TCEQ CZP APPLICATION

For

Palmera Bluff Sections 7 & 8 Leander, Texas

Prepared for:

Palmera Bluff Development, Inc. Blake J. Magee 1011 N. Lamar Blvd. Austin, Texas 78703 Phone: (512) 836-4793

Prepared by:

LJA Engineering, Inc. 2700 La Frontera, Suite 150 Round Rock, Texas 78681 TBPE #1386 Phone: (512) 439-4700

April 2023

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Texas Commission on Environmental Quality

Edwards Aquifer Application Cover Page

Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with 30 TAC 213.

Administrative Review

- Edwards Aquifer applications must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.
 - To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: http://www.tceq.texas.gov/field/eapp.
- 2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
- 3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
- 4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.
 - An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.
- 5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
- 6. If the geologic assessment was completed before October 1, 2004 and the site contains "possibly sensitive" features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

Technical Review

- When an application is deemed administratively complete, the technical review period begins. The regional
 office will distribute copies of the application to the identified affected city, county, and groundwater
 conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days
 to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
- 2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

- clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.
- 3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited**.
- 4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

Mid-Review Modifications

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a "Mid-Review Modification". Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ's Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ's San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: Palmera Bluff Sections 7 & 8				2. Regulated Entity No.:				
3. Customer Name: Palmera Bluff Development, Inc.			4. Customer No.:					
5. Project Type: (Please circle/check one)	New	Modification Extens			nsion	Exception		
6. Plan Type: (Please circle/check one)	WPAP CZP	SCS	UST	AST	EXP EXT		Technical Clarification	Optional Enhanced Measures
7. Land Use: (Please circle/check one)	Residential	Non-r	Non-residential		8. Site (acres): 55.72 ac		55.72 ac	
9. Application Fee:	\$8,000.00	10. Permanent B			BMP(s	MP(s): Ex. Wet Pond, Batch Detention F Jellyfish		atch Detention Pond, VFS,
11. SCS (Linear Ft.):	NA	12. AST/UST (No.			o. Tar	. Tanks): NA		
13. County:	Williamson	14. W	14. Watershed:				South San Gabriel River	

Application Distribution

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the "Texas Groundwater Conservation Districts within the EAPP Boundaries" map found at:

http://www.tceq.texas.gov/assets/public/compliance/field_ops/eapp/EAPP%20GWCD%20map.pdf

For more detailed boundaries, please contact the conservation district directly.

Austin Region				
County:	Hays	Travis	Williamson	
Original (1 req.)	_	_	_X_	
Region (1 req.)	_	_	_	
County(ies)			_	
Groundwater Conservation District(s)	Edwards Aquifer AuthorityBarton Springs/ Edwards AquiferHays TrinityPlum Creek	Barton Springs/ Edwards Aquifer	NA	
City(ies) Jurisdiction	AustinBudaDripping SpringsKyleMountain CitySan MarcosWimberleyWoodcreek	AustinBee CavePflugervilleRollingwoodRound RockSunset ValleyWest Lake Hills	AustinCedar ParkFlorenceGeorgetownJerrell _X_LeanderLiberty HillPflugervilleRound Rock	

San Antonio Region					
County:	Bexar	Comal	Kinney	Medina	Uvalde
Original (1 req.)	_		_	_	_
Region (1 req.)	_	_	_		_
County(ies)			_		
Groundwater Conservation District(s)	Edwards Aquifer Authority Trinity-Glen Rose	Edwards Aquifer Authority	Kinney	EAA Medina	EAA Uvalde
City(ies) Jurisdiction	Castle HillsFair Oaks RanchHelotesHill Country VillageHollywood ParkSan Antonio (SAWS)Shavano Park	Bulverde Fair Oaks Ranch Garden Ridge New Braunfels Schertz	NA	San Antonio ETJ (SAWS)	NA

I certify that to the best of my knowledge, that the application is hereby submitted to TCEQ for admi	application is complete and accurate. This nistrative review and technical review.
T. Walter Hoysa, P.E.	
Print Name of Customer/Authorized Agent	
thetern	4/11/23
Signature of Customer/Authorized Agent	Date

Date(s)Reviewed:	Date Ad	ministratively Complete:
Received From:	Correct Number of Copies:	
Received By:	Distribution Date:	
EAPP File Number:	Complex	с:
Admin. Review(s) (No.):	No. AR I	Rounds:
Delinquent Fees (Y/N):	Review 7	Гime Spent:
Lat./Long. Verified:	SOS Customer Verification:	
Agent Authorization Complete/Notarized (Y/N):	Fee	Payable to TCEQ (Y/N):
Core Data Form Complete (Y/N):	Check:	Signed (Y/N):
Core Data Form Incomplete Nos.:	Less than 90 days old (Y/N):	

Contributing Zone Plan Application

Texas Commission on Environmental Quality

for Regulated Activities on the Contributing Zone to the Edwards Aquifer and Relating to 30 TAC §213.24(1), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Contributing Zone Plan Application** is hereby submitted for TCEQ review and Executive Director approval. The application was prepared by:

Print Name of Customer/Agent: T. Walter Hoysa, P.E.

Date: 4/11/23

Signature of Customer/Agent:

Regulated Entity Name: Palmera Bluff Sections 7 & 8

Project Information

1. County: Williamson

2. Stream Basin: South San Gabriel River

Groundwater Conservation District (if applicable): NA

4. Customer (Applicant):

Contact Person: Blake J. Magee

Entity: <u>Palmera Bluff Development, Inc.</u> Mailing Address: <u>1011 N. Lamar Blvd.</u>

City, State: Austin, Texas Zip: 78703
Telephone: (512) 836-4793 Fax:

Email Address: Blake@blakemageeco.com

э.	Age	ent/Representative (if any):	
	Ent Ma City Tel	ntact Person: T. Walter Hoysa, P.E. Eity: LJA Engineering, Inc. Eiling Address: 2700 La Frontera Blvd Ste 150 Ey, State: Round Rock Ephone: 512-507-1732 Eail Address: whoysa@lja.com	Zip: <u>78681</u> Fax:
6.	Pro	eject Location:	
		The project site is located inside the city limits of the project site is located outside the city limits jurisdiction) of The project site is not located within any city's	s but inside the ETJ (extra-territorial
7.		The location of the project site is described beloprovided so that the TCEQ's Regional staff can boundaries for a field investigation.	
		Logan Del Way at San Gabriel Parkway, east of	Ronald Reagan Blvd.
8.		Attachment A - Road Map . A road map showing project site is attached. The map clearly shows	_
9.		Attachment B - USGS Quadrangle Map. A copy Quadrangle Map (Scale: 1" = 2000') is attached	
		☑ Project site boundaries.☑ USGS Quadrangle Name(s).	
10.		Attachment C - Project Narrative . A detailed n project is attached. The project description is contains, at a minimum, the following details:	
		 Area of the site ✓ Offsite areas ✓ Impervious cover ✓ Permanent BMP(s) ✓ Proposed site use ✓ Site history ✓ Previous development ✓ Area(s) to be demolished 	
11.	Exis	sting project site conditions are noted below:	
		Existing commercial site Existing industrial site Existing residential site	

	☐ Existing paved and/or unpaved roads☐ Undeveloped (Cleared)☐ Undeveloped (Undisturbed/Not cleared)
	Other:
12.	The type of project is:
	Residential: # of Lots: 83 Residential: # of Living Unit Equivalents: Commercial Industrial Other:
13.	Total project area (size of site): <u>55.72</u> Acres
	Total disturbed area: <u>50.00</u> Acres

15. The amount and type of impervious cover expected after construction is complete is shown below:

Table 1 - Impervious Cover

14. Estimated projected population: 249

Impervious Cover of Proposed Project	Sq. Ft.	Sq. Ft./Acre	Acres
Structures/Rooftops	283,500	÷ 43,560 =	6.51
Parking	0	÷ 43,560 =	0
Other paved surfaces	207,200	÷ 43,560 =	4.76
Total Impervious Cover	490,700	÷ 43,560 =	11.26

Total Impervious Cover $\underline{11.26}$ ÷ Total Acreage $\underline{55.72}$ X 100 = $\underline{20.22}$ % Impervious Cover

16. X	Attachment D - Factors Affecting Surface Water Quality. A detailed description of all
1	factors that could affect surface water quality is attached. If applicable, this includes the
I	location and description of any discharge associated with industrial activity other than
(construction

17. Only inert materials as defined by 30 TAC 330.2 will be used as fill material.

For Road Projects Only

Complete questions 18 - 23 if this application is exclusively for a road project.

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18.	Type of project:
	 TXDOT road project. County road or roads built to county specifications. City thoroughfare or roads to be dedicated to a municipality. Street or road providing access to private driveways.
19.	Type of pavement or road surface to be used:
	Concrete Asphaltic concrete pavement Other:
20.	Right of Way (R.O.W.):
	Length of R.O.W.: feet. Width of R.O.W.: feet. $L \times W = $ $Ft^2 \div 43,560 Ft^2/Acre = acres.$
21.	Pavement Area:
	Length of pavement area: feet. Width of pavement area: feet. L x W = Ft 2 ÷ 43,560 Ft 2 /Acre = acres. Pavement area acres ÷ R.O.W. area acres x 100 =% impervious cover.
22.	A rest stop will be included in this project.
	A rest stop will not be included in this project.
23.	Maintenance and repair of existing roadways that do not require approval from the TCEQ Executive Director. Modifications to existing roadways such as widening roads/adding shoulders totaling more than one-half (1/2) the width of one (1) existing lane require prior approval from the TCEQ.
St	formwater to be generated by the Proposed Project
24.	Attachment E - Volume and Character of Stormwater. A detailed description of the volume (quantity) and character (quality) of the stormwater runoff which is expected to occur from the proposed project is attached. The estimates of stormwater runoff quality and quantity are based on area and type of impervious cover. Include the runof coefficient of the site for both pre-construction and post-construction conditions.
W	astewater to be generated by the Proposed Project
25.	 Wastewater is to be discharged in the contributing zone. Requirements under 30 TAC §213.6(c) relating to Wastewater Treatment and Disposal Systems have been satisfied. N/A

26. Wastewater will be	disposed of by:			
On-Site Sewage Facility (OSSF/Septic Tank):				
will be used licensing authe land is sthe requirer relating to C Each lot in to size. The sy	to treat and dispose of thority's (authorized age uitable for the use of priments for on-site sewage Pacilities. his project/development stem will be designed by	m Authorized Agent. And the wastewater from this nt) written approval is at vate sewage facilities and facilities as specified under its at least one (1) acre (4) a licensed professional of the installer in compliance was the waste of the compliance was a licensed professional of the compliance wa	site. The appropriate tached. It states that will meet or exceed der 30 TAC Chapter 285	
The sewage collecti	on System (Sewer Lines) ion system will convey th ne) Treatment Plant. The	e wastewater to the <u>Bru</u>	ishy Creek Regional	
Existing. Proposed.				
☐ N/A				
Permanent Ab Gallons	oveground Stoi	rage Tanks(AST	s) ≥ 500	
Complete questions 27 greater than or equal t		des the installation of AS	T(s) with volume(s)	
⊠N/A	5			
27. Tanks and substance	ce stored:			
Table 2 - Tanks and	Substance Storage			
AST Number	Size (Gallons)	Substance to be Stored	Tank Material	
1				
2				
3				
4				
5				
	•	Tot nent structure that is size ity of the system. For fac	•	

5 of 11

·	ystem, the containm cumulative storage c		ed to capture one and	d one-half (1 1/2)
for providi		nment are propose	ent Methods. Altern d. Specifications sho	
29. Inside dimensi	ons and capacity of	containment struct	ure(s):	
Table 3 - Second	dary Containment	ŧ		
Length (L)(Ft.)	Width(W)(Ft.)	Height (H)(Ft.)	L x W x H = (Ft3)	Gallons
Some of th structure. The piping The piping The contain substance(e piping to dispense will be aboveground will be underground nment area must be s) being stored. The	ers or equipment wind d constructed of and e proposed contains	side the containment Il extend outside the I in a material imperv ment structure will be	rious to the e constructed of:
	nt H - AST Containmont nt structure is attacl		ings. A scaled drawing following:	ng of the
Interna Tanks cl	· -		wall and floor thickno collection of any spi	
storage tar		=	for collection and rec controlled drainage a	
	event of a spill, any s	. •	oved from the contain	nment structure

	In the event of a spill, any spillage will be drained from the containment structure through a drain and valve within 24 hours of the spill and disposed of properly. The drain and valve system are shown in detail on the scaled drawing.
Site	Plan Requirements
Items 3	34 - 46 must be included on the Site Plan.
34. 🖂	The Site Plan must have a minimum scale of 1" = 400'.
	Site Plan Scale: 1" = <u>20</u> '.
35. 100	O-year floodplain boundaries:
	Some part(s) of the project site is located within the 100-year floodplain. The floodplain is shown and labeled. No part of the project site is located within the 100-year floodplain. 2 100-year floodplain boundaries are based on the following specific (including date of terial) sources(s): FIRM #48491C0455F eff. 12/10/2019.
36. 🔀	The layout of the development is shown with existing and finished contours at appropriate, but not greater than ten-foot contour intervals. Lots, recreation centers, buildings, roads, etc. are shown on the site plan.
	The layout of the development is shown with existing contours at appropriate, but not greater than ten-foot contour intervals. Finished topographic contours will not differ from the existing topographic configuration and are not shown. Lots, recreation centers, buildings, roads, etc. are shown on the site plan.
37. 🔀	A drainage plan showing all paths of drainage from the site to surface streams.
38. 🖂	The drainage patterns and approximate slopes anticipated after major grading activities.
39. 🖂	Areas of soil disturbance and areas which will not be disturbed.
40. 🔀	Locations of major structural and nonstructural controls. These are the temporary and permanent best management practices.
41. 🖂	Locations where soil stabilization practices are expected to occur.
42. 🖂	Surface waters (including wetlands).
	N/A
43. 🔀	Locations where stormwater discharges to surface water.
	There will be no discharges to surface water.
44.	Temporary aboveground storage tank facilities.
\boxtimes	Temporary aboveground storage tank facilities will not be located on this site.

45. Permanent aboveground storage tank facilities.	
igwedge Permanent aboveground storage tank facilities will not be located on this site.	
46. 🔀 Legal boundaries of the site are shown.	
Permanent Best Management Practices (BMPs)	
Practices and measures that will be used during and after construction is completed.	
47. Permanent BMPs and measures must be implemented to control the discharge of pollution from regulated activities after the completion of construction.	
□ N/A	
48. These practices and measures have been designed, and will be constructed, operate and maintained to insure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity removed. These quantities have been calculated in accordance with technical guida prepared or accepted by the executive director.	ı is
 The TCEQ Technical Guidance Manual (TGM) was used to design permanent BM and measures for this site. A technical guidance other than the TCEQ TGM was used to design permanent B and measures for this site. The complete citation for the technical guidance that was used is: 	MPs
□ N/A	
49. Owners must insure that permanent BMPs and measures are constructed and funct as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the appropriate regional office within 30 days of site completions.	r
∐ N/A	
50. Where a site is used for low density single-family residential development and has 20 % less impervious cover, other permanent BMPs are not required. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.	the
 □ The site will be used for low density single-family residential development and had 20% or less impervious cover. □ The site will be used for low density single-family residential development but had more than 20% impervious cover. □ The site will not be used for low density single-family residential development. 	

fa im re in th an	mily residential developments, schools, or small business sites where 20% or less apervious cover is used at the site. This exemption from permanent BMPs must be corded in the county deed records, with a notice that if the percent impervious cover creases above 20% or land use changes, the exemption for the whole site as described in e property boundaries required by 30 TAC §213.4(g) (relating to Application Processing ad Approval), may no longer apply and the property owner must notify the appropriate gional office of these changes.
	 Attachment I - 20% or Less Impervious Cover Waiver. The site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is attached. □ The site will be used for multi-family residential developments, schools, or small business sites but has more than 20% impervious cover. □ The site will not be used for multi-family residential developments, schools, or small business sites.
52. 🔀	Attachment J - BMPs for Upgradient Stormwater.
	 A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is attached. No surface water, groundwater or stormwater originates upgradient from the site and flows across the site, and an explanation is attached. Permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site, and an explanation is attached.
53. 🗵	Attachment K - BMPs for On-site Stormwater.
	 ✓ A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is attached. ✓ Permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, and an explanation is attached.
54. 🔀	Attachment L - BMPs for Surface Streams. A description of the BMPs and measures that prevent pollutants from entering surface streams is attached.
] N/A
55. 🔀	Attachment M - Construction Plans. Construction plans and design calculations for the proposed permanent BMPs and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer, and are signed, sealed, and dated. Construction plans for the proposed permanent BMPs and measures are

	attached and include: Design calculations, TCEQ Construction Notes, all proposed structural plans and specifications, and appropriate details.
	N/A
56. 🔀	Attachment N - Inspection, Maintenance, Repair and Retrofit Plan . A site and BMP specific plan for the inspection, maintenance, repair, and, if necessary, retrofit of the permanent BMPs and measures is attached. The plan fulfills all of the following:
	Prepared and certified by the engineer designing the permanent BMPs and
	 measures Signed by the owner or responsible party Outlines specific procedures for documenting inspections, maintenance, repairs, and, if necessary, retrofit. Contains a discussion of record keeping procedures
	N/A
57.	Attachment O - Pilot-Scale Field Testing Plan . Pilot studies for BMPs that are not recognized by the Executive Director require prior approval from the TCEQ. A plan for pilot-scale field testing is attached.
	N/A
58.	Attachment P - Measures for Minimizing Surface Stream Contamination. A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity, which increase erosion that result in water quality degradation.
\boxtimes	N/A
-	oonsibility for Maintenance of Permanent BMPs and sures after Construction is Complete.
59.	The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
60.	A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development,

or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.

Administrative Information

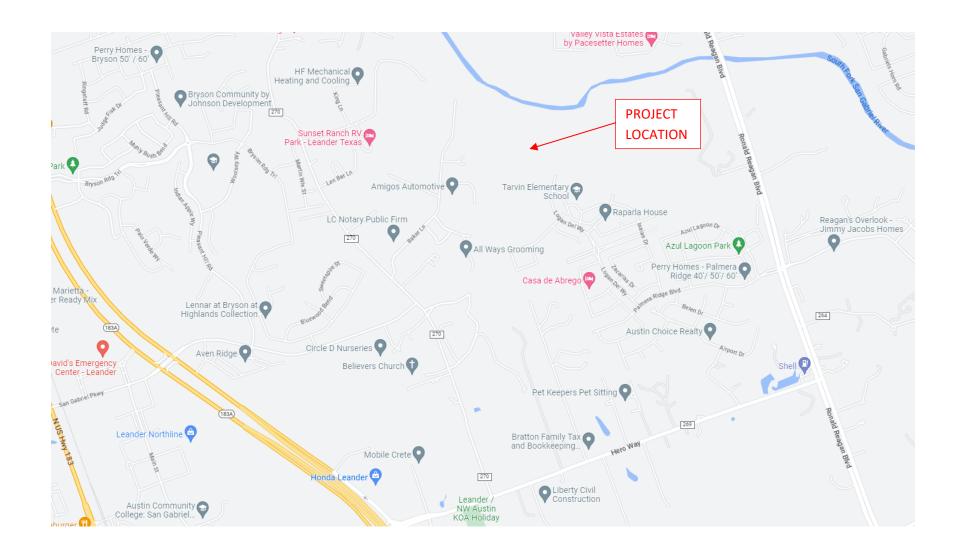
	Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions.
62. 🔀	Any modification of this Contributing Zone Plan may require TCEQ review and Executive Director approval prior to construction, and may require submission of a revised application, with appropriate fees.
63.	The site description, controls, maintenance, and inspection requirements for the storm water pollution prevention plan (SWPPP) developed under the EPA NPDES general permits for stormwater discharges have been submitted to fulfill paragraphs 30 TAC §213.24(1-5) of the technical report. All requirements of 30 TAC §213.24(1-5) have been met by the SWPPP document.
	The Temporary Stormwater Section (TCEQ-0602) is included with the application.

ATTACHMENT A

Palmera Bluff Sections 7 & 8

Leander, Texas

Road Map

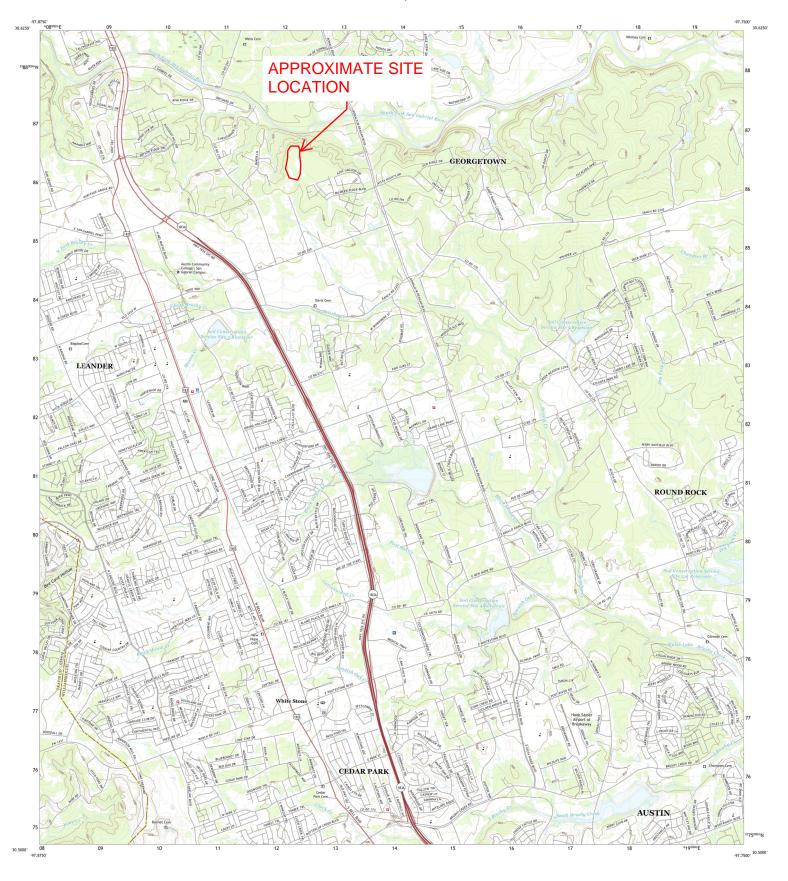


ATTACHMENT B

Palmera Bluff Sections 7 & 8

Leander, Texas

USGS Quadrangle Map









This map was produced to conform with the National Geospatial Program US Tooo Product Standard.



ATTACHMENT C

Palmera Bluff Sections 7 & 8

Leander, Texas

Project Narrative

This project is a total of 55.72 acres made up of Sections 7 and 8 of the Palmera Bluff Preliminary Plat Revision #4. The site is located off Logan Del Way at San Gabriel Parkway, west of Ronald Regan Blvd.

Existing conditions are mostly undisturbed and undeveloped, with trees and grass cover. A proposed road has been cut to subgrade, with a wastewater installed to connect existing sections to the existing lift station that is within Section 8. The proposed development of this tract will include 83 single family lots with associated street, drainage, water, and wastewater improvements. All improvements are designed to meet or exceed the standard of the City Leander, Williamson County, and the TCEQ as applicable.

This site generates a required removal of 17,834 pounds of TSS removal as shown in the TSS removal calculations attached, per the overall drainage basin. 8 BMPs are utilized to treat this site. Two existing ponds, Pond 6 and Pond 2 provide 3,072 pounds and 9,038 pounds of removal respectively and were previously approved with the TCEQ Application for Palmera Bluff Section 6. The proposed BMPs include a batch detention pond, Pond 7, a Jellyfish Filter System, and 4 separate Vegetated Filter Strips, which combined remove a total of 6,706 pounds. The total removal provided is 18,816 pounds of TSS removal. The calculations and details are in the site plan provided.

Texas Commission on Environmental Quality

TSS Removal Calculations 04-20-2009 Project Name: Palmera Bluff

Date Prepared: 9/26/2022

1. The Required Load Reduction for the total project:

Page 3-29 Equation 3.3: $L_{M} = 27.2(A_{N} \times P)$

where: $L_{\text{M TOTAL PROJECT}}$ = Required TSS removal resulting from the proposed development = 80% of increased load.

 A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

Williamson County = Total project area included in plan *= 84.45 acres Predevelopment impervious area within the limits of the plan 0.00 acres Total post-development impervious area within the limits of the plan* 20.49 acres Total post-development impervious cover fraction * = 0.24 inches 32

> 17834 lbs. L_{M TOTAL PROJECT} =

Number of drainage basins / outfalls areas leaving the plan area = 8

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. = (WQ Pond 2)

Total drainage basin/outfall area = 25.04 acres Predevelopment impervious area within drainage basin/outfall area = 0.00 acres Post-development impervious area within drainage basin/outfall area = 8.52 acres Post-development impervious fraction within drainage basin/outfall area = 0.34 7416 lbs.

L_{M THIS BASIN} =

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Wet Basin Removal efficiency = percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

A_C = 25.04 acres A_I = 8.52 acres $A_P =$ 16.52 acres 9038 L_R = lbs

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired $L_{M THIS BASIN} =$ 9038 lhs (+1.622 lbs for untreated)

1.00

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

Rainfall Depth = 4.00 inches Post Development Runoff Coefficient = 0.28 On-site Water Quality Volume = 101.146 cubic feet

Off-site area draining to BMP = 0.00 acres 0.00 Off-site Impervious cover draining to BMP = acres Impervious fraction of off-site area =

Off-site Runoff Coefficient = 0.00 Off-site Water Quality Volume = cubic feet

> Storage for Sediment = 20229

Total Capture Volume (required water quality volume(s) x 1.20) = 121376 cubic feet

The following sections are used to calculate the required water quality volume(s) for the selected BMP The values for BMP Types not selected in cell C45 will show NA.

11. Wet Basins

Required capacity of Permanent Pool = 121376 (128,833 PP Proivded) cubic feet Required capacity at WQV Elevation = (232,305 WQV Proivded) 222522 cubic feet

TSS Removal Calculations 04-20-2009

Project Name: Palmera Bluff

Date Prepared: 9/26/2022

1. The Required Load Reduction for the total project:

Page 3-29 Equation 3.3: L_M = 27.2(A_N x P)

where: L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed

 $\label{eq:continuous} \mbox{development = 80\% of increased load}.$ \mbox{A}_{N} = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson
Total project area included in plan * = 84.45 acres
Predevelopment impervious area within the limits of the plan * = 0.00 acres
Total post-development impervious cover fraction * = 0.24

Total post-development impervious cover fraction * = 0.24 inches

 $L_{M TOTAL PROJECT} = 17834$ lbs.

8

Number of drainage basins / outfalls areas leaving the plan area =

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. = 2 (WQ Pond 6)

Total drainage basin/outfall area = 9.17 acres
Predevelopment impervious area within drainage basin/outfall area = 0.00 acres
Post-development impervious area within drainage basin/outfall area = 3.00 acres
Post-development impervious fraction within drainage basin/outfall area = 0.33

Lather Rasin = 2611 lbs.

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Extended Detention (Batch Detention Pond 6)
Removal efficiency = 91 percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

 $A_{C} = 9.17$ acres $A_{I} = 3.00$ acres $A_{P} = 6.17$ acres $L_{R} = 3120$ lbs

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired L_{M THIS BASIN} = 3072 lbs. (+461 lbs for untreated)

(38,811 WQV Proivded)

F = **0.98**

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

Rainfall Depth = 3.33 inches

Post Development Runoff Coefficient = 0.27

On-site Water Quality Volume = 30.125 cubic feet

Off-site area draining to BMP = 0.00 acres
Off-site Impervious cover draining to BMP = 0.00 acres
Impervious fraction of off-site area = 0

Off-site Runoff Coefficient = 0.00
Off-site Water Quality Volume = 0 cubic feet

Storage for Sediment = 6025

Total Capture Volume (required water quality volume(s) x 1.20) = 36150 cubic feet

The following sections are used to calculate the required water quality volume(s) for the selected BMP The values for BMP Types not selected in cell C45 will show NA.

8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to

Required Water Quality Volume for extended detention basin = 36150 cubic feet

TSS Removal Calculations 04-20-2009

Project Name: Palmera Bluff

Date Prepared: 5/24/2021

Note these calculations are using the updated preliminary plan Sept 24, 2015 and calculating impervious cover by lot sizes and centerline lenghts of streets which differes from the approved method where we assumed a percentage of impervious cover based on lots sizes for overall sections.

\r-data01\eng\LAND1200-1249\1248-900-146\Docs\1248-900-146 TCEQ calculation template 04-20-09.xis

1. The Required Load Reduction for the total project:

Page 3-29 Equation 3.3: $L_{M} = 27.2(A_{N} \times P)$

where: L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed development = 80% of increased load.

A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson
Total project area included in plan * = 84.45 acres
Predevelopment impervious area within the limits of the plan * = 0.00 acres
Total post-development impervious cover fraction * = Total post-development impervious cover fraction * = 0.24

P = 32 inches

L_{M TOTAL PROJECT} = 17834 lbs.

8

Number of drainage basins / outfalls areas leaving the plan area =

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. = 3 (WQ Pond 7)

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Extended Detention (Batch Detention Pond 7)
Removal efficiency = 91 percent

4. Calculate Maximum TSS Load Removed (LR) for this Drainage Basin by the selected BMP Type.

 $A_{C} = 14.66$ acres $A_{I} = 3.65$ acres $A_{P} = 11.01$ acres $L_{R} = 3851$ lbs

$\underline{\textbf{5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area}\\$

Desired L_{M THIS BASIN} = 3407 lbs. (+230 lbs from untreated)

(22.150 WQV Projyded)

F = **0.88**

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

Rainfall Depth = 1.50 inches
Post Development Runoff Coefficient = 0.23
On-site Water Quality Volume = 18,413 cubic feet

Off-site area draining to BMP = 0.00 acres
Off-site Impervious cover draining to BMP = 0.00 acres
Impervious fraction of off-site area = 0
Off-site Runoff Coefficient = 0.00
Off-site Water Quality Volume = 0 cubic feet

Storage for Sediment = 3683

Total Capture Volume (required water quality volume(s) x 1.20) = 22095 cubic feet ing sections are used to calculate the required water quality volume(s) for the selected BMP.

The values for BMP Types not selected in cell C45 will show NA.

8. Extended Detention Basin System

Designed as Required in RG-348

Pages 3-46 to

Required Water Quality Volume for extended detention basin = 22095 cubic feet

TSS Removal Calculations 04-20-2009 Project Name: Palmera Bluff

Date Prepared: 9/26/2022

1. The Required Load Reduction for the total project:

Page 3-29 Equation 3.3: L_M = 27.2(A_N x P)

where: L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed

development = 80% of increased load. A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson
Total project area included in plan * = 84.45 acres
Predevelopment impervious area within the limits of the plan * = 0.00 acres
Total post-development impervious area within the limits of the plan * = 0.24 acres
Total post-development impervious cover fraction * = 0.24

P = 32 inches

L_{M TOTAL PROJECT} = 17834 lbs.

Number of drainage basins / outfalls areas leaving the plan area = 8

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. =	4	(VFS
Total drainage basin/outfall area =	0.77	acres
Predevelopment impervious area within drainage basin/outfall area =	0.00	acres
Post-development impervious area within drainage basin/outfall area =	0.28	acres
Post-development impervious fraction within drainage basin/outfall area =	0.36	
L _{M THIS BASIN} =	244	lbs.

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Vegetated Filter Strips
Removal efficiency = 85 percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

 $A_{C} = \begin{tabular}{ll} 0.77 & acres \\ A_{I} = \begin{tabular}{ll} 0.28 & acres \\ A_{P} = \begin{tabular}{ll} 0.49 & acres \\ L_{R} = \begin{tabular}{ll} 271 & lbs \\ \end{tabular}$

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired $L_{M THIS BASIN} =$ 271 lbs.

F = 1.00

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

Rainfall Depth = 4.00 inches

Post Development Runoff Coefficient = 0.29
On-site Water Quality Volume = 3,237 cubic feet

Off-site area draining to BMP = 0.00 acres
Off-site Impervious cover draining to BMP = 0.00 acres
Impervious fraction of off-site area = 0
Off-site Runoff Coefficient = 0.00

Off-site Water Quality Volume = 0 cubic feet

Storage for Sediment = 647

Total Capture Volume (required water quality volume(s) x 1.20) = 3884 cubic feet llowing sections are used to calculate the required water quality volume(s) for the selected BMP.

The values for BMP Types not selected in cell C45 will show NA.

16. Vegetated Filter Strips Designed as Required in RG-348 Pages 3-55 to

There are no calculations required for determining the load or size of vegetative filter strips.

The 80% removal is provided when the contributing drainage area does not exceed 72 feet (direction of flow) and the sheet flow leaving the impervious cover is directed across 15 feet of engineered filter strips with maximum slope of 20% or across 50 feet of natural vegetation with a maximum slope of 10%. There can be a break in grade as long as no slope exceeds 20%

TSS Removal Calculations 04-20-2009 Project Name: Palmera Bluff

Date Prepared: 9/26/2022

1. The Required Load Reduction for the total project:

Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$

where: L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed

development = 80% of increased load.

A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson
Total project area included in plan * = 84.45 acres
Predevelopment impervious area within the limits of the plan * = 0.00 acres
Total post-development impervious area within the limits of the plan * = 0.24 acres
Total post-development impervious cover fraction * = 0.24

P = 32 inches

L_{M TOTAL PROJECT} = 17834 lbs.

Number of drainage basins / outfalls areas leaving the plan area = 8

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. = 5 (VFS 2)

Total drainage basin/outfall area = 0.83 acres

Predevelopment impervious area within drainage basin/outfall area = 0.00 acres

Post-development impervious area within drainage basin/outfall area = 0.24 acres

Post-development impervious fraction within drainage basin/outfall area = 0.29

L_{M THIS BASIN} = 0.99 lbs.

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Vegetated Filter Strips
Removal efficiency = 85 percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

 $A_{C} = 0.83$ acres $A_{I} = 0.24$ acres $A_{P} = 0.59$ acres $L_{R} = 235$ lbs

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired $L_{M THIS BASIN} =$ 235 lbs.

F = 1.00

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

Rainfall Depth = 4.00 inches

Post Development Runoff Coefficient = 0.25
On-site Water Quality Volume = 3,043 cubic feet

Off-site area draining to BMP = 0.00 acres
Off-site Impervious cover draining to BMP = 0.00 acres
Impervious fraction of off-site area = 0
Off-site Runoff Coefficient = 0.00

Off-site Water Quality Volume = 0 cubic feet

Storage for Sediment = 609

Total Capture Volume (required water quality volume(s) x 1.20) = 3652 cubic feet lowing sections are used to calculate the required water quality volume(s) for the selected BMP.

The values for BMP Types not selected in cell C45 will show NA.

16. Vegetated Filter Strips Designed as Required in RG-348 Pages 3-55 to

There are no calculations required for determining the load or size of vegetative filter strips.

The 80% removal is provided when the contributing drainage area does not exceed 72 feet (direction of flow) and the sheet flow leaving the impervious cover is directed across 15 feet of engineered filter strips with maximum slope of 20% or across 50 feet of natural vegetation with a maximum slope of 10%. There can be a break in grade as long as no slope exceeds 20%

TSS Removal Calculations 04-20-2009 Project Name: Palmera Bluff

Date Prepared: 9/26/2022

1. The Required Load Reduction for the total project:

Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$

where $L_{\text{M TOTAL PROJECT}}$ = Required TSS removal resulting from the proposed

development = 80% of increased load.

A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

Williamson County = Total project area included in plan 84.45 acres Predevelopment impervious area within the limits of the plan acres Total post-development impervious area within the limits of the plan* = acres 0.24 Total post-development impervious cover fraction * 32 inches

> L_{M TOTAL PROJECT} = 17834 lbs.

Number of drainage basins / outfalls areas leaving the plan area =

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. =	6	(VFS 3)
Total drainage basin/outfall area =	1.07	acres
Predevelopment impervious area within drainage basin/outfall area =	0.00	acres
Post-development impervious area within drainage basin/outfall area = Post-development impervious fraction within drainage basin/outfall area =	0.47 0.44	acres
LM THIS BASIN =	409	lbs

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Vegetated Filter Strips 85 Removal efficiency = percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

A_C = $A_{l} =$ 0.47 acres A_P = 0.60 acres 451 lbs

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired L_{M THIS BASIN} = 451 lbs

1.00

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

Rainfall Depth = 4.00 inches Post Development Runoff Coefficient = 0.33 On-site Water Quality Volume = cubic feet 5,064

Off-site area draining to BMP = 0.00 acres Off-site Impervious cover draining to BMP = 0.00 acres Impervious fraction of off-site area = Off-site Runoff Coefficient = 0.00

Off-site Water Quality Volume = cubic feet

> Storage for Sediment = 1013

Total Capture Volume (required water quality volume(s) x 1.20) = 6077 quality volume(s) for the selected BMP.

The values for BMP Types not selected in cell C45 will show NA.

16. Vegetated Filter Strips Designed as Required in RG-348 Pages 3-55 to

There are no calculations required for determining the load or size of vegetative filter strips The 80% removal is provided when the contributing drainage area does not exceed 72 feet (direction of flow) and the sheet flow leaving the impervious cover is directed across 15 feet of engineered filter strips with maximum slope of 20% or across 50 feet of natural vegetation with a maximum slope of 10%. There can be a break in grade as long as no slope exceeds 20% TSS Removal Calculations 04-20-2009 Project Name: Palmera Bluff

Date Prepared: 9/26/2022

1. The Required Load Reduction for the total project:

Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$

where: L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed

development = 80% of increased load.

 A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson
Total project area included in plan * = 84.45 acres
Predevelopment impervious area within the limits of the plan * = 0.00 acres
Total post-development impervious area within the limits of the plan * = 0.24 acres
Total post-development impervious cover fraction * = 0.24

P = 32 inches

L_{M TOTAL PROJECT} = 17834 lbs.

Number of drainage basins / outfalls areas leaving the plan area = 8

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. = 7 (VFS 4)

Total drainage basin/outfall area = 2.02 acres

Predevelopment impervious area within drainage basin/outfall area = 0.00 acres

Post-development impervious area within drainage basin/outfall area = 0.57 acres

Post-development impervious fraction within drainage basin/outfall area = 0.28

L_{M THIS BASIN} = 496 lbs.

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Vegetated Filter Strips
Removal efficiency = 85 percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

 $\begin{array}{lll} A_{C} = & {\bf 2.02} & {\rm acres} \\ A_{I} = & {\bf 0.57} & {\rm acres} \\ A_{P} = & {\bf 1.45} & {\rm acres} \\ L_{R} = & {\bf 558} & {\rm lbs} \end{array}$

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired L_{M THIS BASIN} = 558 lbs.

F = 1.00

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

Rainfall Depth = 4.00 inches

Post Development Runoff Coefficient = 0.25
On-site Water Quality Volume = 7,299 cubic feet

Off-site area draining to BMP = 0.00 acres
Off-site Impervious cover draining to BMP = 0.00 acres
Impervious fraction of off-site area = 0
Off-site Runoff Coefficient = 0.00
Off-site Water Quality Volume = 0 cubic feet

Storage for Sediment = 1460

Total Capture Volume (required water quality volume(s) x 1.20) = 8759 cubic feet wing sections are used to calculate the required water quality volume(s) for the selected BMP.

The following sections are used to calculate the required water of The values for BMP Types not selected in cell C45 will show NA.

16. Vegetated Filter Strips

Designed as Required in RG-348

Pages 3-55 to

There are no calculations required for determining the load or size of vegetative filter strips.

The 80% removal is provided when the contributing drainage area does not exceed 72 feet (direction of flow) and the sheet flow leaving the impervious cover is directed across 15 feet of engineered filter strips with maximum slope of 20% or across 50 feet of natural vegetation with a maximum slope of 10%. There can be a break in grade as long as no slope exceeds 20%

Contech Engineered Solutions Calculations for Texas Commission on Environmental Quality TSS Removal Calculations

Project Name: Palmera Bluff Subdivision

Date Prepared: 1/30/2023

1. The Required Load Reduction for the total project:

Calculations from RG-348

Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$

Pages 3-27 to 3-30

 $L_{M.TOTAL\ PROJECT}$ = Required TSS removal resulting from the proposed development = 80% of increased load

 $A_{\rm N}$ = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Villiamson
Total project area included in plan * = 57.94 acres
Predevelopment impervious area within the limits of the plan * = 0.00 acres
Total post-development impervious cover fraction * = 11.53 acres
Total post-development impervious cover fraction * = 0.20 per sinches

 $L_{M \text{ TOTAL PROJECT}} = 10036$ lbs.

Number of drainage basins / outfalls areas leaving the plan area =

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. = 1

Total drainage basin/outfall area = Predevelopment impervious area within drainage basin/outfall area = Post-development impervious area within drainage basin/outfall area = Post-development impervious fraction within drainage basin/outfall area = LMTHIS BASIN = 1602 lbs.

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = JF abbreviation
Removal efficiency = 86 percent

$\underline{\textbf{4. Calculate Maximum TSS Load Removed}} \; \underline{\textbf{(L}_{R})} \; \text{for this Drainage Basin by the selected BMP Type.}$

RG-348 Page 3-33 Equation 3.7: $LR = (BMP \ efficiency) \ x \ P \ x \ (A_I \ x \ 34.6 + A_P \ x \ 0.54)$

 $A_{C} = \mbox{ Total On-Site drainage}$ area in the BMP catchment area

 $A_{I} = \mbox{Impervious area proposed in the BMP catchment area}$

 A_P = Pervious area remaining in the BMP catchment area

 L_{R} = TSS Load removed from this catchment area by the proposed BMP

 $egin{array}{lll} A_C = & {f 4.00} & {
m acres} \\ A_I = & {f 1.84} & {
m acres} \\ A_P = & {f 2.16} & {
m acres} \\ L_R = & {f 1784} & {
m lbs}. \\ \end{array}$

${\bf 5.\ Calculate\ Fraction\ of\ Annual\ Runoff\ to\ Treat\ the\ drainage\ basin\ /\ outfall\ area}$

Desired $L_{MTHIS BASIN} = 1602$ lbs. F = 0.90

$\underline{\textbf{6. Calculate Treated Flow required by the BMP Type for this drainage basin} \, / \, \textbf{outfall area.}$

Offsite area draining to BMP = 0.00 acres
Offsite impervious cover draining to BMP = 0.00 acres

Calculations from RG-348 Pages Section 3.2.22

Rainfall Intensity = 1.10 inches per hour
Effective Area = 1.72 acres
Cartridge Length = 54 inches

Peak Treatment Flow Required = 1.91 cubic feet per second

7. Jellyfish

Designed as Required in RG-348 Section 3.2.22

Flow Through Jellyfish Size

Jellyfish Size for Flow-Based Configuration = JFPD Jellyfish Treatment Flow Rate = 1.96

JFPD0806-10-2

ATTACHMENT D

Palmera Bluff Sections 7 & 8

Leander, Texas

Factors Affecting Surface Water Quality

Adjacent development including streets, parking, and other buildings are treated within the areas of existing infrastructure and will not impact this site's surface water quality.

ATTACHMENT E

Palmera Bluff Sections 7 & 8

Leander, Texas

Volume and Character of Stormwater

Stormwater runoff calculations have been completed and are provided within the site plan attached to this application. Stormwater will be typical runoff associated with a single-family development for a homes and street infrastructure.

ATTACHMENT J

Palmera Bluff Sections 7 & 8

Leander, Texas

BMPs for Upgradient Stormwater

Stormwater originating upstream from this site is mostly taken care of with development associated with the upstream areas. A small section of single-family lots from Section 6 is included with the treatment calculations for this application within the basin for Pond 7 with pretreatment from an engineering VFS.

ATTACHMENT K

Palmera Bluff Sections 7 & 8

Leander, Texas

BMPs for On-site Stormwater

On-site stormwater for this site is captured and treated on-site via a batch detention pond, engineered vegetative filter strips, a jellyfish filter system, and an existing wet pond. Details and calculations are provided within the site plan provided.

ATTACHMENT L

Palmera Bluff Sections 7 & 8

Leander, Texas

BMPs for Surface Streams

This site is surrounded by an unnamed tributary on the east which drains to the South San Gabriel River that forms the northern border of the project. Typical temporary erosion control measures, such as silt fence and rock berms, will be used during construction to prevent sediment runoff from traveling offsite. After construction, a system of swales, curb and gutter, and storm drains will convey storm runoff to the permanent controls prior to discharge into surface streams.

ATTACHMENT M

Palmera Bluff Sections 7 & 8

Leander, Texas

Construction Plans

Construction plans for this development are attached with this application.

TCEQ CZP Application

ATTACHMENT N

Palmera Bluff Sections 7 & 8

Leander, Texas

Inspection, Maintenance, Repair and Retrofit Plan

Temporary BMP's:

Best Management Practices (BMP's) installed during construction will be maintained in accordance with the requirements of the EPA's NPDES stormwater pollution prevention program. The construction superintendent will inspect temporary erosion controls on a regular basis and adjust the controls and/or remove any sediment buildup in accordance with the erosion/sedimentation control notes and as otherwise directed by the Owner or his designated representative. The following maintenance procedures shall be followed until permanent stabilization occurs.

Silt Fence

- a. Inspect weekly or after each rainfall event and repair or replacement shall be made promptly as needed.
- b. Silt fence shall be removed when the site is completely stabilized so as to not block or impede storm flow or drainage.
- c. Accumulated silt shall be removed when it reaches a depth of 6 inches. The silt shall be disposed of on an approved site and in such a manner that will not contribute to additional siltation.

Rock Berm

- a. Inspect weekly or after each rain and the stone and/or fabric core-woven sheathing shall be replaced when the structure ceases to function as intended due to silt accumulation among the rocks, washout, construction traffic damage, etc. event and repair or replacement shall be made promptly as needed.
- b. When silt reaches a depth equal to one-third the height of the berm or 6", whichever is less, the silt shall be disposed of on an approved site and in such a manner that will not contribute to additional siltation.
- c. Accumulated silt shall be removed when it reaches a depth of 6 inches. The silt shall be disposed of on an approved site and in such a manner that will not contribute to additional siltation.
- d. Severe service rock berms shall be inspected daily. Silt shall be removed when it reaches a depth of 6"
- e. Rock berms shall be removed when the site is completely stabilized so as to not block or impede storm flow or drainage.

Stabilized Construction Entrance

- a. The entrance shall be maintained in a condition that will prevent tracking or flowing of sediment onto public roadway. This may require periodic top dressing with additional stone as conditions demand, as well as repair and clean out of any devices used to trap sediment.
- b. Entrance must be properly graded to incorporate a drain swale or a similar measure to prevent runoff from leaving the construction site.

Inlet Protection

- a. Inspection shall be made weekly or after each rainfall event and replacement or repair shall be made promptly as needed.
- b. Accumulated silt shall be removed when it reaches a depth of 6 inches. The silt shall be disposed of on an approved site and in such a manner that will not contribute to additional siltation.
- c. The dyke shall be removed when the site is completely stabilized so as to not block or impede storm flow or drainage.

Following inspection of the BMP's, deficiencies shall be noted and corrected by the contractor.

Permanent BMP's:

3.5.8 Vegetative Filter Strips

Once a vegetated area is well established, little additional maintenance is generally necessary. The key to establishing a viable vegetated feature is the care and maintenance it receives in the first few months after it is plant ed. Once established, all vegetated BMPs require some basic maintenance to insure the health of the plants including:

- Pest Management. An Integrated Pest Management (IPM) Plan should be developed for vegetated areas. This plan should specify how problem insects and weeds will be controlled with minimal or no use of insecticides and herbicides.
- Seasonal Mowing and Lawn Care. If the filter strip is made up of turf grass, it should be mowed as needed to limit vegetation height to 18 inches, using a mulching mower (or removal of clippings). If native grasses are used, the filter may require less frequent mowing, but a minimum of twice annually. Grass clippings and brush debris should not be deposited on vegetated filter strip areas. Regular mowing should also include weed control practices, however herbicide use should be kept to a minimum (Urbonas et al., 1992). Healthy grass can be maintained without using fertilizers because runoff usually contains sufficient nutrients. Irrigation of the site can help assure a dense and healthy vegetative cover.
- *Inspection*. Inspect filter strips at least twice annually for erosion or damage to vegetation; however, additional inspection after periods of heavy runoff is most desirable. The strip should be checked for uniformity of grass cover, debris and litter, and areas of sediment accumulation. More frequent inspections of the grass cover during the first few years after establishment will help to determine if any

- problems are developing, and to plan for long-term restorative maintenance needs. Bare spots and areas of erosion identified during semi-annual inspections must be replanted and 3-92 restored to meet specifications. Construction of a level spreader device may be necessary to reestablish shallow overland flow.
- Debris and Litter Removal. Trash tends to accumulate in vegetated areas, particularly along highways. Any filter strip structures (i.e. level spreaders) should be kept free of obstructions to reduce floatables being flushed downstream, and for aesthetic reasons. The need for this practice is determined through periodic inspection, but should be performed no less than 4 times per year.
- Sediment Removal. Sediment removal is not normally required in filter strips, since the vegetation normally grows through it and binds it to the soil. However, sediment may accumulate along the upstream boundary of the strip preventing uniform overland flow. Excess sediment should be removed by hand or with flat-bottomed shovels.
- Grass Reseeding and Mulching. A healthy dense grass should be maintained on the filter strip. If areas are eroded, they should be filled, compacted, and reseeded so that the final grade is level. Grass damaged during the sediment removal process should be promptly replaced using the same seed mix used during filter strip establishment. If possible, flow should be diverted from the damaged areas until the grass is firmly established. Bare spots and areas of erosion identified during semi-annual inspections must be replanted and restored to meet specifications. Corrective maintenance, such as weeding or replanting should be done more frequently in the first two to three years after installation to ensure stabilization. Dense vegetation may require irrigation immediately after planting, and during particularly dry periods, particularly as the vegetation is initially established.

Section 3.5.20 Maintenance Guidelines for Batch Detention Basins

Batch detention basins may have somewhat higher maintenance requirements than an extended detention basin since they are active stormwater controls. The maintenance activities are identical to those of extended detention basins with the addition of maintenance and inspections of the automatic controller and the valve at the outlet.

• Inspections. Inspections should take place a minimum of twice a year. One inspection should take place during wet weather to determine if the basin is meeting the target detention time of 12 hours and a drawdown time of no more than 48 hours. The remaining inspections should occur between storm events so that manual operation of the valve and controller can be verified. The level sensor in the basin should be inspected and any debris or sediment in the area should be removed. The outlet structure and the trash screen should be inspected for signs of clogging. Debris and sediment should be removed from the orifice and outlet(s) as described in previous sections. Debris obstructing the valve should be removed.

- During each inspection, erosion areas inside and downstream of this BMP should be identified and repaired/revegetated immediately.
- Mowing. The basin, basin side-slopes, and embankment of the basin must be
 mowed to prevent woody growth and control weeds. A mulching mower should
 be used, or the grass clippings should be caught and removed. Mowing should
 take place at least twice a year, or more frequently if vegetation exceeds 18 inches
 in height. More frequent mowing to maintain aesthetic appeal may be necessary
 in landscaped areas.
- Litter and Debris Removal. Litter and debris removal should take place at least twice a year, as part of the periodic mowing operations and inspections. Debris and litter should be removed from the surface of the basin. Particular attention should be paid to floatable debris around the outlet structure. The outlet should be checked for possible clogging or obstructions and any debris removed.
- *Erosion control*. The basin side slopes and embankment all may periodically suffer from slumping and erosion. To correct these problems, corrective action, such as regrading and revegetation, may be necessary. Correction of erosion control should take place whenever required based on the periodic inspections.
- Nuisance Control. Standing water or soggy conditions may occur in the basin. Some standing water may occur after a storm event since the valve may close with 2 to 3 inches of water in the basin. Some flow into the basin may also occur between storms due to spring flow and residential water use that enters the storm sewer system. Twice a year, the facility should be evaluated in terms of nuisance control (insects, weeds, odors, algae, etc.).
- Structural Repairs and Replacement. With each inspection, any damage to structural elements of the basin (pipes, concrete drainage structures, retaining walls, etc.) should be identified and repaired immediately. An example of this type of repair can include patching of cracked concrete, sealing of voids, removal of vegetation from cracks and joints. The various inlet/outlet structures in a basin will eventually deteriorate and must be replaced.
- Sediment Removal. A properly designed batch detention basin will accumulate quantities of sediment over time. The accumulated sediment can detract from the appearance of the facility and reduce the pollutant removal performance of the facility. The sediment also tends to accumulate near the outlet structure and can interfere with the level sensor operation. Sediment shall be removed from the basin at least every 5 years, when sediment depth exceeds 6 inches, when the sediment interferes with the level sensor or when the basin does not drain within 48 hours. Care should be taken not to compromise the basin lining during maintenance.
- Logic Controller. The Logic Controller should be inspected as part of the twice yearly investigations. Verify that the external indicators (active, cycle in progress) are operating properly by turning the controller off and on, and by initiating a cycle by triggering the level sensor in the basin. The valve should be manually opened and closed using the open/close switch to verify valve operation and to assist in inspecting the valve for debris. The solar panel should be inspected and any dust or debris on the panel should be carefully removed. The controller and all other circuitry and wiring should be inspected for signs of corrosion, damage

from insects, water leaks, or other damage. At the end of the inspection, the controller should be reset.

3.5.25 Jellyfish® Filter Inspection and Maintenance

Jellyfish cartridges are passively backwashed automatically after each storm event, which removes accumulated sediment from the membranes and significantly extends the service life of the cartridges and the maintenance interval. If required, the cartridges can be easily manually backwashed without removing the cartridges. Additionally, the lightweight cartridges can be removed by hand and externally rinsed, and rinsed cartridges then reinstalled. These simple maintenance options allow for cartridge regeneration, thereby minimizing cartridge replacement costs and life-cycle treatment costs while ensuring long-term treatment performance.

Regular inspection and maintenance are proven, cost-effective ways to maximize water resource protection for all stormwater pollution control practices, and are required to insure proper functioning of the Jellyfish® Filter. Inspection of the Jellyfish® Filter is performed from the surface, while proper maintenance requires a combination of procedures conducted from the surface and with worker entry into the structure. Please refer to the following information and guidelines before conducting inspection and maintenance activities:

- When is inspection needed? Post-construction inspection is required prior to putting the Jellyfish Filter into service. Routine inspections are recommended quarterly during the first year of operation to accurately assess the sediment and floatable pollutant accumulation, and to ensure that the automatic backwash feature is functioning properly. Inspection frequency in subsequent years is based on the maintenance plan developed in the first year, but must occur annually at a minimum. Inspections should also be performed immediately after oil, fuel or other chemical spill.
- When is maintenance service needed? The unit must be cleaned annually. This cleaning includes removal and appropriate disposal of all water, sediment, oil and grease, and debris that has accumulated within the unit. The Jellyfish Filter is inspected and maintained by professional vacuum c l e a n i n g service providers with experience in the maintenance of underground tanks, sewers and catch basins. Since some of the maintenance procedures require manned entry into the Jellyfish structure, only professional maintenance service providers trained in confined space entry procedures should enter the vessel. Service provider companies typically have personnel who are trained and certified in confined space entry procedures according to local, state, and federal standards. Filter cartridges should be tested for adequate flow rate, every 12 months and cleaned and re-commissioned, or replaced if necessary. A manual backflush must be performed on a single draindown cartridge using a Jellyfish Cartridge Backflush Pipe (described in the Jellyfish® Filter Owner's Manual). If the time required to drain 14 gallons of backflush water from the Backflush Pipe (from top of pipe to the top of the open flapper valve) exceeds 15 seconds, it is recommended to

- perform a manual backflush on each of the cartridges. After the manual backflush, the draindown test should be repeated on a single cartridge to determine if the cartridge can drain 14 gallons of water in 15 seconds. If the cartridge still does not achieve the design flow rate, it must be replaced. The unit should be cleaned out immediately after an oil, fuel or chemical spill.
- External Rinsing This cartridge cleaning procedure is performed by removing the cartridge from the cartridge deck and externally rinsing the filtration tentacles using a low-pressure water sprayer, as described in the Jellyfish® Filter Owner's Manual. If this procedure is performed within the structure, the cartridge or individual filtration tentacles should be rinsed while safely suspended over the maintenance access wall opening in the cartridge deck, such that rinsate flows into the lower chamber of the Jellyfish® Filter. If the rinsing procedure is performed outside the structure, the cartridge or individual filtration tentacles should be rinsed in a suitable basin such as a plastic barrel or tub, and rinsate subsequently poured into the maintenance access wall opening in the cartridge deck. Sediment is subsequently removed from the lower chamber by standard vacuum service.

Acknowledged by:

Palmera Bluff Development, Inc.

TCEQ CZP Application

ATTACHMENT P

Palmera Bluff Sections 7 & 8

Leander, Texas

Measures for Minimizing Surface Stream Contamination

Surface streams that border the project boundary will be protected from surface contamination by standard erosion control measures. Silt fence will be utilized through various phases of construction along with the existing wet pond. The proposed batch detention pond will be constructed first to mitigate any direct runoff into the unnamed tributary and the South San Gabriel River.

Temporary Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: <u>T. Walter Hoysa, P.E.</u>
Date: 4[11]23
Signature of Customer/Agent:
thetelyon

Regulated Entity Name: Palmera Bluff Sections 7 & 8

Project Information

Potential Sources of Contamination

Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.

1.	Fuels for construction equipment and hazardous substances which will be used during construction:
	The following fuels and/or hazardous substances will be stored on the site:
	These fuels and/or hazardous substances will be stored in:
	Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

	 Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year. Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
	igtimes Fuels and hazardous substances will not be stored on the site.
2.	Attachment A - Spill Response Actions. A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
3.	Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
4.	Attachment B - Potential Sources of Contamination. A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.
S	equence of Construction
5.	Attachment C - Sequence of Major Activities. A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
	 For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given. For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
6.	Name the receiving water(s) at or near the site which will be disturbed or which will

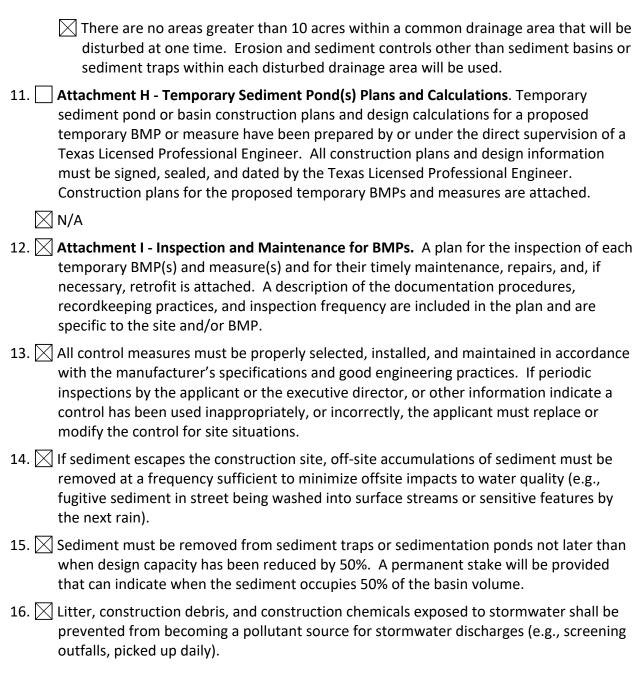
Temporary Best Management Practices (TBMPs)

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

receive discharges from disturbed areas of the project: South San Gabriel River

7. Attachment D – Temporary Best Management Practices and Measures. TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

	 ✓ A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site. ✓ A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by
	 contaminated stormwater runoff from the site. A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer. A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.
8.	The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
	Attachment E - Request to Temporarily Seal a Feature. A request to temporarily seal a feature is attached. The request includes justification as to why no reasonable and practicable alternative exists for each feature. There will be no temporary sealing of naturally-occurring sensitive features on the site.
9.	Attachment F - Structural Practices. A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.
10.	Attachment G - Drainage Area Map. A drainage area map supporting the following requirements is attached:
	 For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided. For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used. For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect
	down slope and side slope boundaries of the construction area. There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.



Soil Stabilization Practices

Examples: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

17. Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices. A schedule of the interim and permanent soil stabilization practices for the site is attached.

- 18. Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 19. Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

Administrative Information

- 20. All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
- 21. If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
- 22. Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

ATTACHMENT A

Palmera Bluff Sections 7 & 8

Leander, Texas

Spill Response Actions

No spills of hydrocarbons or hazardous substances are expected. However, in the event such an incidence does occur, the contractor should carefully follow the following TCEQ guidelines at https://www.tceq.texas.gov/response/spills/spill_rq.html.

Cleanup:

- 1) Clean up leaks and spills immediately.
- 2) Use a rag for small spills on paved surfaces, a damp mop for general cleanup, and absorbent material for larger spills. If the spilled material is hazardous, then the used cleanup materials are also hazardous and must be disposed of as hazardous waste.
- 3) Never hose down or bury dry material spills. Clean up as much of the material as possible and dispose of properly.

Minor Spills:

- 1) Minor spills typically involve small quantities of oil, gasoline, paint, etc. which can be controlled by the first responder at the discovery of the spill.
- 2) Use absorbent materials on small spills rather than hosing down or burying the spill.
- 3) Absorbent materials should be promptly removed and disposed of properly.
- 4) Follow the practice below for a minor spill:
 - a. Contain the spread of the spill.
 - b. Recover spilled materials.
 - c. Clean the contaminated area and properly dispose of contaminated materials.

Semi-Significant Spills:

Semi-significant spills still can be controlled by the first responder along with the aid of other personnel such as laborers and the foreman, etc. This response may require the cessation of all other activities. Spills should be cleaned up immediately:

- 1) Contain spread of the spill.
- 2) Notify the project foreman immediately.
- 3) If the spill occurs on paved or impermeable surfaces, clean up using "dry" methods (absorbent materials, cat litter and/or rags). Contain the spill by encircling with absorbent materials and do not let the spill spread widely.
- 4) If the spill occurs in dirt areas, immediately contain the spill by constructing an earthen dike. Dig up and properly dispose of contaminated soil.

If the spill occurs during rain, cover spill with tarps or other material to prevent contaminating runoff.

ATTACHMENT B

Palmera Bluff Sections 7 & 8

Leander, Texas

Potential Sources of Contamination

No particular activity or process during construction is anticipated to present a significant risk of being a potential source of contamination. However, during regular construction operations, several common and minor risks of contamination are anticipated. Should the unforeseeable mishap occur during construction or regular operation of the facility, the contractor shall follow the guidelines set forth in "Attachment 2A – Spill Response Plan."

Potential sources of sediment to stormwater runoff:

- Clearing and grubbing
- Grading and excavation
- Vehicle tracking
- Topsoil stripping and stockpiling
- Landscaping

Potential pollutants and sources, other than sediment, to stormwater runoff:

- Combined Staging Area small fueling, minor equipment maintenance, sanitary facilities.
- Materials Storage Area solvents, adhesives, paving materials, aggregates, trash, etc.
- Construction Activities paving, concrete pouring
- Concrete Washout Area

Potential Onsite Pollutants:

- Fertilizer
- Concrete
- Glue, adhesives
- Gasoline, diesel fuel, hydraulic fluids, antifreeze
- Sanitary toilets

ATTACHMENT C

Palmera Bluff Sections 7 & 8

Leander, Texas

SEQUENCE of MAJOR ACTIVITIES:

- 1) Written construction notification should be provided to the appropriate TCEQ regional office no later than 48 hours prior to commencement of the regulated activity. Call the One Call Center at 512-472-2822 and the Texas Underground Facility Notification Corporation for utility locations and obtain permit for any work within the right of way. Prior to beginning construction, the owner or his authorized representative shall convene a Pre-Construction Conference between the TCEQ, Travis County, consulting engineer, contractor, and any other affected parties.
- 2) Install temporary erosion control measures, stabilized construction entrance, and tree protection according to the plans and specifications prior to any clearing and grubbing, grading, excavating, etc. Notify Construction Inspection Division when installed.
- 3) Establish spoils area.
- 4) Rough grade streets. Install all utilities to be located under proposed pavement.
- 5) Regrade streets to subgrade.
- 6) Ensure that all underground utility crossings are completed. Lay first course base material on all streets.
- 7) Lay first course base material.
- 8) Install curb and gutter.
- 9) Lay final base course on all streets.
- 10) Lay asphalt.
- 11) Install all traffic control signing, striping, and pavement markers.
- 12) Complete all underground installations within the R.O.W.
- 13) Complete final grading of single family lots.

- 14) Complete permanent erosion control and restoration of site vegetation.
- 15) Remove and dispose of temporary erosion controls and accumulated sediment after approval of Construction Inspection Division.

Clearing and grubbing under a development permit, solely for the purpose of surveying and soil exploration, shall be a hand cutting or blade-up operation.

ATTACHMENT D

Palmera Bluff Sections 7 & 8

Leander, Texas

Temporary Best Management Practices and Measures

Install temporary erosion control measures, stabilized construction entrance, concrete washout area, inlet protection, and tree protection according to the plans and specifications prior to any clearing and grubbing, grading, excavating, etc.

Stormwater flows crossing disturbed areas within the site will be filtered utilizing standard Best Management Practices such as rock berms and silt fences prior to leaving the site. The silt fences will be placed along down gradient areas of the site to prevent any sediment from entering storm sewers or surface streams. There are no upgradient flows crossing this site. No geologic features are located on this site.

ATTACHMENT F

Palmera Bluff Sections 7 & 8

Leander, Texas

Structural Practices

The following temporary BMP structural practices will be employed on the site:

- 1) Silt Fence used as barrier protection around the downslope perimeter of the project. The fence retains sediment primarily by retarding flow and promoting deposition on the uphill side of the slope. Runoff is filtered as it passes through the geotextile fabric.
- 2) Inlet Protection used to prevent sediment from entering the storm drain system.
- 3) Concrete Washout Area used to prevent or reduce the discharge of pollutants to stormwater from concrete waste. The concrete washout area is a designated area to wash out wastes into the temporary pit where the concrete can set, be broken up, and the disposed of properly.
- 4) Stabilized Construction Entrance used to provide a stable entrance/exit condition from the construction site and keep mud and sediment off public roads. The stabilized construction entrance is a stabilized pad of crushed stone and should be located at any point traffic will be entering or leaving the construction site from a public right-of-way.
- 5) Contractor Staging Area used as an area for the contractor to store and prepare equipment and materials before using them during the construction phase.

ATTACHMENT G

Palmera Bluff Sections 7 & 8

Leander, Texas

Drainage Area Map

Drainage area maps are provided within the site plan set provided.

ATTACHMENT H

Palmera Bluff Sections 7 & 8

Leander, Texas

Temporary Sediment Pond Plans and Calculations

No temporary sediment pond is proposed.

ATTACHMENT I

Palmera Bluff Sections 7 & 8

Leander, Texas

Inspection and Maintenance for Best Management Practices

The inspection and maintenance of temporary BMP's will be made according to TCEQ RG-348, Complying with the Edwards Aquifer Rules Technical Guidance on Best Management Practices.

Inspection Personnel:

Inspections shall be conducted by qualified representatives of the contractor acting on behalf of the owner or a designated party if hired separately by the owner. Each operator must delegate authority to the specifically described position or person performing inspections, as provided by 30 TAC 305.128, as an authorized person for signing reports and performing certain activities requested by the director or required by the TPDES general permit. This delegation of authority must be provided to the director of TCEQ in writing and a copy shall be kept along with the signed effective copy of the SWP3.

Inspection Schedule and Procedures - Inspections must comply with the following:

- A) An inspection shall occur weekly and after any rain event. This inspection should include an inspection of the temporary concrete washout area.
- B) The authorized party shall inspect all disturbed areas of the site, areas used for storage of materials that are exposed to precipitation, structural control measures, and locations where vehicles enter or exit the site.
- C) Disturbed areas and areas used for storage of materials that are exposed to precipitation or within limits of the 1% annual chance (100 year) floodplain must be inspected for evidence of, or the potential for, pollutants entering the runoff from the site. Erosion and sediment control measures identified in the plan must be observed to ensure that they are operating correctly. Observations can be made during wet or dry weather conditions. Where discharge locations or points are accessible, they must be inspected to ascertain whether erosion control measures are effective in preventing significant impacts to receiving waters. This can be done by inspecting receiving waters to see whether any signs or erosion or sediment are associated with the discharge location. Locations where vehicles enter or exit the site must be inspected for evidence of off-site sediment tracking.
- D) Based on the results of the inspection, the site description and the pollution prevention measures identified in the plan must be revised as soon as possible after an inspection that reveals inadequacies. The inspection and plan review process must provide for timely implementation of any changes to the plan with 7 calendar days following the inspection.

E) An inspection report that summarizes the scope of the inspection, name(s) and qualifications of personnel conducting the inspection, the dates of the inspection, major observations relating to the implementation of the SWP3. Major observations shall include as a minimum location of discharges of sediment or other pollutants from the site, location of BMPs that need to be maintained, location of BMPs that failed to operate as designed or proved inadequate for a particular location, and locations where BMPs are needed. Actions taken as a result of the inspections must be described within, and retained as a part of, the SWP3. Reports must identify any incidents of non-compliance. Where a report does not identify any incidents of non-compliance, the report must contain a certification that the facility or site is in compliance with the SWP3 and the TPDES general permit. The report must be signed by the authorized representative delegated by the operators in accordance with TAC 305.128.

Maintenance and Corrective Actions - Maintenance of erosion control facilities shall consist of the minimum requirements as follows:

- A) In ongoing construction areas inspect erosion control improvements to confirm facilities are in place and operable. Where facilities have been temporarily set aside or damaged due to construction activity, place facilities in service before leaving job site.
- B) If weather forecast predicts possibility of rain, check entire facilities throughout site to assure facilities are in place and operable. If job site weather conditions indicate high probability of rain, make special inspection of erosion control facilities.
- C) After rainfall events review erosion control facilities as soon as site is accessible. Clean rock berms, berm/swales and other structural facilities. Determine where additional facilities or alternative techniques are needed to control sediment leaving site.
- D) After portions of site have been seeded, review these areas on regular basis in accordance with project specifications to assure proper watering until grass is established. Reseed areas where grass is not well established.
- E) Spills are to be handled as specified by the manufacturer of the product in a timely safe manner by personnel. The site superintendent will be responsible for coordinating spill prevention and cleanup operations.
- F) Concrete trucks will discharge extra concrete or wash out drum only at an approved location on site. Residual product shall be properly disposed of.
- G) Inspect vehicle entrance and exits for evidence of off-site tracking and correct as needed.
- H) If sediment escapes the site, the contractor where feasible and where access is available shall collect and remove sedimentation material by appropriate non-damaging methods. Additionally, the contractor shall correct the condition causing discharges.
- I) If inspections or other information sources reveal a control has been used incorrectly, or that a control is performing inadequately, the contractor must replace, correct or modify the control as soon as practical after discovery of the deficiency.

Silt Fence – Inspection and maintenance guidelines for silt fences are as follows:

- A) Inspect all fencing weekly, and after any rainfall.
- B) Remove sediment when buildup reaches 6 inches.
- C) Replace any torn fabric or install a second line of fencing parallel to the torn section.

- D) Replace or repair any sections crushed or collapsed in the course of construction activity. If a section of fence is obstructing vehicular access, consider relocating it to a spot where it will provide equal protection, but will not obstruct vehicles. A triangular filter dike may be preferable to a silt fence at common vehicle access points.
- E) When construction is complete, the sediment should be disposed of in a manner that will not cause additional siltation and the prior location of the silt fence should be revegetated. The fence itself should be disposed of in an approved landfill.

Inlet Protection – Inspection and maintenance guidelines for inlet protection is as follows:

- A) Inspection should be made weekly and after each rainfall. Repair or replacement should be made promptly as needed by the contractor.
- B) Remove sediment when buildup reaches a depth of 3 inches. Removed sediment should be deposited in a suitable area and in such a manner that it will not erode.
- C) Check placement of device to prevent gaps between device and curb.
- D) Inspect filter fabric and patch or replace if torn or missing.
- E) Structures should be removed, and the area stabilized only after the remaining drainage area has been properly stabilized.

Stabilized Construction Entrance

- A) The entrance should be maintained in a condition, which will prevent tracking or flowing of sediment onto public rights-of-way. This may require periodic top dressing with additional stone as conditions demand and repair and/or cleanout of any measures used to trap sediment.
- B) All sediments spilled, dropped, washed or tracked onto public rights-of-way should be removed immediately by contractor.
- C) When necessary, wheels should be cleaned to remove sediment prior to entrance onto public rights-of-way.
- D) When washing is required, it should be done on an area stabilized with crushed stone that drains into an approved sediment trap or sediment basin.
- E) All sediment should be prevented from entering any storm drain, ditch, or water course by using approved methods.

Concrete Washout Area

- A) Concrete washout areas should be located at least 50 feet from sensitive features, storm drains, open ditches, or water bodies.
- B) Do not allow runoff from this area by constructing a temporary pit or bermed area large enough for liquid and solid waste.
- C) Plastic lining material should be a minimum of 10 mil in polyethylene sheeting and should be free of holes, tears, or other defects that compromise the impermeability of the material.
- D) When temporary concrete washout facilities are no longer required for the work, the hardened concrete should be removed and disposed of. Materials sued to construct

temporary concrete washout facilities should be removed from the site of the work and disposed of. Holes, depressions, or other ground disturbance caused by the removal of the temporary concrete washout facilities should be backfilled and repaired.

ATTACHMENT J

Palmera Bluff Sections 7 & 8

Leander, Texas

Schedule of Interim and Permanent Soil Stabilization Practices

Soil Stabilization for all disturbed areas shall be accomplished by hydraulic planting. Following is an outline to accomplish the required stabilization.

- 1. Preparing Seed Bed. After the designated areas have been rough graded to the lines, grades and typical sections indicated in the Drawings or as provided for in other items of this contract and for any other soil area disturbed by the construction, a suitable seedbed shall be prepared. The seedbed shall consist of a minimum of either 4 inches (100 millimeters) of approved topsoil or 4 inches (100 millimeters) of approved salvaged topsoil, cultivated and rolled sufficiently to enhance the soil to a state of good health, when the soil particles on the surface are small enough and lie closely enough together to prevent the seed from being covered too deeply for optimum germination. The optimum depth for seeding shall be 1/4 inch (6 millimeters). Water shall be gently applied as required to prepare the seedbed prior to the planting operation either by broadcast seeding or hydraulic planting. Bare soils should be seeded or otherwise stabilized within 14 calendar days after final grading or where construction activity has temporarily ceased for more than 21 days. Seeding shall be performed in accordance with the requirements hereinafter described.
- 2. Watering. All watering shall comply with The Subdivision Rules and Regulations. Broadcast seeded areas shall immediately be watered with a minimum of 5 gallons of water per square yard (22.5 liters of water per square meter) or as needed and in the manner and quantity as directed by the Engineer or designated representative. Hydraulic seeded areas and native grass seeded areas shall be watered commencing after the tackifier has dried with a minimum of 5 gallons of water per square yard (22.5 liters of water per square meter) or as needed to keep the seedbed in a wet condition favorable for the growth of grass.

Watering applications shall constantly maintain the seedbed in a wet condition favorable for the growth of grass. Watering shall continue until the grass is uniformly 1 1/2 inches (40 mm) in height and accepted by the Engineer or designated representative. Watering can be postponed immediately after a 1/2 inch (12.5 mm) or greater rainfall on the site but shall be resumed before the soil dries out.

3. Hydraulic Planting. The seedbed shall be prepared as specified above and hydraulic planting equipment, which is capable of placing all materials in a single operation, shall be used.

Hydraulic planting mixture and minimum rate of application pounds per 1000 square feet (kilograms per 100 square meters):

Planting Mixture			
Hulled Bermuda Seed	Fiber Mulch		Soil
(PLS=0.83)	Cellulose	Wood	Tackifier
	45.9 Lbs/1000 ft2		1.4 Lbs/1000 ft2
1 Lbs/1000 ft2	(22.5 kgs/100 m2))		(0.7 kgs/100 m2))
(0.5 kgs/100 m2))			
		57.4 Lbs/1000 ft2	1.5 Lbs/1000 ft2
		(28.01 kgs/100 m2))	(0.75 kgs/100 m2))

September 15 to March 1

Add 1.5 pounds per 1000 square feet (0.75 kilograms per 100 square meters) of cool season cover crop (see Table 1) to above mixture. The fertilizer shall conform to City of Austin Standard Specification Item No. 606S, "Fertilizer".

Table 1: Cool Season Cover Crop			
Common Name	Botanical Name	Applica	ation rates
Common Name	Botanicai Name	Lbs/1000 feet ²	kg/ 100 meter ²
Wheat	Triticum aestivum	0.5	0.25
Oats	Avena sativa	0.5	0.25
Cereal Rye Grain	Secale cereale	0.5	0.25
Total Cool Season Cover Crop Seeding Rate		1.5	0.75
Total Cool Season Seeding Rate (Grass, Wildflowers, & Cover Crop)		4.5	2.25

Agent Authorization Form

For Required Signature
Edwards Aquifer Protection Program
Relating to 30 TAC Chapter 213
Effective June 1, 1999

l	Blake J. Magee	
	Print Name	
	Owner	
	Title - Owner/President/Other	
of	Palmera Bluff Development, Inc. Corporation/Partnership/Entity Name	
have authorized	T. Walter Hoysa, P.E. Print Name of Agent/Engineer	
of	LJA Engineering, Inc.	
	Print Name of Firm	

to represent and act on the behalf of the above-named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

- 1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
- 2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
- 3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
- 4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
- 5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:

Applicant's Signature Date

THE STATE OF Texas §

County of Trans §

BEFORE ME, the undersigned authority, on this day personally appeared <u>Plum J. Mager</u>known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this 114 day of April , 2023.

NOTARY PUBLIC

Holly H. Fullerton
Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 5 - 29 - 2024

HOLLY H. FULLERTON
Notary Public, State of Texas
Comm. Expires 05-29-2024
Notary ID 132499027

Application Fee Form

Texas Commission on Environmental Quality				
Name of Proposed Regulated Entity: Palmera Bluff Sections 7 & 8				
Regulated Entity Location: <u>Leander, TX</u>				
Name of Customer: Palmera Bluff Development, Inc.				
Contact Person: Blake J. Magee	Ph	one: <u>512-836</u>	<u>-4793</u>	
Customer Reference Number (if issue	d):CN <u>605228949</u>	9		
Regulated Entity Reference Number (if issued):RN			
Austin Regional Office (3373)				
☐ Hays	Travis		$\boxtimes w$	illiamson
San Antonio Regional Office (3362)			_	
Bexar	Medina		Ши,	valde
				raiue
Comal	Kinney			
Application fees must be paid by chec		-		
Commission on Environmental Quali				·
form must be submitted with your fe	ee payment. This	s payment is t	eing subm	rttea to:
X Austin Regional Office] San Antonio	Regional C	ffice
Mailed to: TCEQ - Cashier		Overnight D	elivery to: 1	CEQ - Cashier
Revenues Section		12100 Park	35 Circle	
Mail Code 214	Mail Code 214 Building A, 3rd Floor			
P.O. Box 13088 Austin, TX 78753				
Austin, TX 78711-3088 (512)239-0357				
Site Location (Check All That Apply):				
Recharge Zone	Contributing Zor	ne	Transi	tion Zone
Type of Plan		Siz	ze	Fee Due
Water Pollution Abatement Plan, Con	tributing Zone		Culti II	
Plan: One Single Family Residential Dv			Acres	\$
Water Pollution Abatement Plan, Con	•			
Plan: Multiple Single Family Residenti			Acres	\$
Water Pollution Abatement Plan, Con	tributing Zone			
Plan: Non-residential		55	.72 Acres	\$ 8,000.00
Sewage Collection System			L.F.	\$
Lift Stations without sewer lines			Acres	\$
Underground or Aboveground Storage	e Tank Facility		Tanks	\$
Piping System(s)(only)			Each	\$
Exception			Each	\$
Extension of Time			Each	\$
Signature:	Da	te:		

Application Fee Schedule

Texas Commission on Environmental Quality

Edwards Aquifer Protection Program 30 TAC Chapter 213 (effective 05/01/2008)

Water Pollution Abatement Plans and Modifications

Contributing Zone Plans and Modifications

Project	Project Area in Acres	Fee
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5	\$1,500
	5 < 10	\$3,000
	10 < 40	\$4,000
	40 < 100	\$6,500
	100 < 500	\$8,000
	≥ 500	\$10,000
Non-residential (Commercial, industrial, institutional,	< 1	\$3,000
multi-family residential, schools, and other sites	1 < 5	\$4,000
where regulated activities will occur)	5 < 10	\$5,000
	10 < 40	\$6,500
	40 < 100	\$8,000
	≥ 100	\$10,000

Organized Sewage Collection Systems and Modifications

Project	Cost per Linear Foot	Minimum Fee- Maximum Fee
Sewage Collection Systems	\$0.50	\$650 - \$6,500

Underground and Aboveground Storage Tank System Facility Plans and Modifications

Project	Cost per Tank or Piping System	Minimum Fee- Maximum Fee
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500

Exception Requests

Project	Fee
Exception Request	\$500

Extension of Time Requests

Project	Fee
Extension of Time Request	\$150



TCEQ Core Data Form

TCEQ Use Only

For detailed instructions regarding completion of this form, please read the Core Data Form Instructions or call 512-239-5175.

SECTION I: General Informati	on
------------------------------	----

1. Reason fo	r Submis	sion (If other is c	hecked please d	describe in	space j	provide	ed.)				
New Permit, Registration or Authorization (Core Data Form should be submitted with the program application.)											
Renewal (Core Data Form should be submitted with the renewal form)											
2. Customer Reference Number (if issued) Follow this link to search 3. Regulated Entity Reference Number (if issued)											
CN 605228949											
SECTION II: Customer Information											
4. General Customer Information 5. Effective Date for Customer Information Updates (mm/dd/yyyy)											
	□ New Customer □ Update to Customer Information □ Change in Regulated Entity Ownership									Entity Ownership	
									f Public Accounts)		<i>c</i> 'a a
			_	•			•			rrent and	active with the
		State (SOS)				JUIIC .			· •	0	- u b - l
6. Customer	Legai Nai	ne (If an individual	, print iast name ii	irst: eg: Doe,	JONN)		<u> </u>	ii riew Ct	<u>ustomer, enter previ</u>	ous Custome	er below:
		velopment									
7. TX SOS/CI	PA Filing	Number	8. TX State Ta	ax ID (11 digi	ts)		,	9. Feder	ral Tax ID (9 digits)	10. DUN	S Number (if applicable)
11. Type of C				$ \vdash$ \vdash	Individ				artnership: 🔲 Gener	al Limited	
		County Federal	State Other		Sole P	ropriet			Other:		0
12. Number ○ 0-20	of Employ] 21-100	ees 101-250	251-500	☐ 501 aı	nd high	er		13. Inde ⊠ Yes	pendently Owned	and Opera	ted?
14. Custome	r Role (Pro	oposed or Actual) –	as it relates to the	e Regulated	Entity li	isted on	this f	form. Plea	ase check one of the	following	
⊠Owner		Operat	or		wner 8	Opera	ator				
Occupatio	nal Licens	ee 🗌 Respo	nsible Party	□ V	oluntar	y Clea	nup A	Applicant	Other:		
	1011 N	N. Lamar Blv	d								
15. Mailing Address:											
Addicoo.	City	Austin		State	TX		ZIP	787	03	ZIP + 4	
16. Country	Mailing In	formation (if outsi	de USA)	1		17. E	-Mail	Addres	SS (if applicable)		
18. Telephon	e Numbe	•	1	9. Extensi	on or (Code			20. Fax Numbe	r (if applical	ole)
(512)83	6-4793								()	-	
SECTION	III. D	egulated En	tity Inform	nation					I		
		-	-		tv" is sa	alected	helo	w this fo	rm should he acco	mnanied hy	a permit application)
New Regu	_	-	to Regulated Er		-				Entity Information		а ретти аррисацопу
		 -									lards (removal
		ndings such	•	•							
22. Regulate	d Entity N	ame (Enter name	of the site where t	the regulated	action	is takin	g plac	e.)			
Palmera Bluff Sections 7 & 8											

TCEQ-10400 (02/21) Page 1 of 2

23. Street Address of the Regulated Entity:								
(No PO Boxes)	City		State		ZIP		ZIP + 4	
24. County								
		Enter Physical L	ocation Descript	ion if no str	eet addres	s is provided.		
25. Description to Physical Location:	Logan	Del Way at S	an Gabrial B	lvd, East	of Ronal	d Regan Blvo	i	
26. Nearest City					e Jak	State	Nea	rest ZIP Code
Leander						TX	786	541
27. Latitude (N) In Dec	imal:	30.606570		28. L	ongitude (W) In Decimal:	-97.83190)2
Degrees	Minutes		Seconds	Degree	es	Minutes		Seconds
30		36	23.7		97		49	54.9
29. Primary SIC Code	4 digits) 30). Secondary SIC	Code (4 digits)	31. Primar (5 or 6 digits	•		Secondary NAI 6 digits)	CS Code
33. What is the Primar	/ Business	of this entity?	(Do not repeat the SIC	or NAICS desc	ription.)			
34. Mailing Address:								
Addiess.	City		State		ZIP		ZIP+4	
35. E-Mail Addres	s:							
36. Telepl	one Numbe	er	37. Extension	on or Code		38. Fax Nu	ımber (if appli	cable)
()						() •	
9. TCEQ Programs and I	D Numbers instructions f	Check all Programs for additional guidan	s and write in the pe	rmits/registrat	on numbers	that will be affected	d by the updates	submitted on this
☐ Dam Safety	☐ Distric		Edwards Aqu	ifer	☐ Emission	ons Inventory Air	☐ Industrial	Hazardous Waste
Municipal Solid Waste	New S	Source Review Air	OSSF		☐ Petrole	um Storage Tank	PWS	
Sludge	☐ Storm	Water	☐ Title V Air					
oldage	30011	I vvalei	LI Title V Air		Tires		Used Oil	
☐ Voluntary Cleanup	☐ Waste	e Water	☐ Wastewater A	griculture	☐ Water F	Rights	nts Other:	
ECTION IV: Pr	eparer I	nformation						
40. Name: T. Walter H	oysa, P.E			41. Title:	Senio	or Project Ma	nager	
12. Telephone Number	43. Ext./Co	de 44. Fax	Number	45. E-Ma	il Address			
512)767-7351		(whoysa	a@lja.co	m		
ECTION V: Au	thorized	Signature						
6. By my signature below gnature authority to submentified in field 39.	, I certify, to	the best of my kn	nowledge, that the tity specified in So	information ection II, Fie	provided in ld 6 and/or	this form is true as required for th	and complete, a e updates to the	and that I have ID numbers
Company: LJA E	ngineering,	inc.		Job Title:	Senio	r Project Manage	r	
Name (In Print): T. Wa	lter Hoysa, I	P.E.				Phone:	(512)767-7	351
Signature:	W	Uffer				Date:	4/11/2	3

TCEQ-10400 (02/21)

SUBMITTED FOR APPROVAL BY: LJA ENGINEERING, INC.



PALMERA BLUFF SECTIONS 7 & 8

Public Improvement Construction Plan

REVIEWED BY: DEVELOPMENT SERVICES DEPARTMENT SITE PLAN/DEVELOPMENT PERMIT NUMBER SUBDIVISION FILE NUMBER INDUSTRIAL WASTE DATE **AUSTIN WATER UTILITY** CITY OF AUSTIN FIRE DEPARTMENT DATE

WATERSHED STATUS

THIS SITE IS LOCATED WITHIN THE SOUTH SAN GABRIEL WATERSHED

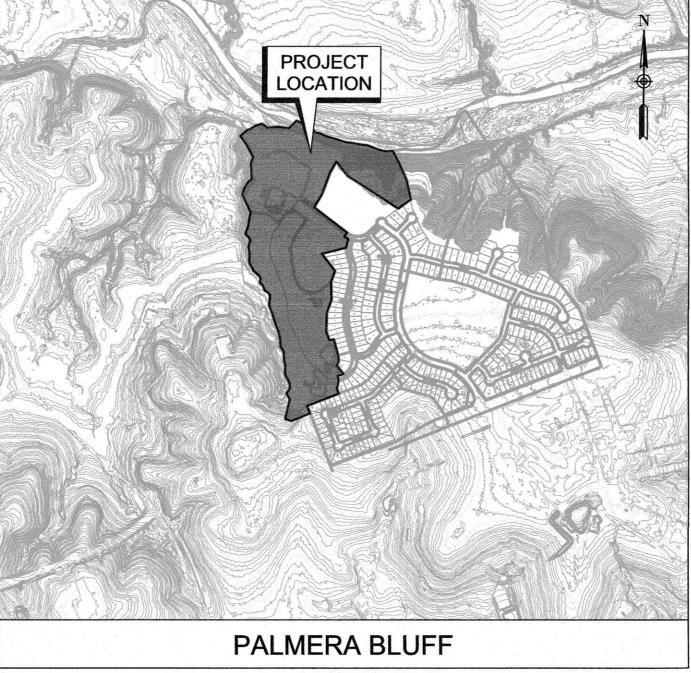
FLOODPLAIN INFORMATION

THE TRACT SHOWN IS ENCUMBERED BY ZONE X ACCORDING TO FIRM PANEL

THIS FLOOD STATEMENT DOES NOT IMPLY THAT THE PROPERTY AND/OR THE STRUCTURES THEREON WILL BE FREE FROM FLOODING OR FLOOD DAMAGE. THIS FLOOD STATEMENT SHALL NOT CREATE LIABILITY ON THE PART OF THE SURVEYOR, OR ENGINEER.

LEGAL DESCRIPTION

XXXXXX FINAL PLAT DOC. # ____



LOCATION MAP

SCALE: N.T.S. SUBMITTAL DATE:

OWNER:

PALMERA BLUFF DEVELOPMENT, INC. 1011 N. LAMAR BLVD

AUSTIN TX, 78703 PHONE: (512) 481-0303

ENGINEER

LJA ENGINEERING INC. FRN # F-1386 2700 LA FRONTERA BLVD, SUITE 150 **ROUND ROCK, TX 78681** CONTACT PERSON: WALTER HOYSA, P.E.

SURVEYOR:

LANDPOINT, LLC 2900 JAZZ STREET **ROUND ROCK, TX 78664** PHONE: (800)348-5254

PHONE: (512)439-4700

Watershed: SOUTH SAN GABRIEL FEMA Map: 48491C0455F DATED 12/10/2019 Zoning: PUD ORD 16-021-00 TCEQ Recharge Zone: CONTRIBUTING ZONE DISTURBED ACREAGE: 45.60 AC

Number	Revision Description				COA
		Revise (R) Add (A) Void (V) Sheet No.'s	NET Change Imp. Cover (SF)	Total Site Imp. Cover (%)	Approval Date
			×		
22					
, e *			0 9 28 7 2		
				residence of a	
			5 H 23 C	* 88 × *	
				t teas	
			2 2		

LOTS WITH 65 PSI OR GREATER REQUIRE A PRV TO BE INSTALLED ON THE PROPERTY OWNERS SIDE OF THE DOMESTIC WATER METER.

3. DOMESTIC WATER METER SIZE WITH AN ASTERISK (*) DENOTES THE NEED FOR TWO (2) METERS OF THE SAME SIZE.

2. MINIMUM FIRE FLOW IS REDUCED FIRE FLOW PER 75% FIRE SPRINKLER REDUCTION (25% OF REQUIRED FIRE FLOW) OR 1,500 GALLONS PER

REVISIONS / CORRECTIONS

Description Sheet No. **COVER SHEET** CV 1 GN 1 **GENERAL NOTES EROSION/SEDIMENTATION CONTROL & TREE PROTECTION PLAN EROSION/SEDIMENTATION CONTROL & TREE PROTECTION PLAN EROSION/SEDIMENTATION CONTROL & TREE PROTECTION PLAN** DM 1 **EXISTING DRAINAGE AREA MAP DEVELOPED DRAINAGE AREA MAP** DM 3 - DM 4 INTERNAL DRAINAGE AREA MAP DRAINAGE AREA CALCULATIONS ST 1 JOLIE ROSE BEND, PLAN & PROFILE, STA. 1+00 TO 10+00 ST 2 JOLIE ROSE BEND, PLAN & PROFILE, STA. 10+00 TO 20+00 ST 3 JOLIE ROSE BEND, PLAN & PROFILE, STA. 20+00 TO 27+31.17 ST 4 HOSANA GRANDE WAY, PLAN & PROFILE, STA. 27+31.17 TO 31+82.63 STARRY NIGHT LANE, PLAN & PROFILE, STA. 31+82.63 TO 34+62.35 LILLY GRACE BEND, PLAN & PROFILE, STA. 34.62.35 TO 41+00 LILLY GRACE BEND, PLAN & PROFILE, STA. 41+00 TO END ST8 TREASURE MAP VIEW, PLAN & PROFILE, STA. 1+00 TO END GP 1 - GP3 **GRADING PLAN** WATER QUALITY POND 7 WATER QUALITY POND 7 SECTIONS & CALCULATIONS WQ3 WATER QUALITY POND DETAILS WQ4 WATER QUALITY POND DETAILS WQ5 WATER QUALITY JELLYFISH FILTER WQ6 WATER QUALITY VEGETATIVE FILTER STRIPS WQ7 WATER QUALITY VEGETATIVE FILTER STRIPS CALCULATIONS SS₁ STORM SEWER LINE A, PLAN & PROFILE, STA. 1+00 TO 8+00 SS 2 STORM SEWER LINE A, PLAN & PROFILE, STA. 8+00 TO 13+00 STORM SEWER LINE A, PLAN & PROFILE, STA. 13+00 TO END STORM SEWER LINE A2-A10 PROFILES STORM SEWER LINE B, PLAN & PROFILE, STA. 1+00 TO 8+00 STORM SEWER LINE B, PLAN & PROFILE, STA. 8+00 TO END STORM SEWER LINE B5, PLAN & PROFILE, STA. 1+00 TO END **SS 8** STORM SEWER LINE B2-B4 & B6-B8 PROFILES STORM SEWER LINE C PLAN & PROFILE STA. 1+00 TO 5+00 SS 10 STORM SEWER LINE C, PLAN & PROFILE, STA. 5+00 TO END SS 11 STORM SEWER LINE C5, PLAN & PROFILE, STA. 1+00 TO END SS 12 STORM SEWER LINE C2-C4 PROFILES STORM SEWER LINE D PLAN & PROFILES STA 1+00 TO END SS 14 STORM SEWER LINE D2-D6 PROFILES **CHANNEL A PLAN & PROFILE** WATER LINE A PLAN & PROFILE STA. 1+00 TO 5+00 WATER LINE A PLAN & PROFILE STA.5+00 TO 10+00 WATER LINE A PLAN & PROFILE STA. 10+00 TO 20+00 WATER LINE A PLAN & PROFILE STA. 28+00 TO 33+00 WATER LINE A PLAN AND PROFILE STA. 33+00 TO 38+00 WL7 WATER LINE A PLAN AND PROFILE STA. 38+00 TO END WL8 WATER LINE B PLAN & PROFILE STA. 1+00 TO END WASTEWATER LINE A, PLAN & PROFILE, STA. 1+00 TO 6+00 WW 1 WASTEWATER LINE A, PLAN & PROFILE, STA. 6+00 TO 14+00 WASTEWATER LINE A, PLAN & PROFILE, STA. 14+00 TO 22+00 WASTEWATER LINE A, PLAN & PROFILE, STA. 22+00 TO 27+00 WW 4 WASTEWATER LINE A, PLAN & PROFILE, STA. 27+00 TO END WW 6 WASTEWATER LINE A1, PLAN & PROFILE, STA. 1+00 TO END WASTEWATER LINE A2, PLAN & PROFILE, STA. 1+00 TO END WW 7 WASTEWATER LINE A3, PLAN & PROFILE, STA. 1+00 TO END **WW8** WASTEWATER FORCEMAIN A, PLAN & PROFILE STA 1+00 TO END

- 1. ALL RESPONSIBILITY FOR THE ADEQUACY OF THESE PLANS REMAINS WITH THE ENGINEER WHO PREPARED THEM. IN APPROVING THESE PLANS, THE CITY OF LEANDER MUST RELY UPON THE ADEQUACY OF THE WORK OF THE DESIGN ENGINEER
- 2. THE ENGINEER OF RECORD IS SOLELY RESPONSIBLE FOR THE COMPLETENESS, ACCURACY, REGULATORY COMPLIANCE, AND ADEQUACY OF THESE PLANS OR SPECIFICATIONS, WHETHER OR NOT THE PLANS AND/OR SPECIFICATIONS WERE REVIEWED BY CITY ENGINEER(S).

ELECTRICAL SITE PLAN & DETAILS

GENERAL DETAILS

- 3. EXISTING UTILITIES ARE SHOWN PER RECORD. CONTRACTOR SHALL VERIFY LOCATIONS & ELEVATIONS OF EXISTING UTILITIES PRIOR TO INSTALLATION OF ANY PIPE AND SHALL NOTIFY ENGINEER OF ANY CONFLICTS.
- THIS SITE IS LOCATED OVER THE EDWARDS AQUIFER CONTRIBUTING ZONE.

62-69

- SURVEY DATA PROVIDED BY: RJ SURVERYING, INC. DATED OCTOBER 2020 TO MAY 2021.
- 6. WATER QUALITY AND DETENTION FACILITIES SHALL BE COMPLETE AND ACCEPTED BY CITY OF LEANDER PRIOR TO ISSUANCE OF CERTIFICATES OF
- 7. WATER, WASTEWATER, STORM DRAIN, STREETS, STREET LIGHTS, ROW, SIDEWALKS, AND PARK TRAILS SHALL BE MAINTAINED BY THE CITY OF LEANDER.
- 8. ANY STRUCTURAL STABILIZATION WITH SLOPES STEEPER THAN 3:1 SHALL BE LIMITED TO USE OF NATIVE STONE (EXCEPT FOR OUTLET STRUCTURES WHICH CAN BE CONCRETE) AND SHALL BE LIMITED TO NO MORE THAN THIRTY (30%) PERCENT OF THE PERIMETER OF THE POND. SUCH PONDS SHALL BE SEAMLESSLY INTEGRATED WITH THE LANDSCAPING. ALL EXPOSED CONCRETE THAT IS VISIBLE IS REQUIRED TO BE MADE OF STONE OR CLAD IN STONE INCLUDING BUT NOT LIMITED TO LEDGESTONE, FIELDSTONE, CAST STONE, OR OTHER DECORATIVE MATERIALS SUCH AS STAMPED AND TINTED CONCRETE THAT RESEMBLES STONE OR BRICK AS APPROVED BY THE DIRECTOR OF PLANNING. ALL OTHER EXPOSED CONCRETE IS REQUIRED TO BE MADE OF STONE OR CLAD IN STONE AS LISTED ABOVE OR TEXTURED AND TINTED IN EARTHERN COLORS.

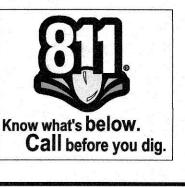
LJA Engineering, Inc.

Round Rock, TX 78681

2700 La Frontera Blvd Suite 150

Phone 512.439.4700 Fax 512.439.4716 FRN - F-1386

UNDERGROUND AND OVERHEAD UTILITIES ARE APPROXIMATE LOCATIONS ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL **EXISTING UTILITIES PRIOR TO** BEGINNING WORK AND SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT OCCUR.



ANY CHANGES TO THESE NOTES SHOULD BE CLOUDED ON THE PLAN SET.

ENGINEERING MAIN LINE: 512-528-2766 PLANNING DEPARTMENT: 512-528-2750 PUBLIC WORKS MAIN LINE: 512-259-2640 STORMWATER INSPECTIONS: 512-285-0055 UTILITIES MAIN LINE: 512-259-1142

UTILITIES ON-CALL: 512-690-4760 UTILITY LOCATE REQUESTS LOCATES@LEANDERTX.GOV

- 1. THE CONTRACTOR SHALL VERIFY ALL DEPTHS AND LOCATIONS OF EXISTING UTILITIES PRIOR TO ANY CONSTRUCTION. ANY DISCREPANCIES WITH CONSTRUCTION PLANS FOUND IN THE FIELD SHALL BE BROUGHT IMMEDIATELY TO THE ATTENTION OF THE ENGINEER.
- 2. THE CONTRACTOR SHALL CONTACT THE TEXAS EXCAVATION SYSTEM AT 1-800-344-8377 FOR EXISTING UTILITY LOCATIONS 48 HOURS PRIOR TO THE START OF CONSTRUCTION. THE CONTRACTOR SHALL VERIFY THE LOCATIONS OF ALL UTILITIES THAT ARE TO BE EXTENDED, TIED TO, CROSSED, OR ALTERED; OR SUBJECT TO DAMAGE/INCONVENIENCE BY THE CONSTRUCTION OPERATIONS.
- 3. CONTACT THE CITY OF LEANDER PUBLIC WORKS DEPARTMENT FOR EXISTING WATER AND WASTEWATER LOCATIONS 48 HOURS PRIOR TO CONSTRUCTION.
- A. LOCATE REQUESTS <u>MUST INCLUDE A COPY OF YOUR 811 TICKET</u>. THE CITY OF LEANDER IS ALLOWED UP TO 48 HOURS TO COMPLY WITH YOUR REQUEST, EXCLUDING WEEKENDS AND DESIGNATED CITY HOLIDAYS.
- B. REFRESH ALL LOCATES <u>BEFORE</u> 14 DAYS LOCATE REFRESH REQUESTS <u>MUST INCLUDE</u> A COPY OF YOUR 811 TICKET. SUBMIT ALL REQUESTS TO LOCATES@LEANDERTX.GOV. TEXAS PIPELINE DAMAGE PREVENTION LAWS REQUIRE THAT A LOCATE REFRESH REQUEST BE SUBMITTED BEFORE 14 DAYS, OR IF LOCATION MARKERS ARE NO LONGER
- C. REPORT PIPELINE DAMAGE IMMEDIATELY IF YOU WITNESS OR EXPERIENCE PIPELINE EXCAVATION DAMAGE, PLEASE CONTACT THE CITY OF LEANDER BY PHONE AT 512-259-
- 4. ANY CHANGES OR REVISIONS TO THESE PLANS MUST FIRST BE SUBMITTED TO THE CITY BY THE DESIGN ENGINEER FOR REVIEW AND WRITTEN APPROVAL PRIOR TO CONSTRUCTION OF
- 5. A TRAFFIC CONTROL PLAN, IN ACCORDANCE WITH THE TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, SHALL BE SUBMITTED TO THE CITY FOR REVIEW AND APPROVAL PRIOR TO ANY PARTIAL OR COMPLETE ROADWAY CLOSURES. TRAFFIC CONTROL PLANS SHALL BE SITE SPECIFIC AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER. LANE CLOSURES ON ARTERIALS AND ANY FULL ROAD CLOSURES REQUIRE MESSAGE BOARDS NOTIFYING THE PUBLIC ONE WEEK PRIOR TO THE CLOSURE.
- 6. NO WORK IS TO BE PERFORMED BETWEEN THE HOURS OF 9:00 P.M. AND 7:00 A.M. THE CITY INSPECTOR RESERVES THE RIGHT TO REQUIRE THE CONTRACTOR TO UNCOVER ALL WORK PERFORMED WITHOUT INSPECTION FURTHER, THERE IS A NOISE ORDINANCE IN EFFECT FOR CONSTRUCTION ACTIVITY BETWEEN THE HOURS OF 9:00 PM AND 7:00 AM. REQUESTS FOR EXCEPTIONS TO THE ORDINANCE MUST BE MADE TO LEANDER CITY COUNCIL.
- 7. CONTACT THE CITY INSPECTOR 4 DAYS PRIOR TO WORK TO SCHEDULE ANY INSPECTIONS ON WEEKENDS OR CITY HOLIDAYS.
- 8. NO STREET LIGHTS OR SIGNS OF ANY KIND ARE TO BE PLACED WITHIN ANY SIDEWALKS.
- NO BLASTING IS ALLOWED. 10. ANY EXISTING UTILITIES, PAVEMENT, CURBS, SIDEWALKS, STRUCTURES, TREES, ETC., THAT ARE DAMAGED OR REMOVED SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR AT NO
- 11. THE CONTRACTOR SHALL GIVE THE CITY OF LEANDER 48 HOURS NOTICE BEFORE BEGINNING EACH PHASE OF CONSTRUCTION, CONTACT ASSIGNED CITY INSPECTOR.
- 12. A PRE-CONSTRUCTION CONFERENCE SHALL BE HELD WITH THE CONTRACTOR, DESIGN ENGINEER/PERMIT APPLICANT AND THE CITY OF LEANDER REPRESENTATIVES PRIOR TO INSTALLATION OF EROSION/SEDIMENTATION CONTROLS AND TREE PROTECTION MEASURES AND PRIOR TO BEGINNING ANY WORK. THE CONTRACTOR SHALL NOTIFY THE CITY OF LEANDER PLANNING DEPARTMENT PLANNING COORDINATOR AT LEAST THREE (3) DAYS PRIOR TO THE MEETING DATE.
- 13. THE CONTRACTOR AND ENGINEER SHALL KEEP ACCURATE RECORDS OF ALL CONSTRUCTION THAT DEVIATES FROM THE PLANS. THE ENGINEER SHALL FURNISH THE CITY OF LEANDER ACCURATE "RECORD DRAWINGS" FOLLOWING THE COMPLETION OF ALL CONSTRUCTION. THESE "RECORD DRAWINGS" SHALL MEET THE SATISFACTION OF THE ENGINEERING DEPARTMENTS PRIOR TO FINAL ACCEPTANCE
- 14. WHEN CONSTRUCTION IS BEING CARRIED OUT WITHIN EASEMENTS, THE CONTRACTOR SHALL CONFINE HIS WORK TO WITHIN THE PERMANENT AND TEMPORARY EASEMENTS. PRIOR TO ACCEPTANCE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING ALL TRASH AND DEBRIS WITHIN THE PERMANENT EASEMENTS. CLEANUP SHALL BE TO THE SATISFACTION OF THE ENGINEER.
- 15. CONTRACTOR TO LOCATE, PROTECT, AND MAINTAIN BENCHMARKS, MONUMENTS, CONTROL POINTS AND PROJECT ENGINEERING REFERENCE POINTS. RE-ESTABLISH DISTURBED OR DESTROYED ITEMS BY REGISTERED PROFESSIONAL LAND SURVEYOR IN THE STATE OF TEXAS, AT NO ADDITIONAL COST TO OWNER.
- 16. THE CONTRACTOR SHALL PROTECT ALL EXISTING FENCES. IN THE EVENT THAT A FENCE MUST BE REMOVED, THE CONTRACTOR SHALL REPLACE SAID FENCE OR PORTION THEREOF WITH THE SAME TYPE OF FENCING TO A QUALITY OF EQUAL OR BETTER THAN THE ORIGINAL
- 17. ALL CONSTRUCTION OPERATIONS SHALL BE ACCOMPLISHED IN ACCORDANCE WITH APPLICABLE REGULATIONS OF THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA). OSHA STANDARDS MAY BE PURCHASED FROM THE GOVERNMENT PRINTING OFFICE; INFORMATION AND RELATED REFERENCE MATERIALS MAY BE PURCHASED FROM OSHA, 1033 LA POSADA DR. SUITE 375, AUSTIN, TEXAS 78752-3832.
- 18. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THE SCOPE OF THIS CONTRACT WHERE NOT SPECIFICALLY COVERED IN THE PROJECT SPECIFICATIONS SHALL CONFORM TO ALL CITY OF LEANDER DETAILS AND CITY OF AUSTIN STANDARD SPECIFICATIONS.
- 19. PROJECT SPECIFICATIONS TAKE PRECEDENCE OVER PLANS AND SPECIAL CONDITIONS GOVERN OVER TECHNICAL SPECIFICATIONS.
- 20. HOT MIX ASPHALTIC CONCRETE PAVEMENT SHALL BE MINIMUM THICKNESS OF 2 INCHES WITH NO RECYCLED ASPHALT SHINGLES CONTENT. 21. CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY QUESTIONS THAT MAY

ACCEPTANCES REQUIRED TO COMPLETE CONSTRUCTION OF THIS PROJECT.

- RISE CONCERNING THE INTENT, PLACEMENT, OR LIMITS OF DIMENSIONS OR GRADES NECESSARY FOR THE CONSTRUCTION OF THIS PROJECT. 22. CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ALL PERMITS, TESTS, APPROVALS AND
- 23. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COORDINATION BETWEEN HIMSELF AND OTHER CONTRACTORS AND UTILITIES IN THE VICINITY OF THE PROJECT. THIS INCLUDES GAS, WATER, WASTEWATER, ELECTRICAL, TELEPHONE, CABLE TV AND STREET DRAINAGE WORK. ONCE THE CONTRACTOR BECOMES AWARE OF A POSSIBLE CONFLICT, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ENGINEER WITHIN TWENTY-FOUR (24) HOURS.

- 24. THE CONTRACTOR MUST OBTAIN A CONSTRUCTION WATER METER FOR ALL WATER USED DURING CONSTRUCTION. A COPY OF THIS PERMIT MUST BE CARRIED AT ALL TIMES BY ALL
- 25. CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING ROADS AND DRIVES ADJACENT TO AND NEAR THE SITE FREE FROM SOIL, SEDIMENT AND DEBRIS. CONTRACTOR WILL NOT REMOVE SOIL, SEDIMENT OR DEBRIS FROM ANY AREA OR VEHICLE BY MEANS OF WATER. ONLY SHOVELING AND SWEEPING WILL BE ALLOWED. CONTRACTOR WILL BE RESPONSIBLE FOR DUST CONTROL FROM THE SITE.
- 26. THE CITY OF LEANDER SHALL NOT BE PETITIONED FOR ACCEPTANCE UNTIL ALL NECESSARY EASEMENT DOCUMENTS HAVE BEEN SIGNED AND RECORDED.
- 27. AN ENGINEER'S CONCURRENCE LETTER AND RECORD DRAWINGS SHALL BE SUBMITTED TO THE ENGINEERING DEPARTMENT PRIOR TO THE ISSUANCE OF CERTIFICATE OF COMPLETION OR SUBDIVISION ACCEPTANCE. THE ENGINEER AND CONTRACTOR SHALL VERIFY THAT ALL FINAL REVISIONS AND CHANGES HAVE BEEN MADE TO THE DIGITAL COPY PRIOR TO CITY SUBMITTAL. RECORD CONSTRUCTION DRAWINGS, INCLUDING ROADWAY AND ALL UTILITIES SHALL BE PROVIDED TO THE CITY IN DIGITAL FORMAT AS AUTOCAD ".DWG" FILES, MICROSTATION ".DGN" FILES OR ESRI ".SHP" FILES ON CD ROM. LINE WEIGHTS, LINE TYPES AND TEXT SIZE SHALL BE SUCH THAT IF HALF-SIZE PRINTS (11"X17") WERE PRODUCED, THE PLANS WOULD STILL BE LEGIBLE. ALL REQUIRED DIGITAL FILES SHALL CONTAIN A MINIMUM OF TWO CONTROL POINTS REFERENCED TO THE STATE PLANE GRID COORDINATE SYSTEM -TEXAS CENTRAL ZONE (4203), IN US SURVEY FEET AND SHALL INCLUDE ROTATION INFORMATION AND SCALE FACTOR REQUIRED TO REDUCE SURFACE COORDINATES TO GRID COORDINATES IN US SURVEY FEET
- 28. TREES IN EXISTING ROW SHOULD BE PROTECTED OR NOTED IN THE PLANS TO BE REMOVED.

EROSION CONTROL NOTES

- 1. THE CONTRACTOR SHALL INSTALL EROSION/SEDIMENTATION CONTROLS AND TREE PROTECTIVE FENCING PRIOR TO ANY WORK (CLEARING, GRUBBING OR EXCAVATION). CONTACT STORMWATER INSPECTOR FOR ON SITE INSPECTION PRIOR TO BEGINNING CONSTRUCTION.
- 2. THE CONTRACTOR IS REQUIRED TO INSPECT THE CONTROLS AND FENCES AT WEEKLY INTERVALS AND AFTER SIGNIFICANT RAINFALL EVENTS TO ENSURE THAT THEY ARE FUNCTIONING PROPERLY. THE PERSON(S) RESPONSIBLE FOR MAINTENANCE OF CONTROLS AND FENCES SHALL IMMEDIATELY MAKE ANY NECESSARY REPAIRS TO DAMAGED AREAS. SILT ACCUMULATION AT CONTROLS MUST BE REMOVED WHEN THE DEPTH REACHES SIX (6) INCHES.
- THE TEMPORARY SPOILS DISPOSAL SITE IS TO BE SHOWN IN THE EROSION CONTROL MAP. 4. ANY ON-SITE SPOILS DISPOSAL SHALL BE REMOVED PRIOR TO ACCEPTANCE UNLESS SPECIFICALLY SHOWN ON THE PLANS. THE DEPTH OF SPOIL SHALL NOT EXCEED 10 FEET IN ANY
- 5. ALL AREAS DISTURBED OR EXPOSED DURING CONSTRUCTION SHALL BE RESTORED WITH A MINIMUM OF 6 INCHES OF TOPSOIL AND COMPOST BLEND. TOPSOIL ON SINGLE FAMILY LOTS MAY BE INSTALLED WITH HOME CONSTRUCTION. THE TOPSOIL AND COMPOST BLEND SHALL CONSIST OF 75% TOPSOIL AND 25% COMPOST.
- SEEDING FOR REESTABLISHING VEGETATION SHALL COMPLY WITH THE AUSTIN GROW GREEN GUIDE OR WILLIAMSON COUNTY'S PROTOCOL FOR SUSTAINABLE ROADSIDES (SPEC 164--WC001 SEEDING FOR EROSION CONTROL). RESEEDING VARIETIES OF BERMUDA SHALL NOT BE USED.
- 7. STABILIZED CONSTRUCTION ENTRANCE IS REQUIRED AT ALL POINTS WHERE CONSTRUCTION TRAFFIC IS EXITING THE PROJECT ONTO EXISTING PAVEMENT. LINEAR CONSTRUCTION PROJECTS
- MAY REQUIRE SPECIAL CONSIDERATION. ROADWAYS SHALL REMAIN CLEAR OF SILT AND MUD. 8. TEMPORARY STOP SIGNS SHOULD BE INSTALLED AT ALL CONSTRUCTION ENTRANCES WHERE A STOP CONDITION DOES NOT ALREADY EXIST.
- 9. IN THE EVENT OF INCLEMENT WEATHER THAT MAY RESULT IN A FLOODING SITUATION, THE CONTRACTOR SHALL REMOVE INLET PROTECTION MEASURES UNTIL SUCH TIME AS THE WEATHER EVENT HAS PASSED.

WATER AND WASTEWATER NOTES

- PRESSURE TAPS SHALL BE IN ACCORDANCE WITH CITY OF LEANDER STANDARD SPECIFICATIONS. THE CONTRACTOR SHALL PERFORM ALL EXCAVATION, ETC. AND SHALL FURNISH, INSTALL AND AIR TEST THE SLEEVE AND VALVE. A CITY OF LEANDER INSPECTOR MUST BE PRESENT WHEN THE CONTRACTOR MAKES A TAP, AND/OR ASSOCIATED TESTS, A MINIMUM OF TWO (2) WORKING DAYS NOTICE IS REQUIRED. "SIZE ON SIZE" TAPS WILL NOT BE PERMITTED UNLESS MADE BY THE USE OF AN APPROVED FULL-CIRCLE GASKETED TAPPING SLEEVE. CONCRETE BLOCKING SHALL BE PLACED BEHIND AND UNDER ALL TAP SLEEVES A MINIMUM OF 24 HOURS PRIOR TO THE BRANCH BEING PLACED INTO SERVICE. BLOCKING SHALL BE INSPECTED PRIOR TO BACKFILL
- 2. FIRE HYDRANTS ON MAINS UNDER CONSTRUCTION SHALL BE SECURELY WRAPPED WITH A BLACK POLY WRAP BAG AND TAPED INTO PLACE. THE POLY WRAP SHALL BE REMOVED WHEN THE MAINS ARE ACCEPTED AND PLACED INTO SERVICE.
- CURVILINEAR WASTEWATER DESIGN LAYOUT IS NOT PERMITTED
- 4. THRUST BLOCKING OR RESTRAINTS SHALL BE IN ACCORDANCE WITH THE CITY OF LEANDER STANDARD SPECIFICATIONS AND REQUIRED AT ALL FITTINGS PER DETAIL OR MANUFACTURER'S RECOMMENDATION. ALL FITTINGS SHALL HAVE BOTH THRUST BLOCKING AND RESTRAINTS.
- 5. MANDREL TESTING WILL BE REQUIRED ON ALL WASTEWATER PIPE. PER TCEQ, THIS TEST MUST BE CONDUCTED AFTER THE FINAL BACKFILL HAS BEEN IN PLACE AT LEAST 30 DAYS.
- 6. ALL NEWLY INSTALLED PIPES AND RELATED PRODUCTS MUST CONFORM TO AMERICAN NATIONAL STANDARDS INSTITUTE/NATIONAL SANITATION FOUNDATION (ANSI/NSF) STANDARD 61 AND MUST BE CERTIFIED BY AND ORGANIZATION ACCREDITED BY ANSI
- 7. DURING PERIODS OF EXTENDED DRY WEATHER, TRENCH BACKFILL MUST BE COMPACTED BY FLOODING THE TRENCHES AS DIRECTED BY THE CITY ENGINEER.
- 8. ALL WATER SERVICE, WASTEWATER SERVICE AND VALVE LOCATIONS SHALL BE APPROPRIATELY STAMPED AS FOLLOWS:

WATER SERVICE "W" ON TOP OF CURB WASTEWATER SERVICE "S" ON TOP OF CURB "V" ON TOP OF CURB

- 9. TOOLS FOR STAMPING THE CURBS SHALL BE PROVIDED BY THE CONTRACTOR. OTHER APPROPRIATE MEANS OF STAMPING SERVICE AND VALVE LOCATIONS SHALL BE PROVIDED IN AREAS WITHOUT CURBS. SUCH MEANS OF STAMPING SHALL BE SPECIFIED BY THE ENGINEER AND ACCEPTED BY THE CITY OF LEANDER
- 10. ALL PLASTIC PIPES FOR USE IN PUBLIC WATER SYSTEMS MUST BEAR THE NATIONAL SANITATION FOUNDATION SEAL OF APPROVAL (NSF-PW) AND HAVE AN ASTM DESIGN PRESSURE RATING OF AT LEAST 200 PSI.
- 11. NO PIPE OR FITTING WHICH HAS BEEN USED FOR ANY PURPOSE OTHER THAN THE CONVEYANCE OF DRINKING WATER SHALL BE ACCEPTED OR RELOCATED FOR USE IN ANY PUBLIC DRINKING 12. TYPICAL DEPTH OF COVER FOR ALL WASTEWATER LINES SHALL BE 48" MINIMUM, WATER LINES
- SHALL BE 36" MINIMUM UNDER BOTH PAVEMENT AND NATURAL GROUND. STORM SEWER SHALL BE 24" MINIMUM UNDER NATURAL GROUND. 13. THE HYDROSTATIC LEAKAGE RATE SHALL NOT EXCEED THE AMOUNT ALLOWED OR
- RECOMMENDED BY AWWA FORMULAS.
- 14. ALL WATER MAINS, DISTRIBUTION LINES AND SERVICE LINES SHALL BE INSTALLED IN ENCASEMENT PIPE UNDERNEATH EXISTING STREETS AND OTHER PAVED SURFACES UNLESS APPROVED WITH PLANS.
- 15. ALL MECHANICAL RESTRAINTS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS.
- 16. ALL DEAD-END WATER MAINS SHALL HAVE THRUST RESTRAINTS INSTALLED ON THE LAST THREE PIPE-LENGTHS (STANDARD 20' LAYING LENGTH), AT MINIMUM, AND THRUST BLOCKS INSTALLED ON THE PLUG. ADDITIONAL THRUST RESTRAINTS MAY BE REQUIRED BASED UPON THE MANUFACTURER'S RECOMMENDATIONS AND/OR CALCULATIONS BY THE ENGINEER OF RECORD.

- 17. WHERE WATER LINES CROSS WASTEWATER LINES AND THERE IS LESS THAN 9 FEET CLEARANCE BETWEEN LINES, THE WASTEWATER LINE SHALL BE PLACED SO THAT THE WASTEWATER PIPE SECTION IS CENTERED ON THE WATER LINE AND CONSTRUCTED IN ACCORDANCE WITH TCEQ CHAPTERS 217.53(B) AND 290.44(E).
- 18. PIPE MATERIAL FOR WATER MAINS SHALL BE PVC (AWWA C900-16 MIN. 235 PSI PRESSURE RATING). WATER SERVICES (2" OR LESS) SHALL BE POLYETHYLENE TUBING (BLACK, 200PSI, SDR-(9)). DUCTILE IRON PIPE (AWWA C115/C151, MIN. PRESSURE CLASS 250) MAY BE USED FOR
- WATER MAINS WITH THE EXPRESS APPROVAL OF CITY OF LEANDER ENGINEERING. 19. PIPE FOR PRESSURE WASTEWATER MAINS SHALL BE PVC (AWWA C900-16), GREEN AND
- MARKED FOR SEWER. PIPE MATERIAL FOR GRAVITY WASTEWATER MAINS SHALL BE PVC (ASTM D2241, D3034 MAX. SDR-26 OR PS115 F679) OR FIBERGLASS WITH PIPE STIFFNESS OF 72 PSI PER
- 20. ALL FIRE HYDRANT LEADS SHALL BE DUCTILE IRON PIPE (AWWA C115/C151 PRESSURE CLASS
- 21. INTERIOR SURFACES OF ALL DUCTILE IRON POTABLE OR RECLAIMED WATER PIPE SHALL BE CEMENT-MORTAR LINED AND SEAL COATED AS REQUIRED BY AWWA C104.
- 22. ALL IRON PIPE AND FITTINGS SHALL BE WRAPPED WITH MINIMUM 8-MIL POLYETHYLENE. 23. THE CONTRACTOR SHALL CONTACT THE ENGINEERING DEPARTMENT INSPECTOR AT 528-2700
- AT LEAST 48 HOURS PRIOR TO CONNECTING TO THE EXISTING WATER LINES. 24. ALL MANHOLES SHALL BE CONCRETE WITH CAST IRON RING AND COVER. TAPPING OF FIBERGLASS MANHOLES SHALL NOT BE ALLOWED.
- 25. EXISTING MANHOLES MODIFIED BY CONSTRUCTION ACTIVITY SHALL BE TESTED FOR LEAKAGE BY VACUUM. ANY EXISTING MANHOLE WHICH FAILS TO PASS THE VACUUM TEST SHALL BE CLOSELY EXAMINED BY THE INSPECTOR AND THE CONTRACTOR TO DETERMINE IF THE MANHOLE CAN BE REPAIRED. THEREAFTER, THE CONTRACTOR SHALL EITHER REPAIR OR REMOVE AND REPLACE THE MANHOLE AS DIRECTED.
- 26. PIPE CONNECTIONS TO EXISTING MANHOLES AND JUNCTION BOXES SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF AUSTIN SPECIFICATION 506.5.F.
- 27. LINE FLUSHING OR ANY ACTIVITY USING A LARGE QUANTITY OF WATER MUST BE COORDINATED WITH THE PUBLIC WORKS DEPARTMENT.
- 28. THE CONTRACTOR, AT HIS EXPENSE, SHALL PERFORM STERILIZATION OF ALL CONSTRUCTED POTABLE WATER LINES AND SHALL PROVIDE ALL EQUIPMENT (INCLUDING TEST GAUGES), SUPPLIES (INCLUDING CONCENTRATED CHLORINE DISINFECTING MATERIAL), AND NECESSARY LABOR REQUIRED FOR THE STERILIZATION PROCEDURE. THE STERILIZATION PROCEDURE SHALL BE MONITORED BY CITY OF LEANDER PERSONNEL. WATER SAMPLES WILL BE COLLECTED BY THE CITY OF LEANDER TO VERIFY EACH TREATED LINE HAS ATTAINED AN INITIAL CHLORINE CONCENTRATION OF 50 PPM. WHERE MEANS OF FLUSHING IS NECESSARY, THE CONTRACTOR, AT HIS EXPENSE, SHALL PROVIDE FLUSHING DEVICES AND REMOVE SAID DEVICES PRIOR TO FINAL ACCEPTANCE BY THE CITY OF LEANDER.
- 29. SAMPLING TAPS SHALL BE BROUGHT UP TO 3 FEET ABOVE GRADE AND SHALL BE EASILY ACCESSIBLE FOR CITY PERSONNEL. AT THE CONTRACTORS' REQUEST, AND IN HIS PRESENCE, SAMPLES FOR BACTERIOLOGICAL TESTING WILL BE COLLECTED BY THE CITY OF LEANDER NOT LESS THAN 24 HOURS AFTER THE TREATED LINE HAS BEEN FLUSHED OF THE CONCENTRATED CHLORINE SOLUTION AND CHARGED WITH WATER APPROVED BY THE CITY.
- 30. TESTING SHALL BE PERFORMED FOR ALL WASTEWATER PIPE INSTALLED AND PRESSURE PIPE HYDROSTATIC TESTING OF ALL WATER LINES CONSTRUCTED. THE OWNER'S CONTRACTOR SHALL PROVIDE ALL EQUIPMENT (INCLUDING PUMPS AND GAUGES), SUPPLIES AND LABOR NECESSARY TO PERFORM THE TESTS. THE CONTRACTOR SHALL NOTIFY THE CITY OF LEANDER ENGINEERING DEPARTMENT NO LESS THAN 48 HOURS PRIOR TO PERFORMING STERILIZATION, QUALITY TESTS, OR PRESSURE TESTS. A CITY OF LEANDER INSPECTOR SHALL BE PRESENT FOR ALL TESTS AND SHALL BE PAID FOR BY THE OWNER/CONTRACTOR. THESE SERVICES ARE PAID FOR AT THE TIME OF CONSTRUCTION PLAN SUBMITTAL.
- 31. THE CONTRACTOR SHALL NOT OPEN OR CLOSE ANY VALVE UNLESS AUTHORIZED BY THE CITY OF
- 32. ALL VALVE BOXES AND COVERS SHALL BE CAST IRON.
- 33. ALL WATER VALVE COVERS ARE TO BE PAINTED BLUE.
- 34. ALL WATER METER BOXES SHALL BE:
- a. SINGLE, 1" METER AND BELOW DFW37F-12-1CA, OR EQUAL b. DUAL, 1" METERS AND BELOW DFW39F-12-1CA, OR EQUAL
- c. 1.5" SINGLE METER DFW65C-14-1CA, OR EQUAL d. 2" SINGLE METER DFW1730F-12-1CA, OR EQUAL
- 35. SAND, AS DESCRIBED IN AUSTIN SPECIFICATION ITEM 510 PIPE, SHALL NOT BE USED AS BEDDING FOR WATER AND WASTEWATER LINES. ACCEPTABLE BEDDING MATERIALS ARE PIPE BEDDING STONE, PEA GRAVEL AND IN LIEU OF SAND, A NATURALLY OCCURRING OR MANUFACTURED STONE MATERIAL CONFORMING TO ASTM C33 FOR STONE QUALITY AND MEETING THE FOLLOWING GRADATION SPECIFICATION:

SIEVE SIZE	PERCENT RETAINED BY WEIGHT
1/2"	0
3/8"	0-2

95-100

- 36. THE CONTRACTOR IS HEREBY NOTIFIED THAT CONNECTING TO, SHUTTING DOWN, OR TERMINATING EXISTING UTILITY LINES MAY HAVE TO OCCUR AT OFF-PEAK HOURS. SUCH HOURS ARE USUALLY OUTSIDE NORMAL WORKING HOURS AND POSSIBLY BETWEEN 12 AM AND
- 37. ALL WASTEWATER CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ) REGULATIONS, 30 TAC CHAPTER 213 AND 30 TAC CHAPTER 217, AS APPLICABLE. WHENEVER TCEQ AND CITY OF LEANDER SPECIFICATION CONFLICT, THE MORE STRINGENT SHALL APPLY.
- 38. MANHOLES SHALL BE COATED PER CITY OF AUSTIN SPL WW-511 (RAVEN 405 OR SPRAYWALL). 39. DENSITY TESTING FOR TRENCH BACKFILL LOCATED WITHIN THE LIMITS OF THE PAVED AREA IS
- TO BE DONE IN 12" LIFTS EVERY 500' AND AT LEAST ONCE PER LINE SEGMENT 40. ALL GRAVITY WASTEWATER MAINS TO BE TESTED BY CAMERA AND PAID FOR BY THE CONTRACTOR. CAMERA TESTING FOR WASTEWATER LINES IN ROADWAY SHALL OCCUR BEFORE PAVING. CONTRACTOR SHALL PROVIDE THE CITY WITH A DVD COPY OF THE FULL CAMERA
- 41. RECLAIMED AND RECYCLED WATER LINE SHALL BE CONSTRUCTED OF "PURPLE PIPE." ALL RECLAIMED AND RECYCLED WATER VALVE COVERS SHALL BE SQUARE AND PAINTED PURPLE.

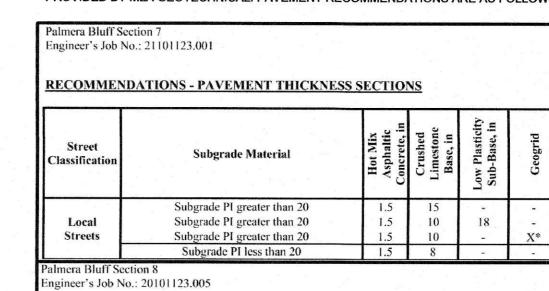
STREET AND DRAINAGE NOTES

- ALL SIDEWALKS SHALL COMPLY WITH THE AMERICANS WITH DISABILITIES ACT. THE CITY OF LEANDER HAS NOT REVIEWED THESE PLANS FOR COMPLIANCE WITH THE AMERICANS WITH DISABILITIES ACT, OR ANY OTHER ACCESSIBILITY LEGISLATION, AND DOES NOT WARRANTY OR APPROVE THESE PLANS FOR ANY ACCESSIBILITY STANDARDS.
- 2. PRIOR TO ACCEPTANCE THE ENGINEER SHALL SUBMIT DOCUMENTATION THAT THE IMPROVEMENTS WERE INSPECTED BY TDLR OR A REGISTERED ACCESSIBLITY SPECIALIST (RAS) AND ARE IN COMPLIANCE WITH THE REQUIREMENTS OF THE TABA.
- 3. CONTRACTOR SHALL PROVIDE QUALITY TESTING FOR ALL INFRASTRUCTURES TO BE ACCEPTED AND MAINTAINED BY THE CITY OF LEANDER AFTER COMPLETION. THE CONTRACTOR SHALL NOTIFY THE CITY OF LEANDER ENGINEERING DEPARTMENT AT 528-2700 NO LESS THAN 48 HOURS PRIOR TO ANY TESTING.
- 4. BACKFILL BEHIND THE CURB SHALL BE COMPACTED TO OBTAIN A MINIMUM OF 95% MAXIMUM DENSITY TO WITHIN 6" OF TOP OF CURB. MATERIAL USED SHALL BE PRIMARILY GRANULAR WITH NO ROCKS LARGER THAN 6" IN THE GREATEST DIMENSION. THE REMAINING 6" SHALL BE CLEAN TOPSOIL FREE FROM ALL CLODS AND SUITABLE FOR SUSTAINING PLANT LIFE
- 5. A MINIMUM OF 6" OF TOPSOIL SHALL BE PLACED BETWEEN THE CURB AND RIGHT-OF-WAY AND IN ALL DRAINAGE CHANNELS EXCEPT CHANNELS CUT IN STABLE ROCK.
- 6. DEPTH OF COVER FOR ALL CROSSINGS UNDER PAVEMENT, INCLUDING GAS, ELECTRIC TELEPHONE, CABLE TV, ETC., SHALL BE A MINIMUM OF 36" BELOW SUBGRADE.
- STREET RIGHT-OF-WAY SHALL BE GRADED AT A SLOPE OF 1/4" PER FOOT TOWARD THE CURB UNLESS OTHERWISE INDICATED. HOWEVER, IN NO CASE SHALL THE WIDTH OF RIGHT-OF-WAY AT 1/4" PER FOOT SLOPE BE LESS THAN 10 FEET UNLESS A SPECIFIC REQUEST FOR AN ALTERNATE GRADING SCHEME IS MADE TO AND ACCEPTED BY THE CITY OF LEANDER PUBLIC WORKS
- 8. BARRICADES BUILT TO THE CITY OF LEANDER STANDARDS SHALL BE ERECTED ON ALL DEAD-END STREETS AND AS NECESSARY DURING CONSTRUCTION TO MAINTAIN JOB AND PUBLIC SAFETY.
- 9. ALL REINFORCED CONCRETE PIPE SHALL BE MINIMUM CLASS III OF TONGUE AND GROOVE OR O-RING JOINT DESIGN.

- 10. THE CONTRACTOR IS TO NOTIFY THE ENGINEERING INSPECTOR 48 HOURS PRIOR TO THE FOLLOWING TESTING: PROOF ROLLING SUB-GRADE AND EVERY LIFT OF ROADWAY EMBANKMENT, IN-PLACE DENSITY TESTING OF EVERY BASE COURSE, AND ASPHALT CORES. ALL OF THIS TESTING MUST BE WITNESSED BY A CITY OF LEANDER REPRESENTATIVE.
- 11. THE CONTRACTOR MUST PROVIDE A PNEUMATIC TRUCK PER TXDOT SPEC FOR PROOF ROLLING. 12. AT INTERSECTIONS WHICH HAVE VALLEY DRAINAGE, THE CROWNS OF THE INTERSECTING STREETS WILL CULMINATE IN A DISTANCE OF 40 FEET FROM INTERSECTING CURB LINE UNLESS OTHERWISE NOTED.
- 13. AT THE INTERSECTION OF TWO 44' STREETS OR LARGER, THE CROWNS OF THE INTERSECTING STREETS WILL CULMINATE IN A DISTANCE OF 40 FEET FROM INTERSECTING CURB LINE UNLESS
- 14. A CURB LAYDOWN IS REQUIRED AT ALL POINTS WHERE THE PROPOSED SIDEWALK INTERSECTS
- 15. ALL STRIPING, WITH THE EXCEPTION OF STOP BARS, CROSS WALKS, WORDS AND ARROWS, IS TO BE TYPE II (WATER BASED). STOP BARS, CROSS WALKS, WORDS AND ARROWS REQUIRE TYPE I
- 16. MANHOLE FRAMES, COVERS, VALVES, CLEAN-OUTS, ETC. SHALL BE RAISED TO GRADE PRIOR TO FINAL PAVEMENT CONSTRUCTION.
- 17. CONTRACTOR SHALL NOTIFY THE LEANDER ENGINEERING DEPARTMENT AT 528-2700 AT LEAST 48 HOURS PRIOR TO THE INSTALLATION OF ANY DRAINAGE FACILITY WITHIN A DRAINAGE EASEMENT OR STREET ROW. THE METHOD OF PLACEMENT AND COMPACTION OF BACKFILL IN THE CITY'S ROW MUST BE APPROVED PRIOR TO THE START OF BACKFILL OPERATIONS.
- 18. A STOP BAR SHALL BE PLACED AT ALL STOP SIGN LOCATIONS.

INTRODUCTION OF PUBLIC VEHICULAR TRAFFIC TO ANY STREETS.

- 19. A MINIMUM OF SEVEN DAYS OF CURE TIME IS REQUIRED FOR HMAC PRIOR TO THE
- 20. THE GEOTECHNICAL ENGINEER SHALL INSPECT THE SUBGRADE FOR COMPLIANCE WITH THE DESIGN ASSUMPTIONS MADE DURING PREPARATION OF THE SOILS REPORT. ANY ADJUSTMENTS THAT ARE REQUIRED SHALL BE MADE THROUGH REVISIONS OF THE CONSTRUCTION PLANS.
- 21. GEOTECHNICAL INVESTIGATION INFORMATION AND PAVEMENT RECOMMENDATIONS WERE PROVIDED BY MLA GEOTECHNICAL. PAVEMENT RECOMMENDATIONS ARE AS FOLLOWS:



RECOMMENDATIONS - PAVEMENT THICKNESS SECTIONS

Street Classification	Subgrade Material	Hot Mix Asphaltic Concrete, in	Crushed Limestone Base, in	Low Plasticity Sub-Base, in	Geogrid
2	Subgrade PI greater than 20	1.5	15	-	-
Local	Subgrade PI greater than 20	1.5	10	18	-
Streets	Subgrade PI greater than 20	1.5	10	To decide the	X*
	Subgrade PI less than 20	1.5	8	•	-
	Subgrade PI greater than 20	2.0	19	-	-
Residential	Subgrade PI greater than 20	2.0	13	18	
Collectors	Subgrade PI greater than 20	2.0	13	-	X*
	Subgrade PI less than 20	2.0	10		
	Subgrade PI greater than 20	2.0	21		-
Neighborhood	Subgrade PI greater than 20	2.0	14	18	
Collectors	Subgrade PI greater than 20	2.0	14	- 1	X*
	Subgrade PI less than 20	2.0	.11	-	-

* - A single layer of Tensar TX-130S or equivalent to be approved by the Geotechnical Enginee

- should be placed below the crushed limestone base layer. Any expansive fill (PI > 20) placed in the subgrade shall be considered expansive subgrade. If low PI (PI < 20) soils are available on site, these soils could be utilized in the pavement areas in order to reduce pavement construction cost. The Client and Geotechnical Engineer should be consulted before any low PI soils are hauled off site. Delineation between these different pavement thickness sections should be completed in the
- field by observation of open utilities trenches and the pavement subgrade by the Geotechnical Engineer or his designate. Given the known variability of the surface soils, the geotechnica engineer must verify the subgrade before installation of the pavement system can proceed. Multiple site visits may be required depending upon the construction schedule. Finalized distinction between pavement thickness section options shall be provided as addendums to this report as these observations are completed. Please contact the geotechnical engineer when the utility trenches are
- Any subgrade improvement should be extended 3 feet beyond the back of the curb line. These pavement thickness designs are intended to transfer the load from the anticipated traffic
- The responsibility of assigning street classification to the streets in this project is left to the civi

If pavement designs other than those listed above are desired, please contact MLA Geotechnical.

TRENCH SAFETY NOTES

1. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT ARE DESCRIBED IN ITEM 509S "TRENCH SAFETY SYSTEMS" OF THE CITY OF AUSTIN STANDARD SPECIFICATIONS AND SHALL BE IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATION SAFETY AND HEALTH ADMINISTRATION REGULATIONS.

GRADING NOTES

- 1. POSITIVE DRAINAGE SHALL BE MAINTAINED ON ALL SURFACE AREAS WITHIN THE SCOPE OF THIS PROJECT. CONTRACTOR SHOULD TAKE PRECAUTIONS NOT TO ALLOW ANY PONDING OF
- 2. THE CONTRACTOR SHALL CONSTRUCT EARTHEN EMBANKMENTS WITH SLOPES NO STEEPER THAN 3:1 AND COMPACT SOIL TO 95% OF MAXIMUM DENSITY IN ACCORDANCE WITH THE CITY OF AUSTIN STANDARD SPECIFICATIONS.
- 3. AREAS OF SOIL DISTURBANCE ARE LIMITED TO GRADING AND IMPROVEMENTS SHOWN, ALL OTHER AREAS WILL NOT BE DISTURBED.

BENCHMARK NOTES

AT THE INTERSECTION OF LOGAN DEL WAY AND CLARISSA LYNN WAY E3084863.4850'.N10191881.8100' TRI ON TOC ELEV=1020.31' AT THE INTERSECTION OF LOGAN DEL WAY AND SIR NATHANIEL LANE

E3084435.2720'.N10192127.1500' "X" ON TOC ELEV=1020.31"

JOB NUMBER: A311-0415

X

T. W. HOYSA

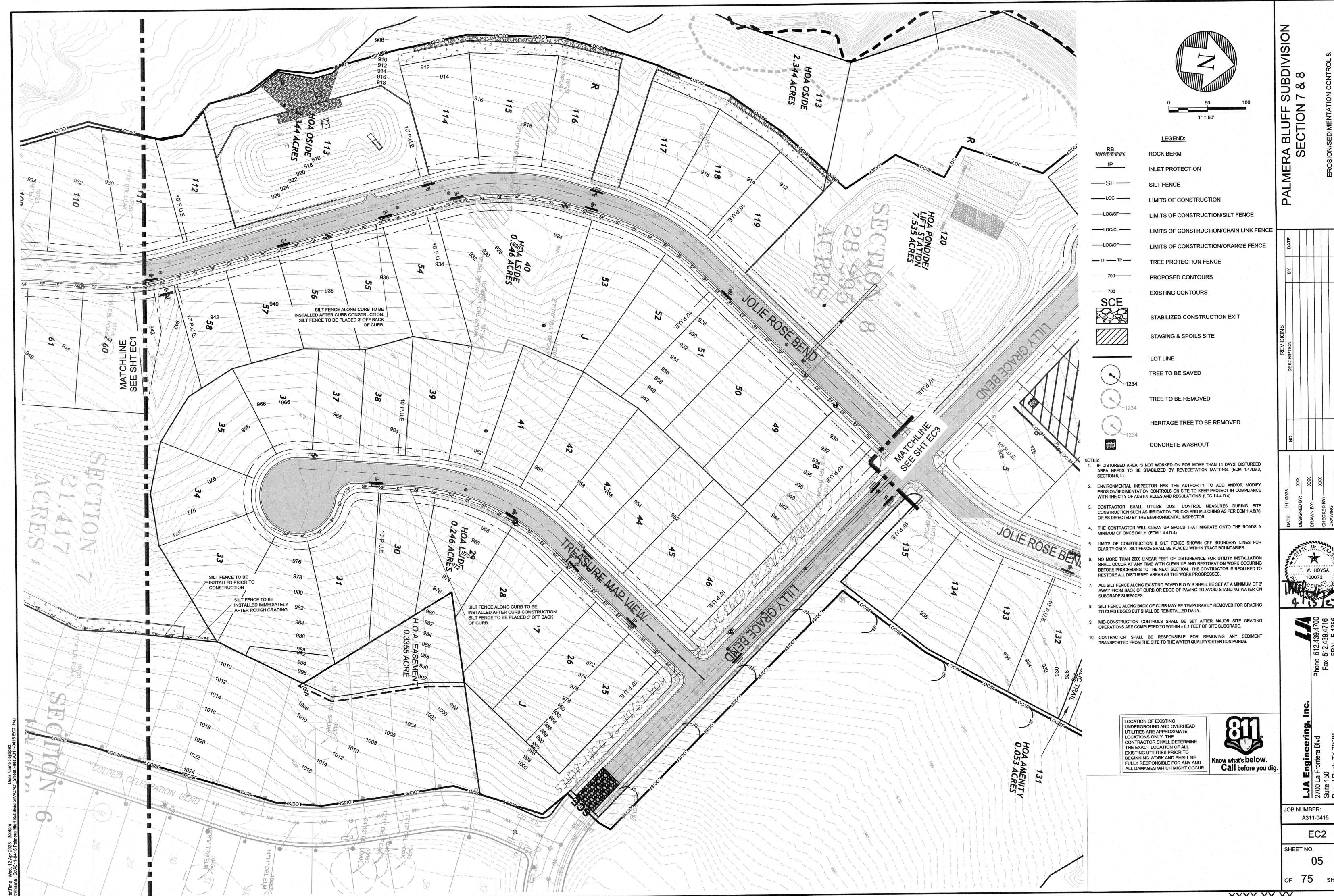
SHEET NO.

XXXX-XX-XX

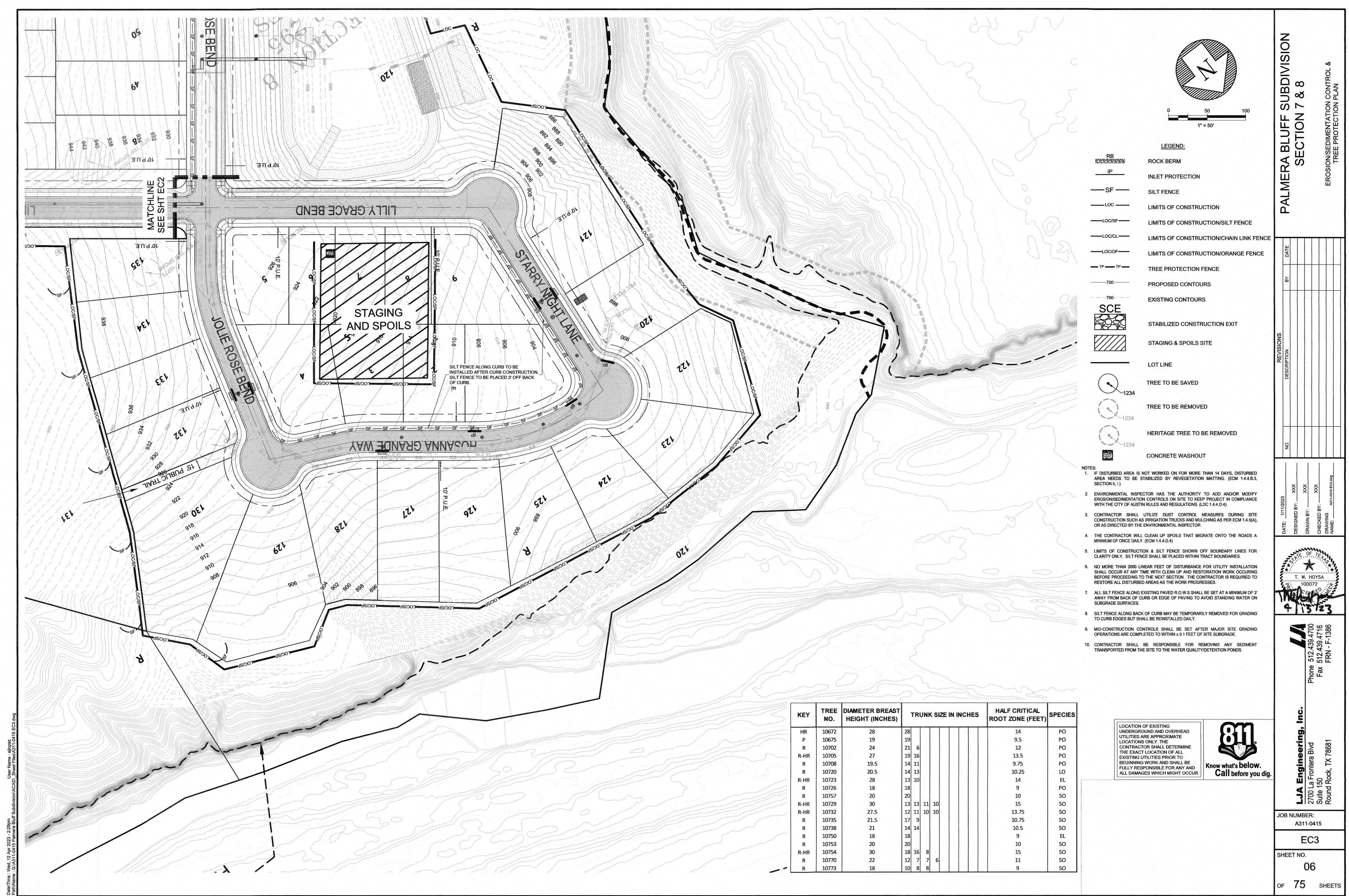
OF 75 SHEETS

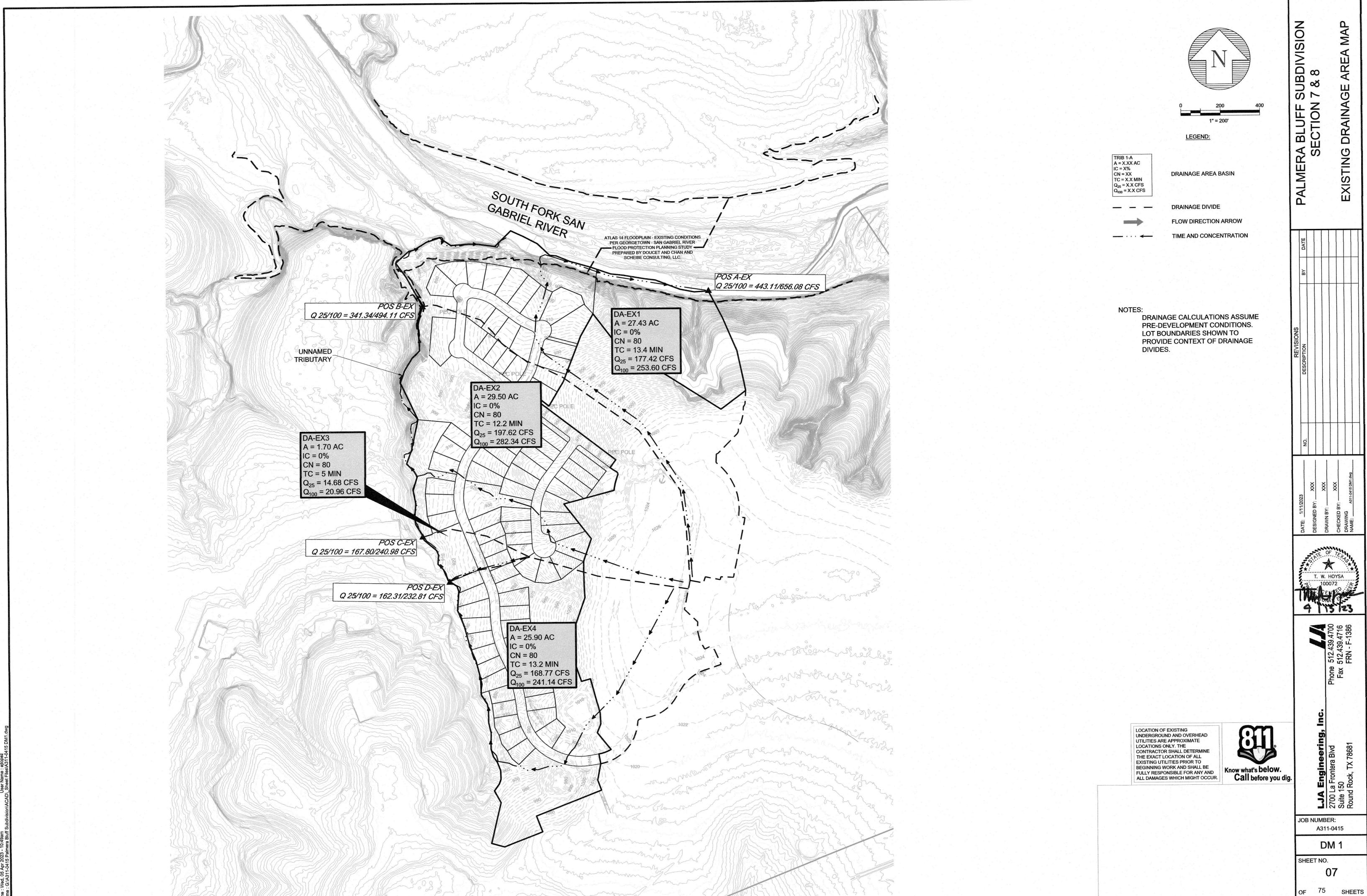


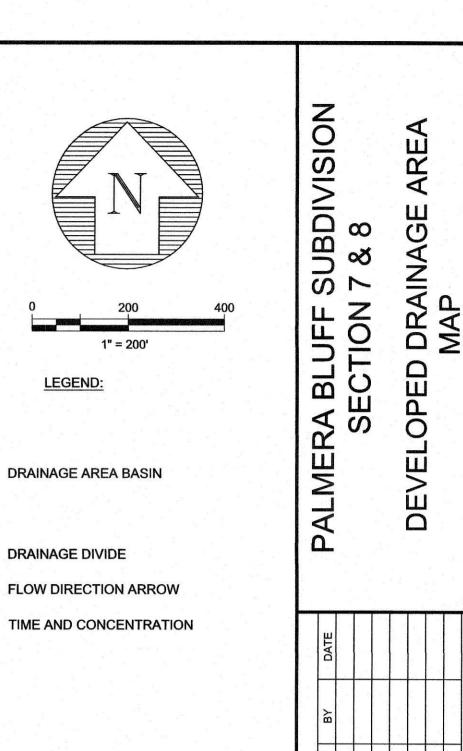
XXXX-XX-XX



XXXX-XX-XX







		Exis	ting		Proposed							
oint Of Study	2	10	25	100	2	10	25	100				
POS A	149.59	320.15	443.11	656.08	94.73	198.44	268.12	508.75				
POS B	124.22	251.49	341.34	494.11	80.83	141.45	183.59	473.62				
POS C	63.59	124.84	167.8	240.98	59.57	97.79	120.46	188.64				
POS D	61.94	120.82	162.31	232.81	19.64	35.93	48.1	69.58				

TRIB 1-A A = X.XX AC IC = X% CN = XX TC = X.X MIN Q_{25} = X.X CFS Q_{100} = X.X CFS

LOCATION OF EXISTING
UNDERGROUND AND OVERHEAD
UTILITIES ARE APPROXIMATE
LOCATIONS ONLY. THE
CONTRACTOR SHALL DETERMINE
THE EXACT LOCATION OF ALL
EXISTING UTILITIES PRIOR TO
BEGINNING WORK AND SHALL BE
FULLY RESPONSIBLE FOR ANY AND
ALL DAMAGES WHICH MIGHT OCCUR.

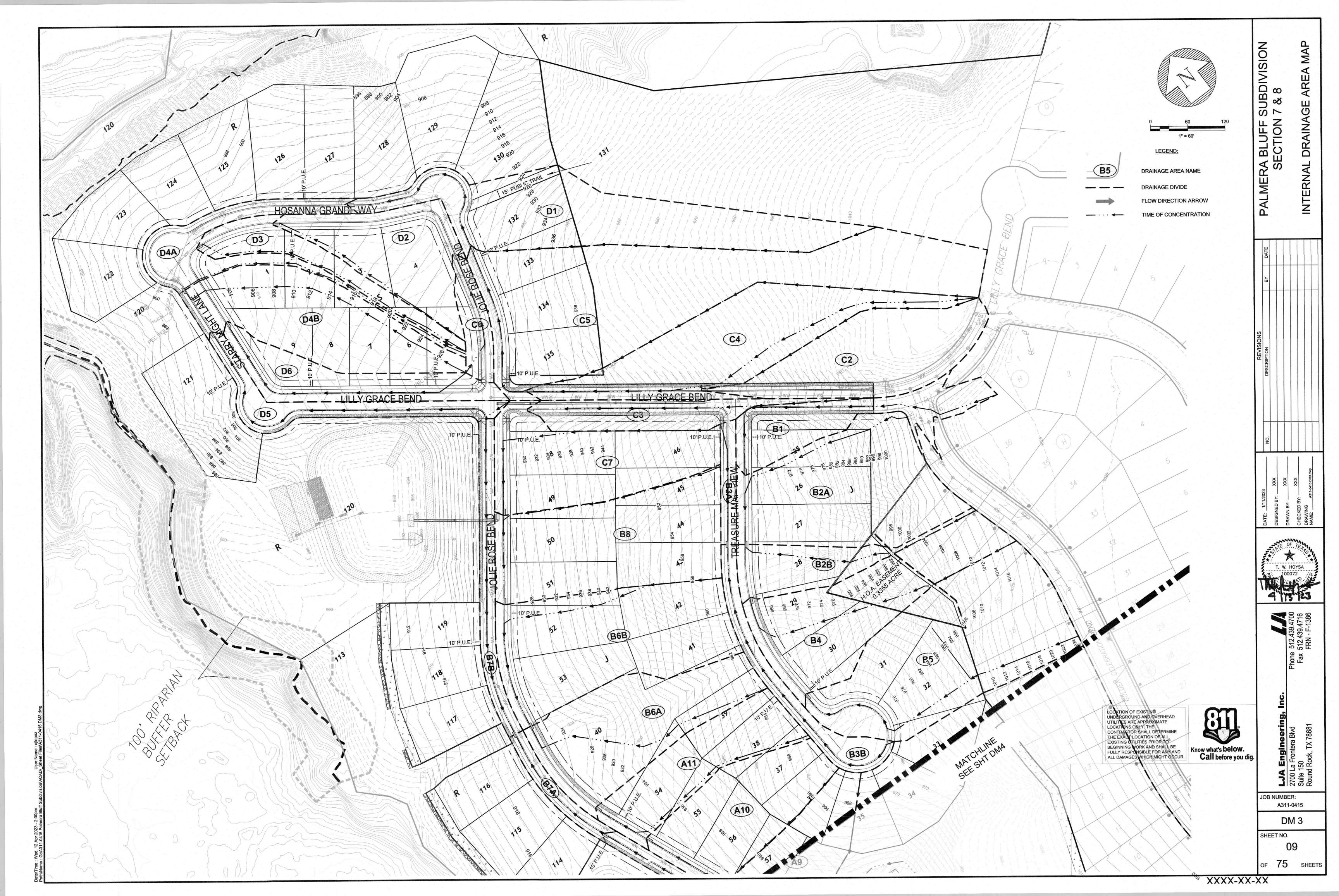


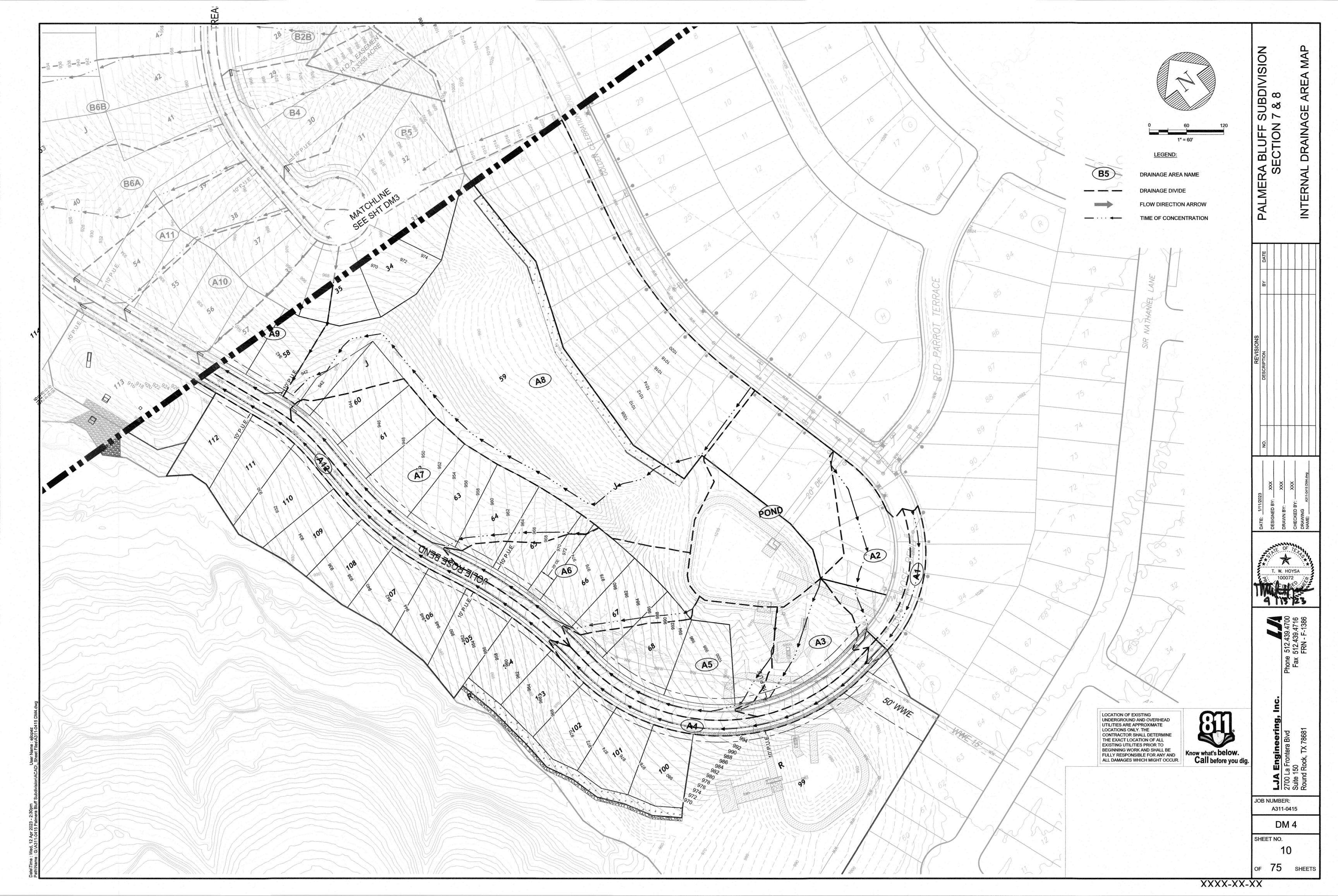
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JOB NUMBER: A311-0415 DM 2

SHEET NO.

OF 75 SHEETS





		IDI	COEFFICIE	NTS			DEVELOPED R	UNOFF COL	EFFICIENTS				
			25-YEAR	100-YEAR			And the second s	25-YEAR	100-YEAR				
		а	89	106.00			SF LOTS	0.61	0.70				
		b	10.16	9.46			ROW T	0.73	0.82				
		c	0.759	0.732			OPEN SPACE	0.29	0.36				
			LOT	ROW	OPEN								
ARI	EA	AREA	AREA	AREA	SPACE	Tc	C25	C100	125	1100	Q25	Q100	INLET
NO		(AC)	(AC)	(AC)	(AC)	(MIN)	COMP	COMP	(IN/HR)	(IN/HR)	(CFS)	(CFS)	TYPE
DA		0.15	0.00	0.15	0.00	0.7	0.73	0.82	14.53	19.37	1.6	2.4	ON-GRADE
DA		0.59	0.37	0.14	0.09	5.7	0.59	0.67	10.91	14.47	3.8	5.8	ON-GRADE
DA	A3	0.51	0.00	0.12	0.38	3.2	0.40	0.47	12.41	16.49	2.5	3.9	ON-GRADE
DA	A4	0.33	0.00	0.33	0.00	1.1	0.73	0.82	14.15	18.85	3.5	5.1	ON-GRADE
DA	A5	1.00	0.54	0.19	0.27	3.4	0.55	0.63	12.30	16.34	6.7	10.2	ON-GRADE
DA	A6	1.02	0.68	0.13	0.21	2.5	0.56	0.64	12.98	17.27	7.4	11.3	ON-GRADE
DA	A7	1.52	1.29	0.20	0.03	3.5	0.62	0.71	12.26	16.28	11.6	17.4	ON-GRADE
DA	A8	6.13	3.00	0.00	3.12	6.0	0.45	0.52	10.77	14.28	29.6	45.9	SUMP
DA	A9	0.66	0.56	0.10	0.00	3.4	0.63	0.71	12.31	16.35	5.1	7.7	ON-GRADE
DA A	A10	0.85	0.77	0.08	0.00	3.3	0.63	0.71	12.39	16.46	6.6	9.8	ON-GRADE
DA A	A11	0.68	0.59	0.05	0.04	3.2	0.61	0.69	12.43	16.51	5.1	7.7	ON-GRADE
DA A	A12	0.54	0.00	0.54	0.00	2.2	0.73	0.82	13.20	17.56	5.3	7.8	ON-GRADE
DA	B1	0.56	0.20	0.27	0.09	3.0	0.62	0.70	12.57	16.70	4.4	6.6	ON-GRADE
DA E	32A	1.28	0.94	0.07	0.27	2.7	0.55	0.63	12.79	17.01	9.0	13.7	SUMP
DA E	32B	0.38	0.30	0.05	0.03	3.2	0.61	0.69	12.46	16.56	2.9	4.3	SUMP
DA E	33A	0.11	0.00	0.11	0.00	2.6	0.73	0.82	12.88	17.13	1.1	1.6	SUMP
DA E	33B	0.33	0.00	0.33	0.00	2.6	0.73	0.82	12.89	17.14	3.2	4.7	SUMP
DA	B4	1.06	0.74	0.11	0.22	3.5	0.56	0.64	12.21	16.22	7.3	11.1	ON-GRADE
DA	B5	1.66	1.50	0.16	0.00	3.6	0.63	0.71	12.16	16.15	12.6	18.9	ON-GRADE
DA E	36A	1.04	0.15	0.10	0.80	3.4	0.38	0.45	12.33	16.38	4.9	7.7	SUMP
DA E	36B	1.35	1.21	0.12	0.02	3.9	0.62	0.70	11.95	15.87	10.0	15.1	SUMP
DA E	37A	0.13	0.00	0.13	0.00	0.8	0.73	0.82	14.51	19.35	1.4	2.0	SUMP
DA E	37B	0.30	0.00	0.30	0.00	2.4	0.73	0.82	13.02	17.31	2.8	4.2	SUMP
DA	B8	1.27	1.19	0.08	0.00	4.2	0.62	0.70	11.80	15.67	9.3	14.0	ON-GRADE
DA	C2	0.91	0.00	0.27	0.64	7.3	0.42	0.50	10.17	13.48	3.9	6.1	ON-GRADE
DA	C3	0.39	0.00	0.23	0.16	0.8	0.56	0.63	14.51	19.35	3.2	4.8	ON-GRADE
DA	C4	1.19	0.00	0.18	1.01	7.4	0.36	0.43	10.12	13.41	4.3	6.9	ON-GRADE
DA	C5	2.33	0.71	0.16	1.46	8.2	0.42	0.49	9.79	12.98	9.6	14.9	ON-GRADE
DA	C6	0.19	0.07	0.12	0.00	4.0	0.69	0.78	11.92	15.82	1.5	2.3	ON-GRADE
DA	C7	0.96	0.80	0.12	0.03	3.6	0.62	0.70	12.18	16.18	7.2	10.8	ON-GRADE
DA	D1	1.37	0.39	0.21	0.77	4.3	0.45	0.53	11.74	15.59	7.3	11.3	ON-GRADE
DA	D2	0.72	0.52	0.20	0.00	4.3	0.65	0.73	11.72	15.56	5.5	8.2	ON-GRADE
DA	D3	0.36	0.28	0.08	0.00	4.7	0.64	0.72	11.49	15.24	2.7	4.0	ON-GRADE
DA I	D4A	0.48	0.14	0.34	0.00	4.8	0.70	0.78	11.44	15.18	3.8	5.7	SUMP
DA I	D4B	1.03	0.93	0.10	0.00	5.1	0.63	0.71	11.27	14.95	7.3	10.9	SUMP
DA	D5	0.37	0.00	0.37	0.00	1.5	0.73	0.82	13.84	18.43	3.7	5.6	ON-GRADE
DΔ	De	0.54	0.24	0.29	0.00	3.5	0.68	0.76	12 25	16 28	4.5	6.7	ON-GRADE

STREET GUTTER DEPTH CALCULATIONS FOR 25YR STORM

AREA#	FLOW (CFS)	QBYPASS (CFS)	QTOTAL (CFS)	STREET CAP (CFS)	s (%)	Y (FT)	QX-OVER (CFS)	Q NET (CFS)	FLOW TO	STREET WIDTH (FT)	SPREAD (FT)	ALLOWABLE SPREAD (FT)	INLET
DA A1	1.6	0.0	1.6	18.8	4.70	0.22	0.0	1.6	NA	28	3.7	14	ON-GRAD
DA A2	3.8	0.0	3.8	18.8	4.70	0.30	0.0	3.8	NA	28	5.3	14	ON-GRAD
DA A3	2.5	0.0	2.5	23.8	7.55	0.24	0.0	2.5	NA	28	4.0	14	ON-GRAD
DA A4	3.5	0.0	3.5	23.8	7.55	0.26	0.0	3.5	NA	28	4.6	14	ON-GRAD
DA A5	6.7	0.0	6.7	23.8	7.55	0.33	0.0	6.7	NA	28	6.1	14	ON-GRAD
DA A6	7.4	0.0	7.4	23.8	7.55	0.34	0.0	7.4	NA	28	6.4	14	ON-GRAD
DA A7	11.6	0.0	11.6	15.2	3.07	0.46	0.0	11.6	NA	28	10.8	14	ON-GRAD
DA A8	29.6	0.0	29.6	NA	NA	NA	NA .	NA	NA	NA	NA	NA	SUMP
DA A9	5.1	0.0	5.1	15.2	3.07	0.35	0.0	5.1	NA	28	6.6	14	ON-GRAD
DA A10	6.6	0.0	6.6	15.2	3.07	0.38	0.0	6.6	NA	28	7.5	14	ON-GRAD
DA A11	5.1	0.0	5.1	15.2	3.07	0.35	0.0	5.1	NA	28	6.6	14	ON-GRAD
DA A12	5.3	0.0	5.3	15.2	3.07	0.35	0.0	5.3	NA	28	6.7	14	ON-GRAD
DA B1	4.4	0.0	4.4	19.4	5.00	0.31	0.0	4.4	NA	28	5.5	14	ON-GRAD
DA B2A	9.0	0.0	9.0	12.9	2.20	0.44	0.0	9.0	NA	28	10.1	14	SUMP
DA B2B	2.9	0.0	2.9	6.8	0.61	0.38	0.0	2.9	NA	28	7.4	14	SUMP
DA B3A	1.1	0.0	1.1	12.9	2.20	0.22	0.0	1.1	NA	28	3.7	14	SUMP
DA B3B	3.2	0.5	4.2	13.1	2.30	0.34	0.0	3.7	NA	28	6.5	14	SUMP
DA B4	7.3	0.0	7.3	6.8	0.61	0.51	0.5	6.8	B3B	28	>14	14	ON-GRAD
DA B5	12.6	0.0	12.6	17.3	4.00	0.45	0.0	12.6	NA	28	10.3	14	ON-GRAD
DA B6A	4.9	0.0	4.9	12.6	2.10	0.37	0.0	4.9	NA	28	7.1	14	SUMP
DA B6B	10.0	0.0	10.0	8.9	1.06	0.52	1.1	8.9	B7B	28	>14	14	SUMP
DA B7A	1.4	0.0	1.4	12.4	2.04	0.24	0.0	1.4	NA	28	4.1	14	SUMP
DA B7B	2.8	1.1	3.9	10.0	1.32	0.37	0.0	3.9	NA	28	7.2	14	SUMP
DA B8	9.3	0.0	9.3	10.0	1.32	0.49	0.0	9.3	NA	28	14.0	14	ON-GRAD
DA C2	3.9	0.0	3.9	33.6	15.00	0.25	0.0	3.9	NA	28	4.2	14	ON-GRAD
DA C3	3.2	0.0	3.2	33.6	15.00	0.23	0.0	3.2	NA	28	3.9	14	ON-GRAD
DA C4	4.3	0.0	4.3	33.6	15.00	0.25	0.0	4.3	NA	28	4.4	14	ON-GRAD
DA C5	9.6	0.0	9.6	12.3	2.00	0.46	0.0	9.6	NA	28	11.0	14	ON-GRAD
DA C6	1.5	0.0	1.5	12.3	2.00	0.25	0.0	1.5	NA	28	4.3	14	ON-GRAD
DA C7	7.2	0.0	7.2	10.0	1.34	0.45	0.0	7.2	NA	28	10.2	14	ON-GRAD
DA D1	7.3	0.2	7.5	17.7	4.19	0.38	0.0	7.5	NA	28	7.4	14	ON-GRAD
DA D2	5.5	0.0	5.5	17.7	4.19	0.34	0.0	5.5	NA	28	6.4	14	ON-GRAD
DA D3	2.7	0.0	2.7	10.5	1.46	0.32	0.0	2.7	NA	28	5.8	14	ON-GRAD
DA D4A	3.8	0.0	3.8	13.4	2.40	0.33	0.0	3.8	NA	28	6.1	14	SUMP
DA D4B	7.3	0.2	7.4	10.6	1.50	0.44	0.0	7.4	NA	28	10.0	14	SUMP
DA D5	3.7	0.0	3.7	21.2	6.00	0.28	0.0	3.7	NA	28	5.0	14	ON-GRAD
DA D6	4.5	0.0	4.5	21.2	6.00	0.30	0.0	4.5	NA	28	5.4	14	ON-GRAD

INLET CALCULATIONS FOR 25YR STORM (ALL INLETS ARE TYPE 1 ON GRADE)

NLET#	AREA#	ST WIDTH (FT)	QTOTAL (CFS)	STREET CAP (CFS)	S (%)	(FT)	QA/LA (CFS/FT)	(FT)	(FT)	L/LA	A/Y	Q/QA	QIN (CFS)	QBYPASS (CFS)	FLOW TO
A1	DA A1	28	1.6	18.8	4.70	0.22	0.7	2.39	10	4.19	1.88	1.0000	1.6	0.0	NA
A2	DA A2	28	3.8	18.8	4.70	0.30	0.7	5.13	10	1.95	1.41	1.0000	3.8	0.0	NA
A3	DA A3	28	2.5	23.8	7.55	0.24	0.7	3.63	10	2.75	1.75	1.0000	2.5	0.0	NA
A4	DA A4	28	3.5	23.8	7.55	0.26	0.7	4.82	10	2.07	1.58	1.0000	3.5	0.0	NA
A5	DA A5	28	6.7	23.8	7.55	0.33	0.8	8.60	10	1.16	1.26	1.0000	6.7	0.0	NA
A6	DA A6	28	7.4	23.8	7.55	0.34	0.8	9.36	10	1.07	1.22	1.0000	7.4	0.0	NA
A7	DA A7	28	11.6	15.2	3.07	0.46	0.9	12.60	20	1.59	0.91	1.0000	11.6	0.0	NA
A9	DA A9	28	5.1	15.2	3.07	0.35	0.8	6.40	10	1.56	1.19	1.0000	5.1	0.0	NA
A10	DA A10	28	6.6	15.2	3.07	0.38	0.8	7.85	10	1.27	1.10	1.0000	6.6	0.0	NA
A11	DA A11	28	5.1	15.2	3.07	0.35	0.8	6.37	10	1.57	1.19	1.0000	5.1	0.0	NA
A12	DA A12	28	5.3	15.2	3.07	0.35	0.8	6.50	10	1.54	1.18	1.0000	5.3	0.0	NA
B1	DA B1	28	4.4	19.4	5.00	0.31	0.8	5.75	. 10	1.74	1.36	1.0000	4.4	0.0	NA
B4	DA B4	28	7.3	6.8	0.61	0.51	1.0	7.41	10	1.35	0.81	1.0000	7.3	0.0	NA
B5	DA B5	28	12.6	17.3	4.00	0.45	0.9	13.79	20	1.45	0.93	1.0000	12.6	0.0	NA
B8	DA B8	28	9.3	10.0	1.32	0.49	1.0	9.76	20	2.05	0.85	1.0000	9.3	0.0	NA
C2	DA C2	28	3.9	33.6	15.00	0.25	0.7	5.62	10	1.78	1.69	1.0000	3.9	0.0	NA
C3	DA C3	28	3.2	33.6	15.00	0.23	0.7	4.64	10	2.15	1.82	1.0000	3.2	0.0	NA
C4	DA C4	28	4.3	33.6	15.00	0.25	0.7	6.10	10	1.64	1.64	1.0000	4.3	0.0	NA
C5	DA C5	28	9.6	12.3	2.00	0.46	0.9	10.33	10	0.97	0.90	0.9835	9.4	0.2	D4B
C6	DA C6	28	1.5	12.3	2.00	0.25	0.7	2.17	10	4.60	1.66	1.0000	1.5	0.0	NA
C7	DA C7	28	7.2	10.0	1.34	0.45	0.9	7.90	10	1.27	0.93	1.0000	7.2	0.0	NA
D1	DA D1	28	7.5	17.7	4.19	0.38	8.0	8.99	10	1.11	1.11	1.0000	7.5	0.0	NA
D2	DA D2	28	5.5	17.7	4.19	0.34	0.8	6.90	10	1.45	1.23	1.0000	5.5	0.0	NA
D3	DA D3	28	2.7	10.5	1.46	0.32	8.0	3.44	10	2.91	1.31	1.0000	2.7	0.0	NA
D5	DA D5	28	3.7	21.2	6.00	0.28	0.7	5.08	10	1.97	1.48	1.0000	3.7	0.0	NA
D6	DA D6	28	4.5	21.2	6.00	0.30	0.8	5.97	10	1.67	1.39	1.0000	4.5	0.0	NA

INLET CALCULATIONS FOR 25YR STORM (ALL INLETS ARE TYPE 1 IN SUMP WITH 10% REDUCTION)

INLET#	AREA #	QTOTAL	L	L x 0.90	Q/L	Yo + 5"	INLET Yo	BELOW	
		(CFS)	(FT)	(FT)	(CFS/FT)	(FT)	(FT)	0.5 FT	
A8	DA A8	29.6	10	9	3.29	1.06	0.64	No	- Area inlet -outside ROW
B2	DA B2A & B2B	11.9	10	9	1.32	0.58	0.16	Yes	T
B3	DA B3A & B3B	5.3	10	9	0.59	0.34	-0.08	Yes	
B6	DA B6A & B6B	14.9	10	9	1.65	0.67	0.25	Yes	
B7	DA B7A & B7B	5.3	10	9	0.59	0.34	-0.08	Yes	
D4	DA D4A & D4B	11.3	10	9	1.25	0.56	0.14	Yes	

STREET GUTTER DEPTH CALCULATIONS FOR 100YR STORM

AREA # FLOW QBYPASS QTOTAL STREET S Y QX-OVER Q NET FLOW TO STREET SPREAD ALLOWABLE INLET

AKEA #	(CFS)	(CFS)	(CFS)	CAP (CFS)	(%)	(FT)	(CFS)	(CFS)	FLOW 10	WIDTH (FT)	(FT)	SPREAD (FT)	
DA A1	2.4	0.0	2.4	18.8	4.70	0.25	0.0	2.4	I NA	28	4.3	14	ON-GRADE
DA A2	5.8	0.0	5.8	18.8	4.70	0.34	0.0	5.8	NA	28	6.3	14	ON-GRADE
DA A3	3.9	0.0	3.9	23.8	7.55	0.28	0.0	3.9	NA	28	4.8	14	ON-GRADE
DA A4	5.1	0.0	5.1	23.8	7.55	0.30	0.0	5.1	NA	28	5.4	14	ON-GRADE
DA A5	10.2	0.0	10.2	23.8	7.55	0.38	0.0	10.2	NA	28	7.5	14	ON-GRADE
DA A6	11.3	1.2	12.5	23.8	7.55	0.40	0.0	12.5	NA	28	8.3	14	ON-GRADE
DA A7	17.4	2.6	20.0	15.2	3.07	0.55	4.9	15.2	A12	28	>14	14	ON-GRADE
DA A8	45.9	0.0	45.9	NA	NA	NA	NA	NA	NA	NA	N/A	NA	SUMP
DA A9	7.7	0.0	7.7	15.2	3.07	0.40	0.0	7.7	NA	28	8.2	14	ON-GRADE
DA A10	9.8	0.0	9.8	15.2	3.07	0.43	0.0	9.8	NA	28	9.5	14	ON-GRADE
DA A11	7.7	0.5	8.2	15.2	3.07	0.41	0.0	8.2	NA	28	8.5	14	ON-GRADE
DA A12	7.8	4.9	12.7	15.2	3.07	0.47	0.0	12.7	NA	28	11.7	14	ON-GRADE
DA B1	6.6	0.0	6.6	19.4	5.00	0.35	0.0	6.6	NA	28	6.6	14	ON-GRADE
DA B2A	13.7	0.0	13.7	12.9	2.20	0.51	0.9	12.9	B3A	28	>14	14	SUMP
DA B2B	4.3	0.2	4.5	6.8	0.61	0.44	0.0	4.5	NA	28	9.6	14	SUMP
DA B3A	1.6	0.9	2.5	12.9	2.20	0.29	0.0	2.5	NA	28	5.1	14	SUMP
DA B3B	4.7	5.9	10.5	13.1	2.30	0.46	0.0	10.5	NA	28	11.3	14	SUMP
DA B4	11.1	0.0	11.1	6.8	0.61	0.59	4.3	6.8	B3B	28	>14	14	ON-GRADE
DA B5	18.9	0.0	18.9	17.3	4.00	0.51	1.6	17.3	B3B	28	>14	14	ON-GRADE
DA B6A	7.7	0.0	7.7	12.6	2.10	0.43	0.0	7.7	NA	28	9.2	14	SUMP
DA B6B	15.1	0.0	15.1	8.9	1.06	0.59	6.1	8.9	B7B	28	>14	14	SUMP
DA B7A	2.0	7.8	9.8	12.4	2.04	0.46	0.0	9.8	NA	28	11.1	14	SUMP
DA B7B	4.2	11.5	15.7	10.0	1.32	0.58	5.7	10.0	B7A	28	14.0	14	SUMP
DA B8	14.0	0.5	14.5	10.0	1.32	0.57	4.6	10.0	B7B	28	>14	14	ON-GRADE
DA C2	6.1	0.0	6.1	33.6	15.00	0.29	0.0	6.1	NA	28	5.0	14	ON-GRADE
DA C3	4.8	0.0	4.8	33.6	15.00	0.26	0.0	4.8	NA	28	4.6	14	ON-GRADE
DA C4	6.9	0.0	6.9	33.6	15.00	0.30	0.0	6.9	NA	28	5.3	14	ON-GRADE
DA C5	14.9	0.0	14.9	12.3	2.00	0.53	2.7	12.3	C6	28	>14	14	ON-GRADE
DA C6	2.3	2.7	4.9	12.3	2.00	0.37	0.0	4.9	NA NA	28	7.2	14	ON-GRADE
DA C7	10.8	0.0	10.8	10.0	1.34	0.51	0.8	10.0	B7B	28	>14	14	ON-GRADE
DA D1	11.3	3.0	14.3	17.7	4.19	0.47	0.0	14.3	NA NA	28	11.3	14	ON-GRADE
DA D2	8.2	0.0	8.2	17.7	4.19	0.39	0.0	8.2	NA	28	7.8	14	ON-GRADE
DA D3	4.0	0.0	4.0	10.5	1.46	0.36	0.0	4.0	NA	28	7.0	14	ON-GRADE
DA D4A	5.7	3.6	9.3	13.4	2.40	0.44	0.0	9.3	NA	28	10.0	14	SUMP
DA D4B	10.9	0.0	10.9	10.6	1.50	0.50	0.3	10.6	D4A	. 28	>14	14	SUMP
DA D5	5.6	0.0	5.6	21.2	6.00	0.32	0.0	5.6	NA NA	28	5.9	14	ON-GRADE
DA D6	6.7	0.0	6.7	21.2	6.00	0.34	0.0	6.7	NA	28	6.4	14	ON-GRADE

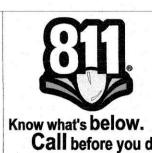
INLET CALCULATIONS FOR 100YR STORM (ALL INLETS ARE TYPE 1 ON GRADE)

INLET#	AREA#	ST WIDTH (FT)	QTOTAL (CFS)	STREET CAP (CFS)	S (%)	Y (FT)	QA/LA (CFS/FT)	LA (FT)	L (FT)	L/LA	A/Y	Q/QA	QIN (CFS)	QBYPASS (CFS)	FLOW TO
A1	DA A1	28	2.4	18.8	4.70	0.25	0.7	3.40	10	2.95	1.64	1.0000	2.4	0.0	NA
A2	DA A2	28	5.8	18.8	4.70	0.34	0.8	7.29	10	1.37	1.23	1.0000	5.8	0.0	NA
A3	DA A3	28	3.9	23.8	7.55	0.28	0.7	5.42	10	1.85	1.51	1.0000	3.9	0.0	NA
A4	DA A4	28	5.1	23.8	7.55	0.30	0.8	6.81	10	1.47	1.38	1.0000	5.1	0.0	NA
A5	DA A5	28	10.2	23.8	7.55	0.38	0.8	12.26	10	0.82	1.10	0.8826	9.0	1.2	A6
A6	DA A6	28	12.5	23.8	7.55	0.40	0.9	14.48	10	0.69	1.03	0.7914	9.9	2.6	A7
A7	DA A7	28	20.0	15.2	3.07	0.55	1.0	19.57	20	1.02	0.76	1.0000	20.0	0.0	NA
A9	DA A9	28	7.7	15.2	3.07	0.40	0.9	9.00	10	1.11	1.04	1.0000	7.7	0.0	NA
A10	DA A10	28	9.8	15.2	3.07	0.43	0.9	11.00	10	0.91	0.96	0.9487	9.3	0.5	A11
A11	DA A11	28	8.2	15.2	3.07	0.41	0.9	9.47	10	1.06	1.02	1.0000	8.2	0.0	NA
A12	DA A12	28	12.7	15.2	3.07	0.47	0.9	13.51	10	0.74	0.88	0.8391	10.6	2.0	B7A
B1	DA B1	28	6.6	19.4	5.00	0.35	0.8	8.15	10	1.23	1.19	1.0000	6.6	0.0	NA
B4	DA B4	28	11.1	6.8	0.61	0.59	1.1	10.32	10	0.97	0.71	0.9859	10.9	0.2	B2B
B5	DA B5	28	18.9	17.3	4.00	0.51	1.0	19.18	20	1.04	0.81	1.0000	18.9	0.0	NA .
B8	DA B8	28	14.5	10.0	1.32	0.57	1.0	13.90	20	1.44	0.74	1.0000	14.5	0.0	NA
C2	DA C2	28	6.1	33.6	15.00	0.29	0.7	8.30	10	1.20	1.46	1.0000	6.1	0.0	NA
C3	DA C3	28	4.8	33.6	15.00	0.26	0.7	6.74	10	1.48	1.58	1.0000	4.8	0.0	NA
C4	DA C4	28	6.9	33.6	15.00	0.30	0.7	9.16	10	1.09	1.41	1.0000	6.9	0.0	NA
C5	DA C5	28	14.9	12.3	2.00	0.53	1.0	14.80	10	0.68	0.78	0.7982	11.9	3.0	D1
C6	DA C6	28	4.9	12.3	2.00	0.37	0.8	5.97	10	1.68	1.13	1.0000	4.9	0.0	NA
C7	DA C7	28	10.8	10.0	1.34	0.51	1.0	11.00	10	0.91	0.81	0.9528	10.3	0.5	B8
D1	DA D1	28	14.3	17.7	4.19	0.47	0.9	15.36	10	0.65	0.89	0.7687	11.0	3.3	D4A
D2	DA D2	28	8.2	17.7	4.19	0.39	0.8	9.70	10	1.03	1.08	1.0000	8.2	0.0	NA
D3	DA D3	28	4.0	10.5	1.46	0.36	0.8	4.85	10	2.06	1.15	1.0000	4.0	0.0	NA
D5	DA D5	28	5.6	21.2	6.00	0.32	0.8	7.16	10	1.40	1.30	1.0000	5.6	0.0	NA
D6	DA D6	28	6.7	21.2	6.00	0.34	0.8	8.42	10	1.19	1.22	1.0000	6.7	0.0	NA

INLET CALCULATIONS FOR 100YR STORM (ALL INLETS ARE TYPE 1 IN SUMP WITH 10% REDUCTION)

	INLET#	AREA#	QTOTAL	L	L x 0.90	Q/L	Yo + 5"	INLET Yo	BELOW	
٠,			(CFS)	(FT)	(FT)	(CFS/FT)	(FT)	(FT)	0.5 FT	
	A8	DA A8	45.9	10	9	5.10	1.42	1.00	No	-Area inlet-outside ROW
	B2	DA B2A & B2B	18.2	10	9	2.02	0.77	0.35	Yes	
	B3	DA B3A & B3B	13.0	10	9	1.44	0.61	0.19	Yes	
	B6	DA B6A & B6B	22.8	10	9	2.53	0.89	0.47	Yes	
ć	B7	DA B7A & B7B	25.5	10	9	2.83	0.96	0.54	No	
	D4	DA D4A & D4B	20.2	10	9	2.25	0.82	0.40	Yes	

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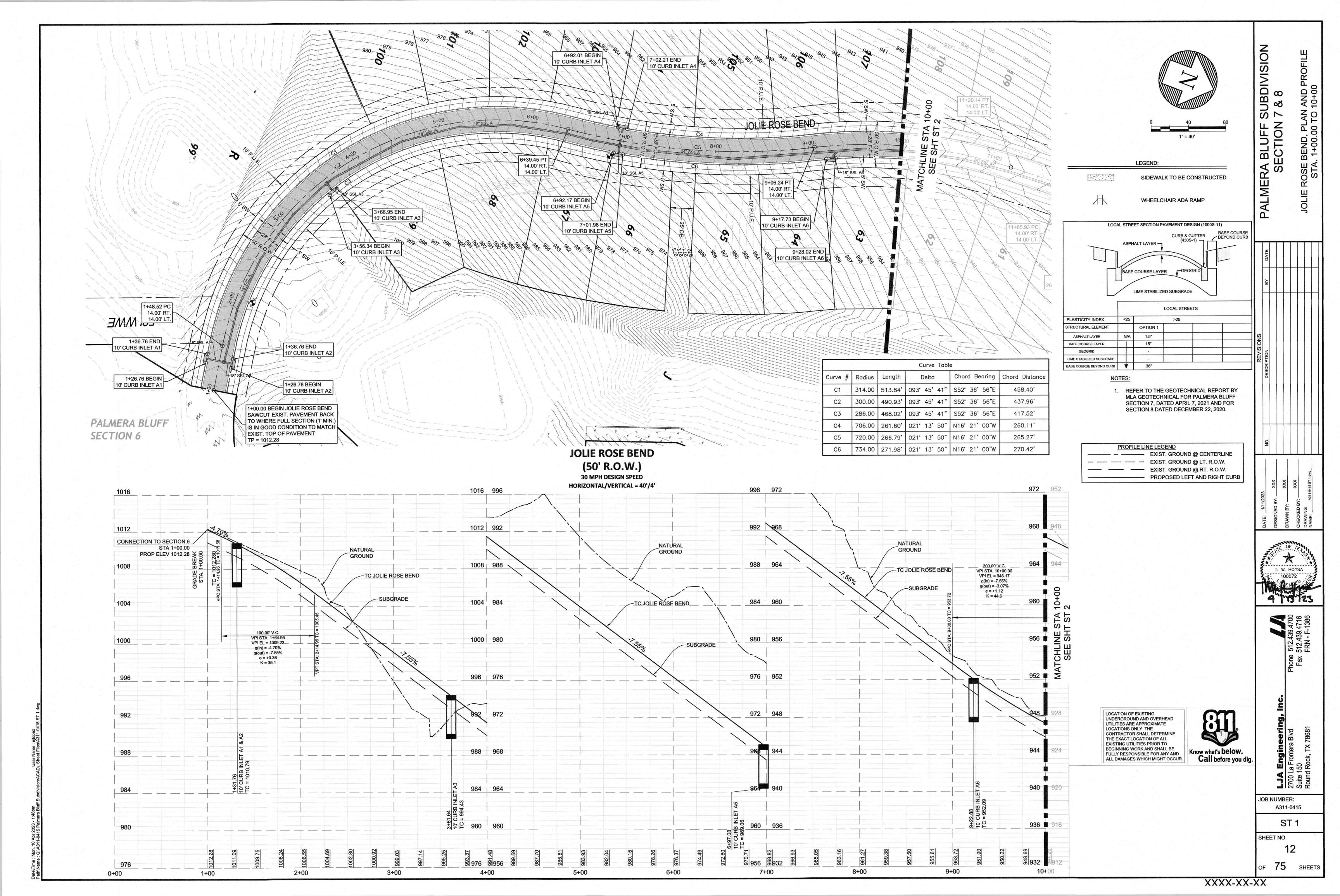


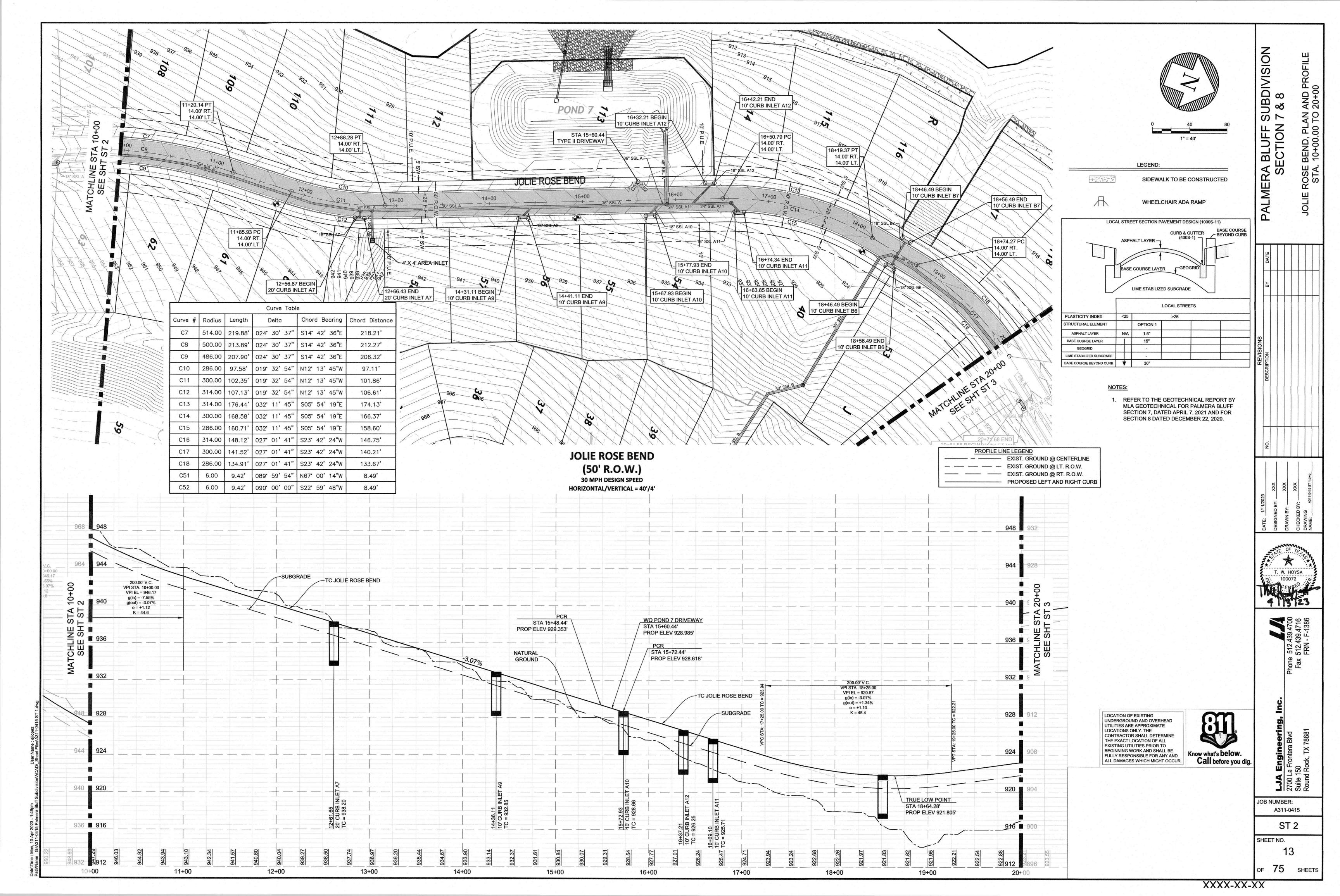
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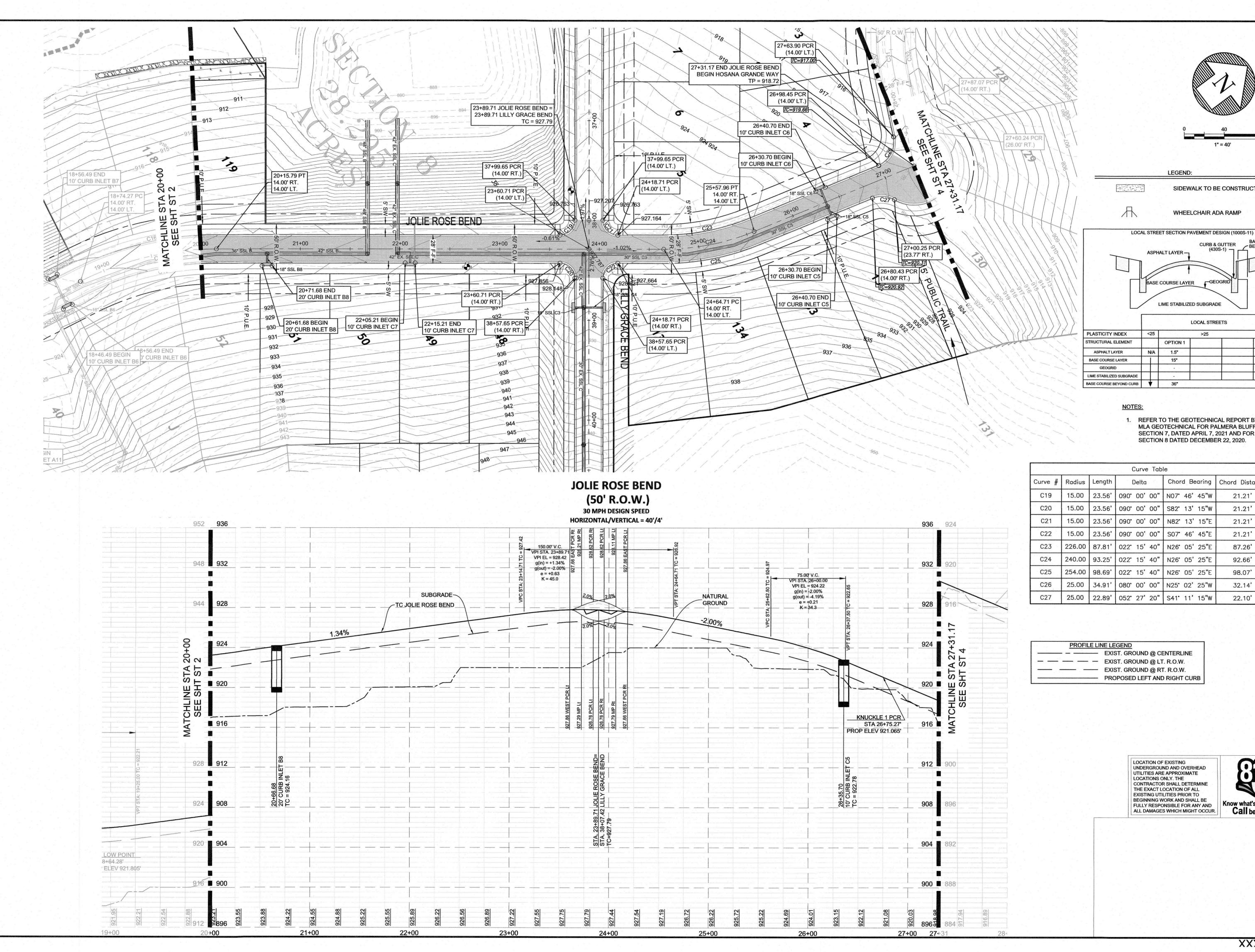
PALMERA BLUFF SUBDIVISION SECTION 7 & 8

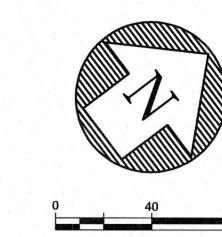
SHEET NO.

OF 75 SHEETS









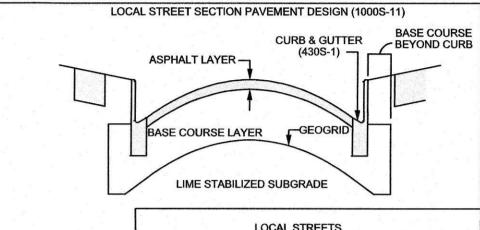
SUBDIVISION 17 & 8

BEND, 20+00

LEGEND:

SIDEWALK TO BE CONSTRUCTED

WHEELCHAIR ADA RAMP



			LOCAL STREET	S	F 1
PLASTICITY INDEX	<25		>25		8
STRUCTURAL ELEMENT		OPTION 1			P 2
ASPHALT LAYER	N/A	1.5"			
BASE COURSE LAYER		15"			
GEOGRID			4 12 TO 12 TO 12		
LIME STABILIZED SUBGRADE					584
BASE COURSE BEYOND CURB	*	36"			

 REFER TO THE GEOTECHNICAL REPORT BY MLA GEOTECHNICAL FOR PALMERA BLUFF SECTION 7, DATED APRIL 7, 2021 AND FOR SECTION 8 DATED DECEMBER 22, 2020.

	1	177			
			Curve Tab	le	
Curve #	Radius	Length	Delta	Chord Bearing	Chord Distance
C19	15.00	23.56'	090, 00, 00,	N07° 46' 45"W	21.21'
C20	15.00	23.56'	090, 00, 00,	S82° 13' 15"W	21.21'
C21	15.00	23.56'	090° 00' 00"	N82° 13' 15"E	21.21'
C22	15.00	23.56'	090' 00' 00"	S07° 46' 45"E	21.21'
C23	226.00	87.81	022 15 40"	N26° 05' 25"E	87.26'
C24	240.00	93.25	022' 15' 40"	N26° 05' 25"E	92.66'
C25	254.00	98.69'	022 15 40"	N26° 05' 25"E	98.07'
C26	25.00	34.91'	080° 00' 00"	N25° 02' 25"W	32.14'
C27	25.00	22.80'	052° 27' 20"	C/1° 11' 15"W	22.10'

— — EXIST. GROUND @ CENTERLINE

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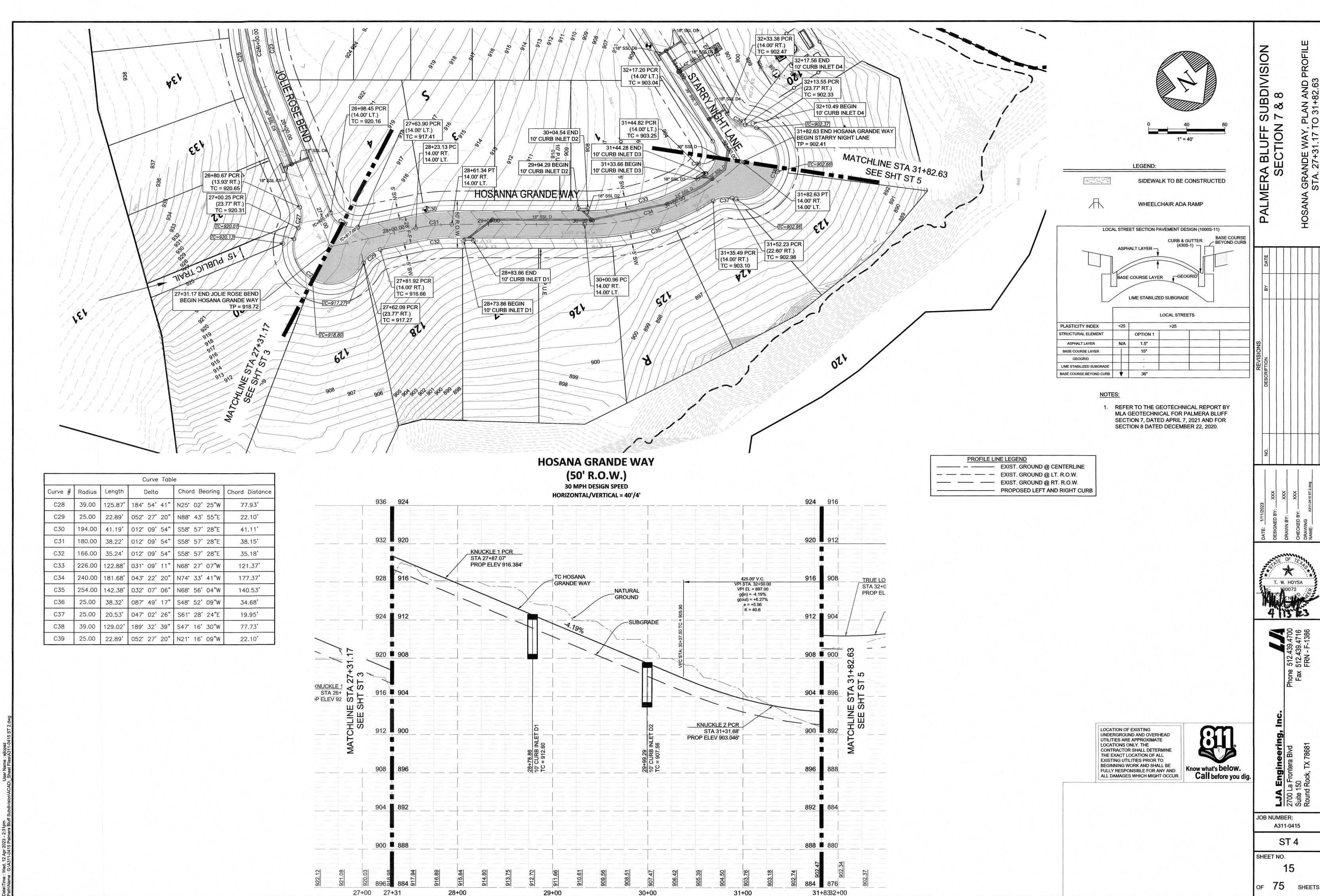
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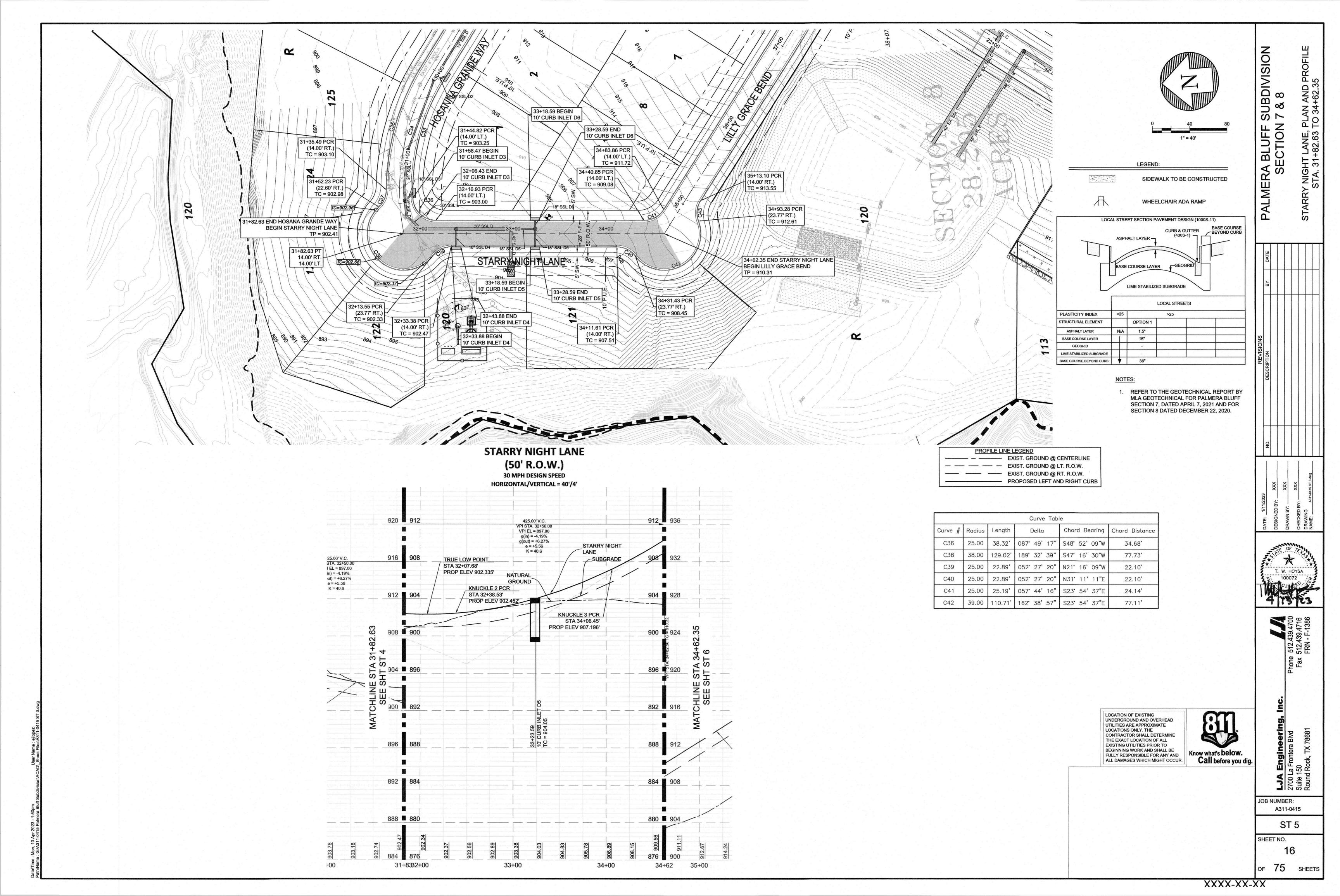
JOB NUMBER: A311-0415

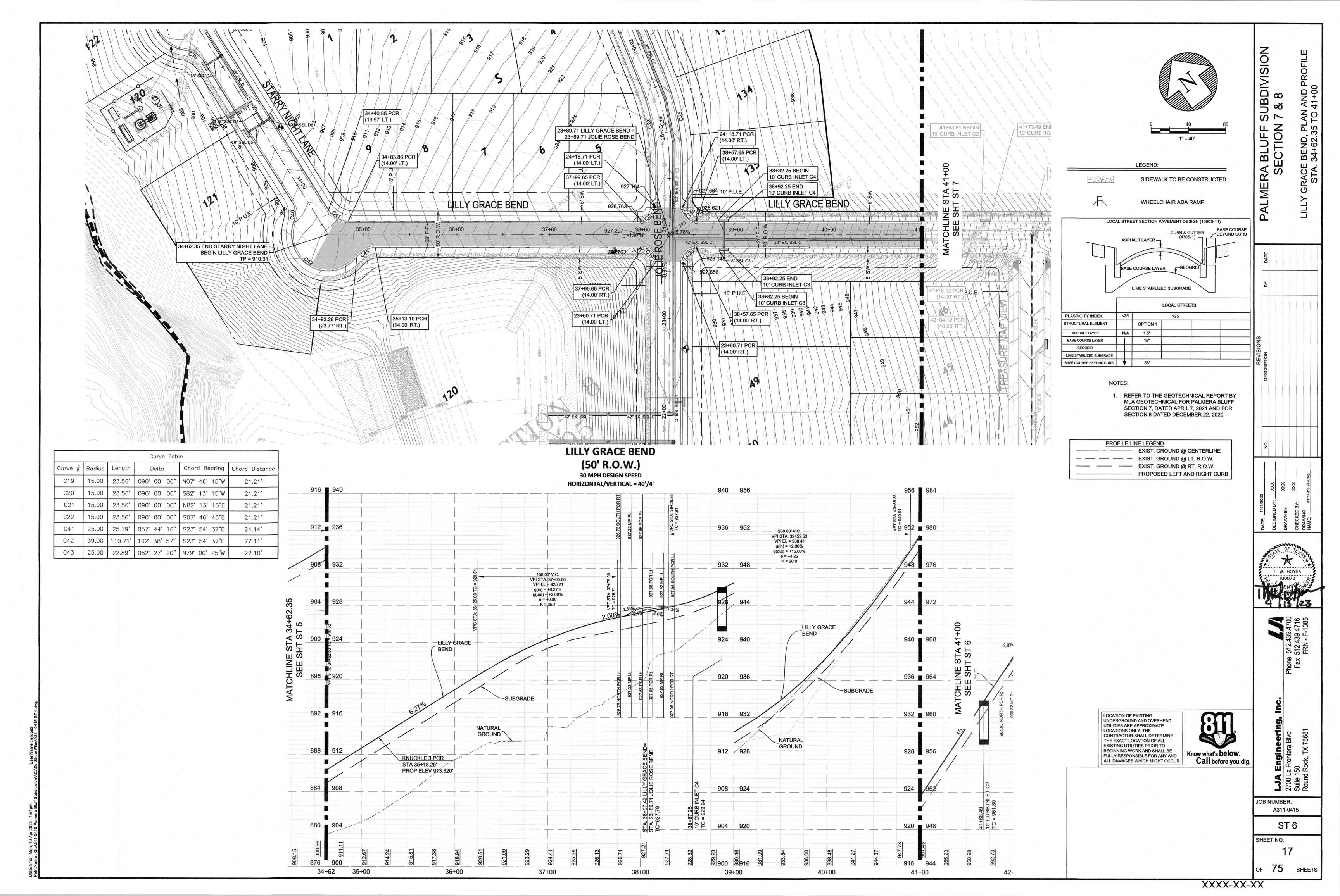
T. W. HOYSA

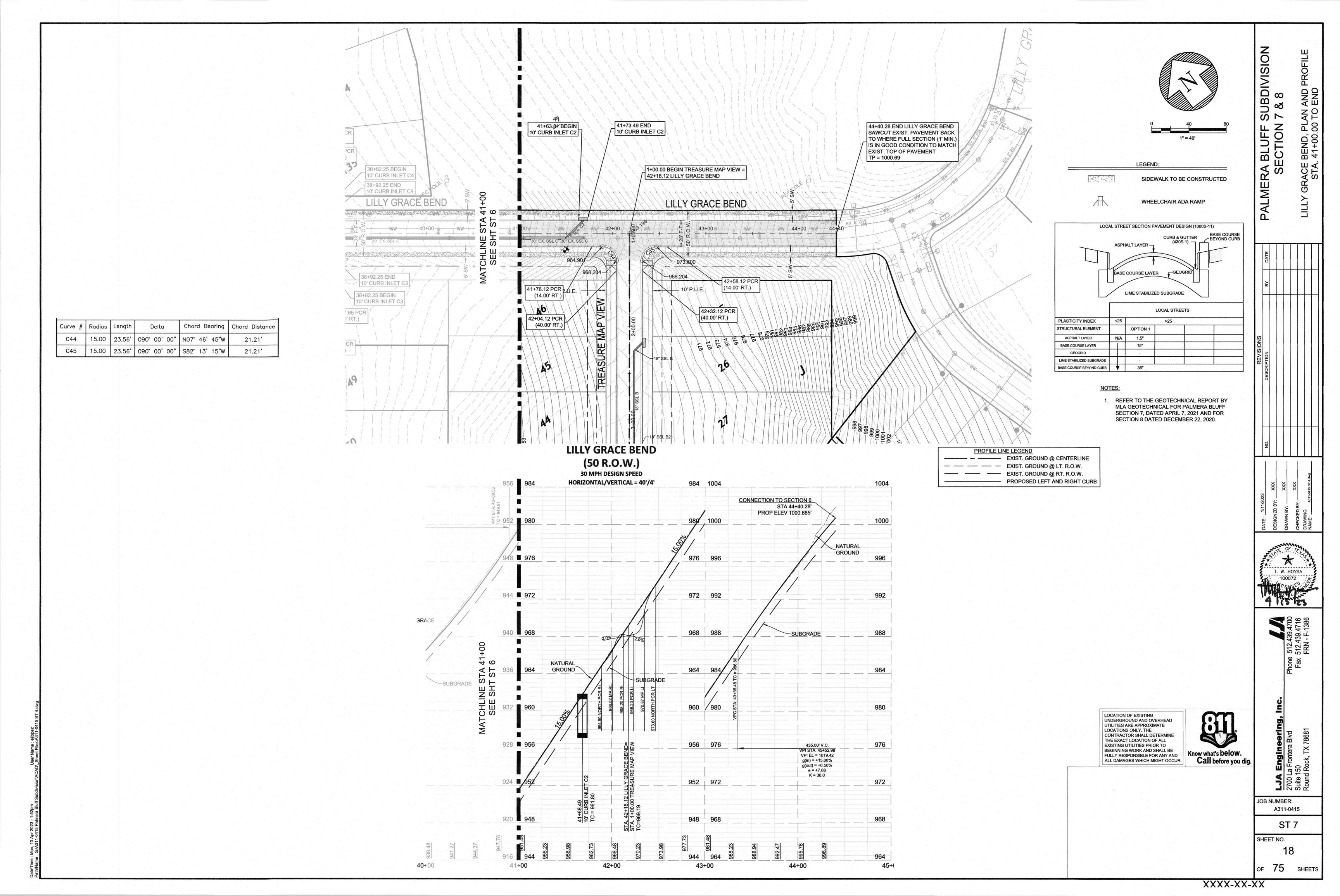
ST3 SHEET NO.

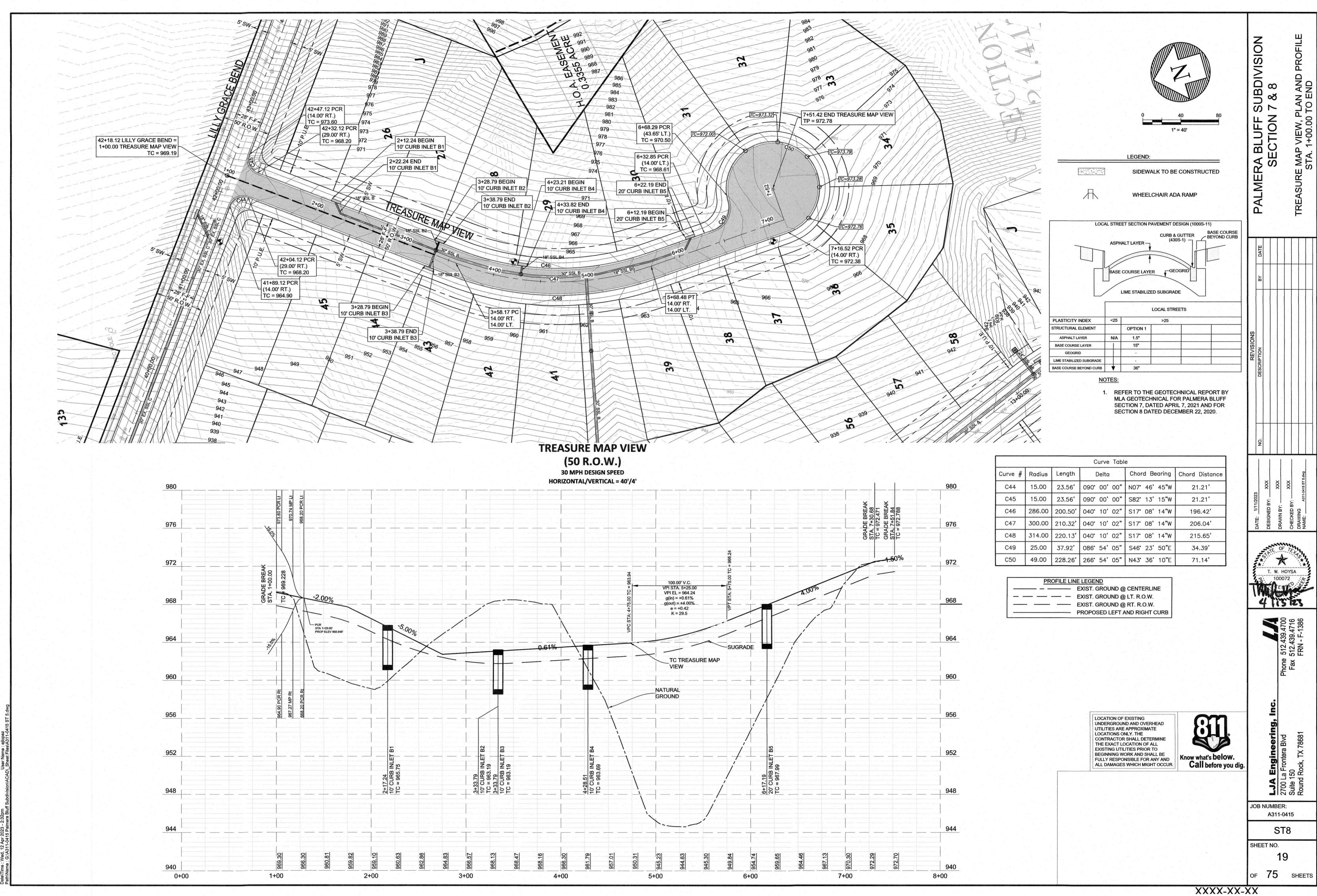
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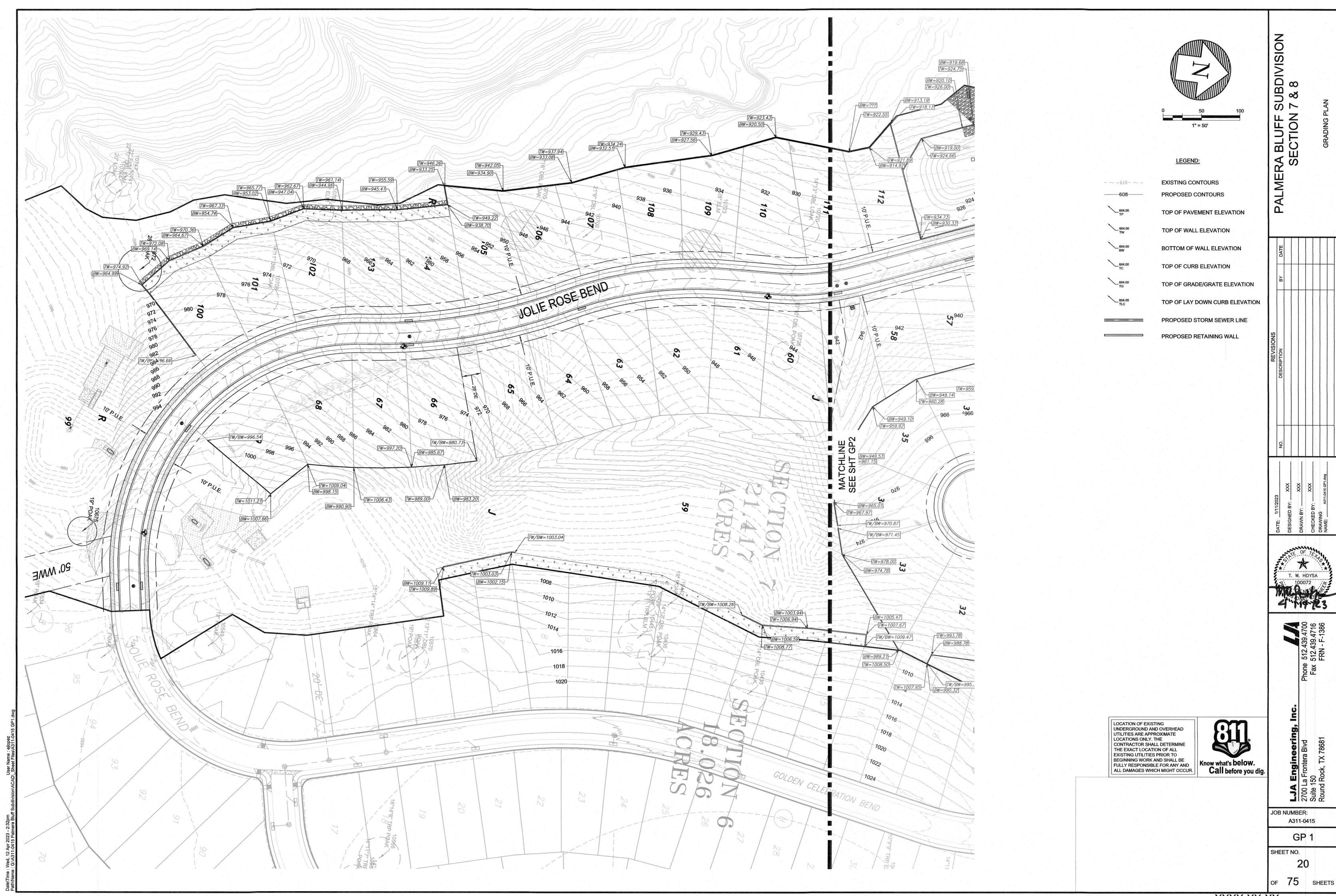


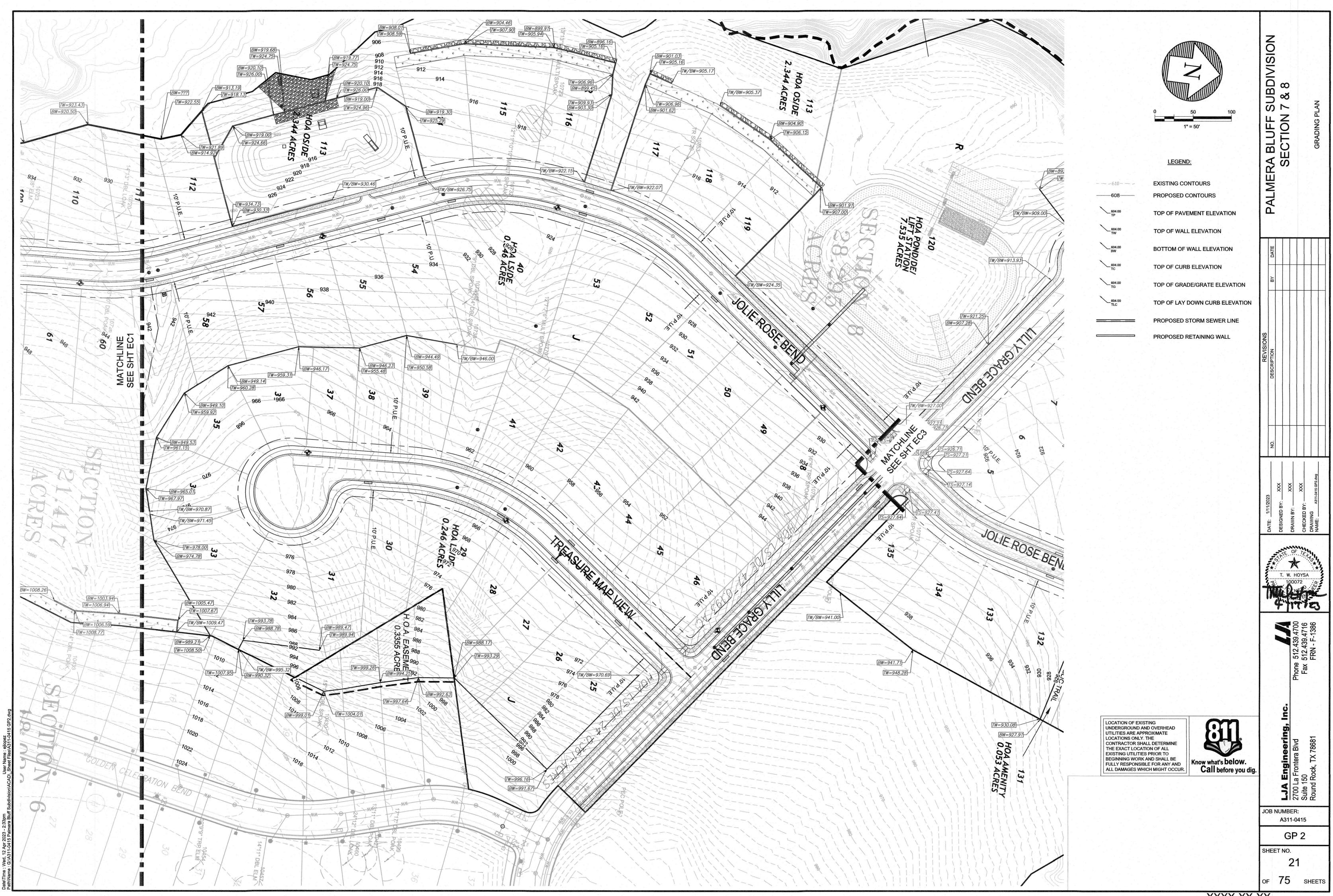


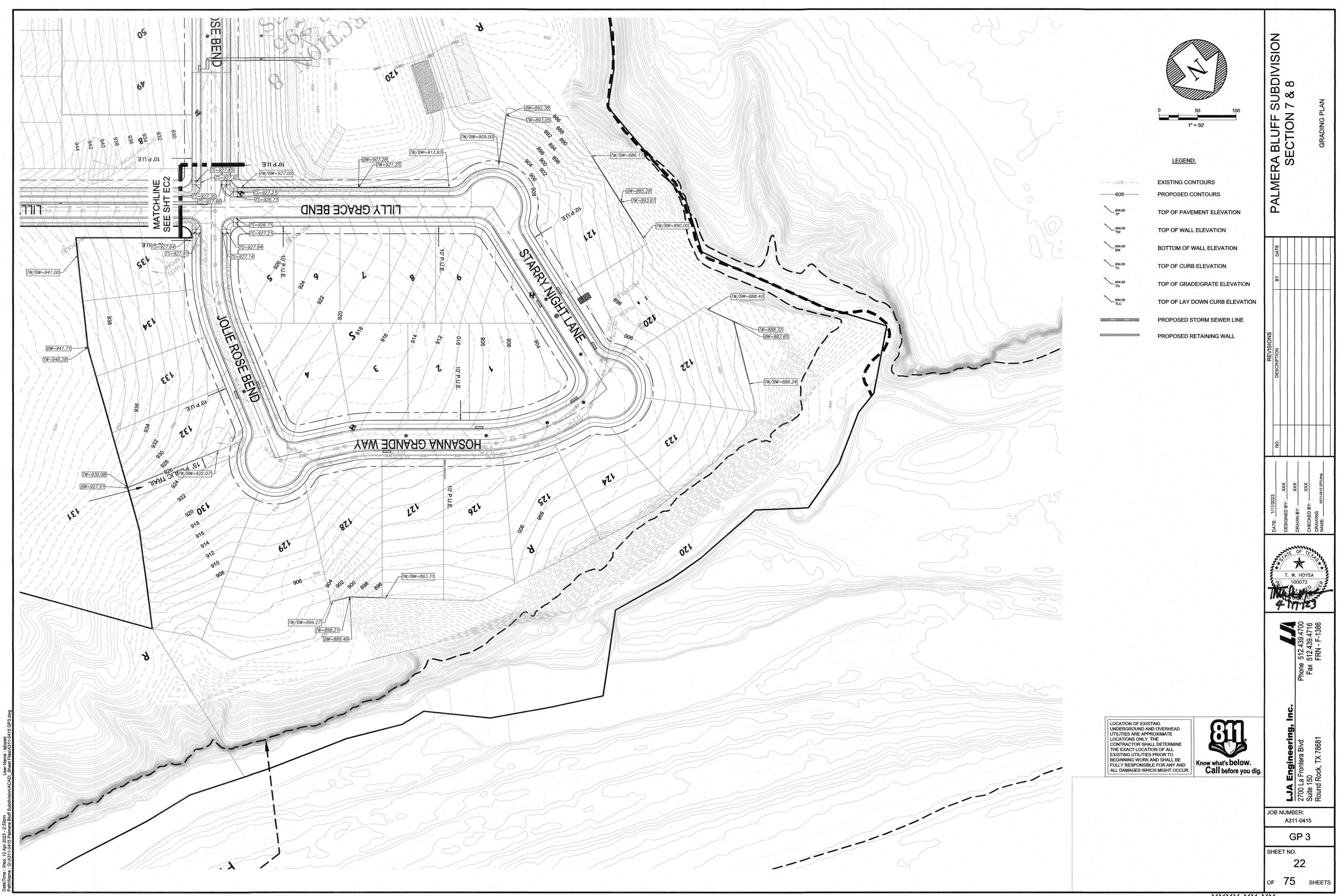


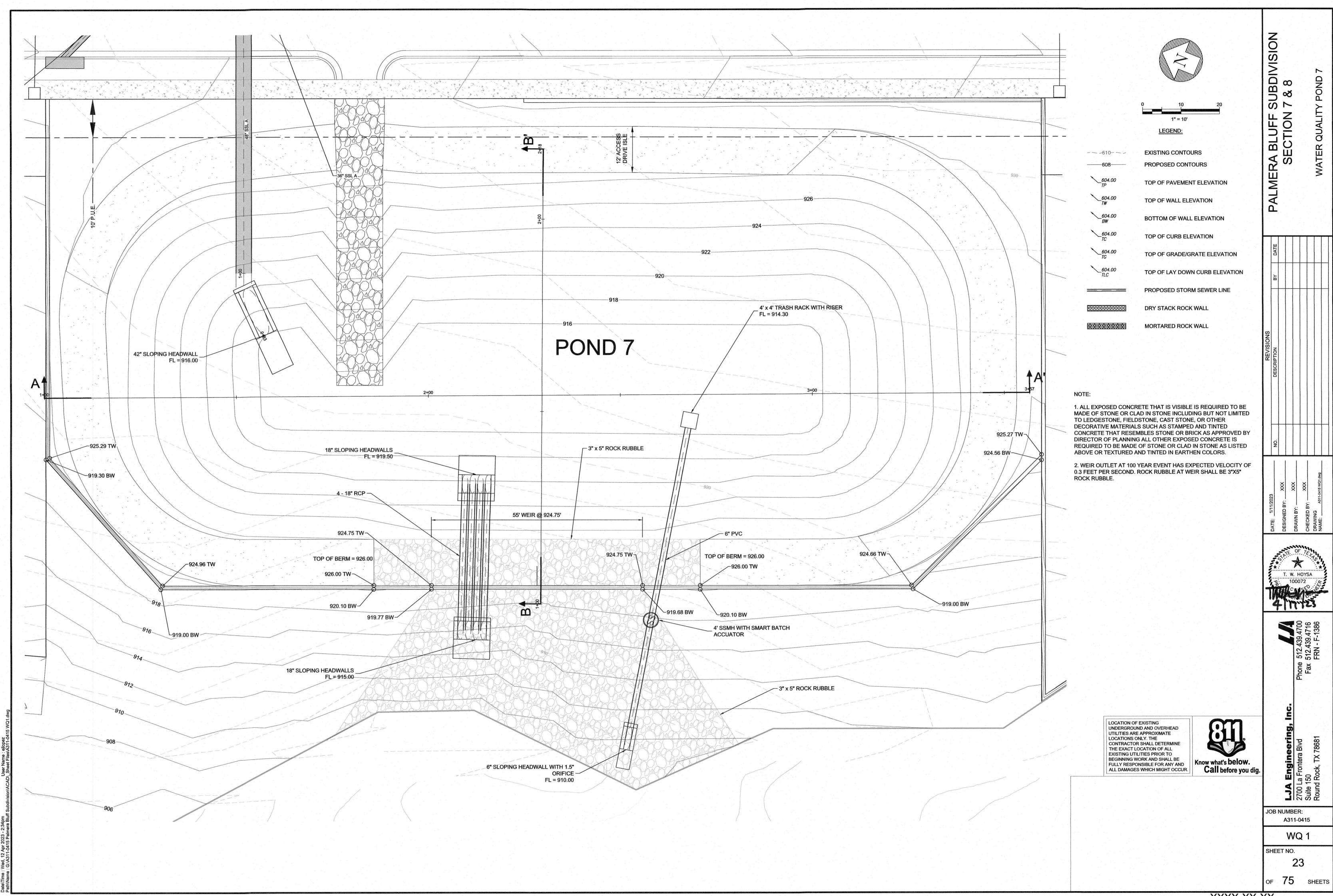












A-A' HORIZONTAL/VERTICAL = 20'/2" 932 928 -NATURAL GROUND 924 920 916 PROPOSED GRADE 912 3+00 4+00 0+50 2+00 1+00

HORIZONTAL/VERTICAL = 20'/2" 932 932 NATURAL GROUND_ 928 FL 924.75 924 924 920 PROPOSED GRADE 916

2+00

3+00

ROCK RUBBLE

912

908

0+50

B-B'

Texas Commission on Environmental Quality

TSS Removal Calculations 04-20-2009

Project Name: Palmera Bluff

Date Prepared: 5/24/2021

Note these calculations are using the updated preliminary plan Sept 24, 2015 and calculating impervious cover by lot sizes and centerline lenghts of streets which differes from the approved method where we assumed a percentage of impervious cover based on lots sizes for overall sections. \\r-data01\eng\LAND1200-1249\1248-900-146\Docs\1248-900-146 TCEQ calculation_template_04-20-09.xls

1. The Required Load Reduction for the total project:

Page 3-29 Equation 3.3: L_M = 27.2(A_N x P)

L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed development = 80% of increased load. A_N = Net increase in impervious area for the project P = Average annual precipitation, inches

Proposed BMP = Extended Detention (Batch Detention Pond 7)

(+230 lbs from untreated)

Pages 3-46 to 3-51

Site Data: Determine Required Load Removal Based on the Entire Project County = Williamson

Total project area included in plan * = 84.45 acres Predevelopment impervious area within the limits of the plan * = 0.00 acres

Total post-development impervious area within the limits of the plan* = 20.49 acres

Total post-development impervious cover fraction * = 0.24

L_{M TOTAL PROJECT} = 17834 lbs.

Number of drainage basins / outfalls areas leaving the plan area = 8

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. =	3	(WQ Pond 7)
Total drainage basin/outfall area =	14.66	acres
Predevelopment impervious area within drainage basin/outfall area =	0.00	acres
Post-development impervious area within drainage basin/outfall area =	3.65	acres
Post-development impervious fraction within drainage basin/outfall area =	0.25	
L _{M THIS BASIN} =	3177	lbs.

3. Indicate the proposed BMP Code for this basin.

		Removal effi	ciency =	91	percent	
4. Calculate Maximum TSS Load Re	moved (L _R) f	for this Drainag	e Basin by	the sele	cted BMP Type.	
			A _C =	14.66	acres	
			A ₁ =	3.65	acres	
			$A_P =$	11.01	acres	
			LR =	3851	lbs	

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

	F=	0.88			
6. Calculate Capture Volume	required by the BMP Type for this drainage	basin / o	utfall area.		
	Rainfall Depth =	1.50	inches		
	Post Development Runoff Coefficient =	0.23	•		
	On-site Water Quality Volume =	18,413	cubic feet		
			•		
	Off-site area draining to BMP =	0.00	acres		
	Off-site Impervious cover draining to BMP =	0.00	acres		
	Impervious fraction of off-site area =	0			
	Off-site Runoff Coefficient =	0.00	•		
	Off-site Water Quality Volume =	0	cubic feet		

Desired L_{M THIS BASIN} = 3407 Ibs.

Storage for Sediment = 3683 Total Capture Volume (required water quality volume(s) x 1.20) = 22095 cubic feet The following sections are used to calculate the required water quality volume(s) for the selected BMP. The values for BMP Types not selected in cell C45 will show NA. Designed as Required in RG-348 8. Extended Detention Basin System

Required Water Quality Volume for extended detention basin = 22095 cubic feet (22,150 WQV Proivded)

WATER QUALITY CONTROL CALCULATIONS 25-year Peak Flow Rate 100-year Peak Flow Rate

129.5 cfs 180.4 cfs

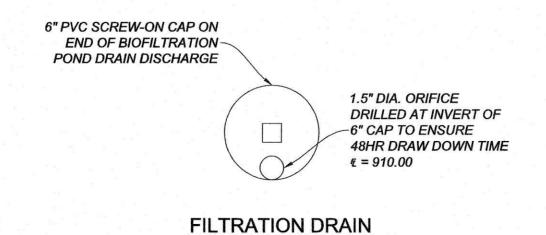
22,095 cf	22,150 cf
919.30 ft MSL	919.30 ft MS
Max 5 ft	5.00 ft
	919.30 ft MSL

WATER QUALITY STAGE/STORAGE RELATIONSHIPS

BA	ATCH DETENTION	ON POND			
	STAGE	AREA (sf)	INC. VOL.	STORAGE (cf)	STORAGE (Ac-Ft)
Orifice FL	914.30	0	0	0	0.0000
Pond Bot	915.00	2,117	494	494	0.01134
	916.00	3,800	2,918	3,411	0.07831
	917.00	4,903	4,340	7,751	0.17794
24 hr DD	918.00	6,068	5,475	13,226	0.30363
	919.00	7,294	6,672	19,898	0.45679
WQV	919.30	7,723	2,252	22,150	0.50849
	920.00	8,582	5,704	27,854	0.63944
	921.00	9,932	9,249	37,103	0.85176
	922.00	11,343	10,630	47,732	1.09579
	923.00	12,816	12,072	59,804	1.37292
	924.00	14,350	13,576	73,380	1.68458
	925.00	15,947	15,142	88,522	2.03218

JNDERDRAIN PIPE ORIFICE OPENII	NG	
Avg Head=	4.6	5 ft
18 hour=	172,800) sec
verage Flow (WQV/48 Hour)=	0.12818	3 cfs
Orifice Area (Q/(0.6*(2gh)^0.5))=	0.0123	5 sf
Orifice Area=	1.78	3 in^2
Orifice Diameter=	1.50	in
Orifice Diameter Used	1.5	in
Drawdown time =	48.29	hours

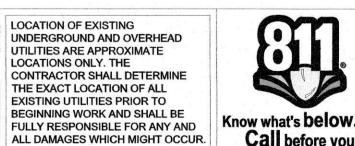
DRAWDOWN 24 HOURS	(including 12 hr hold time)
Start WSEL	919.30 msl
Orifice Elev	910.00 msl
Total Head	9.30 ft
Avg Flow Rate	0.18 cfs
Total Flow for 12 hrs	7,781 cf
Total Drawdown	8,924 cf



OUTLET ORIFICE CAP

SCALE: 1" = 6"

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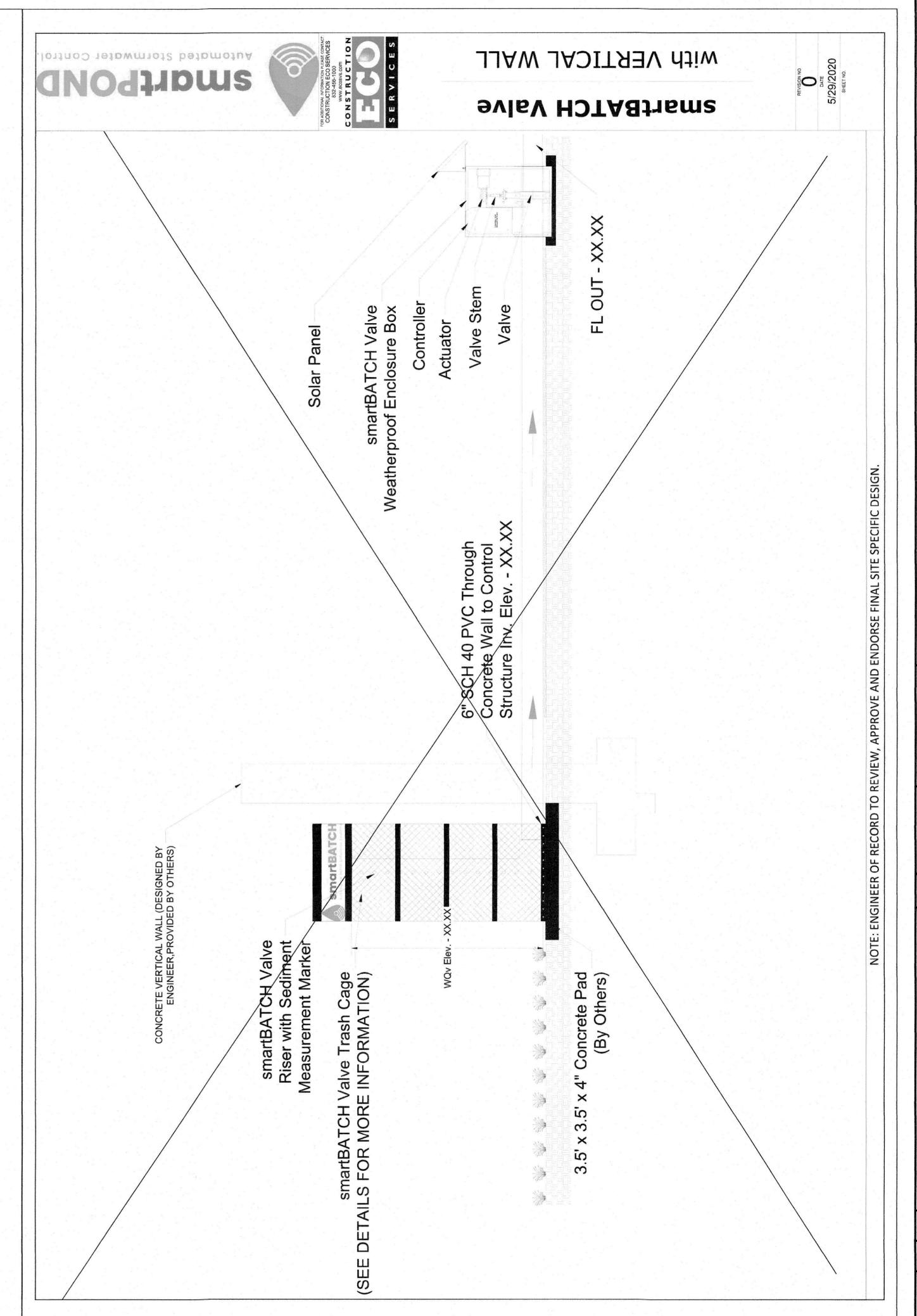


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JOB NUMBER: A311-0415 WQ2 SHEET NO.

PALMERA BLUFF SUBDIVISION SECTION 7 & 8

OF 75 SHEETS



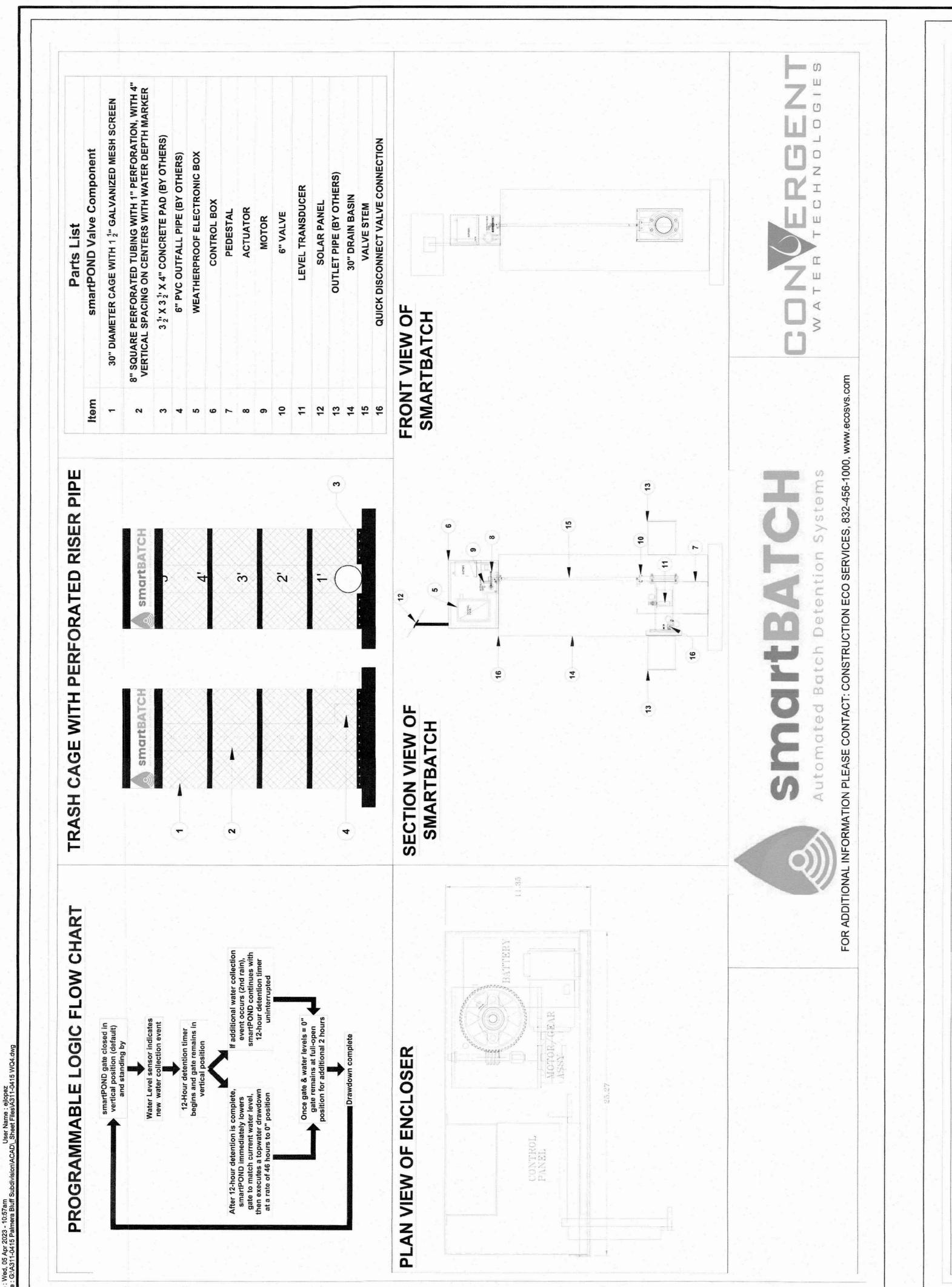
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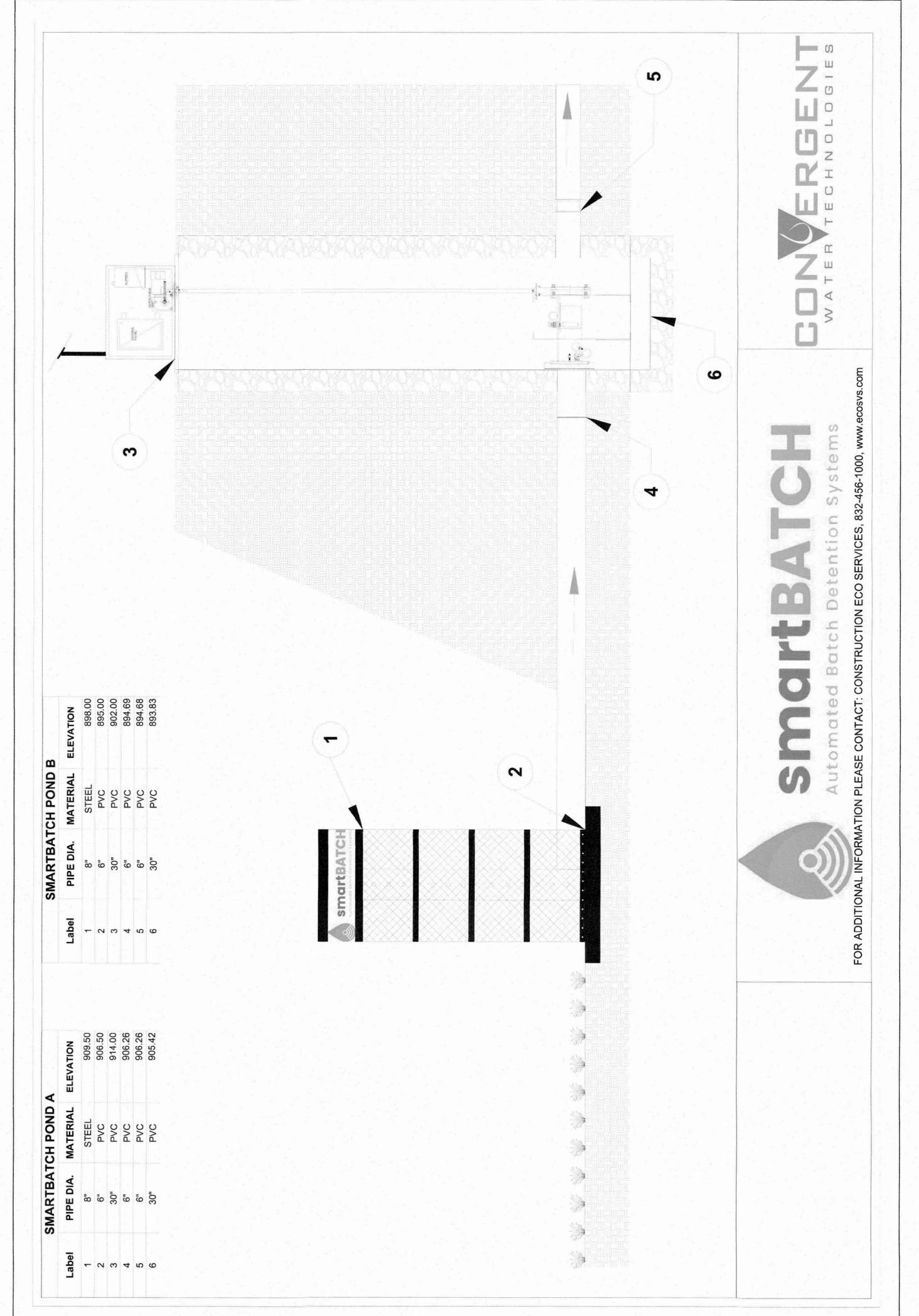
WQ3

XXXX-XX-XX

SHEETS

PALMERA BLUFF SUBDIVISION SECTION 7 & 8



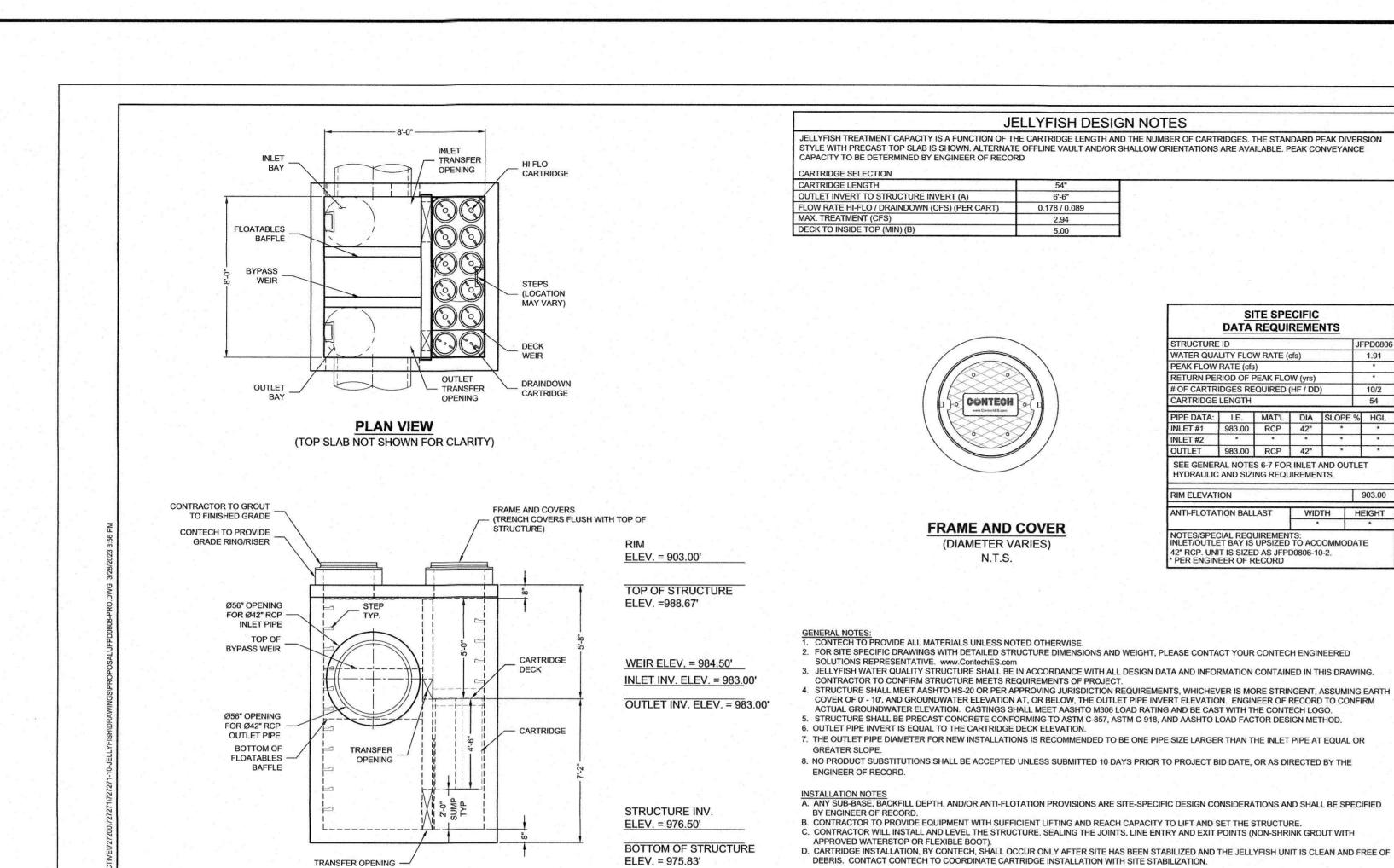


PALMERA BLUFF SUBDIVISION SECTION 7 & 8

WATER QUALITY POND DETAILS

WQ4 SHEET NO. 26 (75 SHEETS

JOB NUMBER: A311-0415



ELEVATION VIEW

CSNTECH

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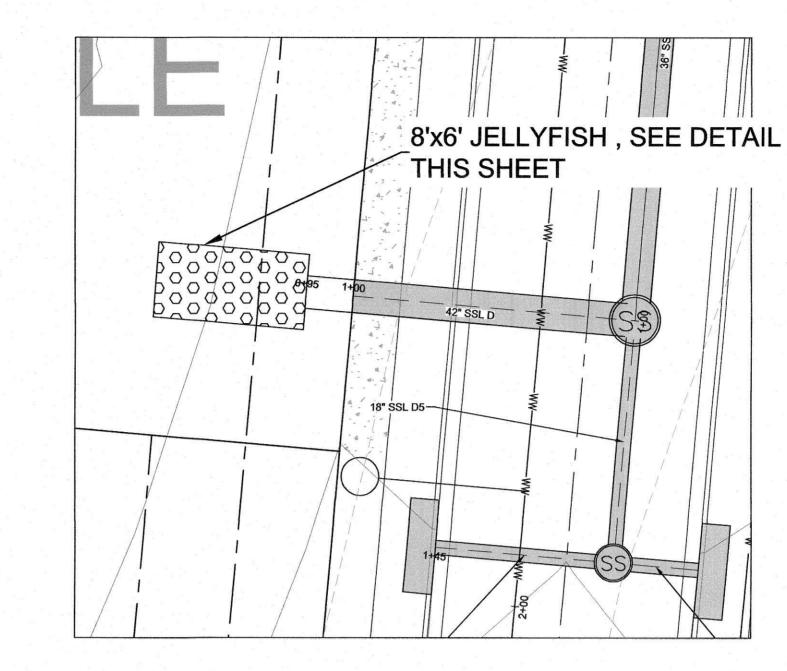
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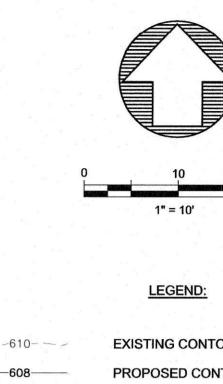
9100 Centre Pointe Dr., Suite 400, West Chester, OH 450

JELLYFISH JFPD0808 - 727271 - 010

PROJECT NAME: PALMERA NEW SECTION LOCATION: LEANDER, TX

SITE DESIGNATION: JELLYFISH 8X6





EXISTING CONTOURS PROPOSED CONTOURS TOP OF PAVEMENT ELEVATION TOP OF WALL ELEVATION _604.00 BW **BOTTOM OF WALL ELEVATION** 604.00 TC TOP OF CURB ELEVATION 604.00 TG TOP OF GRADE/GRATE ELEVATION __604.00 TLC TOP OF LAY DOWN CURB ELEVATION PROPOSED STORM SEWER LINE DRY STACK ROCK WALL MORTARED ROCK WALL

LOCATION OF EXISTING

LOCATIONS ONLY. THE

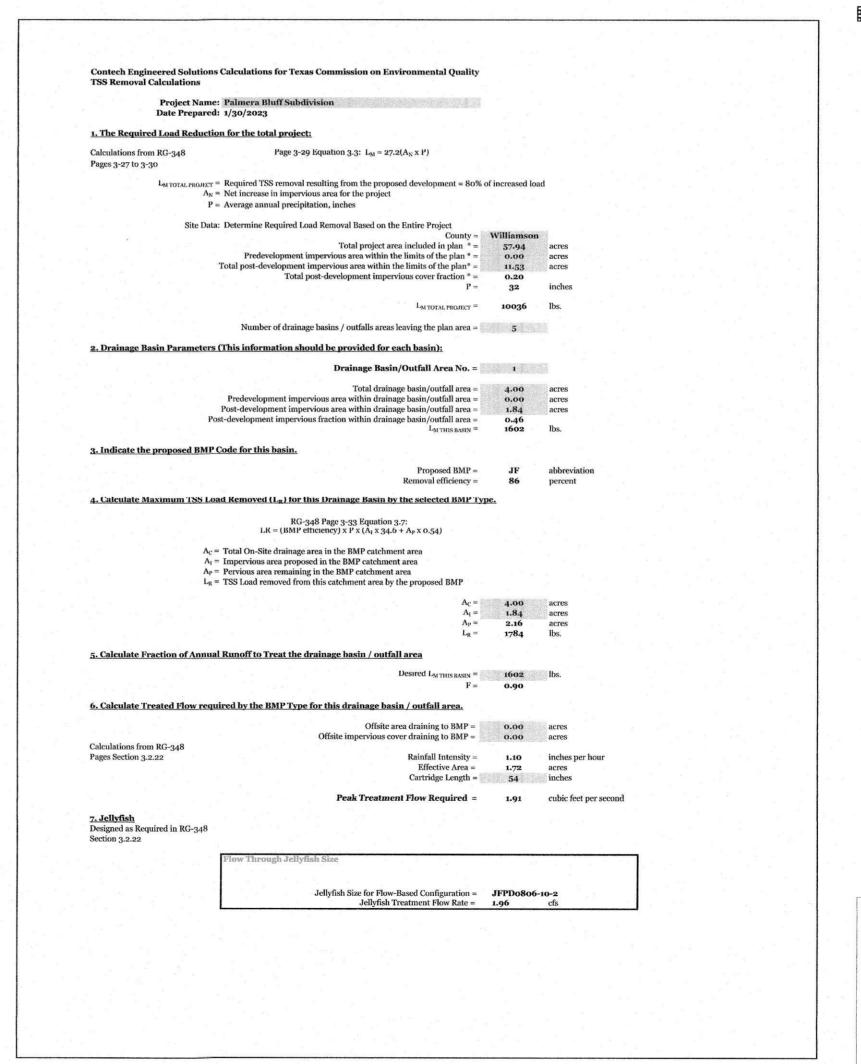
UNDERGROUND AND OVERHEAD UTILITIES ARE APPROXIMATE

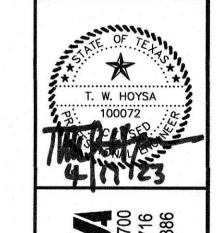
CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL

FULLY RESPONSIBLE FOR ANY AND

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EXISTING UTILITIES PRIOR TO BEGINNING WORK AND SHALL BE



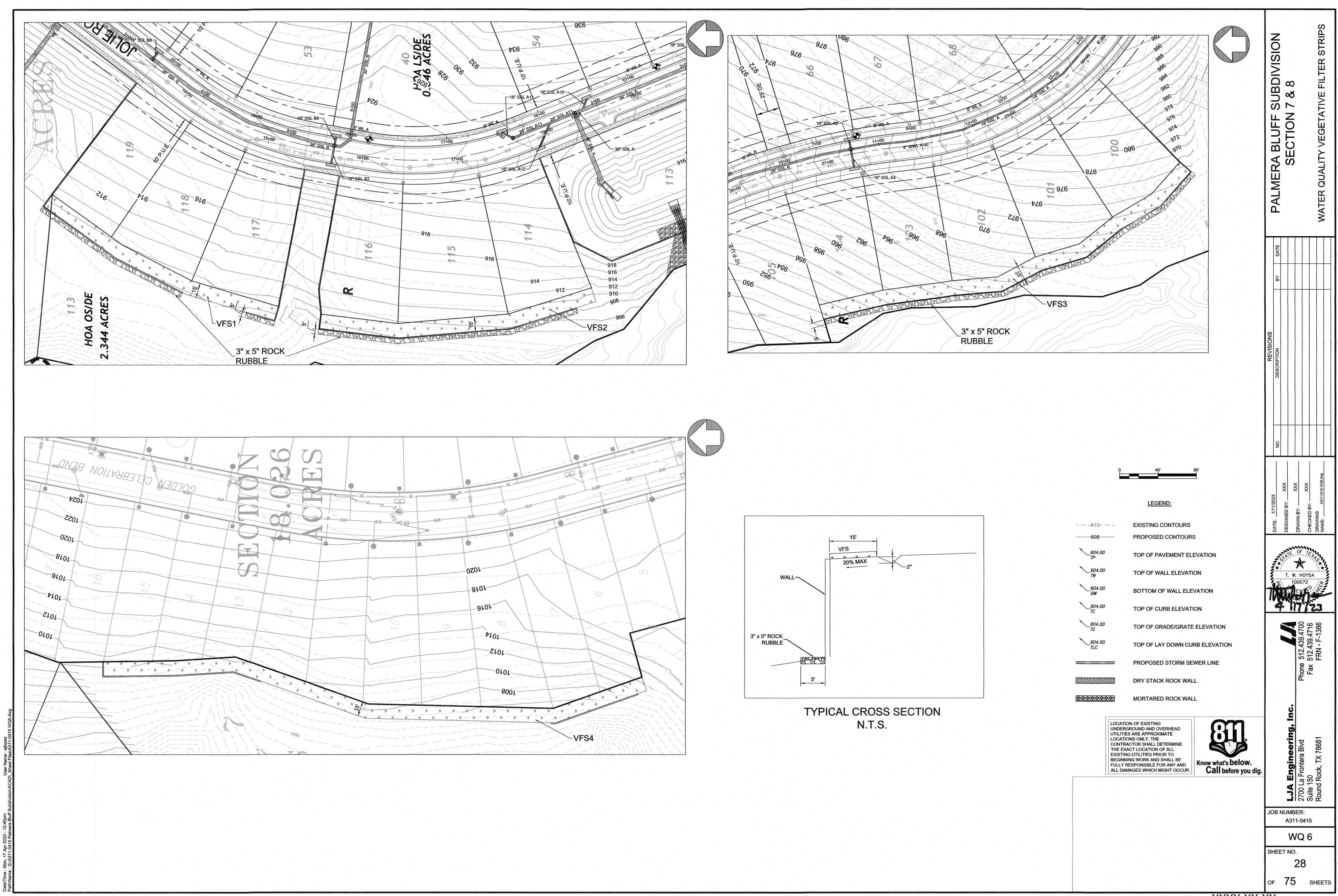


SUBDIVISION 17 & 8

Know what's below. Call before you dig.

JOB NUMBER: A311-0415

SHEET NO. 27 OF 75 SHEETS



Designed as Required in RG-348

16. Vegetated Filter Strips

There are no calculations required for determining the load or size of vegetative filter strips.

The 80% removal is provided when the contributing drainage area does not exceed 72 feet (direction of flow) and

the sheet flow leaving the impervious cover is directed across 15 feet of engineered filter strips with maximum slope of 20% or across 50 feet of natural vegetation with a maximum slope of 10%. There can be a break in grade as long as no slope exceeds 20%.

If vegetative filter strips are proposed for an interim permanent BMP, they may be sized as described on Page 3-56 of RG-348.

Pages 3-55 to 3-57

Project Name: Palmera Bluff TSS Removal Calculations 04-20-2009 Date Prepared: 9/26/2022 1. The Required Load Reduction for the total project: Page 3-29 Equation 3.3: L_M = 27.2(A_N x P) L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed development = 80% of increased load. A_N = Net increase in impervious area for the project P = Average annual precipitation, inches Site Data: Determine Required Load Removal Based on the Entire Project County = Williamson Total project area included in plan * = 84.45 acres Predevelopment impervious area within the limits of the plan * = 0.00 Total post-development impervious area within the limits of the plan* = 20.49 acres Total post-development impervious cover fraction * = L_{M TOTAL PROJECT} = 17834 Number of drainage basins / outfalls areas leaving the plan area = 8 2. Drainage Basin Parameters (This information should be provided for each basin): Drainage Basin/Outfall Area No. = 6 (VFS 3) Total drainage basin/outfall area = 1.07 Predevelopment impervious area within drainage basin/outfall area = 0.00 acres Post-development impervious area within drainage basin/outfall area = 0.47 acres Post-development impervious fraction within drainage basin/outfall area = 0.44 L_{M THIS BASIN} = 409 lbs. 3. Indicate the proposed BMP Code for this basin. Proposed BMP = Vegetated Filter Strips Removal efficiency = 85 percent 4. Calculate Maximum TSS Load Removed (LR) for this Drainage Basin by the selected BMP Type. $A_C = 1.07$ acres A₁ = **0.47** acres A_P = 0.60 acres L_R = 451 lbs 5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area Desired L_{M THIS BASIN} = 451 lbs. F = 1.00 Pages 3-55 to 3-57 Designed as Required in RG-348 16. Vegetated Filter Strips

Texas Commission on Environmental Quality

There are no calculations required for determining the load or size of vegetative filter strips. The 80% removal is provided when the contributing drainage area does not exceed 72 feet (direction of flow) and the sheet flow leaving the impervious cover is directed across 15 feet of engineered filter strips with maximum slope of 20% or across 50 feet of natural vegetation with a maximum slope of 10%. There can be a break in grade as long as no slope exceeds 20%. If vegetative filter strips are proposed for an interim permanent BMP, they may be sized as described on Page 3-56 of RG-348.

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JOB NUMBER:

T. W. HOYSA

RA BLUFF SUBDIVISION SECTION 7 & 8

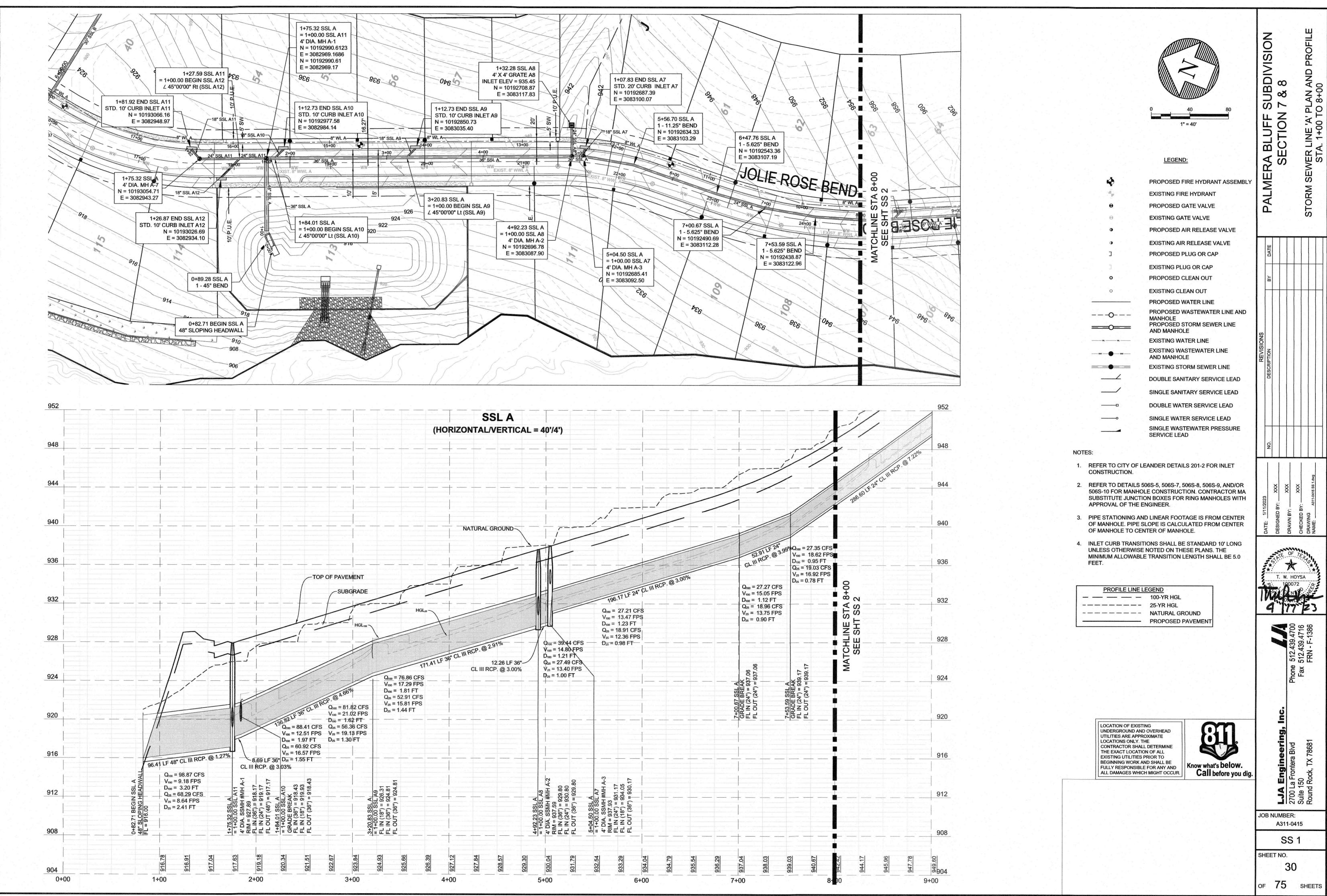
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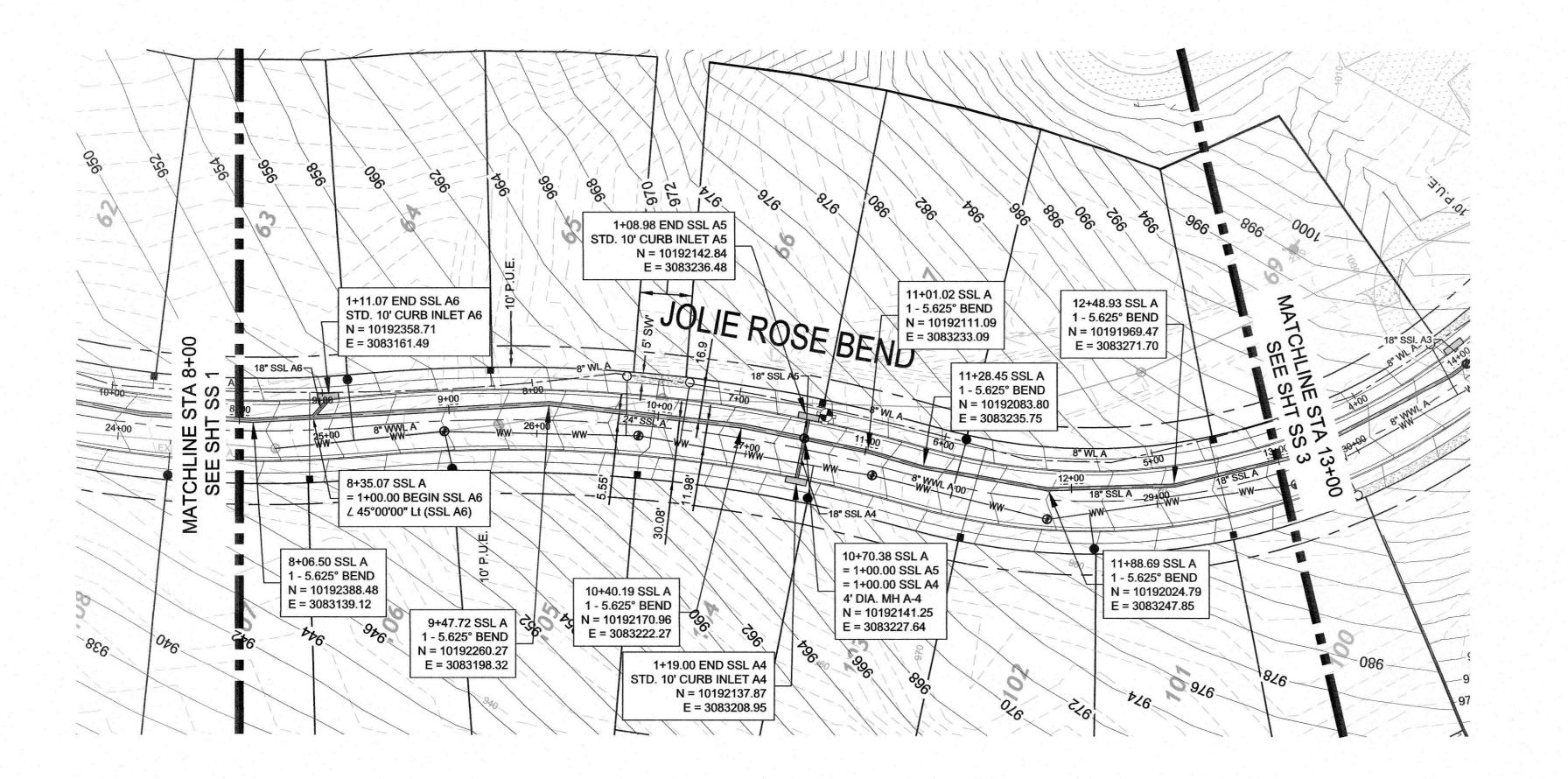
QUALITY VEGETATIVE FILTER CALCULATIONS

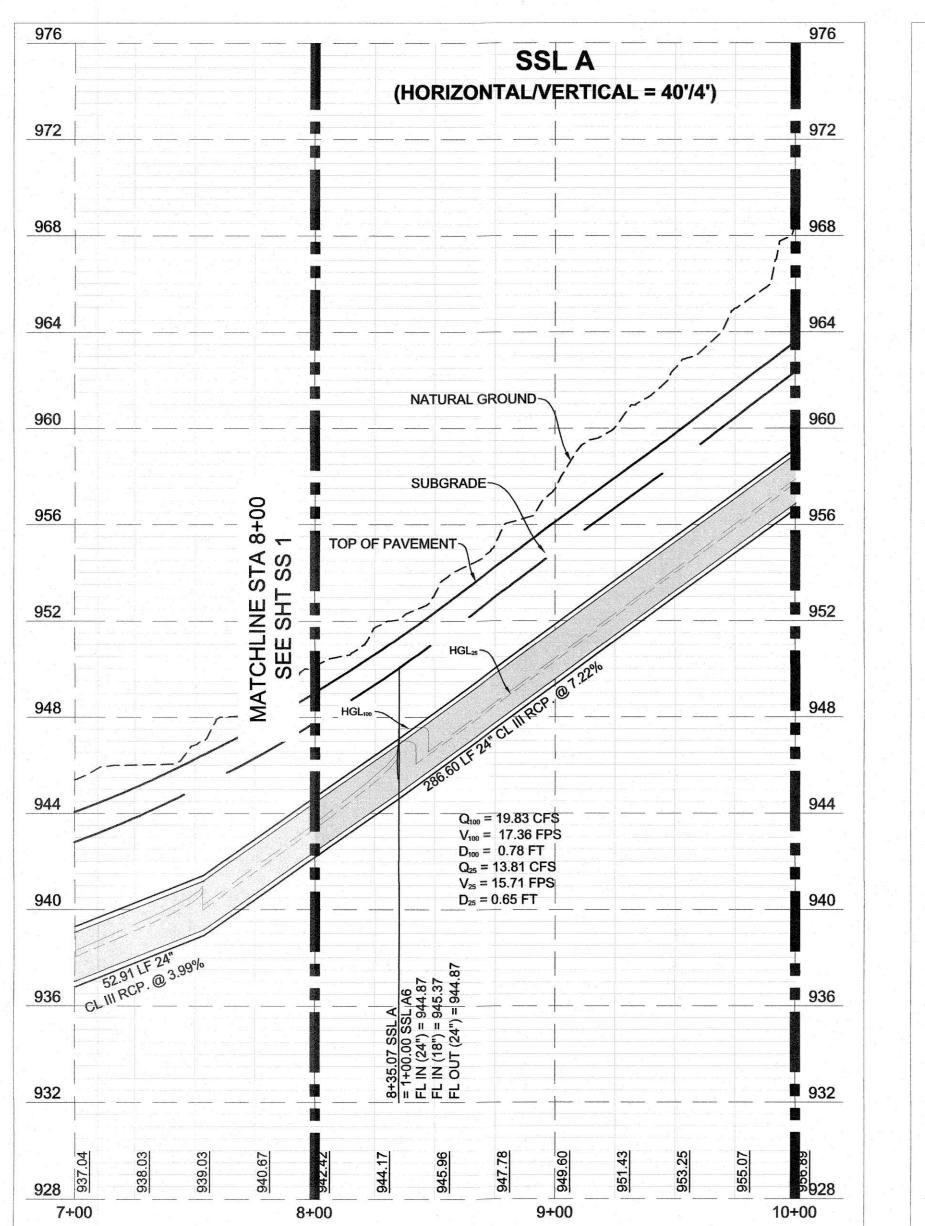
A311-0415

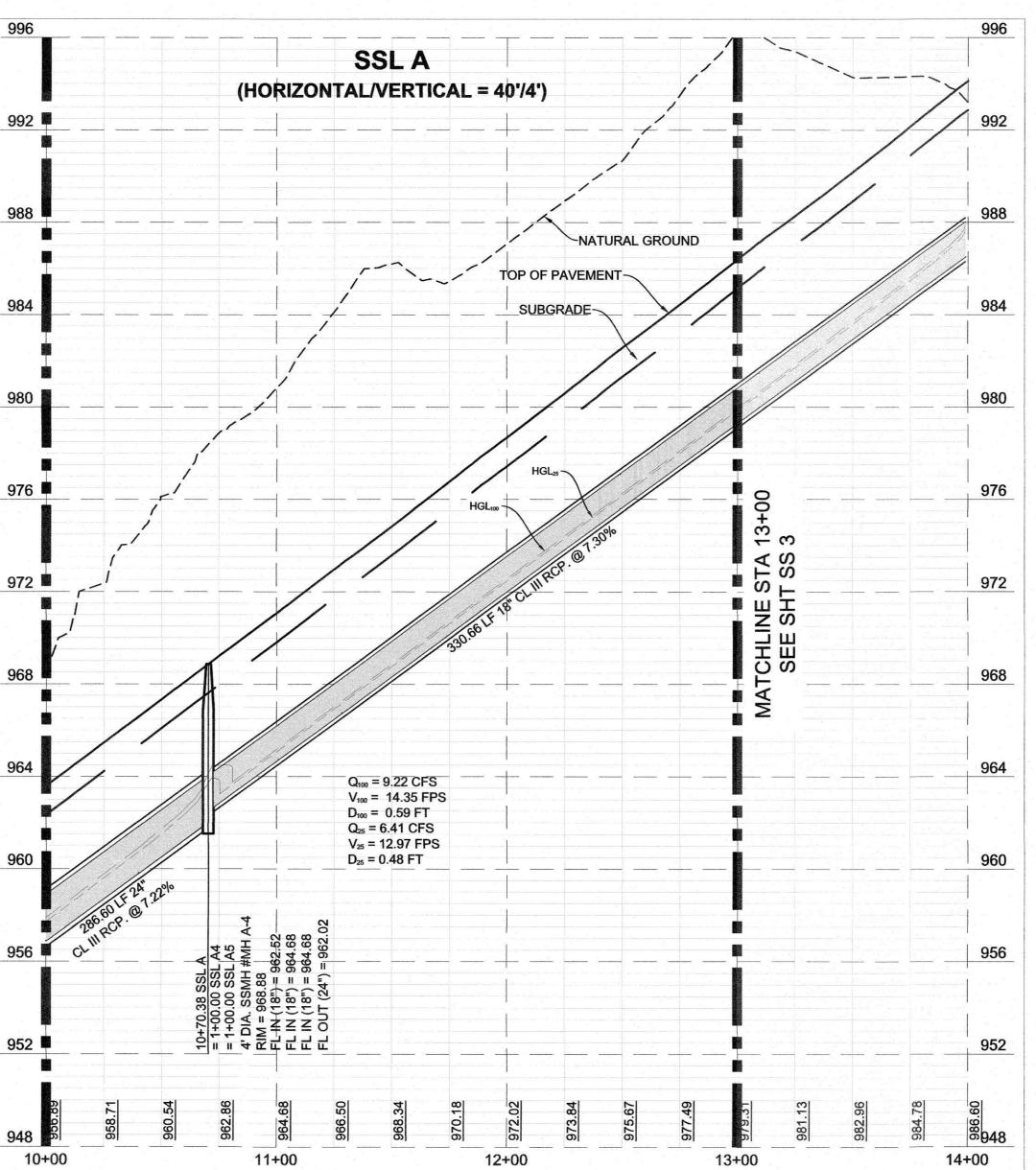
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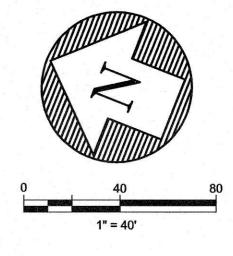
OF 75 SHEETS











SUBDIVISION

RA BLUFF S

VER LINE 'A' PLAN AN STA. 8+00 TO 13+00

LEGEND:

•	PROPOSED FIRE HYDRANT ASSEMBLY
	EXISTING FIRE HYDRANT
•	PROPOSED GATE VALVE
€	EXISTING GATE VALVE
0	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
]	PROPOSED PLUG OR CAP
	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
ww	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
	DOUBLE SANITARY SERVICE LEAD
	SINGLE SANITARY SERVICE LEAD
	DOUBLE WATER SERVICE LEAD
	SINGLE WATER SERVICE LEAD
	SINGLE WASTEWATER PRESSURE SERVICE LEAD

NOTES:

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- 2. REFER TO DETAILS 506S-5, 506S-7, 506S-8, 506S-9, AND/OR 506S-10 FOR MANHOLE CONSTRUCTION. CONTRACTOR MA SUBSTITUTE JUNCTION BOXES FOR RING MANHOLES WITH APPROVAL OF THE ENGINEER.
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PROFILE LINE L	EGEND
	100-YR HGL
	25-YR HGL
	NATURAL GROUND
	PROPOSED PAVEME

LOCATION OF EXISTING UNDERGROUND AND OVERHEAD UTILITIES ARE APPROXIMATE LOCATIONS ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL **EXISTING UTILITIES PRIOR TO** BEGINNING WORK AND SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT OCCUR.



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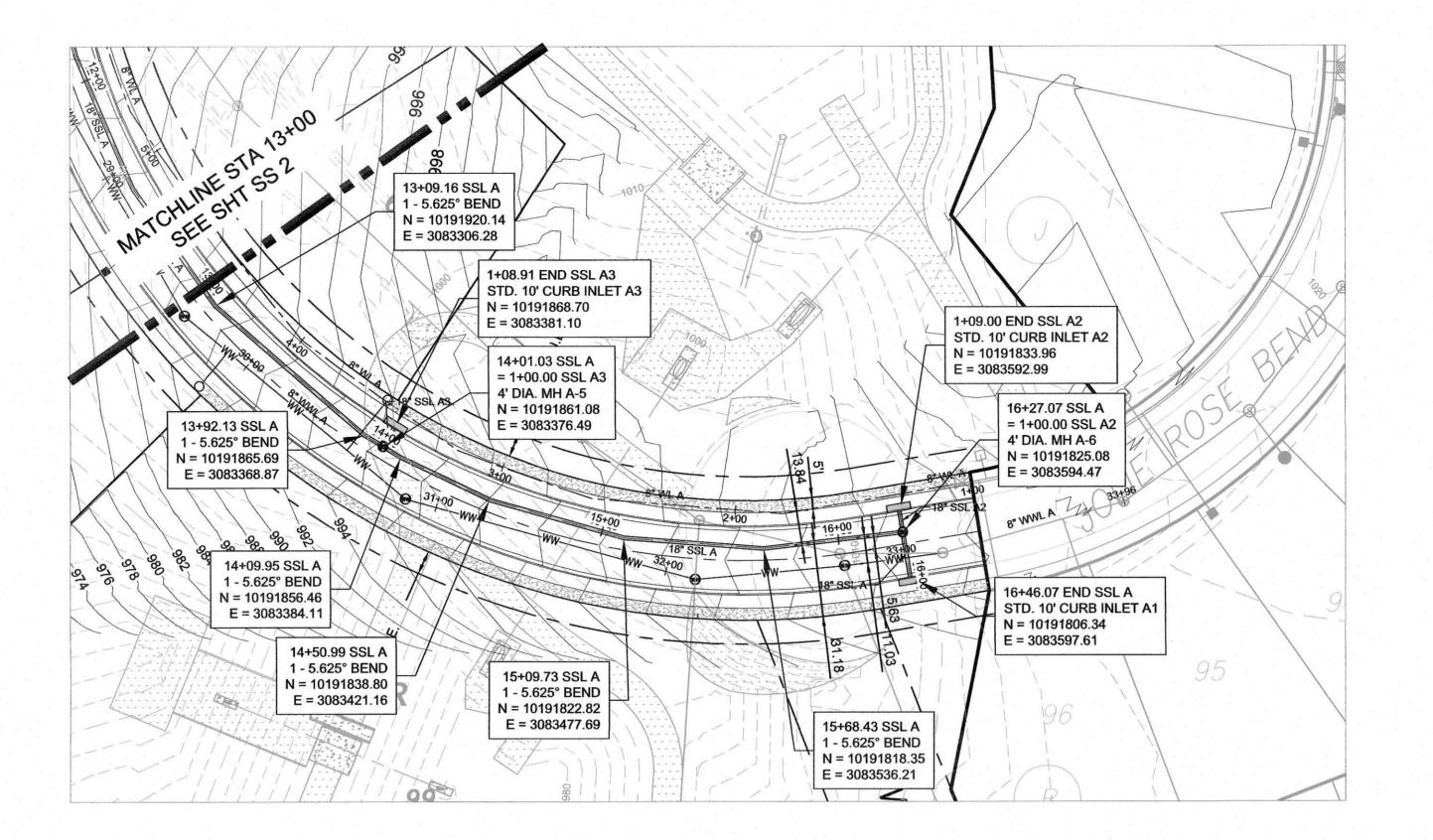
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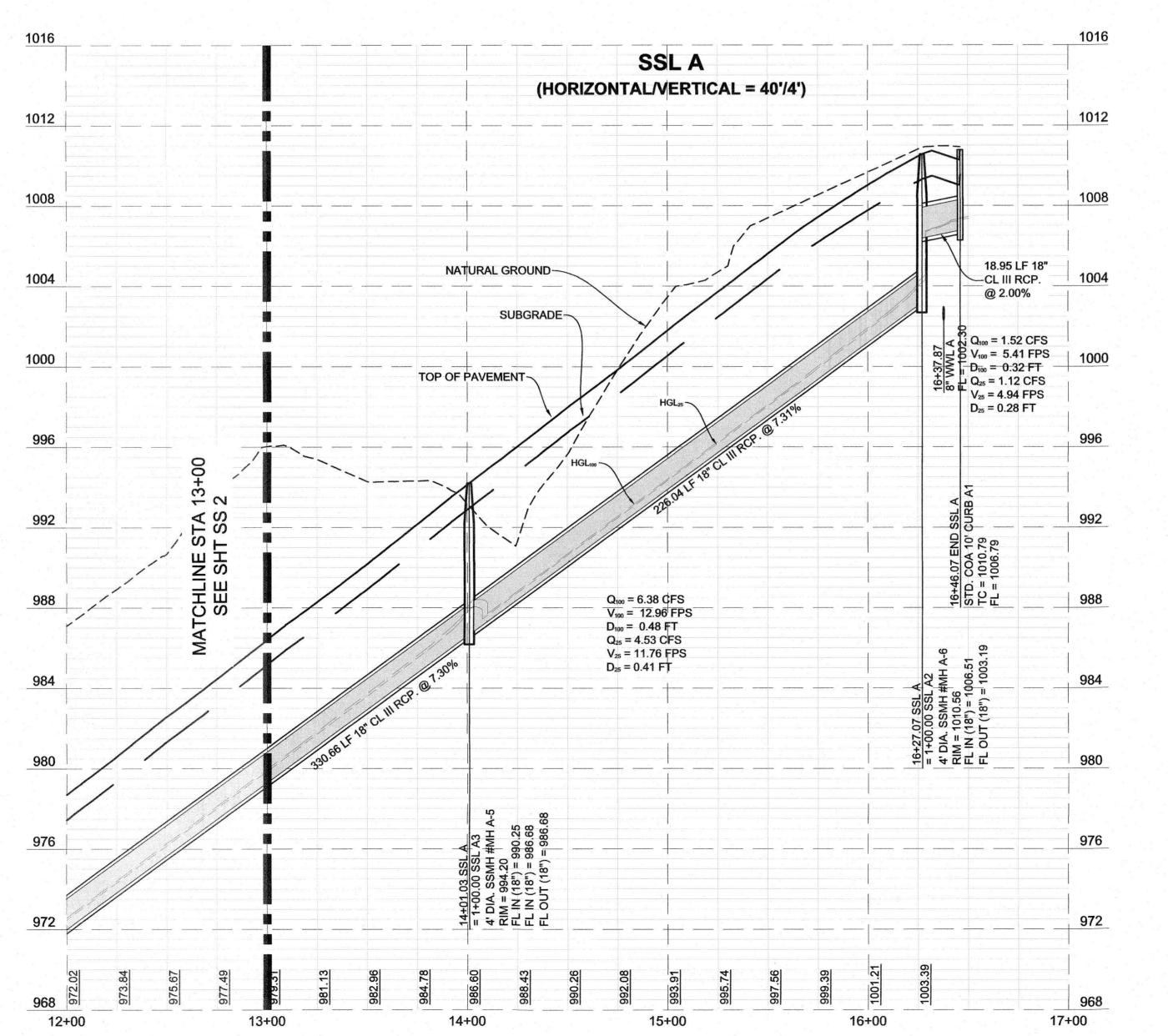
SHEET NO. of 75 sheets

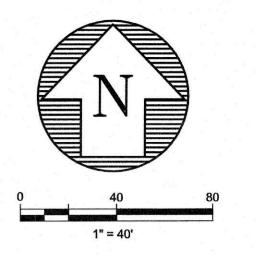
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SS 2

T. W. HOYSA







SUBDIVISION 17&8

PALMERA BLUFF S SECTION 7

LEGEND:

•	PROPOSED FIRE HYDRANT ASSEMBLY
4	EXISTING FIRE HYDRANT
•	PROPOSED GATE VALVE
€	EXISTING GATE VALVE
0	PROPOSED AIR RELEASE VALVE
0	EXISTING AIR RELEASE VALVE
3	PROPOSED PLUG OR CAP
1	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
-	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE PROPOSED STORM SEWER LINE AND MANHOLE
w	EXISTING WATER LINE
ww	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
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PROFILE LINE L	EGEND
	100-YR HGL
	25-YR HGL
	NATURAL GROUND
VOLUME STATE OF THE STATE OF TH	PROPOSED PAVEME

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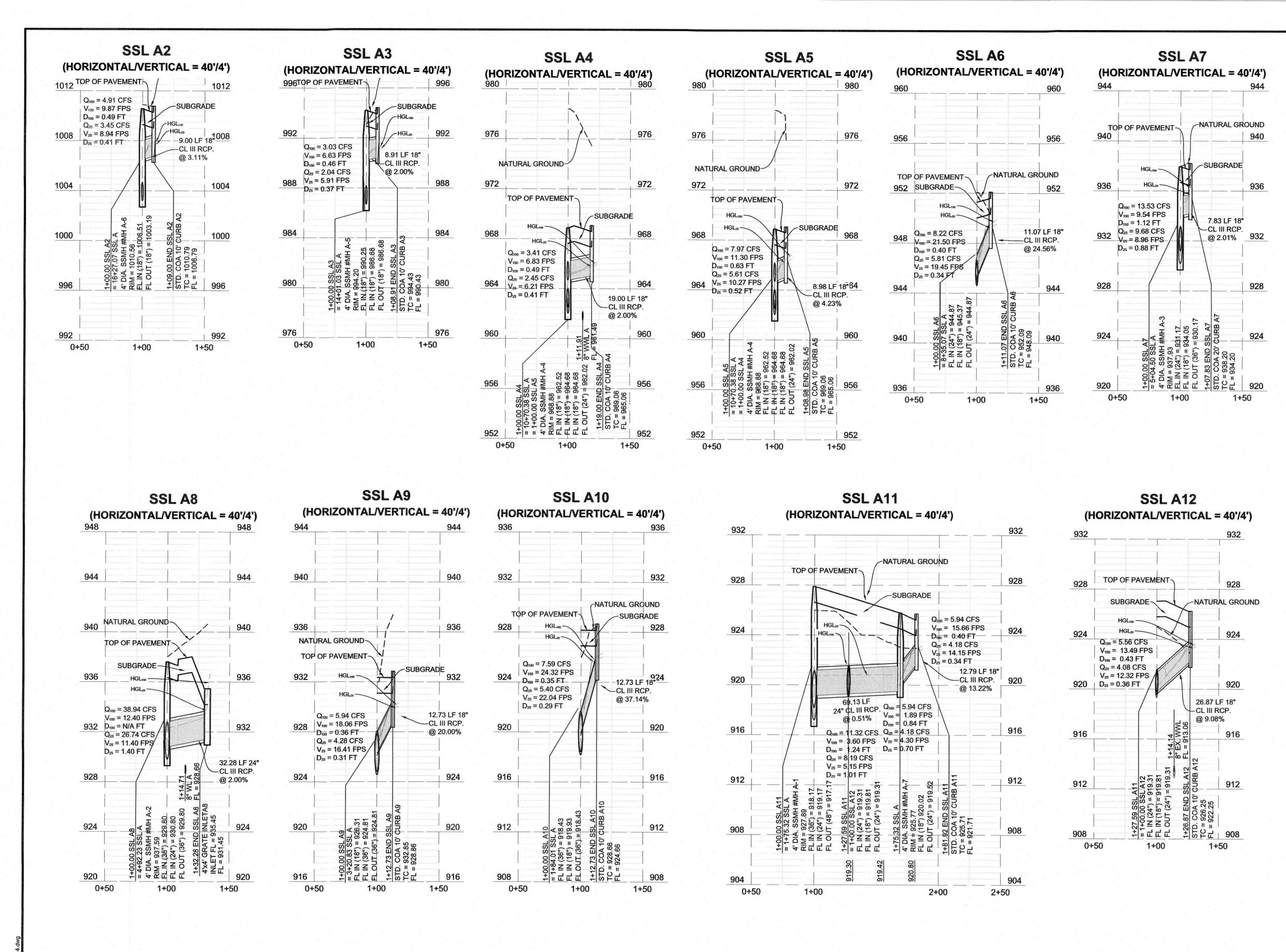
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JOB NUMBER: A311-0415 SS 3

T. W. HOYSA

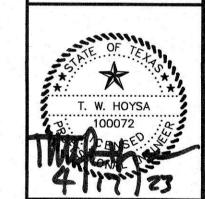
SHEET NO.

of 75 SHEETS



PROFILE LINE LEGEND ---- 25-YR HGL ---- NATURAL GROUND PROPOSED PAVEMEN

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> JOB NUMBER: A311-0415

> > **SS 4**

PALMERA BLUFF SECTION

SUBDIVISION 17 & 8

LOCATION OF EXISTING

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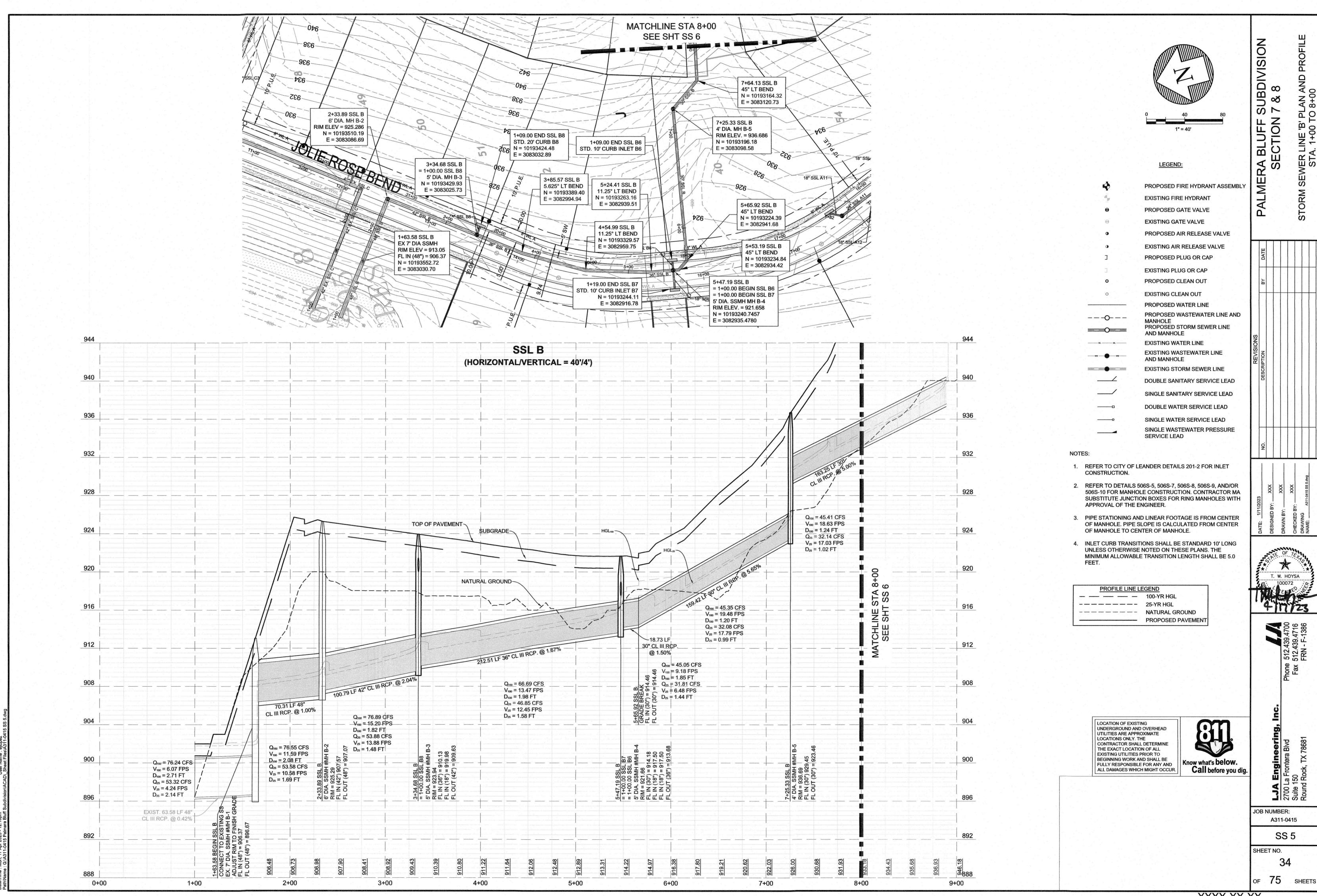
FULLY RESPONSIBLE FOR ANY AND

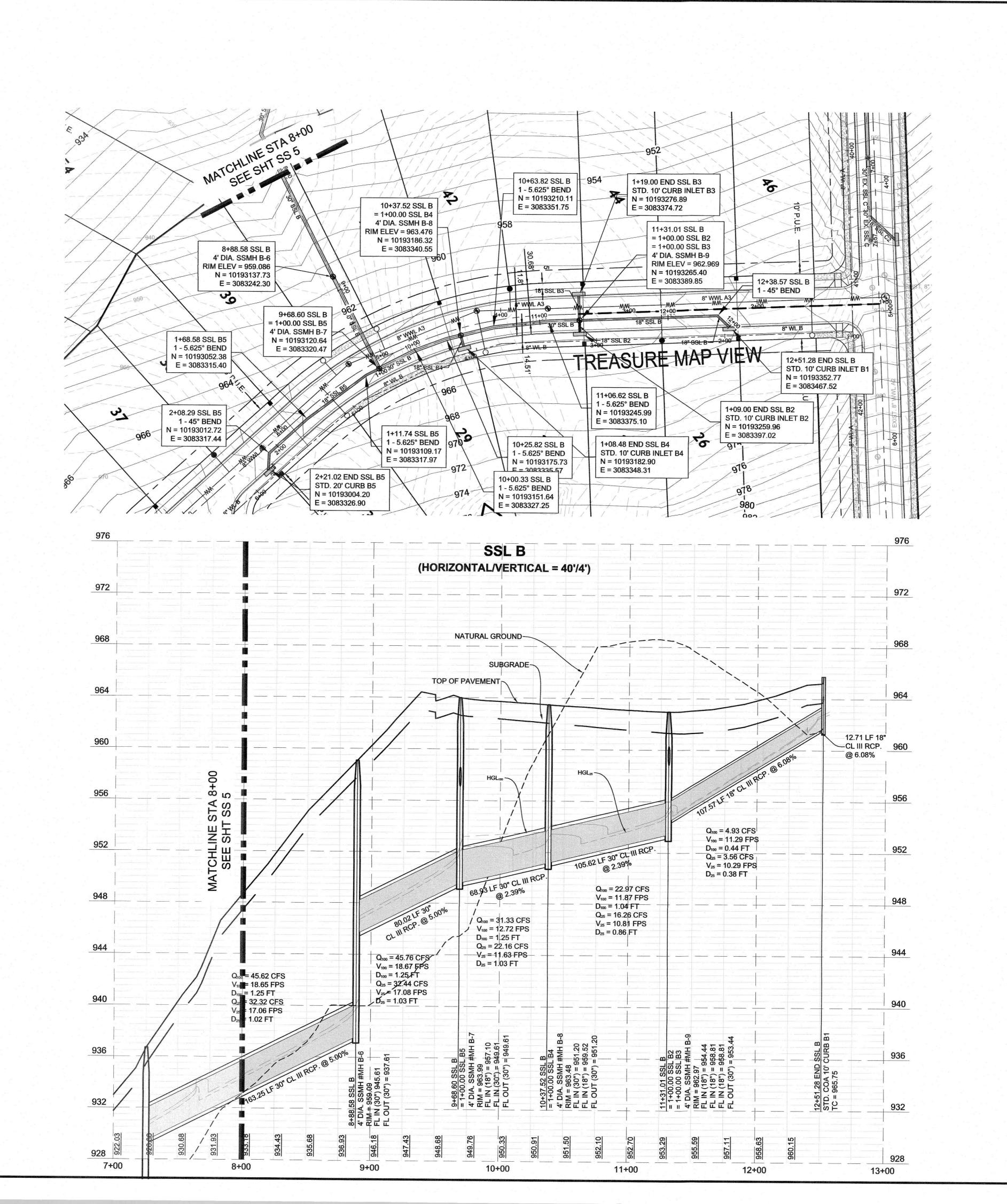
ALL DAMAGES WHICH MIGHT OCCUR.

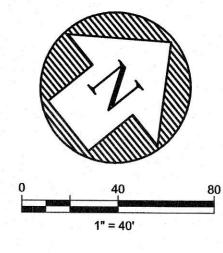
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SHEET NO. 33

of 75 sheets







LEGEND:

•	PROPOSED FIRE HYDRANT ASSEMBLY
4	EXISTING FIRE HYDRANT
•	PROPOSED GATE VALVE
Θ	EXISTING GATE VALVE
•	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
3	PROPOSED PLUG OR CAP
]	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE
	PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
w	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
	DOUBLE SANITARY SERVICE LEAD
	SINGLE SANITARY SERVICE LEAD
	DOUBLE WATER SERVICE LEAD
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PROFILE LINE L	EGEND
	100-YR HGL
	25-YR HGL
	NATURAL GROUND
	PROPOSED PAVEMENT

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Know what's below.

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JOB NUMBER: A311-0415 **SS 6**

SHEET NO.

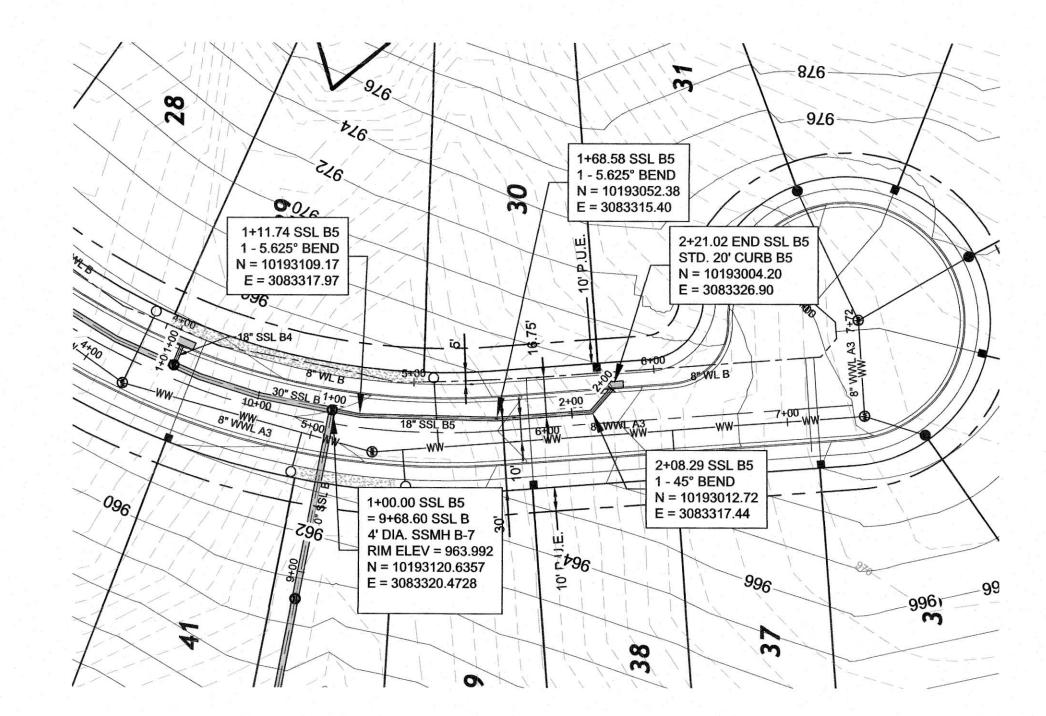
OF 75 SHEETS

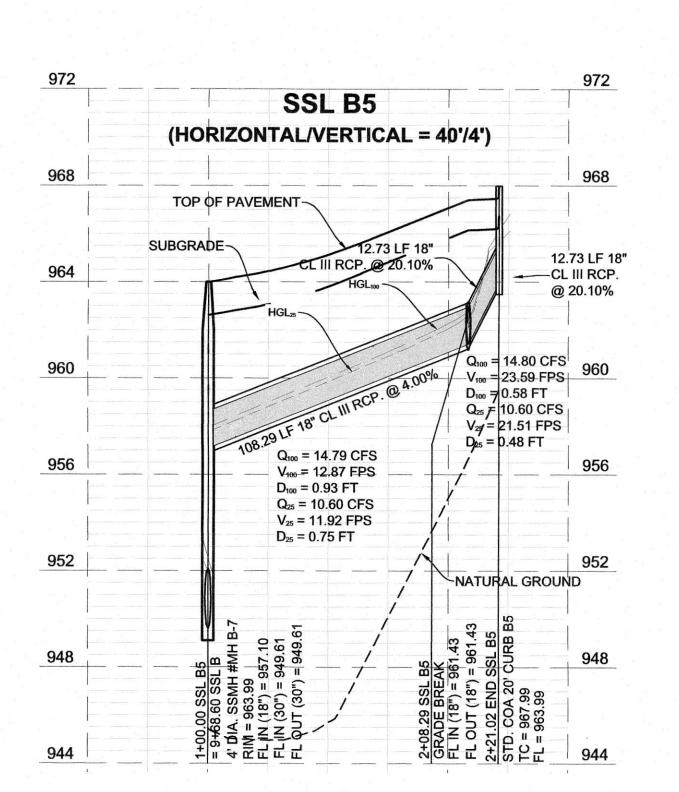
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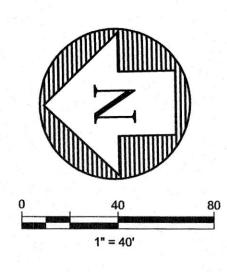
RA BLUFF SUBDIVISION SECTION 7 & 8

PALME

SEWER LINE 'B' PLAN AND STA. 8+00 TO END







ALMERA BLUFF SUBDIVISION SECTION 7 & 8

LEGEND:

•	PROPOSED FIRE HYDRANT ASSEMBLY
4,	EXISTING FIRE HYDRANT
•	PROPOSED GATE VALVE
Θ	EXISTING GATE VALVE
0	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
]	PROPOSED PLUG OR CAP
	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
The state of the s	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
	DOUBLE SANITARY SERVICE LEAD
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	PROFILE LINE LI	EGEND
		100-YR HGL
-		25-YR HGL
		NATURAL GROUND
		PROPOSED PAVEMEN

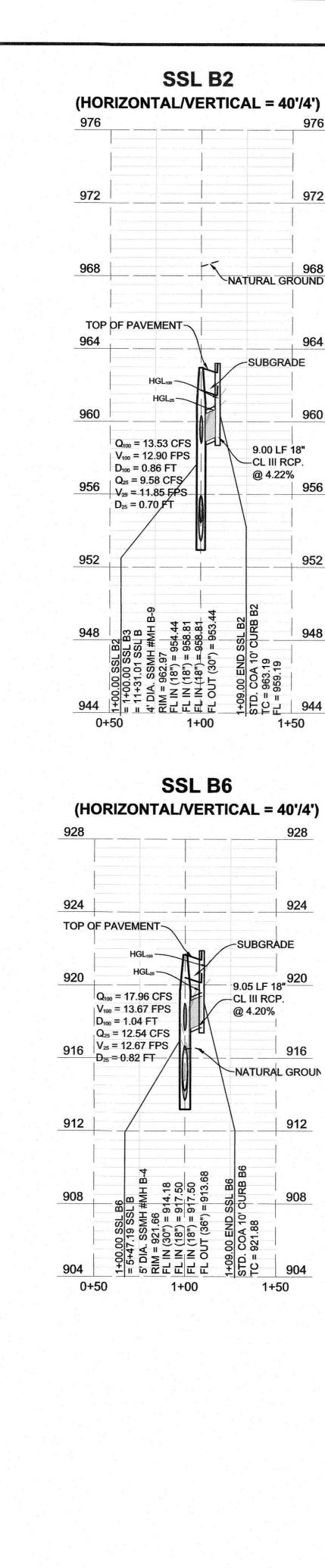
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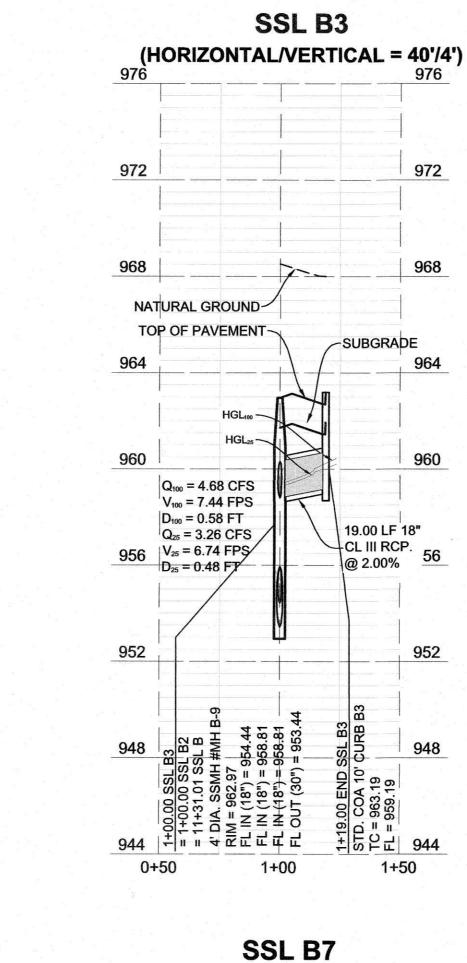


JOB NUMBER: A311-0415 SS 7 SHEET NO.

of 75 sheets

T. W. HOYSA





(HORIZONTAL/VERTICAL = 40'/4')

-SUBGRADE

NATURAL GROUND

1+50

19.00 LF 18"16

CL III RCP.

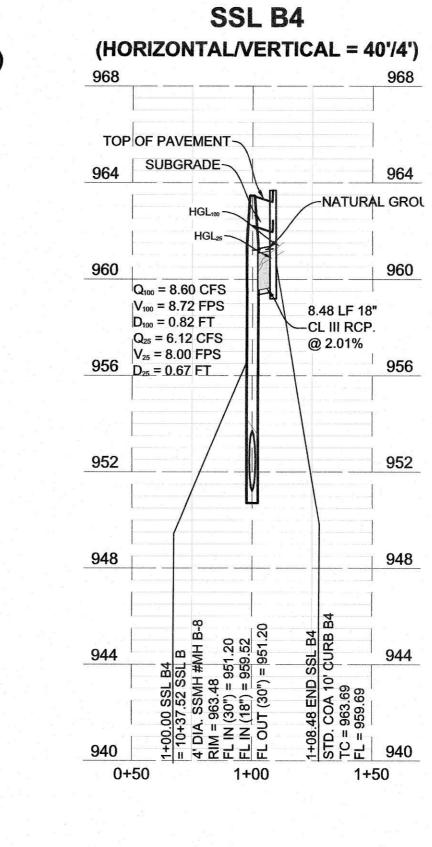
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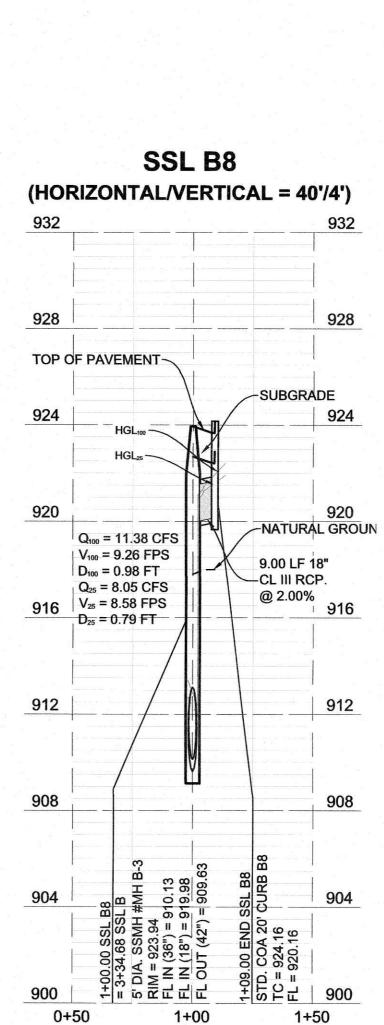
TOP OF PAVEMENT

 $\begin{array}{c} Q_{100} = 4.43 \text{ CFS} \\ V_{100} = 7.34 \text{ FPS} \\ D_{100} = 0.56 \text{ FT} \\ Q_{25} = 3.16 \text{ CFS} \\ V_{25} = 6.68 \text{ FPS} \\ D_{25} = 0.47 \text{ FT} \end{array}$

0+50

1+00





PROFILE LINE LEGEND ----- 25-YR HGL ---- NATURAL GROUND

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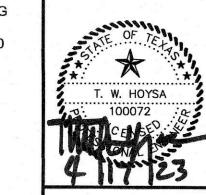
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PALMERA BLUFF SUBDIVISION SECTION 7 & 8

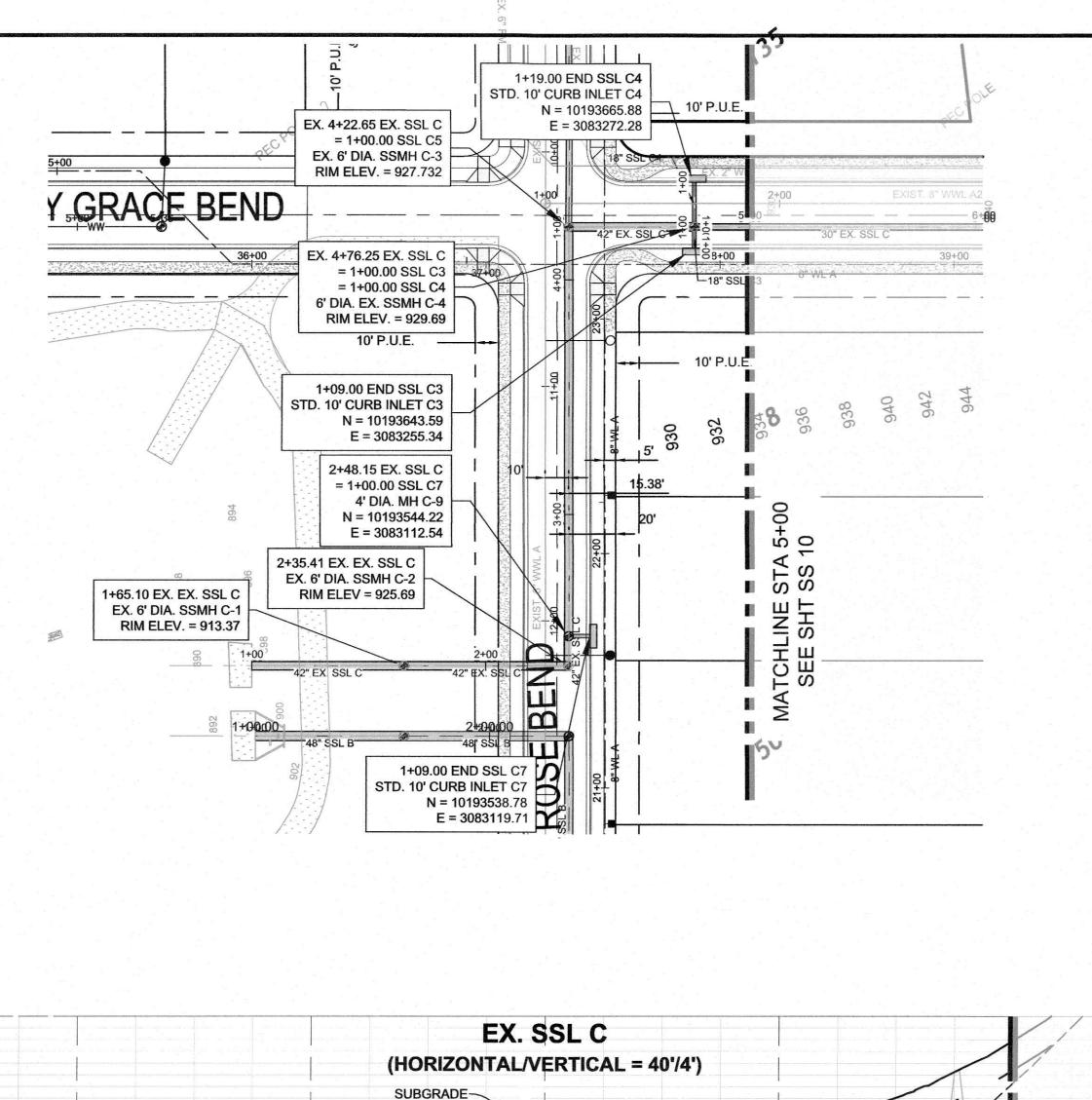
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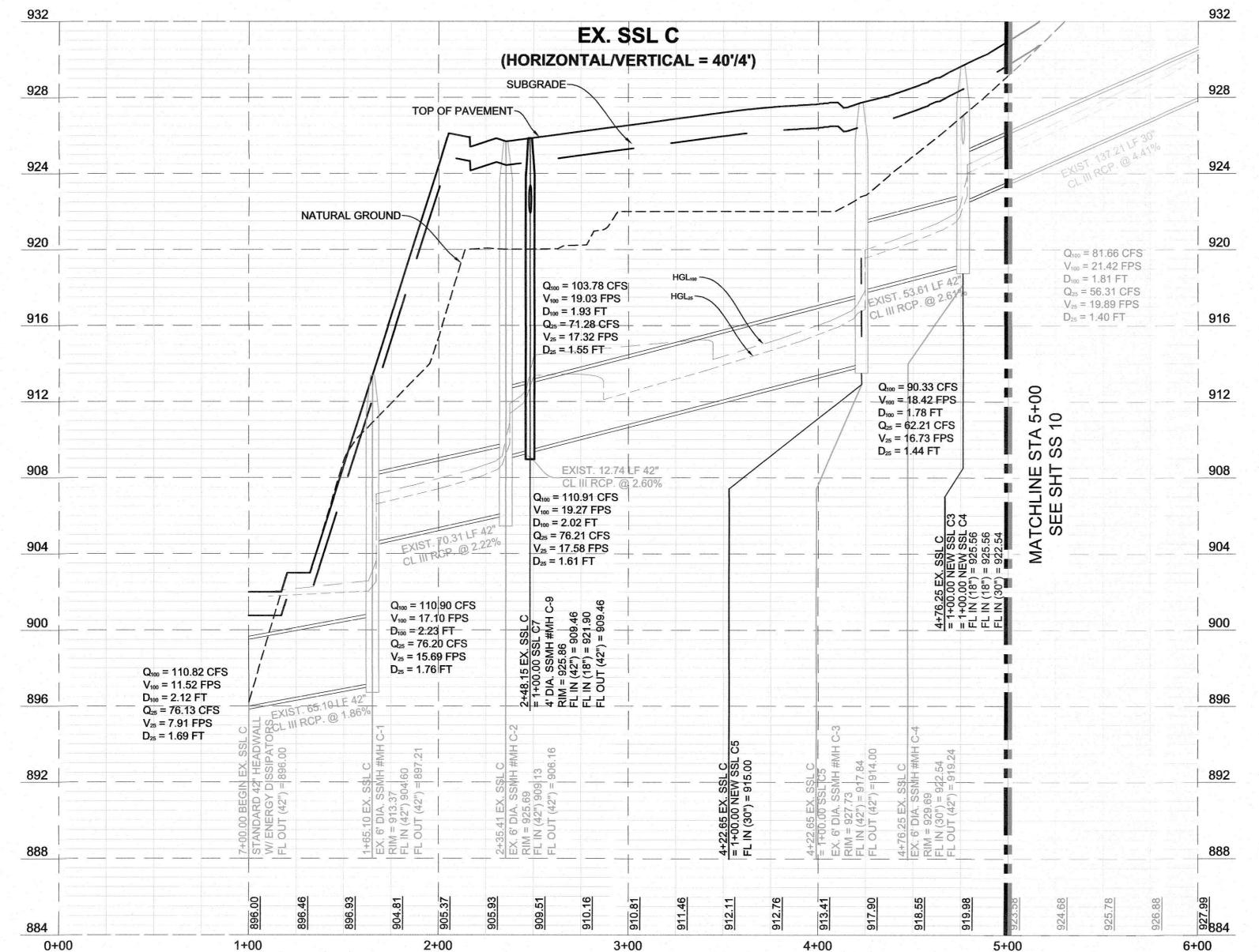
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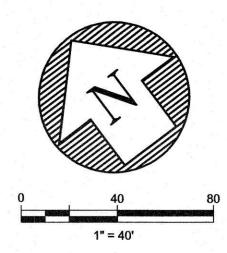
SS 8 SHEET NO.

A311-0415

of 75 sheets



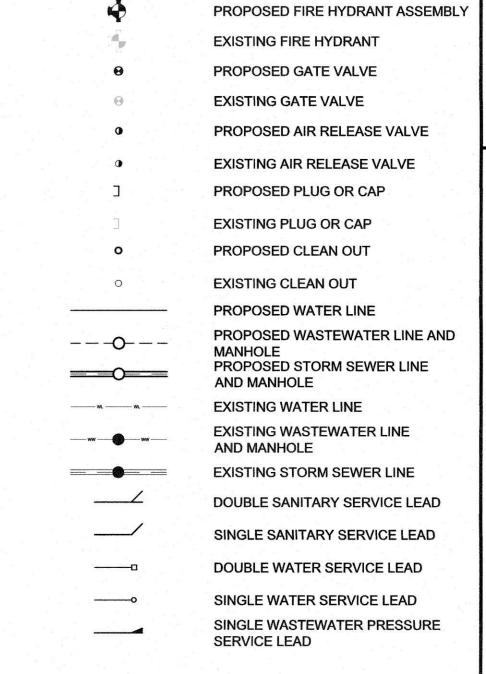




SUBDIVISION 17&8

RA BLUFF (SECTION

LEGEND:



NOTES:

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PROPOSED PAVEMENT

PROFILE LINE LE	EGEND
	100-YR HGL
	25-YR HGL
	NATURAL GROUND

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T. W. HOYSA

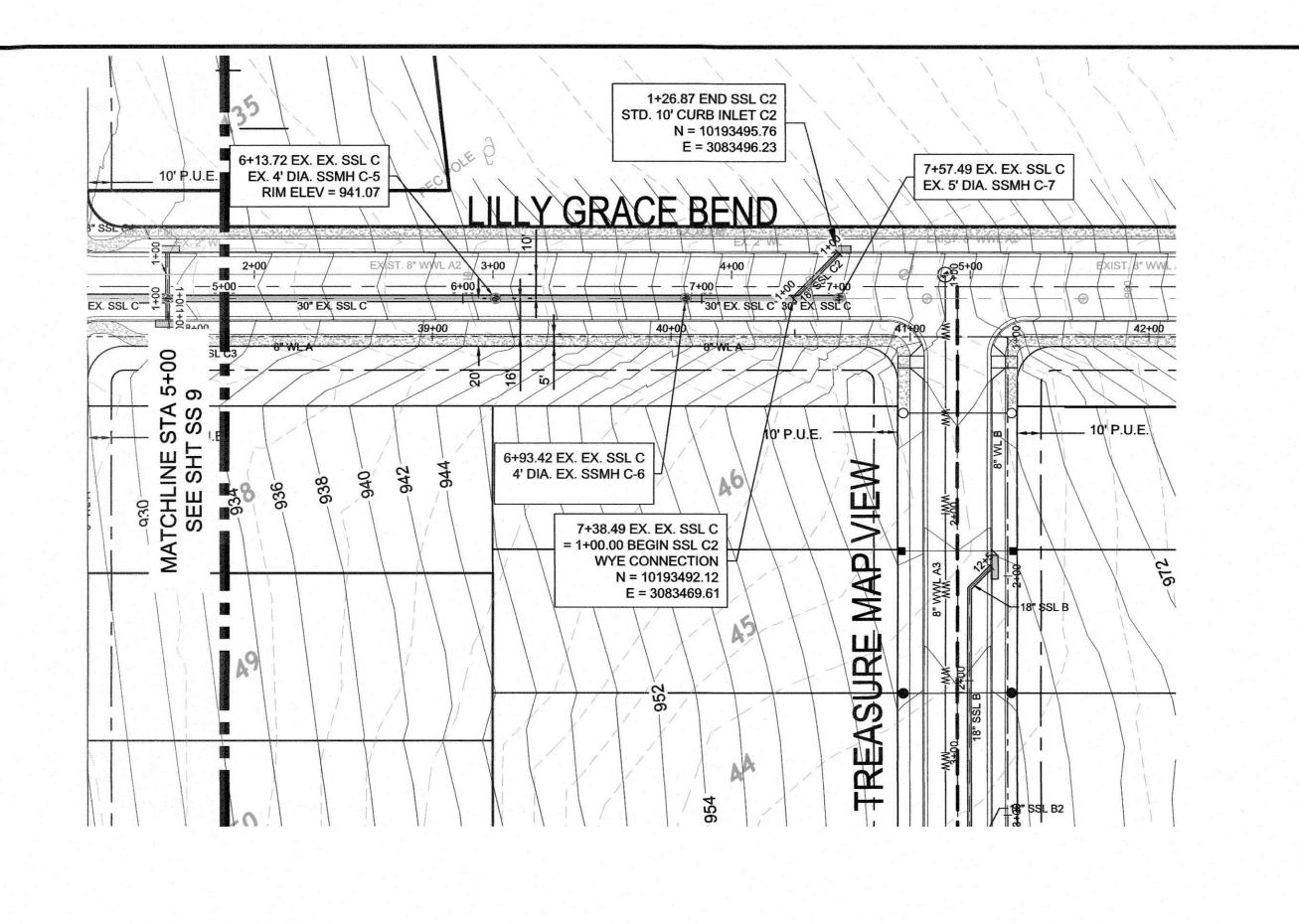
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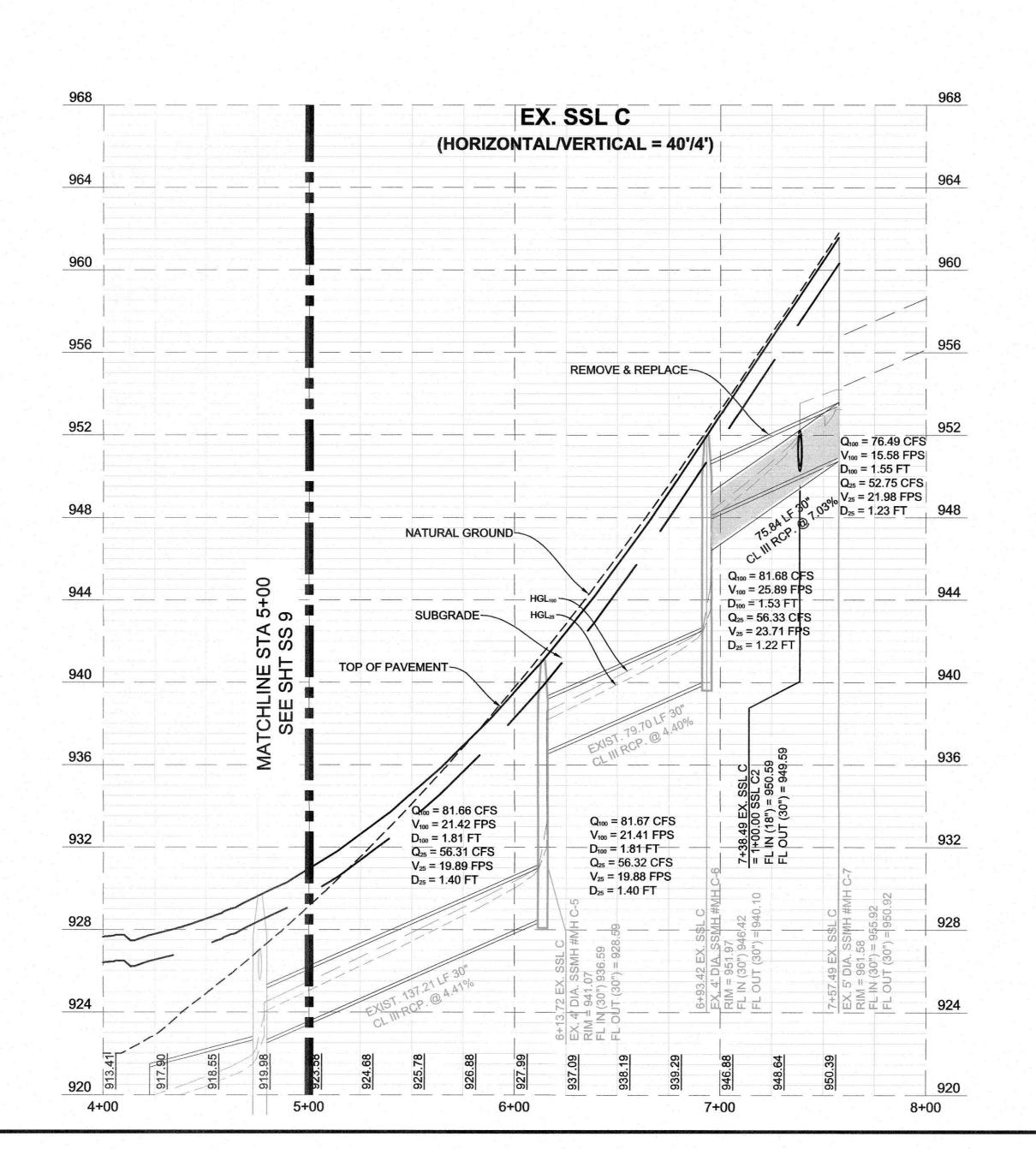
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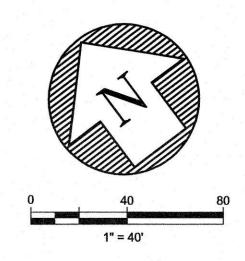
of 75 SHEETS

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1+00 2+00 3+00 4+00 5+00







SUBDIVISION 17&8

RA BLUFF SECTION

LEGEND:

•	PROPOSED FIRE HYDRANT ASSEMBLY
	EXISTING FIRE HYDRANT
•	PROPOSED GATE VALVE
Θ	EXISTING GATE VALVE
0	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
I	PROPOSED PLUG OR CAP
1	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
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	100-YR HGL
	25-YR HGL
	NATURAL GROUND
	PROPOSED PAVEM

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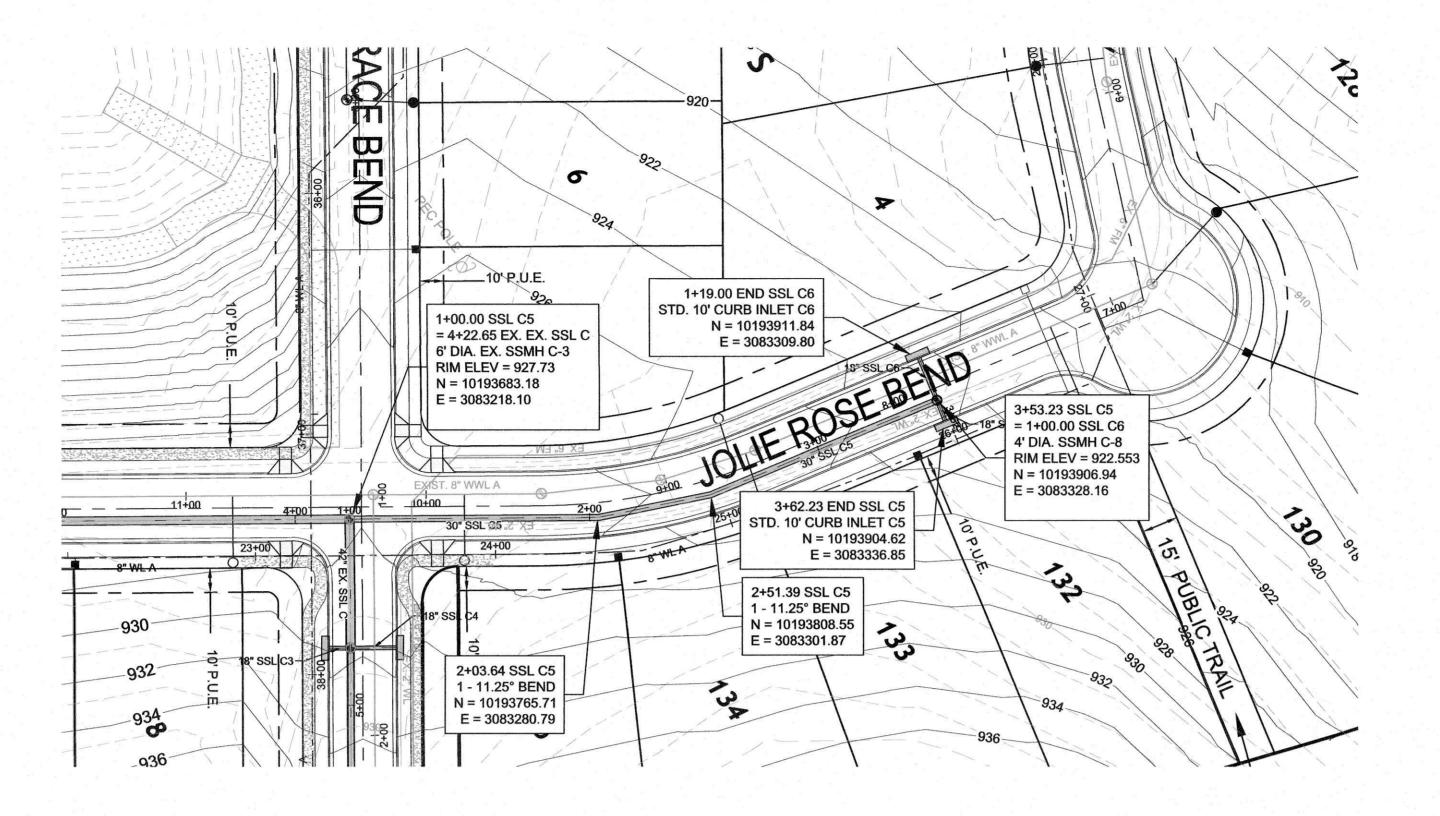
JOB NUMBER: A311-0415

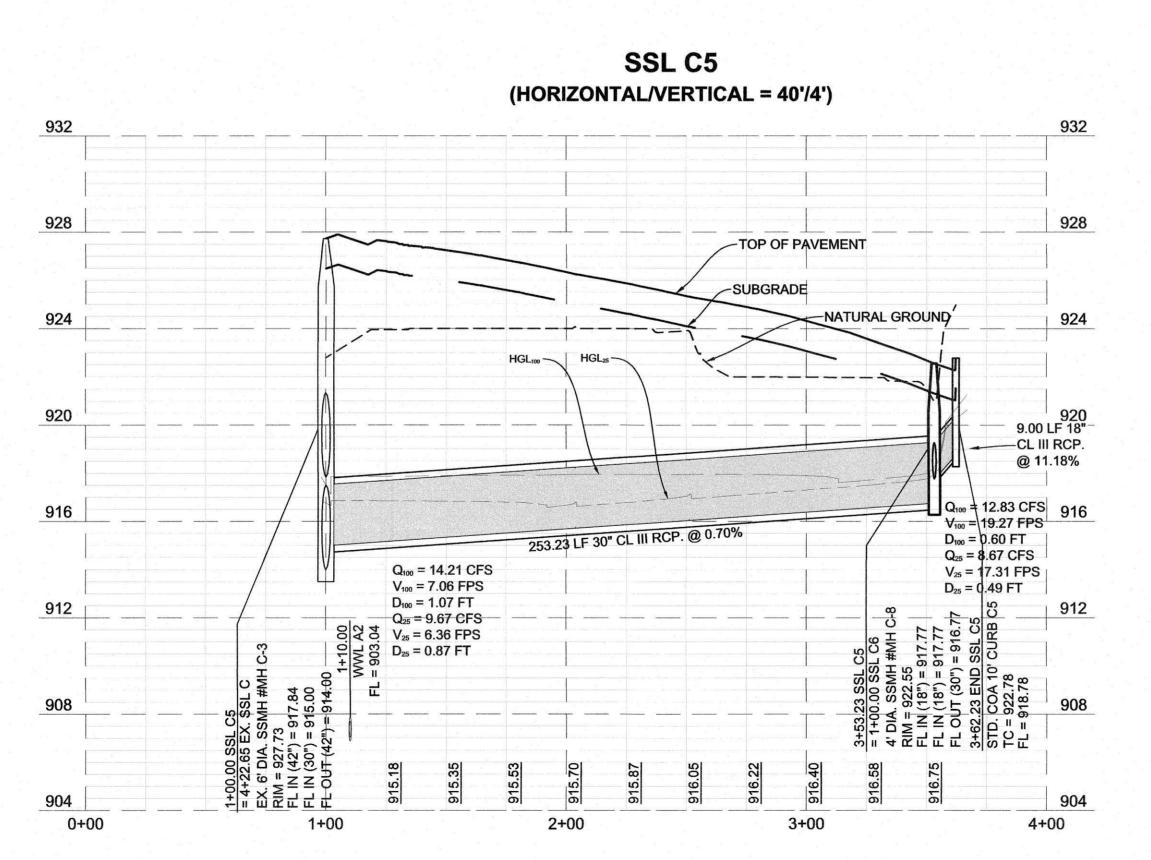
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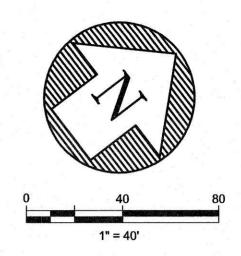
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XXXX-XX-XX

of 75 sheets







RA BLUFF SUBDIVISION SECTION 7 & 8

SEWER LINE C5 PLAN AND STA. 1+00 TO END

STORM

LEGEND:

•	PROPOSED FIRE HYDRANT ASSEMBLY
4	EXISTING FIRE HYDRANT
8	PROPOSED GATE VALVE
9	EXISTING GATE VALVE
•	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
	PROPOSED PLUG OR CAP
	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
O ==O===	PROPOSED WASTEWATER LINE AND MANHOLE PROPOSED STORM SEWER LINE AND MANHOLE
w	EXISTING WATER LINE
	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
	DOUBLE SANITARY SERVICE LEAD
	SINGLE SANITARY SERVICE LEAD
	DOUBLE WATER SERVICE LEAD
.—— o	SINGLE WATER SERVICE LEAD
	SINGLE WASTEWATER PRESSURE SERVICE LEAD

NOTES:

- 1. REFER TO CITY OF LEANDER DETAILS 201-2 FOR INLET CONSTRUCTION.
- 2. REFER TO DETAILS 506S-5, 506S-7, 506S-8, 506S-9, AND/OR 506S-10 FOR MANHOLE CONSTRUCTION. CONTRACTOR MA SUBSTITUTE JUNCTION BOXES FOR RING MANHOLES WITH APPROVAL OF THE ENGINEER.
- 3. PIPE STATIONING AND LINEAR FOOTAGE IS FROM CENTER OF MANHOLE. PIPE SLOPE IS CALCULATED FROM CENTER OF MANHOLE TO CENTER OF MANHOLE.
- 4. INLET CURB TRANSITIONS SHALL BE STANDARD 10' LONG UNLESS OTHERWISE NOTED ON THESE PLANS. THE MINIMUM ALLOWABLE TRANSITION LENGTH SHALL BE 5.0

PROFILE LINE LI	EGEND
	100-YR HGL
	25-YR HGL
	NATURAL GROUND
	PROPOSED PAVEME

LOCATION OF EXISTING UNDERGROUND AND OVERHEAD UTILITIES ARE APPROXIMATE LOCATIONS ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL **EXISTING UTILITIES PRIOR TO** BEGINNING WORK AND SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT OCCUR.



JOB NUMBER:

A311-0415

T. W. HOYSA

SHEET NO. 40 of 75 SHEETS

SS 11

Know what's below.
Call before you dig.



FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT OCCUR.

JOB NUMBER: A311-0415

SHEET NO.

SS 12

of 75 SHEETS

SSL C7 SSL C3 SSL C4 SSL C6 (HORIZONTAL/VERTICAL = 40'/4') PROFILE LINE LEGEND 964 TOP OF PAVEMENT----- 25-YR HGL TOP OF PAVEMENT NATURAL GROUND HGL₂₅ -SUBGRADE PROPOSED PAVEMENT -NATURAL GROUNE NATURAL GROUND— 928 HGL₂₅ HGL100 -NATURAL GROUND-

 $Q_{100} = 5.86 \text{ CFS}$ $V_{100} = 8.39 \text{ FPS}$ $D_{100} = 0.63 \text{ FT}$ $Q_{25} = 3.93 \text{ CFS}$ $V_{25} = 7.52 \text{ FPS}$ $D_{25} = 0.51 \text{ FT}$ 19.00 LF 18" —CL III RCP. @ 2.00% 920 920 916

1+00

1+50

SSL C2

 $\begin{array}{c} Q_{100} = 3.16 \text{ CFS} \\ V_{100} = 9.11 \text{ FPS} \\ D_{100} = 0.38 \text{ FT} \\ Q_{25} = 2.24 \text{ CFS} \\ V_{25} = 8.25 \text{ FPS} \\ \end{array}$

920

0+50

1+00

 $D_{25} = 0.32 \, \text{FT} /$

9.00 LF 18" —CL III RCP.

@ 4.22%

1+50

920

0+50

TOP OF PAVEMENT-

SUBGRADE-

 $Q_{100} = 5.19 \text{ CFS}$ $V_{100} = 20.59 \text{ FPS}$ $D_{100} = 0.30 \text{ FT}$ $Q_{25} = 3.58 \text{ CFS}$

 $V_{25} = 18.48 \text{ FPS}$ $D_{25} = 0.25 \text{ FT}$

0+50

26.87 LF 18" —CL III RCP. @ 26.86%

1+50

1+00

952

960 NATURAL GROUND—

 $Q_{100} = 1.77 \text{ CFS}$ $V_{100} = 8.44 \text{ FPS}$ $D_{100} = 0.26 \text{ FT}$ $Q_{25} = 1.33 \text{ CFS}$ $V_{25} = 7.74 \text{ FPS}$ $Q_{25} = 0.23 \text{ FT}$ 19.00 LF 18" CL III RCP. 912 1+19.00 END SSL STD. COA 10' CUR TC = 922.78 FL = 918.78 908 0+50 1+00 1+50

9.00 LF 18" CL III RCP. @ 2.00% $Q_{100} = 8.47 \text{ CFS}$ $V_{100} = 8.68 \text{ FPS}$ $D_{100} = 0.81 \text{ FT}$ $Q_{25} = 6.01 \text{ CFS}$ 916 V₂₅ = 7.96 FPS D₂₅ = 0.66 FT 916 912 912 908

1+00

1+50

FEET.

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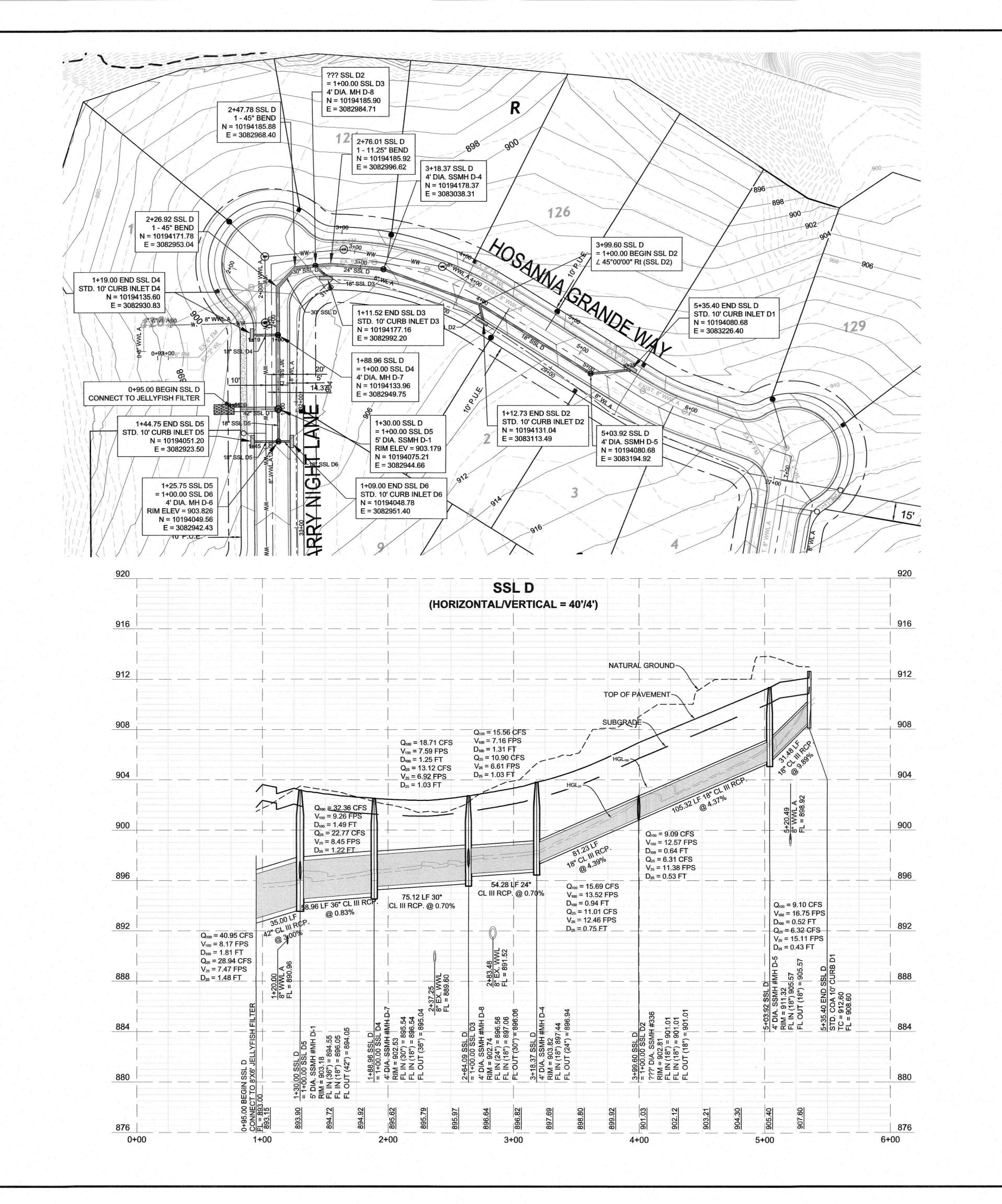
3. PIPE STATIONING AND LINEAR FOOTAGE IS FROM CENTER OF MANHOLE. PIPE SLOPE IS CALCULATED FROM CENTER

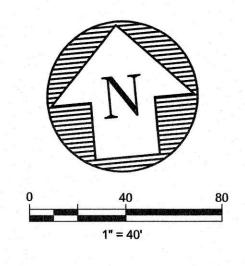
OF MANHOLE TO CENTER OF MANHOLE. 4. INLET CURB TRANSITIONS SHALL BE STANDARD 10' LONG UNLESS OTHERWISE NOTED ON THESE PLANS. THE

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MINIMUM ALLOWABLE TRANSITION LENGTH SHALL BE 5.0





LEGEND:

•	PROPOSED FIRE HYDRANT ASSEMBLY
	EXISTING FIRE HYDRANT
€	PROPOSED GATE VALVE
9	EXISTING GATE VALVE
0	PROPOSED AIR RELEASE VALVE
0	EXISTING AIR RELEASE VALVE
	PROPOSED PLUG OR CAP
j	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE
	PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
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	SINGLE SANITARY SERVICE LEAD
	DOUBLE WATER SERVICE LEAD
	SINGLE WATER SERVICE LEAD
	SINGLE WASTEWATER PRESSURE SERVICE LEAD

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PROFILE LINE L	EGEND
	100-YR HGL
	25-YR HGL
	NATURAL GROUND
	PROPOSED PAVEMEN

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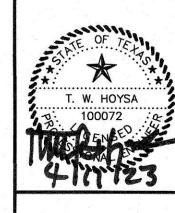
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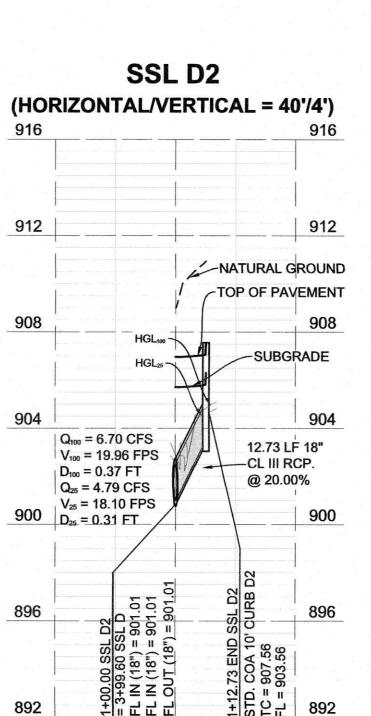
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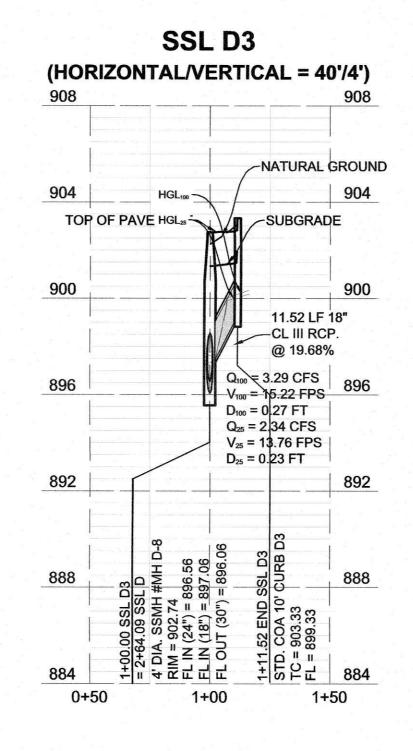
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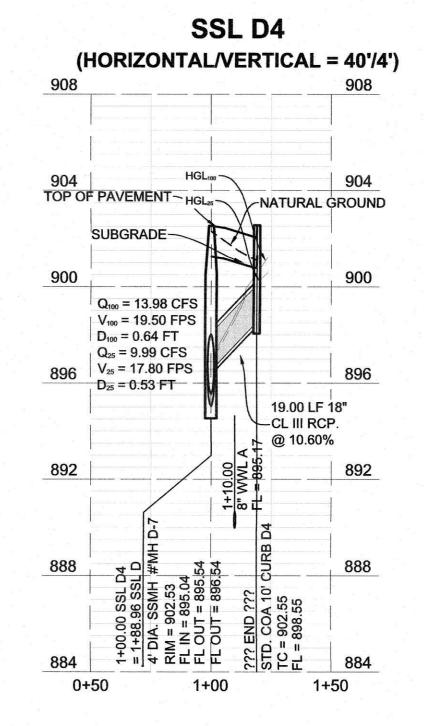
SUBDIVISION 17 & 8

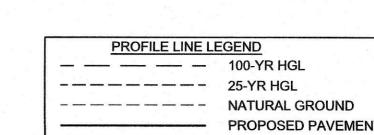
BLUFF











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	RE	DESCRIPTIC		
OF LEANDER DETAILS 201-2 FOR INLET	CHARLE AT A TAKEN THE PROPERTY OF THE PARTY	0.0		50.00

SUBDIVISION 17&8



Phone 512.439.470 Fax 512.439.471 FRN - F-138

Engineering, Inc.

a Frontera Blvd
150
I Rock, TX 78681

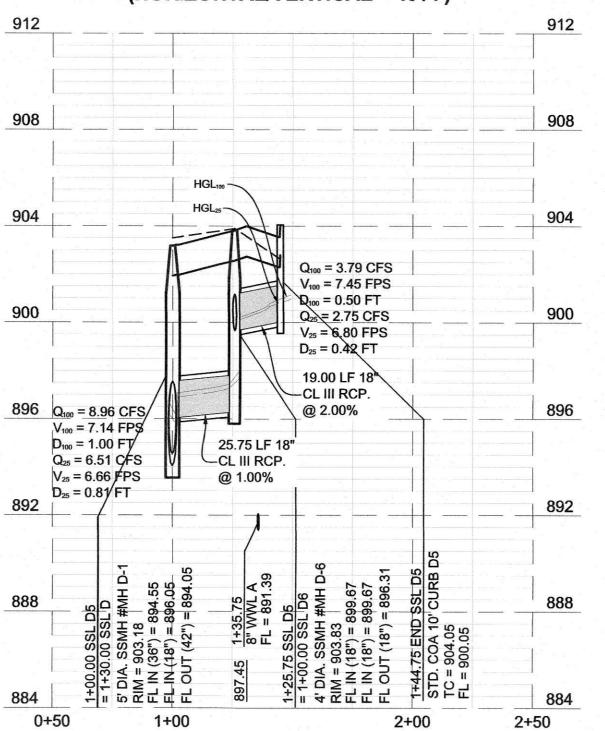
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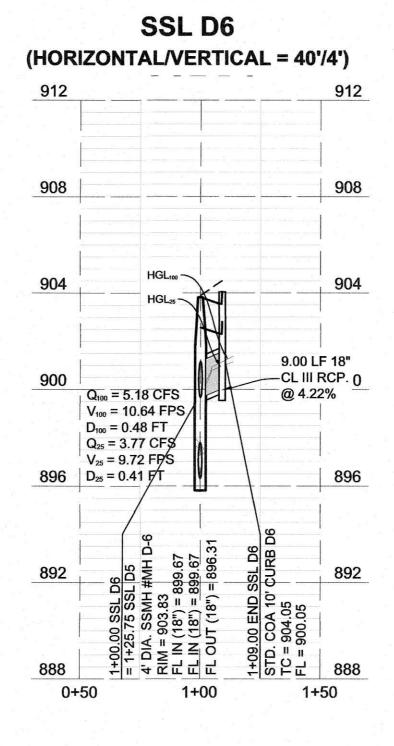
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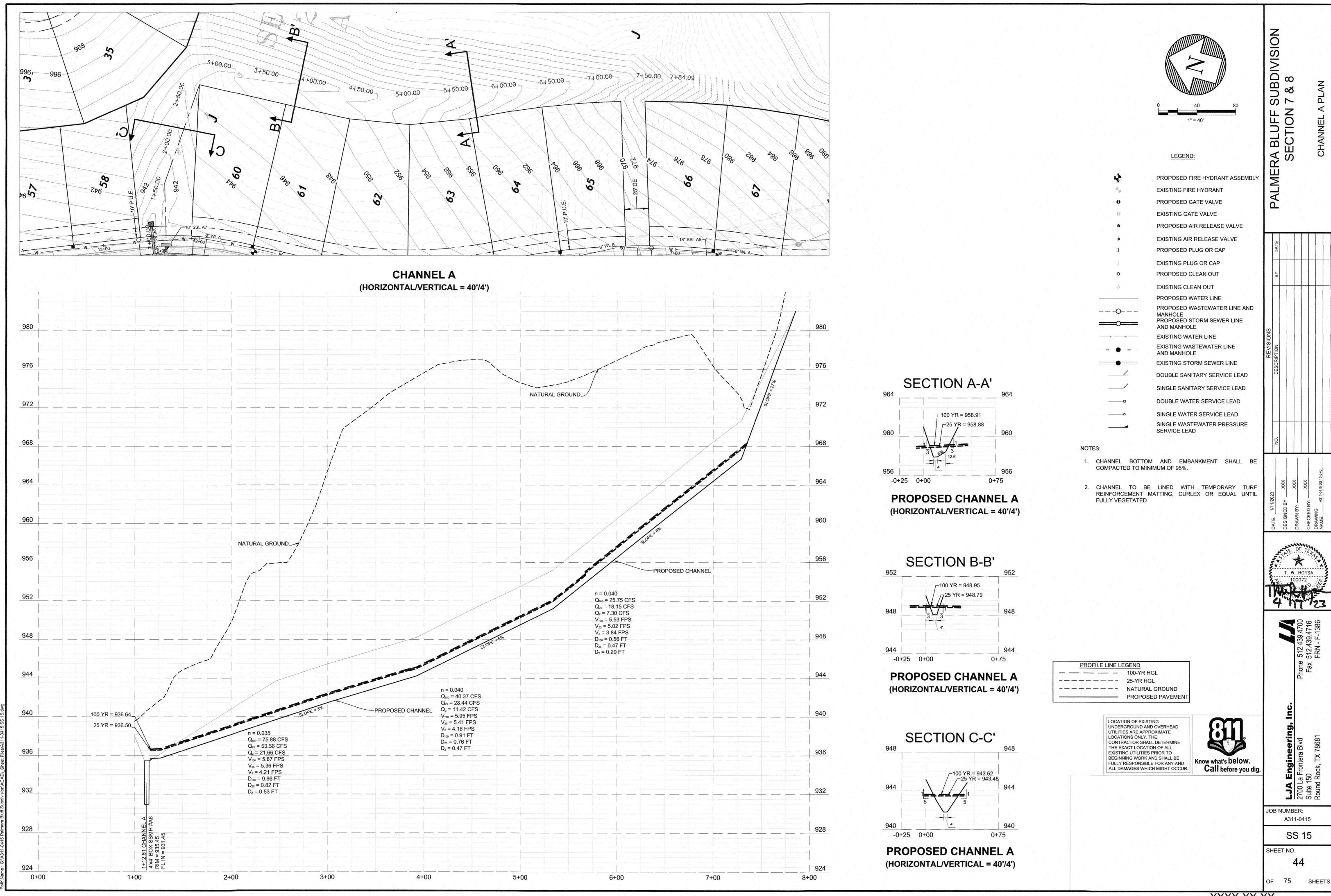
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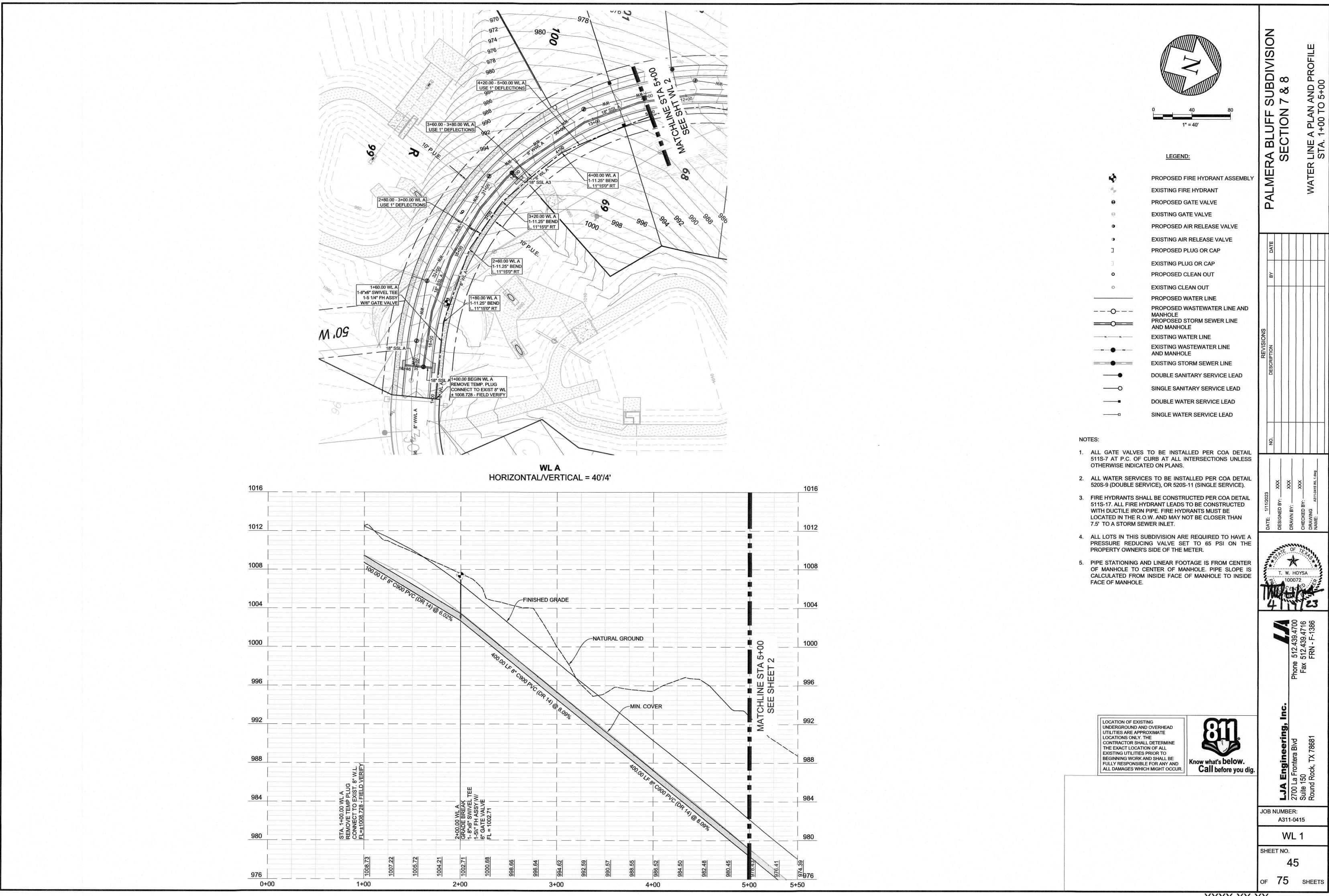
SSL D5 (HORIZONTAL/VERTICAL = 40'/4')

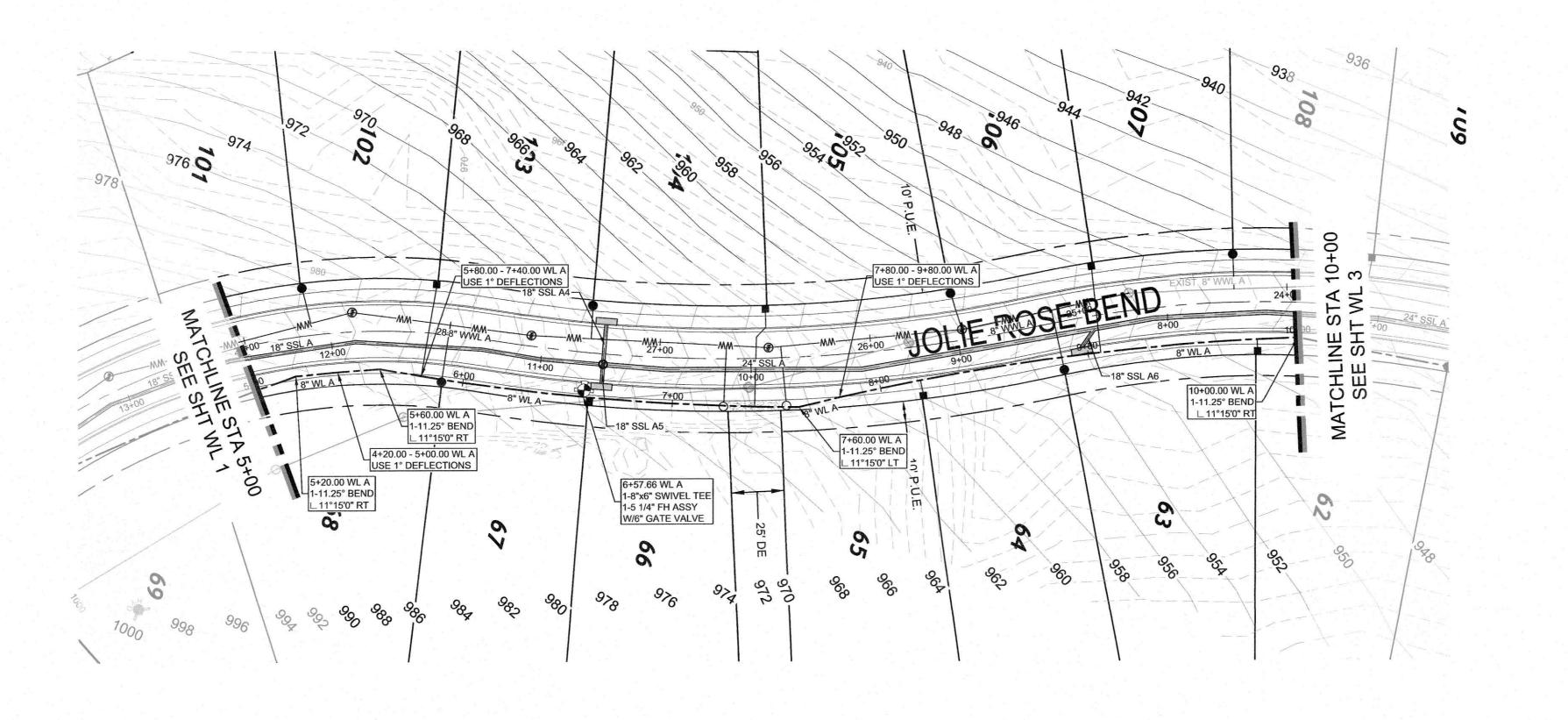


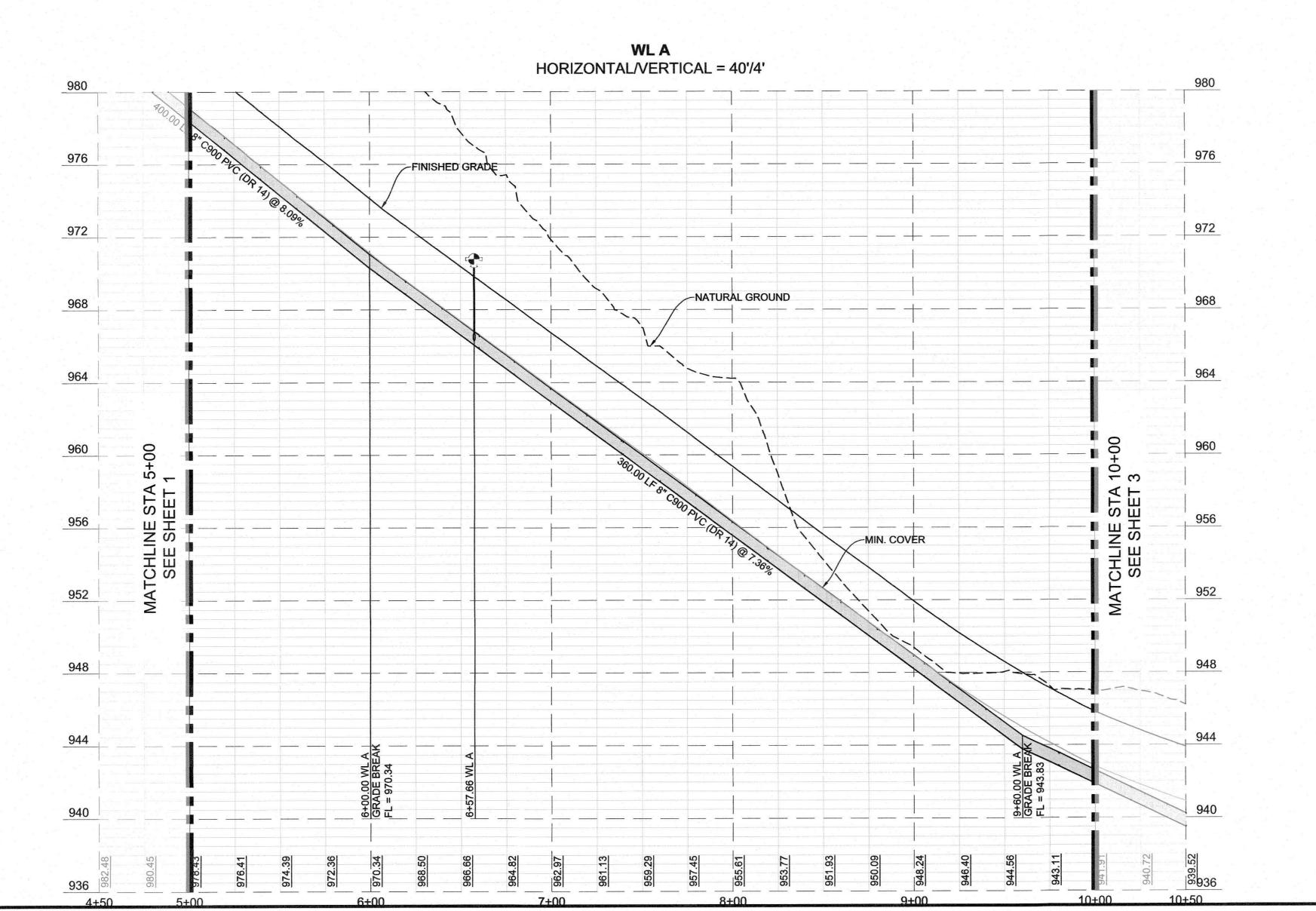


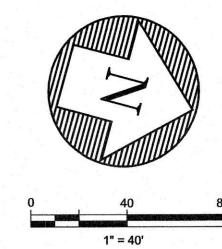
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PALMERA BLUFF SUBDIVISION SECTION 7 & 8

LEGEND:

PROPOSED FIRE HYDRANT ASSEMBLY

Y	
4	EXISTING FIRE HYDRANT
Θ	PROPOSED GATE VALVE
8	EXISTING GATE VALVE
•	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
.	PROPOSED PLUG OR CAP
	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE PROPOSED STORM SEWER LINE
	AND MANHOLE
	EXISTING WATER LINE
	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
-	DOUBLE SANITARY SERVICE LEAD
 0	SINGLE SANITARY SERVICE LEAD
	DOUBLE WATER SERVICE LEAD
	SINGLE WATER SERVICE LEAD

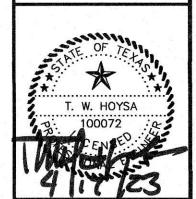
NOTES:

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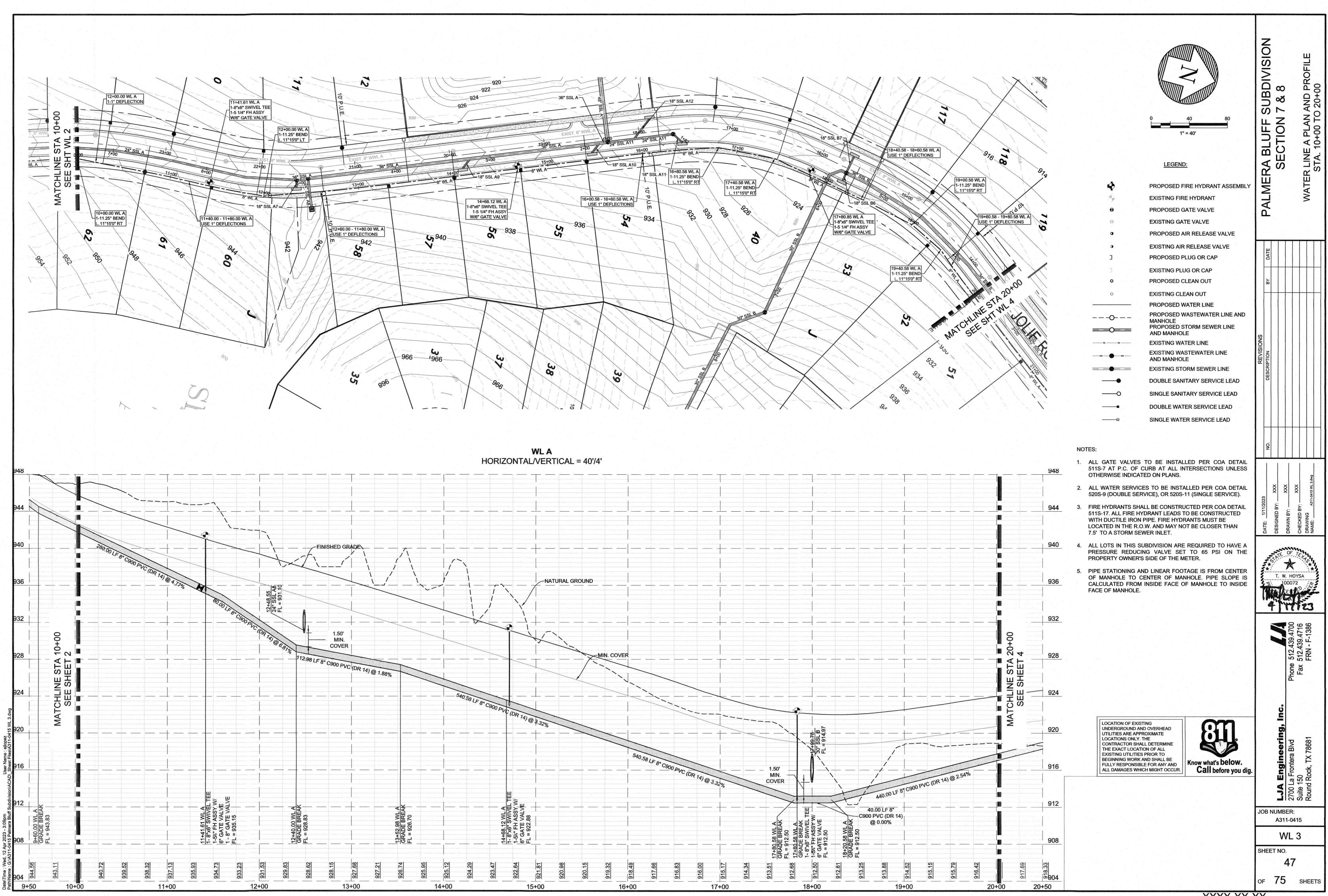


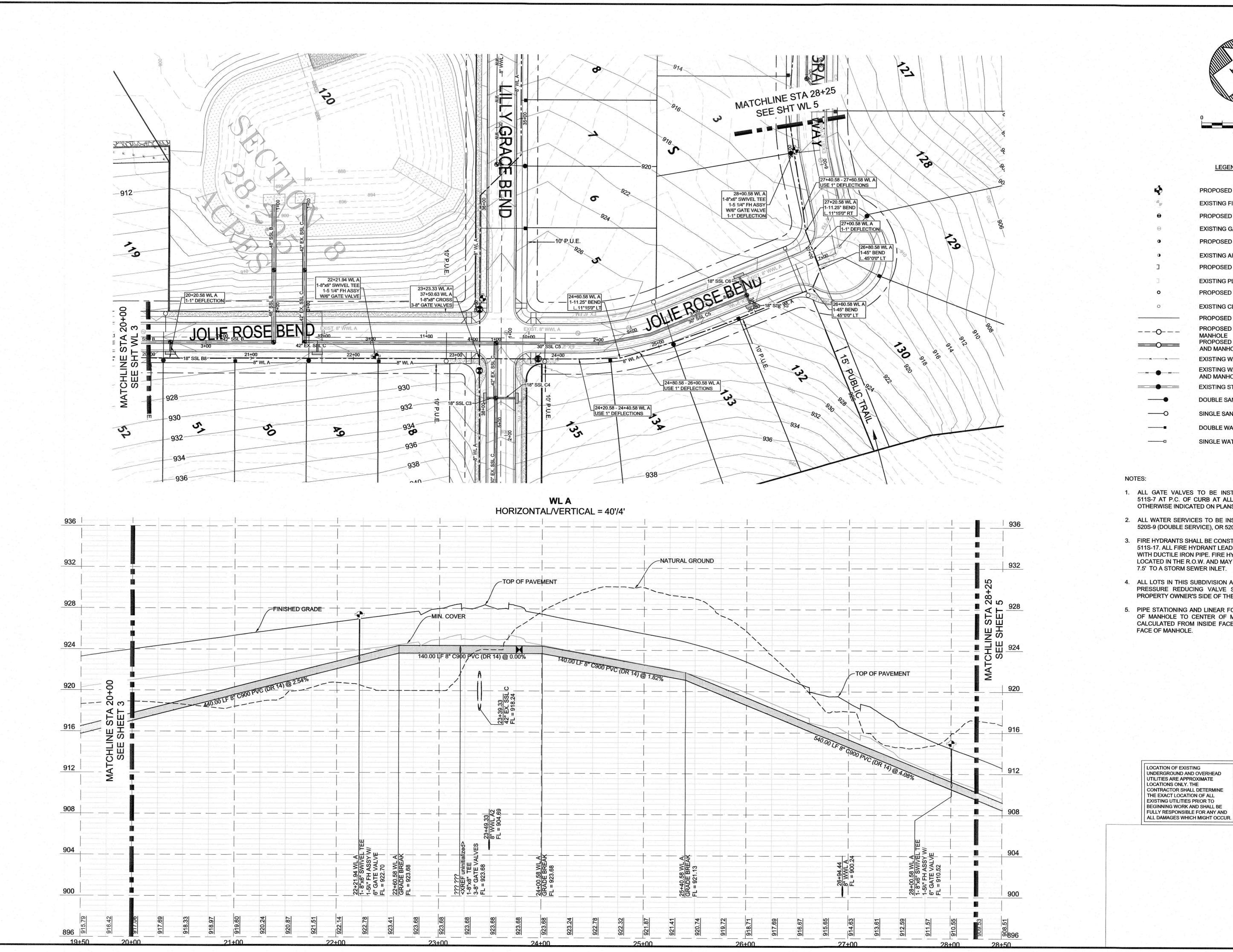
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Call before you dig.

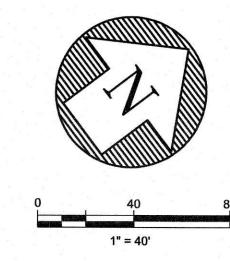
JOB NUMBER: A311-0415

SHEET NO.

WL 2







SUBDIVISION

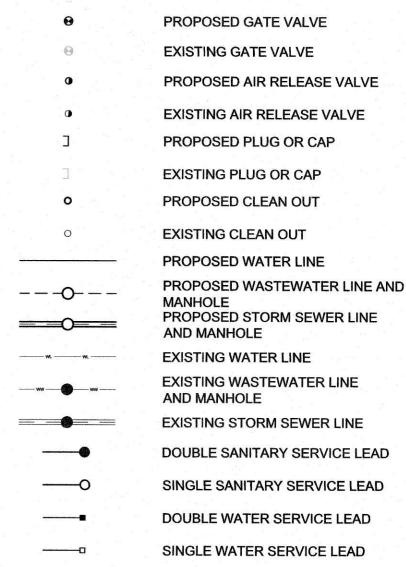
ERA BLUFF SECTION

ATER LINE A PLAN AND PF STA. 20+00 TO 28+25

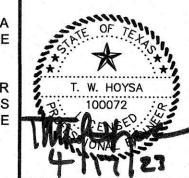
LEGEND:

EXISTING FIRE HYDRANT

PROPOSED FIRE HYDRANT ASSEMBLY



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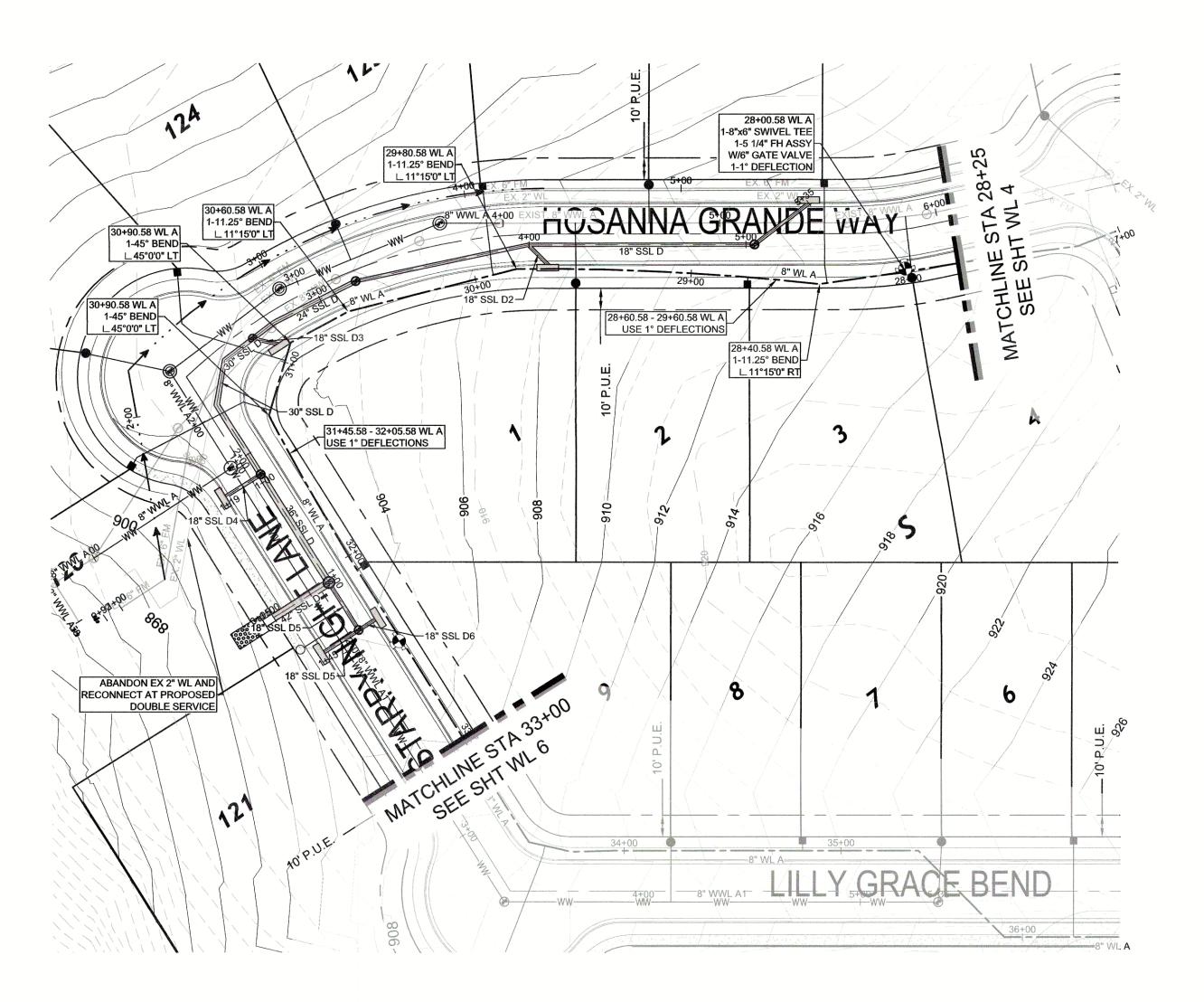
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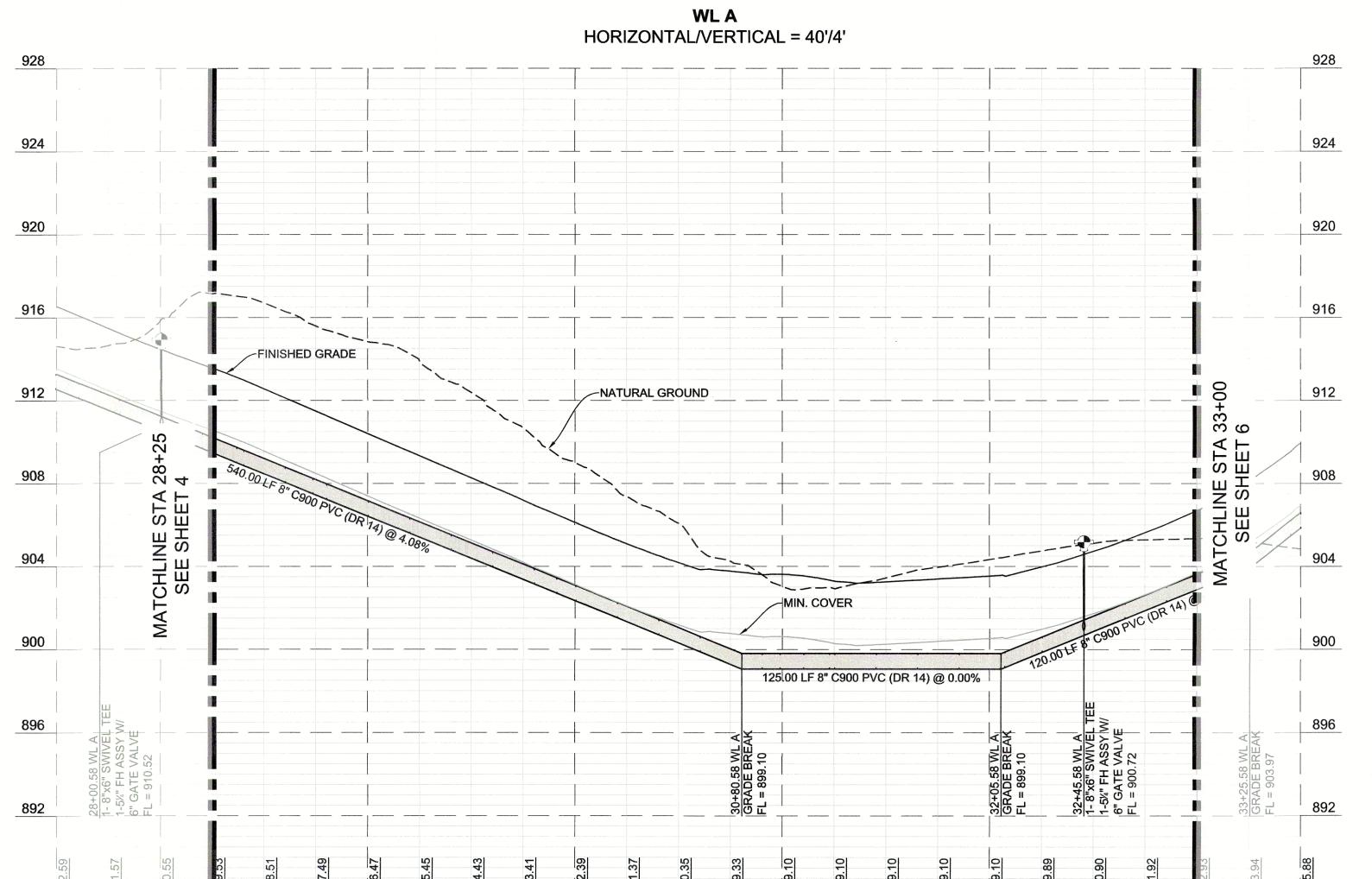
WL 4 SHEET NO.

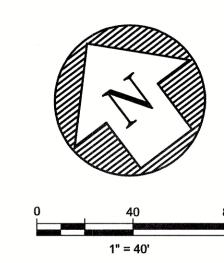
of 75 SHEETS

Know what's below.

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PALMERA BLUFF SUBDIVISION SECTION 7 & 8

.	
	PROPOSED FIRE HYDRANT ASSEMBLY
A W	EXISTING FIRE HYDRANT
8	PROPOSED GATE VALVE
0	EXISTING GATE VALVE
•	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
3	PROPOSED PLUG OR CAP
	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE
	PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
	EXISTING WASTEWATER LINE AND MANHOLE
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	DOUBLE SANITARY SERVICE LEAD
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LOCATION OF EXISTING

LOCATIONS ONLY. THE

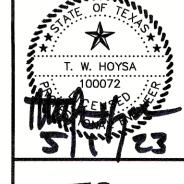
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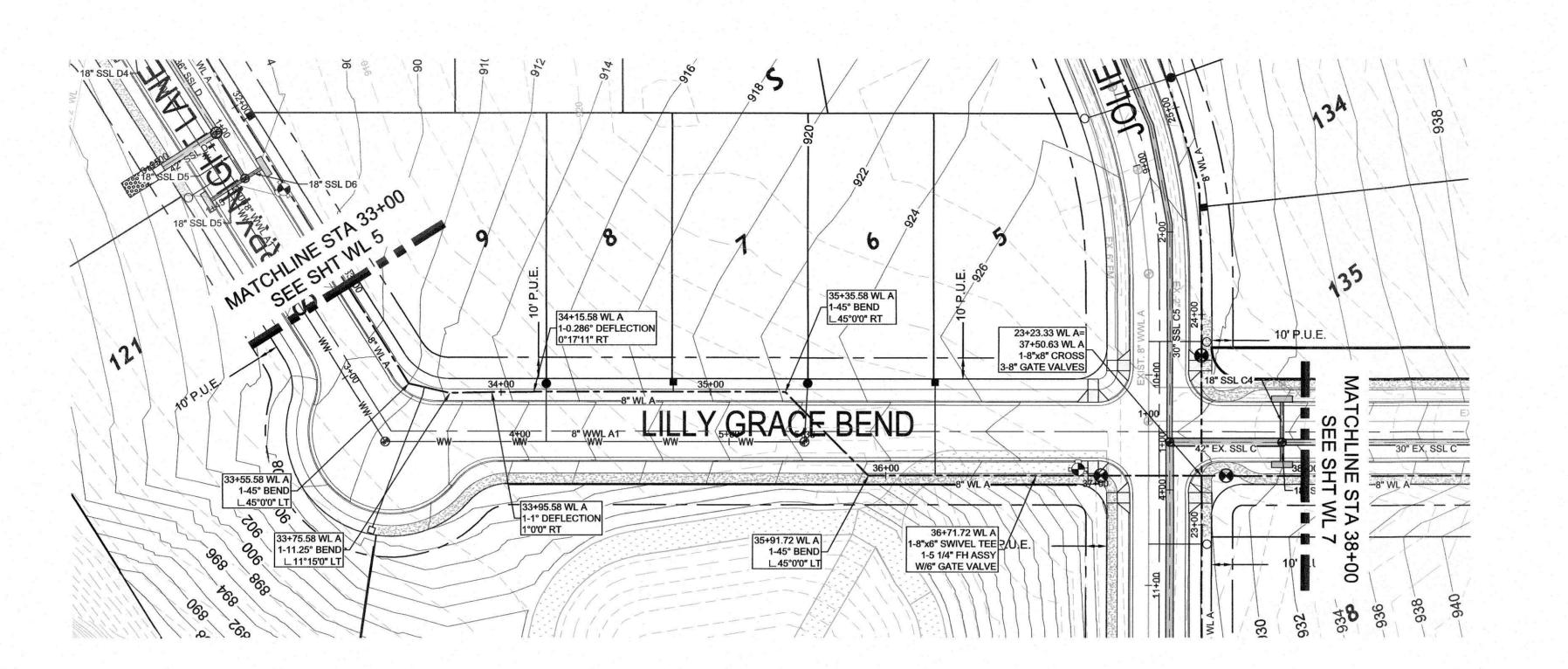
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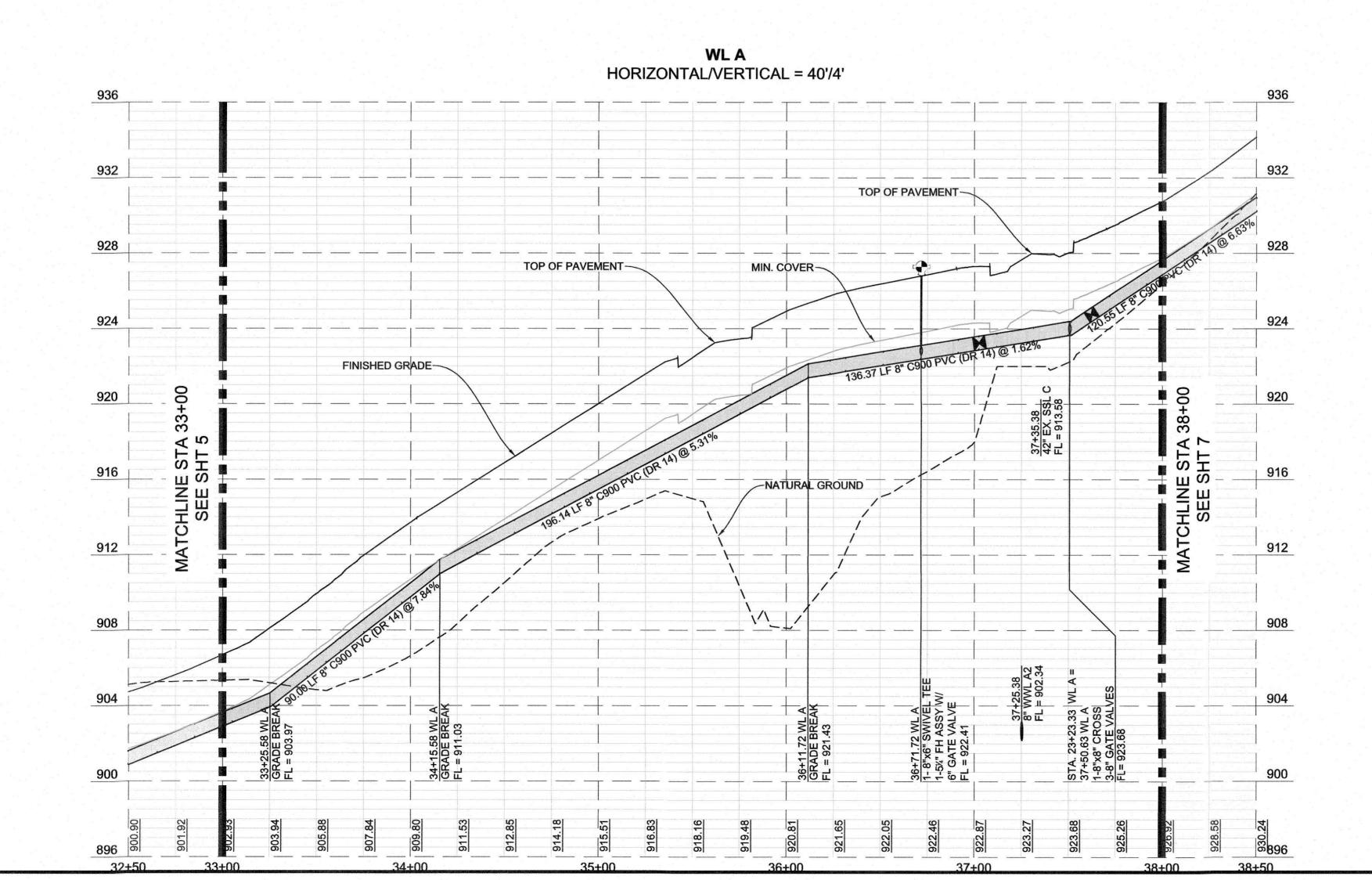
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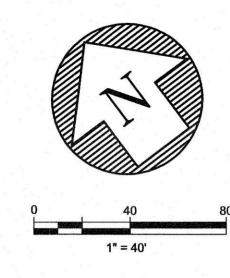
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WL 5 SHEET NO.

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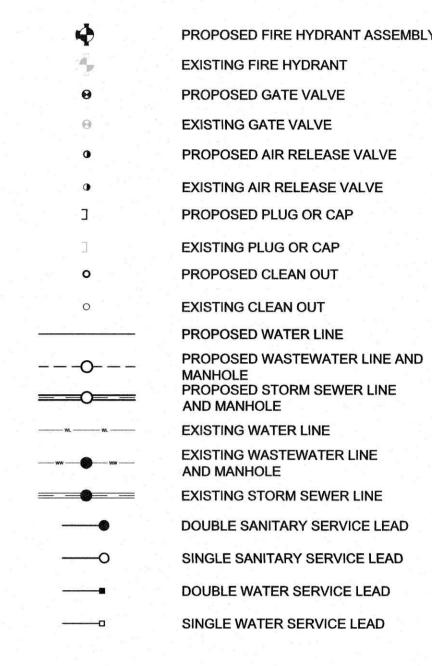




SUBDIVISION 17 & 8

PALMERA BLUFF S SECTION 7

LEGEND:



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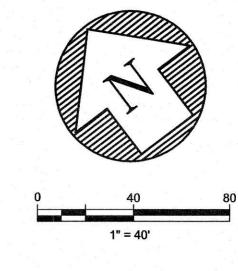
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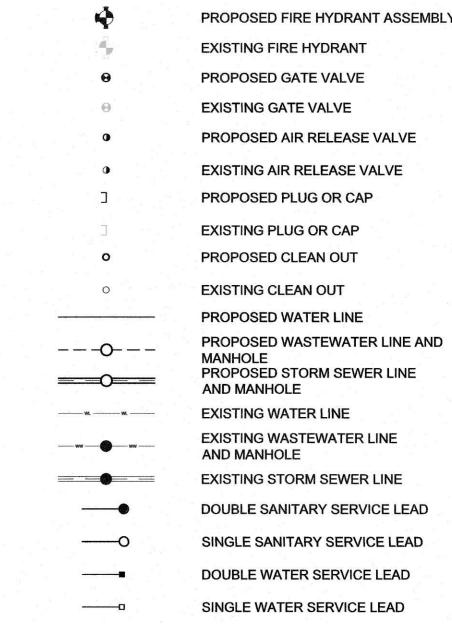
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RA BLUFF SUBDIVISION SECTION 7 & 8

LINE A PLAN AND PROF STA. 38+00 TO END

LEGEND:



NOTES:

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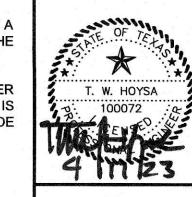
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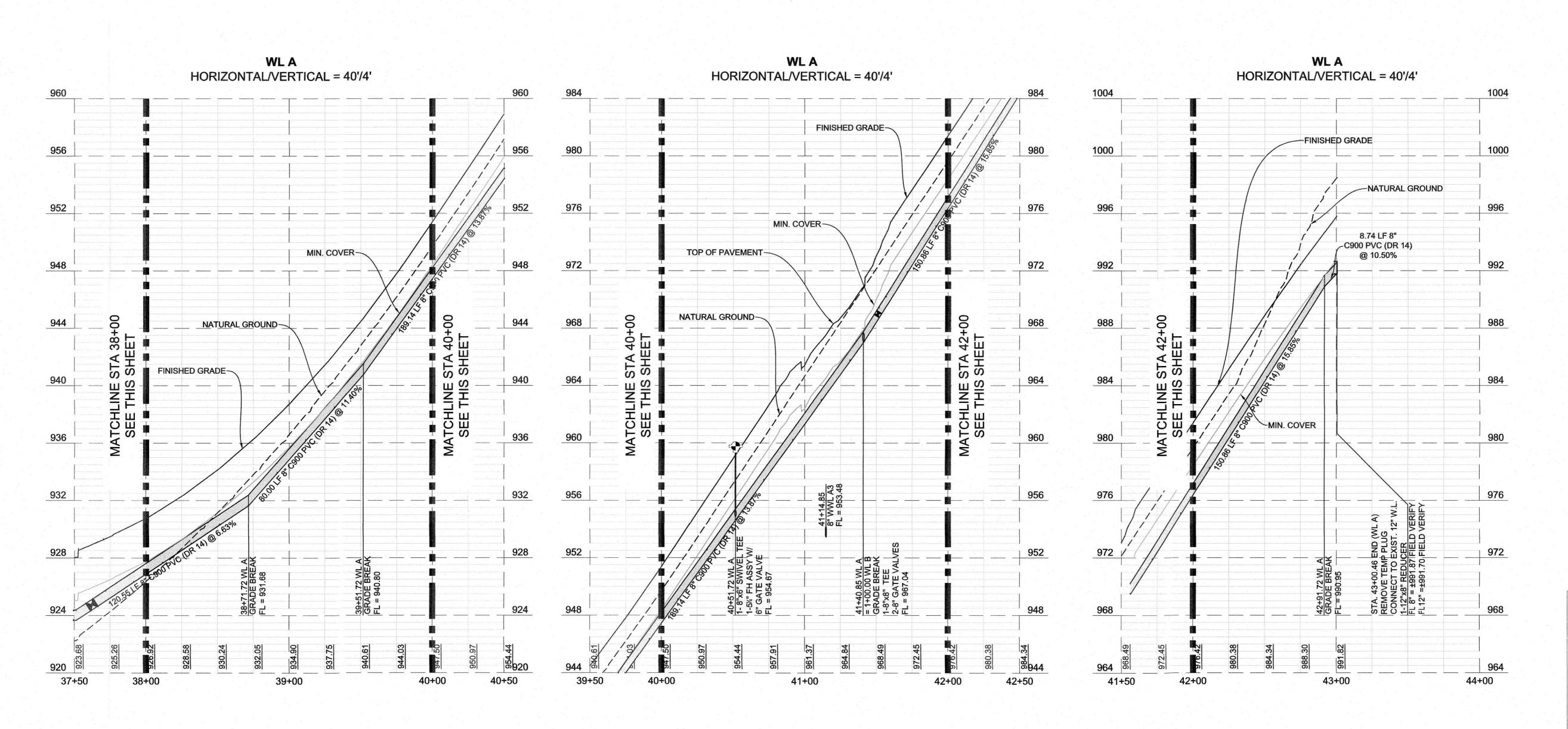


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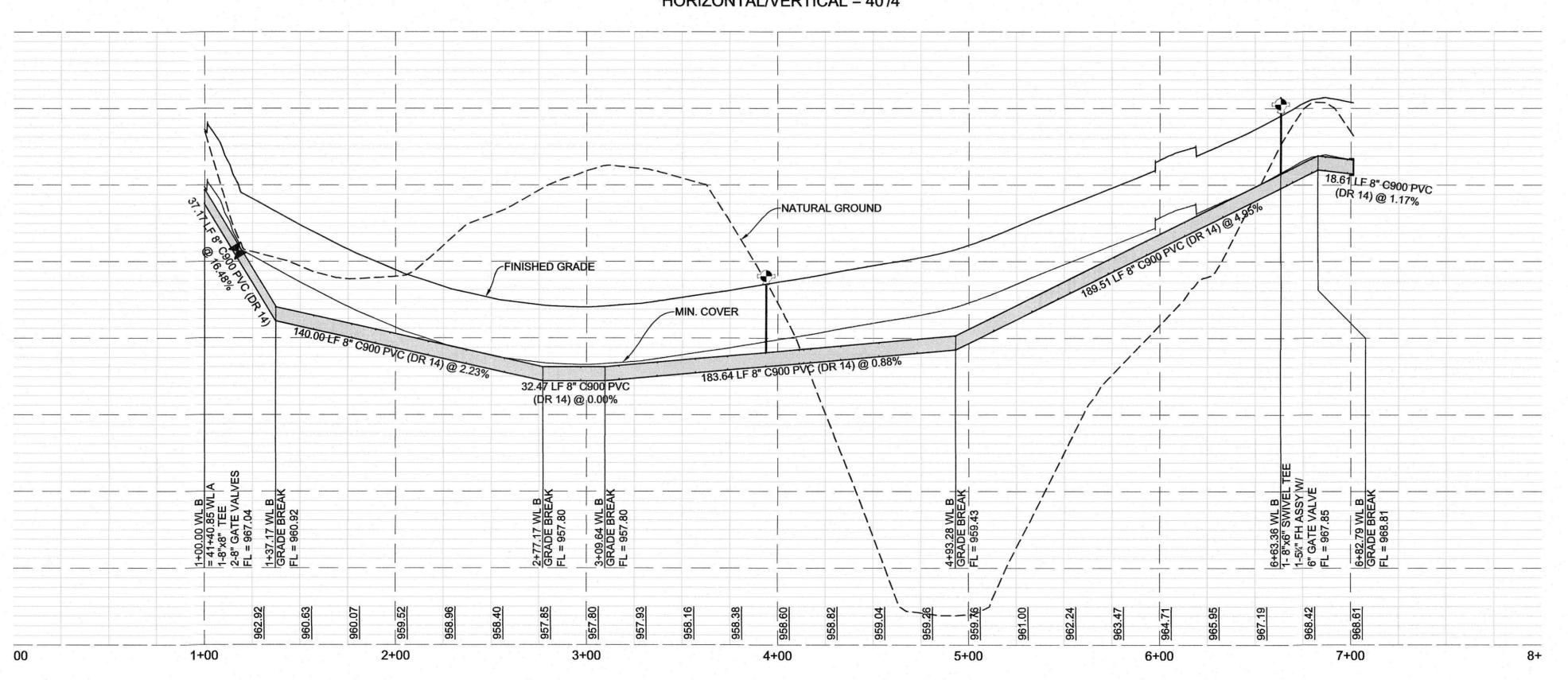
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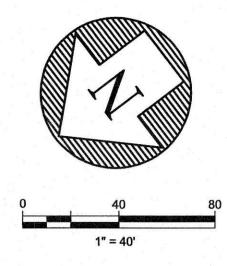
> WL 7 SHEET NO.

40+51.72 WL A 1-8"x6" SWIVEL TEE 43+00.46 END WL A REMOVE TEMP. PLUG 1-12"x8" REDUCER CONNECT TO EXIST 12" WL 41+40.85 WL A= 1+00.00 BEGIN WL B 1-8"x8" TEE 1-5 1/4" FH ASSY W/6" GATE VALVE









SUBDIVISION 17 & 8

PALMERA BLUFF S SECTION 7

WATER LINE B PLAN AND PROF STA. 1+00 TO END

LEGEND:

•	PROPOSED FIRE HYDRANT ASSEM
4	EXISTING FIRE HYDRANT
9	PROPOSED GATE VALVE
Θ	EXISTING GATE VALVE
0	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
I	PROPOSED PLUG OR CAP
	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
-	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AI MANHOLE PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
-	DOUBLE SANITARY SERVICE LEAD
	SINGLE SANITARY SERVICE LEAD
	DOUBLE WATER SERVICE LEAD
	SINGLE WATER SERVICE LEAD

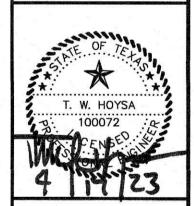
- 1. ALL GATE VALVES TO BE INSTALLED PER COA DETAIL 511S-7 AT P.C. OF CURB AT ALL INTERSECTIONS UNLESS OTHERWISE INDICATED ON PLANS.
- ALL WATER SERVICES TO BE INSTALLED PER COA DETAIL 520S-9 (DOUBLE SERVICE), OR 520S-11 (SINGLE SERVICE).
- 3. FIRE HYDRANTS SHALL BE CONSTRUCTED PER COA DETAIL 511S-17. ALL FIRE HYDRANT LEADS TO BE CONSTRUCTED WITH DUCTILE IRON PIPE. FIRE HYDRANTS MUST BE LOCATED IN THE R.O.W. AND MAY NOT BE CLOSER THAN 7.5' TO A STORM SEWER INLET.
- 4. ALL LOTS IN THIS SUBDIVISION ARE REQUIRED TO HAVE A PRESSURE REDUCING VALVE SET TO 65 PSI ON THE PROPERTY OWNER'S SIDE OF THE METER.
- 5. PIPE STATIONING AND LINEAR FOOTAGE IS FROM CENTER OF MANHOLE TO CENTER OF MANHOLE. PIPE SLOPE IS CALCULATED FROM INSIDE FACE OF MANHOLE TO INSIDE FACE OF MANHOLE.

LOCATION OF EXISTING

UNDERGROUND AND OVERHEAD UTILITIES ARE APPROXIMATE LOCATIONS ONLY. THE

CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES PRIOR TO BEGINNING WORK AND SHALL BE FULLY RESPONSIBLE FOR ANY AND

ALL DAMAGES WHICH MIGHT OCCUR.



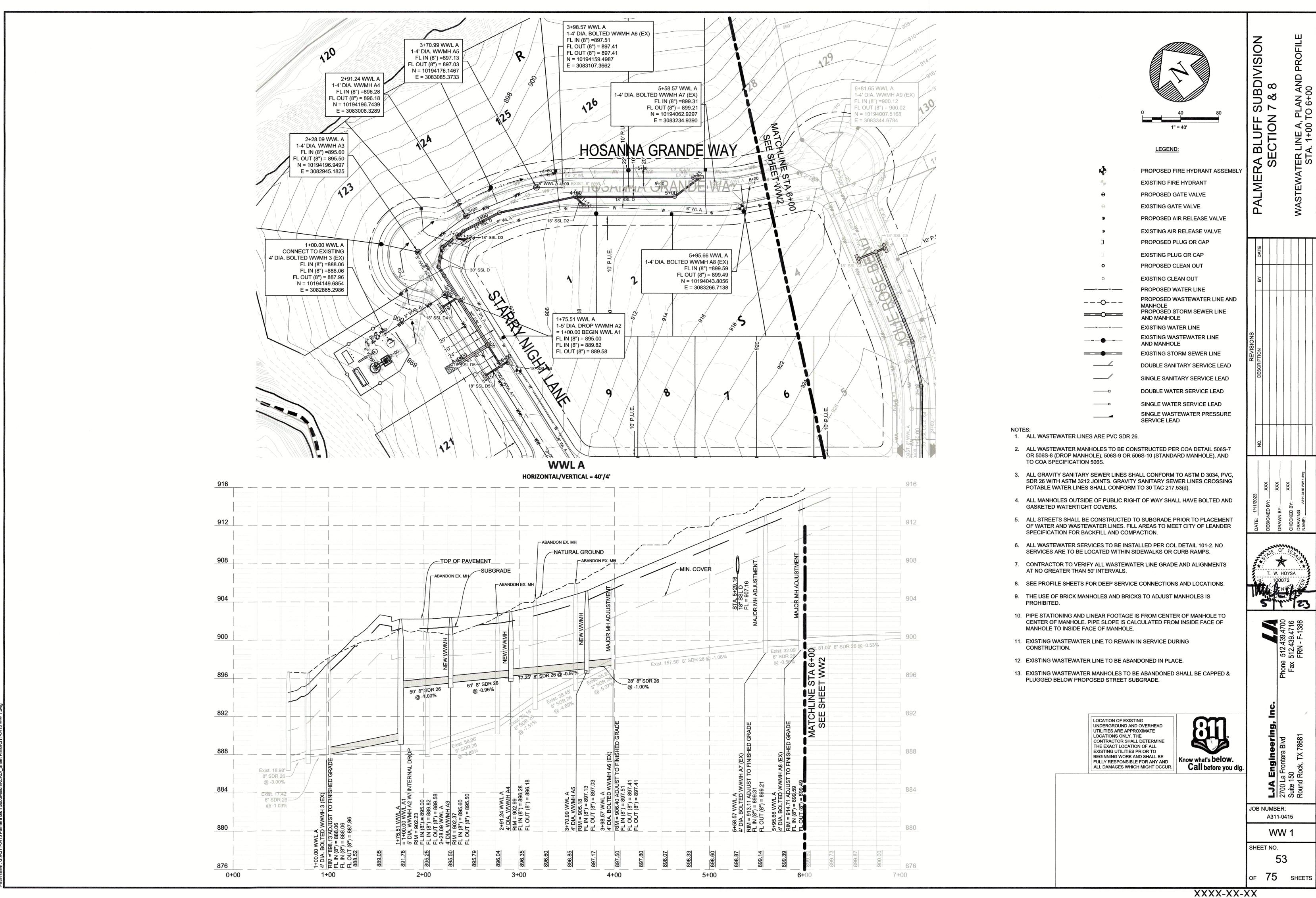


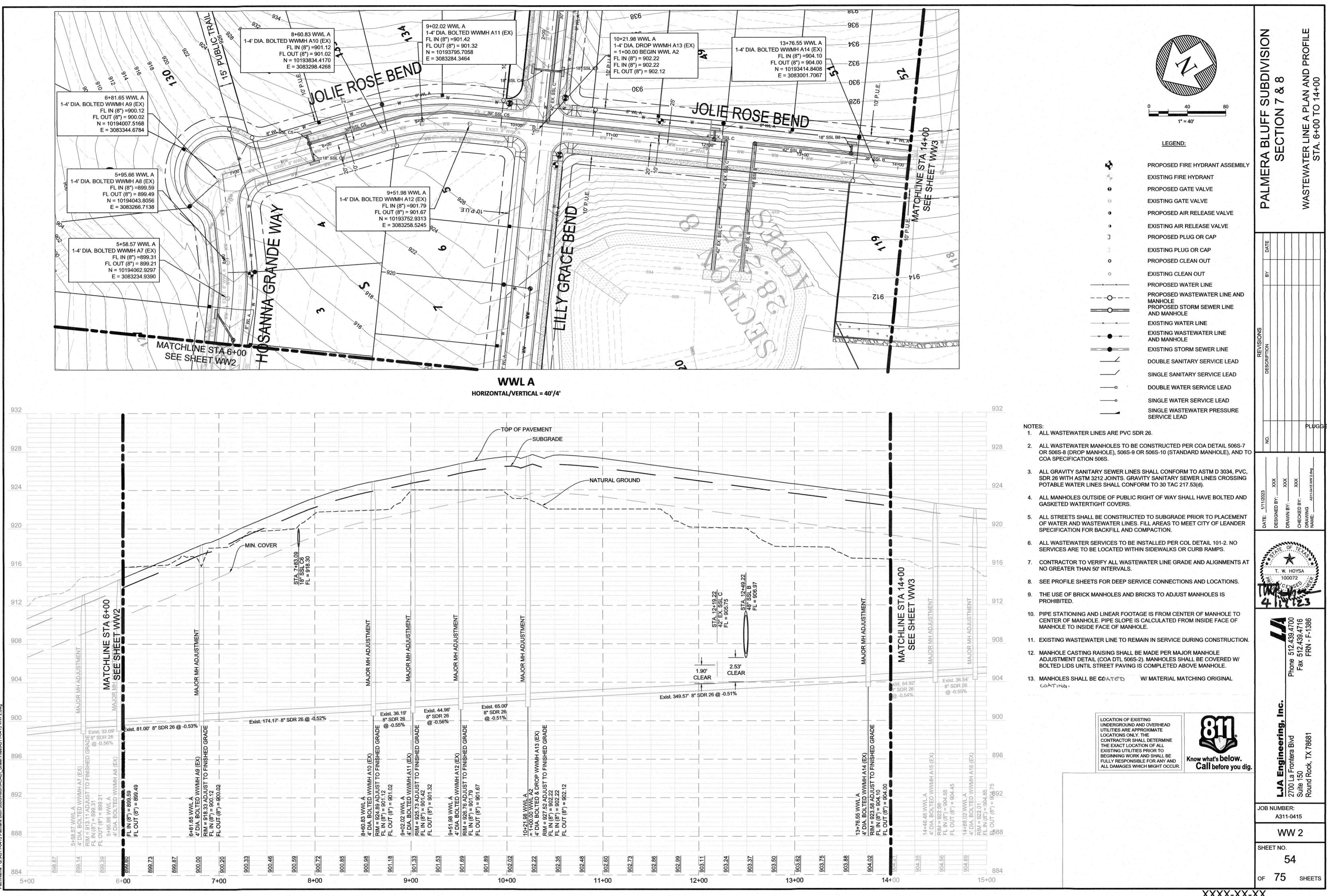
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JOB NUMBER: A311-0415

SHEET NO.

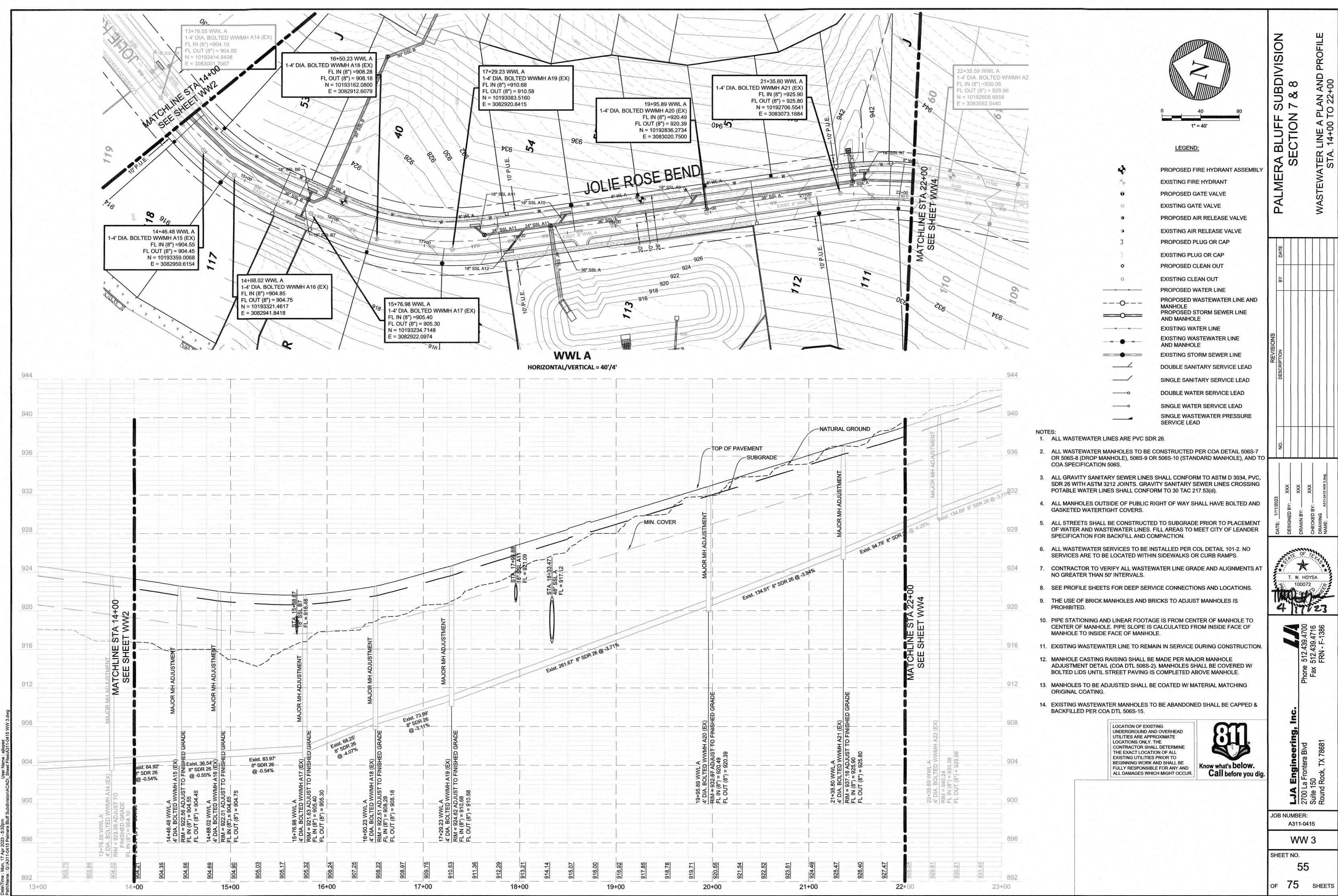
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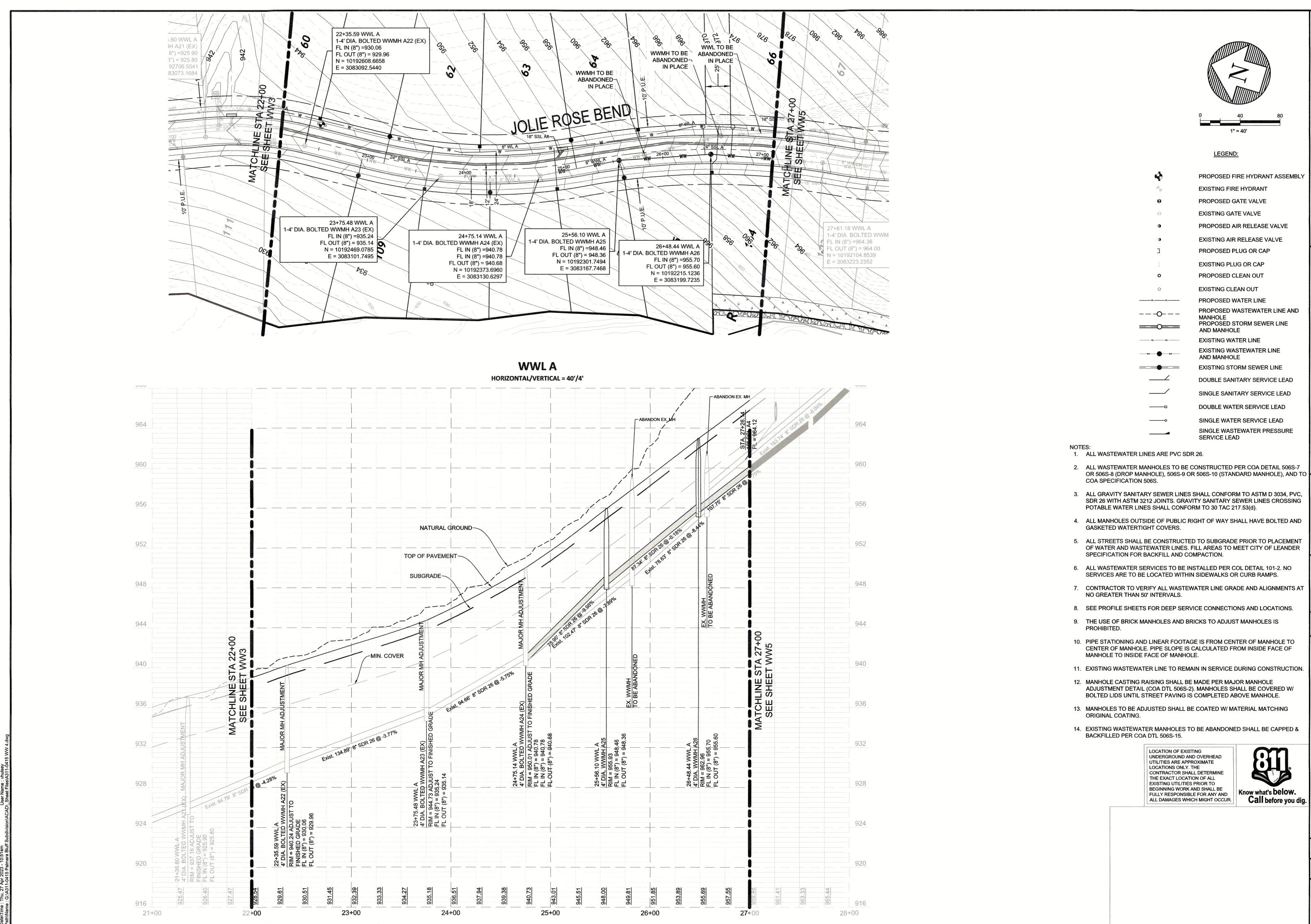




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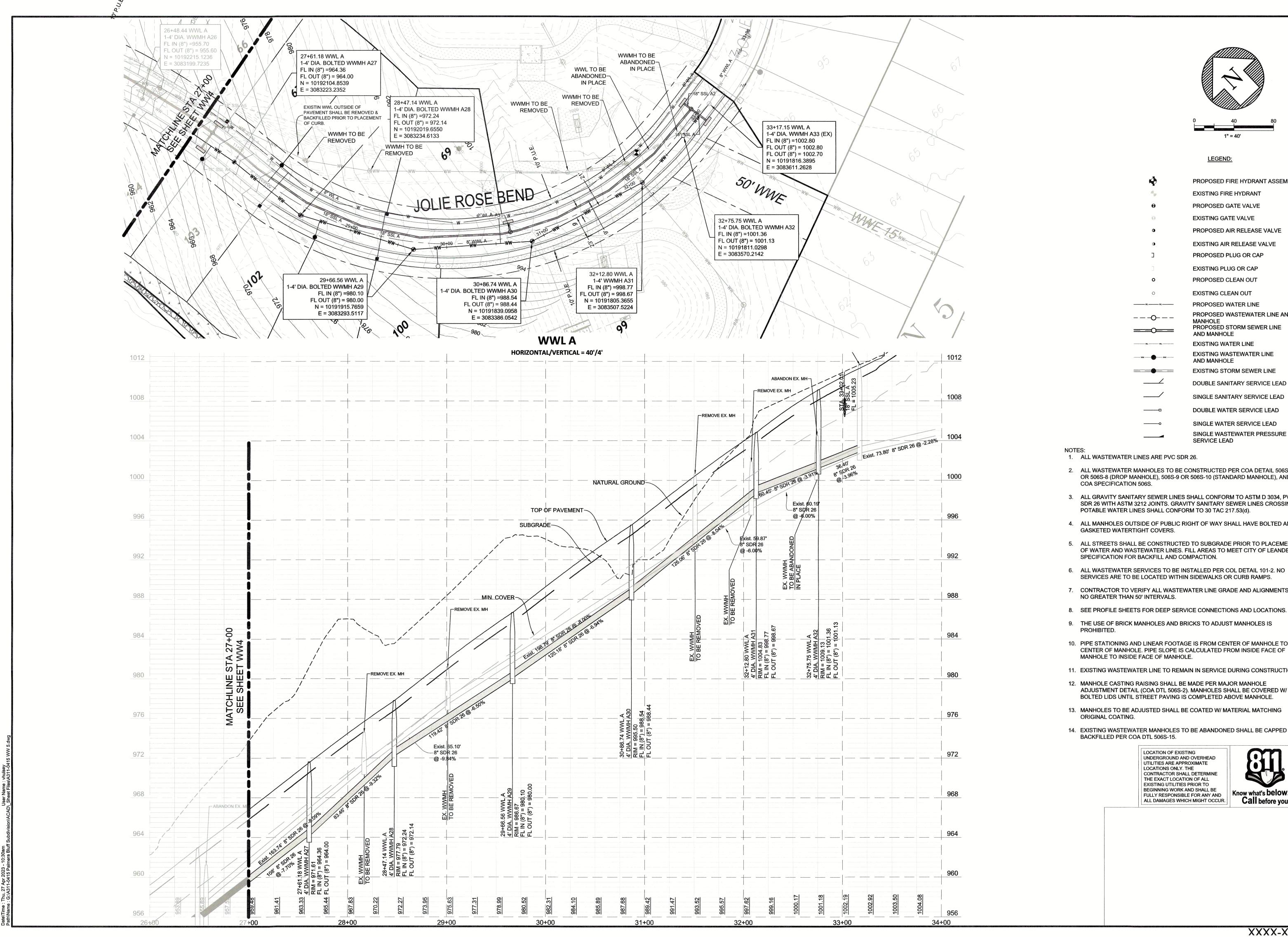
SUBDIVISION 17 & 8

T. W. HOYSA

JOB NUMBER:

A311-0415 WW 4

SHEET NO.



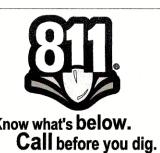
SUBDIVISION 17 & 8

PROPOSED FIRE HYDRANT ASSEMBL' EXISTING FIRE HYDRANT PROPOSED GATE VALVE EXISTING GATE VALVE PROPOSED AIR RELEASE VALVE EXISTING AIR RELEASE VALVE PROPOSED PLUG OR CAP EXISTING PLUG OR CAP PROPOSED CLEAN OUT EXISTING CLEAN OUT PROPOSED WATER LINE PROPOSED WASTEWATER LINE AND MANHOLE PROPOSED STORM SEWER LINE AND MANHOLE EXISTING WATER LINE **EXISTING WASTEWATER LINE**

- 2. ALL WASTEWATER MANHOLES TO BE CONSTRUCTED PER COA DETAIL 506S-7 OR 506S-8 (DROP MANHOLE), 506S-9 OR 506S-10 (STANDARD MANHOLE), AND TO
- 3. ALL GRAVITY SANITARY SEWER LINES SHALL CONFORM TO ASTM D 3034, PVC, SDR 26 WITH ASTM 3212 JOINTS. GRAVITY SANITARY SEWER LINES CROSSING
- 4. ALL MANHOLES OUTSIDE OF PUBLIC RIGHT OF WAY SHALL HAVE BOLTED AND
- 5. ALL STREETS SHALL BE CONSTRUCTED TO SUBGRADE PRIOR TO PLACEMENT OF WATER AND WASTEWATER LINES. FILL AREAS TO MEET CITY OF LEANDER
- 6. ALL WASTEWATER SERVICES TO BE INSTALLED PER COL DETAIL 101-2. NO
- 7. CONTRACTOR TO VERIFY ALL WASTEWATER LINE GRADE AND ALIGNMENTS AT
- 8. SEE PROFILE SHEETS FOR DEEP SERVICE CONNECTIONS AND LOCATIONS.
- 9. THE USE OF BRICK MANHOLES AND BRICKS TO ADJUST MANHOLES IS
- 10. PIPE STATIONING AND LINEAR FOOTAGE IS FROM CENTER OF MANHOLE TO
- CENTER OF MANHOLE. PIPE SLOPE IS CALCULATED FROM INSIDE FACE OF
- 11. EXISTING WASTEWATER LINE TO REMAIN IN SERVICE DURING CONSTRUCTION.
- 12. MANHOLE CASTING RAISING SHALL BE MADE PER MAJOR MANHOLE ADJUSTMENT DETAIL (COA DTL 506S-2). MANHOLES SHALL BE COVERED W/
- 13. MANHOLES TO BE ADJUSTED SHALL BE COATED W/ MATERIAL MATCHING
- 14. EXISTING WASTEWATER MANHOLES TO BE ABANDONED SHALL BE CAPPED &

BEGINNING WORK AND SHALL BE FULLY RESPONSIBLE FOR ANY AND





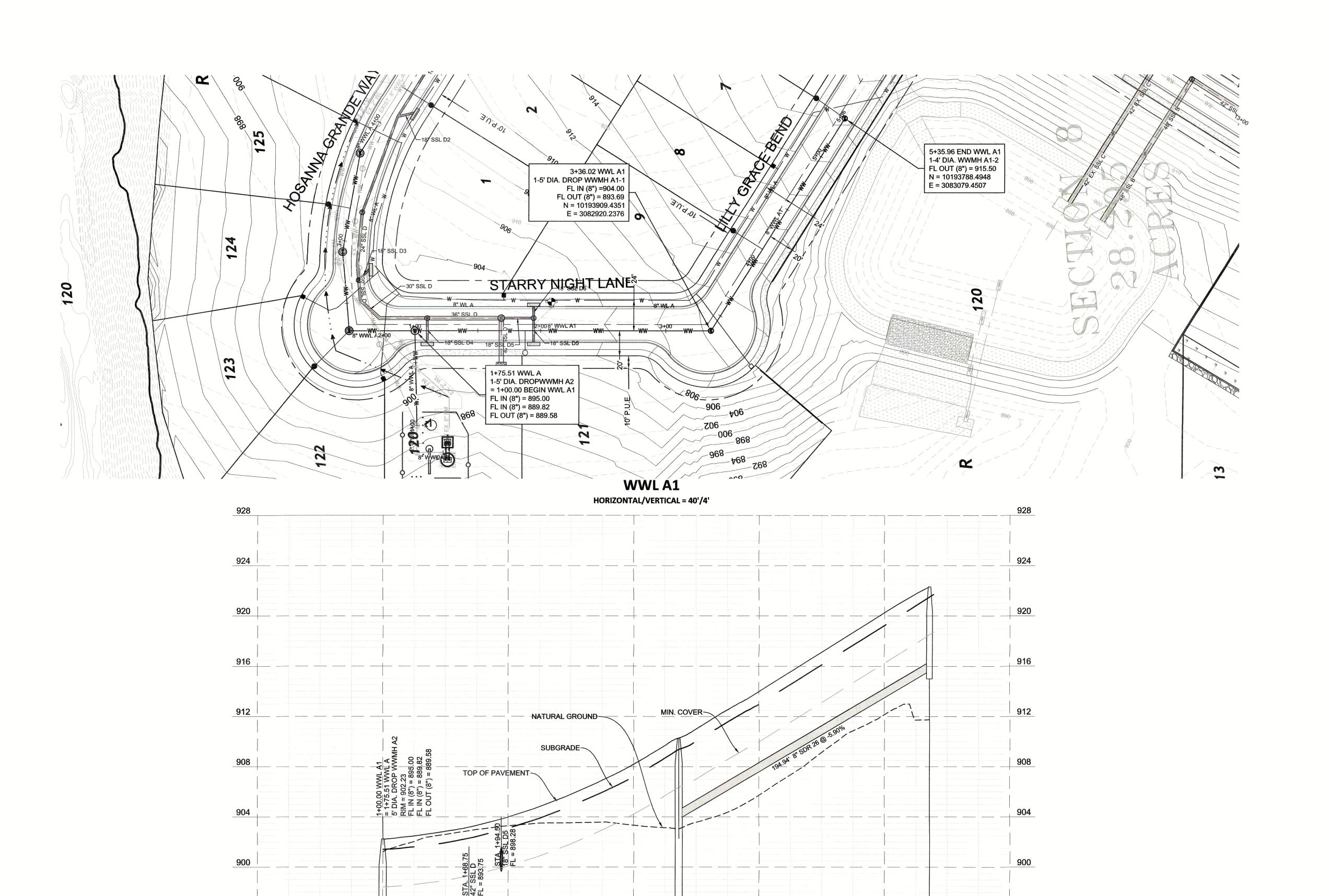
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T. W. HOYSA

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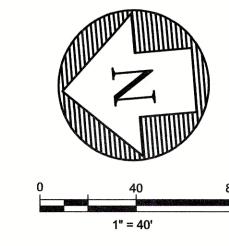
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3+00

5+00

4+00

6+00



RA BLUFF SUBDIVISION SECTION 7 & 8

LEGEND:

❖	PROPOSED FIRE HYDRANT ASSEMBLY
A	EXISTING FIRE HYDRANT
•	PROPOSED GATE VALVE
Ð	EXISTING GATE VALVE
•	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
3	PROPOSED PLUG OR CAP
	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE
	PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
	EXISTING WASTEWATER LINE AND MANHOLE
	EXISTING STORM SEWER LINE
	DOUBLE SANITARY SERVICE LEAD
/	SINGLE SANITARY SERVICE LEAD
	DOUBLE WATER SERVICE LEAD
о	SINGLE WATER SERVICE LEAD
	SINGLE WASTEWATER PRESSURE SERVICE LEAD
TEQ.	

- 1. ALL WASTEWATER LINES ARE PVC SDR 26.
- 2. ALL WASTEWATER MANHOLES TO BE CONSTRUCTED PER COA DETAIL 506S-7 OR 506S-8 (DROP MANHOLE), 506S-9 OR 506S-10 (STANDARD MANHOLE), AND TO COA SPECIFICATION 506S.
- 3. ALL GRAVITY SANITARY SEWER LINES SHALL CONFORM TO ASTM D 3034, PVC, SDR 26 WITH ASTM 3212 JOINTS. GRAVITY SANITARY SEWER LINES CROSSING POTABLE WATER LINES SHALL CONFORM TO 30 TAC 217.53(d).
- 4. ALL MANHOLES OUTSIDE OF PUBLIC RIGHT OF WAY SHALL HAVE BOLTED AND GASKETED WATERTIGHT COVERS.
- 5. ALL STREETS SHALL BE CONSTRUCTED TO SUBGRADE PRIOR TO PLACEMENT OF WATER AND WASTEWATER LINES. FILL AREAS TO MEET CITY OF LEANDER SPECIFICATION FOR BACKFILL AND COMPACTION.
- 6. ALL WASTEWATER SERVICES TO BE INSTALLED PER COL DETAIL 101-2. NO SERVICES ARE TO BE LOCATED WITHIN SIDEWALKS OR CURB RAMPS.
- 7. CONTRACTOR TO VERIFY ALL WASTEWATER LINE GRADE AND ALIGNMENTS AT NO GREATER THAN 50' INTERVALS.
- 8. SEE PROFILE SHEETS FOR DEEP SERVICE CONNECTIONS AND LOCATIONS.
- 9. THE USE OF BRICK MANHOLES AND BRICKS TO ADJUST MANHOLES IS PROHIBITED.
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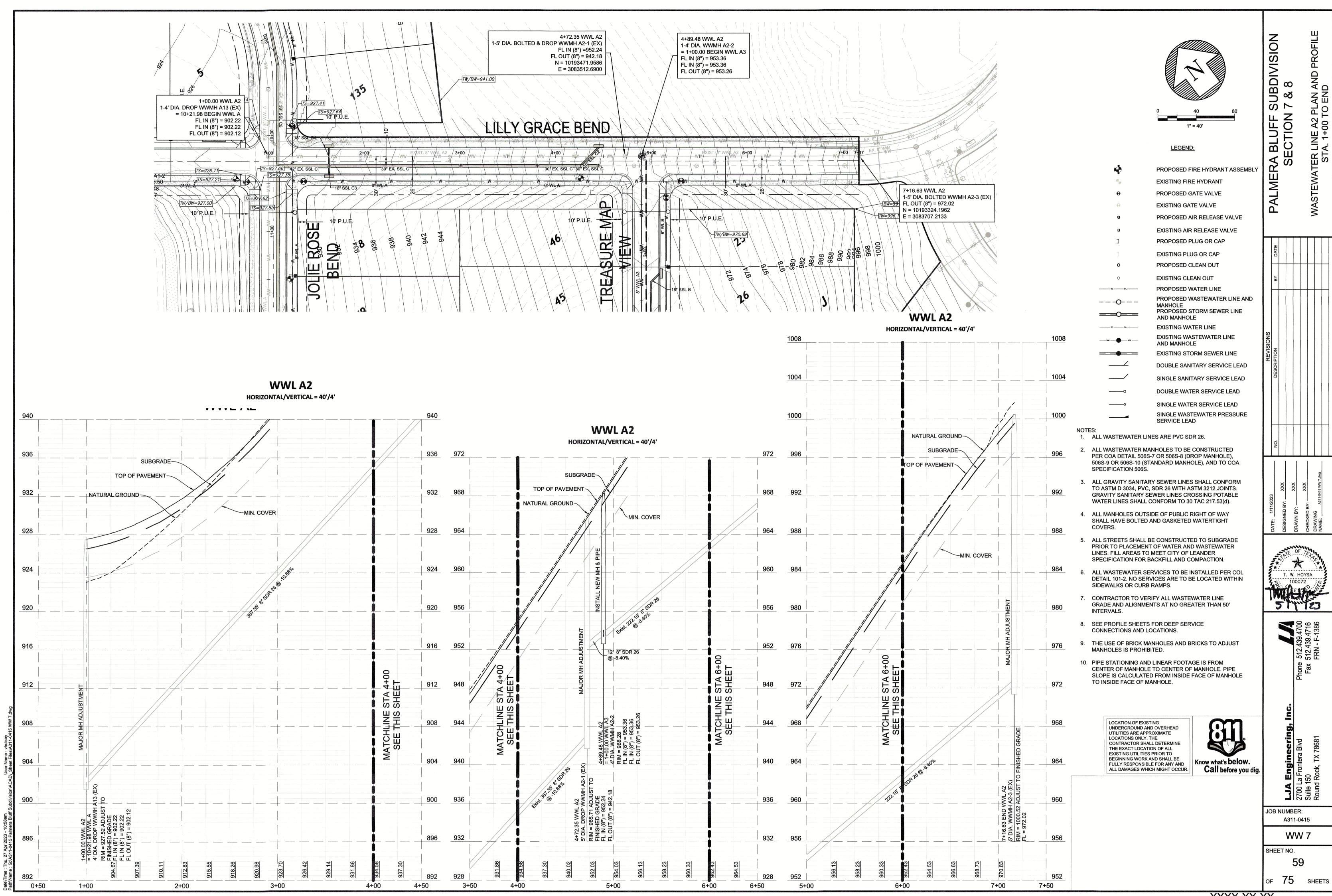
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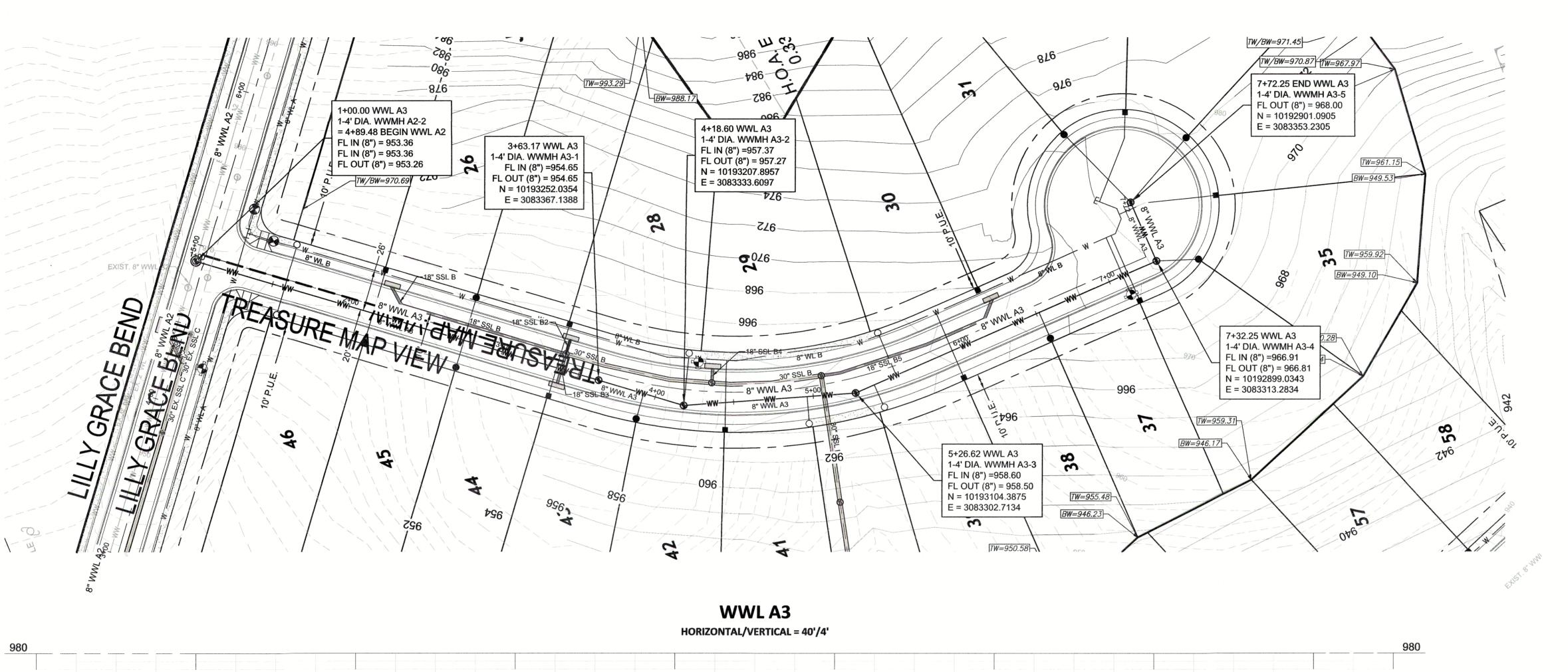
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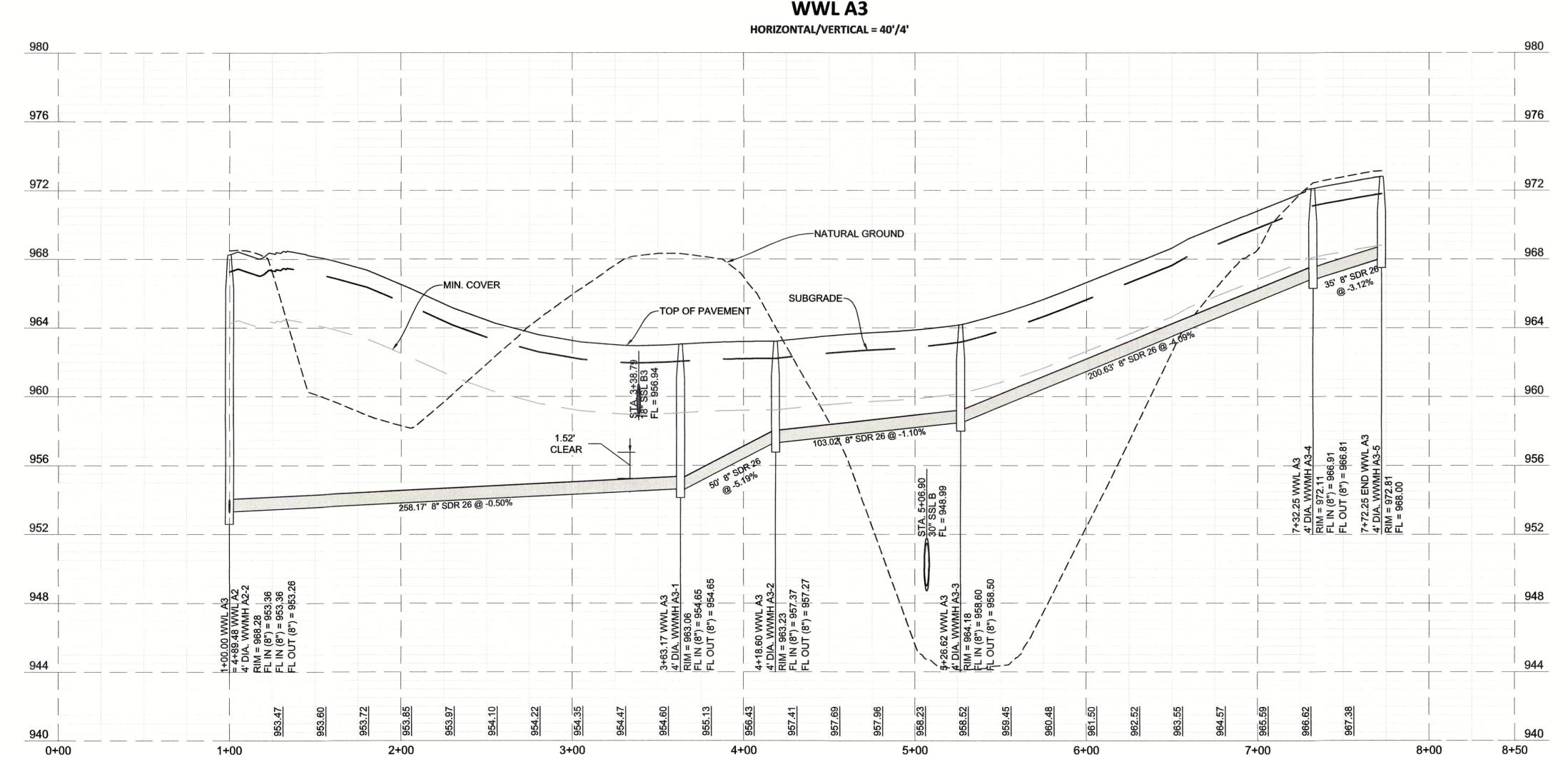
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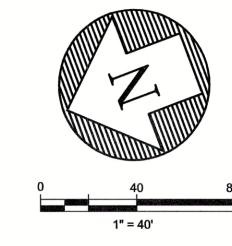
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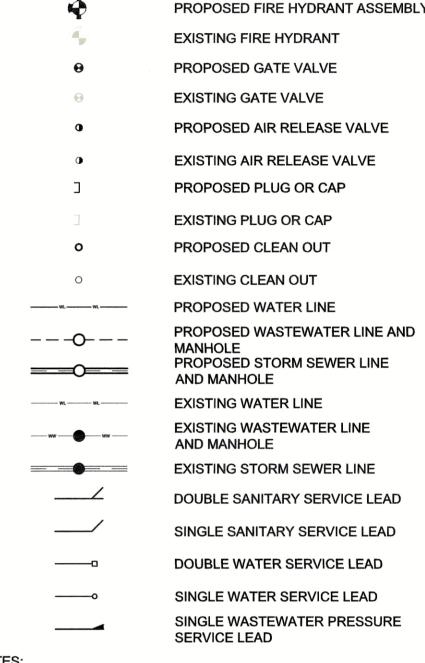




PROFILE

RA BLUFF SUBDIVISION SECTION 7 & 8

LEGEND:



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- 7. CONTRACTOR TO VERIFY ALL WASTEWATER LINE
- GRADE AND ALIGNMENTS AT NO GREATER THAN 50' INTERVALS.
- CONNECTIONS AND LOCATIONS.

8. SEE PROFILE SHEETS FOR DEEP SERVICE

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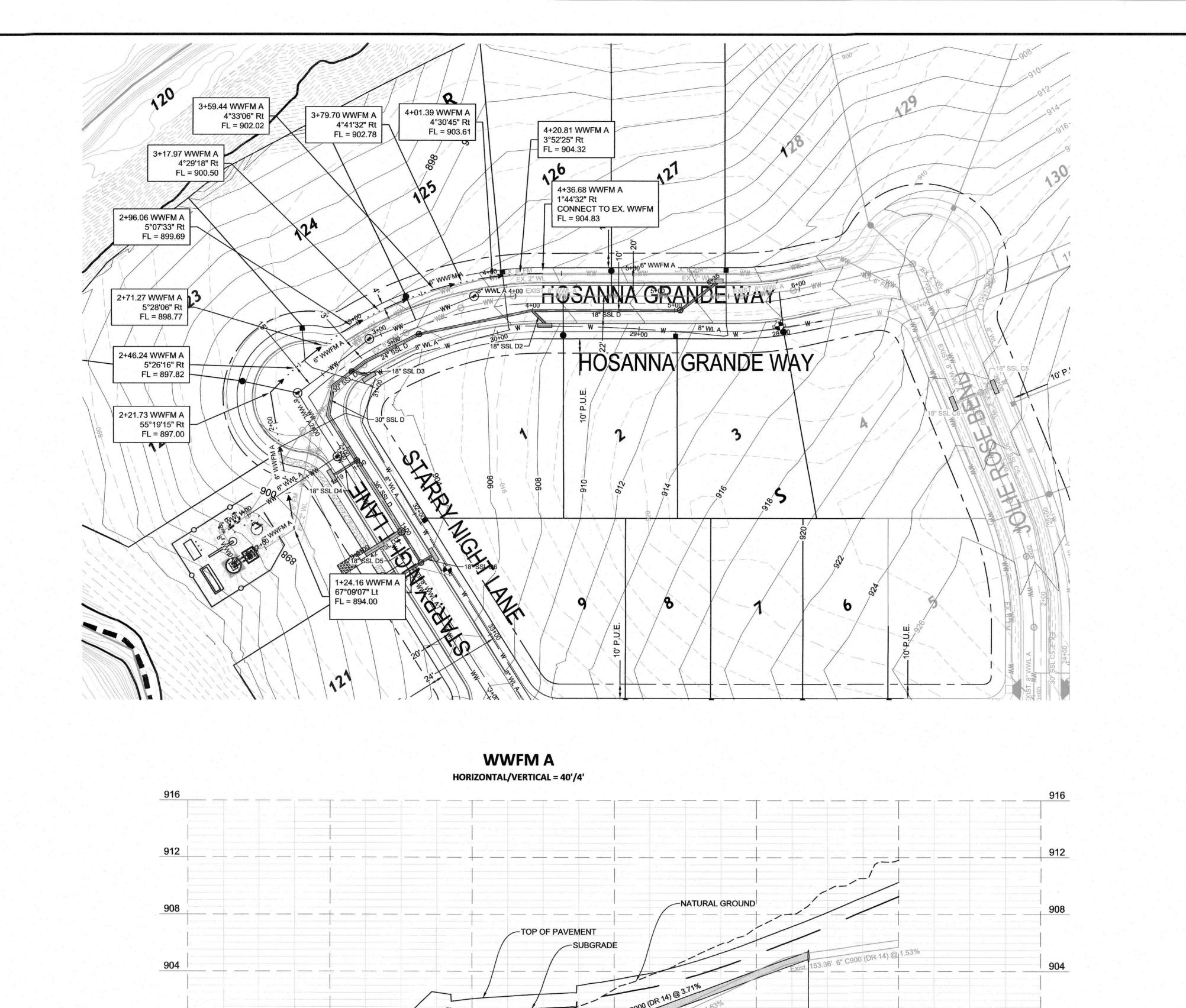
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T. W. HOYSA

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Exist. 22.16' 6" SDR 26

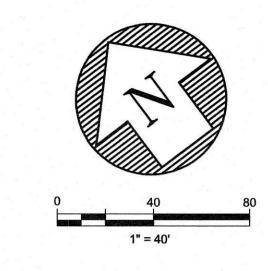
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1+00

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3+00



SUBDIVISION 17&8

RA BLUFF SECTION

ORCEMAIN A, PLAN STA. 1+00 TO END

LEGEND:

EXISTING FIRE HYDRANT

PROPOSED GATE VALVE

PROPOSED FIRE HYDRANT ASSEMBLY

0	EXISTING GATE VALVE
•	PROPOSED AIR RELEASE VALVE
•	EXISTING AIR RELEASE VALVE
1	PROPOSED PLUG OR CAP
1	EXISTING PLUG OR CAP
0	PROPOSED CLEAN OUT
0	EXISTING CLEAN OUT
	PROPOSED WATER LINE
0	PROPOSED WASTEWATER LINE AND MANHOLE
The state of the s	PROPOSED STORM SEWER LINE AND MANHOLE
	EXISTING WATER LINE
	EXISTING WASTEWATER LINE AND MANHOLE
Andrew Sandards Sandards Sandards	EXISTING STORM SEWER LINE
	DOUBLE SANITARY SERVICE LEAD
	SINGLE SANITARY SERVICE LEAD
——а	DOUBLE WATER SERVICE LEAD
	SINGLE WATER SERVICE LEAD
	SINGLE WASTEWATER PRESSURE SERVICE LEAD
OTES:	

900

880

6+00

5+00

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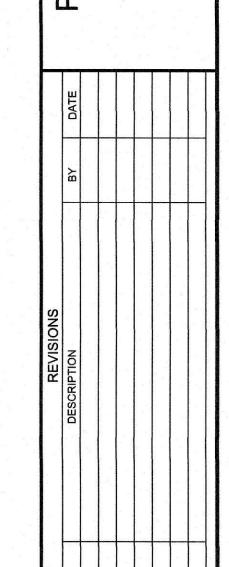
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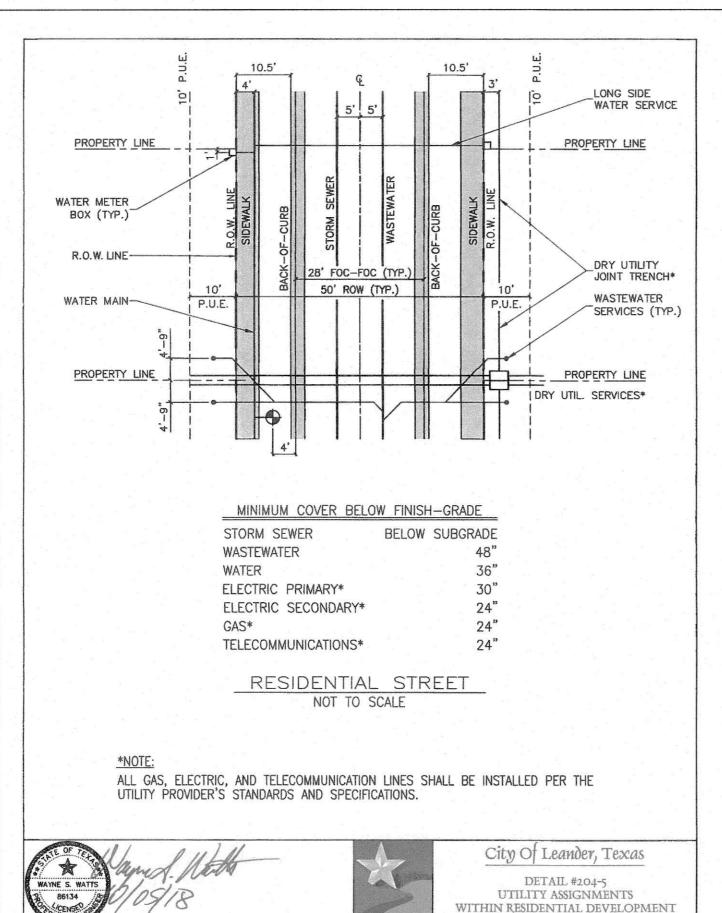


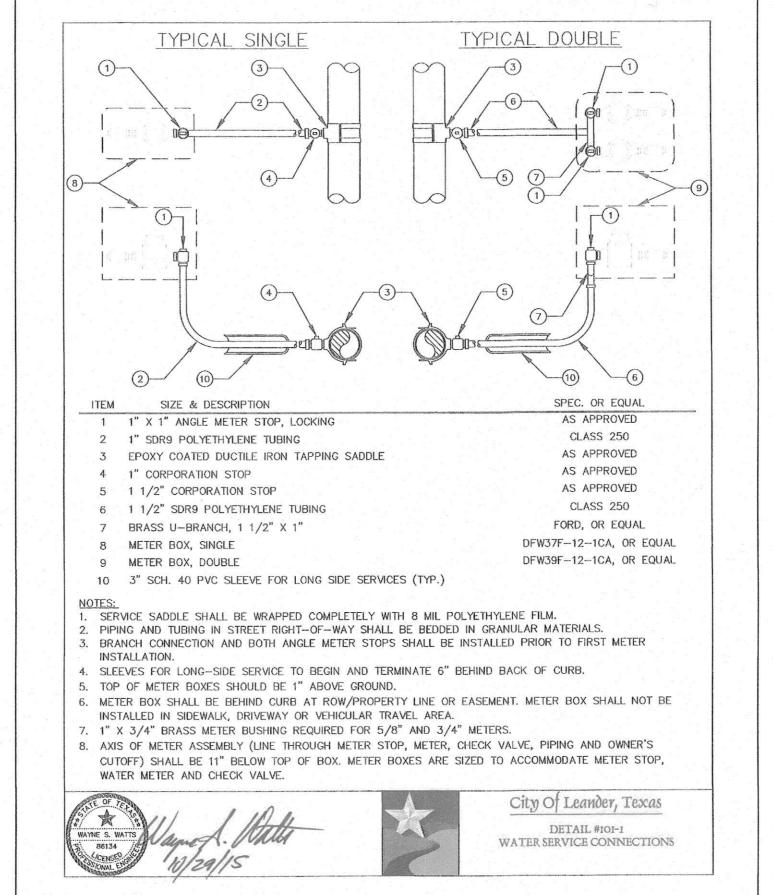


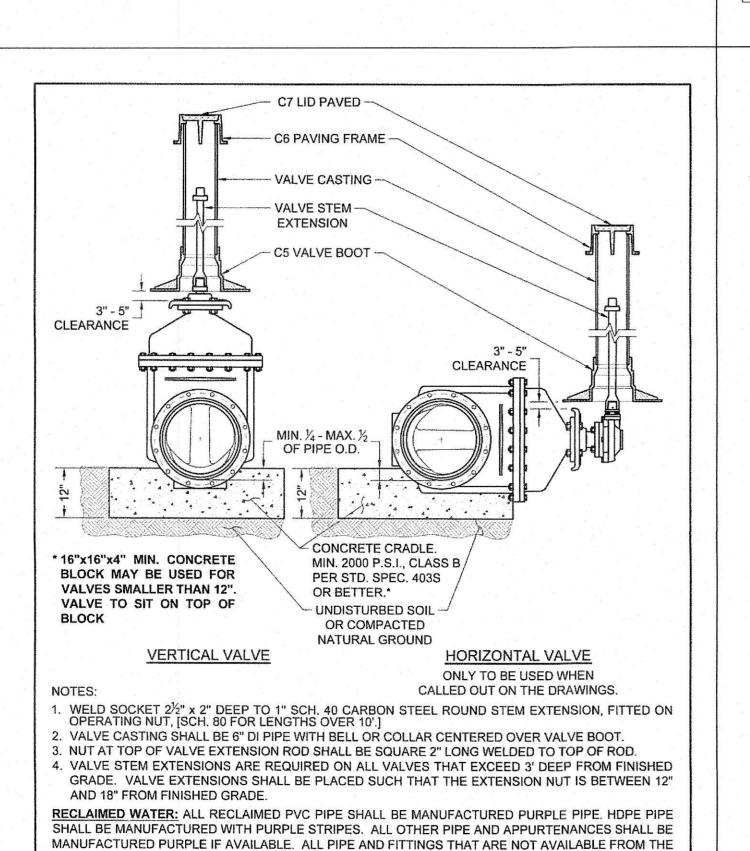
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MANUFACTURER IN PURPLE SHALL BE PAINTED PURPLE PER SPL WW-3C. ALL BURIED DI AND CI PIPE

AND FITTINGS SHALL ALSO BE WRAPPED IN PURPLE POLYETHYLENE PER SPL WW-27D. ALL COVERS

ADOPTED

TYPICAL GATE VALVE 4" - 16"

THE ARCHITECT/ENGINEER ASSUMES

USE OF THIS STANDARD.

05/18/2016 RESPONSIBILITY FOR APPROPRIATE

STANDARD NO.

511-AW-01

1 OF 4

SHALL HAVE "RECLAIMED WATER" CAST INTO THEM.

CITY OF AUSTIN

AUSTIN WATER

RESIDENTIAL LOCAL STREET SECTION

50' MIN. R.O.W.

1. DIMENSIONS FOR ROW AND FOC-FOC ARE FOR RESIDENTIAL LOCAL STREETS ONLY. SEE TRANSPORTATION PLAN

2. BASE COURSE TO EXTEND 1' (MINIMUM) PAST BACK OF CURB FOR SOILS WITH PLOF 20 OR LESS, 3' FOR ALL

ASPHALT THICKNESS, BASE THICKNESS, SUB-BASE AND GEOGRID BASED UPON ON-SITE SOIL CONDITIONS AND

6. SLOPE FROM BACK OF CURB TO RIGHT-OF-WAY SHALL BE 2% UNLESS OTHERWISE INDICATED ON THE GRADING

STREET TREES, WHERE REQUIRED, SHALL BE PLACED BETWEEN SIDEWALK AND BACK OF CURB. SEE DETAIL 204-4

8. METER BOXES AND SEWER SERVICE STUBS SHALL BE PLACED IN THE PUBLIC UTILITY EASEMENT ABUTTING THE

*THE ARCHITECT/ENGINEER ASSUMES RESPONSIBILITY FOR APPROPRIATE USE OF THIS STANDARD. DRAWING NOT TO

FOR ROADWAY AND RIGHT-OF-WAY WIDTHS FOR ALL OTHER ROADWAY CLASSIFICATIONS.

3. PAVEMENT STRUCTURAL SECTION SHALL, IN NO CASE, BE LESS THAN THE VALUES SHOWN ABOVE.

4. GEOTECHNICAL ENGINEER SHALL PROVIDE A PAVEMENT DESIGN TO DETERMINE THE NEED FOR ADDITIONAL

- SEE NOTES 6 & 7

- WASTEWATER

PRIME COAT

— 2" HMAC TYPE D (MIN.)

City Of Leander, Texas

DETAIL #204-3

RESIDENTIAL LOCAL STREET SECTION

VITH STANDARD UTILITY ASSIGNMENTS

WITH STANDARD UTILITY ASSIGNMENT

WATER-

- METER BOX

OTHER SOILS.

TRAFFIC PROJECTIONS.

5. SIDEWALK TO HAVE MAXIMUM 2% CROSS SLOPE.

SEE NOTE 2

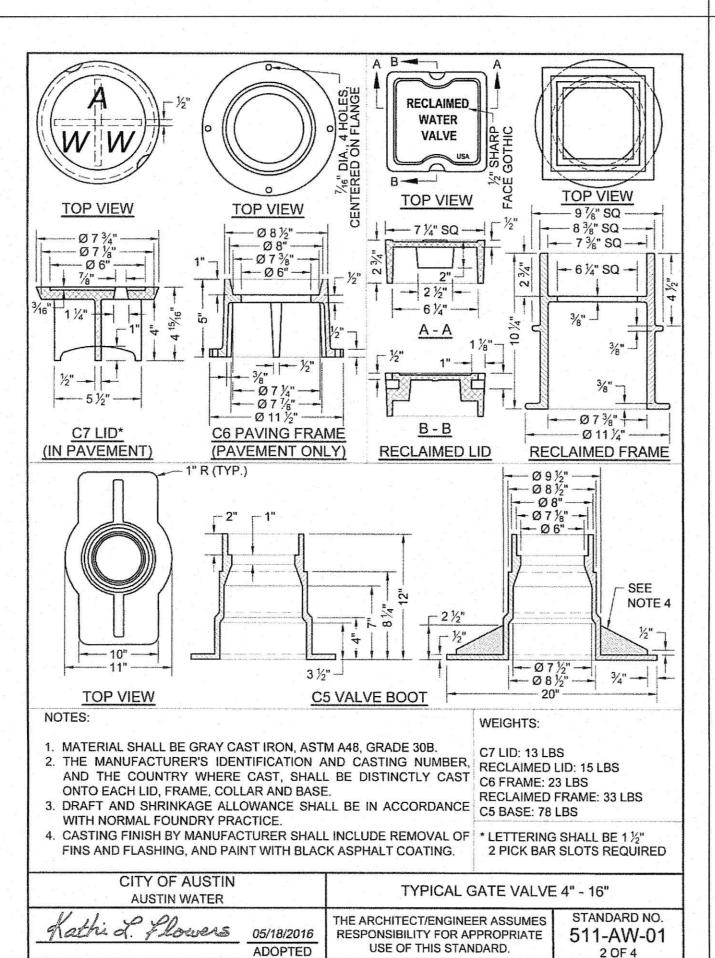
8" FLEXIBLE BASE (MIN.)

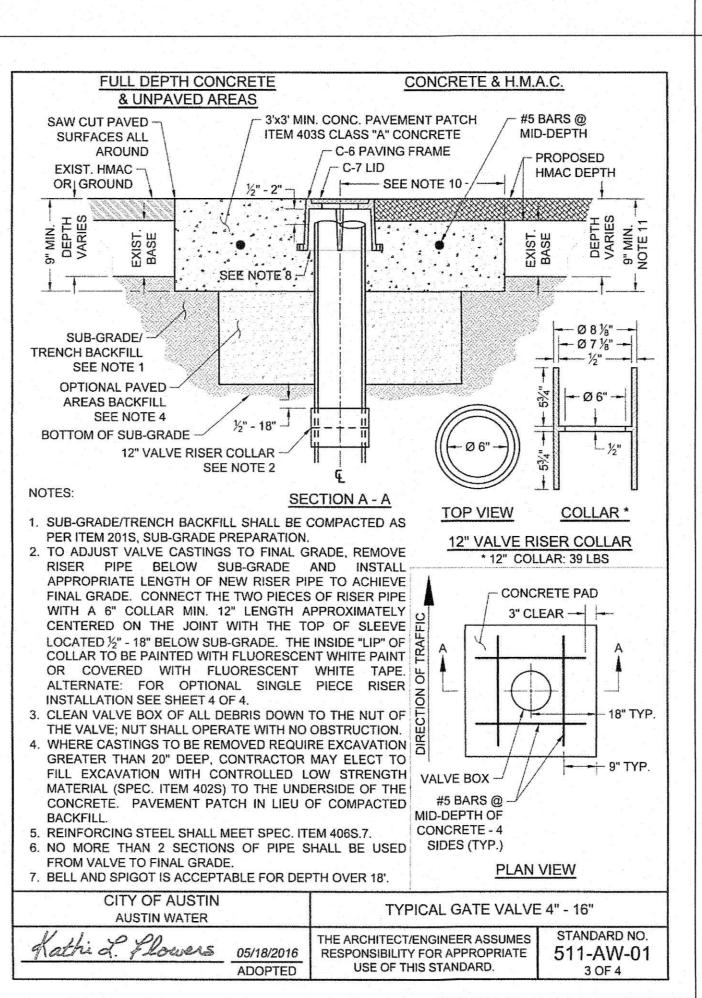
GEOGRID

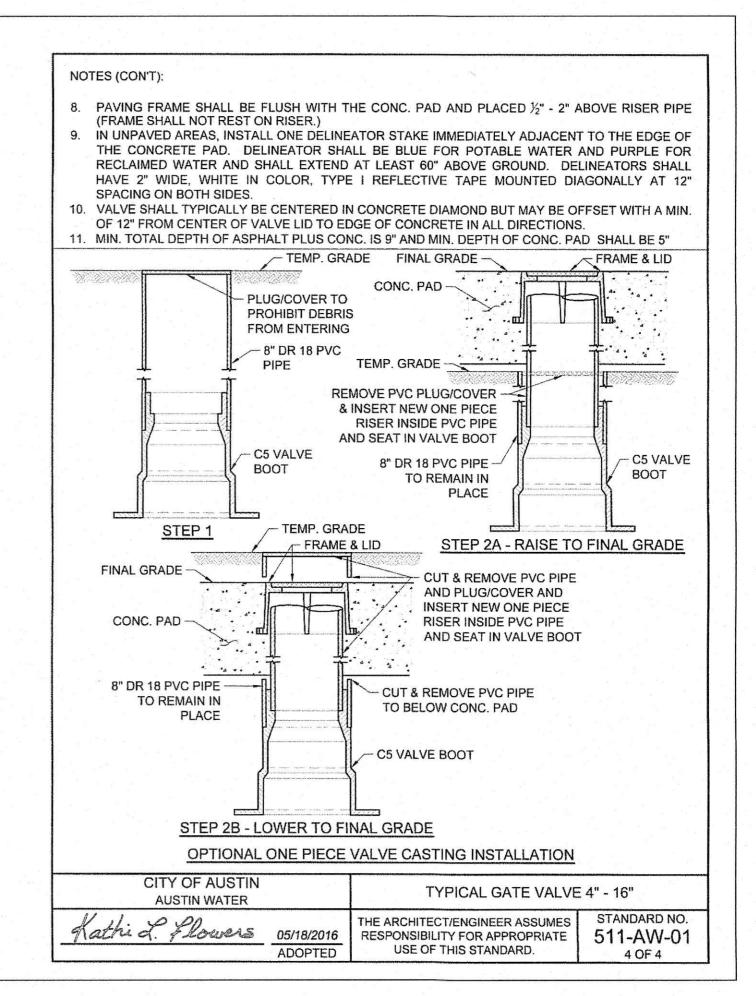
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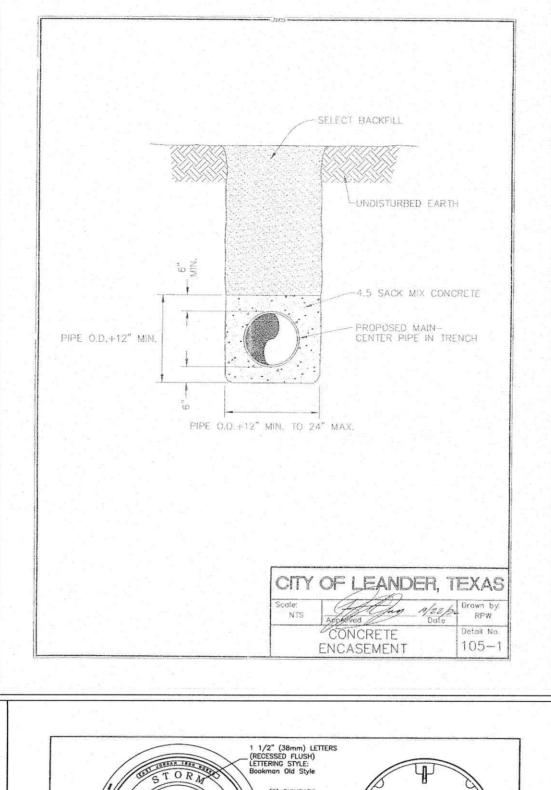
SELECT SUB-BASE

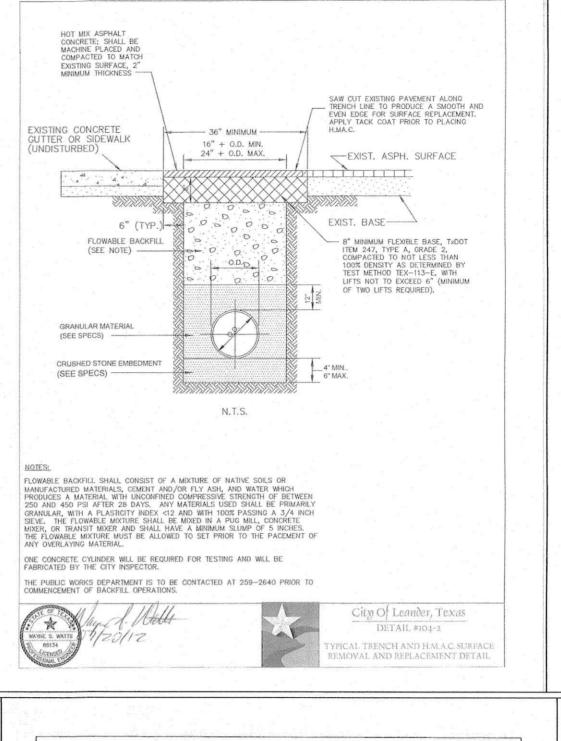
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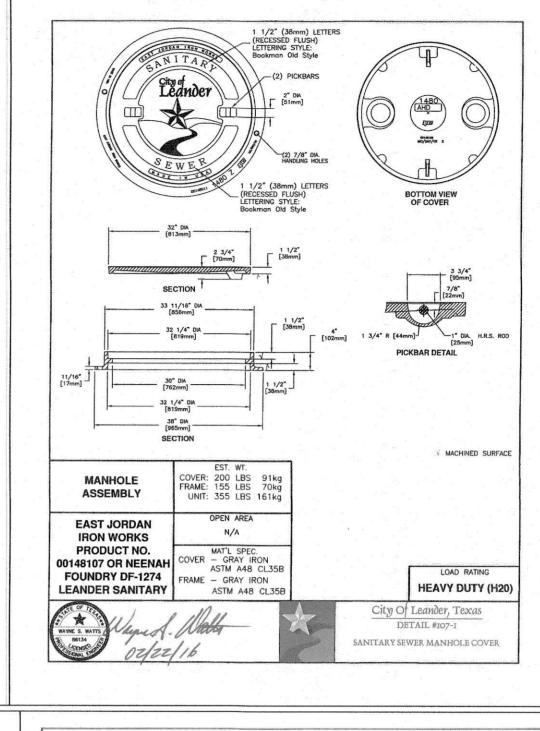


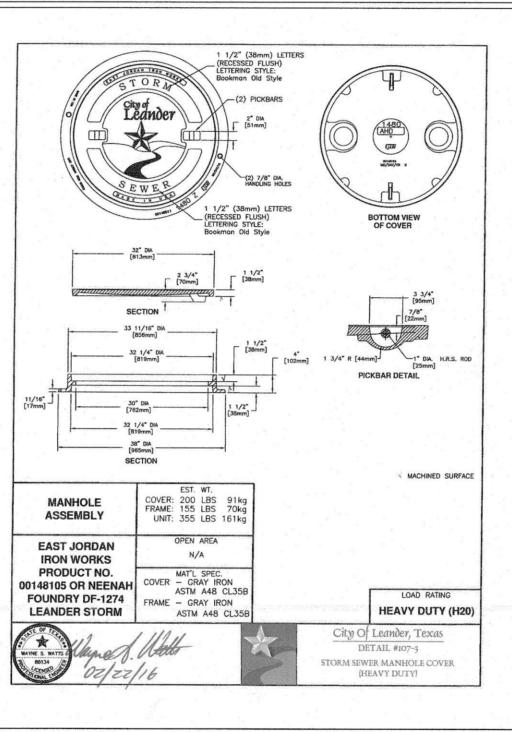


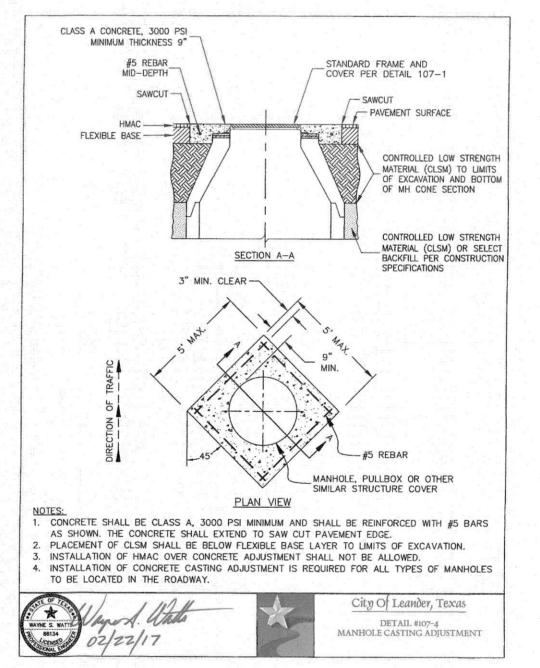


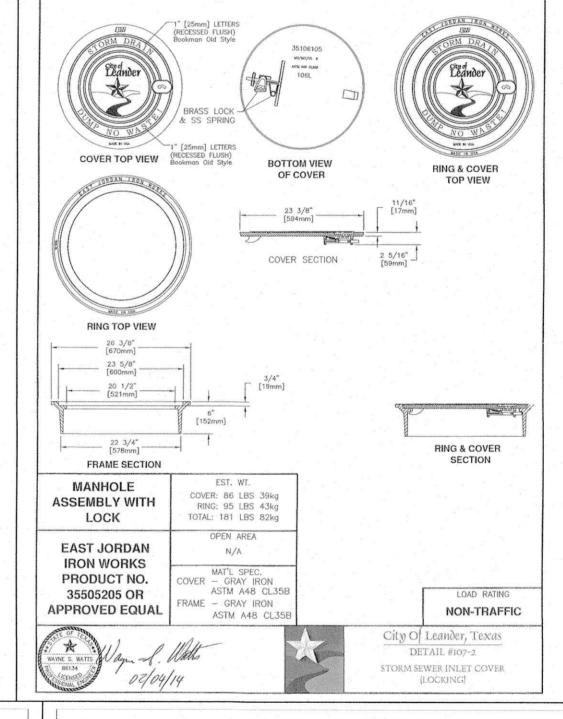


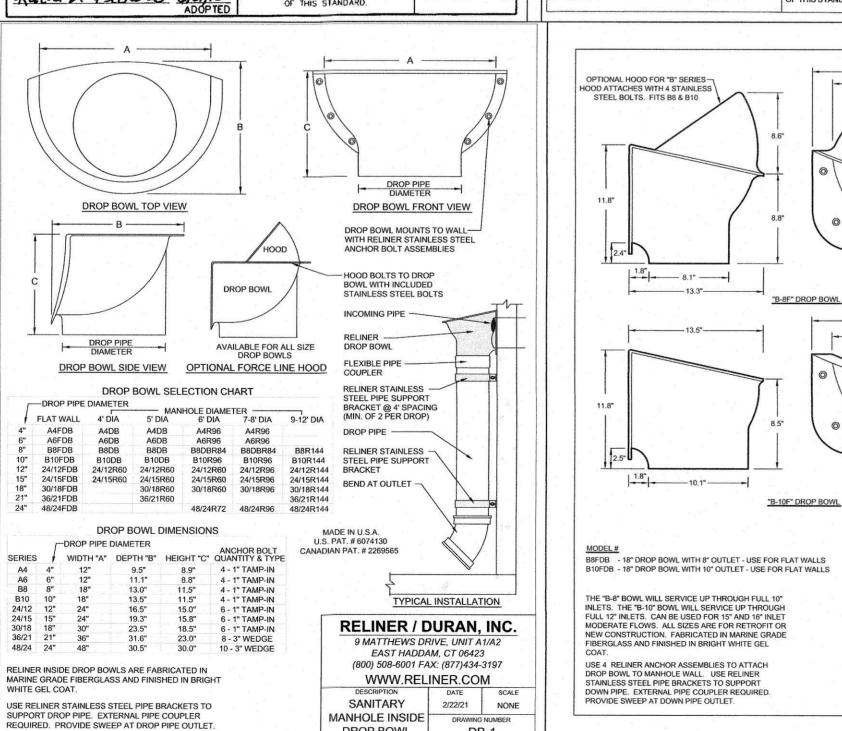






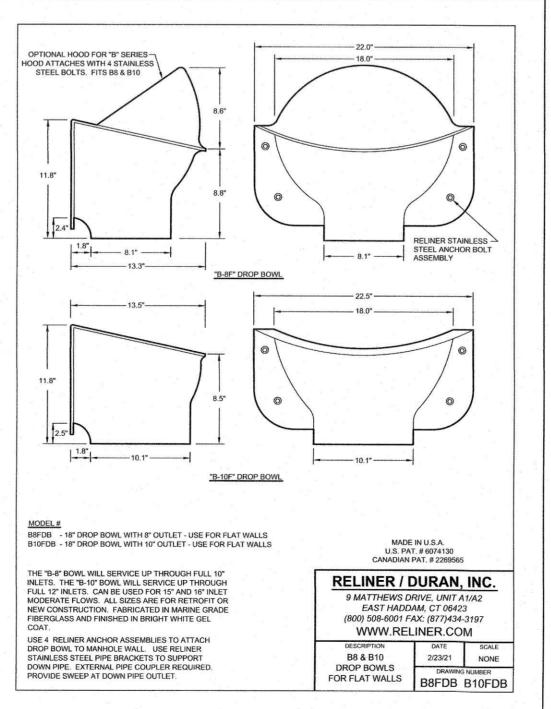


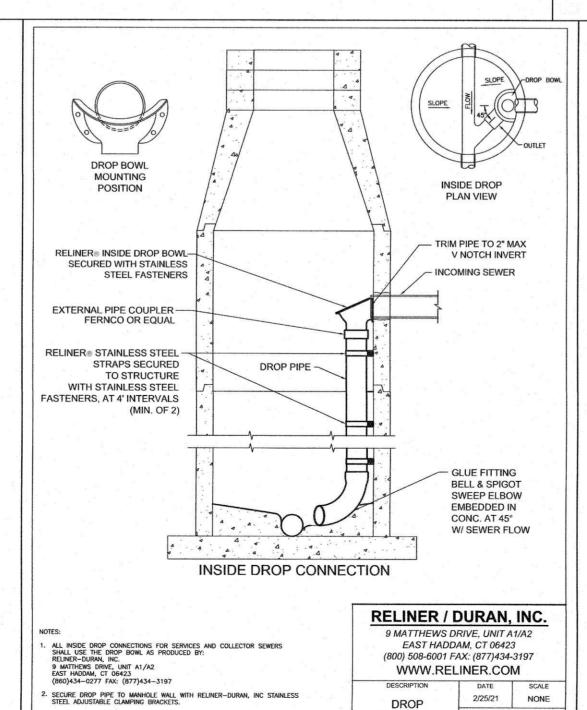




DROP BOWL

DB-1

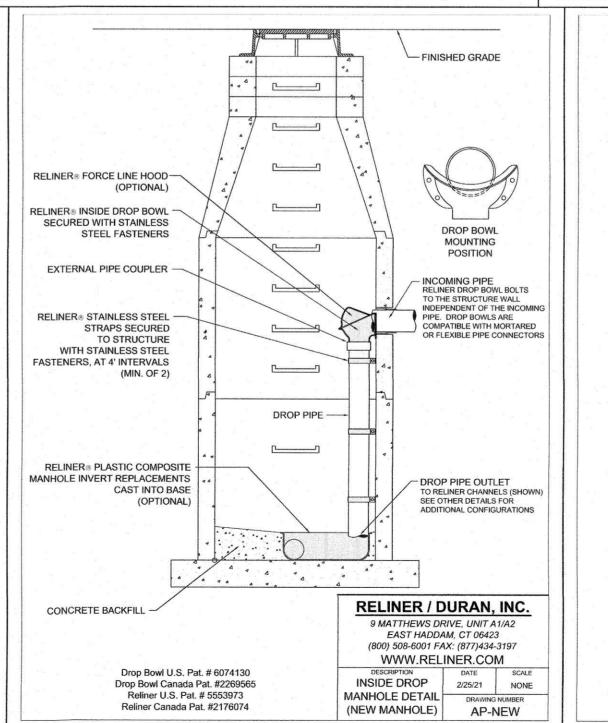


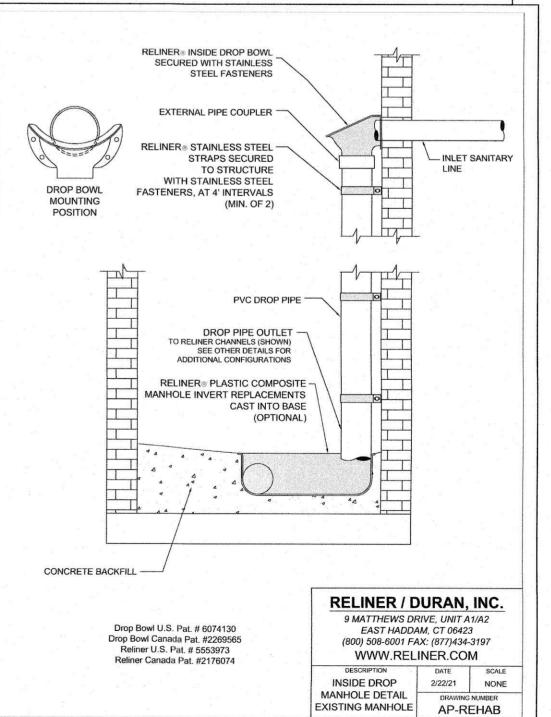


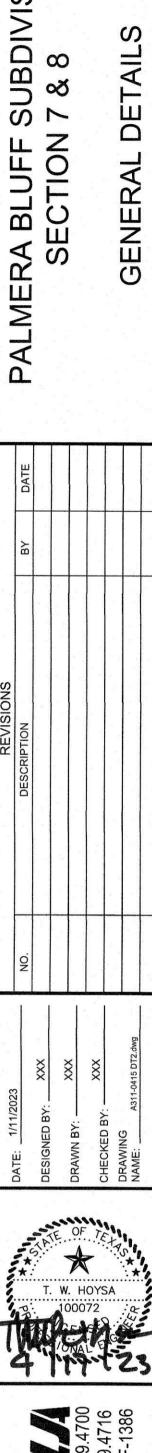
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CONNECTIONS







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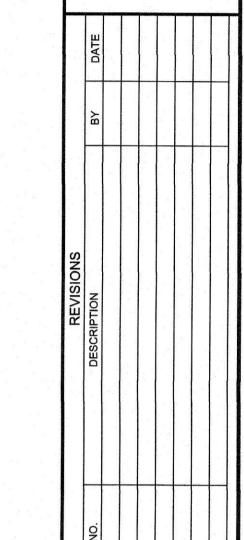
JOB NUMBER:

SHEET NO.

A311-0415

DT 2



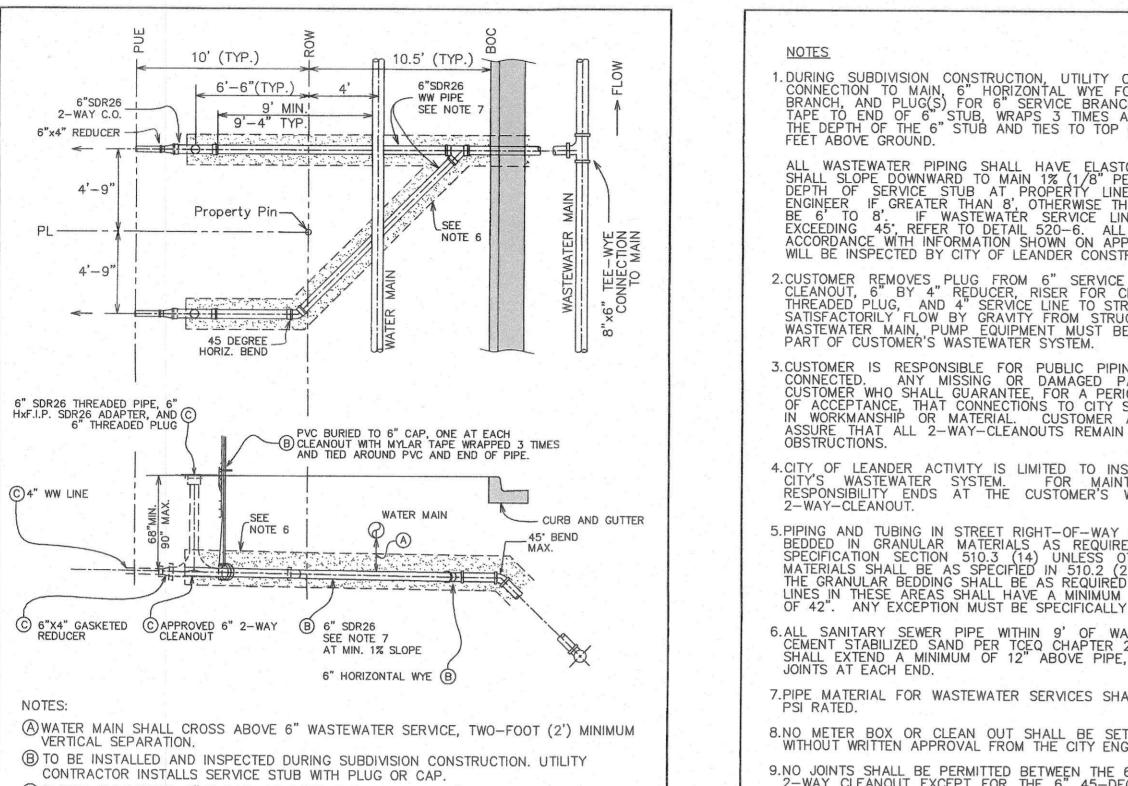


T. W. HOYSA 100072

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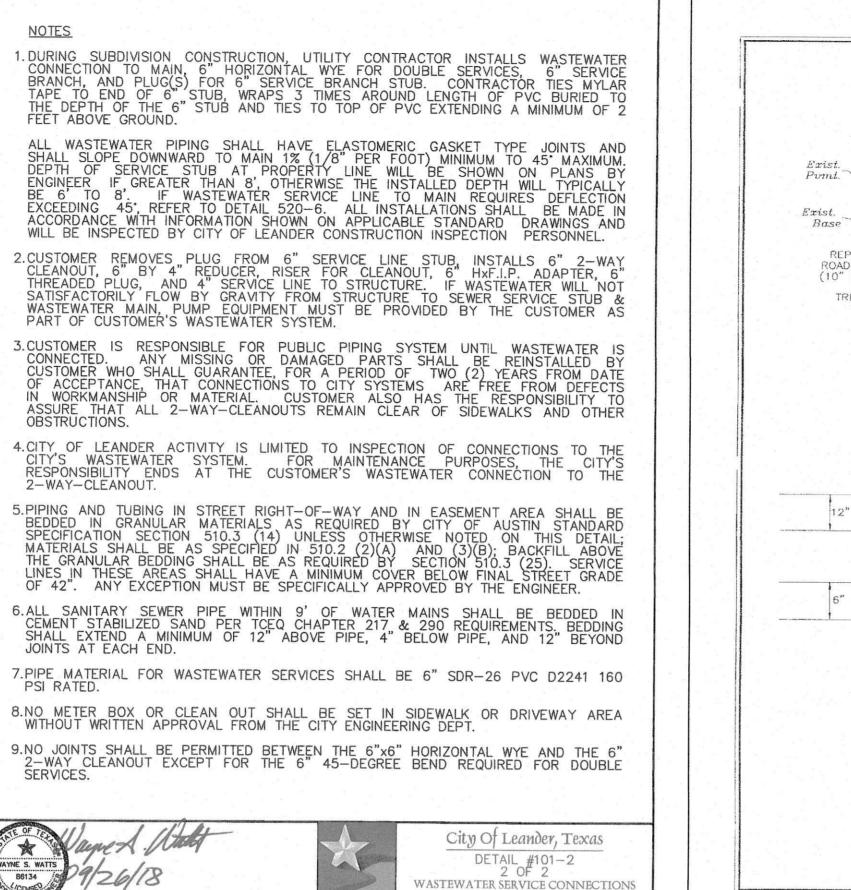
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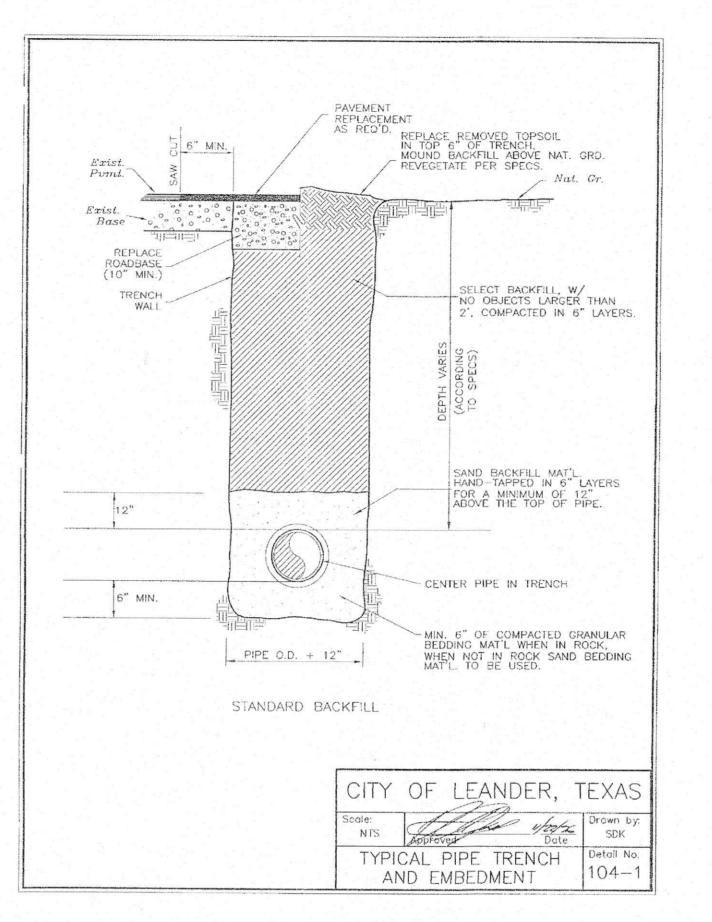
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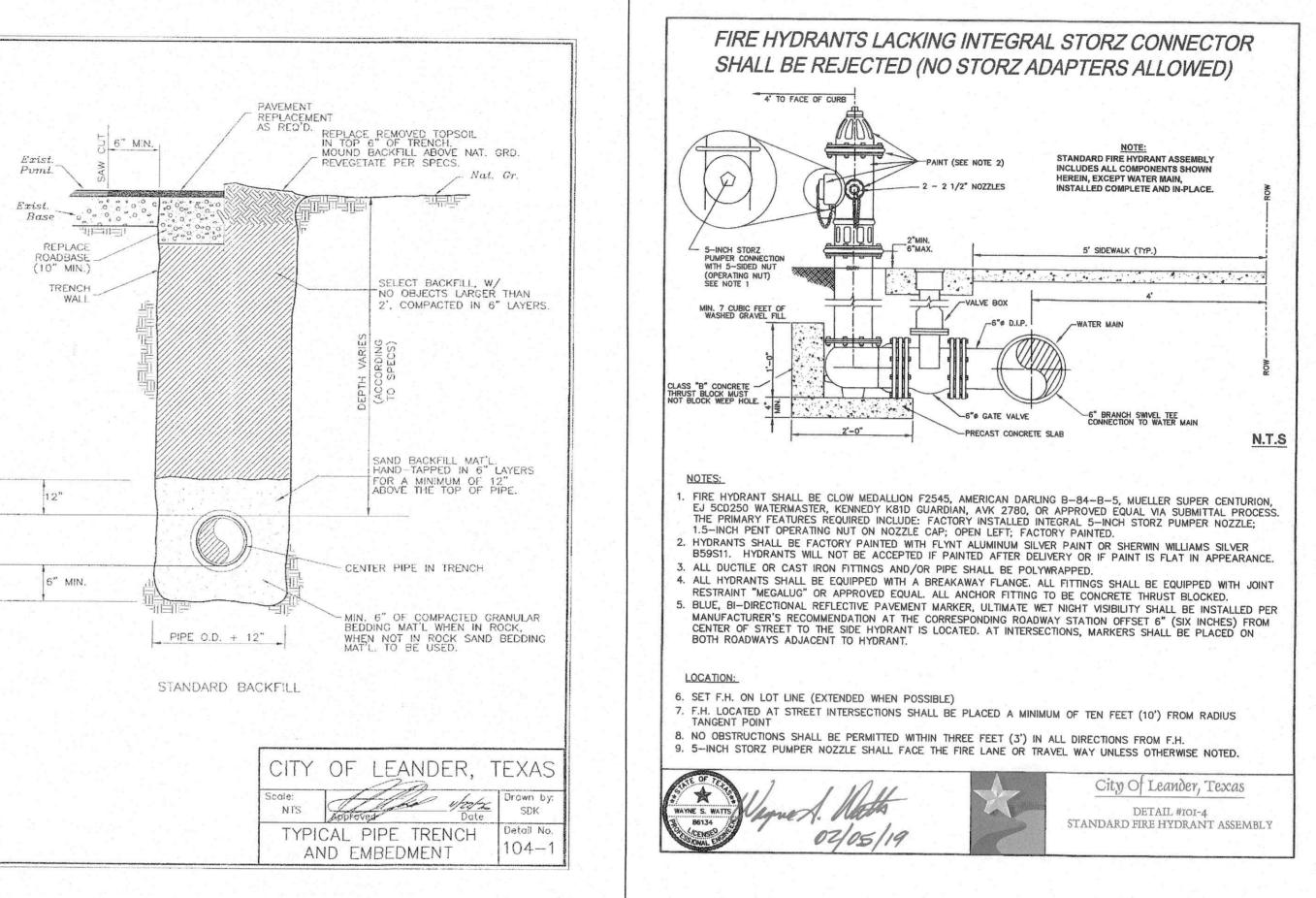


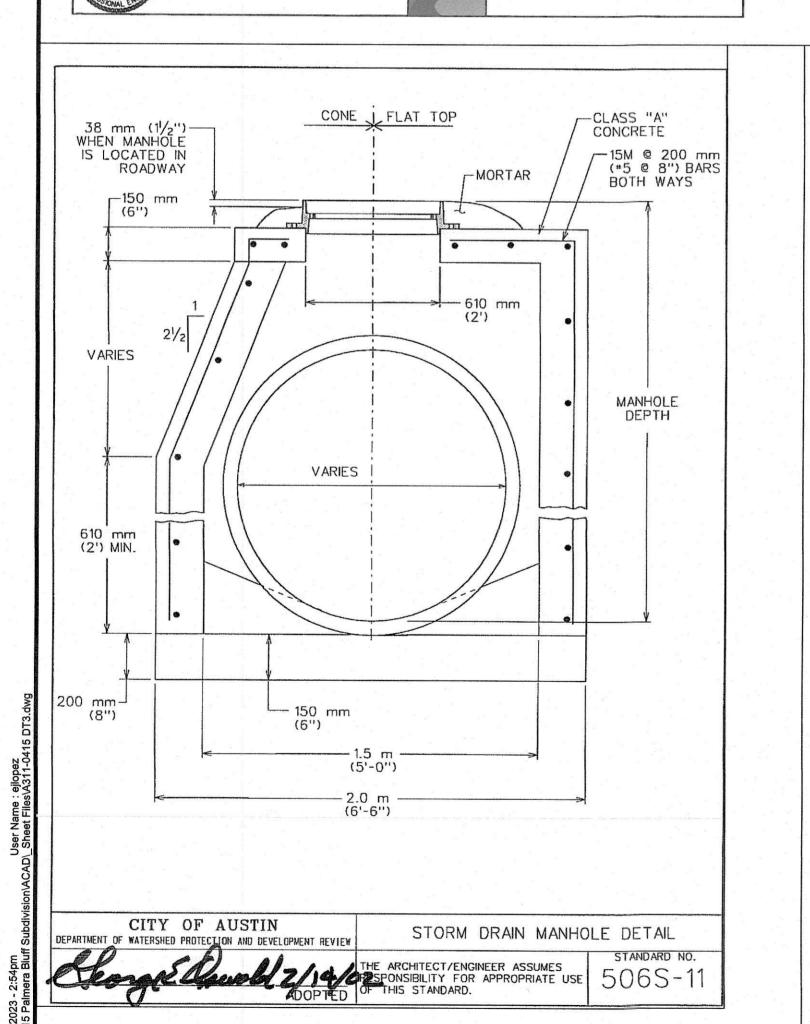
City Of Leander, Texas

WASTEWATER SERVICE CONNECTIONS

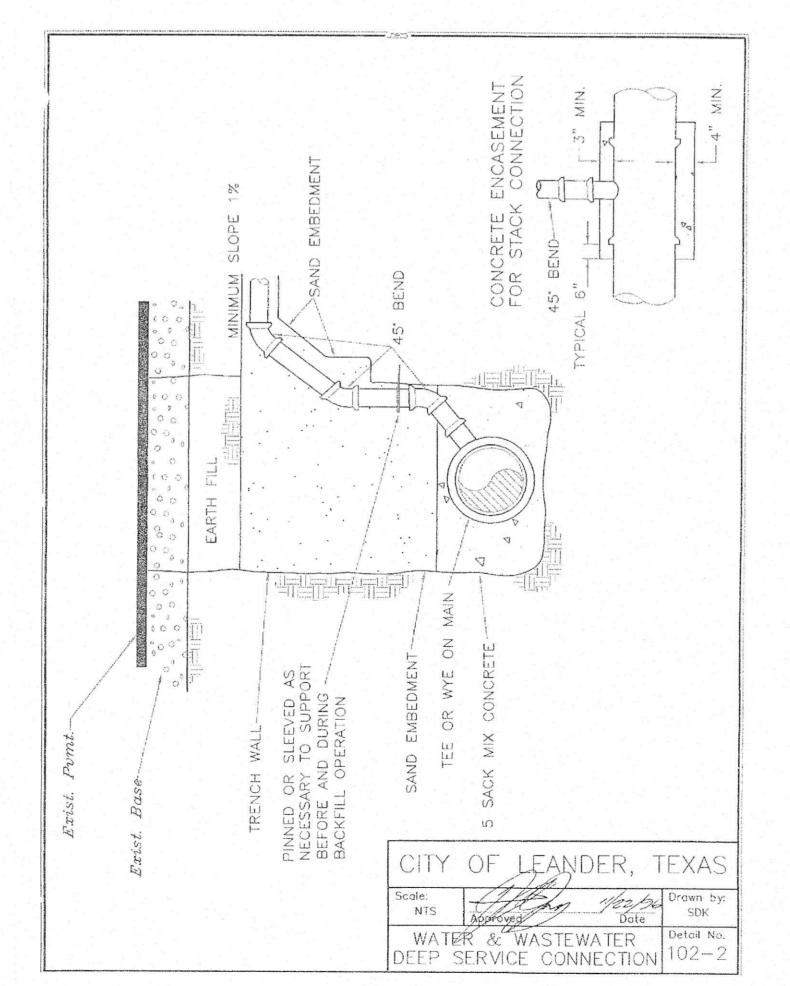


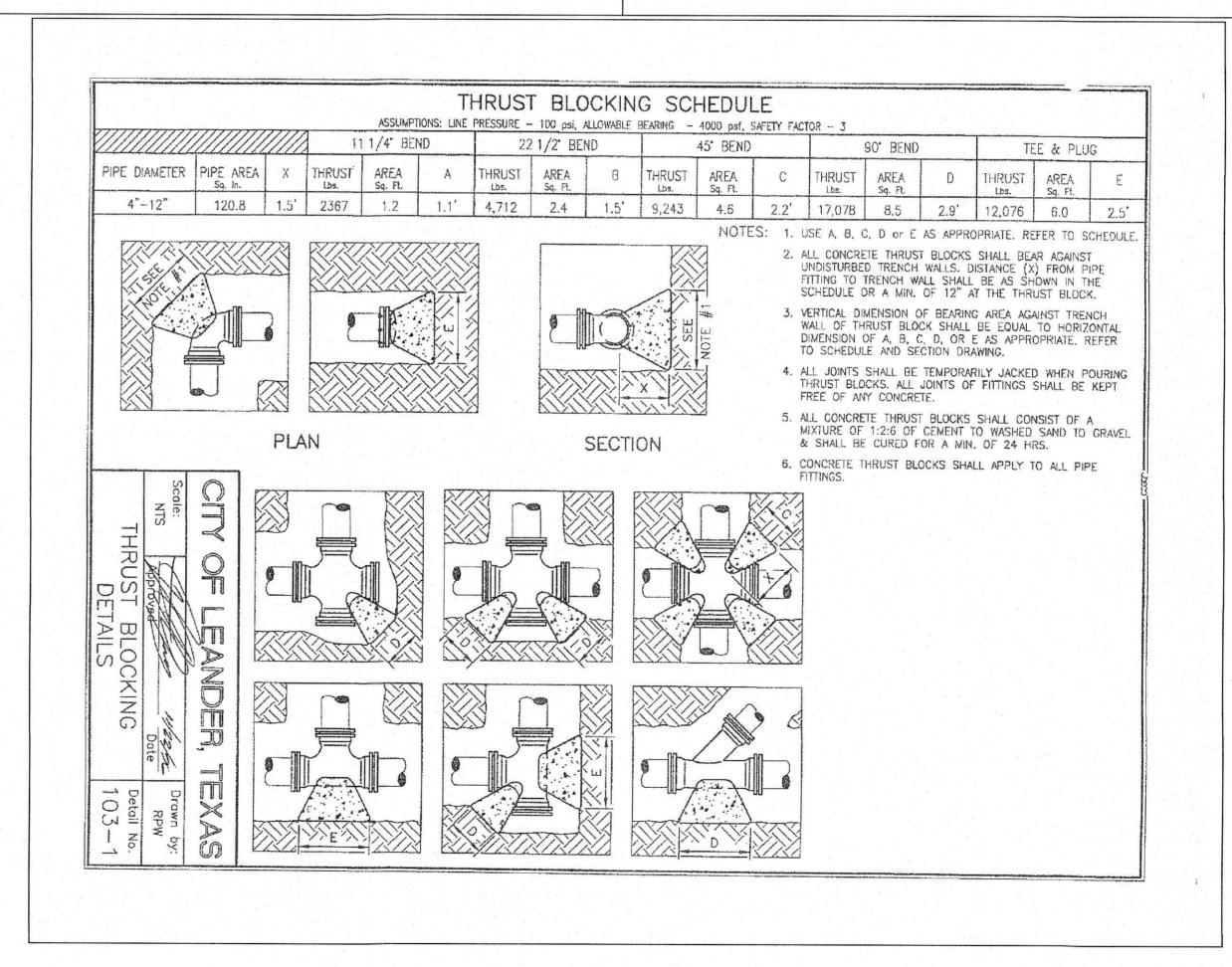




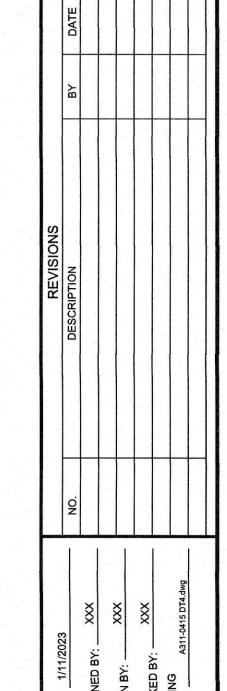


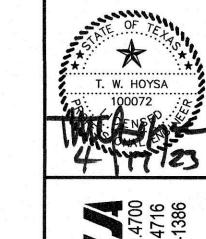
© 2-WAY CLEANOUTS, 6" Hxf.I.P. ADAPTER, REDUCER, AND 4" SERVICE LINE INSTALLED BY BLDG. PLUMBER. PERMITS DEPARTMENT SHALL INSPECT CUSTOMER'S WASTEWATER LINE AND ALL APPURTENANCES INSTALLED BY BUILDING PLUMBER.









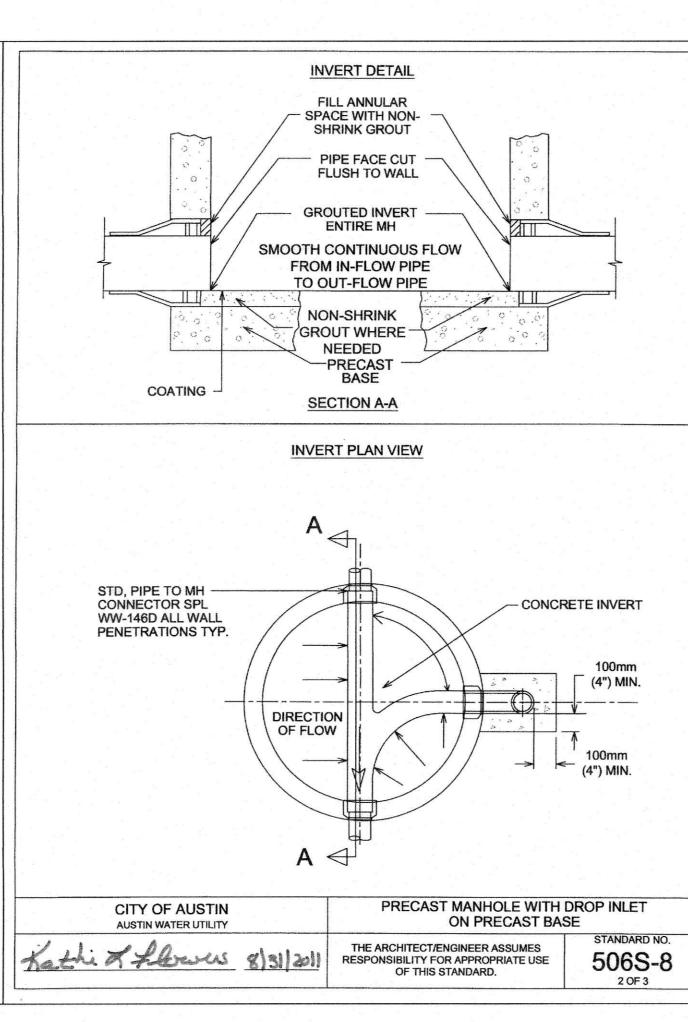


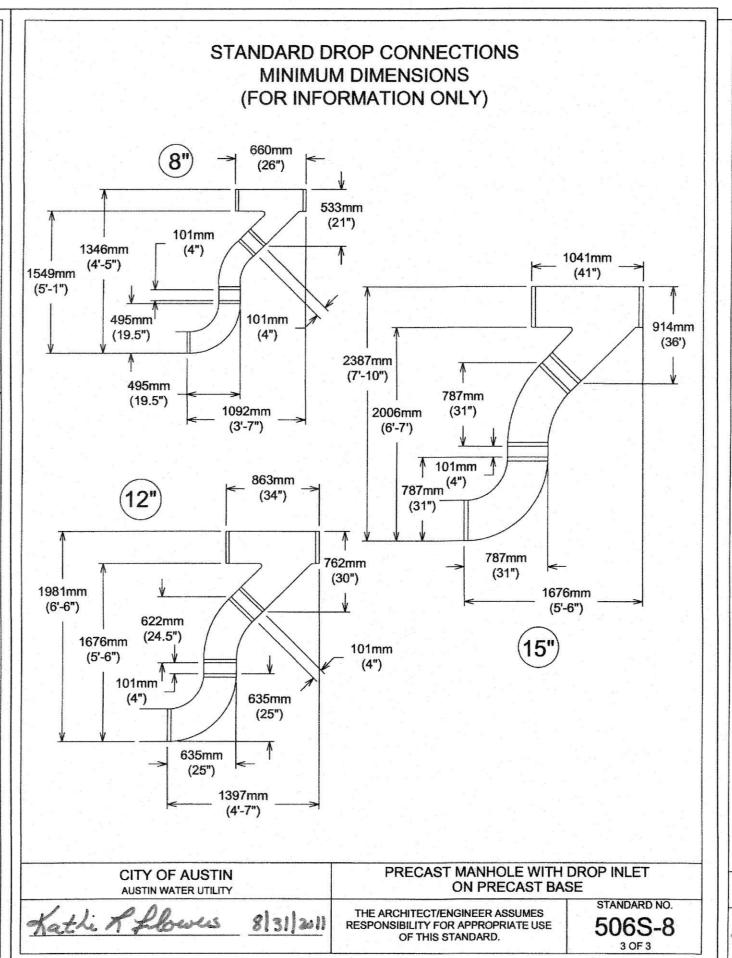


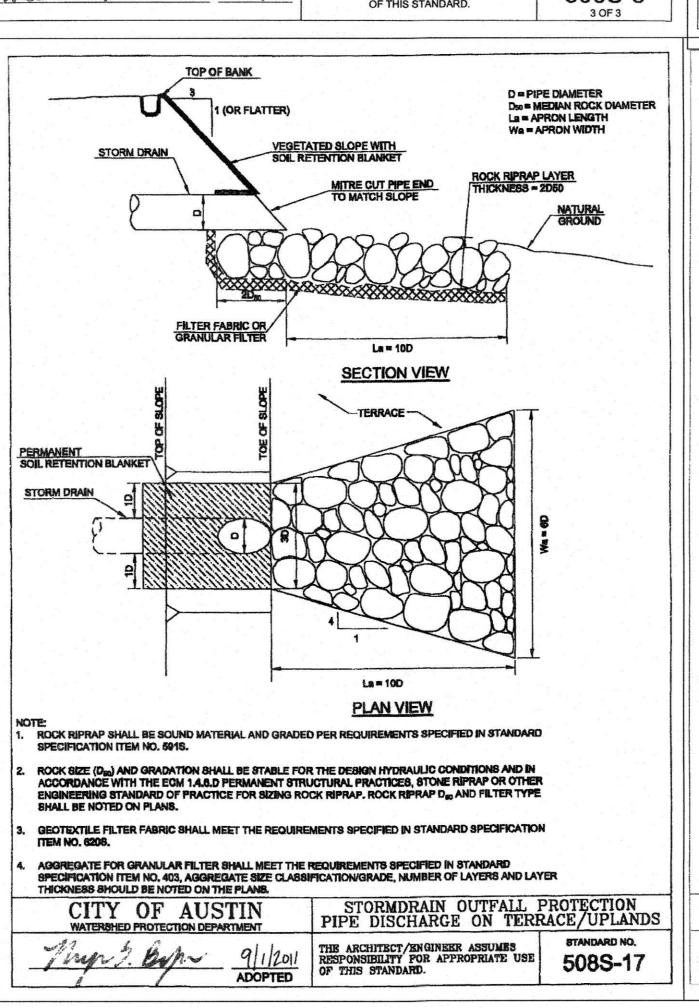
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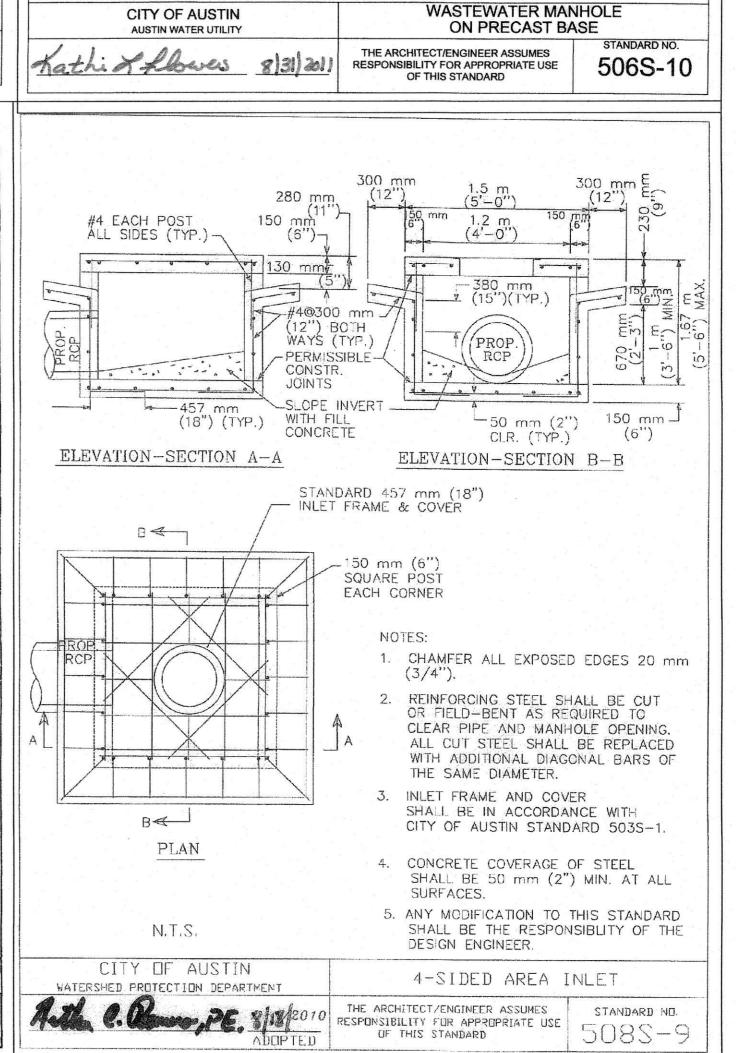
A311-0415

SHEET NO. of 75 SHEETS









INSTALL STANDARD CITY OF — AUSTIN RING AND 800 mm (32")

863mm (34")

±12.5mm (1/2")

1.20m (48")

INSIDE DIÀMETER

12" FOUNDATION

SLOPE

1. THE MANHOLE BASE SHALL BE BEDDED ON 150 mm (6") COARSE AGGREGATE. THE

MANHOLE RISER SECTIONS ON THE PRECAST CONCRETE BASE.

2. MH FOUNDATION SHALL MEET OR EXCEED STD. SPEC. 506.5B.

CONTRACTOR SHALL LEVEL AND PLUMB THE BASE PRIOR TO SETTING THE PRECAST

TYPICAL PRECAST BASE

PIPE TO M.H.

- CONNECTOR

12"

PRECAST BASE WITH

PER ASTM C-478

AND SPL WW-146

- "U" SHAPED

PER SPEC 506.4

STEEL REINFORCEMENT

SPL WW-146D

COVER, VARIES SEE STD.

DETAIL 506-4 OR 506-4A AS

PRECAST SECTION -

MANHOLE SECTION -JOINTS PER STD. NO.

PIPE TO M.H.

CONNECTOR

SPL WW-146D

150 mm (6") MIN. COARSE AGGREGATE

CITY OF AUSTIN

300 mm (12") ALL

AROUND M.H.

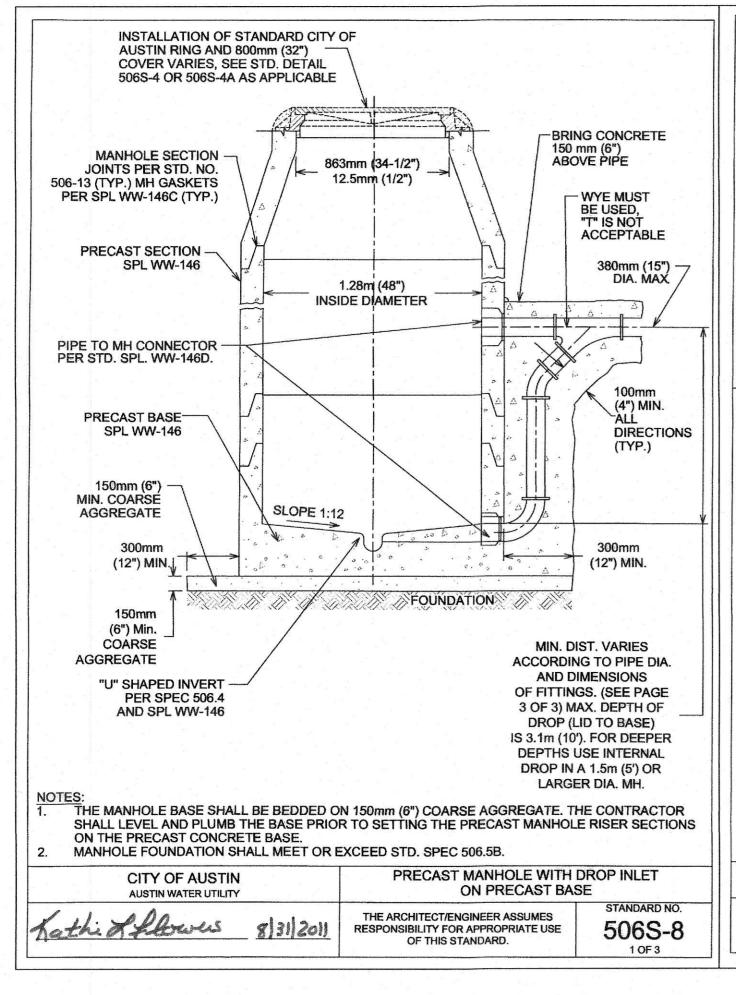
LAYER TO EXTEND MIN.

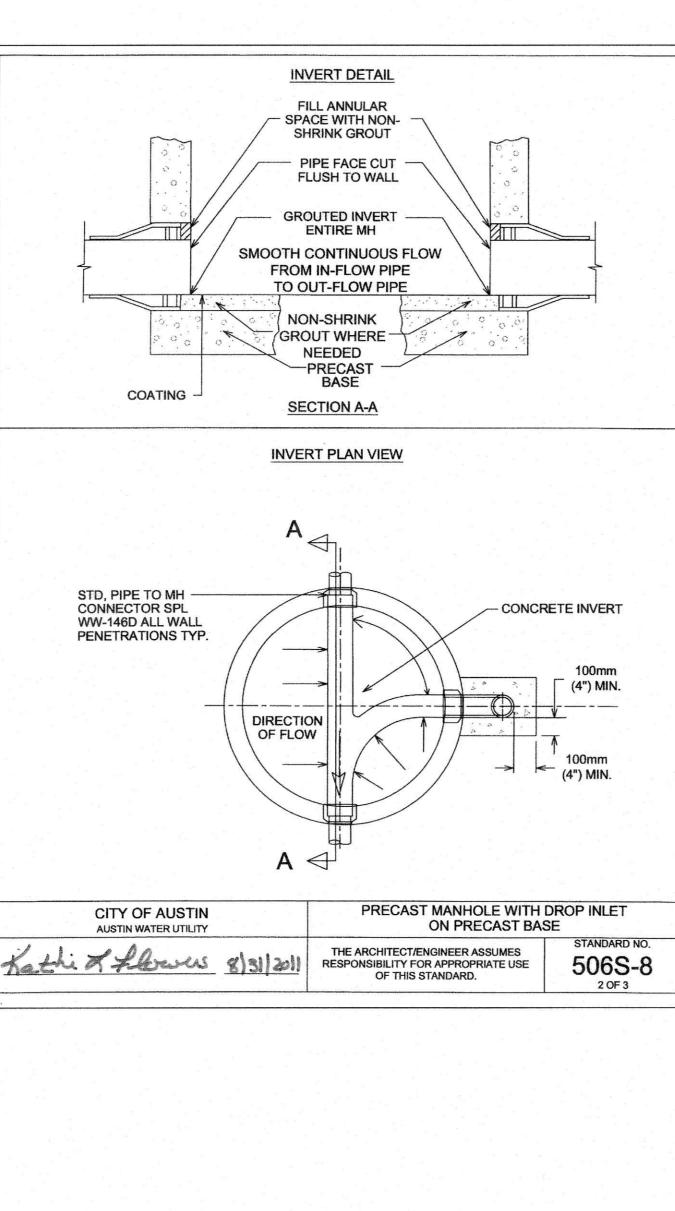
(18")min.

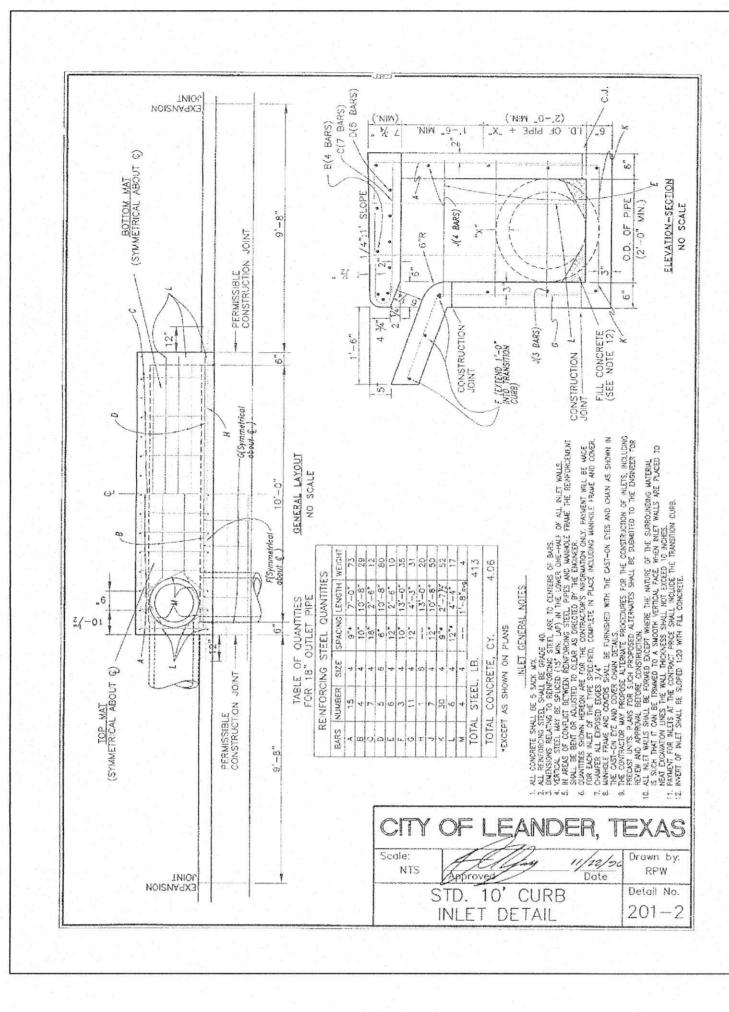
506-13 (TYP.) MH GASKETS PER SPL WW-146C (TYP.)

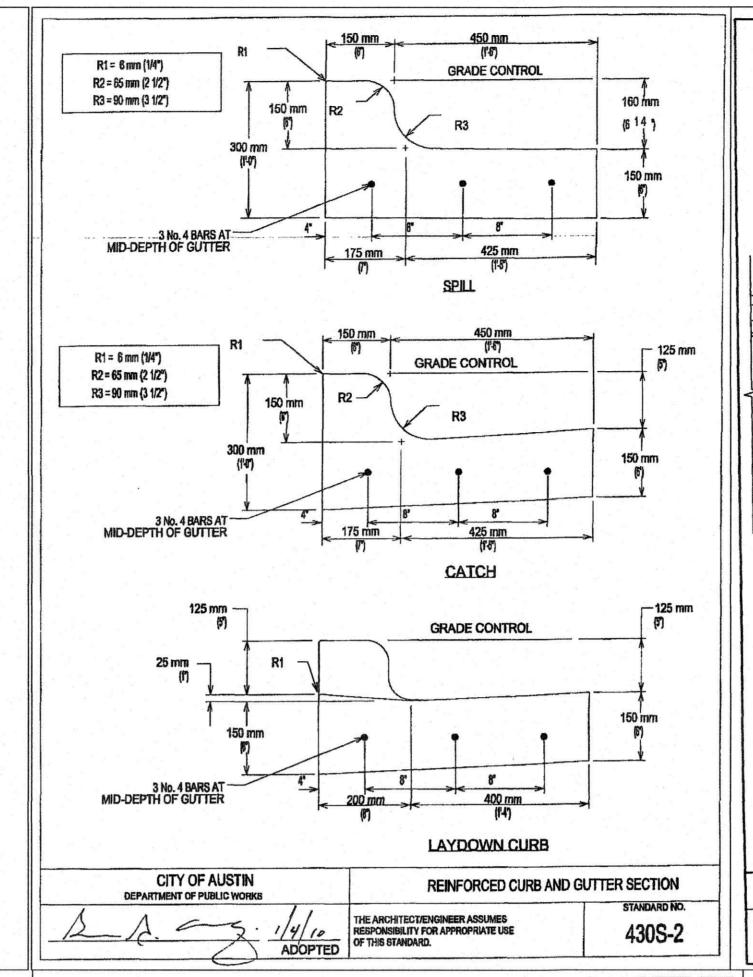
SPL WW-146

APPLICABLE









1. THIS STANDARD IS APPLICABLE FOR RAMP CONSTRUCTION WITHIN RIGHT-OF-WAY OR

2. THIS STANDARD REPRESENTS A TYPICAL CURB RAMP DESIGN FOR A NEWLY CONSTRUCTED INTERSECTION WITH A 6 m (20') RADIUS. LARGER RADII, STREET GRADES, LIMITED SIGHT DISTANCES, STREET CLASSIFICATION, SIGNALIZATION AND OTHER FACTORS MAY REQUIRE THE ENGINEER OR DESIGNATED REPRESENTATIVE TO MODIFY THIS STANDARD. SUFFICIENT RIGHT-OF-WAY SHALL BE DEDICATED SUCH THAT THE ENTIRE CURB RAMP, INCLUDING LANDING, AND THE INTER-CONNECTING SIDEWALK ARE CONTAINED WITHIN THE RIGHT-OF-WAY

3. THE CURB, GUTTER AND RAMP SYSTEM SHALL BE CONFIGURED TO MAINTAIN ALL RUNOFF FROM A 25 YEAR FREQUENCY STORM WITHIN THE RIGHT-OF-WAY (DRAINAGE CRITERIA MANUAL SECTION 1.2.2.B). WHEN THERE IS AN ELEVATION DIFFERENCE BETWEEN THE BOTTOM OF RAMPS (IE. ELEVATION AT THE GUTTER) WITHIN A QUADRANT OF AN INTERSECTION, THE LOWER ELEVATION RAMP SHALL BE CONSTRUCTED WITH A POSITIVE RAMP SLOPE OF 1:12 IN ACCORDANCE WITH STANDARDS 432S-5, 432S-5A

4. STANDARD ASSIGNMENT OF SIDEWALKS IS 600 mm (2') OFF PROPERTY LINE, EXCEPT AS INDICATED ON THE DRAWINGS.

5. ALTHOUGH CURB RAMPS MAY BE PLACED WITHIN THE RADIUS, PLACING THE RAMP OUTSIDE OF THE RADIUS WILL ALLOW FOR THE GREATEST DIFFERENCE IN ELEVATION BETWEEN THE RAMPS.

8. CURB RAMPS WILL BE PERPENDICULAR TO ROADWAY CENTERLINES AND SHALL ALIGN WITH EACH OTHER.

10. THE SLOPE OF THE SIDEWALK WHICH IS LOCATED WITHIN THE RADIUS AND CONNECTS TWO CURB RAMPS SHALL NOT EXCEED 1:20. THE DEVELOPER, AS PART OF THE CURB RAMP INSTALLATION, SHALL CONSTRUCT THIS CONNECTING SIDEWALK.

11. STRIPING AND SIGNAGE NOT REQUIRED IN ALL INSTANCES. STOP BARS, IF REQUIRED, SHALL BE LOCATED 1.2 m (4') FROM CROSSWALK.

13. CURB INLETS SHALL NOT BE LOCATED WITHIN 3.0 m (10') OF A CURB RAMP.

14. GUTTER SHALL PROVIDE SMOOTH TRANSITION TO RAMPS.

CITY OF AUSTIN

DEPARTMENT OF PUBLIC WORKS

12. IF A MEDIAN EXTENDS INTO THE CROSSWALK AREA, AN OPENING SHALL BE PROVIDED IN THE MEDIAN THE SAME WIDTH AS THE CURB RAMP.

TYPE 1 CURB RAMPS - "T" INTERSECTION

THE ARCHITECT/ENGINEER ASSUMES
RESPONSIBILITY FOR APPROPRIATE USE 4325-3A
OF THIS STANDARD.

THE ARCHITECT/ENGINEER ASSUMES
RESPONSIBILITY FOR APPROPRIATE USE 2 OF 2

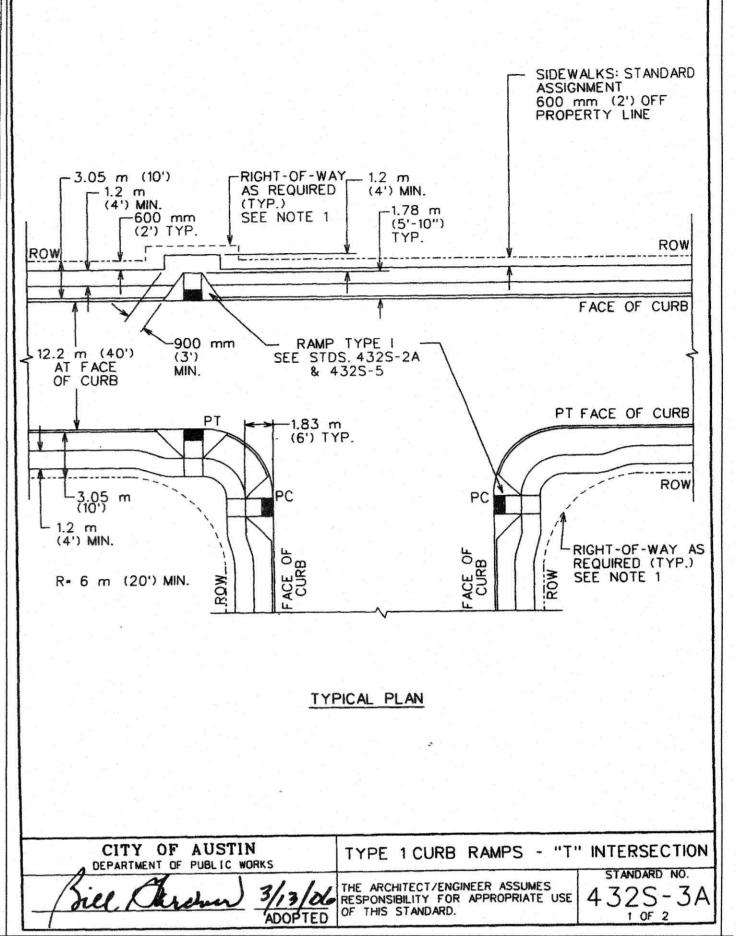
6. WINGS ARE REQUIRED ONLY IF PEDESTRIANS WOULD NORMALLY CROSS THE RAMP.

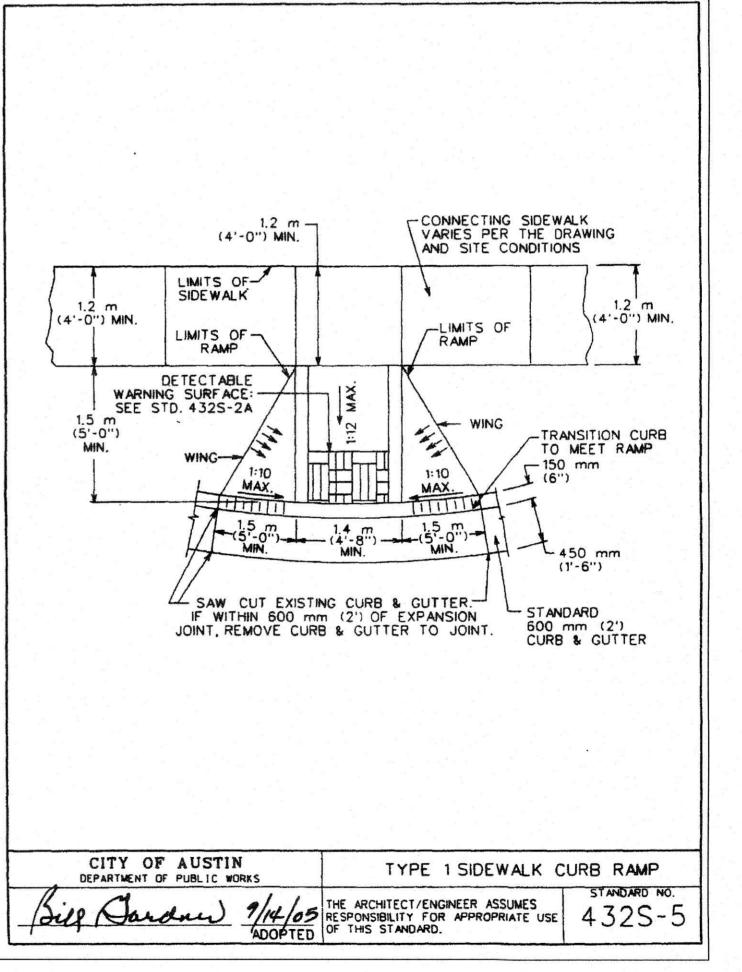
9. GRADES ON SIDEWALKS LEADING TO OR FROM THE RAMPS SHALL FOLLOW CURB GRADES.

7. LANDINGS SHALL BE FLAT AND MATCH PT, PC OR TOP OF CURB ELEVATION.

EASEMENT ONLY.

WITHIN THE RIGHT-OF-WAY.





XXXX-XX-XX

JOB NUMBER:

SHEET NO.

A311-0415

of 75 sheets

SUBDIVISION 17 & 8

PALMERA BLUFF SECTION

*

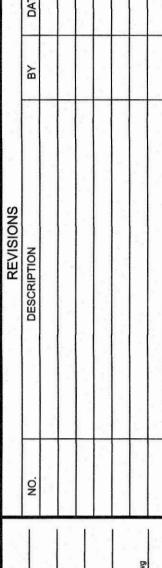
T. W. HOYSA

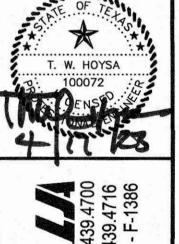
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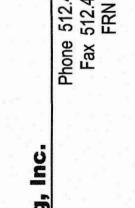
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DETAIL

GENERAL



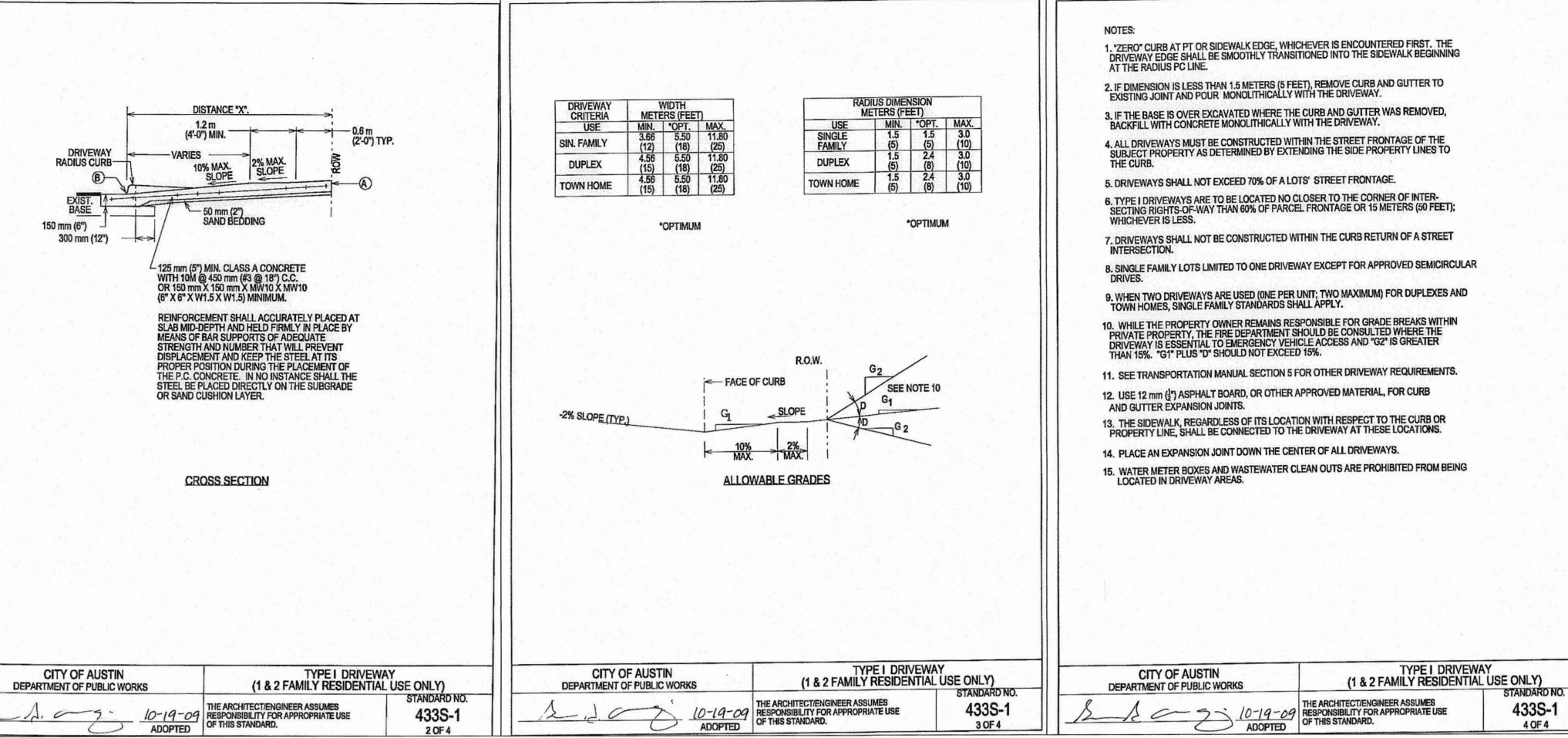


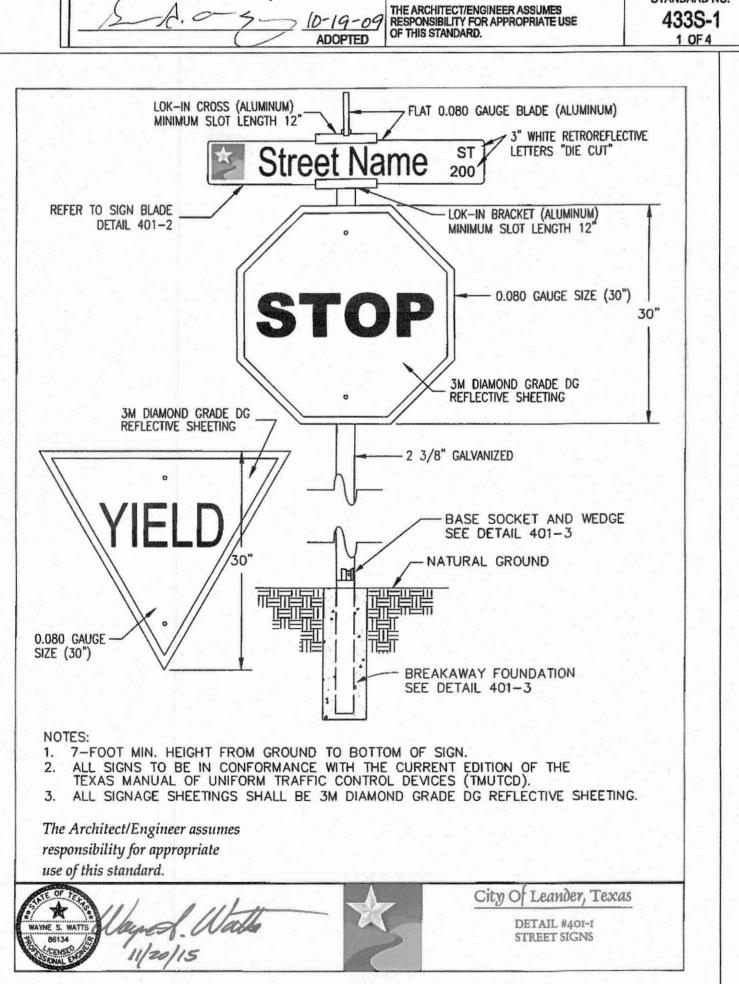


JOB NUMBER:

A311-0415

SHEET NO. 67 of 75 SHEETS





CENTERLINE EXPANSION JOINT SEE DETAIL BELOW AND NOTE 14

(2'-0") TYP.

R.O.W.

SIDEWALK TIE-IN - LOCATION.

SEE NOTE 1

- EXIST

SEE NOTE 13

- CENTERLINE EXPANSION

TYPE I DRIVEWAY
(1 & 2 FAMILY RESIDENTIAL USE ONLY)

STANDARD NO.

ON SHEET 4 OF 4

EXPANSION

CRITERIA CHART

TRANSITION TO ZERO CURB

PLAN

MATCH CATCH/SPILL GUTTER SECTION OF EXIST. CURB & GUTTER

SIDEWALK TIE-IN LOCATION.

INLET

CURB & GUTTER

NOTE: ALL DRIVEWAYS SHALL BE SLOPED TOWARDS
THE STREET FROM THE R.O.W. LINE.
ELEVATION OF POINT (A) ABOVE POINT (B) IS,
TYPICALLY A MINIMUM OF 150 mm (6") PLUS

CITY OF AUSTIN

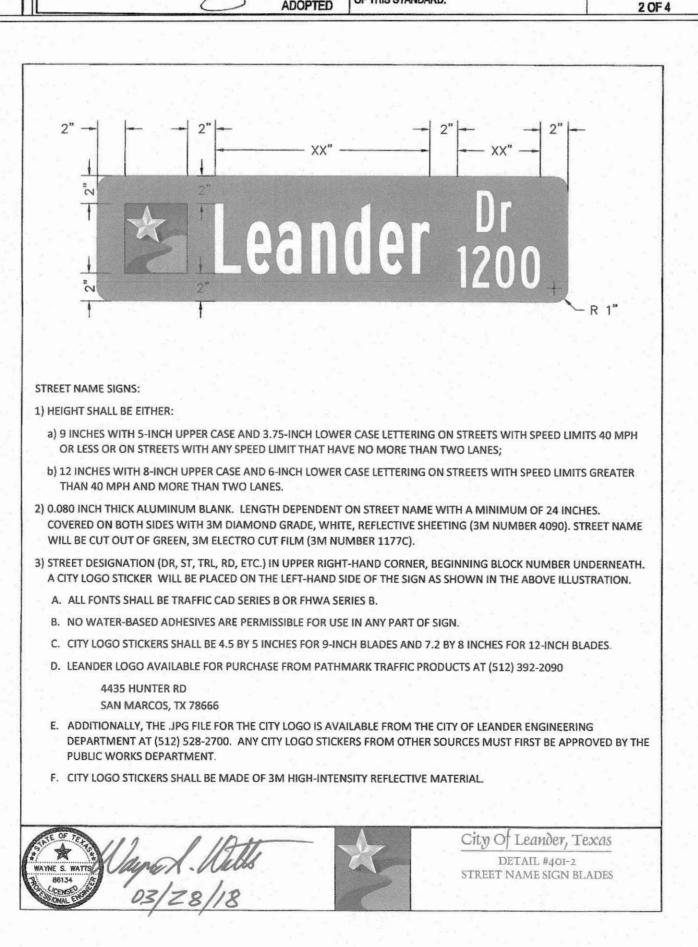
DEPARTMENT OF PUBLIC WORKS

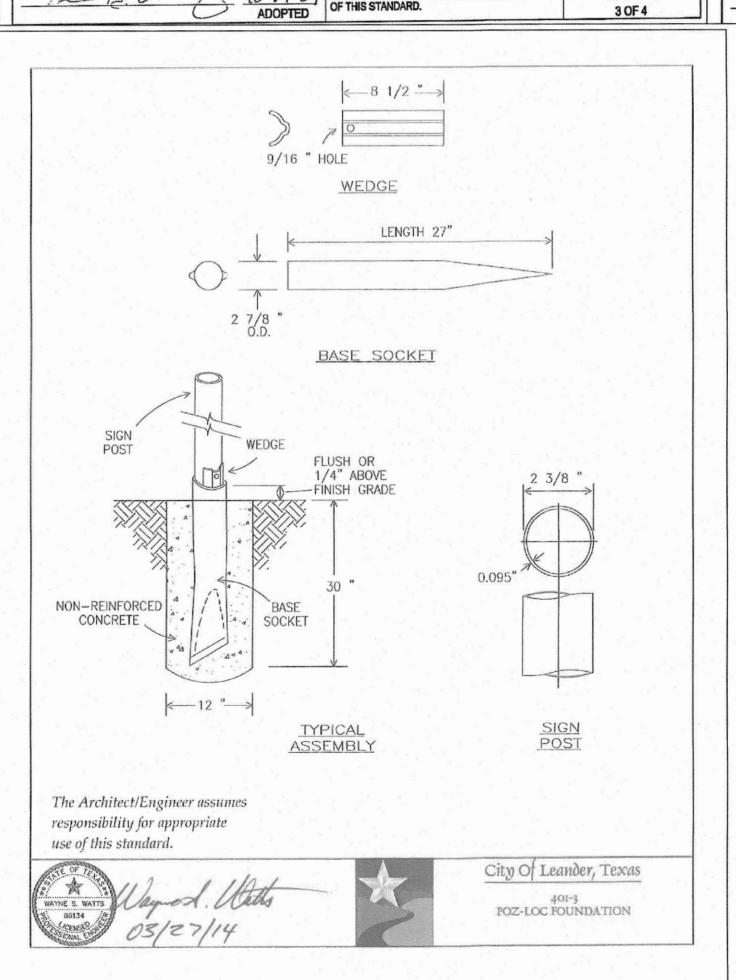
3 m (10'-0") MIN.

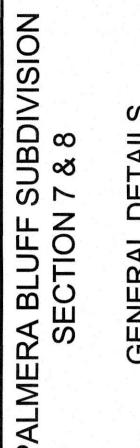
20 mm/m (4" RISE/FOOT) OVER DISTANCE "X" IN METERS (FEET).

__CURB & GUTTER

SEE NOTE 13







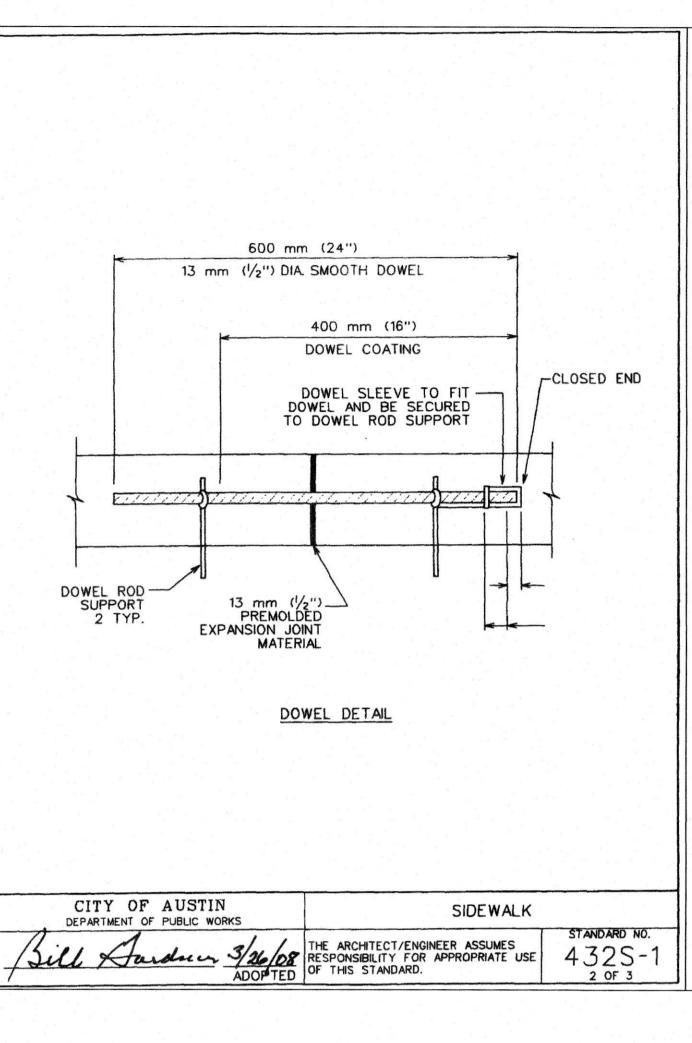




JOB NUMBER: A311-0415

SHEET NO.

of 75 sheets



- 600 mm

VARIES

PROPERTY LINE

EXPANSION

JOINT 1.5 m (5')

SECTION

CITY OF AUSTIN

12 m (40') MAX

CURB & GUTTER

1.2 m (4') MIN. RESIDENTIA

MIN. COMMERCIAL

(1/4"/ FT.) MAX.

XPANSION-

-SMOOTH DOWEL

SHEET 2 OF 2

-600 mm (2')

L100 mm

POLYPROPYLENE FIBRILLATED FIBERS, OR
150 mm X 150 mm X MW9 X MW9
(6" X 6" X W1.4 X W1.4) WELDED WIRE FABRIC
OR ONE LAYER 10M (*3) BARS PLACED NOT
MORE THAN 450 mm (18") C.C. BOTH
DIRECTIONS

REINFORCEMENT SHALL ACCURATELY PLACED AT SLAB MID-DEPTH AND HELD FIRMLY IN PLACE BY MEANS OF BAR SUPPORTS OF ADEQUATE STRENGTH AND NUMBER THAT WILL PREVENT DISPLACEMENT AND KEEP THE STEEL AT ITS PROPER POSITION DURING THE PLACEMENT OF THE P.C. CONCRETE. IN NO INSTANCE SHALL THE STEEL BE PLACED DIRECTLY ON THE SUBGRADE OR SAND CUSHION LAYER

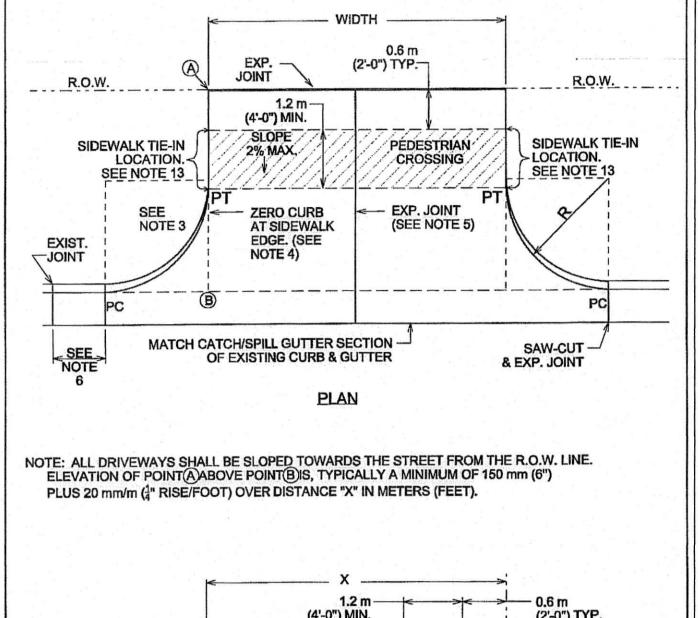
SIDEWALK

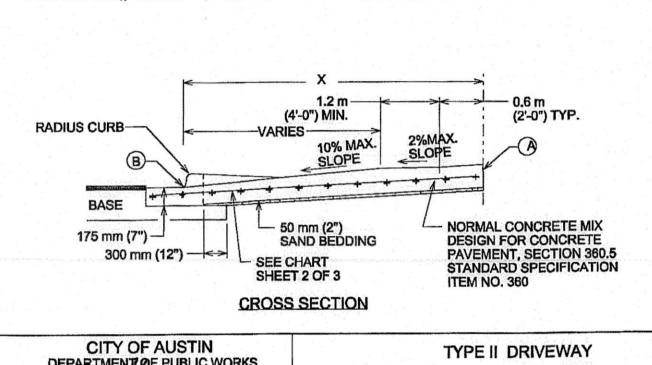
OR SAND CUSHION LAYER.

3/26/08
THE ARCHITECT/ENGINEER ASSUMES
RESPONSIBILITY FOR APPROPRIATE USE
OF THIS STANDARD.

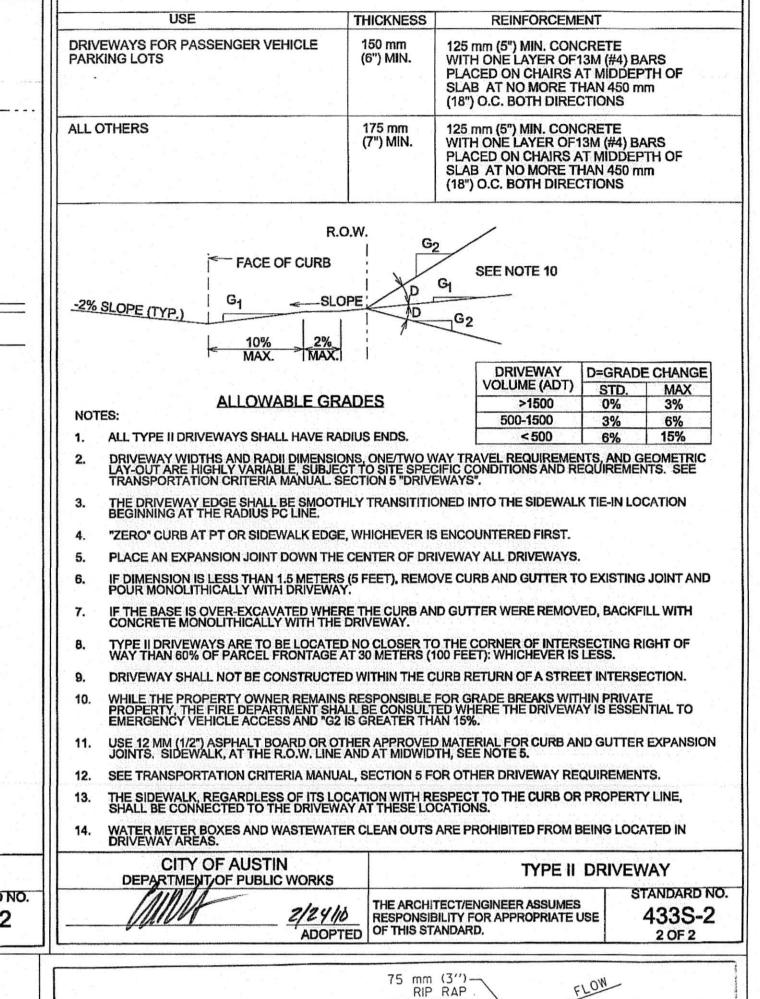
4 TYP.

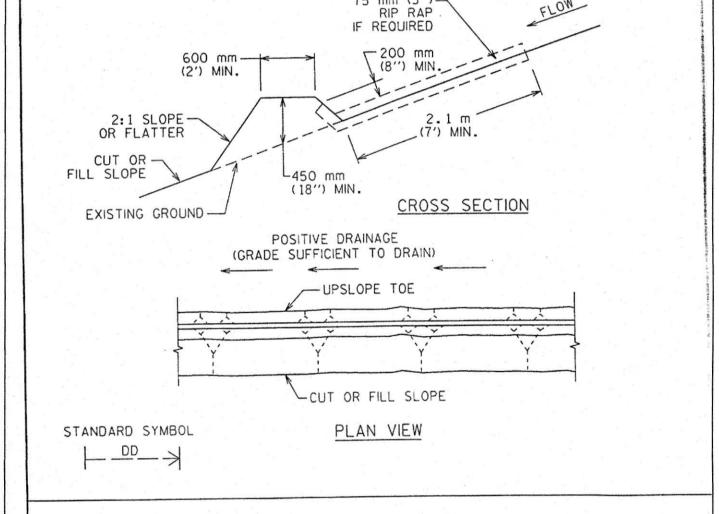
SEE DETAIL





CITY OF AU		TYPE II DRI	1000
MINH	2/24/16	THE ARCHITECT/ENGINEER ASSUMES RESPONSIBILITY FOR APPROPRIATE USE	433S-2
	ADOPTED	OF THIS STANDARD.	1 OF 2





GENERAL NOTES : 1. ALL DIKES SHALL BE MACHINE COMPACTED. 2. ALL DIVERSION DIKES SHALL HAVE POSITIVE DRAINAGE TO AN OUTLET. 3. d. DIVERTED RUNOFF FROM A PROTECTED OR STABILIZED AREA SHALL HAVE ITS OUTLET FLOW DIRECTED TO AN UNDISTURBED STABILIZED AREA OR INTO A LEVEL SPREADER OR GRADE STABILIZATION STRUCTURE. b. DIVERTED RUNOFF FROM A DISTURBED OR EXPOSED AREA SHALL BE CONVEYED TO A SEDIMENT TRAPPING DEVICE, SUCH AS A ROCK BERM, BRUSH BERM, STONE OUTLET STRUCTURE, SEDIMENT TRAP OR SEDIMENT BASIN OR TO AN AREA PRO-

TECTED BY ANY OF THESE PRACTICES. 4. UNLESS OTHERWISE SPECIFIED, EROSION STABILIZATION SHALL BE OPEN GRADED

ROCK 75 TO 125 mm (3 TO 5 inches) IN DIAMETER EMBEDDED IN SOIL SURFACE. 5. INSPECTION SHALL BE CONDUCTED WEEKLY OR AFTER EACH RAINFALL EVENT.

> CITY OF AUSTIN DIVERSION DIKE THE ARCHITECT/ENGINEER ASSUMES
> RESPONSIBILITY FOR APPROPRIATE USE

WATERSHED PROTECTION DEPARTMENT

STANDARD NO.

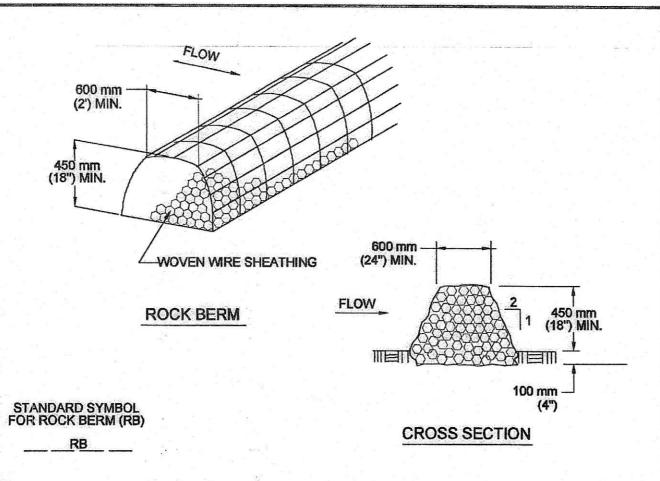
1. INSTALL 4" DIAMETER STAINLESS STEEL "NO DUMPING - DRAINS TO CREEK" MARKER, FACTORY PAINTED BLUE AS SHOWN, 2" ABOVE VERTICAL FACE OF INLET AT MIDPOINT OF ALL INLETS. 2. MARKER SHALL BE AFFIXED TO SURFACE WITH ADHESIVE PER MANUFACTURER'S

RECOMMENDATIONS. . MARKER SHALL BE MANUFACTURED BY ALMATEK INDUSTRIES OR APPROVED EQUAL: ALMATEK INDUSTRIES, INC. 2 JOY DRIVE

HACKETTSTOWN, NJ 07840 (800) 248-2080 WWW.ALMATEK.COM



City Of Leander, Texas STORM DRAIN MARKERS



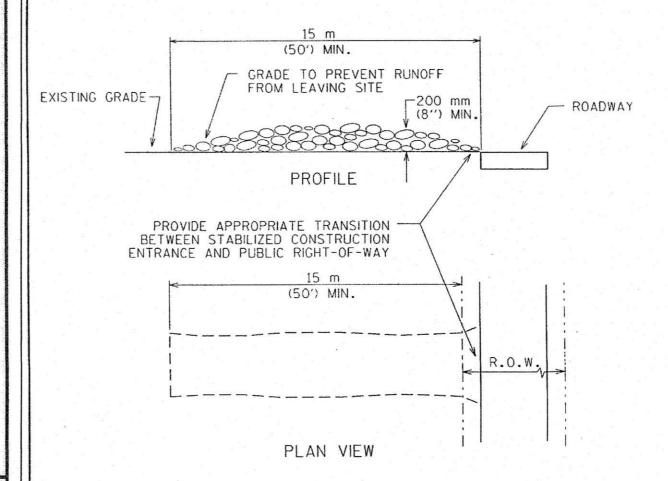
1. USE ONLY OPEN GRADED ROCK 75 to 125 mm (3 to 5") DIAMETER FOR ALL CONDITIONS.

2. THE ROCK BERM SHALL BE SECURED WITH A WOVEN WIRE SHEATHING HAVING MAXIMUM 25 mm (1") OPENING AND MINIMUM WIRE DIAMETER OF 12.9 mm (20 GAUGE). 3. THE ROCK BERM SHALL BE INSPECTED DAILY OR AFTER EACH RAIN, AND THE STONE AND/OR FABRIC CORE-WOVEN SHEATHING SHALL BE REPLACED WHEN THE STRUCTURE CEASES TO FUNCTION AS INTENDED, DUE TO SEDIMENT ACCUMULATION

AMONG THE ROCKS, WASHOUT, CONSTRUCTION TRAFFIC DAMAGE, ETC. 4. IF SEDIMENT REACHES A DEPTH EQUAL TO ONE-THIRD THE HEIGHT OF THE BERM OR 150 mm (6"), WHICHEVER IS LESS, THE SEDIMENT SHALL BE REMOVED AND DISPOSED OF ON AN APPROVED SITE AND IN A MANNER THAT WILL NOT CREATE A SEDIMENTION

5. WHEN THE SITE IS COMPLETELY STABILIZED, THE BERM AND ACCUMULATED SEDIMENT SHALL BE REMOVED AND DISPOSED OF IN AN APPROVED MANNER.

CITY OF AUSTIN WATERSHED PROTECTION DEPARTMENT **ROCK BERM** 8/24/2010
ADOPTED
THE ARCHITECT/ENGINEER ASSUMES
RESPONSIBILITY FOR APPROPRIATE USE
OF THIS STANDARD.



1. STONE SIZE: 75-125 mm (3-5") OPEN GRADED ROCK. 2. LENGTH: AS EFFECTIVE BUT NOT LESS THAN 15 m (50').

3. THICKNESS: NOT LESS THAN 200 mm (8").

4. WIDTH: NOT LESS THAN FULL WIDTH OF ALL POINTS OF INGRESS/EGRESS.

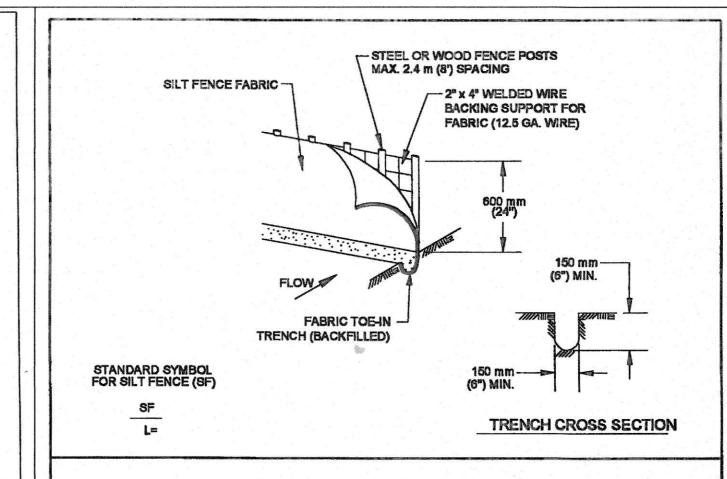
WASHING: WHEN NECESSARY, VEHICLE WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO PUBLIC ROADWAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE AND DRAINS INTO AN APPROVED TRAP OR SEDIMENT BASIN. ALL SEDIMENT SHALL BE PREVENTED FROM ENTERING ANY

STORM DRAIN, DITCH OR WATERCOURSE USING APPROVED METHODS. . MAINTENANCE: THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC ROADWAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND, AS WELL AS REPAIR AND CLEAN OUT OF ANY MEASURE DEVICES USED TO TRAP SEDIMENT. ALL SEDIMENTS THAT IS SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC

ROADWAY MUST BE REMOVED IMMEDIATELY. DRAINAGE: ENTRANCE MUST BE PROPERLY GRADED OR INCORPORATE A DRAINAGE SWALE TO PREVENT RUNOFF FROM LEAVING THE CONSTRUCTION SITE.

STANDARD NO.

STABILIZED CONSTRUCTION ENTRANCE CITY OF AUSTIN 5123 RESPONSIBILITY FOR APPROPRIATE USE



1. STEEL OR WOOD POSTS WHICH SUPPORT THE SILT FENCE SHALL BE INSTALLED ON A SLIGHT ANGLE TOWARD THE ANTICIPATED RUNOFF SOURCE. POST MUST BE EMBEDDED A MINIMUM OF 300 mm (12 INCHES). IF WOOD POSTS CANNOT ACHIEVE 300 mm (12 Inches) DEPTH, USE STEEL POSTS.

2. THE TOE OF THE SILT FENCE SHALL BE TRENCHED IN WITH A SPADE OR MECHANICAL TRENCHER, SO THAT THE DOWNSLOPE FACE OF THE TRENCH IS FLAT AND PERPENDICULAR TO THE LINE OF FLOW.

3. THE TRENCH MUST BE A MINIMUM OF 150 mm (6 Inches) DEEP AND 150 mm (6 Inches) WIDE TO ALLOW FOR THE SILT FENCE FABRIC TO BE LAID IN THE GROUND AND BACKFILLED WITH COMPACTED

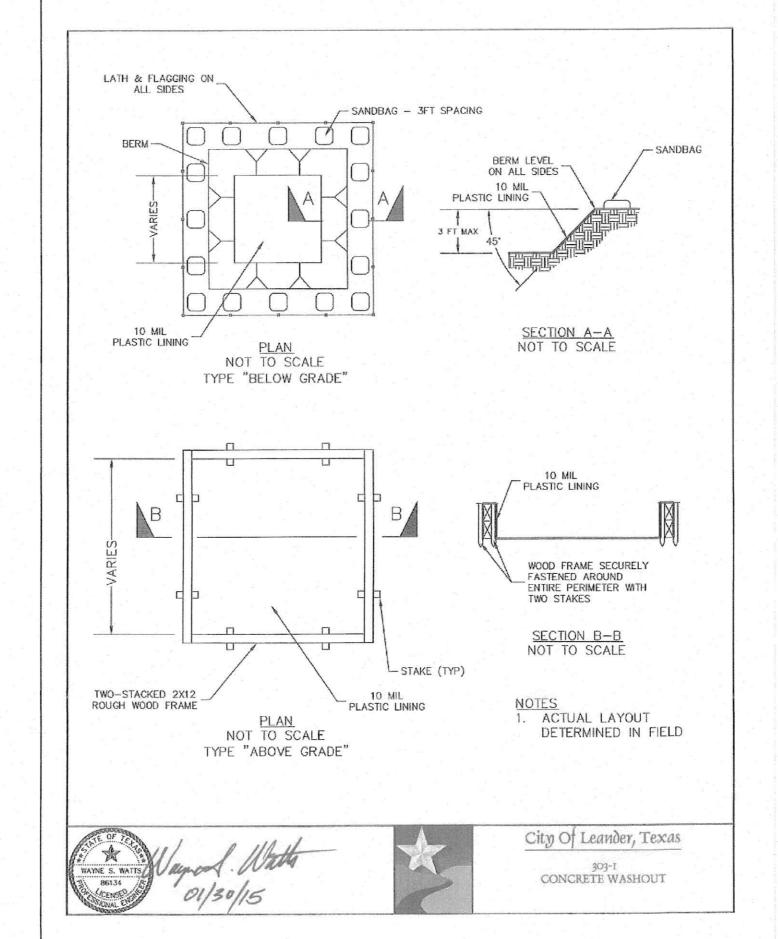
4. SILT FENCE FABRIC SHOULD BE SECURELY FASTENED TO EACH STEEL OR WOOD SUPPORT POST OR TO WOVEN WIRE, WHICH IS IN TURN ATTACHED TO THE STEEL OR WOOD FENCE POST.

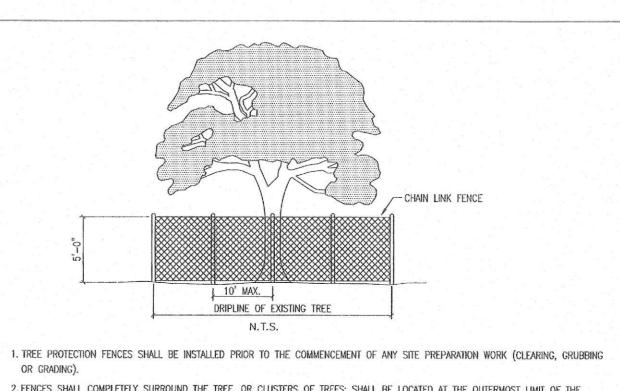
5. INSPECTION SHALL BE MADE WEEKLY OR AFTER EACH RAINFALL EVENT AND REPAIR OR REPLACEMENT SHALL BE MADE PROMPTY AS NEEDED.

8. SILT FENCE SHALL BE REMOVED WHEN THE SITE IS COMPLETELY STABILIZED SO AS NOT TO BLOCK OR IMPEDE STORM FLOW OR DRAINAGE.

7. ACCUMULATED SILT SHALL BE REMOVED WHEN IT REACHES A DEPTH OF 150 mm (6 inches). THE SILT SHALL BE DISPOSED OF ON AN APPROVED SITE AND IN SUCH A MANNER THAT WILL NOT CONTRIBUTE TO ADDITIONAL SILTATION.

CITY OF AUSTIN WATERSHED PROTEOTION DEPARTMENT	SILT FENCE	
My 3. Ry 9/1/2011 ADOPTED	THE ARCHITECT/ENGINEER ASSUMES RESPONSIBILITY FOR APPROPRIATE USE OF THIS STANDARD.	8TANDARD NO. 642S-1





2. FENCES SHALL COMPLETELY SURROUND THE TREE, OR CLUSTERS OF TREES; SHALL BE LOCATED AT THE OUTERMOST LIMIT OF THE TREE BRANCHES (DRIPLINE), AND SHALL BE MAINTAINED THROUGHOUT THE CONSTRUCTION PROJECT IN ORDER TO PREVENT THE

A. SOIL COMPACTION IN THE ROOT ZONE AREA RESULTING FROM VEHICULAR TRAFFIC, OR STORAGE OF EQUIPMENT OR MATERIALS. B. ROOT ZONE DISTURBANCES DUE TO GRADE CHANGES (GREATER THAN SIX INCHES (6")) CUT OR FILL, OR TRENCHING NOT

REVIEWED AND AUTHORIZED BY THE CITY. C. WOUNDS TO EXPOSED ROOTS, TRUNKS OR LIMBS BY MECHANICAL EQUIPMENT.

D. OTHER ACTIVITIES DETRIMENTAL TO TREES, SUCH AS CHEMICAL STORAGE, CEMENT TRUCK CLEANING AND FIRE.

3. EXCEPTIONS TO INSTALLING FENCES AT TREE DRIPLINES MAY BE PERMITTED IN THE FOLLOWING CASES:

A. WHERE PERMEABLE PAVING IS TO BE INSTALLED, ERECT THE FENCE AT THE OUTER LIMITS OF THE PERMEABLE PAVING AREA. B. WHERE TREES ARE CLOSE TO PROPOSED BUILDINGS, ERECT THE FENCE NO CLOSER THAN SIX FEET (6'-0") TO BUILDING. 4. CRITICAL ROOT ZONE REQUIREMENTS

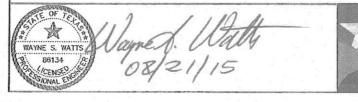
A. NO CONSTRUCTION OR DISTURBANCE SHALL OCCUR WITHIN AN AREA THAT CONSTITUTES MORE THAN FIFTY (50%) OF THE TOTAL CRITICAL ROOT ZONE AND ONE HALF THE RADIAL DISTANCE OF THE CRITICAL ROOT ZONE FOR EACH TREE BEING PRESERVED INCLUDING SIGNIFICANT TREES, HERITAGE TREES, AND ANY OTHER TREES FOR WHICH PRESERVATION IS TO BE CREDITED. THE REMAINING CRITICAL ROOT ZONE SHALL CONSIST OF AT LEAST ONE HUNDRED (100) SQUARE FEET.

B. THIS DEFINED AREA SHALL BE FLAGGED AND ENCIRCLED WITH PROTECTIVE FENCING DURING CONSTRUCTION. THE PLANNING DIRECTOR MAY APPROVE CONSTRUCTION CLOSER TO THE TRUNK THAN ONE HALF (1/2) THE RADIAL DISTANCE, DEPENDING ON THE SIZE, SPACING, OR SPECIES OF THE TREE, THE TYPE OF DISTURBANCE PROPOSED, AND UNIQUENESS OF THE SITUATION.

C. CUT OR FILL THAT IS GREATER THAN FOUR (4) INCHES IN DEPTH AND THE SEVERING OF MAJOR ROOTS SHALL BE CONSIDERED DISTURBANCE FOR THE PURPOSES OF THIS ORDINANCE,

D. WITHIN THE PROTECTED CRITICAL ROOT ZONE, ONLY FLATWORK, DECKING, OR SIMILAR CONSTRUCTION, MAY BE APPROVED AND

SHALL NOT AFFECT THE BRANCHING OF THE TREE. E. IF PROPOSED OR ACTUAL PROTECTION OF THE CRITICAL ROOT ZONE OF A TREE DOES NOT MEET THE REQUIREMENTS OF THIS SECTION, THEN THE TREE SHALL BE CONSIDERED REMOVED AND SHALL REQUIRE MITIGATION IN ACCORDANCE WITH THIS ORDINANCE.





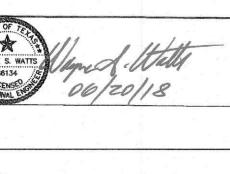
City Of Leander, Texas TREE PROTECTION

JOB NUMBER: A311-0415 SHEET NO.

T. W. HOYSA

of 75 SHEETS







STREET LIGHT ASSEMBLY MODEL INFORMATION:

LUMINARIES: AS REQUIRED BY CITY OF LEANDER. REFER TO THE DETAIL.

POLE: HAPCO 1-B17802 STREETLIGHT POLE ASSEMBLY, ROUND TAPERED ALUMINUM, 11" BOLT HOLE CIRCLE, 14" DIAMETER ROUND FLAT PLATE ON BOTTOM, SUPPLIED WITH FOUR(4) 1"DIA.x2-1/2"LONG HEX HEAD BOLTS WITH SPLIT LOCKWASHERS FACTORY INSTALLED, ALL HOT DIPPED GALVANIZED. ALL LIGHT POLES SHALL BE PROVIDED WITH VIBRATION

FOUNDATION: PROVIDE A DIRECT EMBEDDED STREET LIGHT FOUNDATION, 8.625" OD BY 58 5/8" LONG SHAFT, 1" THICK BY 12" SQUARE BASE PLATE WITH BOLT HOLE CIRCLE TO MATCH THE POLE. 12" DIAMETER ROUND FLAT PLATE ON BOTTOM SUPPLIED WITH FOUR 1" DIAMETER BY 2 1/2" LONG HEX HEAD BOLTS WITH LOCK WASHERS. ALL MATERIAL TO BE HOT DIP GALVANIZED. REFER TO CITY OF LEANDER STANDARD LIGHTING POLE FOUNDATION DETAIL #402-2 ON ES103 FOR ALL REQUIREMENTS. THE POLE FOUNDATION SHALL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF TEXAS.

CONTROLS: REFER TO CITY OF LEANDER DETAIL #402-7 & 402-8 COL RESIDENTIAL ROADWAY LIGHTING STANDARDS FOR CONTROL WIRING.

LAMP: AS REQUIRED BY CITY OF LEANDER. REFER TO THE DETAIL.

FUSING: COOPER BUSSMAN "BREAKAWAY" TYPE FUSE HOLDERS WITH 3 AMP

STREET LIGHT JOINT TRENCH NOTE:

1) STREET LIGHT CONDUIT SHALL BE INSTALLED IN A JOINT TRENCH WITH PEC UTILITIES AS MUCH AS POSSIBLE. REFER TO PEC SPECIFICATIONS FOR PLACEMENT REQUIREMENTS AND SEPARATIONS.

STREET LIGHT NOTES:

1) CONTRACTOR SHALL PROVIDE ALL REQUIRED MATERIAL AND LABOR. UNLESS NOTED OTHERWISE TO ENSURE INSTALLATIONS MEET ALL CURRENT ELECTRIC UTILITY, LOCAL, STATE, NEC, & NESC REQUIREMENTS FOR A COMPLETE AND ENERGIZED SYSTEM.

2) CONTRACTOR SHALL INSTALL SERVICE CONDUCTOR ASSEMBLIES AS REQUIRED BY CITY OF LEANDER/PEC FROM PEC EQUIPMENT TO THE METERS THROUGH 2" CONDUIT INSTALLED BY CONTRACTOR. 2" CONDUIT SHALL BE INSTALLED FROM PEC CLOSEST TRANSFORMER OR SECONDARY ENCLOSURE TO METER SERVICE LOCATIONS.

3) ALL POLE FOUNDATION LOCATIONS ARE SHOWN BASED ON UTILITY INFORMATION AVAILABLE. CONTRACTOR SHALL VERIFY THAT NO CONFLICTS EXIST PRIOR TO THE PLACEMENT OF POLE FOUNDATIONS AND CONDUITS.

SITE LIGHTING DETAIL SCALE: NOT TO SCALE

ELECTRICAL LEGEND

(NOTE: ALL SYMBOLS SHOWN ARE NOT NECESSARILY USED ON DRAWINGS)

SYMBOL LEGEND LIGHTING POLE MOUNTED LIGHT FIXTURE AS SPECIFIED. PROVIDE PER THE CITY OF LEANDER'S SPECIFICATIONS.

CONDUIT AND WIRE BRANCH CIRCUIT 3/4°C -2#8 & 1#10 GROUND.

DISTRIBUTION & CONTROLS

JUNCTION BOX WITH COVER PLATE EQUIPMENT CONNECTION. (PROVIDE ALL BRANCH CIRCUITRY REQUIRED TO CONNECT TO EQUIPMENT)

PEC PAD MOUNTED TRANSFORMER ELECTRICAL METER

AMPERE INTERRUPTING CAPACITY AMERICAN WIRE GAUGE THOUSAND VOLT AMPERE NATIONAL ELECTRICAL CODE NATIONAL ELECTRICAL MANUFACTURERS ASSOCIATION XFMR TRANSFORMER

ABBREVIATIONS

ELECTRICAL SPECIFICATIONS

GENERAL PROVISIONS

THE WORK COVERED SHALL INCLUDE THE FURNISHING OF ALL MATERIALS, LABOR, TRANSPORTATION, TOOLS, PERMITS, FEES, AND INCIDENTALS NECESSARY FOR THE COMPLETE INSTALLATION OF ALL ELECTRICAL WORK REQUIRED IN THE CONTRACT DOCUMENTS, THE INTENT OF THE OF THE CONTRACT DOCUMENTS IS TO PROVIDE AN INSTALLATION COMPLETE IN EVERY RESPECT. IN THE EVENT THAT ADDITIONAL DETAILS OR SPECIAL CONSTRUCTION MAY BE REQUIRED FOR THE WORK INDICATED, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOS TO PROVIDE ALL MATERIAL AND LABOR WHICH IS USUALLY FURNISHED WITH SUCH SYSTEMS IN ORDER TO MAKE THE INSTALLATION COMPLETE AND OPERATIVE. ELEMENTS OF THE WORK SHALL INCLUDE, BUT ARE NOT LIMITED TO, MATERIALS, LABOR, SUPERVISION, SUPPLIES, EQUIPMENT, TRANSPORTATION, HOISTING/RIGGING, STORAGE, UTILITIES, AND ALL REQUIRED PERMITS AND LICENSES.

DRAWINGS ARE SCHEMATIC IN NATURE AND DO NOT NECESSARILY REFLECT ALL WORK REQUIRED TO COMPLETE PROJECT. CONTRACTOR SHALL PROVIDE ALL MATERIALS, LABOR AND EQUIPMENT AS REQUIRED COMPLETE PROJECT WITHIN DESIGN INTENT AT NO ADDITIONAL COST TO OWNER OR TENANT. CONTRACTOR SHALL REQUEST ADDITIONAL INFORMATION IN CASES OF DOUBT

- 3. CONSIDERATION SHALL NOT BE GRANTED FOR MISUNDERSTANDING OF THE SCOPE OR AMOUNT OF WORK TO BE PERFORMED. TENDER OF A PROPOSAL CONVEYS FULL CONTRACTOR AGREEMENT OF THE ITEMS AND CONDITIONS SPECIFIED AND/OR INDICATED, SCHEDULED, OR IMPLIED ON THE CONTRACT DOCUMENTS, AND/OR REQUIRED BY THE NATURE OF THIS WORK.
- 4. ALL WORK IS TO BE PERFORMED BY A LICENSED MASTER ELECTRICIAN PER THE TEXAS ELECTRICAL SAFETY AND LICENSING

DIVISION 26 - ELECTRICAL

26010 ELECTRICAL GENERAL PROVISIONS WORK AND MATERIAL SHALL COMPLY WITH THE LATEST RULES AND REGULATIONS OF THE 2020 NATIONAL ELECTRICAL CODE, THE LOCAL ELECTRICAL CODE AMENDMENTS, NATIONAL ELECTRICAL SAFETY CODE, OCCUPATIONAL SAFETY AND HEALTH ACT; 2015 INTERNATIONAL ENERGY CODE; AND ALL FEDERAL AND STATE CODES, ORDINANCES AND REGULATIONS.

ALL ELECTRICAL PANELBOARDS SHALL BE INSPECTED BEFORE THE PANEL IS CLOSED BY THE ELECTRICAL CONTRACTOR. CONTRACTOR SHALL COORDINATE ARRANGEMENTS FOR THIS INSPECTION. ALL PANELBOARD CIRCUIT DIRECTORIES SHALL BE TYPE WITH CORRESPONDING DEVICES AD EQUIPMENT (I.D. DESIGNATION) SERVED. TYPICAL NEW AND EXISTING PANELBOARDS.

ACCEPTABLE MANUFACTURERS: SUBJECT TO COMPLIANCE WITH REQUIREMENTS, PROVIDE RACEWAY OF ONE OF THE FOLLOWING

RIGID NON-METALLIC CONDUIT: CARLON. CONDUX INTERNATIONAL INC. CAN-TEX INDUSTRIES.

CONDUIT SHALL BE SCHEDULE 40 PVC. UV STABILIZED FOR 90° C CONDUCTORS. USE SCHEDULE 80 PVC UNDER TRAFFIC AREAS. FITTINGS SHALL BE SOLVENT, WELDED SOCKET TYPE. CONDUIT SHALL BE MANUFACTURED IN ACCORDANCE WITH NEMA TC-2, FEDERAL SPECIFICATION WC1094A AND UL STANDARD 651.

INSTALLATION METHODS.

- 1. ALL RACEWAY SYSTEMS SHALL BE COMPLETE BEFORE INSTALLING CONDUCTORS. 2. ALL RACEWAYS SHALL HAVE OPENINGS TEMPORARILY PLUGGED TO EXCLUDE FOREIGN OBJECTS. THE INTERIOR OF ALL RACEWAYS SHALL
- BE CLEANED BEFORE PULLING INSTALLING CONDUCTORS. 3. ALL JOINTS SHALL BE CUT SQUARE AND BE REAMED SMOOTH. ALL FIELD THREADED CONDUITS SHALL BE COATED WITH AN APPROVED
- ZINC CHROMATE OR WITH A 90 PERCENT ZINC PAINT. 4. ALL TURNS SHALL BE MADE WITH STANDARD ELLS OR CONDUIT BENT IN ACCORDANCE WITH THE NEC. CONDUIT BODIES MAY BE USED IN LIEU OF CONDUIT ELLS WHERE EASE OF INSTALLATION AND APPEARANCE WARRANTS THEIR USE. CONDUIT BODIES LARGER THAN 1-INCH MAY BE USED ONLY WHERE SPECIFICALLY APPROVED BY THE ARCHITECT. FURNISH AND INSTALL ACCESS DOORS FOR CONDUIT BODIES LOCATED ABOVE INACCESSIBLE CEILINGS. REFER TO ARCHITECTURAL DRAWINGS FOR REQUIRED ACCESS DOORS' FIRE RATINGS. ALL FIELD BENDS SHALL BE MADE USING EQUIPMENT DESIGNED FOR THE PARTICULAR CONDUIT MATERIAL AND SIZE, BENDS SHALL BE FREE FROM DENTS OR FLATTENING. THERE SHALL BE NO MORE THAN THE EQUIVALENT OF THREE NINETY DEGREE BENDS IN
- ANY RACEWAY BETWEEN TERMINALS AND CABINETS, OR BETWEEN OUTLETS AND JUNCTION BOXES OR PULL BOXES. 5. SECURELY FASTEN AND SUPPORT CONDUIT TO METAL FRAMING USING HOT-DIPPED GALVANIZED, MALLEABLE IRON PIPE STRAPS OR OTHER APPROVED MEANS. REFER TO SECTION 26 05 29. GALVANIZED TIE WIRES FOR SECURING CONDUITS, IS NOT ACCEPTABLE. THE
- USE OF CADI-CLIPS FOR CONDUIT SUPPORTS FROM SUSPENDED CEILING SYSTEMS IS NOT ACCEPTABLE. 6. PROVIDE A NO. 30 NYLON PULL CORD IN ALL EMPTY CONDUITS. IDENTIFY BOTH ENDS OF THE LINE BY MEANS OF LABELS OR TAGS READING "PULLING LINE".
- 7. TERMINATE CONCEALED CONDUIT FOR FUTURE USE WITH A COUPLING AT STRUCTURAL SURFACES. INSTALL AN APPROVED CONDUIT PLUG FLUSH WITH THE SURFACE.
- 8. ALL OPENINGS AROUND ELECTRICAL PENETRATIONS AT FIRE RATED WALLS, PARTITIONS, FLOORS OR CEILINGS SHALL BE SEALED TO
- MAINTAIN THE FIRE RESISTANCE RATING OF THE PENETRATION. 9. ALL CONDUIT IN HAZARDOUS AREAS SHALL CONFORM TO NEC REQUIREMENTS FOR THESE AREAS AND WHERE FEEDING FROM OR TO A HAZARDOUS AREA TO ANOTHER ROOM "SEAL OFFS" SHALL BE USED.

INSTALLATION OF UNDERGROUND RACEWAYS

- 1. THE GROUND SHALL BE EXCAVATED IN OPEN TRENCHES TO THE PROPER WIDTH AND DEPTH FOR THE INSTALLATION OF THE UNDERGROUND CONDUITS. MINIMUM CONDUIT BURIAL DEPTH SHALL BE 24" BELOW FINISHED GRADE TO TOP OF THE CONDUIT. 2. WHERE THE BOTTOM OF THE TRENCH IS EXCAVATED BELOW THE NECESSARY ELEVATION, IT SHALL BE BROUGHT TO PROPER GRADE BY
- THE USE OF SAND OR THREE-EIGHTH INCH GRAVEL 3. NO EXTRA WILL BE ALLOWED BECAUSE OF THE NATURE OF THE GROUND IN WHICH THE TRENCH OR OTHER EXCAVATIONS ARE MADE.
- ALL NECESSARY SHEATHING TO PREVENT CAVE-INS AND BARRICADES SHALL BE PROVIDED IN ACCORDANCE WITH OSHA REQUIREMENTS. 4. WHERE UNSTABLE GROUND IS ENCOUNTERED IN THE BOTTOM OF THE TRENCH, IT SHALL BE EXCAVATED TO A DEPTH OF AT LEAST 12

INCHES BELOW THE LINE OF THE DUCT OR SLAB, AND REPLACED WITH COARSE GRAVEL TO THE PROPER HEIGHT

- 5. WHERE THE EXCAVATION FOR ITS ENTIRE DEPTH IS IN WATER OR WET SAND, PUMP AND TRENCH SO AS TO DRAIN IT EFFECTIVELY. 6. BACKFILL TRENCHES WITH THE EXCAVATED MATERIAL UNLESS OTHERWISE SPECIFIED. IT SHALL BE THOROUGHLY COMPACTED TO INSURE A SATISFACTORY JOB. IN SURFACED AREAS, COMPACTIONS SHALL BE 95% OF SURROUNDING UNDISTURBED SOIL. SODDED AREAS SHALL BE COMPACTED TO 95% UP TO TOPSOIL. TOPSOIL SHALL BE LIGHTLY COMPACTED THEN SOIL MOUNDED TO ALLOW FOR SETTLING.
- 7. WHERE CONDUITS PASS UNDER EXISTING SIDEWALKS, ROADS OR CURBS CUT AND REMOVE SAME IN ORDER TO INSTALL THE CONDUIT OR DUCTS. ALL SIDEWALKS, ROADS OR CURBS SHALL BE REPLACED WITH MATERIAL EQUAL TO THOSE NOW IN PLACE.
- 8. PROVIDE A BURIAL UTILITY TAPE WITH MAGNETIC TRACER, OVER ALL UNDERGROUND ELECTRICAL INSTALLATIONS THAT ARE EXTERIOR TO THE BUILDING. THIS SHALL INCLUDE ALL FEEDERS, BRANCH CIRCUITS AND COMMUNICATIONS CONDUITS.
- a. WARNING TAPE OVER ELECTRICAL INSTALLATION UNDER 600 VOLTS SHALL BE RED WITH BLACK LETTERING STATING "BURIED
- b. Warning tape over electrical installations over 600 volts shall be red with black lettering stating "buried high VOLTAGE LINE".
- c. WARNING TAPE OVER COMMUNICATIONS INSTALLATIONS SHALL BE ORANGE WITH BLACK LETTERING STATING "BURIED TELEPHONE LINE". TAPE SHALL BE INSTALLED ONE FOOT TO SIX INCHES BELOW FINISHED GRADE, 3" WIDE AS MANUFACTURED BY T & B WESTLINE OR EQUAL. TAPE SHALL INCLUDE MAGNETIC TRACER.
- TAPE SHALL BE INSTALLED ONE FOOT TO SIX INCHES BELOW FINISHED GRADE, 3" WIDE AS MANUFACTURED BY T & B WESTLINE OR EQUAL. TAPE SHALL INCLUDE MAGNETIC TRACER.
- 9. ALL RACEWAYS INSTALLED UNDERGROUND SHALL BE SEALED IN ACCORDANCE WITH THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE ARTICLE 300. PROVIDE CONDUIT SEALING BUSHINGS TO PREVENT ENTRANCE OF MOISTURE INTO THE UNDERGROUND RACEWAY SYSTEMS, ACCEPTABLE SEALING BUSHING MANUFACTURER IS O-Z. GEDNEY OR APPROVED EQUAL.

26120 INSULATED CONDUCTORS

BRAND REX CO. CABLEC CORP. COLEMAN CABLE & WIRE CO. CAROL CABLE CO. ESSEX GROUP INC. INDUSTRIAL PRODUCTS. GENERAL CABLE CO. GUARDIAN PRODUCTS. THE OKONITE CO. PIRELLI CABLE CORP. SOUTHWIRE CO. SENATOR WIRE & CABLE CO.

WIRE CONNECTORS. BURNDY. 3M ELECTRICAL PRODUCTS DIVISION. ILSCO. IDEAL. THOMAS & BETTS. ALL CONDUCTORS SHALL BE SOFT_DRAWN ANNEALED COPPER WITH CONDUCTIVITY OF NOT LESS THAN 98% AT 20 DEGREES C (68 DEGREES F). CONDUCTORS NO. 10 AWG AND SMALLER SHALL BE SOLID AND CONDUCTORS NO. 8 AWG AND LARGER SHALL BE STRANDED. MINIMUM WIRE SIZE SHALL BE #10 AWG.

INSULATION SHALL BE AS FOLLOWS: TYPE THW: FOR DRY AND WET LOCATIONS; MAX OPERATING TEMPERATURE 75 DEGREES C (167 DEGREES F). PVC INSULATION, WITH A MINIMUM INSULATION RATING OF 600 VOLTS. TYPE THHN OR THWN: FOR DRY AND WET LOCATIONS; MAXIMUM OPERATING TEMPERATURE SHALL BE 75EC (THWN) OR 90EC (THHN). TYPE XHHW: FOR WET OR DRY LOCATIONS; MAXIMUM OPERATING TEMPERATURE 90EC. INSULATION SHALL BE CROSS-LINKED POLYETHYLENE COMPRESSION CONNECTORS AND LUGS: THE CONNECTORS SHALL BE COPPER WITH TIN PLATING. PUSH-IN WIRE CONNECTORS ARE NOT ACCEPTABLE.

26450 SECONDARY GROUNDING PROVIDE AN EQUIPMENT GROUNDING CONDUCTOR IN ALL RACEWAYS .G. LOPEZ DEL CASTILL 04/04/2023

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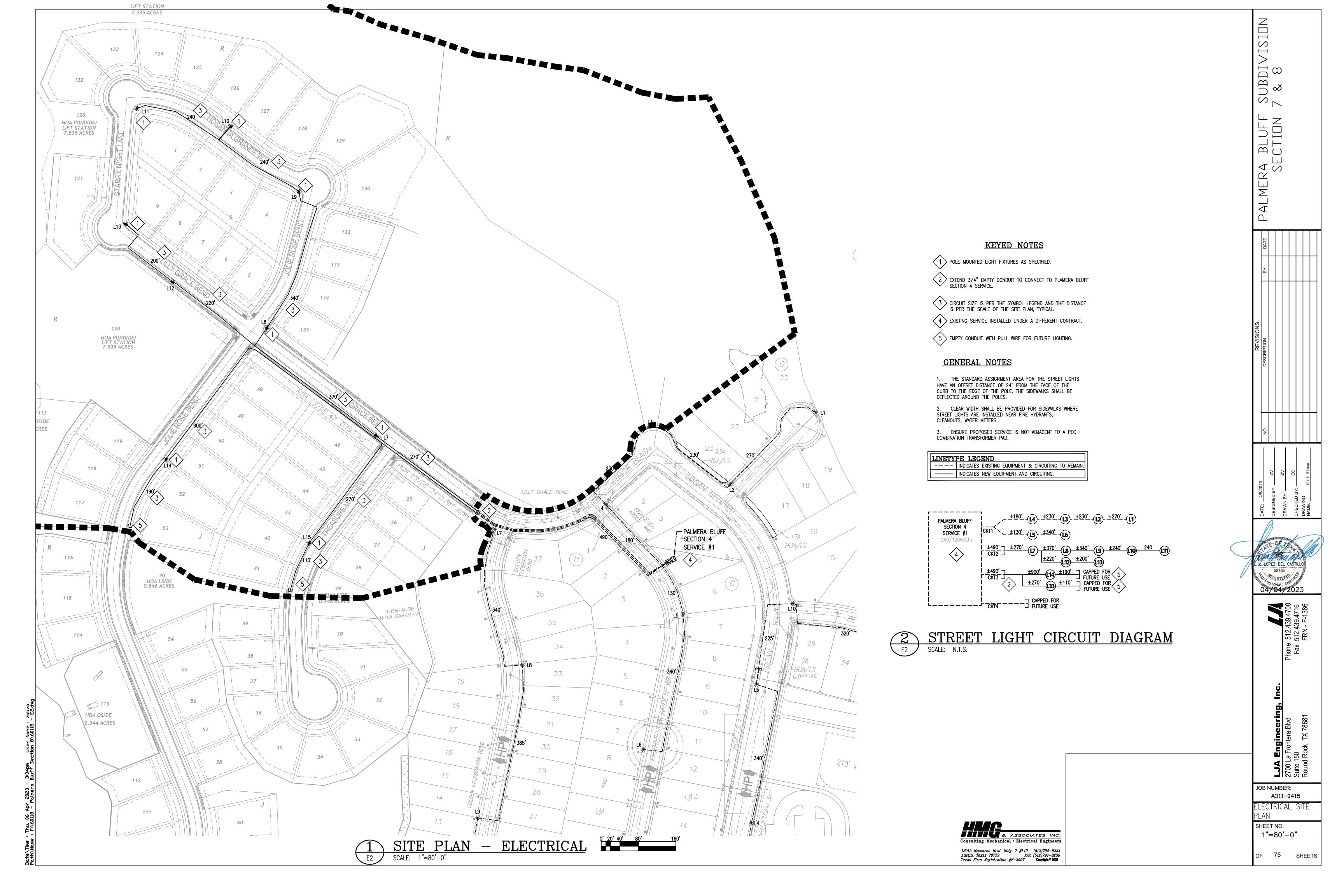
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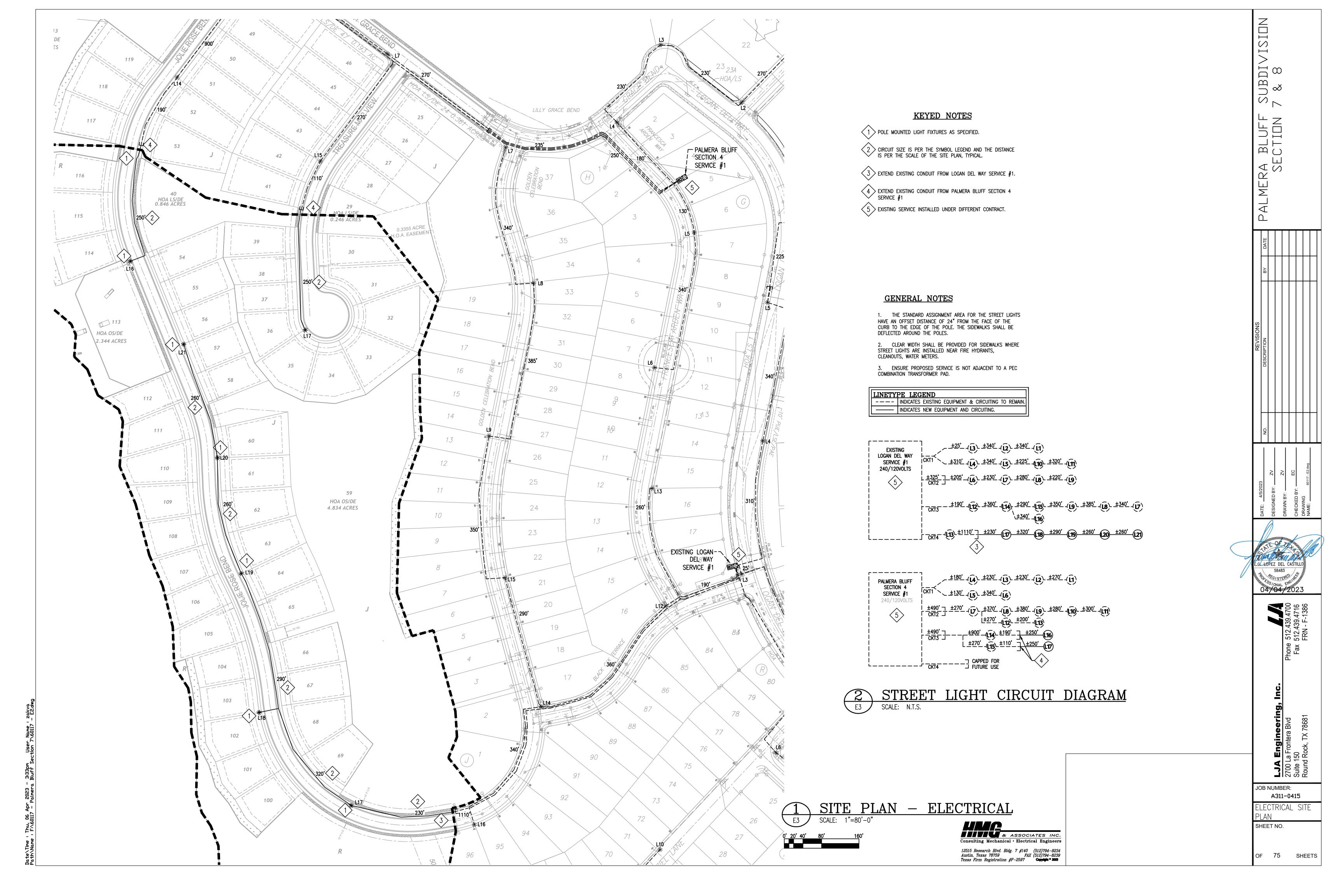
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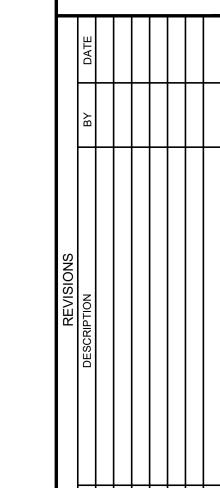
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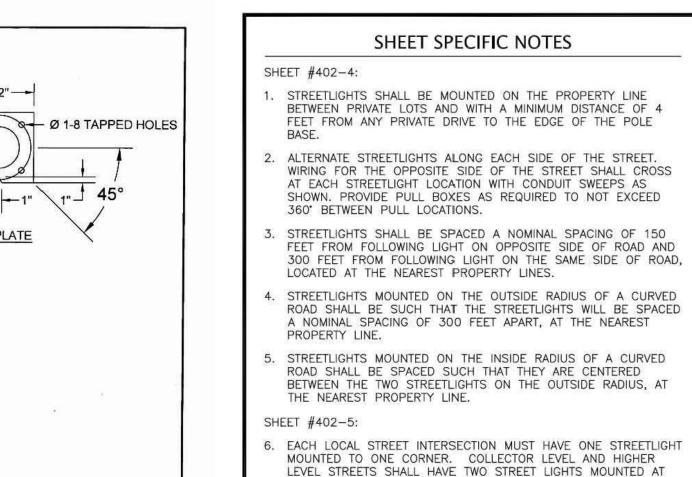


A311-0415

SHEET NO.

SHEETS

OF 75



7 3/4"

- FINISHED

GRADE

THRU BOTH SIDES

8.625" OD X 0.25" WALL

CABLE SLOT

- 8" SCH-20 PIPE

Ø18" CLASS 'A'

CONCRETE COLLAR

3" X 12"

Ø 11"-

TOP PLATE

BOTTOM PLATE

City Of Leander, Texas

DETAIL #402-2

STANDARD LIGHTING POLE FOUNDATION

4" MAX

58 5/8"

12" -

FRONT VIEW

POLE BASE TO BE SALCo 1921-10 OR APPROVED EQUAL

HOLES IN BASE PLATE ARE Ø1"-8 TAPPED HOLES.

BASE TO BE HOT-DIPPED GALVANIZED AFTER FABRICATION.

CONCRETE COLLAR TO EXTEND FROM BOTTOM PLATE TO BOTTOM OF CABLE SLOT.

- 6. EACH LOCAL STREET INTERSECTION MUST HAVE ONE STREETLIGHT MOUNTED TO ONE CORNER. COLLECTOR LEVEL AND HIGHER LEVEL STREETS SHALL HAVE TWO STREET LIGHTS MOUNTED AT OPPOSING CORNERS. ALL SUBSEQUENT STREETLIGHTS TO BE MOUNTED A MINIMUM OF 75 FEET AWAY FROM THE INTERSECTION CORNERS, AT THE NEAREST PROPERTY LINE.
- PROVIDE CONDUIT SWEEPS AT INTERSECTION CORNERS AS REQUIRED. DO NOT EXCEED A MAXIMUM BEND OF 360' BETWEEN PULL LOCATIONS. 8. CONDUIT CROSSING THE ROAD SHALL TAKE THE SHORTEST PATH
- AVAILABLE WITHOUT CROSSING A CORNER OF AN INTERSECTION. 9. PROVIDE ONE PULL BOX IN EACH LANDSCAPE STRIP AS SHOWN WHEN CROSSING A ROAD WHERE THERE IS NOT A STREETLIGHT ON EITHER SIDE OF THE CROSSING.
- SHEET #402-6: 10. ORIENT PEDESTAL SO THAT LIGHTING CONTROL PANEL FACES
- 11. FOLLOW PEDERNALES ELECTRIC COOPERATIVE INC. DRAWING NUMBER 520-010-0911 AND 510-009-0911 FOR PEDESTAL BASE CONSTRUCTION. SHEET #402-7:
- 12. EACH CONTROL PEDESTAL HAS A PHOTOCELL, TORK CATALOG #
- 13. PER 2014 NATIONAL ELECTRICAL CODE (NEC) ARTICLE 409.108, ENCLOSURE SHALL BE LABELED "SUITABLE FOR USE ONLY AS

SHEET SPECIFIC NOTES

SERVICE EQUIPMENT". ENCLOSURE SHALL COMPLY WITH ALL OTHER MARKING REQUIREMENTS FOUND IN ARTICLE 409.110.

- 14. CONNECT ONE #6 BARE COPPER CONDUCTOR FROM THE NEUTRAL BUSS TO THE GROUND ROD IN THE PEDESTAL BASE. CONNECTION TO THE GROUND ROD MAY BE EITHER EXOTHERMIC WELD OR MECHANICAL FITTING RATED FOR DIRECT BURIAL.
- 15. ALL CIRCUIT BREAKERS SHALL HAVE A MINIMUM INTERRUPTING CAPACITY OF 10KAIC.

SHEET #402-9:

SHEET #402-10:

- 16. COORDINATE PLACEMENT OF PEDESTAL WITH ALL OTHER UNDERGROUND UTILITIES. INSTALL PEDESTAL ON A "DRY" LOT LINE UNLESS IN CONFLICT WITH UTILITY EQUIPMENT, WHERE A MINIMUM OF 3-FEET SPACING IS REQUIRED. PEDESTAL SHALL NOT BE INSTALLED NEXT TO A COMBINATION TRANSFORMER PAD.
- 17. IF A PEDESTAL MUST BE INSTALLED NEXT TO A UTILITY TRANSFORMER, CONTRACTOR SHALL SUBMIT A PLAN TO THE CITY OF LEANDER ENGINEER SHOWING PEDESTAL PLACEMENT, CONDUIT ROUTING, AND ALL OTHER DETAILS NECESSARY TO MINIMIZE CONFLICTS WITH ALL UNDERGROUND UTILITIES. PLAN MUST BE APPROVED BY CITY ENGINEER PRIOR TO THE START OF CONSTRUCTION.
- 18. ALL PULL BOXES SHALL BE A HUBBELL QUAZITE 11"x18"x18". 19. ANY WIRE JUNCTIONS MADE IN AN IN GROUND PULL BOX SHALL BE MADE WITH THOMAS & BETTS PART NUMBER USK 2/0.
- 20. DYU 6 CONNECTORS MAY BE ELIMINATED FOR LAST POLE IN CIRCUIT. IF A CONDUCTOR SIZE CHANGE IS NECESSARY AT THE LAST POLE, USE THOMAS & BETTS PART NUMBER SDK M.
- 21. ALL UNDERGROUND CONDUITS SHALL BE SCHEDULE 80 PVC. 22. ALL CONNECTORS TO BE INSTALLED WITH A 3 AMP FUSE, NO
- CONTROL PEDESTALS. PROVIDE CONDUIT AND PULL BOXES AT THE BOUNDARY OF EACH ADJOINING PHASE SO THAT SUBSEQUENT PHASES MAY BE EASILY TIED IN TO THE EXISTING LIGHTING CONTROL PEDESTAL.
 - 6. ALL STREETLIGHT BRANCH CIRCUIT WIRING SHALL BE #10AWG UNLESS OTHERWISE NOTED. MAXIMUM BRANCH CIRCUIT DISTANCE IS 2000' OF WIRE, AND THE MAXIMUM NUMBER OF STREETLIGHTS PER BRANCH CIRCUIT IS (10). IF CIRCUITS OR STREETLIGHT QUANTITIES MUST EXCEED THESE NUMBERS, THE SEALING ENGINEER IS RESPONSIBLE FOR PERFORMING VOLTAGE DROP CALCULATIONS DEMONSTRATING THAT THE CALCULATED VOLTAGE DROP IS AT A TOLERABLE LEVEL PER NEC ARTICLE

City of Leander, lexas

DETAIL #402-3

RESIDENTIAL ROADWAY LIGHTING NOTES

GENERAL NOTES

. THESE STANDARDS APPLY TO RESIDENTIAL ROADWAY LIGHTING

ONLY. ANY LIGHTING FOR OTHER ROADWAY TYPES SUCH AS

ARTERIAL, COLLECTOR, HIGHWAY, ETC. SHALL BE DESIGNED

IESNA RP-8-14 STANDARD AND APPROPRIATE ROADWAY

ANY DEVIATIONS FROM THE FOLLOWING STANDARDS SHALL

CLASSIFICATION.

BASED ON A PHOTOMETRIC STUDY IN ACCORDANCE WITH THE

REQUIRE CONSTRUCTION DOCUMENTS WITH AN ENGINEERS SEAL,

SIGNATURE, AND DATE OF SIGNATURE. SUBMIT TO THE CITY OF

LEANDER FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

3. REFER TO CITY OF LEANDER DETAIL #402-1 AND #402-2 FOR

STANDARD POLE BASE, POLE, AND STREETLIGHT SELECTION.

TEN STREETLIGHTS PER CONTROL PEDESTAL WITH THE INTENT TO DEPLOY AS FEW CONTROL PEDESTALS AS POSSIBLE. A

A MAXIMUM OF 4 CIRCUITS WITH 10 STREETLIGHTS EACH (40

ACCOMMODATION MUST BE MADE FOR PHASED DEVELOPMENTS.

FUTURE PHASES WITHOUT THE NEED FOR ADDITIONAL LIGHTING

ARRANGE STREETLIGHTS AND CIRCUITS SUCH THAT SPARE

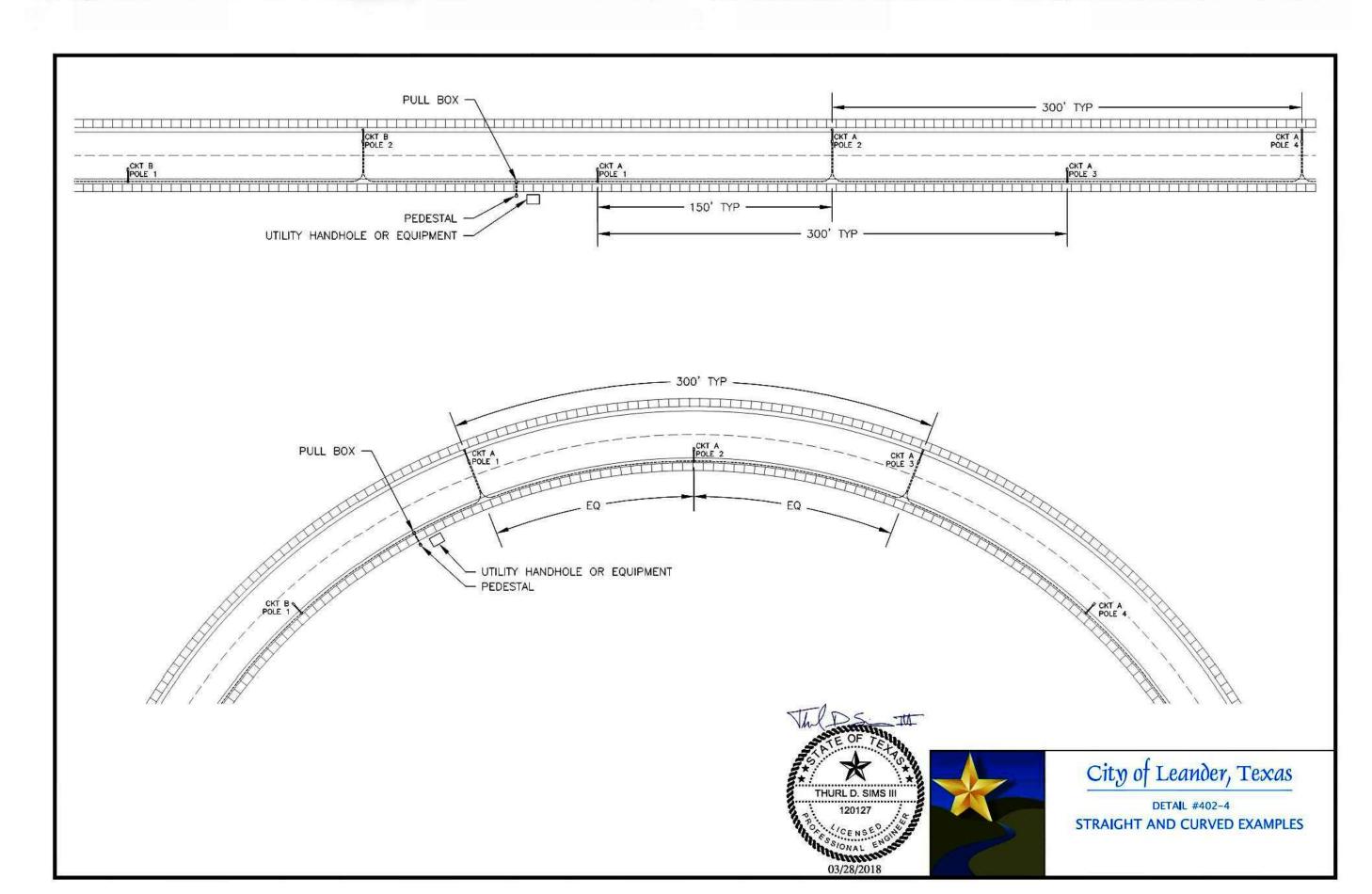
CIRCUITS IN CONTROL PEDESTAL ARE AVAILABLE TO SERVE

SINGLE CONTROL PEDESTAL IS SIZED SUCH THAT IT MAY FEED

4. THE CONTRACTOR SHALL PROVIDE POWER TO A MINIMUM OF







Cooper NAVION Fixture NAV-AF-01-E-UNV-T3-10K-AP

ED Light Fixture Notes

1. Streetlight Electrical Systems shall be designed and constructed per City of Leander Residential

LED fixtures shall be supplied and installed by the contractor with the light pole installation.

All roadway lights shall be on a metered circuit. Meter installations shall be pedestal type.

The Illumination Plan shall show the light pole spacing. Light poles shall be installed as appropriate to

provide continuous illumination of the roadway. Maximum spacing between poles shall be per Detail

The City of Leander will establish an account with the electricity provider.

Mounting Height

Bolt Circle

MOUNTING HARDWARE

(4) 1"-BNC Galv. Stl. Anchor Bolts, AASHTO M314-90 Grade 55,10" of Threaded End Galv. Per ASTM A153.

4) 1"-8NC Galv. Stl. Hex. Nuts) 1" Galv. Stl. Lockwashers) 1" Galv. Stl. Flatwashers

City Of Leander, Texas

DETAIL #402-1

STANDARD LIGHTING POLE

pered Alum. Tube (4" O.D.)

.125" Wall Alloy 6063-T6

Wire Hole With 1" I.D. Rubber Grommet

HAPCO 25'-0" LIGHTING POLE

Extruded Alum. Pole Plate Alloy 6063-T6

With 1/2"-13NC Stainless Stl. Hardware

OR APPROVED EQUAL

Roadway Lighting Standards.

#402-3 Roadway Lighting Notes.

Tapered Alum. Tube

For Grounding

156" Wall Alloy 6063-T6

Pole foundations shall be per Detail #402-2.

Handhole Frame Tapped 3/8"-16NC

Reinforced Handhole (4" X 6") With Cover And Stainless Stl. Hex. Hd. Screws

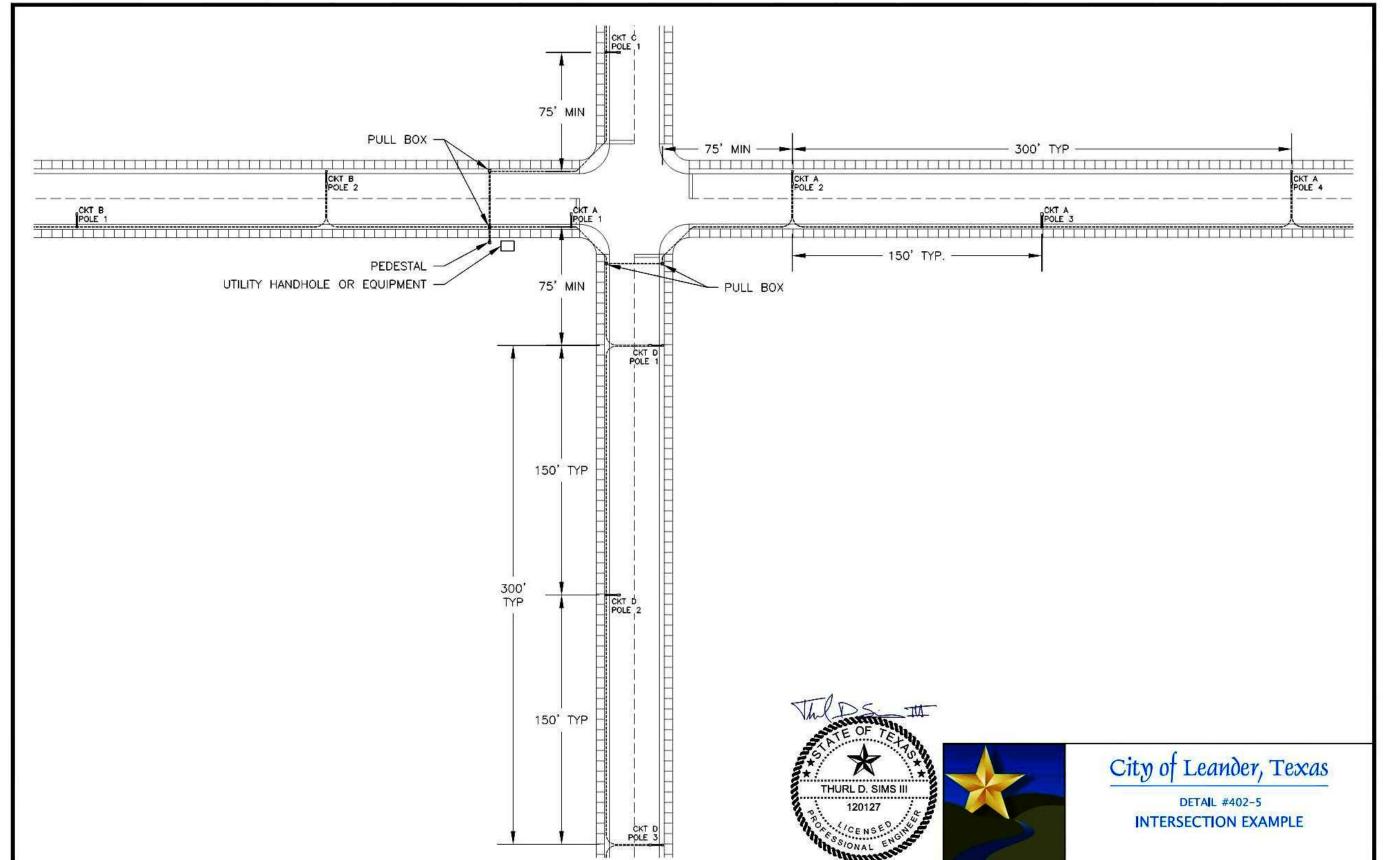
Base Flange Alloy 356-T6 With Bolt

Covers And Stainless Stl. Hex. Hd. Screws

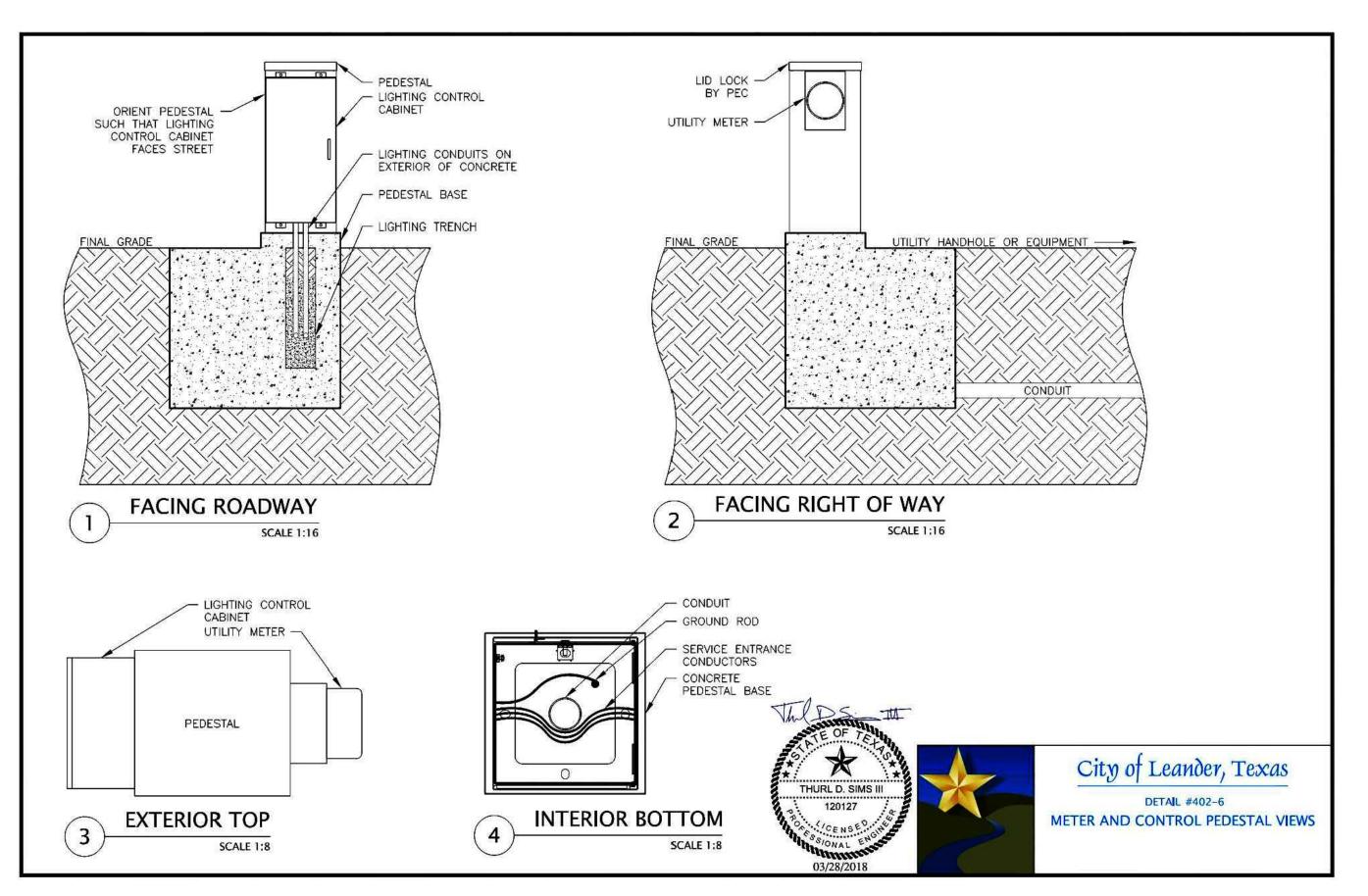
2 1/2" x 5 1/8"

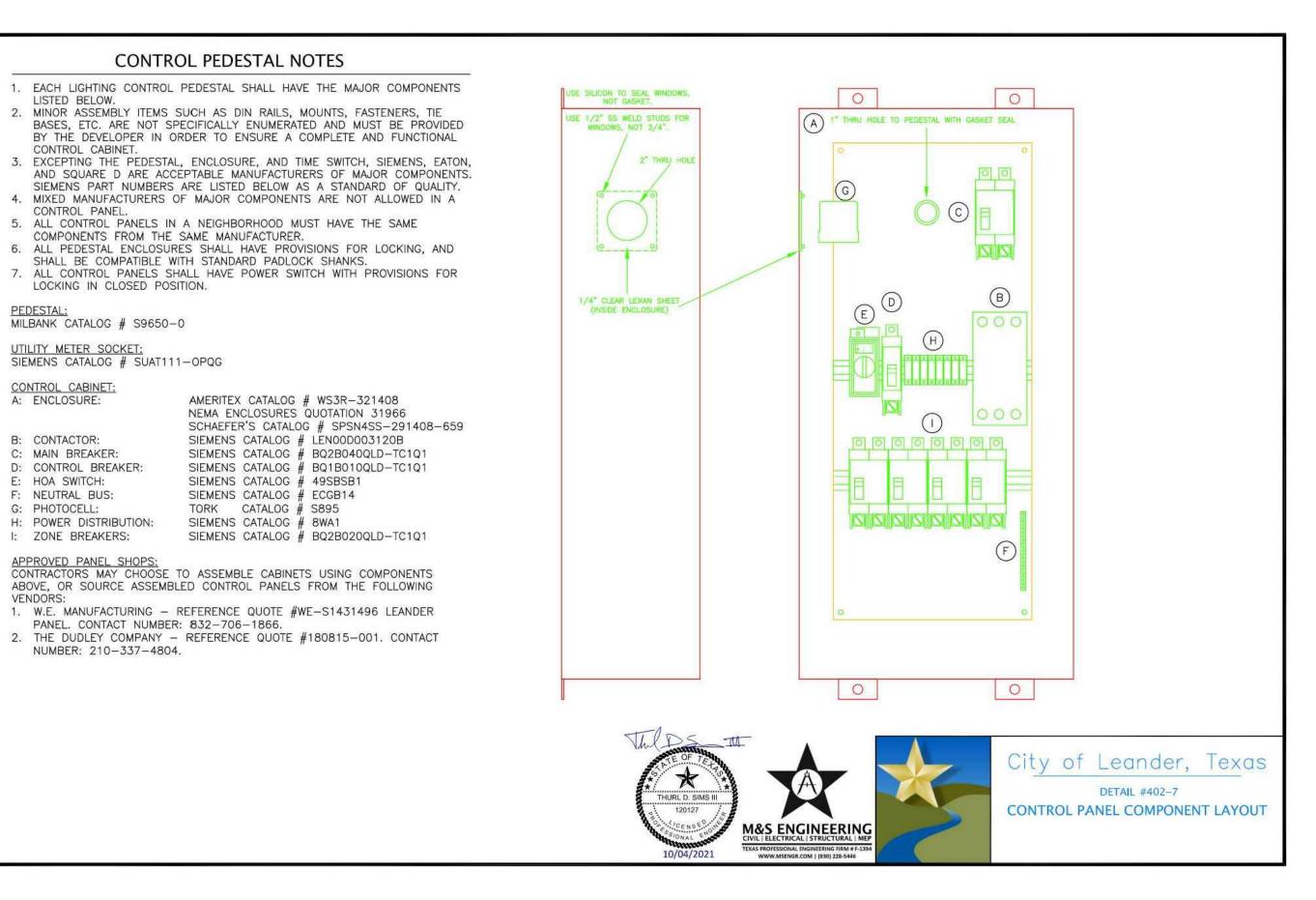
Elliptical Section

4 1/2" O.D.

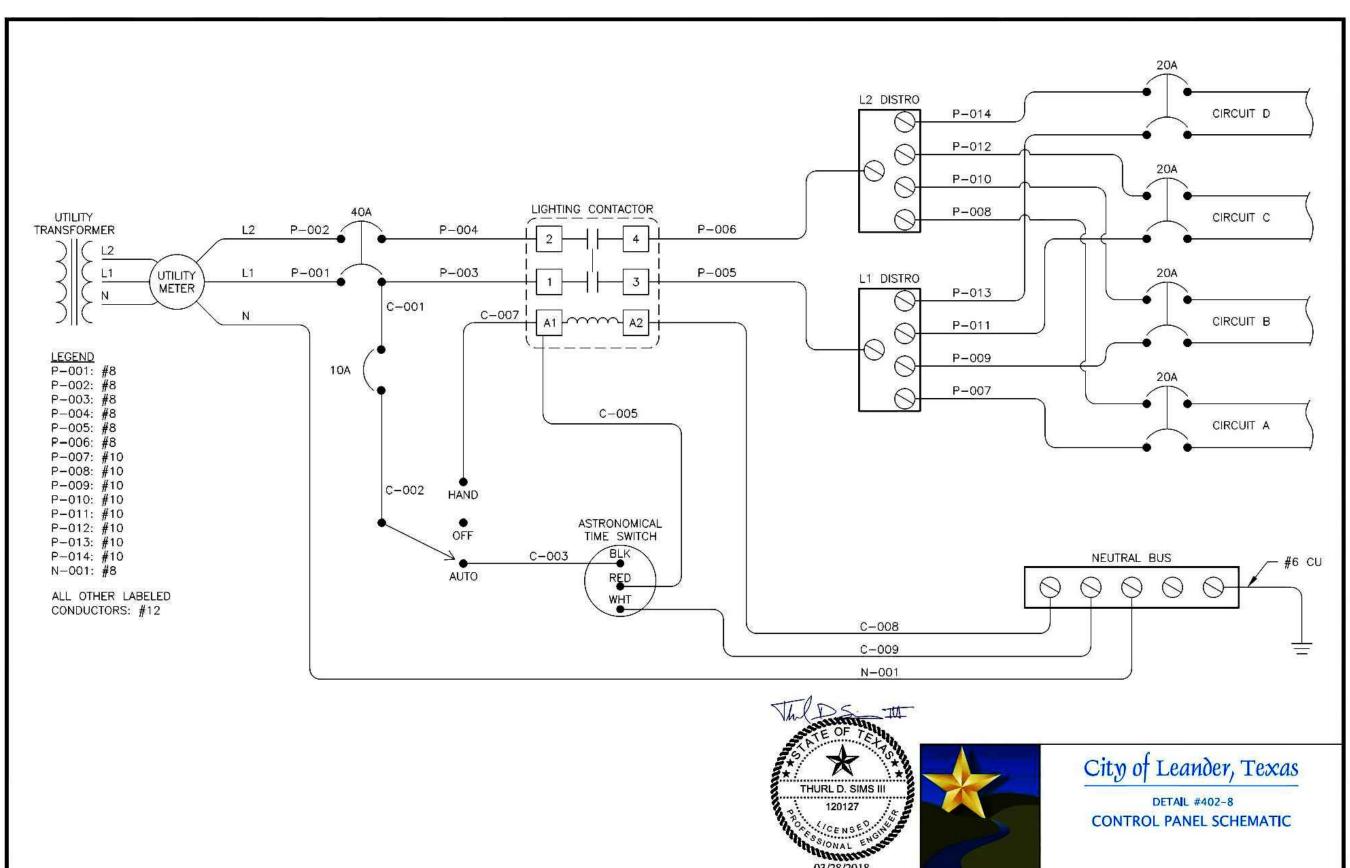


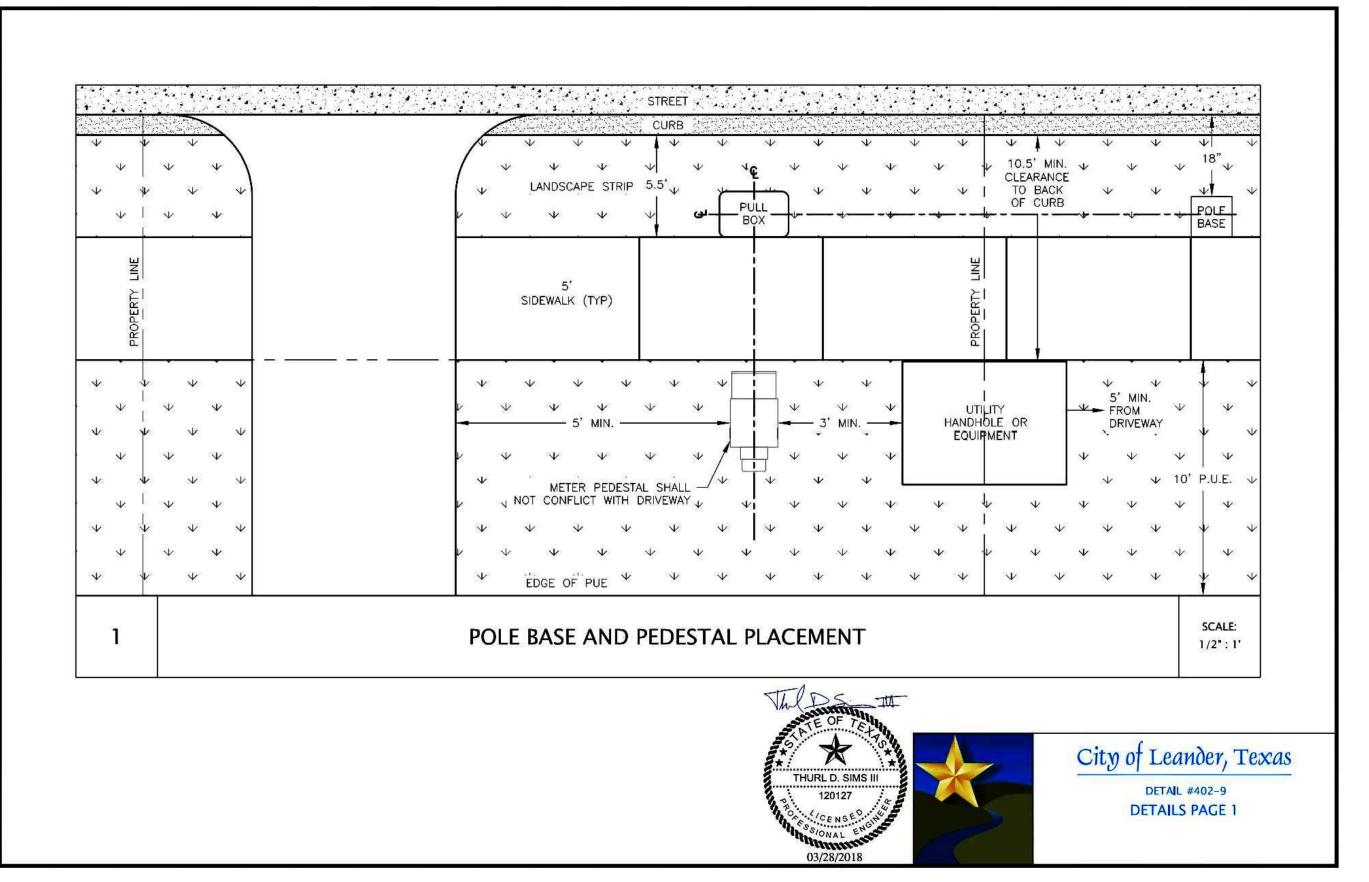
& ASSOCIATES INC.





VENDORS:





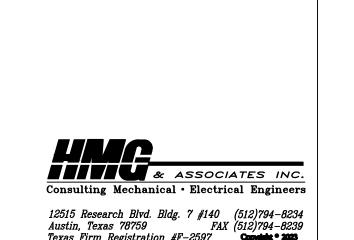


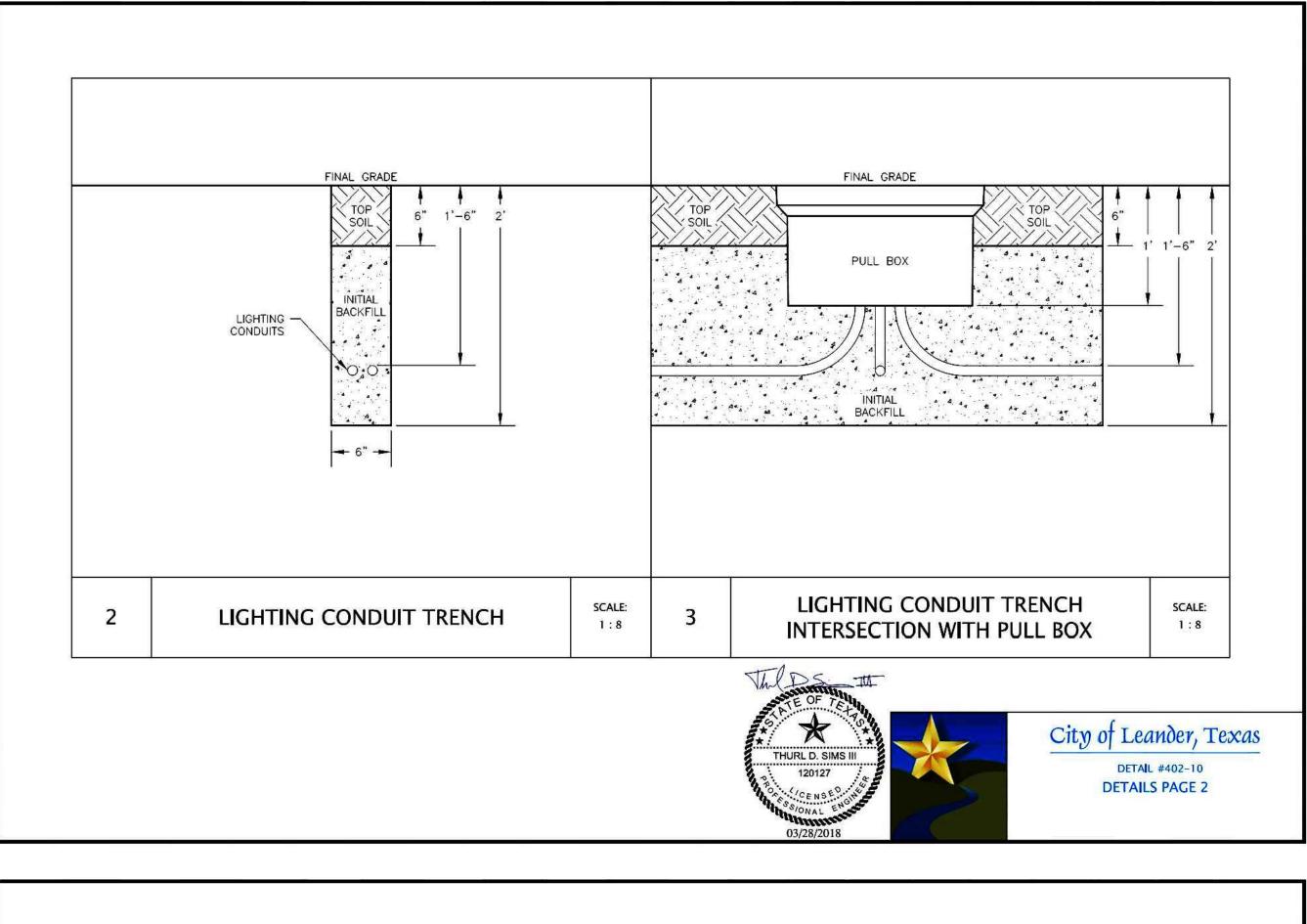
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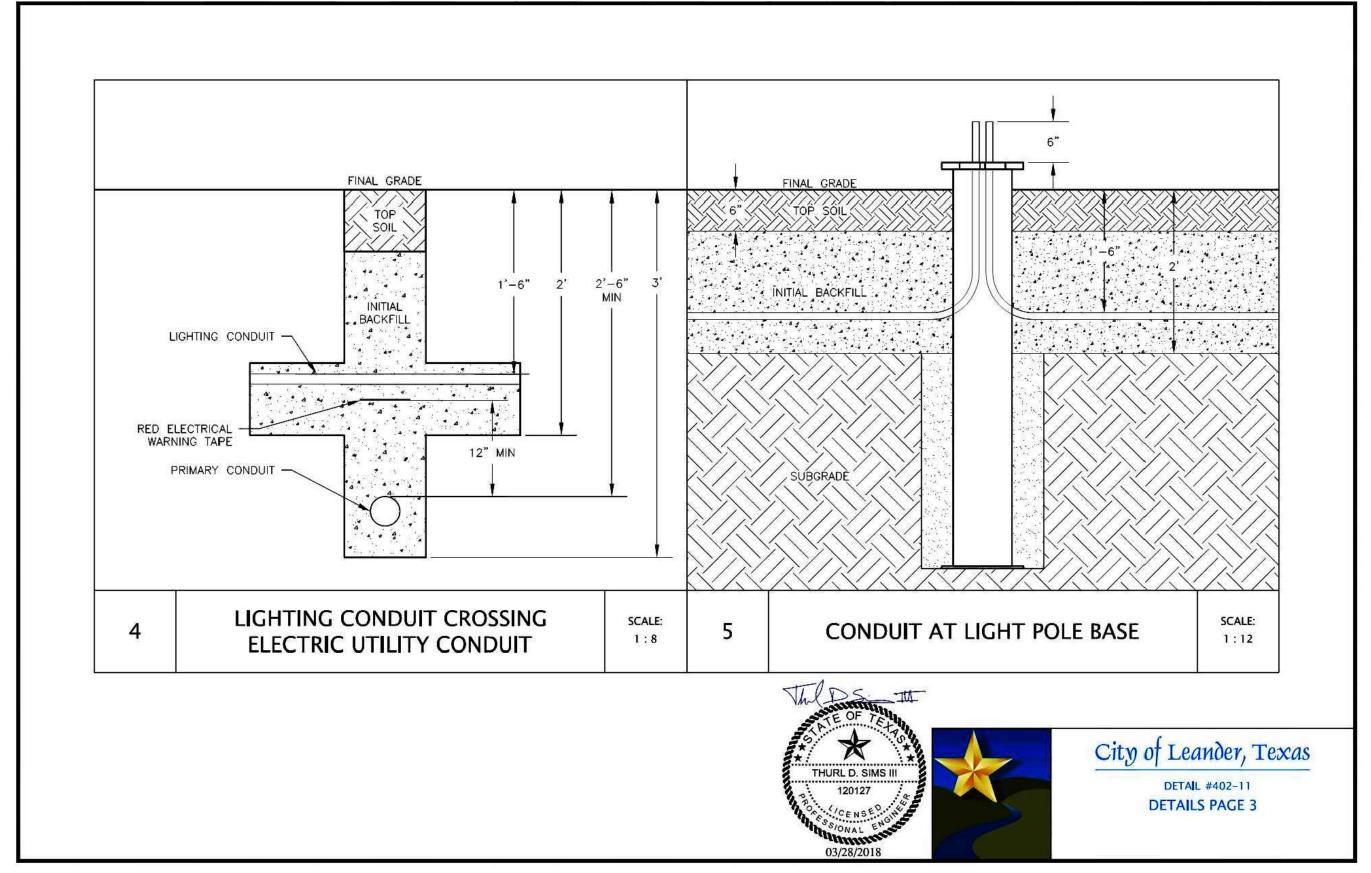
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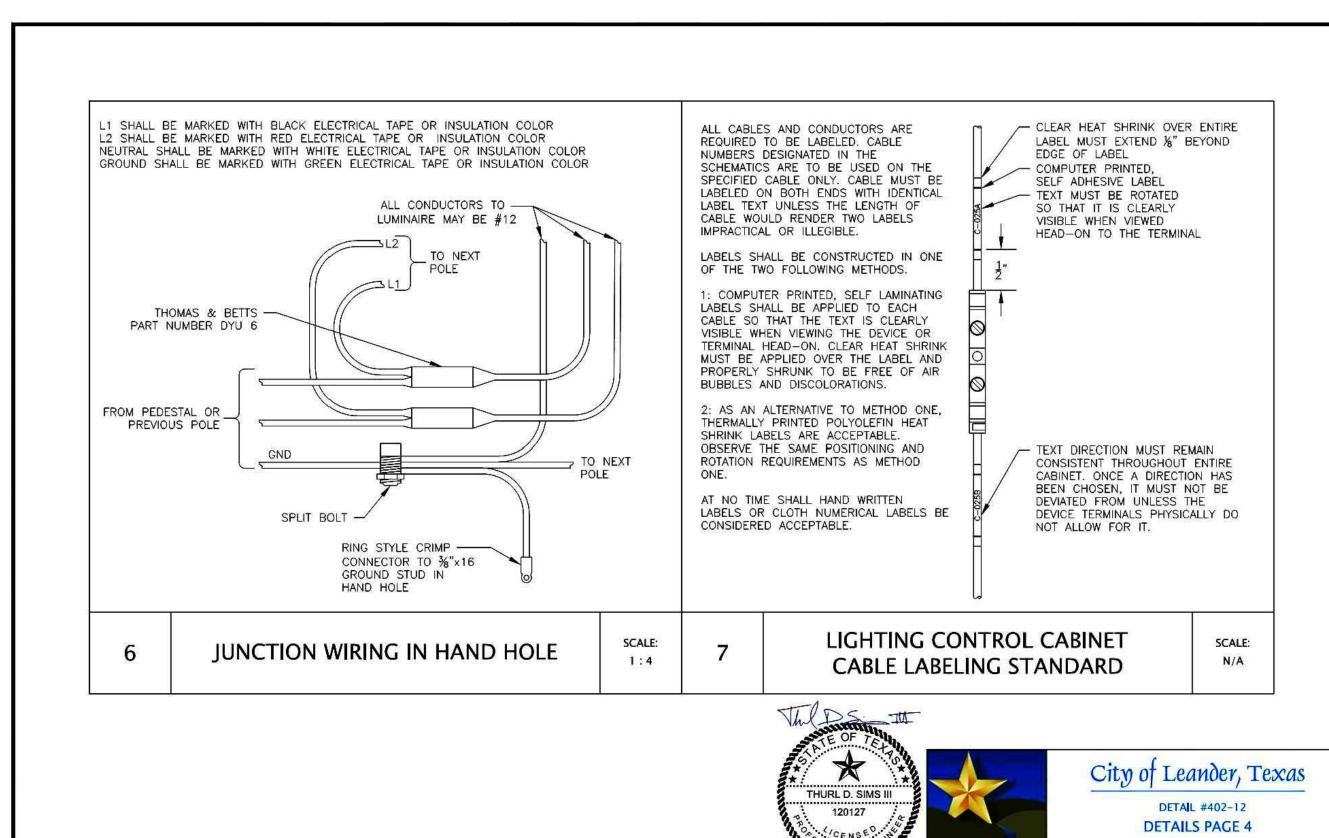
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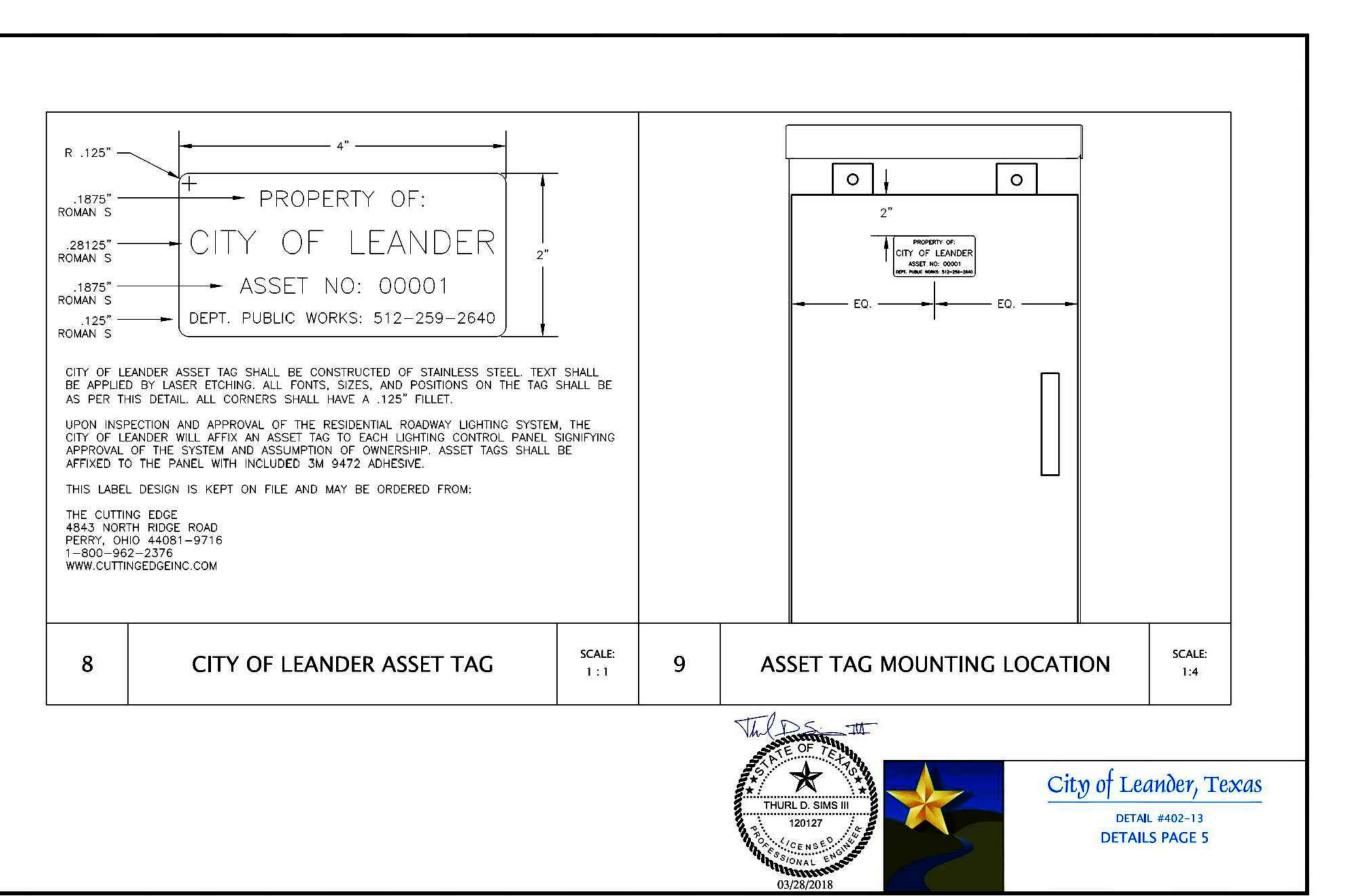
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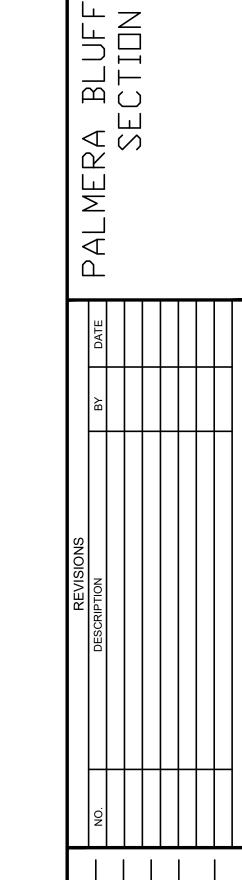












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