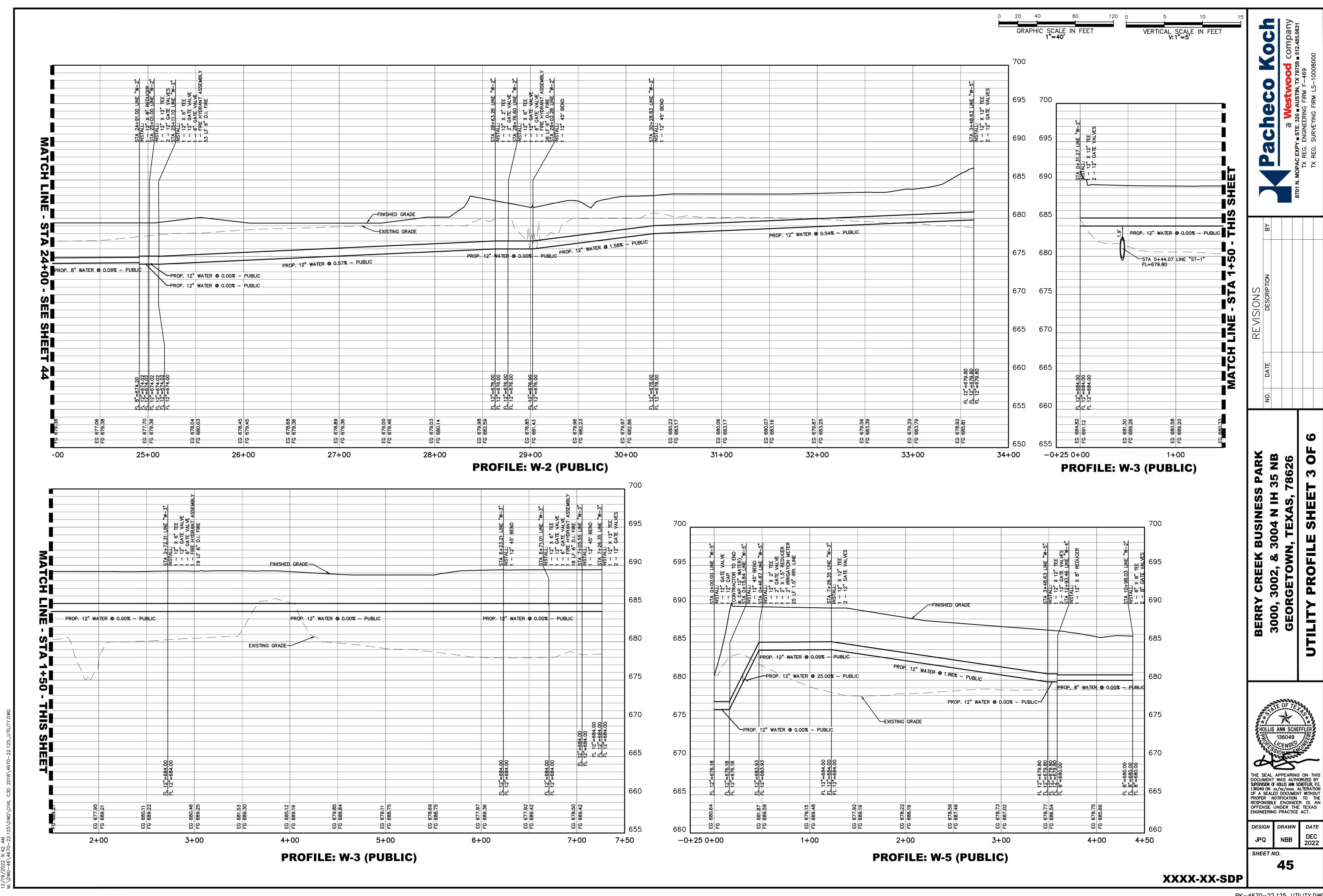
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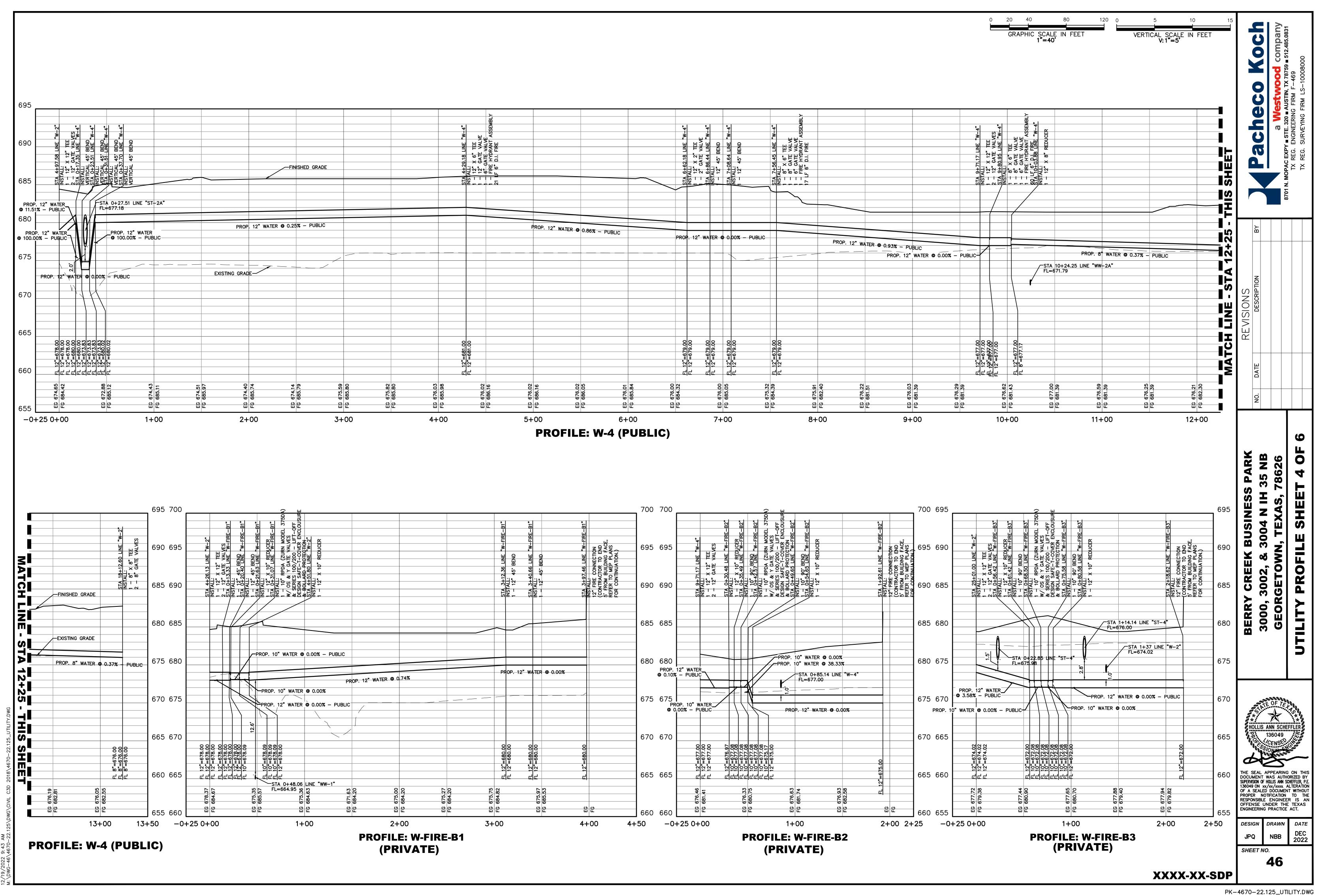
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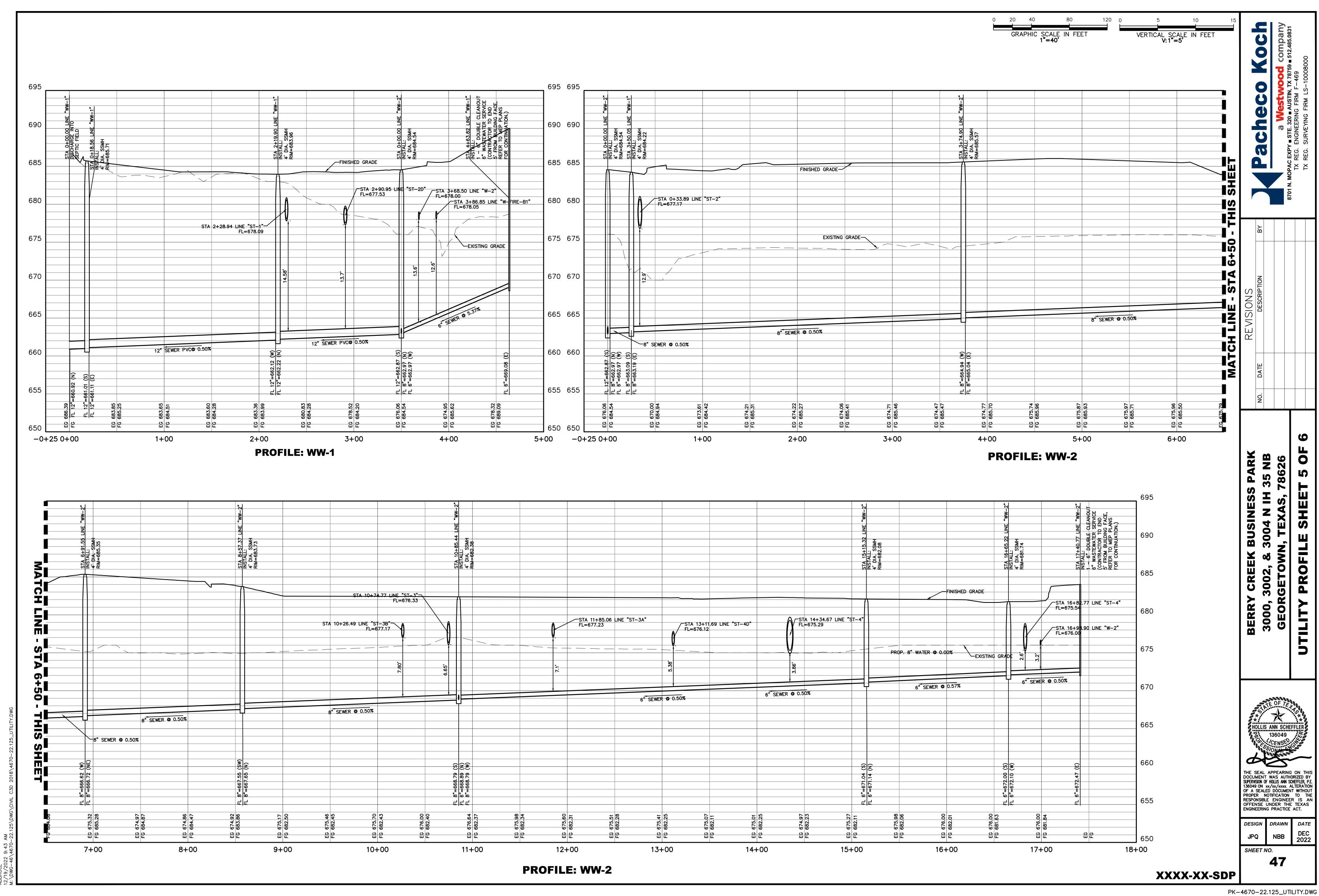
SHEET NO. **43**

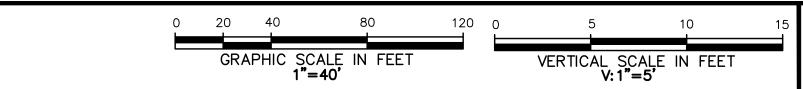
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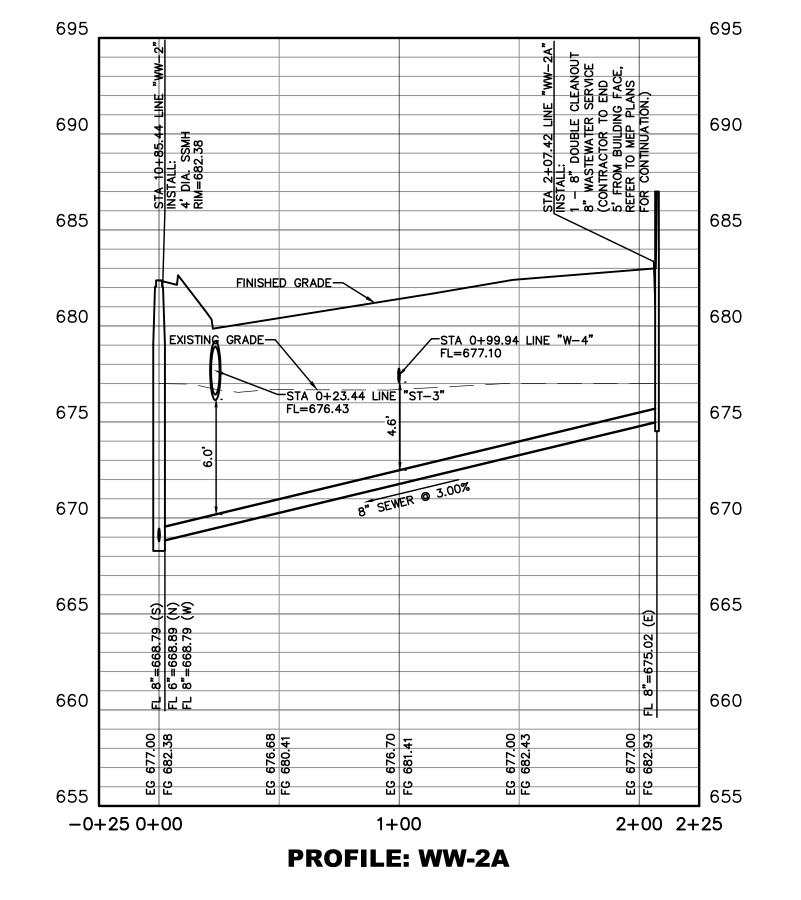
9 **OF** BERRY CREEK BUSINESS PARK 3000, 3002, & 3004 N IH 35 NB GEORGETOWN, TEXAS, 78626 SHEET 6 **PROFILE**

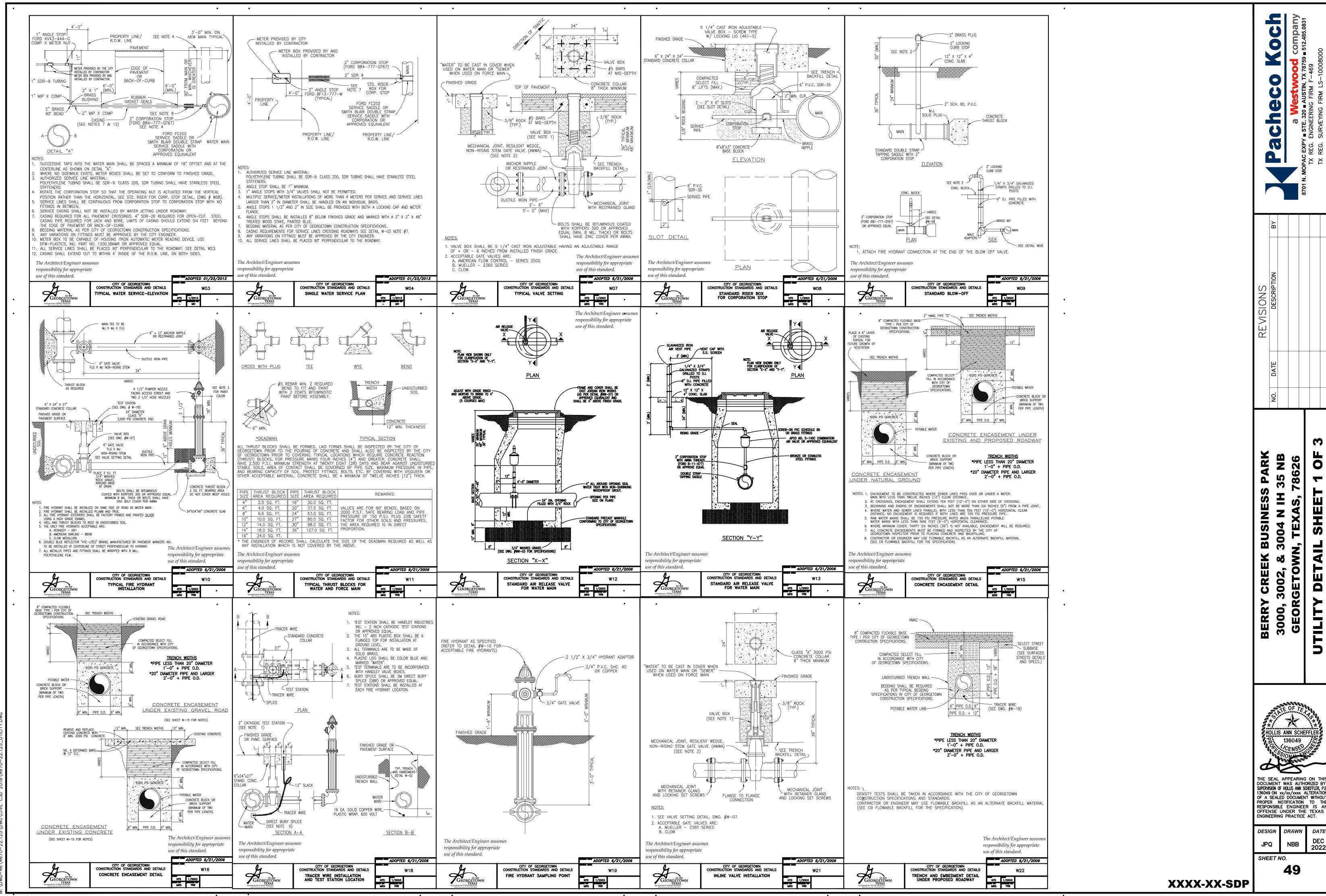
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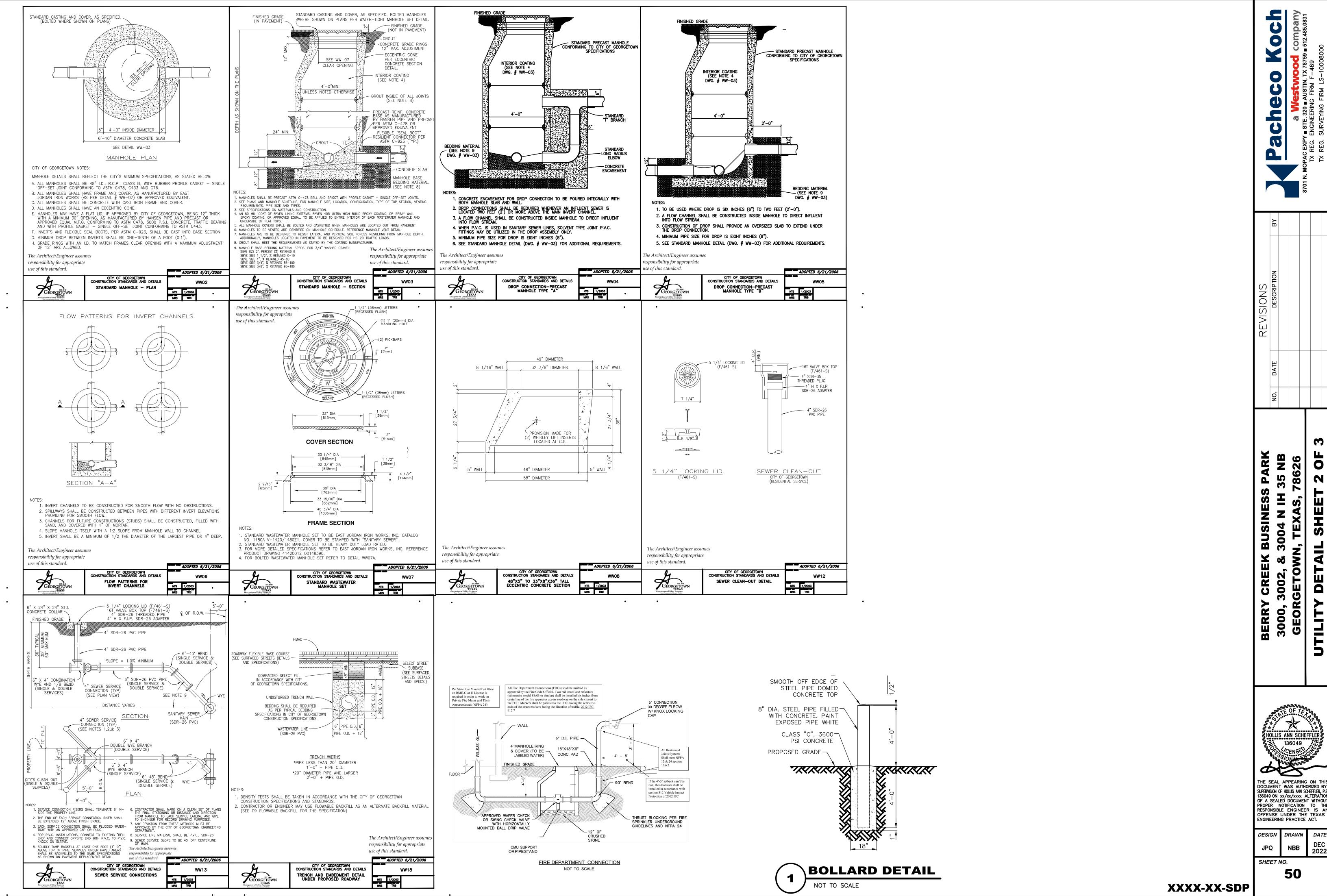
DESIGN DEC 2022 JPQ NBB

SHEET NO. 48

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NBB

DEC 2022

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GEOR

86

Reduced Pressure Detector Assembly

(Suffixes can be combined)

LM - less water meter

Air gap (Model AG)

Repair kit (rubber only)

Model 375DA (flange body) and

QT-SET Quick Test Fitting Set

CFM -

with OS & Y gate valves (standard)

with gpm meter (standard)

FG - with flanged inlet gate connection and

grooved outlet gate connection

monitor switches (2 1/2" - 10")

with grooved end butterfly valves with integral

with Post Indicator Gate Valve

GF - with flanged inlet connection and grooved

with cu ft/min meter

outlet connection

■ Thermal expansion tank (Model XT)

OS & Y Gate valve tamper switch (OSY-40)

G - with groove end gate valves

less shut-off valves (flanged body connections)

Designed for installation on water lines in fire protection systems to protect against both backsiphonage and backpressure of contaminated water into the potable water supply. The Model 375DA shall provide protection where a potential health hazard exists. Incorporates metered by-pass to detect leaks and unauthorized water use.

Standards Compliance

(Unless otherwise noted, applies to sizes 2 1/2" thru 10") ASSE® Listed 1047

 UL® Classified AWWA Compliant C550

 CSA® Certified B64.4 (4" & 6") C-UL® Classified

 FM® Approved NYC MEA 218-01-M VOL 3 · Approved by the Foundation for Cross Connection

Control and Hydraulic Research at the University of Southern California

 Meets the requirements of NSF/ANSI 61* *(0.25% MAX. WEIGHTED AVERAGE LEAD CONTENT)

By-Pass Backflow Assembly 3/4" Model 975XLD

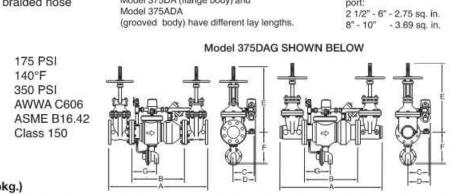
Materials Ductile Iron ASTM A 536 Main valve body Ductile Iron ASTM A 536 Access covers Coatings NSF Approved fusion epoxy finish Stainless steel, 300 Series Internals

NORYL™ Stainless Steel, 300 Series Fasteners EPDM (FDA approved) Elastomers Buna Nitrile (FDA approved) Polymers NORYL™

Stainless steel, 300 series Springs Stainless steel, braided hose Sensing line

Features Sizes: 2 1/2", 3", 4", 6", 8", 10" Maximum working water pressure

175 PSI Maximum working water temperature 350 PSI Hydrostatic test pressure End connections (Grooved for steel pipe) (Flanged bolt pattern) Class 150



Dimensions & Weights (do not include pkg.)

										DIMEN	PON	(appro	ximate	9):											VVE	EIGHT			
375 SIZ	DA	А		WITE BUTTE VAL	RFLY	LES GA VAL	SS TE	(3	C)	OS OP		OS: CLOS		WIT BUTTE VALV	RFLY	F		G		SH	SS UT- FF VES	G/ VAL	AY TE VES IGED	G/ VAL	S&Y ATE LVES OVED		VES
in.	mm	in.	mm	in.	mm	in.	mm	in.	mm	in.	mm	in.	mm	in.	mm	in.	mm	in.	mm	in.	mm	lbs.	kg	lbs.	kg	lbs.	kg	lbs.	kg
2.1/2	65	31	787	28	711	15 7/6	403	7 1/4	184	9	229	17.3/4	451	15.3/8	391	13 3/4	349	9 1/2	241	8 3/8	213	75	34	185	84	167	76	147	67
3	80	32	813	28 1/2	724	15 7/8	403	7.1/4	184	9	229	20 1/4	514	17	432	13.3/4	349	9 1/2	241	8 3/8	213	78	35	208	94	160	73	130	59
4	100	37 5/B	956	32.8/9	835	19 1/2	495	8	203	9	229	22 1/2	572	18 1/4	464	17	432	11	279	9 1/4	235	116	53	306	139	292	132	200	91
6	150	44 5/8	1133	37.5/8	956	23 1/2	597	10	264	10.1/2	267	30 1/2	775	24 1/4	616	17 1/2	445	12 3/8	314	9 1/4	235	194	88	494	224	468	212	312	142
8	200	60.7/8	1546	53.7/8	1369	37 3/4	959	11	279	15 1/2	394	37	940	28 1/2	724	16 15/16	430	15 3/8	391	16 3/4	426	382	173	858	389	810	367	556	252
10	250	63.7/8	1622	57.7/8	1470	37 3/4	959	11	279	15.1/2	394	45 5/8	1159	34 3/4	883	16 15/16	430	15.3/8	391	16 3/4	426	412	187	1230	558	1164	528	800	363

Zurn Industries, LLC | Wilkins 1747 Commerce Way, Paso Robles, CA U.S.A. 93446 Ph. 855-663-9876, Fax 805-238-5766

In Canada | Zurn Industries Limited

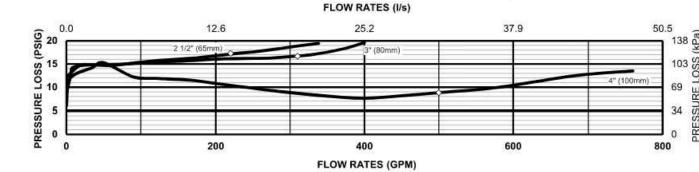
7900 Goreway Drive, Unit 10, Brampton, Ontario L6T 5W6, 877-892-5216 www.zurn.com

Rev. N Date: 3/20 Document No. BF-375DA Patent No. 5, 913, 331 Product No. Model 375DA

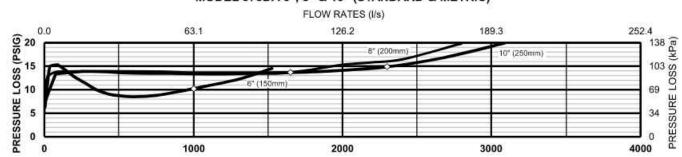
Relief Valve discharge

Flow Characteristics

ORated Flow (established by approval agencies) MODEL 375DA 2 1/2", 3" & 4" (STANDARD & METRIC)



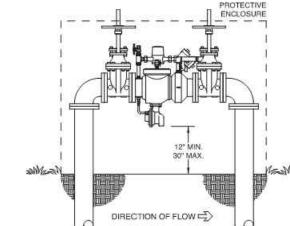
MODEL 375DA 6", 8" & 10" (STANDARD & METRIC)



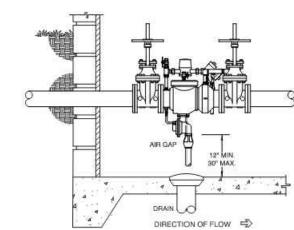
FLOW RATES (GPM)

Typical Installation Local codes shall govern installation requirements. To be installed in accordance with the manufacturer's ins and the latest edition of the Uniform Plumbing Co otherwise specified, the assembly shall be mounted mum of 12" (305mm) and a maximum of 30" (762 adequate drains with sufficient side clearance for maintenance. The installation shall be made so th the unit can be submerged.

instructions Code. Unless	2 1/2°	75	112	149	224
nted at a mini-	3"	115	173	230	346
32mm) above	4"	198	298	397	595
or testing and	6"	450	675	900	1351
that no part of	8"	780	1169	1559	2339
	10"	1229	1843	2458	3687
	12"	1763	2644	3525	5288
IVE URE	20 2				>: -



OUTDOOR INSTALLATION



Capacity thru Schedule 40 Pipe (GPM)

Pipe size 5 ft/sec 7.5 ft/sec 10 ft/sec 15 ft/sec

INDOOR INSTALLATION

Page 2 of 2

The Reduced Pressure Detector Backflow Prevention Assembly shall be certified to NSF/ANSI 61, ASSE® Listed 1047, and supplied with full port OS & Y gate valves. The main body and access cover shall be epoxy coated ductile iron (ASTM A 536), the seat ring and check valve shall be NORYL™, the stem shall be stainless steel (ASTM A 276) and the seat disc elastomers shall be EPDM. The checks and the relief valve shall be accessible for maintenance without removing the device from the line. The Reduced Pressure Detector Backflow Prevention Assembly shall be a ZURN WILKINS Model 375DA.

Zurn Industries, LLC | Wilkins

1747 Commerce Way, Paso Robles, CA U.S.A. 93446 Ph. 855-663-9876, Fax 805-238-5766 In Canada | Zurn Industries Limited 7900 Goreway Drive, Unit 10, Brampton, Ontario L6T 5W6, 877-892-5216

www.zurn.com

- Roof, walls, drain flap 5052-H32 marine grade aluminum (.050/18 gauge), mill finish, ASTM B209 outside
- Drain flap hinge and spring stainless steel • Insulation 1 1/2" (9 "R" value) minimum thickness polyisocyanurate foam laminated to a glass fiber reinforced facer (each side).
- Mounting hardware 5052-H32 marine grade aluminum
- Masonry fasteners metal hit anchors



Advantages

- Fast and simple installation, no special tools required Durable
- Lockable

Series 100/200 - Lift-off Design Insulated Enclosure for Backflow Prevention Assemblies

Specification Submittal Sheet

The enclosure is designed to provide freeze and vandal protection of above ground backflow prevention assemblies. The enclosure provides for safe and easy testing and maintenance or replacement of the backflow prevention assembly.

Heating Required

☐ Yes – see separate specification submittal sheet

Dimensions

Model	Insi	ide Diam	eter	Co	ncrete F	Pad	Weight	Drain	Opening
2	W	L	Н	w	L	Н	526	W	Н
100S-AL	7	32	22	14	39	4	26	7	5 1/4
200S-AL	14	43	29	21	50	4	44	14	5 1/4

All dimensions in inches.

Specifications

A freeze and vandal protection enclosure shall be installed with above ground backflow prevention assemblies. The enclosure shall be mounted securely to a concrete pad and completely removable by way of a lockable stainless steel rod only. All mounting brackets shall be on the inside of the enclosure. The enclosure shall be constructed of 5052-H32 marine grade aluminum with a minimum R-value of 9 in the walls and roof. Drain panel shall be fully insulated and designed to remain closed except when backflow prevention device is discharging water. Drain panel shall be sized to accommodate the maximum discharge of a 1" device (Series 100) or 2" device (Series 200). All mounting hardware shall be furnished. The enclosure shall be certified to ASSE Standard 1060 (Class I, Class II and Class III). The insulated lift-off enclosure shall be a Safe-T-Cover Series 100/200.

> Hydrocowl, Inc. Nashville, TN • Phone 1-800-245-6333 FAX (615) 259-4481 • www.safe-t-cover.com

SS2 1212



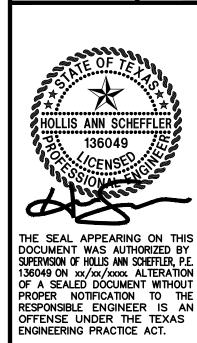
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BUSINESS PA

BERRY CREEK

GEORGETOW UTILITY

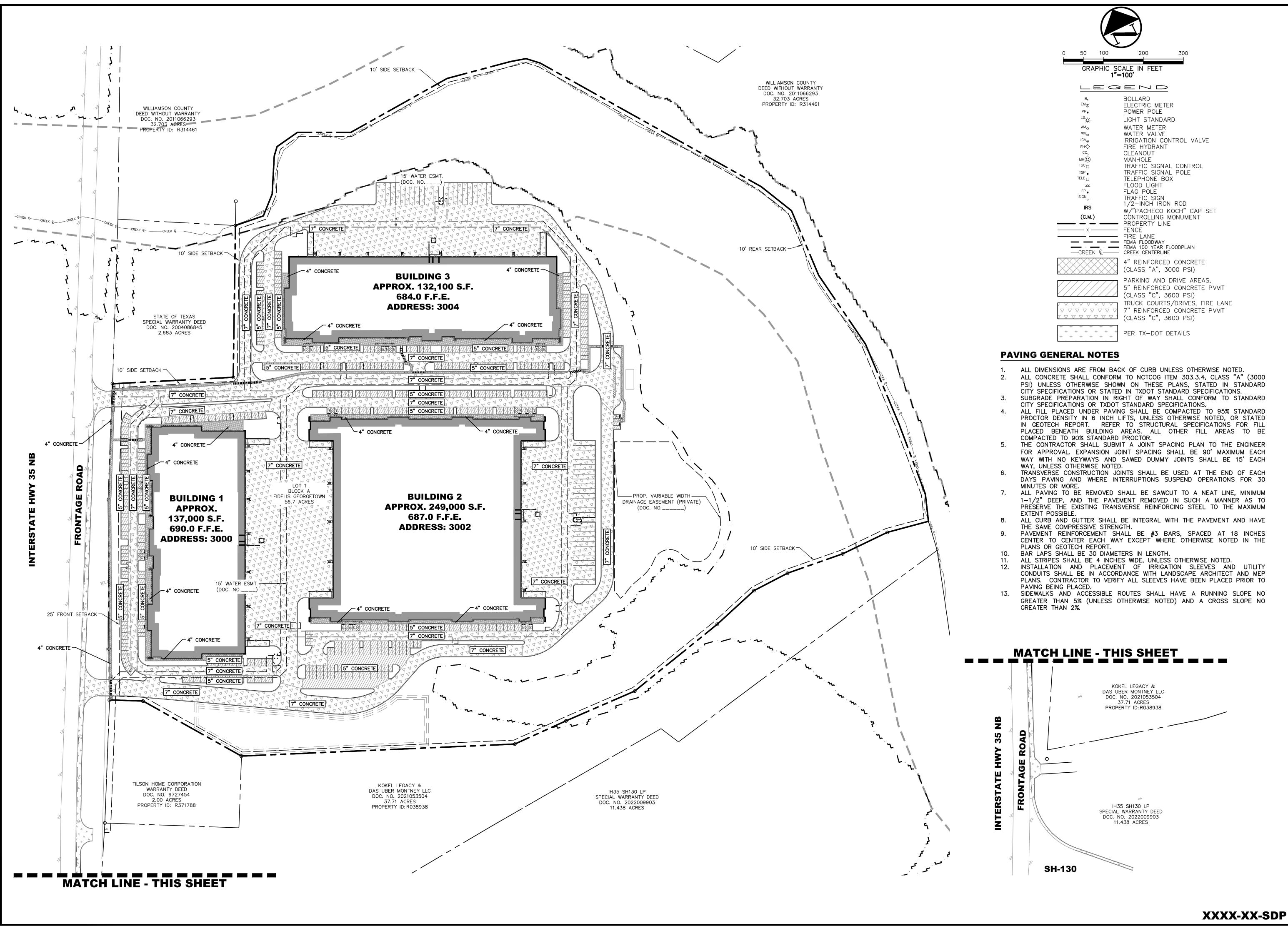


DEC 2022

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SHEET NO. **51**

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8701 N. MOPAC EXPY = STE. 320 = AUSTIN, TX 78759 = 512.485.0831

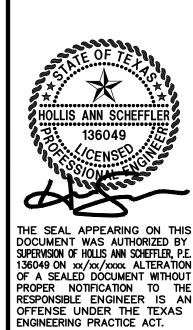
TX REG. ENGINEERING FIRM F-469

TX REG. SURVEYING FIRM LS-10008000

TOWN, TEXAS, 786. AVING PLAN

BERRY CREEK BUSINES 3000, 3002, & 3004 N IH GEORGETOWN, TEXAS,

PA

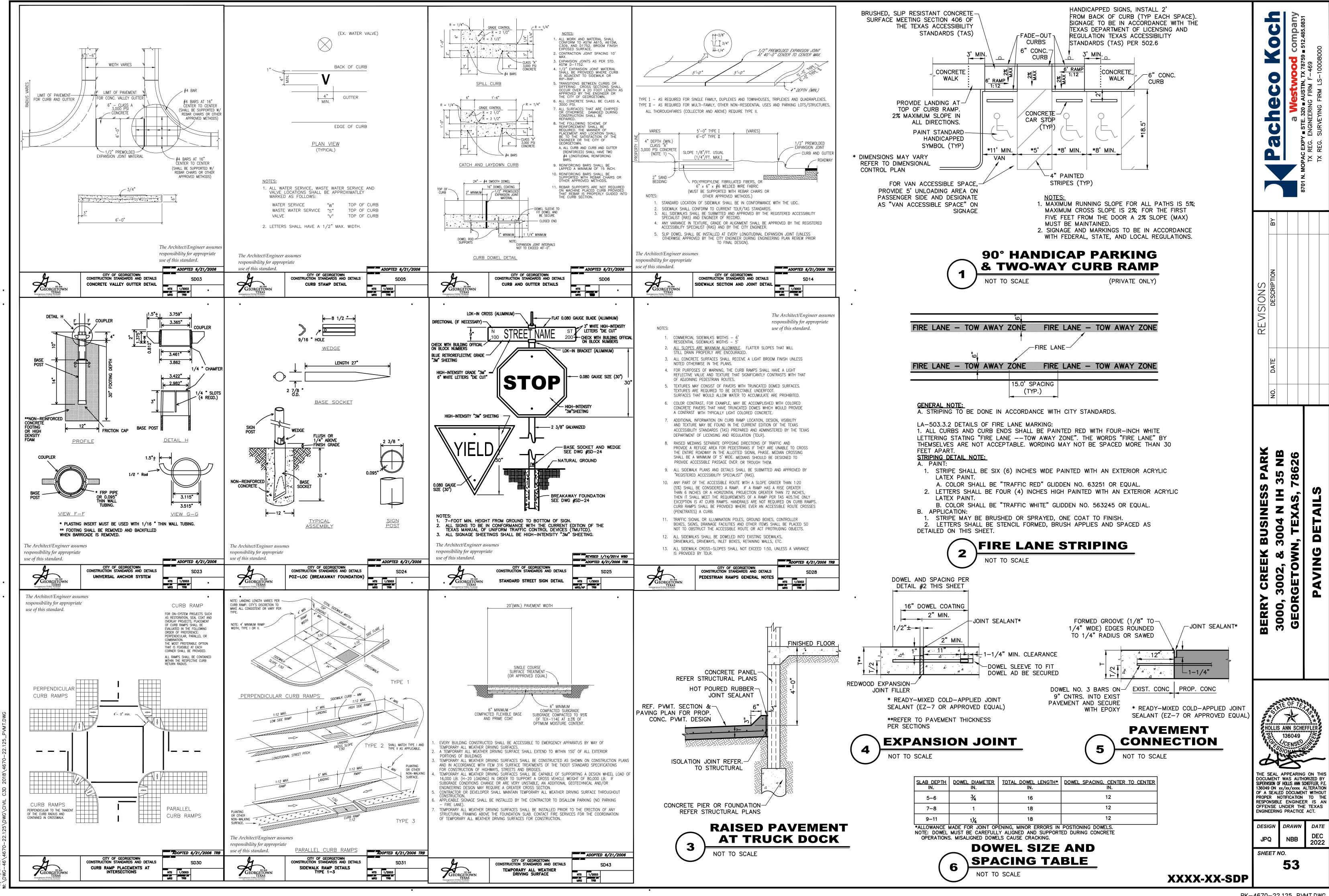


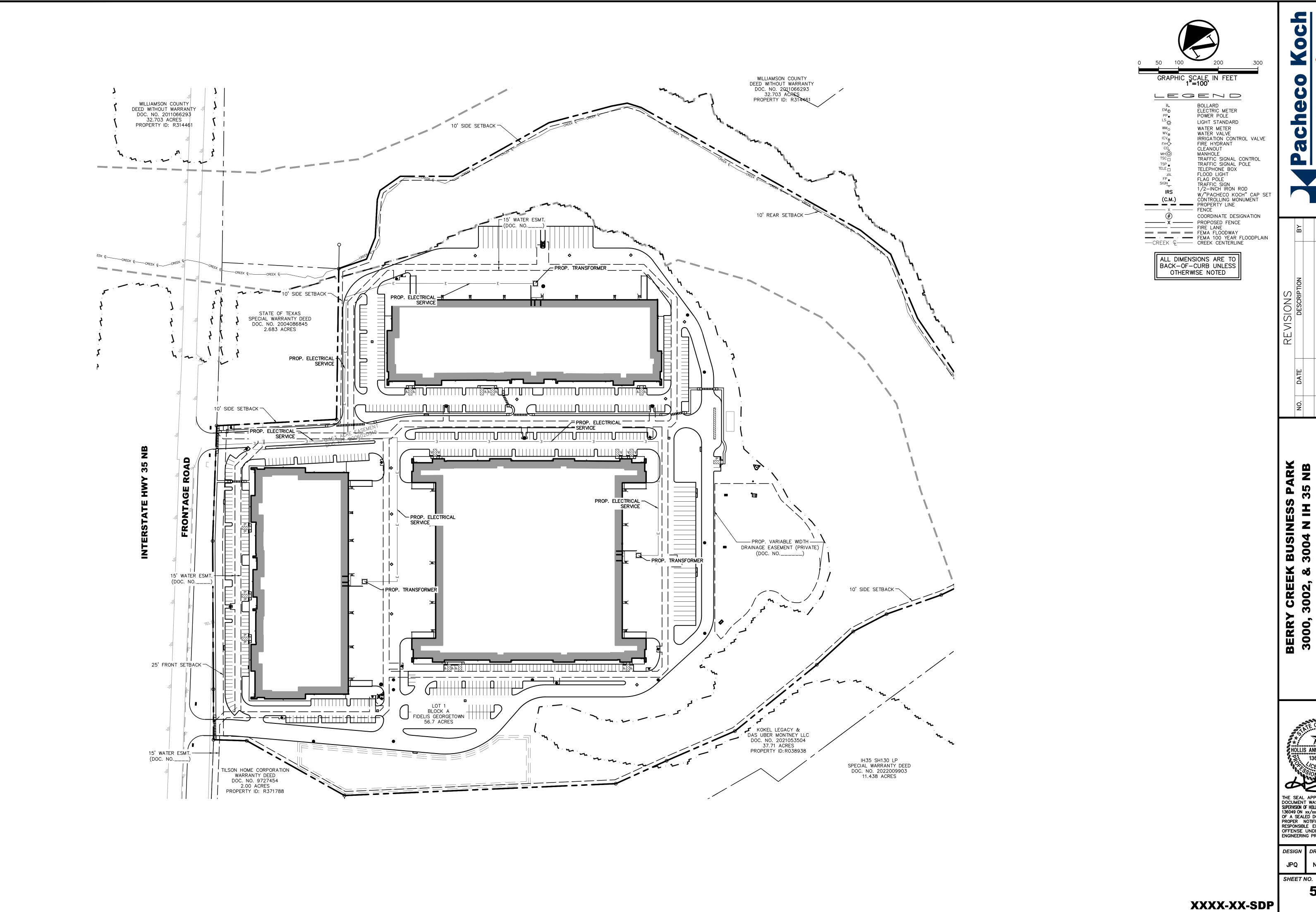
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JPQ NBB DEC 2022

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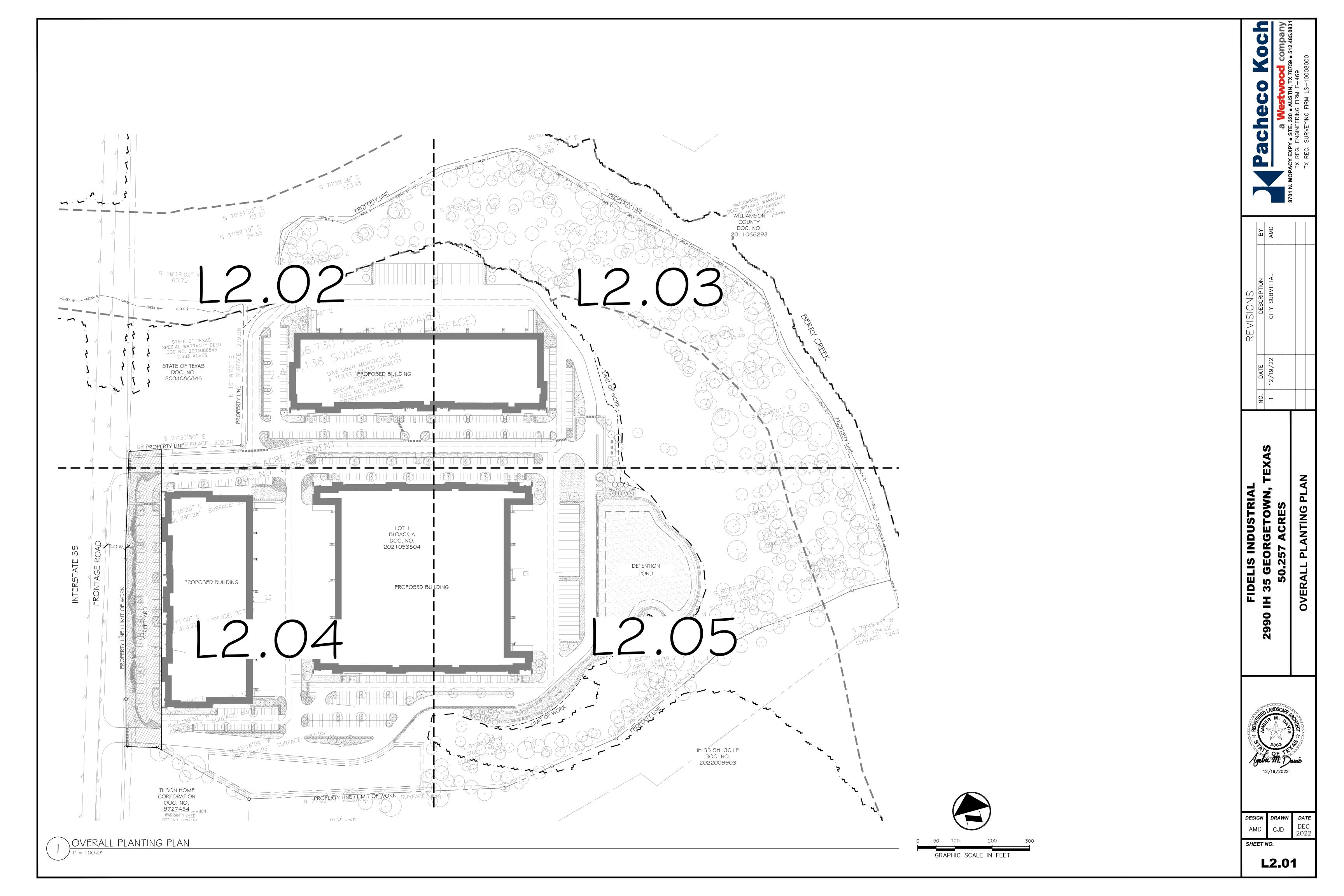
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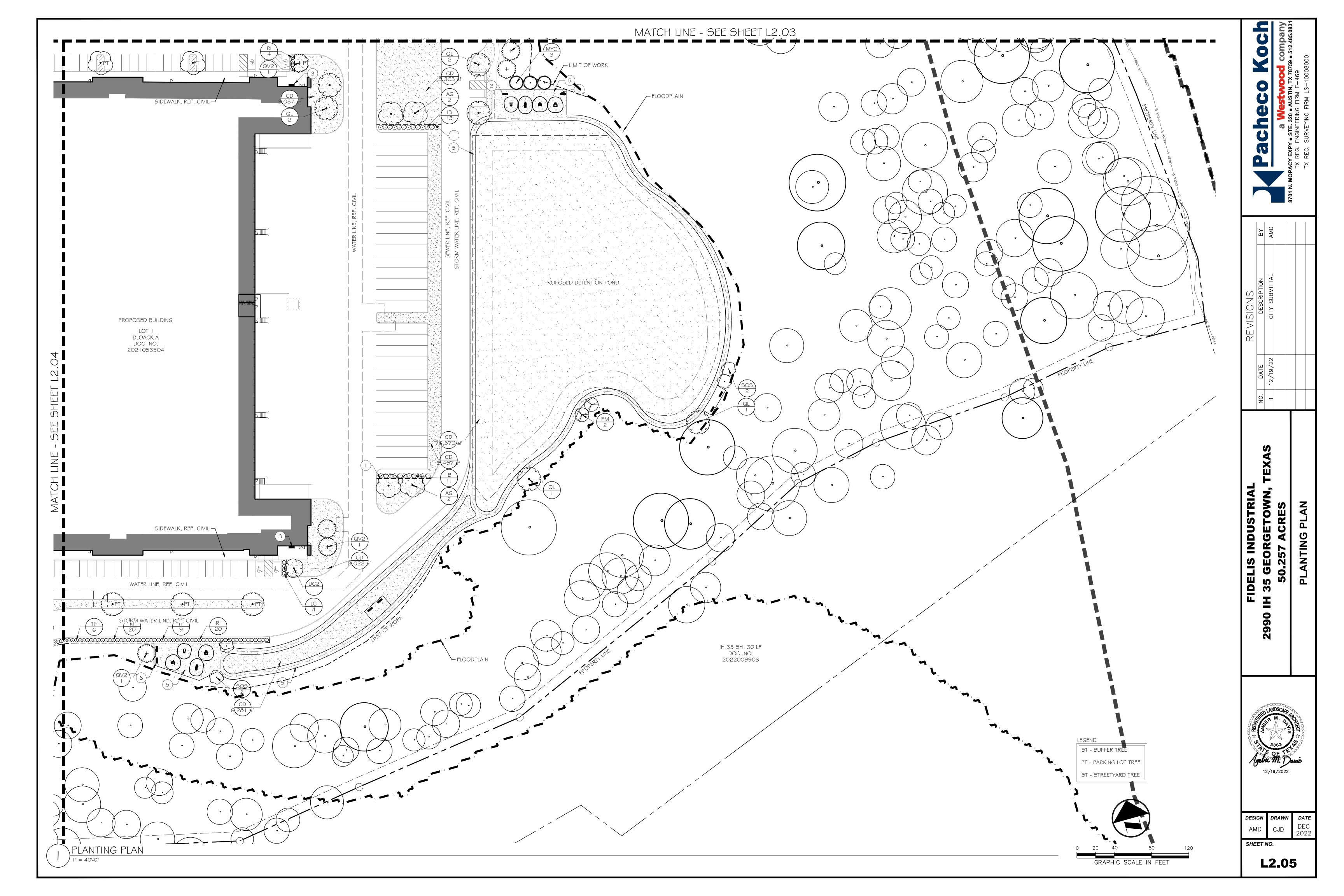




786 GEORGETOW THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY SUPERVISION OF HOLLIS ANN SCHEFFLER, P.E. 136049 ON xx/xx/xxxx ALTERATION OF A SEALED DOCUMENT WITHOUT PROPER NOTIFICATION TO THE RESPONSIBLE ENGINEER IS AN OFFENSE UNDER THE TEXAS ENGINEERING PRACTICE ACT. DEC 2022 NBB **54**

PK-4670-22.125_ESP.DWG





Attachment G – Inspection, Maintenance, Repair and Retrofit Plan

Batch Detention

- Batch detention. Basins may have somewhat higher maintenance requirements than an extended detention basin since they are active stormwater controls. The maintenance activities are identical to those of extended detention basins with the addition of maintenance and inspections of the automatic controller and the valve at the outlet.
- Inspections. Inspections should take place a minimum of twice a year. One inspection should take place during wet weather to determine if the basin is meeting the target detention time of 12 hours and a drawdown time of no more than 48 hours. The remaining inspections should occur between storm events so that manual operation of the valve and controller can be verified. The level sensor in the basin should be inspected and any debris or sediment in the area should be removed. The outlet structure and the trash screen should be inspected for signs of clogging. Debris and sediment should be removed from the orifice and outlet(s) as described in previous sections. Debris obstructing the valve should be removed. During each inspection, erosion areas inside and downstream of this BMP should be identified and repaired/revegetated immediately.
- Mowing. The basin, basin side-slopes, and embankment of the basin must be mowed to
 prevent woody growth and control weeds. A mulching mower should be used, or the
 grass clippings should be caught and removed. Mowing should take place at least twice
 a year, or more frequently if vegetation exceeds 18 inches in height. More frequent
 mowing to maintain aesthetic appeal may be necessary in landscaped areas
- Litter and Debris Removal, Litter and debris removal should take place at least twice a year, as part of the periodic mowing operations and inspections. Debris and litter should be removed from the surface of the basin. Particular attention should be paid to floatable debris around the outlet structure. The outlet should be checked for possible Clogging or obstructions and any debris removed
- Erosion control. The basin side slopes and embankment all may periodically suffer from slumping and erosion. To correct these problems, corrective action, such as regrading and revegetation, may be necessary. Correction of erosion control should take place whenever required based on the periodic inspections.
- Nuisance Control. Standing water or soggy conditions may occur in the basin. Some standing water may occur after a storm event since the valve may close with 2 to 3 inches of water in the basin. Some flow into the basin may also occur between storms due to spring flow and residential water use that enters the storm sewer system. Twice a year, the facility should be evaluated in terms of nuisance control (insects, weeds, odors, algae, etc.).
- Structural Repairs and Replacement. With each inspection, any damage to structural elements of the basin (pipes, concrete drainage structures, retaining walls, etc.) should be identified and repaired immediately. An example of this type of repair can include patching of cracked concrete, sealing of voids, removal of vegetation from cracks and joints. The various inlet/outlet structures in a basin will eventually deteriorate and must be replaced.
- Sediment Removal, A properly designed batch detention basin will accumulate quantities of sediment over time. The accumulated sediment can detract from the appearance of the facility and reduce the pollutant removal performance of the facility. The sediment also tends to accumulate near the outlet structure and can interfere with



- the level sensor operation. Sediment shall be removed from the basin at least every 5 years, when sediment depth exceeds 6 inches, when the sediment interferes with the level sensor or when the basin does not drain within 48 hours. Care should be taken not to compromise the basin lining during maintenance.
- Logic Controller. The Logic Controller should be inspected as part of the twice yearly investigations. Verify that the external indicators (active, cycle in progress) are operating properly by turning the controller off and on, and by initiating a cycle by triggering the level sensor in the basin. The valve should be manually opened and closed using the open/close switch to verify valve operation and to assist in inspecting the valve for debris, The solar panel should be inspected and any dust or debris on the panel should be carefully removed. The controller and all other circuitry and wiring should be inspected for signs of corrosion, damage from insects, water leaks, or other damage. At the end of the inspection, the controller should be reset.

Engineer Signature	
Hollis Scheffler, P.E.	
Printed Name	

Project Manager

12/05/2022 Date

Owner Signature

KEVIN C ONGIL

Printed Name

Application Fee Form

Texas Commission on Environmental Quality

Name of Proposed Regulated Entity: <u>Berry Creek Business Park</u> Regulated Entity Location: Georgetown; Williamson County

Name of Customer: Kevin O'Neil

Phone: <u>512-485-0831</u> Contact Person: Hollis Scheffler, P.E.

Customer Reference Number (if issued):CN 603034521

Regulated Entity Reference Number (if issued):RN Not yet assigned

Austin Regional Office (3373)		, <u></u>
Hays	Travis	
San Antonio Regional Office (33	62)	
Bexar	Medina	Uvalde
Comal	Kinney	
	Quality. Your canceled	or money order, payable to the Texas check will serve as your receipt. This payment is being submitted to:
Austin Regional Office		San Antonio Regional Office
Mailed to: TCEQ - Cashier		Overnight Delivery to: TCEQ - Cashier
Revenues Section		12100 Park 35 Circle
Mail Code 214		Building A, 3rd Floor
P.O. Box 13088		Austin, TX 78753
Austin, TX 78711-3088		(512)239-0357
Site Location (Check All That Ap	ply):	
Recharge Zone	Contributing Zon	e Transition Zone

	<u>—</u>	
Type of Plan	Size	Fee Due
Water Pollution Abatement Plan, Contributing Zone		
Plan: One Single Family Residential Dwelling	Acres	\$
Water Pollution Abatement Plan, Contributing Zone		
Plan: Multiple Single Family Residential and Parks	Acres	\$
Water Pollution Abatement Plan, Contributing Zone		
Plan: Non-residential	56.73 Acres	\$ 8,000
Sewage Collection System	L.F.	\$
Lift Stations without sewer lines	Acres	\$
Underground or Aboveground Storage Tank Facility	Tanks	\$
Piping System(s)(only)	Each	\$
Exception	Each	\$
Extension of Time	Each	\$

Signature: _	H	Date: 12/20/2022
-		

Application Fee Schedule

Texas Commission on Environmental Quality

Edwards Aquifer Protection Program 30 TAC Chapter 213 (effective 05/01/2008)

Water Pollution Abatement Plans and Modifications

Contributing Zone Plans and Modifications

Project	Project Area in Acres	Fee
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5	\$1,500
	5 < 10	\$3,000
	10 < 40	\$4,000
	40 < 100	\$6,500
	100 < 500	\$8,000
	≥ 500	\$10,000
Non-residential (Commercial, industrial, institutional,	< 1	\$3,000
multi-family residential, schools, and other sites	1 < 5	\$4,000
where regulated activities will occur)	5 < 10	\$5,000
	10 < 40	\$6,500
	40 < 100	\$8,000
	≥ 100	\$10,000

Organized Sewage Collection Systems and Modifications

Project	Cost per Linear Foot	Minimum Fee- Maximum Fee
Sewage Collection Systems	\$0.50	\$650 - \$6,500

Underground and Aboveground Storage Tank System Facility Plans and Modifications

Project	Cost per Tank or Piping System	Minimum Fee- Maximum Fee
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500

Exception Requests

Project	Fee
Exception Request	\$500

Extension of Time Requests

Project	Fee
Extension of Time Request	\$150



TCEQ Core Data Form

TCEQ Use Only	

For detailed instructions regarding completion of this form, please read the Core Data Form Instructions or call 512-239-5175.

SECTION I: General Information

		. ,											
 1. Reason for Submission (If other is checked please describe in space provided.) New Permit, Registration or Authorization (Core Data Form should be submitted with the program application.) 													
			th the re	enewa	al form))		Othe		E.C. D.C.	N	<i>C</i>	
2. Customer CN 6030		e Number <i>(if i</i> ss	suea)	Follow to for CN control	or RN i		s in					r issuea)	
SECTION	II: Cu	stomer Info	ormation				L						
4. General C	ustomer lı	nformation	5. Effective	Date for	r Cus	tomer	Inforr	natio	on Up	date	s (mm/dd/yyyy)	12/20/	/2022
New Cust			· —	Jpdate to								Regulated E	Intity Ownership
											Public Accounts)		
		ne submitted State (SOS)	•	-				•				rrent and	active with the
		ne (If an individua					ibiic i			•	stomer, enter previ	ous Custome	er below.
			, print lact riamo	mot. og.	. 200,	001111)			<u></u>	<u> </u>	tomor, ontor provi	oud Guotonii	<u> </u>
Fidelis Re			. = 1/ 0/ / =					4				140 51111	
7. 1 X SOS/C 08042648	7. TX SOS/CPA Filing Number 8. TX State 0804264832 3208139				11 digits	s)					I Tax ID (9 digits)	10. DUN	S Number (if applicable)
			<u> </u>	103					87-3066602				
11. Type of (Corporati		☐ Individual Partnership: ☐ General									
12. Number		County Federal	State Other			Sole Pi	ropriet		•		Other: Limited I endently Owned		
	21-100	101-250	251-500	<u></u> 5	01 an	d highe	er		3. 		endently Owned ☐ No	and Opera	teu r
14. Custome	r Role (Pro	posed or Actual) -	- as it relates to t	the Regu	ılated L	Entity lis	sted on	this t	form. F	Pleas	e check one of the	following	
Owner		Operat	tor		Ov	vner &	Opera	ator					
Occupatio	nal License	ee 🗵 Respo	nsible Party		Vo	luntary	/ Clear	nup A	Applic	ant	Other:		
	8140 V	Valnut Hill L	Lane, Suite	400									
15. Mailing Address:													
	City	Dallas		Sta	ate	TX		ZIP	7	523	31	ZIP + 4	
16. Country	Mailing Inf	ormation (if outsi	ide USA)				17. E	-Mai	l Add	ress	(if applicable)		
							skin	nosl	h@f	rplt	d.com		
18. Telephor	e Number			19. Ext	ensio	n or C	ode				20. Fax Numbe	r (if applicat	ole)
(512)55	4-6191										()	-	
SECTION	III: Re	egulated En	ntity Infor	matio	on								
						y" is se	elected	belo	ow this	s forn	n should be acco	mpanied by	a permit application)
New Reg	ulated Enti	y 🔲 Update	to Regulated E	Entity Na	ame	□ ι	Jpdate	to R	Regula	ated I	Entity Information		, , ,
		•	•	•		d in c	order	to r	meet	TC	EQ Agency D	ata Stano	lards (removal
		ndings such			•								
		ame (Enter name	of the site where	the regu	ulated	action i	is takin	g plac	ce.)				
Berry Creek Business Park													

TCEQ-10400 (02/21) Page 1 of 3

23. Street Address of	3000	N IH	35 NB									
the Regulated Entity:												
(No PO Boxes)	City	G	eorgetow	n s	tate	TX	ZIP	78620	5	ZIP + 4		
24. County	Willi	amson	1	•								
		Enter	Physical Lo	ocation	n Descript	tion if no str	eet address	s is provi	ded.			
25. Description to	This 1	nroner	tv is loca	ted at	t the end	d of the W	illiam So	cotsmai	n drive	down the	I 35 Service	
Physical Location:	Road		., 15 15 5 1			01 0110 11		0 0011100			200 201 / 100	
26. Nearest City								State		Nea	rest ZIP Code	
Georgetown								TX		786	533	
27. Latitude (N) In Decin	nal:	30	.688779			28. L	ongitude (\	N) In Dec	imal:	-97.6526	87	
Degrees	Minutes		(Seconds		Degre	es	N	inutes		Seconds	
30		41			19.6		37		3	89	09.7	
29. Primary SIC Code (4	digits)	30. Sec	ondary SIC	Code	(4 digits)	31. Prima (5 or 6 digits	ry NAICS C	ode	32. Se (5 or 6 c	econdary NA digits)	ICS Code	
6531						53531						
33. What is the Primary	Busines	s of this	s entity?	(Do not r	epeat the SI	C or NAICS des	cription.)					
Industrial Warehou	se		<u>-</u>									
						3000	N IH 35 NB					
34. Mailing												
Address:	City Georgetown			,	State	тх	ZIP		3664	ZIP + 4		
35. E-Mail Address:				•	Otato	I A	Lii		7004	E II · 4		
36. Telepho		nber		3	7. Extensi	ion or Code		38	Fax Nur	mber <i>(if appl</i>	icable)	
	554-6191				or. Extension of oods oo. 1 dx Hd) <u>-</u>	<u> </u>	
9. TCEQ Programs and ID orm. See the Core Data Form) Numbe	ers Check			rite in the po	ermits/registra	tion numbers	that will be	e affected	by the updates	submitted on this	
Dam Safety		stricts	Tilonal guldan		dwards Aq	uifer	☐ Emissi	ons Invent	orv Air	□ Industria	I Hazardous Waste	
		Ju 1010		,				0110 111101110	21 7 111		Trazardous Trasto	
☐ Municipal Solid Waste	☐ Ne	w Source	Review Air	n/a)SSF		☐ Petrole	um Storag	e Tank	☐ PWS		
,												
Sludge	☐ Sto	orm Wate	r	П	itle V Air		Tires			Used Oil		
☐ Voluntary Cleanup	☐ Wa	aste Wate	er	□ V	Vastewater	Agriculture	re Water Rights			Other:		
SECTION IV: Pre	parer	Info	rmation									
40. Name: Hollis Schef	fler, P.	E.				41. Title:	Senio	or Proje	ct Man	ager		
42. Telephone Number	43. Ext./0	Code	44. Fax	(Numb	oer	45. E-M	ail Address	i				
(512)485-0831			() .	-	hollis.	scheffler	@west	woodps	s.com		
SECTION V: Aut	horiz	ed Sig	nature									
6. By my signature below,												
ignature authority to submi	1 certify	, to the t	oest of my k	nowled	lge, that th	e information	n provided i	n this fori	n is true a	and complete,	and that I have	

Name (In Print):Hollis SchefflerPhone:(512) 485- 0831

Job Title:

Senior Project Manager

Company:

Westwood Professional Services

TCEQ-10400 (02/21) Page 2 of 3

Signature:		Date:	12/20/2022
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TCEQ-10400 (02/21) Page 3 of 3

Agent Authorization Form

For Required Signature Edwards Aquifer Protection Program Relating to 30 TAC Chapter 213 Effective June 1, 1999

	Kevin O'Neil	
	Print Name	
	President	
	Title - Owner/President/Other	
of	Fidelis Realty Partners	
	Corporation/Partnership/Entity Name	
have authorized	Hollis Scheffler, P.E.	
·····	Print Name of Agent/Engineer	
of	Westwood Professional Services	
	Print Name of Firm	

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

- 1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
- 2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
- 3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
- 4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
- 5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:

Applicant's Signature

4 C.O'Ville 12/6/22 Date

THE STATE OF TEXAS §

County of DALLAS §

BEFORE ME, the undersigned authority, on this day personally appeared <u>KEVIN O'NEIL</u> known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this 6th day of DECEMBER, 2022.



Melanie Messica NOTARY PUBLIC

MELANIE MESSICK
Typed or Printed Name of Notary

MY COMMISSION EXPIRES: DECEMBER 27, 2025

Owner Authorization Form

for Required Signature for submitting and signing an application for an Edwards Aquifer Protection Plan (Plan) and conducting regulated activities in accordance with an approved Plan.

Texas Commission on Environmental Quality Edwards Aquifer Protection Program

Relating to the Edwards Aquifer Rules of Title 30 of the Texas Administrative Code (30 TAC), Chapter 213

Effective June 1, 1999

Effective June 1, 1999	
Land Owner Authorization	
I. LARRY D. KOKEL OF	KOKEL LEGACY LLC & DAS UBER MONTNEY LLC
I, LARRY D. KOKEL of Land Owner Name (Individual)	Firm (applicable to Legal Entities)
am the Owner of Record or Title Holder of the	property located at:
5401 IH 35 N, Georgetown, TX 78633	
(Legal description of the property	referenced in the application)
and being duly authorized under 30 TAC \S 213. and \S 213.23(d) to submit and sign an applicat	
Fidelis Realty Partners	
(Applicant Name / Plan Holder	Legal Entity or Individual))
to conduct:	
Water Pollution Abatement Plan and Se	wer Collection System Plan
(Description of the propose	ed regulated activities)
on the property described above or at:	
3000, 3002, 3004 N IH 35 NB, Georget	own, Texas 78626
(If applicable to a precise location for the	authorized regulated activities)
Land Owner Acknowledgement	
I, Land Owner Name (Individual)	KOKEL LEGACY LLC & DAS UBER MONTNEY LLC
Land Owner Name (Individual)	Firm (applicable to Legal Entities)
understand that while Fidelis Realty Partners Applicant Name	ers
Applicant Name	/ Plan Holder (Legal Entity or Individual)
is responsible for compliance with the approve special conditions of the approved Plan throug	

1 of 3

I, Land Owner Name (Individual)	Firm (applicable to Legal Entities)
as Owner of Record or Title Holder of the propresponsible for ensuring that compliance with Plan and any special conditions of the approve implementation, is achieved even if the responsorsess and control of the property referenced contractually assumed by another legal entity. I, Arry D. Koke of Land Owner Name (Individual)	the approved or conditionally approved d Plan, through all phases of Plan sibility for compliance and the right to
Land Owner Name (Individual)	Firm (applicable to Legal Entities)
further understand that any failure to comply of Director's approval is a violation and is subject penalties as provided under 30 TAC § 213.10 (may also be subject to civil penalties and injunity).	to administrative rule or orders and relating to Enforcement). Such violation
Land Owner Signature Land Owner Signature	12/6/22 Date
THE STATE OF § TEXAS	
County of § WILLIAMSON	
BEFORE ME, the undersigned authority, on this day the person whose name is subscribed to the forego that (s)he executed same for the purpose and consi	ing instrument and acknowledged to me ideration therein expressed.
GIVEN under my hand and seal of office on thi	day of December NOTARY PUBLIC Jeannie C. Coffman Typed or Printed Name of Notary
MY COMM	MISSION EXPIRES:
Attached: (Mark all that apply) Lease Agreement Signed Contract Deed Recorded Easement	JEANNIE C. COFFMAN Notary Public, State of Texas Comm. Expires 02-02-2024 Notary ID 1626743

Other legally binding document

Applicant Acknowleagement	
I, Kevin O'Neil of	Fidelis Realty Partners
Applicant Name (Individual)	Firm (applicable to Legal Entities)
acknowledge that Larry Kokel Land Owner Name (Legal Enti	tv or Individual)
has provided Fidelis Realty Partners Applicant Name (Legal Entity	y or Individual)
with the right to possess and control the propert Protection Plan (Plan).	ty referenced in the Edwards Aquifer
I understand that Fidelis Realty Partners Applicant Name (Legal Entity	
Applicant Name (Legal Entity	or Individual)
is responsible, contractually or not, for compliant approved Plan and any special conditions of the Plan implementation. I further understand that for the Executive Director's approval is a violation or orders and penalties as provided under § 213 violation may also be subject to civil penalties at Applicant Signature	approved Plan through all phases of ailure to comply with any condition and is subject to administrative rule 10 (relating to Enforcement). Such
Applicant Signature Applicant Signature	12/6/22 Date
THE STATE OF § TEXAS	
County of § _DALLAS	
BEFORE ME, the undersigned authority, on this day per the person whose name is subscribed to the foregoin, that (s)he executed same for the purpose and consider	g instrument and acknowledged to me
GIVEN under my hand and seal of office on this_	day of <u>DECEMBER</u> , 2027
MELANIE MESSICK My Notary ID # 131078242 Expires December 27, 2025	MUMILIANIE MESSICK Typed or Printed Name of Notary

3 of 3

MY COMMISSION EXPIRES: DECEMBER 27, ZOZ5

Owner Authorization Form

for Required Signature for submitting and signing an application for an Edwards Aquifer Protection Plan (Plan) and conducting regulated activities in accordance with an approved Plan.

Texas Commission on Environmental Quality Edwards Aquifer Protection Program

Relating to the Edwards Aquifer Rules of Title 30 of the Texas Administrative Code (30 TAC), Chapter 213 *Effective June 1, 1999*

1	and	Owner	Δ	uth	oriz	atio	ท
•	JATILA	INVITED	/		.,, ,,		"

I, Dale Illig, President

Land Owner Name (Individual)

DAS UBER MONTNEY LLC

Firm (applicable to Legal Entities)

am the Owner of Record or Title Holder of the property located at:

of

5401 IH 35 N, Georgetown, TX 78633

(Legal description of the property referenced in the application)

and being duly authorized under 30 TAC § 213.4(c)(2) and § 213.4(d)(1) or § 213.23(c)(2) and § 213.23(d) to submit and sign an application for a Plan, do hereby authorize:

Fidelis Realty Partners

(Applicant Name / Plan Holder (Legal Entity or Individual))

to conduct:

Water Pollution Abatement Plan and Sewer Collection System Plan

(Description of the proposed regulated activities)

on the property described above or at:

3000, 3002, 3004 N IH 35 NB, Georgetown, Texas 78626

(If applicable to a precise location for the authorized regulated activities)

Land Owner Acknowledgement

I, Dale Illig, President
Land Owner Name (Individual)

DAS UBER MONTNEY LLC

Firm (applicable to Legal Entities)

understand that while Fidelis Realty Partners

Applicant Name / Plan Holder (Legal Entity or Individual)

is responsible for compliance with the approved or conditionally approved Plan and any special conditions of the approved Plan through all phases of Plan implementation,

1 of 3

I, Dale Illig, President of Land Owner Name (Individual)	DAS UBER MONTNEY LLC Firm (applicable to Legal Entities)	
as Owner of Record or Title Holder of the property described above, I am ultimately responsible for ensuring that compliance with the approved or conditionally approved Plan and any special conditions of the approved Plan, through all phases of Plan implementation, is achieved even if the responsibility for compliance and the right to possess and control of the property referenced in the application has been contractually assumed by another legal entity.		
I, Dale Illig, President Land Owner Name (Individual)	DAS UBER MONTNEY LLC Firm (applicable to Legal Entities)	
further understand that any failure to comply with any condition of the Executive Director's approval is a violation and is subject to administrative rule or orders and penalties as provided under 30 TAC § 213.10 (relating to Enforcement). Such violation may also be subject to civil penalties and injunction.		
Land Owner Signature Land Owner Signature THE STATE OF § Texas County of § Williamson	12/6/22 Date	
BEFORE ME, the undersigned authority, on this day personally appeared known to me to be the person whose name is subscribed to the foregoing instrument and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.		
GIVEN under my hand and seal of office on	this 6th day of December	
CARL ILLIG Notary ID #129631763 My Commission Expires November 19, 2025 MY CO	NOTARY PUBLIC Typed or Printed Name of Notary MMISSION EXPIRES: Nov. 19, 2025	
Attached: (Mark all that apply)	,	

Lease Agreement Signed Contract

Deed Recorded Easement

Other legally binding document

Applicant Acknowledgement	
I, Kevin O'Neil Applicant Name (Individual)	Fidelis Realty Partners
Applicant Name (Individual)	Firm (applicable to Legal Entities)
acknowledge that Dale Illig	
Land Owner Name (Legal Entity or Individual)	
has provided Fidelis Realty Partners	
Applicant Name (Legal Enti	ty or Individual)
with the right to possess and control the proper Protection Plan (Plan).	rty referenced in the Edwards Aquifer
Lunderstand that Fidelis Realty Partners	
I understand that Fidelis Realty Partners Applicant Name (Legal Entit	ty or Individual)
is responsible, contractually or not, for complia approved Plan and any special conditions of the Plan implementation. I further understand that of the Executive Director's approval is a violatio or orders and penalties as provided under § 213 violation may also be subject to civil penalties a	e approved Plan through all phases of failure to comply with any condition on and is subject to administrative rule 3.10 (relating to Enforcement). Such
Applicant Signature Applicant Signature THE STATE OF § TEXAS	/2/6/22 Date
County of § _ DALLAS	
BEFORE ME, the undersigned authority, on this day the person whose name is subscribed to the foregoing that (s)he executed same for the purpose and considerable to the purpose and considerab	ng instrument and acknowledged to me
GIVEN under my hand and seal of office on this	day of DECEMBER
	Melanie Messici
MELANIE MESSICK	NOTARY PUBLIC
My Notary ID # 131078242 Expires December 27, 2025	MELANIE MESSICK
Typings Document at 1 and 1	True od ou Driveto d Norma of Motores

MY COMMISSION EXPIRES: DECEMBER 27, 2025

MELANIE MESSICK
Typed or Printed Name of Notary