NOTICE OF CONFIDENTIALITY RIGHTS: IF YOU ARE A NATURAL PERSON, YOU MAY REMOVE OR STRIKE ANY OR ALL OF THE FOLLOWING INFORMATION FROM ANY INSTRUMENT THAT TRANSFERS AN INTEREST IN REAL PROPERTY BEFORE IT IS FILED FOR RECORD IN THE PUBLIC RECORDS: YOUR SOCIAL SECURITY NUMBER OR YOUR DRIVER'S LICENSE NUMBER.

Corrective Special Warranty Deed with Vendor's Lien

Date: July 20, 2022

Grantor: Larry Everett Foust and Maydale Foust

Grantor's Mailing Address:

Grantee: CHV Liberty Hill 29 Property Owner LLC, a Delaware limited liability company

Grantee's Mailing Address: 6601-E Hillcrest Avenue, Suite 212, Dallas, TX 75205

Consideration: Cash and a note of even date executed by Grantee and payable to the order of Simmons Bank, in the principal amount as set forth thereon. The note is secured by a first and superior vendor's lien and superior title retained in this deed and by a first-lien deed of trust of even date from Grantee to Amanda R. Grainger, trustee.

Property (including any improvements):

That certain approximately <u>42.682</u>-acre tract of land, as more particularly described in Exhibit "A" Legal Description, attached hereto and incorporated herein by this reference for all purposes.

Reservations from Conveyance: NONE.

Exceptions to Conveyance and Warranty: Liens described as part of the Consideration and any other liens described in this deed as being either assumed by Grantee or subject to which title is taken by Grantee; validly existing easements, rights-of-way, and prescriptive rights, appearing of record in the Official Public Records of Real Property of Williamson County, Texas, relative to the Property; all presently recorded and validly existing restrictions, reservations, covenants, conditions, oil and gas leases, mineral interests, and water interests outstanding in persons other than Grantor that affect the Property; validly existing rights of adjoining owners in any walls and fences situated on a common boundary; taxes for 2022, which Grantee assumes and agrees to pay on a prorated basis, and subsequent assessments for that and prior years due to change in land usage, ownership, or both, the payment of which Grantee assumes; and any validly existing titles or rights asserted by anyone, including but not limited to persons, the public, corporations, governments, or other entities, to (a) tidelands or lands comprising the shores or beds of navigable or perennial rivers and streams, lakes, bays, gulfs, or oceans, (b) lands beyond the line of the harbor

or bulkhead lines as established or changed by any government, (c) filled-in lands or artificial islands, (d) water rights, including riparian rights, or (e) the area extending from the line of mean low tide to the line of vegetation or the right of access to that area or easement along and across that area.

The Property conveyed shall include all right, title and interest, if any, of Grantor in and to, (1) any land lying in a street, road, tollway, accessway or easement (including any drainage or flood control easement) open or proposed, in front of, at the side of, adjoining, or within the Property, (2) the bed and banks of any bayou, stream, canal or ditch adjoining or adjacent to the Property, (3) all reversionary rights attributable to the Property, and (4) all rights of ingress and egress to the Property by way of open or dedicated roads and streets adjoining the Property.

Grantor, for the Consideration and subject to the Exceptions to Conveyance and Warranty, grants, sells, and conveys to Grantee the Property, together with all and singular the rights and appurtenances thereto in any way belonging, to have and to hold it to Grantee and Grantee's heirs, successors, and assigns forever. Grantor binds Grantor and Grantor's heirs and successors to warrant and forever defend all and singular the Property to Grantee and Grantee's heirs, successors, and assigns against every person whomsoever lawfully claiming or to claim the same or any part thereof, except as to the Exceptions to Conveyance and Warranty.

Simmons Bank, at Grantee's request, has paid in cash to Grantor that portion of the purchase price of the Property that is evidenced by the note. The first and superior vendor's lien against and superior title to the Property are retained for the benefit of Simmons Bank, and are transferred to Simmons Bank, without recourse against Grantor.

When the context requires, singular nouns and pronouns include the plural.

Larry Everett Foust and Maydale Foust

STATE OF TEXAS	§
COUNTY OF WILLIAMSON	§ §
This instrument was acknowled to the second of the second	edged before me on this the 37 day of ary Everett Forest. I Mary Public State of Texas
3/9/2025	MARCIA A. HACE Printed Name of Notary Public
STATE OF TEXAS COUNTY OF WILLIAMSON	<pre>\$ 8 8 8</pre>
This instrument was acknowl September, 2022, by	edged before me on this the 21 day of) as clave for st.
MARCIA A. HALL My Notary ID # 126830910 Expires March 9, 2025 My Constitution Expires	Motary Public State of Texas
3/9/ 2025	Printed Name of Notary Public

Exhibit A

BEING a tract situated in the N. Smithwick Survey, Abstract Number 590, City of Liberty Hill, Williamson County, Texas, being part of the tract described in the deed to Larry Everett Foust, recorded in Volume 764, Page 801, Deed Records, Williamson County, Texas (D.R.W.C.T.).

BEGINNING at a point for the northwest corner of said Foust tract and the herein described tract, from which a 4 inch wood post found bears NORTH 69 degrees WEST, a distance of 1.60 feet;

THENCE NORTH 68 degrees 48 minutes 24 seconds EAST, with the north line of said Foust tract, a distance of 216.00 feet to a 1/2 inch rebar found for a re-entrant corner of a tract of land described by deed to Clyde Krause as recorded under Volume 1729, Page 108, D.R.W.C.T.;

THENCE with the south line of said Krause tract, the following calls:

- 1. NORTH 68 degrees 45 minutes 38 seconds EAST, a distance of 457.41 feet to a 1/2 inch rebar found;
- 2. NORTH 69 degrees 48 minutes 23 seconds EAST, a distance of 1156.72 feet to a 1/2 inch rebar with pink plastic cap stamped "BARTON CHAPA" set (hereinafter "capped rebar set");
- 3. NORTH 70 degrees 04 minutes 28 seconds EAST, a distance of 282.13 feet to a 1/2 inch rebar found for the southeast corner of said Krause tract:

THENCE NORTH 66 degrees 30 minutes 08 seconds EAST, with the north line of said Foust tract, a distance of 30.08 feet to a Mag nail set within an asphalt road under apparent public use known as C.R. 260 (no right-of-way document found);

THENCE through the interior of said Foust tract, the following calls:

- 1. SOUTH 21 degrees 18 minutes 23 seconds EAST, a distance of 484.24 feet to a Mag nail set at the beginning of a tangent curve to the right, having a radius of 558.27 feet, a central angle of 16 degrees 02 minutes 44 seconds, and a chord bearing and distance of SOUTH 13 degrees 17 minutes 01 seconds EAST, 155.83 feet;
- 2. Along the arc of said curve, an arc distance of 156.34 feet to a Mag nail set;
- 3. SOUTH 06 degrees 50 minutes 29 seconds EAST, a distance of 265.76 feet to a Mag nail set in the south line of said Foust tract;

THENCE with the south line of said Foust tract, NORTH 69 degrees 09 minutes 25 seconds WEST, a distance of 22.42 feet to a 1/2 inch rebar found;

THENCE with the south line of said Foust tract, SOUTH 69 degrees 37 minutes 35 seconds WEST, a distance of 1827.32 feet to a 1/2 inch rebar found;

THENCE through the interior of said Foust tract, NORTH 70 degrees 14 minutes 35 seconds WEST, a distance of 281.55 feet to a 1/2 inch rebar with cap stamped, "BARTON CHAPA" set in the west line of said Foust tract;

THENCE with the west line of said Foust tract, NORTH 20 degrees 45 minutes 25 seconds WEST, a distance of 513.22 feet, to a point from which a 4 inch wood post bears SOUTH 63 degrees EAST, a distance of 0.69 feet;

THENCE NORTH 22 degrees 11 minutes 28 seconds WEST, with the west line of said Foust tract, a distance of 182.04 feet, returning to the POINT OF BEGINNING and enclosing 42.682 acres (1,859,225 square feet) of land, more or less.

NOTE: COMPANY DOES NOT REPRESENT THAT THE ABOVE ACREAGE AND/OR SQUARE FOOTAGE CALCULATIONS ARE CORRECT.

ELECTRONICALLY RECORDED OFFICIAL PUBLIC RECORDS

2022117399

Pages: 6 Fee: \$46.00 10/12/2022 11:45 AM MBARRICK

The state of the s

Nancy E. Rister, County Clerk Williamson County, Texas

Daney E. Ruter

Agent Authorization Form

For Required Signature
Edwards Aquifer Protection Program
Relating to 30 TAC Chapter 213
Effective June 1, 1999

l	LANGFORD STUBER	
	Print Name	
	OWNER	
	Title - Owner/President/Other	
of	CHV LIBERTY HILL 29 PROPERTY OWNER LLC	
	Corporation/Partnership/Entity Name	
have authorized	SANDY BRANTLEY	
	Print Name of Agent/Engineer	
of	KIRKMAN ENGINEERING	
	Print Name of Firm	

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

- 1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
- 2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
- 3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
- 4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
- 5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

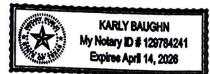
SIGNATURE PAGE:

Sandy Town Change	
	11/1/2022
Applicant's Signature	Date

THE STATE OF TEXAS §
County of Williamson §

BEFORE ME, the undersigned authority, on this day personally appeared Sandy Som Hy known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this day of Awenber, 2002.



NOTARY PUBLIC

Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 4/14/2026

Texas Commission on Environmental Quality

Edwards Aquifer Application Cover Page

Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with 30 TAC 213.

Administrative Review

- 1. <u>Edwards Aquifer applications</u> must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.
 - To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: http://www.tceq.texas.gov/field/eapp.
- 2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
- 3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
- 4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.
 - An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.
- 5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
- 6. If the geologic assessment was completed before October 1, 2004 and the site contains "possibly sensitive" features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

Technical Review

- When an application is deemed administratively complete, the technical review period begins. The regional
 office will distribute copies of the application to the identified affected city, county, and groundwater
 conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days
 to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
- 2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

- clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.
- 3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited**.
- 4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

Mid-Review Modifications

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a "Mid-Review Modification". Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ's Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ's San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: Chalk Hill Ranch					2. Regulated Entity No.:				
3. Customer Name: CHV Liberty Hill 29 Property Owner LLC						4. Customer No.:			
5. Project Type: (Please circle/check one)	New		Modif	Modification		Extension		Exception	
6. Plan Type: (Please circle/check one)	WPAP	(CZP)	4 SCS TUSTTASLIEXPIEXTT		Technical Clarification	Optional Enhanced Measures			
7. Land Use: (Please circle/check one)	Resider	ntial	Non-r	Non-residential 8. S		8. Sit	e (acres):	42.68	
9. Application Fee:	\$ 8,00	0	10. Permanent BMP(s):		s):	Jellyfish- Storm water treatment			
11. SCS (Linear Ft.):	0		12. AST/UST (No. Tanks):			ıks):	0		
13. County:	William	son	14. Watershed:					South Fork Sa	n Gabriel River

Application Distribution

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the "Texas Groundwater Conservation Districts within the EAPP Boundaries" map found at:

http://www.tceq.texas.gov/assets/public/compliance/field_ops/eapp/EAPP%20GWCD%20map.pdf

For more detailed boundaries, please contact the conservation district directly.

Austin Region						
County:	Hays	Travis	Williamson			
Original (1 req.)						
Region (1 req.)	_	_				
County(ies)			_			
Groundwater Conservation District(s)	Edwards Aquifer AuthorityBarton Springs/ Edwards AquiferHays TrinityPlum Creek	Barton Springs/ Edwards Aquifer	NA			
City(ies) Jurisdiction	AustinBudaDripping SpringsKyleMountain CitySan MarcosWimberleyWoodcreek	AustinBee CavePflugervilleRollingwoodRound RockSunset ValleyWest Lake Hills	AustinCedar ParkFlorenceGeorgetownJerrellLeander _X Liberty HillPflugerville _ Round Rock			

	Sa	an Antonio Region			
County:	Bexar	Comal	Kinney	Medina	Uvalde
Original (1 req.)					
Region (1 req.)					
County(ies)					
Groundwater Conservation District(s)	Edwards Aquifer Authority Trinity-Glen Rose	Edwards Aquifer Authority	Kinney	EAA Medina	EAA Uvalde
City(ies) Jurisdiction	Castle HillsFair Oaks RanchHelotesHill Country VillageHollywood ParkSan Antonio (SAWS)Shavano Park	Bulverde Fair Oaks Ranch Garden Ridge New Braunfels Schertz	NA	San Antonio ETJ (SAWS)	NA

I certify that to the best of my knowledge, that the a application is hereby submitted to TCEQ for admini	
Sandy Brantley	
Print Name of Customer/Authorized Agent	
Suly 15-lly	5/25/2023
Signature of Customer/Authorized Agent	Date

FOR TCEQ INTERNAL USE ONLY				
Date(s)Reviewed:	Date Ad	Date Administratively Complete:		
Received From:	Correct	Number of Copies:		
Received By:	Distribu	ntion Date:		
EAPP File Number:	Complex	x:		
Admin. Review(s) (No.):	No. AR	Rounds:		
Delinquent Fees (Y/N):	Review '	Time Spent:		
Lat./Long. Verified:	SOS Cus	stomer Verification:		
Agent Authorization Complete/Notarized (Y/N):	Fee	Payable to TCEQ (Y/N):		
Core Data Form Complete (Y/N):	Check:	Signed (Y/N):		
Core Data Form Incomplete Nos.:		Less than 90 days old (Y/N):		

Contributing Zone Plan Application

Texas Commission on Environmental Quality

for Regulated Activities on the Contributing Zone to the Edwards Aquifer and Relating to 30 TAC §213.24(1), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Contributing Zone Plan Application** is hereby submitted for TCEQ review and Executive Director approval. The application was prepared by:

Print Name of Customer/Agent: Langford Stuber

Date: <u>5/25/</u>2023

Signature of Customer/Agent:

Langford Stuber

Regulated Entity Name: Chalk Hill Ranch

Project Information

1. County: Williamson

2. Stream Basin: Brazos River Basin

3. Groundwater Conservation District (if applicable): N/A

4. Customer (Applicant):

Contact Person: Langford Stuber

Entity: <u>CHV Liberty Hill 29 Property Owner LLC</u>
Mailing Address: 6601 E Hillcrest Avenue, Ste 212

 City, State:
 Dallas, Texas
 Zip: 75205

 Telephone:
 214-435-7510
 Fax: ______

Email Address: <u>Langford@chalkhillventures.com</u>

5.	Agent/Representative (If any):
	Contact Person: Sandy Brantley Entity: Kirkman Engineering Mailing Address: 1130 Cottonwood Creek Trail, Suit C3 City, State: Cedar Park, Texas Zip:78613 Telephone: 512-428-8586 Fax: Email Address: sandy.brantley@trustke.com
6.	Project Location:
	 The project site is located inside the city limits of Liberty Hill The project site is located outside the city limits but inside the ETJ (extra-territorial jurisdiction) of Liberty Hill The project site is not located within any city's limits or ETJ.
7.	X The location of the project site is described below. Sufficient detail and clarity has been provided so that the TCEQ's Regional staff can easily locate the project and site boundaries for a field investigation.
	2,000 ft NW from the corner of W state Highway 29 and County Road 260.
8.	X Attachment A - Road Map. A road map showing directions to and the location of the project site is attached. The map clearly shows the boundary of the project site.
9.	\overline{X} Attachment B - USGS Quadrangle Map. A copy of the official 7 ½ minute USGS Quadrangle Map (Scale: 1" = 2000') is attached. The map(s) clearly show:
	X Project site boundaries.X USGS Quadrangle Name(s).
10.	Attachment C - Project Narrative. A detailed narrative description of the proposed project is attached. The project description is consistent throughout the application and contains, at a minimum, the following details:
	 X Area of the site X Offsite areas X Impervious cover X Permanent BMP(s) X Proposed site use X Site history X Previous development
	X Area(s) to be demolished
11.	. Existing project site conditions are noted below:
	 Existing commercial site Existing industrial site X Existing residential site

Existing paved and/ Undeveloped (Clear Undeveloped (Undir	•
12. The type of project is:	
Residential: # of Lot X Residential: # of Livi Commercial Industrial Other:	s: ing Unit Equivalents: <u>231</u>
13. Total project area (size	of site): <u>42.68</u> Acres
Total disturbed area: 41	<u>.07</u> Acres
14. Estimated projected po	pulation: 0
15. The amount and type o	f impervious cover expected after construction is complete is showr

Table 1 - Impervious Cover

below:

Impervious Cover of Proposed Project	Sq. Ft.	Sq. Ft./Acre	Acres
Structures/Rooftops	139,392	÷ 43,560 =	3.20
Parking	339,332	÷ 43,560 =	7.77
Other paved surfaces		÷ 43,560 =	
Total Impervious Cover	478,724	÷ 43,560 =	10.99

Total Impervious Cover $\underline{10.99}$ ÷ Total Acreage $\underline{42.68}$ X 100 = $\underline{25.75}$ % Impervious Cover

- 16. X Attachment D Factors Affecting Surface Water Quality. A detailed description of all factors that could affect surface water quality is attached. If applicable, this includes the location and description of any discharge associated with industrial activity other than construction.
- 17. X Only inert materials as defined by 30 TAC 330.2 will be used as fill material.

For Road Projects Only

Complete questions 18 - 23 if this application is exclusively for a road project.

X N/A

18. Type of project:
 TXDOT road project. County road or roads built to county specifications. City thoroughfare or roads to be dedicated to a municipality. Street or road providing access to private driveways.
19. Type of pavement or road surface to be used:
ConcreteAsphaltic concrete pavementOther:
20. Right of Way (R.O.W.):
Length of R.O.W.: feet. Width of R.O.W.: feet. L x W = $Ft^2 \div 43,560 Ft^2/Acre = acres.$
21. Pavement Area:
Length of pavement area: feet. Width of pavement area: feet. L x W = Ft² ÷ 43,560 Ft²/Acre = acres. Pavement area acres ÷ R.O.W. area acres x 100 = % impervious cover.
22. A rest stop will be included in this project.
A rest stop will not be included in this project.
23. Maintenance and repair of existing roadways that do not require approval from the TCEQ Executive Director. Modifications to existing roadways such as widening roads/adding shoulders totaling more than one-half (1/2) the width of one (1) existing lane require prior approval from the TCEQ.
Stormwater to be generated by the Proposed Project
24. X Attachment E - Volume and Character of Stormwater. A detailed description of the volume (quantity) and character (quality) of the stormwater runoff which is expected to occur from the proposed project is attached. The estimates of stormwater runoff quality and quantity are based on area and type of impervious cover. Include the runof coefficient of the site for both pre-construction and post-construction conditions.
Wastewater to be generated by the Proposed Project
25. Wastewater is to be discharged in the contributing zone. Requirements under 30 TAC §213.6(c) relating to Wastewater Treatment and Disposal Systems have been satisfied. X N/A

26. Wastewater will be	disposed of by:		
On-Site Sewage	Facility (OSSF/Septic Tar	nk):	
will be used licensing au the land is so the required relating to C Each lot in t size. The sy	to treat and dispose of t thority's (authorized age uitable for the use of pri	the wastewater from the nt) written approval is vate sewage facilities a facilities as specified units at least one (1) acrest a licensed professional	attached. It states that nd will meet or exceed inder 30 TAC Chapter 285 (43,560 square feet) in I engineer or registered
	on System (Sewer Lines) ion system will convey th nt facility is:	·	iberty Hill Vaste Vater (name) Treatment
X Existing. Proposed.			
□ N/A			
Permanent Ab Gallons	oveground Stor	rage Tanks (AS	Ts) ≥ 500
Complete questions 27 greater than or equal to	' - 33 if this project includ to 500 gallons	des the installation of A	AST(s) with volume(s)
XN/A	ie see gamene.		
27. Tanks and substance	ce stored:		
Table 2 - Tanks and	Substance Storage		
AST Number	Size (Gallons)	Substance to be Stored	Tank Material
1			
2			
3			
4			
5			
	placed within a containn times the storage capac	nent structure that is si	•

5 of 11

•	stem, the containm umulative storage c		ed to capture one and	d one-half (1 1/2)
for providir		nment are proposed	ent Methods. Alternd. Specifications sho	
29. Inside dimensi	ons and capacity of	containment structu	ure(s):	
Table 3 - Second	dary Containment	t .		
Length (L)(Ft.)	Width(W)(Ft.)	Height (H)(Ft.)	L x W x H = (Ft3)	Gallons
			То	tal: Gallons
Some of the structure. The piping The piping of the piping of the piping of the contain substance (state of the contain substance).	e piping to dispense will be aboveground will be underground nment area must be s) being stored. The	ers or equipment wild d constructed of and e proposed containn	ings. A scaled drawi	containment vious to the e constructed of:
	nt structure is attacl		-	ing or the
Interna Tanks cl Piping c	· -	=	wall and floor thickno collection of any spi	
storage tan			or collection and rec controlled drainage a	
<u></u>		pillage will be remo	ved from the contain	nment structure

In the event of a spill, any spillage will be drained from the containment structure through a drain and valve within 24 hours of the spill and disposed of properly. The drain and valve system are shown in detail on the scaled drawing.
Site Plan Requirements
Items 34 - 46 must be included on the Site Plan.
34. \boxed{X} The Site Plan must have a minimum scale of 1" = 400'.
Site Plan Scale: 1" = 20'.
35. 100-year floodplain boundaries:
 Some part(s) of the project site is located within the 100-year floodplain. The floodplain is shown and labeled. No part of the project site is located within the 100-year floodplain. The 100-year floodplain boundaries are based on the following specific (including date of material) sources(s):
36. X The layout of the development is shown with existing and finished contours at appropriate, but not greater than ten-foot contour intervals. Lots, recreation centers, buildings, roads, etc. are shown on the site plan.
The layout of the development is shown with existing contours at appropriate, but not greater than ten-foot contour intervals. Finished topographic contours will not differ from the existing topographic configuration and are not shown. Lots, recreation centers, buildings, roads, etc. are shown on the site plan.
37. \boxed{X} A drainage plan showing all paths of drainage from the site to surface streams.
38. $\boxed{\text{X}}$ The drainage patterns and approximate slopes anticipated after major grading activities.
39. X Areas of soil disturbance and areas which will not be disturbed.
40. X Locations of major structural and nonstructural controls. These are the temporary and permanent best management practices.
41. X Locations where soil stabilization practices are expected to occur.
42. Surface waters (including wetlands).
X N/A
43. X Locations where stormwater discharges to surface water.
There will be no discharges to surface water.
44. Temporary aboveground storage tank facilities.
X Temporary aboveground storage tank facilities will not be located on this site.

45.	Permanent aboveground storage tank facilities.
X	Permanent aboveground storage tank facilities will not be located on this site.
46. X	Legal boundaries of the site are shown.
Per	manent Best Management Practices (BMPs)
Practi	ices and measures that will be used during and after construction is completed.
47. X	Permanent BMPs and measures must be implemented to control the discharge of pollution from regulated activities after the completion of construction.
] N/A
48. X	These practices and measures have been designed, and will be constructed, operated, and maintained to insure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity is removed. These quantities have been calculated in accordance with technical guidance prepared or accepted by the executive director.
	 X The TCEQ Technical Guidance Manual (TGM) was used to design permanent BMPs and measures for this site. A technical guidance other than the TCEQ TGM was used to design permanent BMPs and measures for this site. The complete citation for the technical guidance that was used is:
] N/A
49. X	Owners must insure that permanent BMPs and measures are constructed and function as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the appropriate regional office within 30 days of site completion. N/A
les pe pe wl Ar	there a site is used for low density single-family residential development and has 20 % or as impervious cover, other permanent BMPs are not required. This exemption from ermanent BMPs must be recorded in the county deed records, with a notice that if the ercent impervious cover increases above 20% or land use changes, the exemption for the hole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to application Processing and Approval), may no longer apply and the property owner must obtify the appropriate regional office of these changes.
	 The site will be used for low density single-family residential development and has 20% or less impervious cover. The site will be used for low density single-family residential development but has more than 20% impervious cover.
	X The site will not be used for low density single-family residential development.

51. The executive director may waive the requirement for other permanent BMPs for multifamily residential developments, schools, or small business sites where 20% or less impervious cover is used at the site. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
 Attachment I - 20% or Less Impervious Cover Waiver. The site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is attached. X The site will be used for multi-family residential developments, schools, or small business sites but has more than 20% impervious cover. The site will not be used for multi-family residential developments, schools, or small business sites.
52. X Attachment J - BMPs for Upgradient Stormwater.
 X A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is attached. X No surface water, groundwater or stormwater originates upgradient from the site and flows across the site, and an explanation is attached. X Permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site, and an explanation is attached.
53. X Attachment K - BMPs for On-site Stormwater.
 A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is attached. Permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, and an explanation is attached.
54. Attachment L - BMPs for Surface Streams. A description of the BMPs and measures that prevent pollutants from entering surface streams is attached.
X N/A
55. X Attachment M - Construction Plans. Construction plans and design calculations for the proposed permanent BMPs and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer, and are signed, sealed, and dated. Construction plans for the proposed permanent BMPs and measures are

	attached and include: Design calculations, TCEQ Construction Notes, all proposed structural plans and specifications, and appropriate details.
	N/A
56. X	Attachment N - Inspection, Maintenance, Repair and Retrofit Plan . A site and BMP specific plan for the inspection, maintenance, repair, and, if necessary, retrofit of the permanent BMPs and measures is attached. The plan fulfills all of the following:
	X Prepared and certified by the engineer designing the permanent BMPs and measures
	 X Signed by the owner or responsible party X Outlines specific procedures for documenting inspections, maintenance, repairs, and, if necessary, retrofit. X Contains a discussion of record keeping procedures
	N/A
57. 🗌	Attachment O - Pilot-Scale Field Testing Plan. Pilot studies for BMPs that are not recognized by the Executive Director require prior approval from the TCEQ. A plan for pilot-scale field testing is attached.
X	N/A
58 	Attachment P - Measures for Minimizing Surface Stream Contamination. A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity, which increase erosion that result in water quality degradation. N/A

Responsibility for Maintenance of Permanent BMPs and Measures after Construction is Complete.

- 59. X The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
- 60. X A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development,

or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.

Administrative Information

- 61. X Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions.
- 62. X Any modification of this Contributing Zone Plan may require TCEQ review and Executive Director approval prior to construction, and may require submission of a revised application, with appropriate fees.
- 63. X The site description, controls, maintenance, and inspection requirements for the storm water pollution prevention plan (SWPPP) developed under the EPA NPDES general permits for stormwater discharges have been submitted to fulfill paragraphs 30 TAC §213.24(1-5) of the technical report. All requirements of 30 TAC §213.24(1-5) have been met by the SWPPP document.
 - X The Temporary Stormwater Section (TCEQ-0602) is included with the application.



TCEQ Core Data Form

TCEQ Use Only

For detailed instructions regarding completion of this form, please read the Core Data Form Instructions or call 512-239-5175.

SECTION I: General Information

1. Reason fo	r Submis	sion (If other is c	hecked please	describe in	space p	orovide	d.)				
New Permit, Registration or Authorization (Core Data Form should be submitted with the program application.)											
Renewal (Core Data Form should be submitted with the renewal form)											
2. Customer	Referenc	e Number <i>(if iss</i>	•	Follow this link to search 3. Regulated Entity Reference Number (if issued)					if issued)		
CN for CN or RN numbers in Central Registry** RN											
SECTION	II: Cu	stomer Info	<u>ormation</u>								
4. General C	ustomer l	nformation	5. Effective D	Date for Customer Information Updates (mm/dd/yyyy)							
	New Customer ☐ Update to Customer Information ☐ Change in Regulated Entity Ownership ☐ Change in Legal Name (Verifiable with the Texas Secretary of State or Texas Comptroller of Public Accounts)										
										rrent and	active with the
		State (SOS)	_	•			•				
6. Customer	Legal Nar	me (If an individual	, print last name	first: eg: Doe	, John)		<u> </u>	f new Cu	stomer, enter previ	ous Custome	er below:
CHV Libe	rty Hill	29 Property	Owner LLC	2							
7. TX SOS/CI	•		8. TX State T		its)		(9. Federa	al Tax ID (9 digits)	10. DUN	S Number (if applicable)
08046258	53		32085229	139							
11. Type of C	ustomer:		on		Individ	ual		Pai	rtnership: 🔲 Gener	al 🛛 Limited	
Government:	☐ City ☐ (County 🔲 Federal 🗆] State ☐ Other		Sole P	roprieto	orship	。	Other:		
12. Number ○ 0-20	of Employ 21-100	ees 101-250	<u></u>	☐ 501 a	nd high	er		13. Inder ⊠ Yes	pendently Owned	and Opera	ted?
14. Custome	r Role (Pro	oposed or Actual) -	as it relates to th	he Regulated	Entity li	isted on	this fo	orm. Pleas	se check one of the	following	
Owner		Operat	or	⊠ 0	wner &	Opera	tor				
Occupatio	nal Licens	ee Respo	nsible Party	V	oluntar	y Clean	up A	pplicant	Other:		
45 Mailina	6601 E	E Hillcrest Av	venue, Ste 2	212							
15. Mailing Address:											
	City	Dallas		State	TX		ZIP	7520	05	ZIP + 4	
16. Country	Mailing In	formation (if outsi	de USA)			17. E-	Mail	Addres	S (if applicable)		
						Lang	gfor	d@cha	alkhillventure	s.com	
18. Telephon	e Numbe			19. Extensi	on or (Code			20. Fax Numbe	r (if applical	ole)
(214) 435-7510									()	-	
SECTION	III: Re	egulated En	tity Infori	mation							
		-	-		ty" is se	elected	belo	w this for	m should be acco	mpanied by	a permit application)
New Regulation New	ulated Enti	ty 🔲 Update	to Regulated E	ntity Name		Update	to R	egulated	Entity Information		
		•	•	•	ed in	order	to n	neet TC	CEQ Agency D	ata Stand	lards (removal
		ndings such									
		ame (Enter name			d action	is taking	plac	e.)			
CHV Libe	CHV Liberty Hill 29 Property Owner LLC										

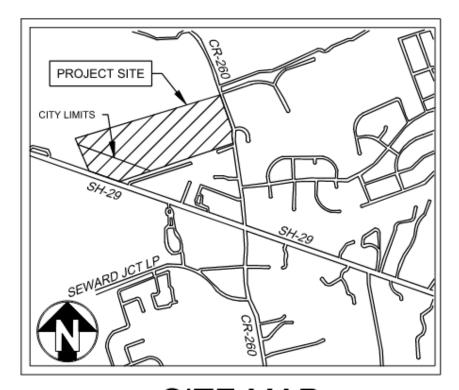
TCEQ-10400 (04/20) Page 1 of 2

23. Street Address of the Regulated Entity:											
(No PO Boxes)		City			State		ZIP			ZIP + 4	
24. County								•			
		Е	nter Phy	ysical Lo	ocation Descripti	on if no stre	et addres	s is pro	vided.		
25. Description to Physical Location		2,000 ft	NW f	rom th	e corner of W	state Hig	ghway 2	9 and	County	Road 260.	
26. Nearest City								State		Nea	rest ZIP Code
Liberty Hill								TX		780	642
27. Latitude (N) In	n Decima	al:	30.55	585692	2	28. Lo	ngitude (W) In D	ecimal:	97.79958	886
Degrees		Minutes		5	Seconds	Degree	S		Minutes		Seconds
29. Primary SIC C	Sode (4 d	igits) 30.	Second	ary SIC	Code (4 digits)	31. Primar (5 or 6 digits)		ode		econdary NA digits)	ICS Code
6552						531110					
33. What is the Pr		Business o	f this en	itity?	(Do not repeat the SIC	or NAICS desc	ription.)		•		
Multi - Family	У										
34. Mailing							tbd				
Address:											
		City			State		ZIP			ZIP + 4	
35. E-Mail Ad	ddress:					justin@ch	alkhillven	tures.c	om		
36. Telephone Number				37. Extension		3	38. Fax Number (if applicable)				
				1	OTT EXTORIOR						
(512) 76								() -	
9. TCEQ Programs	and ID	61-5025 Numbers (Check all	Programs	s and write in the pe	mits/registrati	on numbers		() -	•
9. TCEQ Programs orm. See the Core Data Dam Safety	and ID	61-5025 Numbers (Check all	Programs nal guidan	s and write in the pe			s that will	() -	•
9. TCEQ Programs orm. See the Core Data	and ID	Numbers (structions fo	Check all	Programs aal guidan	s and write in the pe			s that will	be affected) -	submitted on this
9. TCEQ Programs orm. See the Core Data	and ID a Form in	Numbers (structions fo	Check all	nal guidan	s and write in the pe		☐ Emissi	s that will	be affected) -	submitted on this
9. TCEQ Programs orm. See the Core Data Dam Safety Municipal Solid Wa	and ID a Form in	Numbers (structions fo District	Check all or addition ts	nal guidan	s and write in the perice. Edwards Aqu OSSF		☐ Emissi	s that will	be affected entory Air) - I by the updates Industria	submitted on this I Hazardous Waste
9. TCEQ Programs orm. See the Core Data Dam Safety	and ID a Form in	Numbers Constructions for District New So Storm	Check all praddition ts ource Re	nal guidan	s and write in the per nce. Edwards Aqu		☐ Emissi	s that will	be affected entory Air) - I by the updates	submitted on this I Hazardous Waste
9. TCEQ Programs orm. See the Core Data Dam Safety Municipal Solid Wa	and ID a Form in	Numbers (structions fo District New So Storm	Check all or addition ts ource Re	nal guidan	s and write in the perice. Edwards Aqu OSSF Title V Air	fer	☐ Emissi	s that will	be affected entory Air) - I by the updates Industria PWS Used Oil	submitted on this I Hazardous Waste
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9. TCEQ Programs orm. See the Core Data Dam Safety Municipal Solid Wa Sludge Voluntary Cleanup SECTION IV:	and ID a Form in asste	Numbers (structions fo District New St Storm TXR150 Waste	Check all or addition ts ource Re Water 0000 Water	al guidan	s and write in the perice. Edwards Aqu OSSF Title V Air	fer	☐ Emissi ☐ Petrole ☐ Tires ☐ Water	s that will	tentory Air) - I by the updates Industria PWS Used Oil	submitted on this I Hazardous Waste
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9. TCEQ Programs orm. See the Core Data Dam Safety Municipal Solid Water Sludge Voluntary Cleanup SECTION IV: 40. Name: Sandy E	and ID a Form in l'aste Prep Brantle	Numbers (structions fo District New St Storm TXR150 Waste	Check all or addition ts ource Re Water 10000 Water 110000	view Air	s and write in the perice. Edwards Aqu OSSF Title V Air Wastewater A	griculture 41. Title: 45. E-Ma	☐ Emissi ☐ Petrole ☐ Tires ☐ Water ☐ Civil	s that will ions Inve	tentory Air	by the updates Industria PWS Used Oil	submitted on this I Hazardous Waste
9. TCEQ Programs orm. See the Core Data Dam Safety Municipal Solid Wa Sludge Voluntary Cleanup SECTION IV: 40. Name: Sandy F 42. Telephone Num (512) 428-8586	and ID a Form in l'aste Preg Brantle nber 4	Numbers (structions fo District New So Storm TXR150 Waste Darer In Ey 3. Ext./Coc	Check all or addition its ource Re Water 0000 Water	view Air ation 44. Fax	s and write in the perice. Edwards Aqu OSSF Title V Air Wastewater A	griculture 41. Title: 45. E-Ma	☐ Emissi ☐ Petrole ☐ Tires ☐ Water ☐ Civil	s that will ions Inve	tentory Air rage Tank	by the updates Industria PWS Used Oil	submitted on this I Hazardous Waste
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9. TCEQ Programs orm. See the Core Data Dam Safety Municipal Solid Water Sludge Voluntary Cleanup SECTION IV: 40. Name: Sandy F 42. Telephone Num (512) 428-8586 SECTION V: 66. By my signature ignature authority to dentified in field 39.	and ID a Form in daste 2 2 3 3 3 4 4 6 4 4 6 Auth below, 1 submit	Numbers (structions fo District New So Storm TXR150 Waste Darer Ir Ey 3. Ext./Coc	Check all or addition its ource Re Water 1000 Water 1form de Signa the best in behalf	ation 44. Fax (ature of my katority in the er	s and write in the perice. Edwards Aqu OSSF Title V Air Wastewater A Number nowledge, that the attity specified in S	griculture 41. Title: 45. E-Ma sandy.	Petrole Tires Water Civil il Address brantley provided in the stand of the sta	s that will ions Inve	the affected entory Air rage Tank neer extke.com	by the updates Industria PWS Used Oil Other:	s submitted on this I Hazardous Waste
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ATTACHMENT A ROAD MAP





SITE MAP

N.T.S.

ATTACHMENT B USGS QUADRANGLE MAP

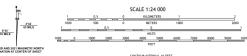
















7.5-MINUTE TOPO 1, TX 2022

ATTACHMENT C PROJECT NARRATIVE



The property is 2000 ft from State highway 29 and County Road 260 intersection, The site's address is 350 County Road 260, Liberty Hill, Texas. Existing land use of the 42.68 acres site is agricultural and has one single family residential unit. The proposed land use for the site is for multifamily and commercial development. 6.84 acres for the site that is on the city limits will be future commercial.

Roadway and utility Infrastructure will be constructed with this phase to serve future and proposed developments. 16.371 acres will be used as multifamily commercial in Phase one. 18.934 ac north of multifamily commercial phase one will remain undeveloped. The existing site is predominantly grass. The existing impervious area includes a residential home and access driveways, of which 488 SF of access driveway will be demolished. The proposed impervious area is 10.99 acres.

There is no offsite area draining into this property. Of the 41.07 acres of disturbed land, 10.93 acres will ultimately be impervious surface.

The site will feature temporary BMPs during constructions which will include silt fence, a stabilized construction exists and inlet protection for new inlets before stabilization.

Jellyfish storm water treatment system from Contech will be used on site as a permanent BMP measure.

ATTACHMENT D FACTORS AFFECTING SURFACE WATER QUALITY



The factors that could affect surface water quality are as follows:

- 1. Lack of natural infiltration
- 2. Pollutants from vehicles and surface

ATTACHMENT E VOLUME AND CHARACTER OF STORMWATER



Preconstruction conditions of the site shows the land sheet flowing across the site and exiting at the southwest boundary of the property following the existing contour. Using the Modified Rational Method, 134.45 cfs drains to the site at the southwest towards State Highway 29 (SH 29) and 101.10 cfs drains to the southeast at one discharge point into the existing adjacent property in the 100-year storm event. Preconstruction runoff coefficient for the site is 0.42.

For the post construction condition of the site, the stormwater sheet flows to the underground storm system. The underground storm system outfalls at either of the two proposed detention pond on site. Approximately 18.97 acres of the site with a runoff coefficient of 0.6 flows off-site towards the SH29. Approximately 179.91 cfs enters the southeastern detention pond and discharges at 101.10 cfs at peak discharge of the ultimate 100-year flow, with 100-year runoff with coefficient of 0.63. Approximately 69.53 cfs enters the western detention pond and discharges at 101.10 cfs at peak discharge, with 100-year runoff coefficient of 0.72.

Reference Attachment M- Construction Plans for more detailed information.

Permanent Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(C), (D)(Ii), (E), and (5), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Permanent Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

	ecutive director approval. The application was prepared by:
Pri	nt Name of Customer/Agent: <u>Langford Stuber</u>
Da	te: <u>5/25/202</u> 3
Sig	nature of Customer/Agent
<u></u>	angford Stuber
Re	gulated Entity Name: Chalk Hill Ranch
P	ermanent Best Management Practices (BMPs)
	rmanent best management practices and measures that will be used during and after nstruction is completed.
1.	X Permanent BMPs and measures must be implemented to control the discharge of pollution from regulated activities after the completion of construction.
	□ N/A
2.	These practices and measures have been designed, and will be constructed, operated, and maintained to insure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity is removed. These quantities have been calculated in accordance with technical guidance prepared or accepted by the executive director.
	X The TCEQ Technical Guidance Manual (TGM) was used to design permanent BMPs

and measures for this site.

	A technical guidance other than the TCEQ TGM was used to design permanent BMPs and measures for this site. The complete citation for the technical guidance that was used is:
	□ N/A
3.	Owners must insure that permanent BMPs and measures are constructed and function as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the appropriate regional office within 30 days of site completion.
	□ N/A
4.	Where a site is used for low density single-family residential development and has 20 % or less impervious cover, other permanent BMPs are not required. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
	 □ The site will be used for low density single-family residential development and has 20% or less impervious cover. □ The site will be used for low density single-family residential development but has more than 20% impervious cover. ✓ The site will not be used for low density single-family residential development.
5.	The executive director may waive the requirement for other permanent BMPs for multifamily residential developments, schools, or small business sites where 20% or less impervious cover is used at the site. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
	 Attachment A - 20% or Less Impervious Cover Waiver. The site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is attached.
6	Attachment B - BMPs for Ungradient Stormwater

		 A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is attached. No surface water, groundwater or stormwater originates upgradient from the site and flows across the site, and an explanation is attached. Permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and
7.	X	flows across the site, and an explanation is attached. Attachment C - BMPs for On-site Stormwater.
		 A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is attached. Permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, and an explanation is attached.
8.		Attachment D - BMPs for Surface Streams . A description of the BMPs and measures that prevent pollutants from entering surface streams, sensitive features, or the aquifer is attached. Each feature identified in the Geologic Assessment as sensitive has been addressed.
	Χ	N/A
9.	Χ	The applicant understands that to the extent practicable, BMPs and measures must maintain flow to naturally occurring sensitive features identified in either the geologic assessment, executive director review, or during excavation, blasting, or construction.
		 ✓ The permanent sealing of or diversion of flow from a naturally-occurring sensitive feature that accepts recharge to the Edwards Aquifer as a permanent pollution abatement measure has not been proposed. ✓ Attachment E - Request to Seal Features. A request to seal a naturally-occurring sensitive feature, that includes, for each feature, a justification as to why no reasonable and practicable alternative exists, is attached.
10.	X	Attachment F - Construction Plans . All construction plans and design calculations for the proposed permanent BMP(s) and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer, and are signed, sealed, and dated. The plans are attached and, if applicable include:
		 X Design calculations (TSS removal calculations) X TCEQ construction notes X All geologic features X All proposed structural BMP(s) plans and specifications
		N/A

11. X	Attachment G - Inspection, Maintenance, Repair and Retrofit Plan . A plan for the inspection, maintenance, repairs, and, if necessary, retrofit of the permanent BMPs and measures is attached. The plan includes all of the following:
	 X Prepared and certified by the engineer designing the permanent BMPs and measures X Signed by the owner or responsible party X Procedures for documenting inspections, maintenance, repairs, and, if necessary retrofit X A discussion of record keeping procedures
	N/A
12.	Attachment H - Pilot-Scale Field Testing Plan . Pilot studies for BMPs that are not recognized by the Executive Director require prior approval from the TCEQ. A plan for pilot-scale field testing is attached.
X	N/A
13.	Attachment I -Measures for Minimizing Surface Stream Contamination. A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity, which increase erosion that results in water quality degradation.
X	N/A
Res	ponsibility for Maintenance of Permanent BMP(s)
-	nsibility for maintenance of best management practices and measures after uction is complete.
14. X	The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
] N/A
15. X	A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development, or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.
	N/A

ATTACHMENT J BMPs FOR UPGRADIENT STORMWATER



The site does not feature upgradient stormwater. The offsite stormwater enters a bypass swale at the property line and flows southwest to State Highway 29.

Reference Attachment M- Construction Plans for more detailed information.

ATTACHMENT K BMPs FOR ON-SITE STORMWATER



The following temporary and permanent BMPs have been identified on the Erosion Control Plan and the Water Quality Plan in Attachment M – Construction Plans.

- 1. Silt Fence
- 2. Inlet Protection
- 3. Stabilized Construction Exit
- 4. Concrete Washout Pit
- 5. Vegetation
- 6. Jellyfish Stormwater treatment

Onsite stormwater that enters the site will flow into the designed stormwater system and be treated by the Jellyfish stormwater treatment system, which then enters the detention ponds. There are four Jellyfish units designed to treat on site stormwater. See below for removal efficiency for each BMP's.

BMP #1 has 86% removal efficiency. BMP #2 has 86% removal efficiency. BMP #3 has 84% removal efficiency. BMP #4 has 86% removal efficiency.

Refer to the following attachments for the BMP calculations.

Project Name: Foust Tract - Chalk Hill Ventures

Date Prepared: 1/24/2023

1. The Required Load Reduction for the total project:

Calculations from RG-348

Page 3-29 Equation 3.3: $L_{M} = 27.2(A_{N} \times P)$

Pages 3-27 to 3-30

 $L_{\text{M TOTAL PROJECT}} = \text{ Required TSS removal resulting from the proposed development} = 80\% \text{ of increased load}$

 A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

Williamson County =Total project area included in plan * = 26.18 acres Predevelopment impervious area within the limits of the plan * = 0.09 acres Total post-development impervious area within the limits of the plan* = 11.30 acres Total post-development impervious cover fraction * = 0.43 inches 32 lbs. $L_{M TOTAL PROJECT} =$ 9753

Number of drainage basins / outfalls areas leaving the plan area =

2. Drainage Basin Parameters (This information should be provided for each basin):

	DMI I	Dramage Basin/Outlan Area No. =
acres	5.40	Total drainage basin/outfall area =
acres	0.00	Predevelopment impervious area within drainage basin/outfall area =
acres	2.98	Post-development impervious area within drainage basin/outfall area =
	0.55	Post-development impervious fraction within drainage basin/outfall area =
lbs.	2593	$L_{\text{M THIS RASIN}} =$

Drainaga Racin/Outfall Area No. -

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = **JF** abbreviation Removal efficiency = **86** percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

RG-348 Page 3-33 Equation 3.7: $LR = (BMP \text{ efficiency}) \times P \times (A_1 \times 34.6 + A_P \times 0.54)$

 $A_{\rm C}$ = Total On-Site drainage area in the BMP catchment area

 $A_{\rm I}$ = Impervious area proposed in the BMP catchment area

 A_P = Pervious area remaining in the BMP catchment area

 L_R = TSS Load removed from this catchment area by the proposed BMP

${\bf 5.\ Calculate\ Fraction\ of\ Annual\ Runoff\ to\ Treat\ the\ drainage\ basin\ /\ outfall\ area}$

Desired $L_{M THIS BASIN} =$ 2593 lbs. F = 0.90

$\underline{\textbf{6. Calculate Treated Flow required by the BMP Type for this drainage basin / outfall area.}\\$

Offsite area draining to BMP = 0.00 acres
Offsite impervious cover draining to BMP = 0.00 acres

Calculations from RG-348 Pages Section 3.2.22

Rainfall Intensity = 1.10 inches per hour Effective Area = 2.75 acres Cartridge Length = 54 inches

Peak Treatment Flow Required = 3.05 cubic feet per second

7. Jellyfish

Designed as Required in RG-348 Section 3.2.22

Flow Through Jellyfish Size

Jellyfish Size for Flow-Based Configuration = JFPD0811-16-4

Jellyfish Treatment Flow Rate = 3.21 cf

Project Name: Foust Tract - Chalk Hill Ventures

Date Prepared: 1/24/2023

1. The Required Load Reduction for the total project:

Calculations from RG-348 Pages 3-27 to 3-30 Page 3-29 Equation 3.3: $L_{M} = 27.2(A_{N} \times P)$

 $L_{\text{M TOTAL PROJECT}} = \text{ Required TSS removal resulting from the proposed development} = 80\% \text{ of increased load}$

 A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

Williamson County =Total project area included in plan * = 26.18 acres Predevelopment impervious area within the limits of the plan * = 0.09 acres Total post-development impervious area within the limits of the plan* = 11.30 acres Total post-development impervious cover fraction * = 0.43 inches 32 $L_{M TOTAL PROJECT} =$ 9753

Number of drainage basins / outfalls areas leaving the plan area = 4

2. Drainage Basin Parameters (This information should be provided for each basin):

	DMP 2	Dramage Basin/Outlan Area No. =
acres	2.69	Total drainage basin/outfall area =
acres	0.00	Predevelopment impervious area within drainage basin/outfall area =
acres	1.45	Post-development impervious area within drainage basin/outfall area =
	0.54	Post-development impervious fraction within drainage basin/outfall area =
lbs.	1262	$L_{M THIS BASIN} =$

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = **JF** abbreviation Removal efficiency = **86** percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

RG-348 Page 3-33 Equation 3.7: LR = (BMP efficiency) x P x (A_I x 34.6 + A_P x 0.54)

 A_C = Total On-Site drainage area in the BMP catchment area

 $A_{\rm I}$ = Impervious area proposed in the BMP catchment area

 A_P = Pervious area remaining in the BMP catchment area L_R = TSS Load removed from this catchment area by the proposed BMP

 $egin{array}{lll} A_C = & {f 2.69} & {
m acres} \\ A_I = & {f 1.45} & {
m acres} \\ A_P = & {f 1.24} & {
m acres} \\ L_R = & {f 1399} & {
m lbs.} \\ \end{array}$

${\bf 5.\ Calculate\ Fraction\ of\ Annual\ Runoff\ to\ Treat\ the\ drainage\ basin\ /\ outfall\ area}$

Desired $L_{M THIS BASIN} =$ 1262 lbs. F = 0.90

$\underline{\textbf{6. Calculate Treated Flow required by the BMP Type for this drainage basin / outfall area.}\\$

Offsite area draining to BMP = 0.00 acres
Offsite impervious cover draining to BMP = 0.00 acres

Calculations from RG-348

Pages Section 3.2.22

Rainfall Intensity = 1.10 inches per hour
Effective Area = 1.34 acres
Cartridge Length = 54 inches

Peak Treatment Flow Required = 1.49 cubic feet per second

7. Jellyfish

Designed as Required in RG-348 Section 3.2.22

Flow Through Jellyfish Size

Jellyfish Size for Flow-Based Configuration = JFPD0806-8-2

Jellyfish Treatment Flow Rate = 1.60 cfs

Project Name: Foust Tract - Chalk Hill Ventures

Date Prepared: 1/24/2023

1. The Required Load Reduction for the total project:

Calculations from RG-348 Pages 3-27 to 3-30 Page 3-29 Equation 3.3: $L_{M} = 27.2(A_{N} \times P)$

 $L_{\text{M TOTAL PROJECT}} = \text{ Required TSS removal resulting from the proposed development} = 80\% \text{ of increased load}$

 A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

Williamson County =Total project area included in plan * = 26.18 acres Predevelopment impervious area within the limits of the plan * = 0.09 acres Total post-development impervious area within the limits of the plan* = 11.30 acres Total post-development impervious cover fraction * = 0.43 inches 32 lbs. $L_{M TOTAL PROJECT} =$ 9753

Number of drainage basins / outfalls areas leaving the plan area = 4

2. Drainage Basin Parameters (This information should be provided for each basin):

	DMP 3	Dramage basin/Outian Area No. =
acres	16.91	Total drainage basin/outfall area =
acres	0.09	Predevelopment impervious area within drainage basin/outfall area =
acres	5.91	Post-development impervious area within drainage basin/outfall area =
	0.35	Post-development impervious fraction within drainage basin/outfall area =
lbs.	5065	$L_{M THIS BASIN} =$

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = **JF** abbreviation Removal efficiency = **84** percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

RG-348 Page 3-33 Equation 3.7: LR = (BMP efficiency) x P x (A_I x 34.6 + A_P x 0.54)

 A_{C} = Total On-Site drainage area in the BMP catchment area

 $A_{\rm I}$ = Impervious area proposed in the BMP catchment area

 A_P = Pervious area remaining in the BMP catchment area

 $L_{R} = \, TSS \, Load \, removed \, from \, this \, catchment \, area \, by \, the \, proposed \, BMP$

 $egin{array}{lll} A_C = & & {\bf 16.91} & & {
m acres} \\ A_I = & {\bf 5.91} & & {
m acres} \\ A_P = & {\bf 11.00} & & {
m acres} \\ L_R = & {\bf 5629} & & {
m lbs}. \\ \end{array}$

${\bf 5.\ Calculate\ Fraction\ of\ Annual\ Runoff\ to\ Treat\ the\ drainage\ basin\ /\ outfall\ area}$

Desired $L_{M THIS BASIN} = 5065$ lbs. F = 0.90

$\underline{\textbf{6. Calculate Treated Flow required by the BMP Type for this drainage basin / outfall area.}\\$

Offsite area draining to BMP = 0.00 acres
Offsite impervious cover draining to BMP = 0.00 acres

Rainfall Intensity = 1.10 inches per hour
Effective Area = 5.65 acres

Effective Area = **5.65** acres
Cartridge Length = **54** inches

Peak Treatment Flow Required = 6.26 cubic feet per second

7. Jellyfish

Designed as Required in RG-348 Section 3.2.22

Calculations from RG-348 Pages Section 3.2.22

Flow Through Jellyfish Size

Jellyfish Size for Flow-Based Configuration = JFPD0816-32-7

Jellyfish Treatment Flow Rate = 6.33 cfs

Project Name: Foust Tract - Chalk Hill Ventures

Date Prepared: 1/24/2023

1. The Required Load Reduction for the total project:

Calculations from RG-348 Pages 3-27 to 3-30 Page 3-29 Equation 3.3: $L_{M} = 27.2(A_{N} \times P)$

 $L_{\text{M TOTAL PROJECT}} = \text{ Required TSS removal resulting from the proposed development} = 80\% \text{ of increased load}$

 A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

Williamson County =Total project area included in plan * = 26.18 acres Predevelopment impervious area within the limits of the plan * = 0.09 acres Total post-development impervious area within the limits of the plan* = 11.30 acres Total post-development impervious cover fraction * = 0.43 inches 32 lbs. $L_{M TOTAL PROJECT} =$ 9753

Number of drainage basins / outfalls areas leaving the plan area =

2. Drainage Basin Parameters (This information should be provided for each basin):

	DMP 4	Dramage basin/Outlan Area No. =
acres	1.18	Total drainage basin/outfall area =
acres	0.00	Predevelopment impervious area within drainage basin/outfall area =
acres	0.96	Post-development impervious area within drainage basin/outfall area =
	0.81	Post-development impervious fraction within drainage basin/outfall area =
lbs.	833	$L_{M THIS BASIN} =$

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = **JF** abbreviation Removal efficiency = **86** percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

RG-348 Page 3-33 Equation 3.7: $LR = (BMP \ efficiency) \ x \ P \ x \ (A_I \ x \ 34.6 + A_P \ x \ 0.54)$

 A_C = Total On-Site drainage area in the BMP catchment area

 $A_{\rm I}$ = Impervious area proposed in the BMP catchment area

 A_P = Pervious area remaining in the BMP catchment area

 $L_{R} = \, TSS \, Load \, removed \, from \, this \, catchment \, area \, by \, the \, proposed \, BMP$

${\bf 5.\ Calculate\ Fraction\ of\ Annual\ Runoff\ to\ Treat\ the\ drainage\ basin\ /\ outfall\ area}$

Desired $L_{M \text{ THIS BASIN}} = \frac{833}{\text{lbs.}}$ lbs. F = 0.91

$\underline{\textbf{6. Calculate Treated Flow required by the BMP Type for this drainage basin / outfall area.}\\$

Offsite area draining to BMP = 0.00 acres
Offsite impervious cover draining to BMP = 0.00 acres

Rainfall Intensity = 1.15 inches per hour
Effective Area = 0.87 acres

Effective Area = **0.8**7 acres
Cartridge Length = **54** inches

Peak Treatment Flow Required = 1.01 cubic feet per second

7. Jellyfish

Designed as Required in RG-348 Section 3.2.22

Calculations from RG-348 Pages Section 3.2.22

Flow Through Jellyfish Size

Jellyfish Size for Flow-Based Configuration = JFPD0806-5-2

Jellyfish Treatment Flow Rate = 1.07 cfs

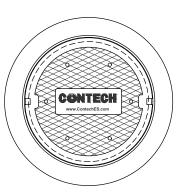
SECTION A-A

FLOW

StormGate[™]

STORMGATE DESIGN NOTES

THE STORMGATE IS A WEIR STYLE DIVERSION STRUCTURE. A 8' [2438] WIDE VAULT CONFIGURATION IS SHOWN. THE UNIT IS DESIGNED SUCH THAT THE TREATMENT FLOW IS DIVERTED TO THE OFFLINE WATER QUALITY DEVICE. THE HIGH FLOWS ARE BYPASSED OVER THE WEIR DIRECTLY TO THE DOWNSTREAM CONVEYANCE. THE STORMGATE TYPICALLY HAS ONE INLET, A LOW FLOW OUTLET FOR TREATMENT FLOW, AND A HIGH FLOW BYPASS OUTLET. THE STORMGATE INCORPORATES A FIELD-ADJUSTABLE ALUMINUM WEIR PLATE WHICH ALLOWS FOR 6" [152] OF WEIR HEIGHT ADJUSTMENT AFTER INSTALLATION. THE CAPACITY OF THE STORMGATE IS A FUNCTION OF THE AVAILABLE HEIGHT ABOVE THE WEIR (HEAD) AND UNIT GEOMETRY. PLEASE CONTACT YOUR LOCAL CONTECH DESIGN ENGINEER FOR SIZING ASSISTANCE.



FRAME AND COVER (DIAMETER VARIES) NOT TO SCALE

SITE SPECIFIC					
DATA REQUIREMENTS					
STRUCTURE ID				ST-C BYPASS	
WATER QUALITY (WQ) FLOW	RATE (cfs [L/s])		6.32 cfs	
PEAK FLOW RATE	(cfs [L/s])			*	
RIM ELEVATION				995.56	
PIPE DATA:	INVERT	ORIENTATION	MATERIAL	DIAMETER	
INLET PIPE	989.41	16' SIDE	RCB	(2) 5'x3'	
WQ FLOW OUTLET PIPE	989.41	8' SIDE	RCP	18"	
PEAK FLOW OUTLET PIPE	(2) 5'x3'				
ORIFICE TYPE (PIF					
ORIFICE DIAMETE	R (in. [mm])				
WEIR CREST ELEV	990.51				
WEIR WALL ELEVATION				991.01	
HEAD OVER WEIR,					
WSE AT PEAK FLO					
WEIR ORIENTATION					
FLOOR ELEVATION					
				•	

NOTES / SPECIAL REQUIREMENTS:

UNIT IS 8'x16'

SPECIAL STRUCTURAL DESIGN REQUIRED DUE TO (2) 5'x3' RCB

GENERAL NOTES

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.ContechES.com
- 3. STORMGATE WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS THE REQUIREMENTS OF THE PROJECT.
- 4. STORMGATE STRUCTURE SHALL MEET AASHTO HS-20 LOAD RATING, ASSUMING EARTH COVER OF 0' 5' [1524] AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.
- 5. STORMGATE STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C857, ASTM C918 AND ACI-318 LOAD FACTOR DESIGN METHOD.
- 6. WEIR FRAME AND ADJUSTABLE WEIR PLATE SHALL BE FABRICATED OF ALUMINUM ALLOY ASTM B209, T-5052, T-6061, PROVIDED AND INSTALLED BY CONTECH.
- 7. ALTERNATE UNITS ARE SHOWN IN [mm].

INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMGATE STRUCTURE.
- C. CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
- D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET AND OUTLET PIPE(S). MATCH PIPE INVERTS WITH ELEVATIONS SHOWN ON SITE SPECIFIC DRAWINGS. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES.
- E. CONTRACTOR TO INSTALL GRADE RINGS/RISERS OR BLOCK REQUIRED BETWEEN THE TOP OF THE STRUCTURE AND THE BASE OF THE MANHOLE FRAMES.
- F. CONTRACTOR TO ADJUST STORMGATE WEIR TO DESIGN ELEVATION SPECIFIED IN DRAWING. DO NOT EXCEED 5.0 FT-LBS TORQUE WHEN TIGHTENING SCREWS ON WEIR FRAME. SEAL WEIR TO FRAME WITH SIKAFLEX 1A SEALANT AFTER FINAL ADJUSTMENT.

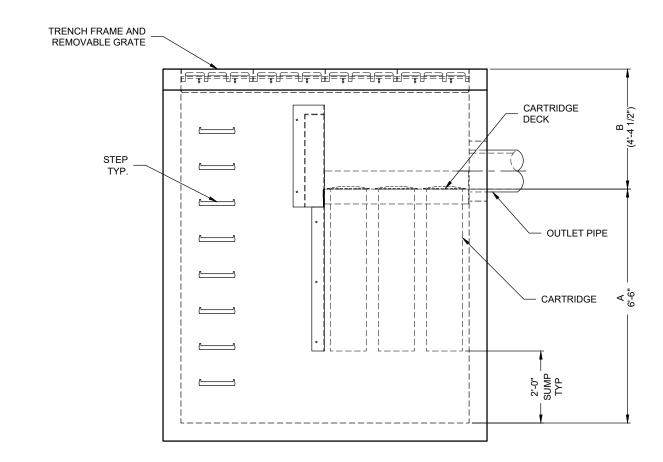


800-338-1122 513-645-7000 513-645-7993 FAX

8' WIDTH x VARYING LENGTH STORMGATE VAULT STANDARD DETAIL

PLAN VIEW

(TOP SLAB NOT SHOWN FOR CLARITY)



ELEVATION VIEW

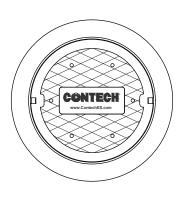


JELLYFISH DESIGN NOTES

JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD SURFACE INLET STYLE WITH TRENCH GRATE AND COVER IS SHOWN. ALTERNATE CURB INLET, PIPE INLET, OR SLAB TOP WITH EARTH COVER OPTIONS ARE AVAILABLE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD.

CARTRIDGE SELECTION

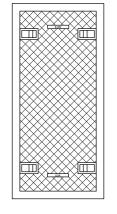
CARTRIDGE LENGTH	54"	40"	27"	15"
OUTLET INVERT TO STRUCTURE INVERT (A)	6'-6"	5'-4"	4'-3"	3'-3"
FLOW RATE HIGH-FLO / DRAINDOWN (CFS) (PER CART)	0.178 / 0.089	0.133 / 0.067	0.089 / 0.045	0.049 / 0.025
MAX. TREATMENT (CFS)	1.34	1.00	0.67	0.37
OUTLET INVERT TO RIM (MIN) (B)	3'-4"	3'-4"	3'-4"	3'-4"



FRAME AND COVER

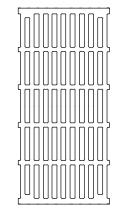
(DIAMETER VARIES)

N.T.S.



24"

TRENCH COVER



24" **TRENCH GRATE**

OF CARTRIDGES REQUIRED (HF / DD) 5/2 CARTRIDGE LENGTH 54 MAT'L DIA SLOPE % HGL PIPE DATA: I.E. INLET #1 INLET #2 979.50 RCP OUTLET SEE GENERAL NOTES 6-7 FOR INLET AND OUTLET

SITE SPECIFIC **DATA REQUIREMENTS**

> ST-G 0.99

HYDRAULIC AND SIZING REQUIREMENTS.

RIM ELEVATION	983.89	
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT
	*	*

NOTES/SPECIAL REQUIREMENTS:

TOP OF BAFFLE WALL ELEV. = 981.83'

* PER ENGINEER OF RECORD

STRUCTURE ID

PEAK FLOW RATE (cfs)

WATER QUALITY FLOW RATE (cfs)

RETURN PERIOD OF PEAK FLOW (yrs)

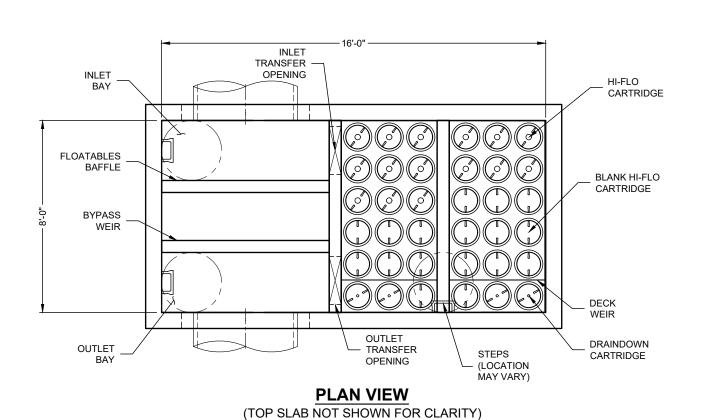
- CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS REPRESENTATIVE. www.ContechES.com
- 3. JELLYFISH WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
- 4. STRUCTURE SHALL MEET AASHTO HS-20 OR PER APPROVING JURISDICTION REQUIREMENTS, WHICHEVER IS MORE STRINGENT, ASSUMING EARTH COVER OF 0' TO 10', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 LOAD RATING AND BE CAST WITH THE CONTECH LOGO.
- STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C-857, ASTM C-918, AND AASHTO LOAD FACTOR DESIGN METHOD.
- 6. OUTLET PIPE INVERT IS EQUAL TO THE CARTRIDGE DECK ELEVATION.
- 7. THE OUTLET PIPE DIAMETER FOR NEW INSTALLATIONS IS RECOMMENDED TO BE ONE PIPE SIZE LARGER THAN THE INLET PIPE (WHERE APPLICABLE) AT EQUAL OR GREATER SLOPE.
- 8. NO PRODUCT SUBSTITUTIONS SHALL BE ACCEPTED UNLESS SUBMITTED 10 DAYS PRIOR TO PROJECT BID DATE, OR AS DIRECTED BY THE ENGINEER OF RECORD.

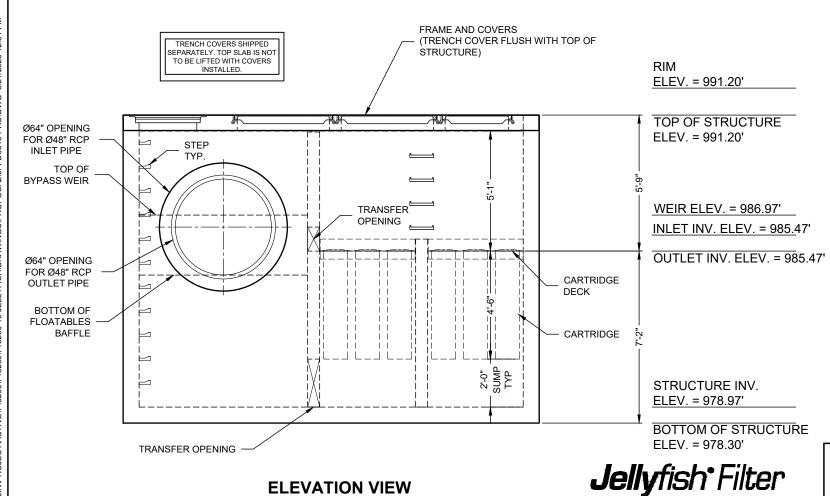
- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STRUCTURE
- C. CONTRACTOR WILL INSTALL AND LEVEL THE STRUCTURE, SEALING THE JOINTS, LINE ENTRY AND EXIT POINTS (NON-SHRINK GROUT WITH APPROVED WATERSTOP OR FLEXIBLE BOOT).
- D. CARTRIDGE INSTALLATION, BY CONTECH, SHALL OCCUR ONLY AFTER SITE HAS BEEN STABILIZED AND THE JELLYFISH UNIT IS CLEAN AND FREE OF DEBRIS. CONTACT CONTECH TO COORDINATE CARTRIDGE INSTALLATION WITH SITE STABILIZATION.



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JELLYFISH JFSI0408 STANDARD DETAIL SURFACE INLET CONFIGURATION



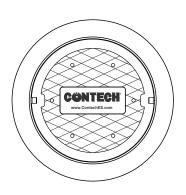


JELLYFISH DESIGN NOTES

JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD PEAK DIVERSION STYLE WITH PRECAST TOP SLAB IS SHOWN. ALTERNATE OFFLINE VAULT AND/OR SHALLOW ORIENTATIONS ARE AVAILABLE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD

CARTRIDGE SELECTION

CARTRIDGE LENGTH	54"
OUTLET INVERT TO STRUCTURE INVERT (A)	6'-6"
FLOW RATE HI-FLO / DRAINDOWN (CFS) (PER CART)	0.178 / 0.089
MAX. TREATMENT (CFS)	7.84
DECK TO INSIDE TOP (MIN) (B)	5.00



FRAME AND COVER (DIAMETER VARIES) N.T.S.

SITE SPECIFIC	
DATA REQUIREMENTS	
TRUCTURE ID	Γ

STRUCTURE ID	ST-A
VATER QUALITY FLOW RATE (cfs)	2.94
PEAK FLOW RATE (cfs)	*
RETURN PERIOD OF PEAK FLOW (yrs)	*
OF CARTRIDGES REQUIRED (HF / DD)	15/4
CARTRIDGE LENGTH	54

PIPE DATA:	I.E.	MAT'L	DIA	SLOPE %	HGL
INLET #1	985.47	RCP	48"	*	*
INLET #2	*	*	*	*	*
OUTLET	985.47	RCP	48"	*	*

SEE GENERAL NOTES 6-7 FOR INLET AND OUTLET HYDRAULIC AND SIZING REQUIREMENTS.

RIM ELEVATION	991.20	
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT

NOTES/SPECIAL REQUIREMENTS:
UNIT IS SIZED AS JFPD0811-15-4. VAULT SIZE IS

8X16 TO ACCOMMODATE 48" RCP INLET/OUTLET.

* PER ENGINEER OF RECORD

GENERAL NOTES:

- CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS REPRESENTATIVE. www.ContechES.com
- JELLYFISH WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
- 4. STRUCTURE SHALL MEET AASHTO HS-20 OR PER APPROVING JURISDICTION REQUIREMENTS, WHICHEVER IS MORE STRINGENT, ASSUMING EARTH COVER OF 0' 10', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 LOAD RATING AND BE CAST WITH THE CONTECH LOGO.
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- 6. OUTLET PIPE INVERT IS EQUAL TO THE CARTRIDGE DECK ELEVATION.
- THE OUTLET PIPE DIAMETER FOR NEW INSTALLATIONS IS RECOMMENDED TO BE ONE PIPE SIZE LARGER THAN THE INLET PIPE AT EQUAL OR GREATER SLOPE.
- 8. NO PRODUCT SUBSTITUTIONS SHALL BE ACCEPTED UNLESS SUBMITTED 10 DAYS PRIOR TO PROJECT BID DATE, OR AS DIRECTED BY THE ENGINEER OF RECORD.

INSTALLATION NOTES

FOLLOWING: U.S. PATENT NO. 8,287,726; 8,221,618; US 8,123,935; OTHER INTERNATIONAL PATENTS PENDING

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STRUCTURE
- C. CONTRACTOR WILL INSTALL AND LEVEL THE STRUCTURE, SEALING THE JOINTS, LINE ENTRY AND EXIT POINTS (NON-SHRINK GROUT WITH APPROVED WATERSTOP OR FLEXIBLE BOOT).
- D. CARTRIDGE INSTALLATION, BY CONTECH, SHALL OCCUR ONLY AFTER SITE HAS BEEN STABILIZED AND THE JELLYFISH UNIT IS CLEAN AND FREE OF DEBRIS. CONTACT CONTECH TO COORDINATE CARTRIDGE INSTALLATION WITH SITE STABILIZATION.



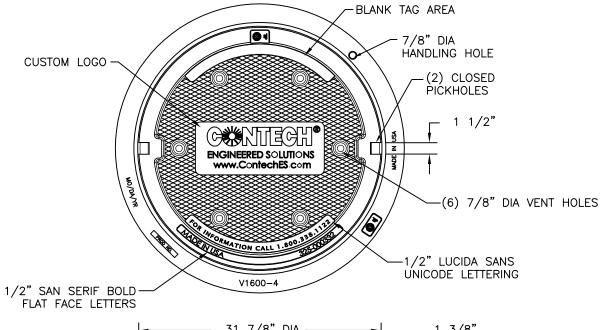
 www.ContechES.com

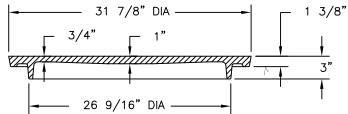
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 800-338-1122
 513-645-7000
 513-645-7993 FAX

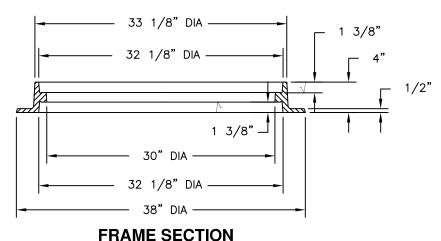
8' x 16' JELLYFISH - 715260 - 010 FOUST TRACK - CHALK HILL VENTURES LIBERTY HILL, TX SITE DESIGNATION: ST-A

1810B4 V1600-4 Assembly





COVER SECTION





Product Number 41600483

Design Features

-Materials Cover Gray Iron (CL35B) Frame Gray Iron (CL35B)

-Design Load

Heavy Duty

-Open Area

n/a

-Coating

Undipped

- √ Designates Machined Surface

Certification

-

-ASTM A48

-Country of Origin: USA

Major Components

00180783 41600410

Drawing Revision

05/09/2007 Designer: SMH 6/26/2017 Revised By: DAE

Disclaimer

Weights (lbs./kg) dimensions (inches/mm) and drawings provided for your guidance. We reserve the right to modify specifications without prior notice.

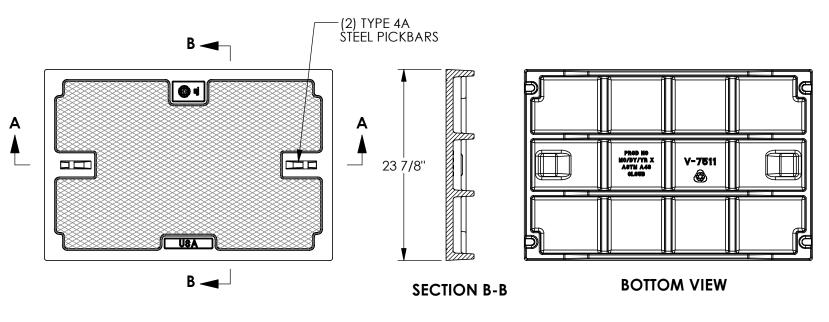
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Contact

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V7511 Trench Cover





Product Number

47511031

Design Features

- -Materials
- Gray Iron (CL35B)
- -Design Load
- Heavy Duty
- -Open Área
- n/a
- -Coating
- Undipped
- √ Designates Machined Surface

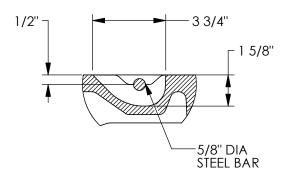
Certification

-ASTM A48

-Country of Origin: USA

36" ₋2 1/2" ^L 3 1/2" 26"

SECTION A-A



DETAIL C SCALE 1:5

Drawing Revision

10/05/2017 Designer: DJH 11/28/2017 Revised By: DJH

Disclaimer

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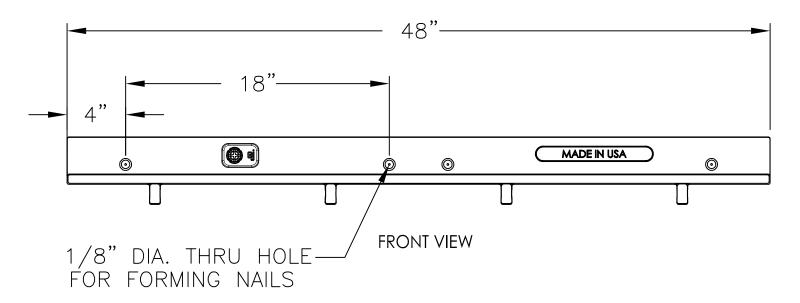
Contact

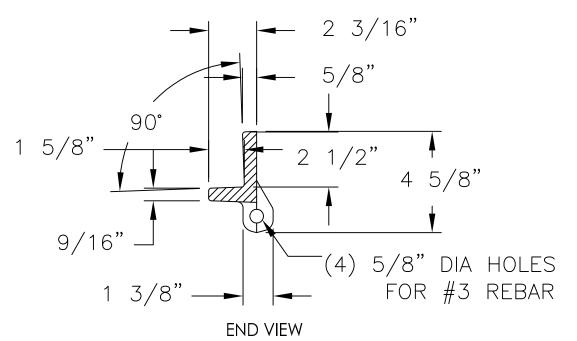
800 626 4653

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V7300-3 Trench Rail







Product Number 47300311

Design Features

-Materials

Gray Iron (CL35B)

-Design Load

Heavy Duty
-Open Area

n/a

-Coating

Undipped

- V Designates Machined Surface

Certification

- ASTM A536

-Country of Origin: USA

EstImated Weight:

- 38 lbs

Drawing Revision

4/16/2005 Designer: SBB 4/18/2018 Revised By: DAE

Disclaimer

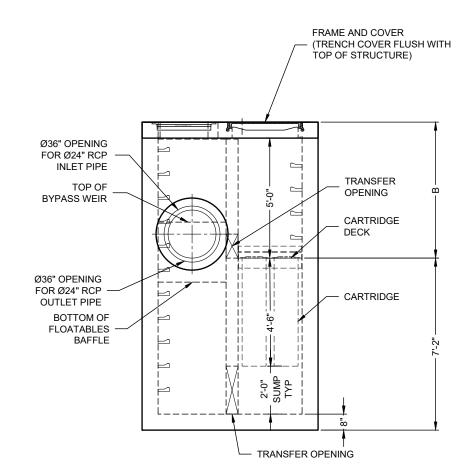
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PLAN VIEW (TOP SLAB NOT SHOWN FOR CLARITY)



ELEV. = 994.90'

TOP OF STRUCTURE ELEV. = 994.90'

WEIR ELEV. =991.16' INLET INV. ELEV. = 989.66'

OUTLET INV. ELEV. = 989.66'

STRUCTURE INV. ELEV. = 983.16'

BOTTOM OF STRUCTURE ELEV. = 982.49'

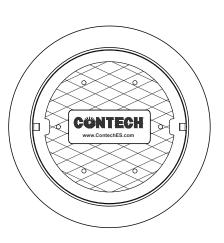
ELEVATION VIEW



JELLYFISH DESIGN NOTES

JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD PEAK DIVERSION STYLE WITH PRECAST TOP SLAB IS SHOWN. ALTERNATE OFFLINE VAULT AND/OR SHALLOW ORIENTATIONS ARE AVAILABLE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD

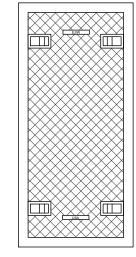
CARTRIDGE LENGTH	54"
OUTLET INVERT TO STRUCTURE INVERT (A)	6'-6"
FLOW RATE HI-FLO / DRAINDOWN (CFS) (PER CART)	0.178 / 0.089
MAX. TREATMENT (CFS)	1.96
DECK TO INSIDE TOP (MIN) (B)	5.00



FRAME AND COVER

(DIAMETER VARIES)

N.T.S.



STRUCTURE	E ID					5
WATER QUA	ALITY FLO	W RATE (cfs)			
PEAK FLOW	RATE (cfs	;)				
RETURN PE	RIOD OF F	PEAK FLO	W (yrs)			_
OF CARTR	IDGES RE	QUIRED ((HF / DD)		_
CARTRIDGE LENGTH						
PIPE DATA:	I.E.	MAT'L	DIA	SLOPE	%	-
NLET #1	989.66	RCP	24	*		_
NLET #2	*	*	*	*		
OUTLET 989.66 RCP 24 *						_

24" **TRENCH COVER** (LENGTH VARIES)

RIM ELEVATION	994.90				
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT			
	*	*			
NOTES/SPECIAL REQUIREMENTS:					
* PER ENGINEER OF RECORD					

ST-B

1.50

8/2

54

HGL

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE
- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS REPRESENTATIVE. www.ContechES.com
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N.T.S.

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INSTALLATION NOTES

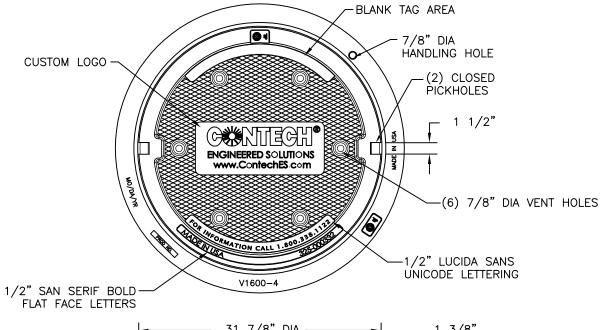
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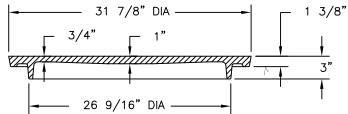


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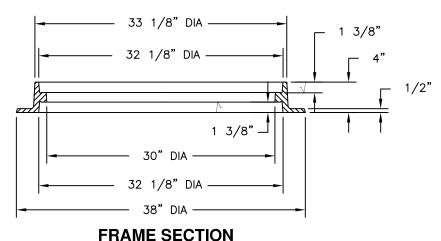
8' x 6' JELLYFISH - 715260- 020 PROJECT NAME: FOUST TRACT - CHALK HILL VENTURES LOCATION: LIBERT HILL, TX SITE DESIGNATION: ST-B

1810B4 V1600-4 Assembly





COVER SECTION





Product Number 41600483

Design Features

-Materials Cover Gray Iron (CL35B) Frame Gray Iron (CL35B)

-Design Load

Heavy Duty

-Open Area

n/a

-Coating

Undipped

- √ Designates Machined Surface

Certification

-

-ASTM A48

-Country of Origin: USA

Major Components

00180783 41600410

Drawing Revision

05/09/2007 Designer: SMH 6/26/2017 Revised By: DAE

Disclaimer

Weights (lbs./kg) dimensions (inches/mm) and drawings provided for your guidance. We reserve the right to modify specifications without prior notice.

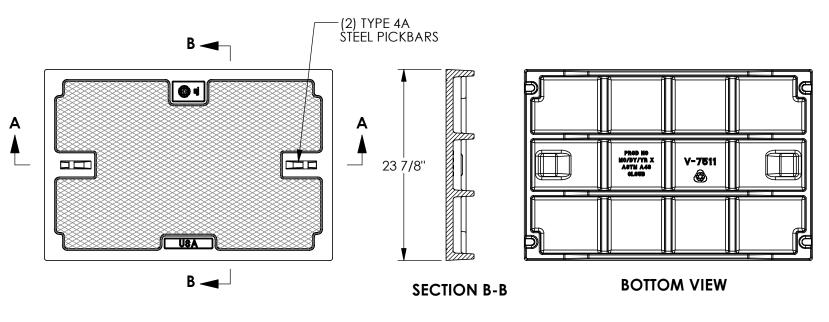
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Contact

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V7511 Trench Cover





Product Number

47511031

Design Features

- -Materials
- Gray Iron (CL35B)
- -Design Load
- Heavy Duty
- -Open Área
- n/a
- -Coating
- Undipped
- √ Designates Machined Surface

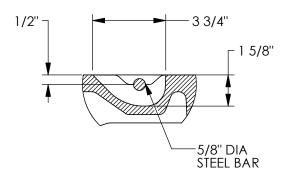
Certification

-ASTM A48

-Country of Origin: USA

36" ₋2 1/2" ^L 3 1/2" 26"

SECTION A-A



DETAIL C SCALE 1:5

Drawing Revision

10/05/2017 Designer: DJH 11/28/2017 Revised By: DJH

Disclaimer

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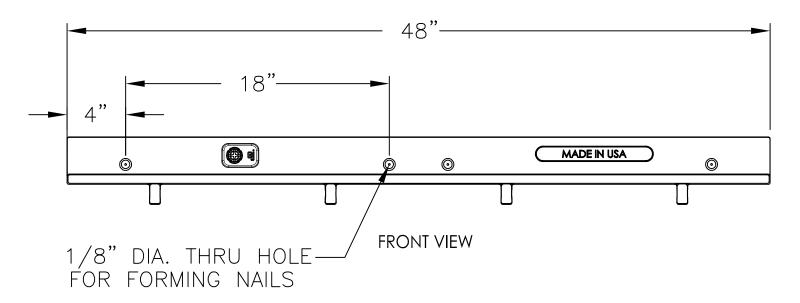
Contact

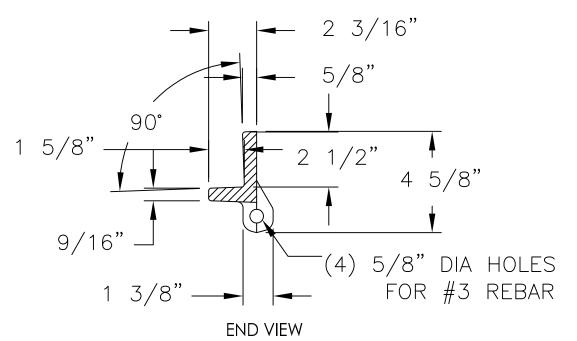
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V7300-3 Trench Rail







Product Number 47300311

Design Features

-Materials

Gray Iron (CL35B)

-Design Load

Heavy Duty
-Open Area

n/a

-Coating

Undipped

- V Designates Machined Surface

Certification

- ASTM A536

-Country of Origin: USA

EstImated Weight:

- 38 lbs

Drawing Revision

4/16/2005 Designer: SBB 4/18/2018 Revised By: DAE

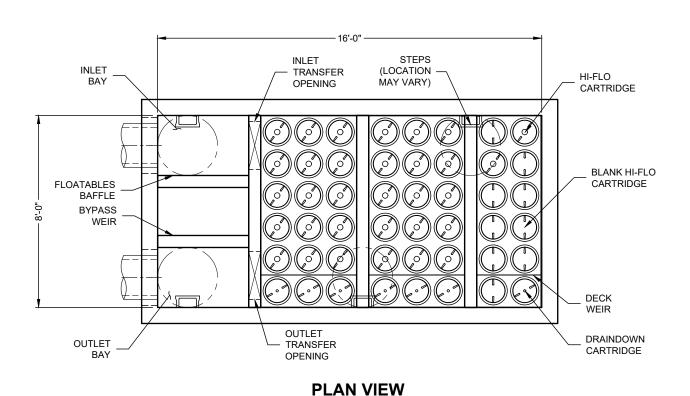
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Contact

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(TOP SLAB NOT SHOWN FOR CLARITY)

CONTRACTOR TO GROUT TO FINISHED GRADE FRAME AND COVERS CONTECH TO PROVIDE (TRENCH COVER FLUSH WITH TOP OF GRADE RING/RISER STRUCTURE) ELEV. = 995.56' TOP OF STRUCTURE TOP OF ELEV. = 995.07' BYPASS WEIR STEP TYP Ø28" OPENING FOR Ø18" RCP **INLET PIPE** WEIR ELEV. = 990.91' TRANSFER **OPENING** INLET INV. ELEV. = 989.41' OUTLET INV. ELEV. = 989.41' Ø28" OPENING FOR Ø18" RCP CARTRIDGE **OUTLET PIPE** DECK **BOTTOM OF** FLOATABLES **BAFFLE** CARTRIDGE STRUCTURE INV. ELEV. = 982.91' **BOTTOM OF STRUCTURE**

TRANSFER OPENING

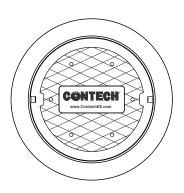
ELEVATION VIEW

JELLYFISH DESIGN NOTES

JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD PEAK DIVERSION STYLE WITH PRECAST TOP SLAB IS SHOWN. ALTERNATE OFFLINE VAULT AND/OR SHALLOW ORIENTATIONS ARE AVAILABLE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD

CARTRIDGE SELECTION

CARTRIDGE LENGTH	54"
OUTLET INVERT TO STRUCTURE INVERT (A)	6'-6"
FLOW RATE HI-FLO / DRAINDOWN (CFS) (PER CART)	0.178 / 0.089
MAX. TREATMENT (CFS)	7.84
DECK TO INSIDE TOP (MIN) (B)	5.00



FRAME AND COVER (DIAMETER VARIES) N.T.S.

DATA REQUIREMENTS		

STRUCTURE ID	ST-C
WATER QUALITY FLOW RATE (cfs)	6.32
PEAK FLOW RATE (cfs)	*
RETURN PERIOD OF PEAK FLOW (yrs)	*
# OF CARTRIDGES REQUIRED (HF / DD)	32/7
CARTRIDGE LENGTH	54

PIPE DATA:	I.E.	MAT'L	DIA	SLOPE %	HGL
INLET #1	989.41	RCP	18	*	*
INLET #2	*	*	*	*	*
OUTLET	989.41	RCP	18	*	*

SEE GENERAL NOTES 6-7 FOR INLET AND OUTLET HYDRAULIC AND SIZING REQUIREMENTS.

RIM ELEVATION		995.56
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT
	*	*

NOTES/SPECIAL REQUIREMENTS: UNIT IS PLACED OFFILINE AND PAIRED WITH CONTECH STORMGATE DUE TO (2) 5'x3' RCB MAINLINE. * PER ENGINEER OF RECORD

GENERAL NOTES:

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS REPRESENTATIVE. www.ContechES.com
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INSTALLATION NOTES

ELEV. = 982.24'

FOLLOWING: U.S. PATENT NO. 8,287,726; 8,221,618; US 8,123,935; OTHER INTERNATIONAL PATENTS PENDING

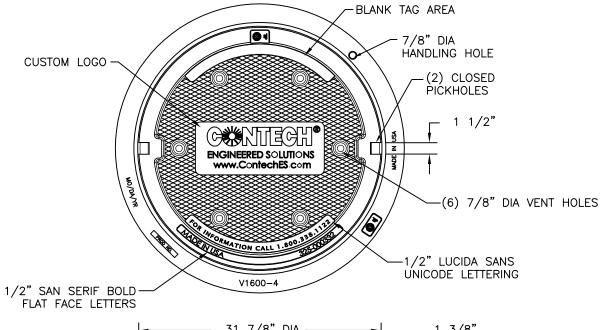
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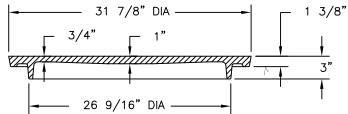


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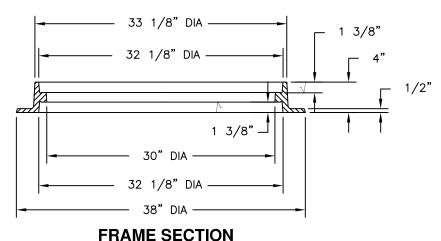
8' x 16' JELLYFISH - 715260 - 030 FOUST TRACT - CHALK HILL VENTURES LIBERTY HILL, TX SITE DESIGNATION: ST-C

1810B4 V1600-4 Assembly





COVER SECTION





Product Number 41600483

Design Features

-Materials Cover Gray Iron (CL35B) Frame Gray Iron (CL35B)

-Design Load

Heavy Duty

-Open Area

n/a

-Coating

Undipped

- √ Designates Machined Surface

Certification

-

-ASTM A48

-Country of Origin: USA

Major Components

00180783 41600410

Drawing Revision

05/09/2007 Designer: SMH 6/26/2017 Revised By: DAE

Disclaimer

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Contact

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ATTACHMENT M CONSTRUCTION PLANS



OWNER/DEVELOPER CHV LIBERTY HILL 29 PROPERTY OWNER LLC 6601-E HILLCREST AVE, STE 212

DALLAS, TEXAS 75205 PHONE: (214) 435 - 7510 **CONTACT: LANGFORD STUBER** langford@chalkhillventures.com

ENGINEER: KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL, SUITE C3 CEDAR PARK, TX 78613 PHONE: (512) 428-8586 CONTACT: SHEA KIRKMAN, P.E. shea.kirkman@trustke.com

SURVEYOR: **BARTON CHAPA SURVEYING** 5200 STATE HIGHWAY 121 COLLEYVILLE, TX 76034 PHONE: (817) 864-1957 CONTACT: JACK BARTON, RPLS jack@bcsdfw.com

URTIS STEGER,	P.E., CITY	ENGINEER:	

BASED ON THE DESIGN ENGINEER'S CERTIFICATION OF COMPLIANCE WITH ALL APPLICABLE CITY, STATE AND FEDERAL REGULATIONS, THE PLANS AND SPECIFICATIONS CONTAINED HEREIN HAVE BEEN REVIEWED AND ARE FOUND TO BE IN COMPLIANCE WITH THE REQUIREMENTS OF THE CITY OF LIBERTY HILL

DATE

DATE

DATE

DATE

DATE

IEDDY I MILLADD ID DIDECTOD OF DLANKING
JERRY L.MILLARD, JR., DIRECTOR OF PLANNING
, ,

IZ BRANIGAN, MAYO	R

ELAINE SIMPSON, CITY SECRETARY

REVIEWED FOR COMPLIANCE WITH COUNTY REQUIREMENTS

WILLIAMSON COUNTY

NOTES:

- CERTIFICATE OF COMPLIANCE NUMBER 2023-49-COC
- TXDOT DRIVEWAY PERMIT NUMBER 21-42318
- 3. WILLIAMSON COUNTY DRIVEWAY PERMIT NUMBERS
- 4. THE CONTRACTOR SHALL OBTAIN A "NOTICE OF PROPOSED INSTALLATION OF UTILITY LINE" PERMIT FROM WILLIAMSON COUNTY FOR ANY WORK PERFORMED IN THE EXISTING COUNTY RIGHT-OF-WAY (DRIVEWAY APRON, WATER MAIN TIE-IN, ETC.) THIS PERMIT APPLICATION WILL REQUIRE A LIABILITY AGREEMENT, A CONSTRUCTION COST ESTIMATE FOR WORK WITHIN THE RIGHT-OF-WAY INCLUDING PAVEMENT REPAIR (IF NEEDED), A PERFORMANCE BOND, CONSTRUCTION PLANS AND, IF NECESSARY, A TRAFFIC CONTROL PLAN. AN INSPECTION FEE, AND A PRE-CONSTRUCTION MEETING MAY ALSO BE REQUIRED, DEPENDING ON THE SCOPE OF WORK. THE PERMIT WILL BE REVIEWED AND APPROVED BY THE COUNTY ENGINEER, AND MUST ALSO BE APPROVED BY THE WILLIAMSON COUNTY COMMISSIONERS COURT IF ANY ROAD CLOSURE IS INVOLVED."

SITE DEVELOPMENT PLANS FOR THE CONSTRUCTION OF

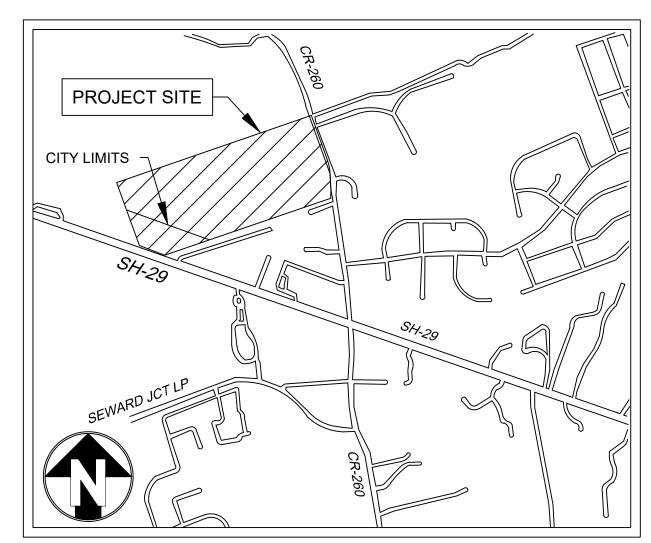
WATER, WASTEWATER, PAVING, GRADING & DRAINAGE IMPROVEMENTS

TO SERVE

CHALK HILL RANCH

LOTS 1 & 2, BLOCK A LOTS 1 & 2, BLOCK B

THE CITY OF LIBERTY HILL AND LIBERTY HILL ETJ, WILLIAMSON COUNTY, TEXAS



VICINITY MAP HORIZONTAL SCALE: 1" = 1000'



TEXAS FIRM NO. 15874

MAY 2023



	0010		
C1.0	COVER SHEET	C6.13	STORM PROFILE I
C1.1	GENERAL CONSTRUCTION NOTES	C6.14	STORM PROFILE II
C1.2	CITY OF LIBERTY HILL GENERAL CONSTRUCTION NOTES	C6.15	STORM PROFILE III
ЛР1.0	MINOR PLAT BLOCK A	C6.16	STORM PROFILE IV
лг 1.0 ЛР1.1	MINOR PLAT BLOCK A	C6.17	HYDRAULIC CALCULATIONS
лР 1.1 ЛР2.0	MINOR PLAT BLOCK B	C6.18	PROPOSED DRAINAGE AREA CALCULATIONS
ЛР2.1	MINOR PLAT BLOCK B	C6.19	COMPOSITE C CALCULATIONS
лР2.2	MINOR PLAT BLOCK B	C6.20	WATER QUALITY CALCULATIONS I
C2.0	DEMOLITION PLAN I	C6.21	WATER QUALITY CALCULATIONS II
C2.1	DEMOLITION PLAN II	C6.22	HEC-HMS EX. DRAINAGE AREA MAP
C3.0	OVERALL SITE PLAN	C6.23	HEC-HMS PROP. DRAINAGE AREA MAP
C3.1	SITE PLAN I	C6.24	HEC-HMS CALCULATIONS
C3.2	SITE PLAN II	C7.0	UTILITY PLAN OVERALL
C3.3	SITE PLAN III		UTILITY PLAN OVERALL UTILITY PLAN I
		C7.1	
C3.4	SITE PLAN IV	C7.2	UTILITY PLAN II
C3.5	SITE PLAN V	C7.3	UTILITY PLAN III
C3.6	SITE PLAN VI	C7.4	UTILITY PLAN IV
C3.7	SITE PLAN VII	C7.5	UTILITY PLAN V
C3.8	SITE PLAN VIII	C7.6	UTILITY PLAN VI
C3.9	SITE PLAN IX	C7.7	UTILITY PLAN VII
C4.0	DIMENSIONAL CONTROL PLAN I	C7.8	UTILITY PLAN VIII
C4.1	DIMENSIONAL CONTROL PLAN II	C7.9	UTILITY PLAN IX
C4.2	DIMENSIONAL CONTROL PLAN III	C7.10	UTILITY PROFILE I
C4.3	DIMENSIONAL CONTROL PLAN IV	C7.11	UTILITY PROFILE II
C4.4	DIMENSIONAL CONTROL PLAN V	C7.12	UTILITY PROFILE III
C4.5	DIMENSIONAL CONTROL PLAN VI	C7.13	UTILITY PROFILE IV
C4.6	DIMENSIONAL CONTROL PLAN VII	C7.14	UTILITY PROFILE V
C4.7	DIMENSIONAL CONTROL PLAN VIII	C8.0	PAVING PLAN
C4.8	DIMENSIONAL CONTROL PLAN IX	C9.0	EROSION CONTROL PLAN
C5.0	OVERALL GRADING PLAN	C10.0	TRAFFIC CONTROL PLAN
C5.1	GRADING PLAN I	C11.0	SITE DETAILS
C5.2	GRADING PLAN II	C12.0	DRAINAGE DETAILS I
C5.3	GRADING PLAN III	C12.1	DRAINAGE DETAILS II
C5.4	GRADING PLAN IV	C12.2	WATER QUALITY DETAILS I
C5.5	GRADING PLAN V	C12.3	WATER QUALITY DETAILS II
C5.6	GRADING PLAN VI	C12.4	WATER QUALITY DETAILS III
C5.7	GRADING PLAN VII	C12.5	WATER QUALITY DETAILS IV
C5.8	GRADING PLAN VIII	C12.6	WATER QUALITY DETAILS V
C5.9	GRADING PLAN IX	C13.0	WATER DETAILS I
C5.10	DETENTION POND GRADING PLAN	C13.1	WATER DETAILS II
C5.11	DETAILED GRADING PLAN	C13.2	WATER DETAILS III
C6.0	EXISTING DRAINAGE AREA MAP	C13.3	WATER DETAILS IV
C6.1	INLET DRAINAGE AREA MAP	C13.4	WATER DETAILS V
C6.2	OVERALL STORM PLAN	C13.5	WATER DETAILS VI
26.3	STORM PLAN I	C13.6	WATER DETAILS VII
C6.4	STORM PLAN II	C13.7	WATER NOTES I
C6.5	STORM PLAN III	C13.8	WATER NOTES II
26.6	STORM PLAN IV	C13.9	WATER NOTES III
26.7	STORM PLAN V	C13.10	WATER NOTES IV
C6.8	STORM PLAN VI	C14.0	WASTEWATER DETAILS
C6.9	STORM PLAN VII	C15.0	PAVING DETAILS I
26.10	STORM PLAN VIII	C15.1	PAVING DETAILS II
26.10 26.11	STORM PLAN IX	C 16.0	EROSION CONTROL DETAILS
C6.12	NW DETENTION POND PLAN	C 17.0	WCESD NO.4 DETAILS
, J. 12	W PETERITORY OND LEAR	· · ·	

Sheet List Table

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UTILITY PROVIDER INDEX				
UTILITY	PROVIDER	CONTACT	PHONE NUMBER	EMAIL ADDRESS
ELECTRIC	PEDERNALES ELECTRIC COOPERATIVE, INC.	CARRIE GARRET	877-372-0391	carrie.garret@peci.com
TELEPHONE/FIBER	AT&T	GREGORY SIFUENTEZ	512-413-2122	gs7821@att.com
WATER	CITY OF GEORGETOWN	DAVID MUNK	512-930-3640	david.munk@georgetown.org
WASTEWATER	CITY OF LIBERTY HILL	CURTIS STEGER, PE	512-930-9412	curtis.steger@stegerbizzell.com

PROJECT NO. CHV21004

ACTIVITIES ON THE PROJECT.

- STANDARDS AND SPECIFICATIONS: ALL MATERIALS, CONSTRUCTION METHODS, WORKMANSHIP, EQUIPMENT, SERVICES AND TESTING FOR ALL PUBLIC IMPROVEMENTS SHALL BE IN ACCORDANCE WITH THE GOVERNING AUTHORITIES' ORDINANCES, REGULATIONS, REQUIREMENTS, STATUTES, SPECIFICATIONS AND DETAILS, LATEST PRINTING AND AMENDMENTS THERETO. THE GOVERNING AUTHORITIES' PUBLIC WORKS AND WATER DEPARTMENT REQUIREMENTS, PLUMBING CODES, AND FIRE DEPARTMENT REGULATIONS SHALL TAKE PRECEDENT FOR ALL PRIVATE IMPROVEMENTS WHERE APPLICABLE. ALL OTHER PRIVATE CONSTRUCTION, NOT REGULATED
- COUNCIL OF GOVERNMENTS, LATEST PRINTING AND AMENDMENTS THERETO, EXCEPT AS MODIFIED BY THE PROJECT CONTRACT DOCUMENTS. EXAMINATION OF PLANS: PRIOR TO COMMENCING ANY CONSTRUCTION, THE CONTRACTOR SHALL FAMILIARIZE THEMSELVES WITH THE CONTRACTOR DOCUMENTS CONSTRUCTION DOCUMENTS, AND SPECIFICATIONS. FAILURE ON THE PART OF THE CONTRACTOR TO FAMILIARIZE THEIR SELF WITH ALL STANDARDS AND SPECIFICATIONS PERTAINING TO THE WORK SHALL IN NO WAY RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR PERFORMING THE WORK IN ACCORDANCE WITH

BY THE GOVERNING AUTHORITY. SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION. NORTH CENTRAL TEXAS

ALL SUCH APPLICABLE STANDARDS AND SPECIFICATIONS. EXAMINATION OF SITE: THE CONTRACTOR SHALL BE RESPONSIBLE FOR INVESTIGATING AND SATISFYING THEIR SELF AS TO THE CONDITIONS AFFECTING THE WORK INCLUDING BUT NOT RESTRICTED TO THE BEARING UPON TRANSPORTATION, DISPOSAL, HANDLING AND STORAGE OF MATERIALS, AVAILABILITY OF LABOR, WATER, ELECTRIC POWER, ROADS AND UNCERTAINTIES OF WEATHER, OR SIMILAR PHYSICAL CONDITIONS AT THE SITE, CONDITIONS OF THE GROUND, THE CHARACTER OF EQUIPMENT AND FACILITIES NEEDED PRELIMINARY TO AND DURING THE PERFORMANCE OF THE WORK.

FAILURE BY THE CONTRACTOR TO ACQUAINT HIMSELF WITH THE AVAILABLE INFORMATION WILL NOT RELIEVE HIM FROM RESPONSIBILITY FOR ESTIMATING THE DIFFICULTY OR COST OF SUCCESSFULLY PERFORMING THE WORK.

- ADEQUATE AREA TO PERFORM WORK: CONTRACTOR TO VERIFY ADEQUATE AREA EXISTS ON-SITE TO PERFORM THE WORK SHOWN IN THESE CONSTRUCTION DOCUMENTS. IF ADDITIONAL AREA IS REQUIRED TO PERFORM THE WORK. THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER PRIOR TO CONSTRUCTION SUBSURFACE INVESTIGATION: SUBSURFACE EXPLORATION TO ASCERTAIN THE NATURE OF SOILS HAS BEEN PERFORMED BY THE GEOTECHNICAL ENGINEER OF
- RECORD ON THE PROJECT. THE SUBSURFACE INFORMATION WILL BE MADE AVAILABLE FOR THE CONTRACTOR'S USE. THE ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR THE ACCURACY, TRUE LOCATION AND EXTENT OF THE SOILS INFORMATION PREPARED BY OTHERS. TOPOGRAPHY SURVEY: TOPOGRAPHIC SURVEY INFORMATION SHOWN ON THE PLANS IS PROVIDED FOR INFORMATIONAL PURPOSES. THE CONTRACTOR SHALL BE
- RESPONSIBLE FOR VERIFYING THAT THE INFORMATION SHOWN IS CORRECT, AND SHALL NOTIFY THE ENGINEER IMMEDIATELY OF ANY ERRORS, DISCREPANCIES OR OMISSIONS TO THE SURVEY INFORMATION PROVIDED. COMPLIANCE WITH LAWS: THE CONTRACTOR SHALL FULLY COMPLY WITH ALL LOCAL, STATE AND FEDERAL LAWS, INCLUDING ALL CODES, ORDINANCES AND REGULATIONS APPLICABLE TO THIS CONTRACT AND THE WORK TO BE DONE THEREUNDER, WHICH EXIST OR MAY BE ENACTED LATER BY GOVERNMENTAL BODIES
- HAVING JURISDICTION OR AUTHORITY FOR SUCH ENACTMENT. ALL WORK REQUIRED UNDER THIS CONTRACT SHALL COMPLY WITH ALL REQUIREMENTS OF LAW, REGULATION, PERMIT OR LICENSE. IF THE CONTRACTOR FINDS THAT THERE IS A VARIANCE, HE SHALL IMMEDIATELY REPORT THIS TO THE OWNER FOR RESOLUTION PUBLIC CONVENIENCE AND SAFETY: IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONNEL AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. MATERIALS STORED ON THE WORK SITE SHALL BE PLACED. AND THE WORK SHALL AT ALL TIMES BE SO CONDUCTED, AS TO CAUSE NO GREATER OBSTRUCTION TO THE TRAVELING PUBLIC THAN IS CONSIDERED ACCEPTABLE BY THE
- GOVERNING AUTHORITIES AND THE DEVELOPER AND NOT TO PREVENT FREE UNINTERRUPTED ACCESS TO ALL FIRE HYDRANTS, WATER VALVES, GAS VALVES, MANHOLES AND FIRE ALARM OR POLICE CALL BOXES IN THE VICINITY. STORM WATER POLLUTION PREVENTION PLAN (SWPPP): THE CONTRACTOR SHALL COMPLY WITH THE CONDITIONS OF THE SWPPP WHILE CONDUCTING THEIR
- PERMITS AND LICENSES: THE CONTRACTOR SHALL SECURE AND PAY FOR ALL PERMITS AND LICENSES NECESSARY FOR THE EXECUTION OF THE WORK AND SHALL FULLY COMPLY WITH ALL THEIR TERMS AND CONDITIONS. WHENEVER THE WORK UNDER THIS CONTRACT REQUIRES THE OBTAINING OF PERMITS FROM THE GOVERNING AUTHORITIES, THE CONTRACTOR SHALL FURNISH DUPLICATE COPIES OF SUCH PERMITS TO THE DEVELOPER BEFORE THE WORK COVERED THEREBY IS STARTED. NO WORK WILL BE ALLOWED TO PROCEED BEFORE SUCH PERMITS HAVE BEEN OBTAINED. COSTS ASSOCIATED WITH PERMITS SHALL BE INCLUDED IN THE
- APPROVED PLANS: THE CONTRACTOR SHALL HAVE AT LEAST ONE SET OF APPROVED PLANS ON-SITE AT ALL TIMES. WORK PERFORMED WITHOUT THE USE OF APPROVED PLAN SETS IS NOT AUTHORIZED AND SHALL BE AT THE RISK OF THE CONTRACTOR.
- BONDS: PERFORMANCE, PAYMENT AND MAINTENANCE BONDS MAY BE REQUIRED FROM THE CONTRACTOR FOR "PUBLIC" IMPROVEMENTS. IF REQUIRED, THE CONTRACTOR SHALL PROVIDE THE BONDS IN THE FORM AND IN THE AMOUNTS AS REQUIRED BY THE GOVERNING AUTHORITIES. COSTS ASSOCIATED WITH PROVIDING THE BONDS SHALL BE INCLUDED IN THE CONTRACT AMOUNT.
- TESTING: THE TESTING AND CONTROL OF ALL MATERIALS USED IN THE WORK SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY, EMPLOYED AND PAID DIRECTLY BY THE CONTRACTOR. IN THE EVENT THE RESULTS OF THE INITIAL TESTING DO NOT COMPLY WITH THE PLANS AND SPECIFICATIONS, SUBSEQUENT TESTS NECESSARY TO DETERMINE THE ACCEPTABILITY OF MATERIALS OR CONSTRUCTION SHALL BE AT THE CONTRACTOR'S EXPENSE
- INSPECTION: THE GOVERNING AUTHORITIES AND/OR THE DEVELOPER WILL PROVIDE INSPECTION OF THE PROPOSED CONSTRUCTION. THE OWNER WILL PAY THE COSTS FOR INSPECTION SERVICES. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE WELL IN ADVANCE OF PENDING CONSTRUCTION ACTIVITIES TO THE GOVERNING AUTHORITIES AND/OR OWNER FOR SCHEDULING OF INSPECTION SERVICES. SHOP DRAWINGS: THE CONTRACTOR SHALL PREPARE, REVIEW, AND SUBMIT ALL SHOP DRAWING, PRODUCT DATA AND SAMPLES REQUIRED BY THE GOVERNING
- AUTHORITIES AND THE PROJECT CONTRACT DOCUMENTS IN ACCORDANCE WITH ITEM 1.28 OF THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, NORTH CENTRAL TEXAS - NORTH CENTRAL TEXAS COUNCIL OF GOVERNMENTS. SURVEYING: ALL SURVEYING REQUIRED FOR CONSTRUCTION STAKING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE OWNER SHALL PROVIDE TWO BENCHMARKS FOR USE AS HORIZONTAL AND VERTICAL DATUM. THE CONTRACTOR SHALL EMPLOY A REGISTERED PROFESSIONAL LAND SURVEY TO PERFORM ALL
- ADDITIONAL SURVEY, LAYOUT AND MEASUREMENT WORK NECESSARY FOR THE COMPLETION OF THE PROJECT. THE COSTS ASSOCIATED WITH THE CONSTRUCTION STAKING SHALL BE INCLUDED IN THE CONTRACT AMOUNT. PROTECTION OF PROPERTY CORNERS AND BENCHMARKS: THE CONTRACTOR SHALL PROTECT ALL PROPERTY CORNER MARKERS AND BENCHMARKS. WHEN ANY SUCH
- MARKERS OR MONUMENTS ARE IN DANGER OF BEING DISTURBED, THEY SHALL BE PROPERLY REFERENCED AND IF DISTURBED SHALL BE RESET BY A REGISTERED PUBLIC SURVEYOR AT THE EXPENSE OF THE CONTRACTOR. EXISTING STRUCTURES: THE PLANS SHOW THE LOCATION OF ALL KNOWN SURFACE AND SUB SURFACE STRUCTURES, HOWEVER, THE DEVELOPER AND ENGINEER
- ASSUME NO RESPONSIBILITY FOR THE FAILURE TO SHOW ANY OR ALL OF THESE STRUCTURES ON THE PLANS, OR TO SHOW THEM IN THEIR EXACT LOCATION. SUCH FAILURE SHALL NOT BE CONSIDERED SUFFICIENT BASIS FOR CLAIMS FOR ADDITIONAL COMPENSATION FOR EXTRA WORK OR FOR INCREASING THE PAY QUANTITIES IN ANY MANNER WHATSOEVER, UNLESS THE OBSTRUCTION ENCOUNTERED IN SUCH AS TO REQUIRE CHANGES IN THE LINES OR GRADES, OR REQUIRE THE CONSTRUCTION OF SPECIAL WORK, FOR WHICH PROVISIONS ARE NOT MADE IN THE PLANS.
- PROTECTION OF EXISTING UTILITIES: AS REQUIRED BY "THE TEXAS UNDERGROUND FACILITY DAMAGE PREVENTION AND SAFETY ACT". TEXAS ONE CALL SYSTEM MUST BE CONTACTED (800-245-4545) AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION OPERATIONS BEING PERFORMED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT TEXAS ONE CALL SYSTEM. THE LOCATION OF EXISTING UTILITIES SHOWN ON THE PLANS ARE BASED ON THE BEST RECORDS AND/OR FIELD INFORMATION AVAILABLE AND ARE NOT GUARANTEED BY THE DEVELOPER OR ENGINEER TO BE ACCURATE AS TO THE LOCATION AND DEPTH. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO VERIFY LOCATIONS OF ADJACENT AND/OR CONFLICTING UTILITIES SUFFICIENTLY IN ADVANCE OF HIS ACTIVITIES IN ORDER THAT HE MAY NEGOTIATE SUCH LOCAL ADJUSTMENTS AS NECESSARY IN THE CONSTRUCTION PROCESS TO PROVIDE ADEQUATE CLEARANCES. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS IN ORDER TO PROTECT ALL EXISTING UTILITIES, SERVICES, AND STRUCTURES ENCOUNTERED WHETHER OR NOT THEY ARE ON THE PLANS ANY DAMAGE TO UTILITIES RESULTING FROM THE CONTRACTOR'S OPERATIONS SHALL BE RESTORED AT HIS EXPENSE. TO AVOID UNNECESSARY INTERFERENCE'S OR DELAYS, THE CONTRACTOR SHALL COORDINATE ALL UTILITY REMOVALS, REPLACEMENTS AND CONSTRUCTION WITH THE APPROPRIATE GOVERNING AUTHORITIES. THE DEVELOPER WILL NOT BE LIABLE FOR DAMAGES DUE TO DELAY BECAUSE OF THE ABOVE.
- DAMAGE TO EXISTING FACILITIES: ALL DAMAGE DURING CONSTRUCTION ACTIVITIES SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE TO A CONDITION AS GOOD
- AS OR BETTER THAN THE CONDITIONS PRIOR TO STARTING THE WORK. FIRE AND LIFE SAFETY SYSTEMS: THE CONTRACTOR SHALL NOT REMOVE, DISABLE OR DISRUPT EXISTING FIRE OR LIFE SAFETY SYSTEMS WITHOUT RECEIVING PRIOR
- WRITTEN PERMISSION FROM THE GOVERNING AUTHORITY. TRENCH SAFETY: THE CONTRACTOR IS RESPONSIBLE FOR HAVING A TRENCH SAFETY PLAN PREPARED IN ACCORDANCE WITH OSHA REQUIREMENTS BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF TEXAS FOR THE IMPLEMENTATION OF TRENCH SAFETY CONTROL MEASURES THAT WILL BE IN EFFECT DURING
- THE CONSTRUCTION OF THE PROJECT. THE COSTS FOR PREPARATION OF THE TRENCH SAFETY PLAN SHALL BE INCLUDED IN THE CONTRACT AMOUNT. TRAFFIC CONTROL: IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO IMPLEMENT TRAFFIC CONTROL. THE COSTS ASSOCIATED WITH THE IMPLEMENTATION THE TRAFFIC CONTROL PLAN SHALL BE INCLUDED IN THE CONTRACT AMOUNT.
- 24. ACCESS TO ADJACENT PROPERTIES: ACCESS TO ADJACENT PROPERTIES SHALL BE MAINTAINED AT ALL TIMES UNLESS OTHERWISE DIRECTED BY THE GOVERNING AUTHORITIES AND/OR OWNER.
- ACCESS ROUTES, STAGING AREAS AND STORAGE AREAS: ALL PRIVATE HAUL ROADS AND ACCESS ROUTES AND THE LOCATION OF ALL STAGING AREAS AND STORAGE AREAS SHALL BE SUBJECT TO THE APPROVAL OF THE OWNER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING AND REPAIRING ALL ROADS AND OTHER FACILITIES USED DURING CONSTRUCTION. UPON COMPLETION OF THE PROJECT, ALL HAUL ROADS, ACCESS ROADS, STAGING AREAS AND STORAGE AREAS SHALL BE RESTORED TO A CONDITION EQUAL TO OR BETTER THAN THAT AT THE TIME THE CONTRACTOR COMMENCES WORK ON THE PROJECT.
- PARKING OF CONSTRUCTION EQUIPMENT: AT NIGHT AND DURING ALL PERIODS OF TIME WHEN EQUIPMENT IS NOT BEING ACTIVELY USED FOR THE CONSTRUCTION WORK, THE CONTRACTOR SHALL PARK THE EQUIPMENT AT LOCATIONS WHICH ARE APPROVED BY THE OWNER. DURING THE CONSTRUCTION OF THE PROJECT, THE CONTRACTOR SHALL COMPLY WITH THE PRESENT ZONING REQUIREMENTS OF THE GOVERNING AUTHORITIES IN THE USE OF VACANT PROPERTY FOR STORAGE PURPOSES. THE CONTRACTOR SHALL ALSO PROVIDE ADEQUATE BARRICADES, MARKERS AND LIGHTS TO PROTECT THE OWNER, THE GOVERNING AUTHORITIES, THE PUBLIC AND THE OTHER WORK, ALL BARRICADES, LIGHTS, AND MARKERS MUST MEET THE REQUIREMENTS OF THE GOVERNING AUTHORITIES' REGULATIONS.
- WATER FOR CONSTRUCTION: THE CONTRACTOR SHALL MAKE THE NECESSARY ARRANGEMENTS FOR PURCHASING WATER FROM THE GOVERNING AUTHORITY FOR HIS LISE ON THE PROJECT SITE. COST ASSOCIATED WITH THIS SERVICE SHALL BE INCLUDED IN THE CONTRACT AMOUNT. TEMPORARY ELECTRIC AND COMMUNICATIONS FOR CONSTRUCTION: THE CONTRACTOR SHALL MAKE THE NECESSARY ARRANGEMENTS FOR THE INSTALLATION AND URCHASING OF TEMPORARY ELECTRIC AND COMMUNICATIONS SERVICES FROM THE GOVERNING AUTHORITIES FOR HIS USE ON THE PROJECT SITE. COSTS
- ASSOCIATED WITH THIS SERVICE SHALL BE INCLUDED IN THE CONTRACT AMOUNT. 29. FENCES: ALL FENCES ENCOUNTERED AND REMOVED DURING CONSTRUCTION, EXCEPT THOSE DESIGNATED TO BE REMOVED OR RELOCATED. SHALL BE RESTORED TO THE ORIGINAL OR BETTER THAN CONDITION UPON COMPLETION OF THE PROJECT. WHERE WIRE FENCING, EITHER WIRE MESH OR BARBED WIRE, IS NOT TO BE CROSSED, THE CONTRACTOR SHALL SET
- CROSS-BRACED POSTS ON EITHER SIDE OF THE CROSSING. TEMPORARY FENCING SHALL BE ERECTED IN PLACE OF THE FENCING REMOVED WHENEVER THE WORK IS NOT IN PROGRESS AND WHEN THE SITE IS VACATED OVERNIGHT AND/OR AT ALL TIMES TO PREVENT PERSONS AND/OR LIVESTOCK FROM ENTERING THE CONSTRUCTION AREA. THE COST OF FENCE REMOVAL, TEMPORARY CLOSURES AND REPLACEMENT SHALL BE INCLUDED IN THE CONTRACT AMOUNT. COORDINATION WITH OTHERS: IN THE EVENT THAT OTHER CONTRACTORS ARE DOING WORK IN THE SAME AREA SIMULTANEOUSLY WITH THE PROJECT, THE
- CONTRACTOR SHALL COORDINATE HIS PROPOSED CONSTRUCTION WITH THAT OF THE OTHER CONTRACTORS.
- CONDITION OF THE SITE DURING CONSTRUCTION: THE CONTRACTOR SHALL KEEP THE SITE OF THE WORK AND ADJACENT PREMISES AS FREE FROM MATERIAL, DEBRIS AND RUBBISH AS IS PRACTICABLE. THE CONTRACTOR SHALL REMOVE MATERIAL, DEBRIS AND RUBBISH FROM ANY PORTION OF THE SITE IF, IN THE OPINION OF THE DEVELOPER, SUCH MATERIAL, DEBRIS AND RUBBISH CONSTITUTES A NUISANCE OR IS OBJECTIONABLE.
- EXISTING ROADWAYS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE CLEANLINESS OF EXISTING PAVED ROADS. COSTS ASSOCIATED WITH MAINTAINING THE CLEANLINESS OF EXISTING ROADS SHALL BE INCLUDED IN THE CONTRACT AMOUNT.
- 33. SITE RECONNAISSANCE: THE CONTRACTOR SHALL VISIT THE SITE PRIOR TO BIDDING TO DETERMINE EXISTING CONDITIONS.
- CONSULTANT COORDINATION: CONTRACTOR SHALL THOROUGHLY CHECK COORDINATION OF CIVIL, LANDSCAPE, MEP, ARCHITECTURAL, AND OTHER PLANS PRIOR TO COMMENCING CONSTRUCTION. OWNER/ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY PRIOR TO COMMENCING WITH CONSTRUCTION. 35. DUST CONTROL: THE CONTRACTOR SHALL TAKE ALL PRECAUTIONS NECESSARY TO CONTROL DUST ON THE PROJECT SITE BY SPRINKLING OF WATER, OR ANY OTHER
- METHODS APPROVED BY THE GOVERNING AUTHORITIES. COSTS ASSOCIATED WITH DUST CONTROL SHALL BE INCLUDED IN THE CONTRACT AMOUNT. 36. CLEAN UP FOR FINAL ACCEPTANCE: THE CONTRACTOR SHALL MAKE A FINAL CLEAN UP OF ALL PARTS OF THE WORK BEFORE ACCEPTANCE BY THE OWNER. THIS CLEAN UP SHALL INCLUDE REMOVAL OF ALL OBJECTIONABLE E MATERIALS AND, IN GENERAL, PREPARING THE SITE OF THE WORK IN AN ORDERLY MANNER OF
- APPEARANCE. REMOVAL OF DEFECTIVE AND UNAUTHORIZED WORK: ALL WORK, WHICH HAS BEEN REJECTED OR CONDEMNED, SHALL BE REPAIRED, OR IF IT CANNOT BE REPAIRED SATISFACTORILY. IT SHALL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE. DEFECTIVE MATERIALS SHALL BE IMMEDIATELY REMOVED FROM THE WORK SITE. WORK DONE BEYOND THE LINE OR NOT IN THE CONFORMITY WITH THE GRADES SHOWN ON THE DRAWINGS OR AS WRITTEN AUTHORITY AND PRIOR AGREEMENT IN WRITING AS TO PRICES, SHALL BE AT THE CONTRACTOR'S RISK, AND WILL BE CONSIDERED UNAUTHORIZED, AND AT THE OPTION OF THE OWNER MAY NOT BE MEASURED AND PAID FOR AND MAY BE ORDERED REMOVED AT THE CONTRACTOR'S EXPENSE. UPON FAILURE OF THE CONTRACTOR TO REPAIR SATISFACTORY OR TO REMOVE AND REPLACE, IF SO DIRECTED, REJECTED, UNAUTHORIZED OR CONDEMNED WORK OR MATERIALS IMMEDIATELY AFTER RECEIVING NOTICE FROM THE OWNER, THE OWNER WILL, AFTER GIVING WRITTEN NOTICE TO THE CONTRACTOR. HAVE THE AUTHORITY TO CAUSE DEFECTIVE WORK TO BE REMEDIED OR REMOVED AND REPLACED, OR TO CAUSE UNAUTHORIZED WORK TO BE REMOVED AND TO DEDUCT THE COST THEREOF ANY MONIES DUE OR TO
- BECOME DUE THE CONTRACTOR DISPOSITION AND DISPOSAL OF EXCESS AND UNSUITABLE MATERIALS: ALL MATERIALS TO BE REMOVED FROM THE SITE INCLUDED BUT NOT LIMITED TO EXCESS MATERIAL AND UNSUITABLE MATERIALS SUCH AS CONCRETE, ASPHALT, LARGE ROCKS, REFUSE, AND OTHER DEBRIS SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE DISPOSED OF OUTSIDE THE LIMITS OF THE PROJECT. CONTRACTOR SHALL ALSO COMPLY WITH ALL APPLICABLE LAWS GOVERNING SPILLAGE OF DEBRIS WHILE TRANSPORTING TO A DISPOSAL SITE. COSTS ASSOCIATED WITH THE DISPOSAL OF EXCESS AND UNSUITABLE MATERIALS SHALL BE INCLUDED IN THE CONTRACT AMOUNT.
- RECORD DRAWINGS: THE CONTRACT SHALL MAINTAIN AN ACCURATE RECORD OF THE INSTALLATION OF ALL MATERIALS AND SYSTEM COVERED BY THE PROJECT CONTRACT DOCUMENTS. THE COMPLETE SET OF "RECORD DRAWINGS" MUST BE DELIVERED TO THE OWNER AND/OR ENGINEER BEFORE REQUESTING FINAL PAYMENT. FRANCHISE UTILITIES: THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE INSTALLATION OF FRANCHISE UTILITIES THAT ARE NECESSARY FOR ON-SITE AND OFF-SITE CONSTRUCTION, AND SERVICE TO THE PROPOSED DEVELOPMENT.
- SCOPE OF WORK: THE SCOPE OF WORK FOR THE CIVIL IMPROVEMENTS SHOWN ON THESE PLANS TERMINATES 5-FEET FROM THE BUILDING. REFERENCE THE BUILDING PLANS (E.G. ARCHITECTURAL/STRUCTURAL/MEP) FOR AREAS WITHIN 5-FEET OF THE BUILDING AND WITHIN THE BUILDING FOOTPRINT.
- 42. SITE DRAINAGE: CONTRACTOR SHALL MAINTAIN ADEQUATE SITE DRAINAGE DURING ALL PHASES OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO, MAINTAINING EXISTING DITCHES OR CULVERTS FREE OF OBSTRUCTIONS AT ALL TIMES.

EROSION CONTROL NOTES:

- LAND DISTURBING ACTIVITIES SHALL NOT COMMENCE UNTIL APPROVAL TO DO SO HAS BEEN RECEIVED BY THE GOVERNING AUTHORITIES, PERMITS ARE OBTAINED, AND ALL EROSION CONTROL MEASURES ARE IN PLACE.
- CONTRACTOR SHALL COMPLY WITH ALL STATE AND LOCAL ORDINANCES THAT APPLY. THE GENERAL CONTRACTOR (AND ALL SUBCONTRACTORS INVOLVED WITH ANY CONSTRUCTION ACTIVITIES RELATED TO EARTHWORK, EROSION CONTROL.
- ETC. OR WHICH UTILIZE POSSIBLE POLLUTANTS AS DEFINED IN THE TPDES GENERAL PERMIT) SHALL REVIEW AND ADHERE TO THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP) FOR THE PROJECT, AS WELL AS ALL THE TCEQ REQUIREMENTS SET FORTH IN THE TPDES GENERAL PERMIT. THIS EROSION CONTROL PLAN IS A SUPPLEMENT TO THE SWPPP PREPARED BY OTHERS. REFER TO THE SWPPP FOR ADDITIONAL REQUIREMENTS.
- ALL WASH WATER SHALL BE DISPOSED OF IN A MANNER THAT PREVENTS CONTACT BETWEEN WASH WATER POLLUTANTS AND STORM RUNOFF DISCHARGED 6. OIL AND GREASE ABSORBING MATERIALS SHALL BE READILY AVAILABLE ON-SITE AND SHALL BE PROMPTLY USED TO CONTAIN AND/OR CLEAN UP ALL FUEL OR
- CHEMICAL SPILLS OR LEAKS. DUST CONTROL SHALL BE ACCOMPLISHED BY WATERING DRY, EXPOSED AREAS ON A REGULAR BASIS. SPRAYING OF PETROLEUM BASED OR TOXIC LIQUIDS FOR THIS IS PROHIBITED.
- 8. DISTURBED AREAS ON THE SITE WHERE CONSTRUCTION ACTIVITY HAS CEASED FOR AT LEAST 14 DAYS SHALL BE TEMPORARILY PLANTED AND/OR SEEDED 9. DISTURBED AREAS ON THE SITE WHERE CONSTRUCTION ACTIVITY HAS PERMANENTLY CEASED SHALL BE PERMANENTLY PLANTED AND/OR SEEDED WITHIN 14
- 10. PLANTING AND/OR SEEDING OF VEGETATED AREAS TO ACCOMPLISH STABILIZATION SHALL BE PERFORMED IN ACCORDANCE WITH THE LANDSCAPING PLAN. AREAS BEYOND THE LIMITS OF THE LANDSCAPING PLAN (OR WHEN A LANDSCAPING PLAN DOES NOT EXIST) SHALL BE HYDROMULCHED WITH HIGHWAY MIX AND WATERED WITH TEMPORARY ABOVE GROUND IRRIGATION UNTIL THE VEGETATION IS ESTABLISHED.
- 11. ALL VEHICLES SHALL BE CLEANED AT THE CONSTRUCTION EXIT POINT(S) BEFORE LEAVING THE SITE. 12. ALL MATERIALS SPILLED, DROPPED, WASHED OR TRACKED ONTO ADJACENT ROADWAYS BY ANY VEHICLES EXITING THE SITE SHALL BE CLEANED OR
- REMOVED IMMEDIATELY 13. THE CONTRACTOR SHALL REMOVE ALL ACCUMULATED SILT IN ANY STORM SEWER INLETS AND PIPES, AND ALONG SILT FENCES, WITHIN 48 HOURS AFTER
- INSPECTION OF DEVICES REVEALS THE PRESENCE OF EXCESS SILTATION. 14. SILT FENCES SHALL BE PLACED AROUND ANY STOCKPILES USED ON THE SITE. STONE OVERFLOW POINTS SHALL BE ADDED AT ALL LOW POINTS ALONG SILT
- 15. ADDITIONAL EROSION CONTROL MEASURES MAY BE IMPLEMENTED BY THE CONTRACTOR AT HIS DISCRETION AT NO ADDITIONAL EXPENSE TO THE OWNER. THE ADDITION OR DELETION OF ANY EROSION CONTROL MEASURE MAY REQUIRE THAT THE SWPPP BE MODIFIED IN ACCORDANCE WITH THE TCEQ'S TPDES
- GENERAL PERMIT GUIDELINES. 16. ALL TEMPORARY EROSION CONTROL DEVICES (SILT FENCE, ETC.) SHALL BE REMOVED AND PROPERLY DISPOSED OF OFF SITE WITHIN THIRTY DAYS AFTER
- STABILIZATION OF ALL DISTURBED SURFACES IS COMPLETE.
- 17. THE CONTRACTOR SHALL ASSUME LIABILITY FOR DAMAGE TO ADJACENT PROPERTIES AND/OR PUBLIC RIGHT OF WAY RESULTING FROM FAILURE TO FULLY IMPLEMENT AND EXECUTE ALL EROSION CONTROL PROCEDURES SHOWN AND NOTED IN THESE PLANS AND IN THE SWPPP 18. THE CONTRACTOR SHALL MODIFY THIS PLAN TO SHOW LOCATIONS OF TEMPORARY WASH DOWN AREA, PORTABLE TOILETS, EQUIPMENT
- MAINTENANCE/REPAIR AREAS, STOCKPILE AREAS, FUEL STORAGE AREAS, ETC. AND POLLUTANT CONTROLS FOR EACH. 19. THE GENERAL CONTRACTOR, AS THE TCEQ DEFINED "OPERATOR," SHALL PERFORM ALL REQUIRED INSPECTIONS OF STORM WATER CONTROLS AND PRACTICES AT FREQUENCIES OUTLINED IN THE TPDES GENERAL PERMIT, AND SHALL FILL OUT APPROPRIATE INSPECTION FORMS (AS PROVIDED IN THE
- 20. IF DIRT OR ROCK IS EXPORTED FROM THIS SITE, OR IF DIRT OR ROCK IS IMPORTED FROM AN OFF SITE BORROW LOCATION, THE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR COMPLIANCE WITH ALL TCEQ STORM WATER REQUIREMENTS FOR THE REMOTE SITE. THE CONTRACTOR SHALL FURNISH THE OWNER WITH A COPY OF THE WRITTEN AGREEMENT WITH THE LANDOWNER OF THE REMOTE SITE INDICATING PERMITTING AND EROSION CONTROL MEASURES WILL BE IMPLEMENTED THEREON
- 21. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REESTABLISHING VEGETATION IN ALL DISTURBED AREAS WHETHER SHOWN IN THIS PLAN SET OR NOT. VEGETATION SHALL BE REESTABLISHED IN ACCORDANCE WITH THE STANDARDS OF THE GOVERNING MUNICIPALITY.
- 22. CONTRACTOR TO MAINTAIN EXISTING DRAINAGE PATTERNS DURING CONSTRUCTION UNTIL SUCH TIME THAT THE PROPOSED DRAINAGE INFRASTRUCTURE SHOWN IN THESE CONSTRUCTION PLANS IS INSTALLED AND OPERATIONAL.

- 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COST OF OF PASSING FIELD DENSITY TESTS ON THE STABILIZED SUBGRADE FOR SITE PAVING EQUAL TO THE RATIO OF 1 PER 5,000 SQUARE FEET OF PAVEMENT (AND ALL FAILING DENSITY TESTS AND REQUIRED MOISTURE DENSITY CURVES). ADDITIONAL FIELD DENSITY TESTS MAY BE REQUIRED FOR FOUNDATIONS. REFER TO STRUCTURAL PLANS AND SPECIFICATIONS FOR SUCH. IN ADDITION, THE CONTRACTOR SHALL PROVIDE THE OWNER TEN (10) PASSING SITE PAVEMENT CORES FOR THE OWNERS USE IN THE OWNER'S TESTING FOR THICKNESS AND COMPRESSIVE STRENGTH. CORE LOCATIONS SHALL BE DESIGNATED BY THE OWNER. CONTRACTOR SHALL PATCH CORE HOLES AND FINISH WITH LIKE AND MATCHING MATERIALS. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL TESTING COSTS SHOULD THE ABOVE TESTS FAIL MINIMUM CRITERIA AS ESTABLISHED BY NCTCOG. ANY NON-CONFORMING PAVING SHALL BE REPLACED OR RESOLVED IN ACCORDANCE WITH NCTCOG SPECIFICATIONS AND THESE
- ALL EARTHWORK AND SUBGRADE PREPARATION SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION AND REPORT FOR THIS PROJECT AND THOSE RECOMMENDATIONS LISTED WITHIN THE REPORT. REFER TO THIS REPORT FOR ALL EARTHWORK AND RELATED ITEMS. REFER TO STRUCTURAL FOR BUILDING PREP. THE REPORT REFERENCES AGENCY/INDUSTRY STANDARDS. IN THE EVENT THAT THERE IS A QUESTION OR DISPUTE BETWEEN GOVERNING SPECIFICATIONS, THE MOST STRINGENT SHALL APPLY SUCH THAT THE OWNER RECEIVES THE MOST ADVANTAGEOUS FINISHED PRODUCT.
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PERFORMING ALL CONSTRUCTION LAYOUTS FROM THE SITE LAYOUT CONTROL POINTS AND FROM THE DIMENSIONS SHOWN. THE CONTRACTOR MUST NOTIFY THE ENGINEER OF ANY DISCREPANCIES IN ADVANCE AND ALLOW FOR THE ENGINEER'S RESPONSE BEFORE PROCEEDING WITH THE WORK.
- ALL PAVING DIMENSIONS ARE TO FACE OF CURB, AND EDGE OF PAVEMENT UNLESS OTHERWISE NOTED IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO SUPPLY THE CITY AND THE ENGINEER WITH A CONCRETE MIX DESIGN AT THE PRE-CONSTRUCTION
- MEETING FOR REVIEW AND APPROVAL. THE COST OF THIS DESIGN SHALL BE INCLUDED IN THE UNIT PRICE OF PAVEMENT MATERIAL. THE CONTRACTOR SHALL PROTECT ANY EXISTING AND/OR PROPOSED UTILITIES, WHICH ARE IN THE PROPOSED SUBGRADE DURING THE SUBGRADE
- STABILIZATION PROCESS. CONTRACTOR SHALL ADJUST ALL UTILITIES (EXISTING AND PROPOSED) TO FINAL GRADE AT CONTRACTORS EXPENSE. ALL UTILITIES AND APPURTENANCES SHALL BE EXTENDED UP TO FINAL GRADE. UTILITY CLEAN-OUTS, VALVES, MANHOLES, ETC. LOCATED WITHIN PAVED AREAS SHALL BE PAVED PER DETAIL. IN NON-PAVED AREAS, SAID APPURTENANCES SHALL HAVE A 4" THICK CONCRETE PAD EXTENDING 12" BEYOND SAID APPURTENANCE (BLOCK OUT) POURED AT
- FINAL GRADE FOR PROTECTION AGAINST DAMAGE FROM MOWING AND MAINTENANCE EQUIPMENT. 8. CONTRACTOR SHALL PLACE IRRIGATION, UTILITY CONDUITS, AND OTHER SLEEVES AS NECESSARY FOR CONSTRUCTION PRIOR TO ANY PAVING CONSTRUCTION, PER THE IRRIGATION AND ARCHITECTURAL/MEP PLANS, OR AS DIRECTED BY THE OWNER'S REPRESENTATIVE WITH THE CURBS SCORED TO
- IDENTIFY THE SLEEVE LOCATIONS. UNLESS OTHERWISE NOTED, SUBGRADE SHALL BE STABILIZED TO 12" BEYOND THE BACK OF CURB OR EDGE OF PAVEMENT PER GEOTECH RECOMMENDATIONS UNLESS STATED OTHERWISE. ALL CONCRETE STRENGTH AND REINFORCING STEEL SHALL BE PER PROJECT GEOTECHNICAL
- RECOMMENDATIONS. 10. FIRE LANES, PARKING, AND ROADWAY STRIPING & MARKINGS SHALL CONFORM TO CITY STANDARDS.
- 11. SIDEWALKS WITHIN LANDSCAPE AREAS SHALL BE MINIMUM 4" THICK. LARGE EXPANSES OF CONCRETE FLATWORK (SUCH AS MAJOR PEDESTRIAN AREAS. PLAZA AREAS BETWEEN BUILDINGS OR OTHER STRUCTURES) SHALL BE TREATED LIKE VEHICULAR CONCRETE PAVEMENT AND RECEIVE SAME SUBGRADE STABILIZATION AS VEHICULAR PAVEMENT (6" DEEP MINIMUM AND IN ACCORDANCE WITH A LIME SERIES TEST) AND ALL JOINTS (CONTRACTION AND EXPANSION JOINTS) SHALL BE SEALED WITH SELF LEVELING POLYURETHANE SEALANT.
- ALL PAVEMENT WITHIN 5' OF PROPOSED BUILDING(S) SHALL ADHERE TO THE STRUCTURAL RECOMMENDATIONS AND OR ARCHITECTURAL REQUIREMENTS REFER TO STRUCTURAL AND ARCHITECTURAL PLANS AND RELATED TECHNICAL SPECIFICATIONS. CIVIL PAVEMENT LIMITS BEGIN 5' OUTSIDE THE BUILDING. IN THE EVENT OF OF A CONFLICT WITH THE STRUCTURAL AND OR ARCHITECTURAL WITHIN THIS AREA, THE STRUCTURAL/ ARCHITECT REQUIREMENTS SHALL
- 13. FOR "CURB INLETS" SUBTRACT 0.5' (6 INCHES) FOR STANDARD THROAT RECESS AT INLETS PER STANDARD DETAILS. SURROUNDING PAVEMENT AND GUTTER
- SHALL BE WARPED TO DRAIN FOR INLETS ON GRADE, FLUMES, AND SAG INLETS. INLETS ON GRADE SHALL BE SET IN PLACE TO MATCH THE CURB GRADE LINE. 14. ALL REINFORCING STEEL AND DOWEL BARS IN PAVEMENT SHALL BE SUPPORTED AND MAINTAINED AT THE CORRECT CLEARANCES BY THE USE OF BAR CHAIRS OR OTHER APPROVED SUPPORT.
- 15. CONNECTION OF THE PROPOSED SIDEWALK TO EXISTING PAVING, SIDEWALK, BUILDING, AND WHEELCHAIR RAMPS SHALL BE CONSIDERED SUBSIDIARY TO THE COST OF THE CONSTRUCTION OF THE SIDEWALK. ALL JOINTS (EXPANSION, ISOLATION, CONTRACTION, & CONSTRUCTION) FOR CONCRETE PAVING AND INCIDENTAL CRACKS SHALL BE SEALED AND INSTALLED IN ACCORDANCE WITH THE AMERICAN CONCRETE PAVEMENT ASSOCIATION (ACPA) RECOMMENDATIONS. CONTRACTOR SHALL OBSERVE THE ARCHITECTURAL AND STRUCTURAL JOINTING LAYOUTS. IN THE EVENT OF A DISCREPANCY OR CONFLICT FOR SITE PAVING, THE CONTRACTOR SHALL REFER TO ACPA PUBLICATION IS061.01P AND IS400.01P FOR THE JOINT SPECIFICATIONS AND THE
- LAYOUT OF PAVEMENT JOINTS (NON-PAY ITEM). 16. THE CONTRACTOR SHALL USE CARE DURING SOIL STABILIZATION AND COMPACTION ACTIVITIES SO AS NOT TO ADVERSELY AFFECT LANDSCAPE AREAS OR UTILITY LINES WITH SOIL STABILIZATION TREATMENTS. AFTER COMPACTION AND PRIOR TO PLACING GRASS, THE UPPER 8 INCHES (8") OF ALL LANDSCAPED AREAS SHALL BE AERATED, TILLED, OR OTHERWISE PROCESSED SO AS TO PROMOTE HEALTHY ROOT GROWTH FOR TURF AND OTHER VEGETATION. THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY REPAIRS, UNDERCUTTING, REMOVAL, DISPOSAL, AND BACKFILLING OF THESE AREAS IF STABILIZATION IS DISCOVERED (NON-PAY ITEM).
- 17. THE CONTRACTOR SHALL SUBMIT A JOINT SPACING PLAN FOR ENGINEER APPROVAL PRIOR TO CONSTRUCTION.

RETAINING WALLS:

- 1. RETAINING WALLS SHOWN ARE FOR SITE GRADING PURPOSES ONLY, AND INCLUDE ONLY LOCATION AND SURFACE SPOT ELEVATIONS AT THE TOP AND BOTTOM OF THE WALL.
- RETAINING WALL TYPE OR SYSTEM SHALL BE SELECTED BY THE OWNER.
- RETAINING WALL DESIGN SHALL BE PROVIDED BY OTHERS AND SHALL FIT IN THE WALL ZONE OR LOCATION SHOWN ON THESE PLANS. STRUCTURAL DESIGN AND PERMITTING OF RETAINING WALLS, RAILINGS, AND OTHER WALL SAFETY DEVICES SHALL BE PERFORMED BY A LICENSED ENGINEER AND ARE NOT PART
- 4. RETAINING WALL DESIGN SHALL MEET THE INTENT OF THE GRADING PLAN AND SHALL ACCOUNT FOR ANY INFLUENCE ON ADJACENT BUILDING FOUNDATIONS,
- UTILITIES. PROPERTY LINES AND OTHER CONSTRUCTABILITY NOTES. RETAINING WALL ENGINEER SHALL CONSULT THESE PLANS AND THE GEOTECHNICAL REPORT FOR POTENTIAL CONFLICTS.

DEMOLITION NOTES

- NO EARTH-DISTURBING ACTIVITIES SHALL COMMENCE UNTIL ALL PERMITS ARE OBTAINED AND PERIMETER EROSION CONTROL MEASURES ARE IN PLACE. 2. ALL DEMOLITION SHALL BE CLOSELY COORDINATED WITH THE OWNER'S REPRESENTATIVE REGARDING ITEMS TO BE SALVAGED, THOSE TO BE REMOVED, ETC. INCLUDING ANY AND ALL TREE PRESERVATION AND TRANSPLANTING ACTIVITIES, AS OUTLINED IN THE PRE-CONSTRUCTION MEETING. REMOVAL RELOCATION AND/OR DISPOSAL OF ANY PRE-EXISTING ON-SITE TRASH, DEBRIS, OR STOCKPILES SHALL BE INCLUDED IN THE TOTAL COST OF DEMOLITION
- AND SHALL BE COORDINATED WITH THE OWNER'S REPRESENTATIVE AT ALL TIMES. CONTRACTOR SHALL COMPLY TO THE FULLEST EXTENT WITH ALL REGULATIONS GOVERNING AGENCIES REGARDING THE DEMOLITION, REMOVAL
- TRANSPORTATION AND DISPOSAL OF ALL DEMOLITION DEBRIS. INGRESS AND EGRESS POINTS, PROPOSED DISPOSAL SITES, AND HAUL ROUTES MUST BE APPROVED BY CITY OFFICIALS PRIOR TO REMOVAL OF
- DEMOLITION DEBRIS OFF-SITE THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING DISCONNECTION OF ALL UTILITIES SERVING THE EXISTING SITE WITH THE APPROPRIATE UTILITY COMPANY, AND SHALL OBTAIN APPROVAL FROM SAME TO COMMENCE DEMOLITION ACTIVITIES.
- CONTRACTOR SHALL COMPLY TO THE FULLEST EXTENT WITH THE LATEST OSHA STANDARDS FOR EXCAVATION AND TRENCHING PROCEDURES CONTRACTOR SHALL USE SUPPORT SYSTEMS, SLOPING, BENCHING, ETC. AS NECESSARY FOR THESE OPERATIONS, AND SHALL COMPLY WITH ALL OSHA PERFORMANCE CRITERIA
- THE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR THE PROTECTION OF ALL PROPERTY CORNER MONUMENTS, BENCHMARKS, CONTROL POINTS, ETC, AND SHALL HAVE, AT HIS EXPENSE, ALL CORNER MONUMENTS REPLACED WHICH ARE DISTURBED BY CONSTRUCTION ACTIVITIES.
- THE CONTRACTOR SHALL INCUR ALL COSTS FOR MAINTENANCE AND REPAIR OF THE EXISTING FENCES TO REMAIN, IRRIGATION SYSTEMS TO REMAIN, UTILITY LINES, ETC, AS OUTLINED IN THE SPECIFICATIONS.
- THE CONTRACTOR SHALL LOCATE, REMOVE, RELOCATE, AND LOWER ALL UNDERGROUND UTILITY CABLES (ELECTRIC, TELEPHONE, ETC.) UP TO A DEPTH OF 24 INCHES BELOW GRADE AS PART OF THE BASE BID.
- 10. THE CONTRACTOR SHALL LOCATE AND REMOVE ALL UNDERGROUND UTILITY PIPING, CONDUIT, AND CABLES, REGARDLESS OF DEPTH, IN THE AREA OF THE PROPOSED BUILDING(S) FOUNDATIONS.
- 11. NOTES SHOWN HEREON REGARDING SPECIFIC ITEMS OF DEMOLITION ARE GENERAL IN NATURE, AND ARE NOT INTENDED TO BE WHOLLY INCLUSIVE. THE CONTRACTOR SHALL DEMOLISH AND REMOVE ALL EXISTING IMPROVEMENTS TO THE SATISFACTION OF THE OWNER, AS NECESSARY FOR THE
- CONSTRUCTION OF THE PROPOSED IMPROVEMENTS, AND TO THE EXTENT AS NOTED IN THE SPECIFICATIONS. 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PLUGGING, CAPPING, OR OTHERWISE TERMINATING UTILITY SERVICE LINES AT EXISTING METER
- LOCATIONS, CLEANOUTS, ETC. A MIN. DISTANCE OF 1 FOOT OUTSIDE THE LIMITS OF THE TRACT SHOWN. 13. THE CONTRACTOR SHALL CREATE AMPLE STAGING AND STOCKPILING AREAS FOR THE DELIVERIES OF CONSTRUCTION MATERIALS, CONCRETE DELIVERIES
- TOPSOIL, ETC. IN ACCORDANCE WITH THE OWNER'S REPRESENTATIVE AND THE PROJECT SPECIFICATIONS. 14. IF ASBESTOS, LEAD-BASED ITEMS OR ANY OTHER HAZARDOUS MATERIALS ARE ENCOUNTERED THE CONTRACTOR IS REQUIRED TO FOLLOW ALL LOCAL, STATE, AND FEDERAL GUIDELINES FOR THE CONTAINMENT, REMOVAL, AND DISPOSAL PROCEDURES.
- 15. KE IS NOT RESPONSIBLE FOR THE MEANS AND METHODS EMPLOYED BY THE CONTRACTOR TO IMPLEMENT THIS DEMOLITION PLAN. THIS DEMOLITION PLAN SIMPLY INDICATES THE KNOWN OBJECTS ON THE SUBJECT TRACT THAT ARE TO BE DEMOLISHED AND REMOVED FROM THE SITE.
- 16. KE DOES NOT WARRANT OR REPRESENT THAT THE PLAN, WHICH WAS PREPARED BASED ON SURVEY AND UTILITY INFORMATION PROVIDED BY OTHERS, SHOWS ALL IMPROVEMENTS AND UTILITIES, THAT THE IMPROVEMENTS AND UTILITIES ARE SHOWN ACCURATELY, OR THAT THE UTILITIES SHOWN CAN BE REMOVED. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING ITS OWN SITE RECONNAISSANCE TO SCOPE ITS WORK AND TO CONFIRM WITH THE OWNERS OF IMPROVEMENTS AND UTILITIES THE ABILITY AND PROCESS FOR THE REMOVAL OR THEIR FACILITIES.
- 17. THIS DEMOLITION PLAN IS INTENDED TO GIVE GENERAL GUIDANCE TO THE CONTRACTOR, NOTHING MORE. THE GOAL OF THE DEMOLITION IS TO LEAVE THI SITE IN A STATE SUITABLE FOR THE CONSTRUCTION OF THE PROPOSED DEVELOPMENT. REMOVAL OR PRESERVATION OF IMPROVEMENTS, UTILITIES, ETC.
- TO ACCOMPLISH THIS GOAL ARE THE RESPONSIBILITY OF THE CONTRACTOR. 18. CONTRACTOR IS STRONGLY CAUTIONED TO REVIEW THE FOLLOWING REPORTS DESCRIBING SITE CONDITIONS PRIOR TO BIDDING AND IMPLEMENTING THI
- 18.1. ENVIRONMENTAL SITE ASSESSMENT PROVIDED BY THE OWNER,
- 18.2. ASBESTOS BUILDING INSPECTION REPORT(S) PROVIDED BY THE OWNER,
- 18.3. GEOTECHNICAL REPORT PROVIDED BY THE OWNER,
- 18.4. OTHER REPORTS THAT ARE APPLICABLE AND AVAILABLE. 19. CONTRACTOR SHALL CONTACT THE OWNER TO VERIFY WHETHER ADDITIONAL REPORTS OR AMENDMENTS TO THE ABOVE CITED REPORTS HAVE BEEN
- PREPARED AND TO OBTAIN/REVIEW/AND COMPLY WITH THE RECOMMENDATION OF SUCH STUDIES PRIOR TO STARTING ANY WORK ON THE SITE. 20. CONTRACTOR SHALL COMPLY WITH ALL LOCAL, STATE, AND FEDERAL REGULATIONS REGARDING THE DEMOLITION OF OBJECTS ON THE SITE AND THE DISPOSAL OF THE DEMOLISHED MATERIALS OFF-SITE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO REVIEW THE SITE, DETERMINE THE APPLICABLE
- REGULATIONS, RECEIVE THE REQUIRED PERMITS AND AUTHORIZATIONS, AND COMPLY. 21. KE DOES NOT REPRESENT THAT THE REPORTS AND SURVEYS REFERENCED ABOVE ARE ACCURATE, COMPLETE, OR COMPREHENSIVE SHOWING ALL ITEMS
- THAT WILL NEED TO BE DEMOLISHED AND REMOVED 22. SURFACE PAVEMENT INDICATED MAY OVERLAY OTHER HIDDEN STRUCTURES, SUCH AS ADDITIONAL LAYERS OF PAVEMENT, FOUNDATIONS OR WALLS, THAT ARE ALSO TO BE REMOVED.

UTILITY NOTES

- THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL UTILITIES, WHETHER PRIVATE OR PUBLIC, PRIOR TO MOBILIZATION. CONTRACTOR SHALL VISIT THE SITE AND MAKE ALL NECESSARY OBSERVATIONS AND INSPECTIONS TO FAMILIARIZE THEMSELVES WITH THE SITE AND THE SITE FACILITIES. THE INFORMATION AND DATA SHOWN WITH RESPECT TO EXISTING UNDERGROUND FACILITIES AT OR CONTIGUOUS TO THE SITE IS APPROXIMATE AND BASED ON INFORMATION FURNISHED BY THE OWNERS OF SUCH UNDERGROUND FACILITIES OR ON PHYSICAL APPURTENANCES OBSERVED IN THE FIELD. THE OWNER AND ENGINEER SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF ANY SUCH INFORMATION OR DATA; AND, THE CONTRACTOR, SHALL HAVE FULL RESPONSIBILITY FOR REVIEWING AND CHECKING ALL SUCH INFORMATION AND DATA, FOR LOCATING ALL UNDERGROUND FACILITIES, FOR COORDINATION OF THE WORK WITH THE OWNERS OF SUCH UNDERGROUND FACILITIES DURING CONSTRUCTION, FOR THE SAFETY AND PROTECTION THEREOF, AND REPAIRING ANY DAMAGE THERETO RESULTING FROM THE WORK. THE COST OF ALL WILL BE CONSIDERED AS HAVING BEEN INCLUDED IN THE CONTRACT PRICE.
- CONTRACTOR SHALL, IN BASE BID PROVIDE ALL NECESSARY FITTINGS AND APPURTENANCES REQUIRED TO COMPLETE ALL CONNECTIONS, RESOLVE UTILITY CONFLICTS AND OTHER INCIDENTAL UTILITY WORK SHOWN ON THE PLANS OR CONTAINED IN THE SPECIFICATIONS OR REQUIRED BY GOVERNING AGENCIES TO INCLUDE, BUT NOT LIMITED TO TEMPORARY SERVICES: VALVES, BOXES, METERS, BACKFLOW PREVENTORS, FIRE DEPARTMENT CONNECTIONS, ETC. INCLUDING THE REPAIR OR REPLACEMENT OF ANY EXISTING IRRIGATION SYSTEM. CONTRACTOR SHALL RAISE/LOWER OR ADJUST ALL EXISTING UTILITY MAINS IN CONFLICT WITH PROPOSED UTILITIES AS PART OF THE BASE BID FOR ALL KNOWN OR
- THE CONTRACTOR SHALL NOTIFY ALL AFFECTED UTILITY COMPANIES OR AGENCIES IN WRITING AT LEAST 1 WEEK PRIOR TO BEGINNING CONSTRUCTION.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR AND MAKE ARRANGEMENTS FOR ANY AND ALL TEMPORARY UTILITIES, PERMITS, AND AGREEMENTS. THE CONTRACTOR SHALL PROTECT ALL UTILITIES DURING THE CONSTRUCTION OF THIS PROJECT. THE CONTRACTOR SHALL GIVE THE CITY, RESIDENTS
- AND BUSINESSES AFFECTED BY ANY ANTICIPATED WATER OR SEWER SERVICE DISRUPTIONS AT LEAST FORTY-EIGHT (48) HOURS PRIOR NOTICE. CONTRACTOR SHALL EXERCISE CAUTION AND MAINTAIN ADEQUATE CLEAR ZONE BETWEEN THE CONTRACTOR'S EQUIPMENT AND ANY POWER LINES. THE CONTRACTOR SHALL PROTECT ALL EXISTING POWER POLES, SIGNS, MANHOLES, TELEPHONES RISERS, WATER VALVES, UTILITIES, ETC, DURING ALL CONSTRUCTION PHASES. CONTRACTOR WILL BE RESPONSIBLE TO REPLACE ANY DAMAGED ITEMS AND RESTORE ANY SERVICES THAT HAVE BEEN
 - DISTURBED. ALL MANHOLES, CLEAN-OUTS, WATER VALVES, FIRE HYDRANTS AND OTHER APPURTENANCES MUST BE ADJUSTED TO FINAL GRADE BEFORE THE OWNER WILL ACCEPT THE WORK THE CONTRACTOR SHALL SALVAGE ALL EXISTING CITY UTILITIES (INCLUDING SIGNS, VALVES, FIRE HYDRANTS, ETC.) IN ACCORDANCE WITH CITY
- REQUIREMENTS AND PROVIDE TO THE CITY. ALL UTILITIES WITHIN 5' OF PROPOSED BUILDING(S) SHALL ADHERE TO THE MEP'S RECOMMENDATIONS AND OR REQUIREMENTS. CONTRACTOR SHALL PROVIDE STORM DRAIN CONNECTIONS FOR ALL ROOF DRAIN LINES. REFER TO MEP'S PLANS AND RELATED. TECHNICAL SPECIFICATIONS. CIVIL UTILITIES (WATER, SANITARY SEWER & STORM SEWER) LIMITS BEGIN 5' OUTSIDE THE BUILDING. IN THE EVENT OF OF A CONFLICT WITH THE MEP'S WITHIN THIS
- AREA. THE MEP'S REQUIREMENTS SHALL GOVERN TESTING OF UTILITY TRENCH BACKFILL COMPACTION SHALL BE AT 75' INTERVALS AND EACH LIFT'S BACKFILL UNLESS OTHERWISE DEFINED IN THE GEOTECHNICAL REPORT FOR THIS PROJECT. BACKFILL SHALL BE PROCESSED SUCH THAT NO DIRT CLODS ARE IN EXCESS OF 4" DIAMETER. ALL SANITARY SEWER LINES AND STORM SEWER LINES SHALL BE TV TESTED AT THE COMPLETION OF THE PROJECT (IN ADDITION TO MINIMUM CODE OR OTHER REQUIREMENTS) TO CHECK FOR DAMAGE CAUSED BY OTHER TRADES, UTILITY CONFLICTS, TRENCH SETTLEMENT, ETC. THE COST OF SUCH SHALL BE INCLUDED IN THE CONTRACTORS BASE PRICE.

MBC

LOW POINT

MULTIPLE BOX CULVERT

MATCH EXISTING

I FFT

STANDARD ABBREVIATIONS:

APPROXIMATELY

BACK OF CURE

ASPHALT

APPROX

ASPH

DO	Brior of Cord	IVIL	W/ (1 OI 1 EXIOTING
B-B	BACK TO BACK OF CURB	MH	MANHOLE
BFR	BARRIER FREE RAMPS	N/A	NOT APPLICABLE
BM	BENCHMARK	NG	NATURAL GROUND (EXISTING)
BW	BOTTOM OF WALL	PC	POINT OF CURVATURE
CATV	CABLE TV	PCC	POINT OF COMPOUND CURVATURE
CFS	CUBIC FEET PER SECOND	PI	POINT OF INTERSECTION
CI	CURB INLET	PIV	POST INDICATOR VALVE
CMP	CORRUGATED METAL PIPE	PL	PROPERTY LINE
CO	CLEANOUT	PP	POWER POLE
CONC	CONCRETE	PRC	POINT OF REVERSE CURVATURE
CONN	CONNECTION	PROP	PROPOSED
CONST	CONSTRUCT	PT	POINT OF TANGENCY
CL	CENTER LINE	PVC	POLYVINYL CHLORIDE PIPE
CRCP	CONTINUOUSLY REINFORCED CONCRETE PAVEMENT	PVMT	PAVEMENT
DCO	DOUBLE CLEANOUT	OCEW	ON CENTER EACH WAY
DE	DRAINAGE EASEMENT	OHE	OVERHEAD ELECTRIC
DI	DROP INLET	R	RADIUS
DIA	DIAMETER	RCB	REINFORCED CONCRETE BOX
DIP	DUCTILE IRON PIPE	RCI	RECESSED CURB INLET
DW	DOMESTIC WATER	RCP	REINFORCED CONCRETE PIPE
EJ	EXPANSION JOINT	RCCP	REINFORCED CONCRETE CYLINDRICAL PIPE
ELEV	ELEVATION	REINF	REINFORCED CONCRETE CTLINDRICAL FIFE
EMH			RIDGE LINE
EMIN EP	ELECTRIC MANHOLE	RL ROW	
ESMT	EDGE OF PAVEMENT EASEMENT		RIGHT OF WAY RIGHT
EX	EXISTING	RT SF	SQUARE FEET
EW	END OF WALL	SD	STORM DRAIN
FC	FACE OF CURB	SQ	SQUARE
F-F	FACE TO FACE OF CURB	SS	SANITARY SEWER
FFE	FINISH FLOOR ELEVATION	SSE	SANITARY SEWER EASEMENT
FH	FIRE HYDRANT	STA	STATION
FM	FORCE MAIN	SY	SQUARE YARD
FO	FIBER OPTICS	T	TELEPHONE
FG	FINISHED GRADE	TC	TOP OF CURB
FP	FINISHED PAD	TG	TOP OF GROUND
FPS	FEET PER SECOND	TMH	TELEPHONE MANHOLE
FL	FLOW LINE	TP	TOP OF PAVEMENT
G		TPIPE	
	GUTTER		TOP OF PIPE
GI	GRATE INLET	TW TYP	TOP OF WALL
GM	GAS METER	UE	TYPICAL
HDPE HDWL	HIGH DENSITY POLYETHYLENE PIPE	UGE	UTILITY EASEMENT
	HEADWALL		UNDERGROUND ELECTRIC
HMAC	HOT MIX ASPHALTIC CONCRETE	VCP	VITRIFIED CLAY PIPE
HORIZ	HORIZONTAL	WTR	WATER
HP	HIGH POINT	WE	WATER EASEMENT
HVAC	HEATING, VENTILATION AND AIR CONDITIONING	WL	WATER METER
IRR	IRRIGATION	WM	WATER METER
JB	JUNCTION BOX	WMH	WATER MANHOLE
JT	JOINT	WV	WATER VALVE
LF	LINEAR FEET	WW	WASTE WATER



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

> X B ANI S S O I ≥



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874 JOB NUMBER: CHV21004

ISSUE DATE:

GENERAL CONSTRUCTION

- 2. ANY EXISTING UTILITIES, PAVEMENT, CURBS, SIDEWALKS, STRUCTURES, TREES, ETC., NOT PLANNED FOR DESTRUCTION OR REMOVAL THAT ARE DAMAGED OR REMOVED SHALL BE REPAIRED OR REPLACED AT CONTRACTOR'S EXPENSE.
- 3. THE CONTRACTOR SHALL VERIFY ALL DEPTHS AND LOCATIONS OF EXISTING UTILITIES PRIOR TO ANY CONSTRUCTION. ANY DISCREPANCIES WITH THE CONSTRUCTION PLANS FOUND IN THE FIELD SHALL BE BROUGHT IMMEDIATELY TO THE ATTENTION OF THE ENGINEER WHO SHALL BE RESPONSIBLE FOR
- 4. MANHOLE FRAMES, COVERS, VALVES, CLEANOUTS, ETC. SHALL BE RAISED TO FINISHED GRADE PRIOR TO FINAL PAVING CONSTRUCTION.
- 5. THE CONTRACTOR SHALL GIVE THE CITY OF LIBERTY HILL 48 HOURS NOTICE BEFORE BEGINNING EACH PHASE OF CONSTRUCTION. TELEPHONE 512-778-5449 (PLANNING & DEVELOPMENT DEPARTMENT).
- 6. ALL AREAS DISTURBED OR EXPOSED DURING CONSTRUCTION SHALL BE REVEGETATED IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. REVEGETATION OF ALL DISTURBED OR EXPOSED AREAS SHALL CONSIST OF SODDING OR SEEDING, AT THE CONTRACTOR'S OPTION. HOWEVER, THE TYPE OF REVEGETATION MUST EQUAL OR EXCEED THE TYPE OF VEGETATION PRESENT BEFORE CONSTRUCTION.
- 7. PRIOR TO ANY CONSTRUCTION. THE ENGINEER SHALL CONVENE A PRECONSTRUCTION CONFERENCE BETWEEN THE CITY OF LIBERTY HILL. HIMSELF. THE CONTRACTOR, OTHER UTILITY COMPANIES, ANY AFFECTED PARTIES AND ANY OTHER ENTITY THE CITY OR ENGINEER MAY REQUIRE.
- 8. THE CONTRACTOR AND THE ENGINEER SHALL KEEP ACCURATE RECORDS OF ALL CONSTRUCTION THAT DEVIATES FROM THE PLANS. THE ENGINEER SHALL FURNISH THE CITY OF LIBERTY HILL ACCURATE "AS-BUILT" DRAWINGS FOLLOWING COMPLETION OF ALL CONSTRUCTION. THESE "AS-BUILT" DRAWINGS SHALL MEET WITH THE SATISFACTION OF THE PLANNING & DEVELOPMENT DEPARTMENT PRIOR TO FINAL ACCEPTANCE
- 9. THE LIBERTY HILL CITY COUNCIL SHALL NOT BE PETITIONED FOR ACCEPTANCE UNTIL ALL NECESSARY EASEMENT DOCUMENTS HAVE BEEN SIGNED AND RECORDED.
- 10. WHEN CONSTRUCTION IS BEING CARRIED OUT WITHIN EASEMENTS, THE CONTRACTOR SHALL CONFINE CONTRACTOR'S WORK TO WITHIN THE PERMANENT AND ANY TEMPORARY EASEMENTS. PRIOR TO FINAL ACCEPTANCE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING ALL TRASH AND DEBRIS WITHIN THE PERMANENT AND TEMPORARY EASEMENTS. CLEAN-UP SHALL BE TO THE SATISFACTION OF THE CITY ENGINEER AND/OR CITY INSPECTOR.
- 11. PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR SHALL APPLY FOR AND SECURE ALL PROPER PERMITS FROM THE APPROPRIATE AUTHORITIES.
- 12. BENCHMARKS UTILIZED FOR THE CONSTRUCTION OF THIS PROJECT ARE DESCRIBED AS FOLLOWS:

TRENCH SAFETY NOTES:

- IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, ALL TRENCHES OVER 5 FEET IN DEPTH IN EITHER HARD AND COMPACT OR SOFT AND UNSTABLE SOIL SHALL BE SLOPED, SHORED, SHEETED, BRACED OR OTHERWISE SUPPORTED. FURTHERMORE, ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT (WILL BE PROVIDED BY THE CONTRACTOR; ARE ON
- 2. IN ACCORDANCE WITH THE U. S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4-FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED SO AS TO REQUIRE NO MORE THAN 25 FEET OF LATERAL
- 3. IF TRENCH SAFETY SYSTEM DETAILS WERE NOT PROVIDED IN THE PLANS BECAUSE TRENCHES WERE ANTICIPATED TO BE LESS THAN 5 FEET IN DEPTH AND DURING CONSTRUCTION IT IS FOUND THAT TRENCHES ARE IN FACT 5 FEET OR MORE IN DEPTH OR TRENCHES LESS THAN 5 FEET IN DEPTH ARE IN AN AREA WHERE HAZARDOUS GROUND MOVEMENT IS EXPECTED, ALL CONSTRUCTION SHALL CEASE, THE TRENCHED AREA SHALL BE BARRICADED AND THE ENGINEER NOTIFIED IMMEDIATELY. CONSTRUCTION SHALL NOT RESUME UNTIL APPROPRIATE TRENCH SAFETY SYSTEM DETAILS, AS DESIGNED BY A PROFESSIONAL ENGINEER, ARE RETAINED AND COPIES SUBMITTED TO THE CITY OF LIBERTY HILL

STREET AND DRAINAGE NOTES:

- 1. ALL TESTING SHALL BE DONE BY AN INDEPENDENT LABORATORY AT THE OWNER'S EXPENSE. ANY RETESTING SHALL BE PAID FOR BY THE CONTRACTOR. A CITY INSPECTOR SHALL BE PRESENT DURING ALL TESTS. TESTING SHALL BE COORDINATED WITH THE CITY INSPECTOR AND HE SHALL BE GIVEN A MINIMUM OF 24 HOURS NOTICE PRIOR TO ANY TESTING. TELEPHONE 512-778-5449 (INSPECTIONS).
- BACKFILL BEHIND THE CURB SHALL BE COMPACTED TO OBTAIN A MINIMUM OF 95% MAXIMUM DENSITY TO WITHIN 3" OF TOP OF CURB. MATERIAL USED SHALL BE PRIMARILY GRANULAR WITH NO ROCKS LARGER THAN 6" IN THE GREATEST DIMENSION. THE REMAINING 3" SHALL BE CLEAN TOPSOIL FREE FROM ALL
- 3. DEPTH OF COVER FOR ALL CROSSINGS UNDER PAVEMENT INCLUDING GAS, ELECTRIC, TELEPHONE, CABLE TV, WATER SERVICES, ETC., SHALL BE A MINIMUM OF 30" BELOW SUBGRADE.
- 4. STREET RIGHTS-OF-WAY SHALL BE GRADED AT A SLOPE OF 1/4" PER FOOT TOWARD THE CURB UNLESS OTHERWISE INDICATED. HOWEVER, IN NO CASE SHALL THE WIDTH OF RIGHT-OF-WAY AT 1/4" PER FOOT SLOPE BE LESS THAN 10 FEET UNLESS A SPECIFIC REQUEST FOR AN ALTERNATE GRADING SCHEME IS MADE TO AND ACCEPTED BY THE CITY OF LIBERTY HILL PLANNING & DEVELOPMENT DEPARTMENT
- 5. BARRICADES BUILT TO CITY OF LIBERTY HILL STANDARDS SHALL BE CONSTRUCTED ON ALL DEAD-END STREETS AND AS NECESSARY DURING CONSTRUCTION TO MAINTAIN JOB AND PUBLIC SAFETY.
- 6. ALL R.C.P. SHALL BE MINIMUM CLASS III.

STATION THICKNESS

7. THE SUBGRADE MATERIAL FOR THE STREETS SHOWN HEREIN WAS TESTED BY AND THE PAVING SECTIONS DESIGNED IN ACCORDANCE WITH THE CURRENT CITY OF LIBERTY HILL DESIGN CRITERIA. THE PAVING SECTIONS ARE TO BE CONSTRUCTED AS FOLLOWS: FLEX. BASE HMAC LIME STAB. THICKNESS

THE GEOTECHNICAL ENGINEER SHALL INSPECT THE SUBGRADE FOR COMPLIANCE WITH THE DESIGN ASSUMPTIONS MADE DURING PREPARATION OF THE SOILS REPORT. ANY ADJUSTMENTS THAT ARE REQUIRED SHALL BE MADE THROUGH REVISION OF THE CONSTRUCTION PLANS

8. WHERE PI'S ARE OVER 20, SUBGRADES MUST BE STABILIZED UTILIZING A METHOD ACCEPTABLE TO THE CITY ENGINEER. THE GEOTECHNICAL ENGINEER SHALL RECOMMEND AN APPROPRIATE SUBGRADE STABILIZATION IF SULFATES ARE DETERMINED TO BE PRESENT.

WATER AND WASTEWATER NOTES:

- . PIPE MATERIAL FOR WATER MAINS SHALL BE PVC (AWWA C-900, DR-18), OR DUCTILE IRON (AWWA C-100, MIN. CLASS 200). WATER SERVICES (2" OR LESS) SHALL BE POLYETHYLENE TUBING (BLACK, 200 PSI, DR 9).
- 2. PIPE MATERIAL FOR PRESSURE WASTEWATER MAINS SHALL BE GREEN PVC (AWWA C-900, DR-18), OR DUCTILE IRON (AWWA C-100, MIN. CLASS 200). PIPE MATERIAL FOR GRAVITY WASTEWATER MAINS SHALL BE GREEN PVC (ASTM D2241 OR D3034, MAX. DR-26), DUCTILE IRON (AWWA C-100, MIN. CLASS 200). PIPE FOR BOTH PRESSURE AND GRAVITY WASTEWATER MAINS SHALL BE GREEN IN COLOR. ALL DUCTILE IRON PIPE AND FITTINGS SHALL BE LINED WITH INDURON
- 8. UNLESS OTHERWISE ACCEPTED BY THE CITY ENGINEER, DEPTH OF COVER FOR ALL LINES OUT OF THE PAVEMENT SHALL BE 42" MIN., AND DEPTH OF COVER FOR ALL LINES UNDER PAVEMENT SHALL BE A MIN. OF 30" BELOW SUBGRADE.
- 4. ALL FIRE HYDRANT LEADS SHALL BE DUCTILE IRON PIPE (AWWA C-100, MIN. CLASS 200).
- 5. ALL IRON PIPE AND FITTINGS SHALL BE WRAPPED WITH MINIMUM 8-MIL POLYETHYLENE AND SEALED WITH DUCT TAPE OR EQUAL ACCEPTED BY THE CITY
- 6. THE CONTRACTOR SHALL CONTACT THE CITY INSPECTOR AT 512-778-5449 TO COORDINATE UTILITY TIE-INS AND NOTIFY HIM AT LEAST 48 HOURS PRIOR TO CONNECTING TO EXISTING LINES
- 7. ALL MANHOLES SHALL BE CONCRETE WITH CAST IRON RING AND COVER. ALL MANHOLES LOCATED OUTSIDE OF THE PAVEMENT SHALL HAVE BOLTED COVERS AND GASKETED COVERS AND HAVE RIM ELEVATIONS 6" MIN. ABOVE FINISHED GRADE. TAPPING OF FIBERGLASS MANHOLES SHALL NOT BE ALLOWED.
- 8. ALL MANHOLES SHALL BE LINED WITH RAVEN LINING SYSTEMS RAVEN 405 OR APPROVED EQUAL.
- 9. THE CONTRACTOR MUST OBTAIN A BULK WATER PERMIT OR PURCHASE AND INSTALL A WATER METER FOR ALL WATER USED DURING CONSTRUCTION. A COPY OF THIS PERMIT MUST BE CARRIED AT ALL TIMES BY ALL WHO USE WATER.
- 10. LINE FLUSHING OR ANY ACTIVITY USING A LARGE QUANTITY OF WATER MUST BE SCHEDULED WITH THE WATER & WASTEWATER SUPERINTENDENT,
- 11. THE CONTRACTOR, AT CONTRACTOR'S EXPENSE, SHALL PERFORM STERILIZATION OF ALL POTABLE WATER LINES CONSTRUCTED AND SHALL PROVIDE ALL EQUIPMENT (INCLUDING TEST GAUGES), SUPPLIES (INCLUDING CONCENTRATED CHLORINE DISINFECTING MATERIAL), AND NECESSARY LABOR REQUIRED FOR THE STERILIZATION PROCEDURE. THE STERILIZATION PROCEDURE SHALL BE MONITORED BY CITY OF LIBERTY HILL PERSONNEL. WATER SAMPLES WILL BE COLLECTED BY THE CITY OF LIBERTY HILL TO VERIFY EACH TREATED LINE HAS ATTAINED AN INITIAL CHLORINE CONCENTRATION OF 50 PPM. WHERE MEANS OF FLUSHING IS NECESSARY, THE CONTRACTOR, AT CONTRACTOR'S EXPENSE, SHALL PROVIDE FLUSHING DEVICES AND REMOVE SAID DEVICES PRIOR TO FINAL ACCEPTANCE BY THE CITY OF LIBERTY HILL.
- 12. SAMPLING TAPS SHALL BE BROUGHT UP TO 3 FEET ABOVE GRADE AND SHALL BE EASILY ACCESSIBLE FOR CITY PERSONNEL. AT THE CONTRACTOR'S REQUEST, AND IN CONTRACTOR'S PRESENCE, SAMPLES FOR BACTERIOLOGICAL TESTING WILL BE COLLECTED BY THE CITY OF LIBERTY HILL NOT LESS THAN 24 HOURS AFTER THE TREATED LINE HAS BEEN FLUSHED OF THE CONCENTRATED CHLORINE SOLUTION AND CHARGED WITH WATER APPROVED BY THE CITY. THE CONTRACTOR SHALL SUPPLY A CHECK OR MONEY ORDER, PAYABLE TO THE CITY OF LIBERTY HILL, TO COVER THE FEE CHARGED FOR TESTING EACH WATER SAMPLE. CITY OF LIBERTY HILL FEE AMOUNTS MAY BE OBTAINED BY CALLING THE PLANNING & DEVELOPMENT DEPARTMENT AT 512-778-5449.
- 13. THE CONTRACTOR, AT CONTRACTOR'S EXPENSE, SHALL PERFORM QUALITY TESTING FOR ALL WASTEWATER PIPE INSTALLED AND PRESSURE PIPE HYDROSTATIC TESTING OF ALL WATER LINES CONSTRUCTED AND SHALL PROVIDE ALL EQUIPMENT (INCLUDING PUMPS AND GAUGES), SUPPLIES AND LABOR NECESSARY TO PERFORM THE TESTS. QUALITY AND PRESSURE TESTING SHALL BE MONITORED BY CITY OF LIBERTY HILL PERSONNEL.
- 14. THE CONTRACTOR SHALL COORDINATE TESTING WITH THE CITY OF INSPECTOR AND PROVIDE NO LESS THAN 24 HOURS NOTICE PRIOR TO PERFORMING STERILIZATION. QUALITY TESTING OR PRESSURE TESTING.

15. THE CONTRACTOR SHALL NOT OPEN OR CLOSE ANY VALVES UNLESS AUTHORIZED BY THE CITY OF LIBERTY HILL.

16. ALL VALVE BOXES AND COVERS SHALL BE CAST IRON.

17. ALL WATER SERVICE, WASTEWATER SERVICE AND VALVE LOCATIONS SHALL BE APPROPRIATELY MARKED AS FOLLOWS:

WATER SERVICE "W" ON TOP OF CURB WASTEWATER SERVICE "S" ON TOP OF CURB

"V" ON FACE OF CURB.

TOOLS FOR MARKING THE CURB SHALL BE PROVIDED BY THE CONTRACTOR. OTHER APPROPRIATE MEANS OF MARKING SERVICE AND VALVE LOCATIONS SHALL BE PROVIDED IN AREAS WITHOUT CURBS. SUCH MEANS OF MARKING SHALL BE AS SPECIFIED BY THE ENGINEER AND ACCEPTED BY

- 18. CONTACT THE CITY OF LIBERTY HILL WATER & WASTEWATER SUPERINTENDENT AT 512-778-5449 FOR ASSISTANCE IN OBTAINING EXISTING WATER AND
- 19. THE CITY OF LIBERTY HILL FIRE DEPARTMENT SHALL BE NOTIFIED 48 HOURS PRIOR TO TESTING OF ANY BUILDING SPRINKLER PIPING IN ORDER THAT THE FIRE DEPARTMENT MAY MONITOR SUCH TESTING.
- 20. SAND, AS DESCRIBED IN SPECIFICATION ITEM 510 PIPE, SHALL NOT BE USED AS BEDDING FOR WATER AND WASTEWATER LINES. ACCEPTABLE BEDDING MATERIALS ARE PIPE BEDDING STONE, PEA GRAVEL AND IN LIEU OF SAND, A NATURALLY OCCURRING OR MANUFACTURED STONE MATERIAL CONFORMING TO ASTM C33 FOR STONE QUALITY AND MEETING THE FOLLOWING GRADATION SPECIFICATION:

SIEVE SIZE PERCENT RETAINED

BY WEIGHT 1/2" 3/8" 40-85

- 21. THE CONTRACTOR IS HEREBY NOTIFIED THAT CONNECTING TO, SHUTTING DOWN, OR TERMINATING EXISTING UTILITY LINES MAY HAVE TO OCCUR AT OFF-PEAK HOURS. SUCH HOURS ARE USUALLY OUTSIDE NORMAL WORKING HOURS AND POSSIBLY BETWEEN 12 A.M. AND 6 A.M.
- 22. ALL WASTEWATER CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ) REGULATIONS, 30 TAC CHAPTER 213 AND 317, AS APPLICABLE. WHENEVER TCEQ AND CITY OF LIBERTY HILL SPECIFICATIONS CONFLICT, THE MORE STRINGENT SHALL

TRAFFIC MARKING NOTES:

- ANY METHODS, STREET MARKINGS AND SIGNAGE NECESSARY FOR WARNING MOTORISTS, WARNING PEDESTRIANS OR DIVERTING TRAFFIC DURING CONSTRUCTION SHALL CONFORM TO THE TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS, LATEST EDITION.
- ALL PAVEMENT MARKINGS, MARKERS, PAINT, TRAFFIC BUTTONS, TRAFFIC CONTROLS AND SIGNS SHALL BE INSTALLED IN ACCORDANCE WITH THE TEXAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR CONSTRUCTION OF HIGHWAYS, STREETS AND BRIDGES AND, THE TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS, LATEST EDITIONS.

EROSION AND SEDIMENTATION CONTROL NOTES:

- EROSION CONTROL MEASURES, SITE WORK AND RESTORATION WORK SHALL BE IN ACCORDANCE WITH THE CITY OF LIBERTY HILL EROSION AND
- 2. ALL SLOPES SHALL BE SODDED OR SEEDED WITH APPROVED GRASS, GRASS MIXTURES OR GROUND COVER SUITABLE TO THE AREA AND SEASON IN WHICH THEY ARE APPLIED.
- 3. SILT FENCES, ROCK BERMS, SEDIMENTATION BASINS AND SIMILARLY RECOGNIZED TECHNIQUES AND MATERIALS SHALL BE EMPLOYED DURING CONSTRUCTION TO PREVENT POINT SOURCE SEDIMENTATION LOADING OF DOWNSTREAM FACILITIES. SUCH INSTALLATION SHALL BE REGULARLY INSPECTED BY THE CITY OF LIBERTY HILL FOR EFFECTIVENESS. ADDITIONAL MEASURES MAY BE REQUIRED IF, IN THE OPINION OF THE CITY ENGINEER, THEY ARE WARRANTED.
- ALL TEMPORARY EROSION CONTROL MEASURES SHALL NOT BE REMOVED UNTIL FINAL INSPECTION AND APPROVAL OF THE PROJECT BY THE ENGINEER. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN ALL TEMPORARY EROSION CONTROL STRUCTURES AND TO REMOVE EACH STRUCTURE AS APPROVED BY THE ENGINEER.
- 5. ALL MUD, DIRT, ROCKS, DEBRIS, ETC., SPILLED, TRACKED OR OTHERWISE DEPOSITED ON EXISTING PAVED STREETS, DRIVES AND AREAS USED BY THE PUBLIC SHALL BE CLEANED UP IMMEDIATELY.



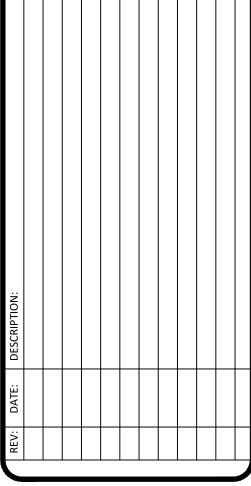
CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

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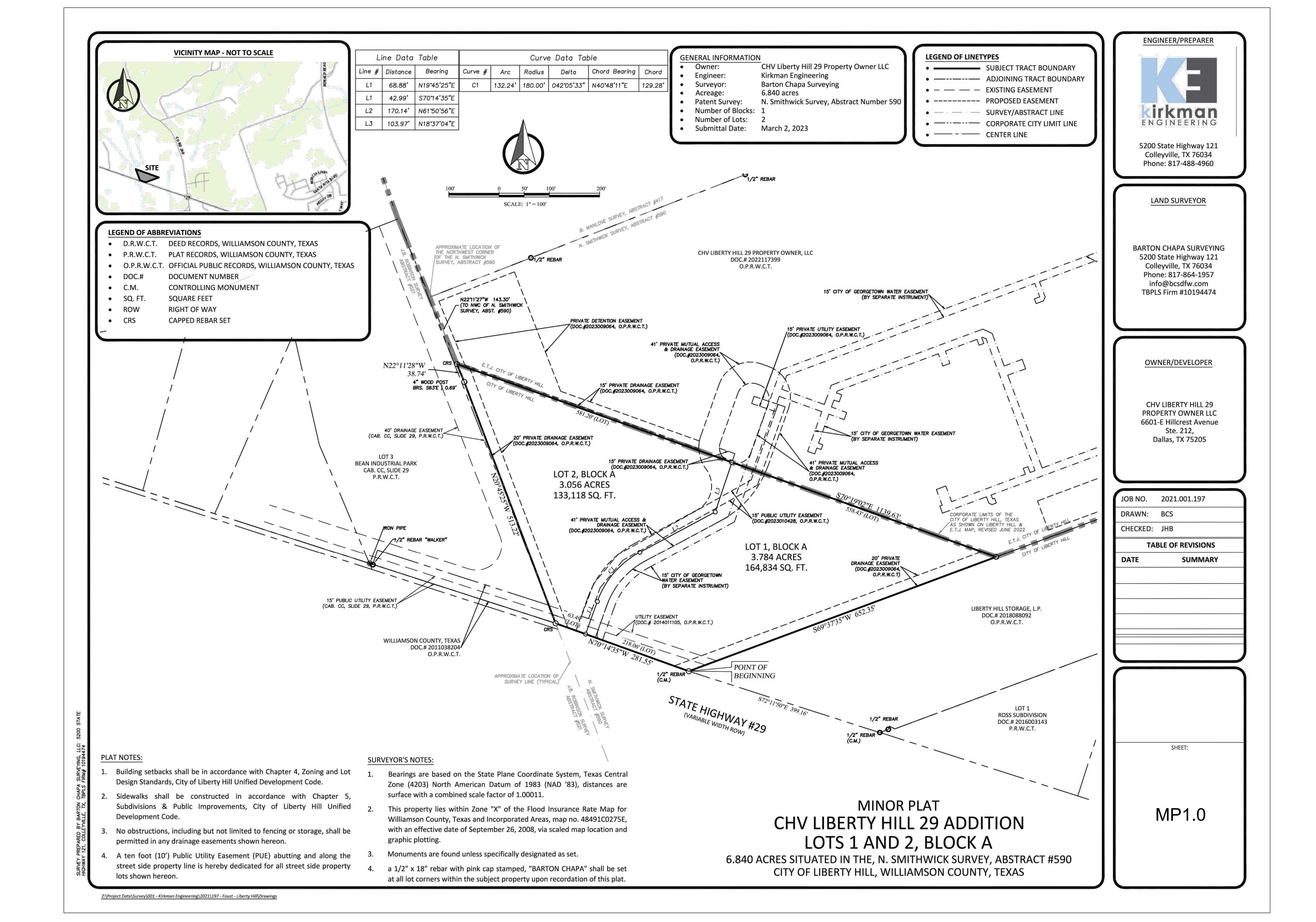


KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

CITY OF LIBERTY HILL GENERAL CONSTRUCTION **NOTES**



THE STATE OF TEXAS

COUNTY OF WILLIAMSON

WHEREAS CHV LIBERTY HILL 29 PROPERTY OWNER LLC is the owner of a tract COUNTY OF WILLIAMSON situated in the N. Smithwick Survey, Abstract Number 590, City of Liberty Hill, Williamson County, Texas, and being a portion of the tract of land described by deed to CHV LIBERTY HILL 29 Property Owner, LLC as recorded under Document Number 2022117399, Official Public Records, Williamson County, Texas, (O.P.R.W.C.T.), the subject tract being more particularly described by metes and bounds as follows (bearings are based on State Plane Coordinate System, Texas Central Zone (4203) North American Datum of 1983 (NAD 83)):

BEGINNING at a 1/2 inch rebar found for the southeast corner of said CHV Liberty Hill 29 tract and the herein described tract;

THENCE North 70 degrees 14 minutes 35 seconds West, with the south line of said CHV Liberty Hill 29 tract, a distance of 281.55 feet to a 1/2 inch rebar with pink cap stamped, "BARTON CHAPA" set (hereinafter called "capped rebar set") for the southwest corner thereof;

THENCE North 20 degrees 45 minutes 25 seconds West, with the west line of said CHV Liberty Hill 29 tract, a distance of 513.22 feet to a point for corner, from which a 4 inch wood post found bears South 63 degrees East, a distance of 0.69 feet;

THENCE North 22 degrees 11 minutes 28 seconds West, with the west line of said CHV Liberty Hill 29 tract, a distance of 38.74 feet to a capped rebar set;

THENCE South 70 degrees 19 minutes 02 seconds East, through the interior of said CHV Liberty Hill 29 tract, a distance of 1,139.63 feet to a capped rebar set in the southeast line of said CHV Liberty Hill 29 tract;

THENCE South 69 degrees 37 minutes 35 seconds West, with the southeast line of said CHV Liberty Hill 29 tract, a distance of 652.35 feet, returning to the POINT OF **BEGINNING** and enclosing 6.840 acres (297,951 square feet) of land, more or less.

THE STATE OF TEXAS

COUNTY OF DALLAS

That CHV LIBERTY HILL 29 PROPERTY OWNER LLC, a Delaware limited liability company, as the owner of that certain 42.682 acre tract of land recorded in Document Number 2022117399 of the Official Records of Williamson County, Texas do hereby dedicate to the public forever use of the streets, alleys, easements and all other lands intended for public dedication as shown hereon to be known as LOTS 1 AND 2, BLOCK A, CHV LIBERTY HILL 29 ADDITION.

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

JUSTIN WALKER, AUTHORIZED SIGNATORY

THE STATE OF TEXAS

COUNTY OF WILLIAMSON

This instrument was acknowledged before me on the_____day of CHV LIBERTY HILL 29 PROPERTY OWNER LLC, a Delaware limited liability company, on behalf of said CHV LIBERTY HILL 29 PROPERTY OWNER LLC.

Printed Name:

Notary Public State of Texas

My Commission Expires:

THE STATE OF TEXAS

COUNTY OF WILLIAMSON

That SIMMONS BANK, the Lien Holder of that certain 42.682 acre tract of land recorded in Document Number 2022117399 of the Official Records of Williamson County, Texas do hereby consent to the subdivision of that certain 42.682 acre tract of land situated in the City of Liberty Hill, Williamson County, Texas, and do further hereby join, approve, and consent to the dedication to the public forever use of the streets, alley, easements and all other lands intended for public dedication as shown hereon.

SIMMONS BANK

, its Managing Director Wes Hardin

UTILITY PROVIDERS Water: City of Georgetown Wastewater: City of Liberty Hill Electric: Pedernales Electric Cooperative Telecom: AT&T

THE STATE OF TEXAS

That I John H. Barton, III, do hereby certify that I prepared this plat from an actual and accurate on the ground survey of the land and that the corner monuments . shown thereon were properly placed under my personal supervision, in accordance with Chapter 5, Subdivisions, Public Improvements, City of Liberty Hill Unified Development Code.

John H. Barton III, RPLS# 6737

CERTIFICATE OF APPROVAL

I, Jerry Millard, Director of Planning, designee, of the City of Liberty Hill, Texas, under the authority granted me in Section 3.09.02 of the Unified Development Code, in accordance with the Texas Local Government Code, do hereby certify this plat as approved for filing of record with the County Clerk of Williamson County,

Jerry Millard, Director of Planning Date

THE STATE OF TEXAS

COUNTY OF WILLIAMSON

That I,	, Clerk of the County Court of said County	, do
•	oregoing instrument in writing, with its certification for record in my office on the day	
	AD 20 at alask M and	
recorded on the	_ day of A.D., 20	_, a
o'clockM. in	n the Plat Records of said County, in Cabi	net
Slide(s)		

WITNESS MY HAND AND SEAL of the County Court of said County, at office in Georgetown, Texas, the date last above written.

Clerk

County Court Williamson County, Texas

, Deputy

MINOR PLAT CHV LIBERTY HILL 29 ADDITION LOTS 1 AND 2, BLOCK A

6.840 ACRES SITUATED IN THE, N. SMITHWICK SURVEY, ABSTRACT #590 CITY OF LIBERTY HILL, WILLIAMSON COUNTY, TEXAS

ENGINEER/PREPARER



5200 State Highway 121 Colleyville, TX 76034 Phone: 817-488-4960

LAND SURVEYOR

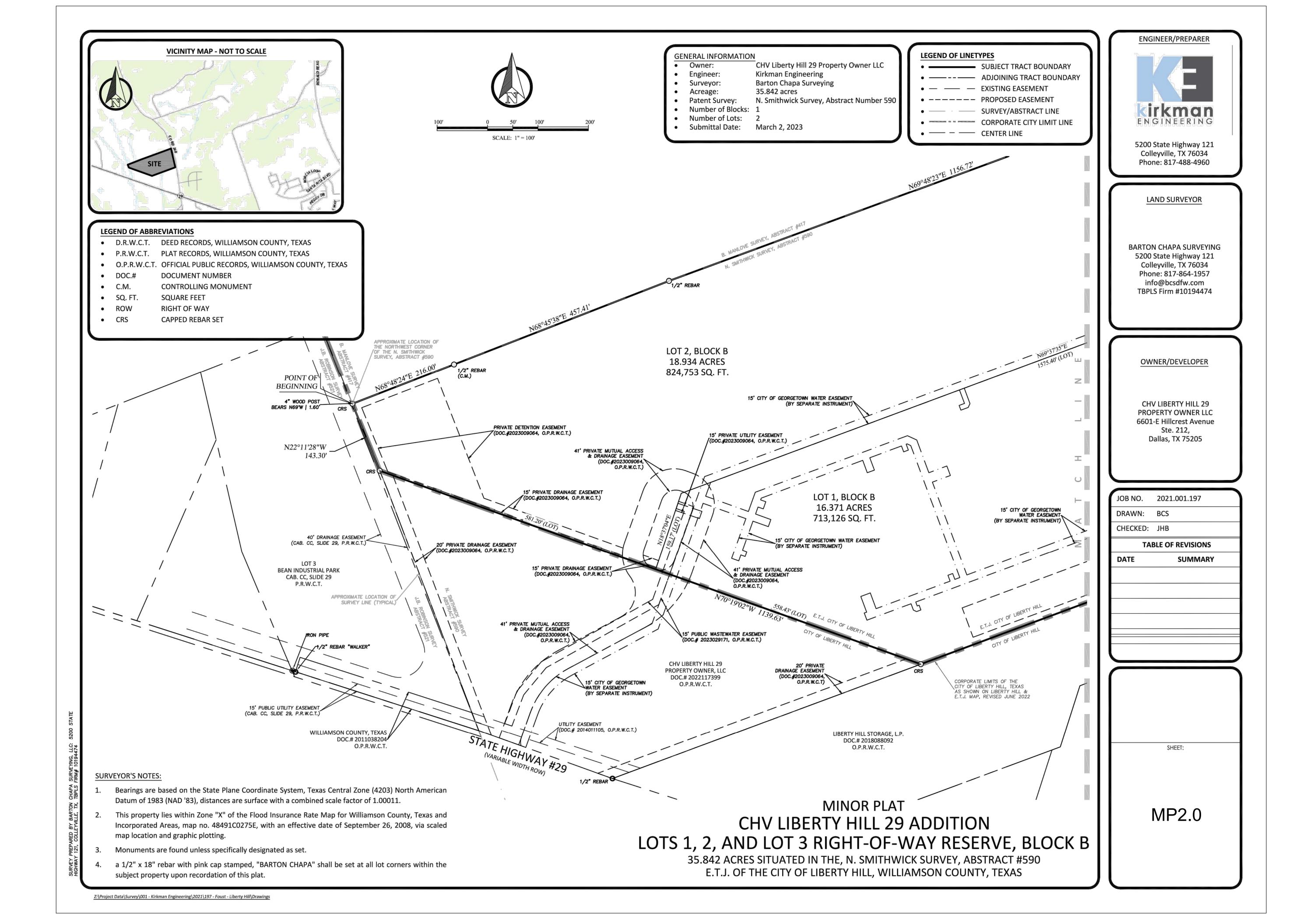
BARTON CHAPA SURVEYING 5200 State Highway 121 Colleyville, TX 76034 Phone: 817-864-1957 info@bcsdfw.com TBPLS Firm #10194474

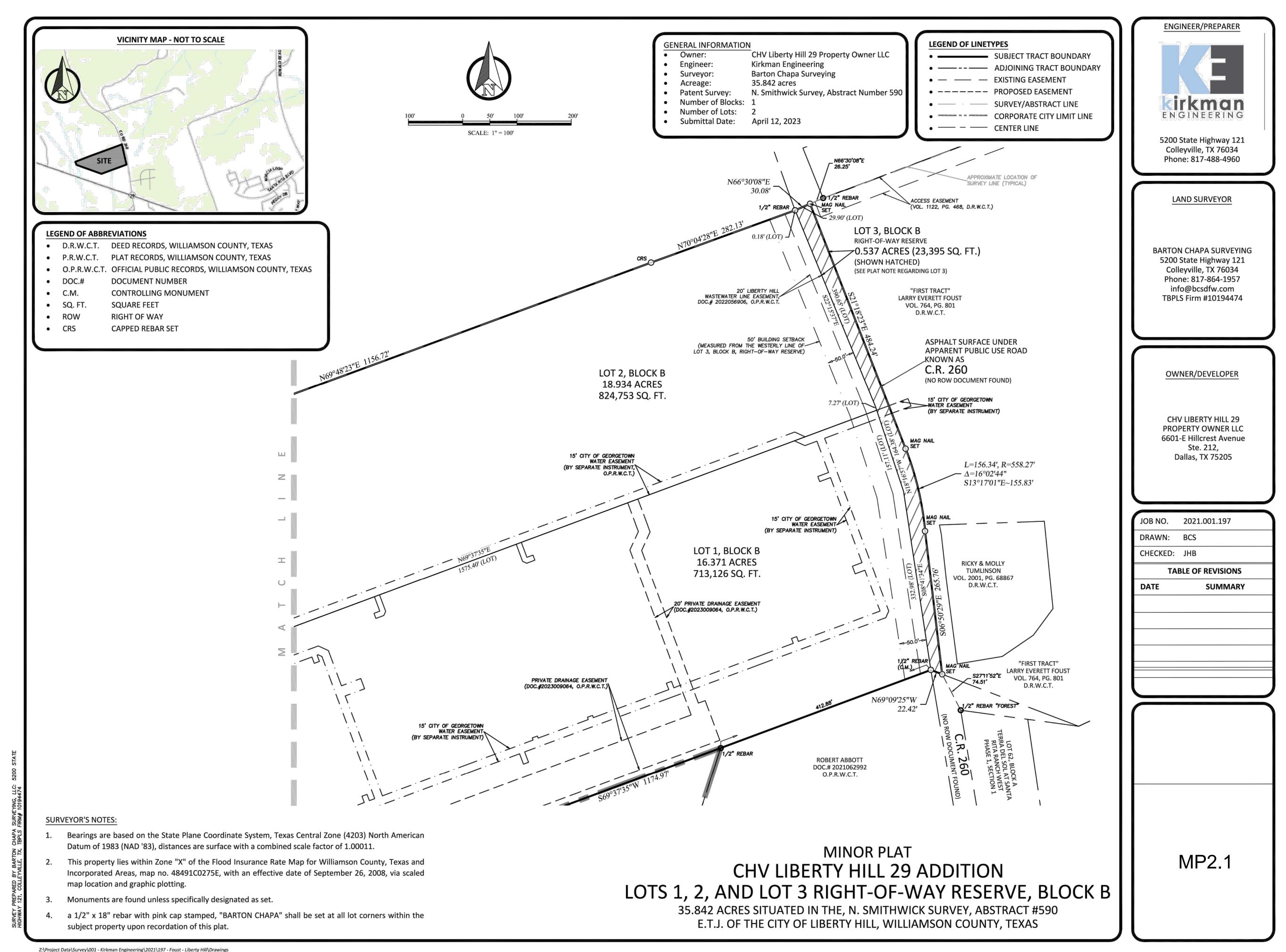
OWNER/DEVELOPER

CHV LIBERTY HILL 29 PROPERTY OWNER LLC 6601-E Hillcrest Avenue Ste. 212, Dallas, TX 75205

7	JOB NO.	2021.001.197		
	DRAWN:	BCS		
	CHECKED:	JHB		
	TABLE OF REVISIONS			
DATE		SUMMARY		
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THE STATE OF TEXAS

COUNTY OF WILLIAMSON

WHEREAS CHV LIBERTY HILL 29 PROPERTY OWNER LLC is the owner of a tract situated in the N. Smithwick Survey, Abstract Number 590, City of Liberty Hill, Williamson County, Texas, and being a portion of the tract of land described by deed to CHV Liberty Hill 29 Property Owner LLC as recorded under Document Number 2022117399, Official Public Records, Williamson County, Texas, (O.P.R.W.C.T.), the subject tract being more particularly described by metes and bounds as follows (bearings are based on State Plane Coordinate System, Texas Central Zone (4203) North American Datum of 1983 (NAD 83)):

BEGINNING at a 1/2 inch rebar found with pink cap stamped, "BARTON CHAPA" set (hereinafter called "capped rebar set") for the northwest corner of said CHV Liberty Hill 29 Property Owner, LLC tract and the herein described tract, from which a 4 inch wood post found bears North 69 degrees West, a distance of 1.60 feet;

THENCE with the perimeter and to the corners of said CHV Liberty Hill 29, Property Owner, LLC tract, the following calls:

- 1. North 68 degrees 48 minutes 24 seconds East, a distance of 216.00 feet to a 1/2 inch rebar
- 2. North 68 degrees 45 minutes 38 seconds East, a distance of 457.41 feet to a 1/2 inch rebar
- 3. North 69 degrees 48 minutes 23 seconds East, a distance of 1,156.72 feet to a capped rebar
- 4. North 70 degrees 04 minutes 28 seconds East, a distance of 282.13 feet to a 1/2 inch rebar
- 5. North 66 degrees 30 minutes 08 seconds East, a distance of 30.08 feet to a MAG nail set;
- 6. South 21 degrees 18 minutes 23 seconds East, a distance of 484.24 feet to a MAG nail set at the beginning of a tangent curve to the right, having a radius of 558.27 feet, with a delta angle of 16 degrees 02 minutes 44 seconds, whose chord bears South 13 degrees 17 minutes 01 seconds East, a distance of 155.83 feet;
- 7. Along said tangent curve to the right, an arc length of 156.34 feet to a MAG nail set;
- 8. South 06 degrees 50 minutes 29 seconds East, a distance of 265.76 feet to a MAG nail set;
- 9. North 69 degrees 09 minutes 25 seconds West, a distance of 22.42 feet to a 1/2 inch rebar
- 10. South 69 degrees 37 minutes 35 seconds West, a distance of 1,174.97 feet to a capped rebar

THENCE North 70 degrees 19 minutes 02 seconds West, through the interior of said CHV Liberty Hill 29 Property Owner, LLC tract, a distance of 1,139.63 feet to a capped rebar set in the west line thereof;

THENCE North 22 degrees 11 minutes 28 seconds West, with the west line of said CHV Liberty Hill 29 Property Owner, LLC tract, a distance of 143.30 feet, returning to the **POINT OF BEGINNING** and enclosing 35.842 acres (1,561,273 square feet) of land, more or less.

THE STATE OF TEXAS

COUNTY OF WILLIAMSON

That CHV LIBERTY HILL 29 PROPERTY OWNER LLC, a Delaware limited liability company, as the owner of that certain 42.682 acre tract of land recorded in Document Number 2022117399 of the Official Records of Williamson County, Texas do hereby dedicate to the public forever use of the streets, alleys, easements and all other lands intended for public dedication as shown hereon to be known as LOTS 1, 2, AND 3, BLOCK B, CHV LIBERTY HILL 29 ADDITION.

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

JUSTIN WALKER, AUTHORIZED SIGNATORY

THE STATE OF TEXAS

COUNTY OF DALLAS

his	instrument w	as acknowledged	before me on	the	_day_of				_
20	, by		, as		of	CHV	LIBERTY	HILL 2	9
ROP	PERTY OWNER	LLC, a Delaware l	imited liability	company, on	behalf of	said	CHV LIBE	RTY HIL	L
29 PI	ROPERTY OWN	IER LLC.							

Notary Public State of Texas

Printed Name:

My Commission Expires:

THE STATE OF TEXAS

COUNTY OF DALLAS

That SIMMONS BANK, the Lien Holder of that certain 42.682 acre tract of land recorded in Document Number 2022117399 of the Official Records of Williamson County, Texas do hereby consent to the subdivision of that certain 42.682 acre tract of land situated in the City of Liberty Hill, Williamson County, Texas, and do further hereby join, approve, and consent to the dedication to the public forever use of the streets, alley, easements and all other lands intended for public dedication as shown hereon.

SIMMONS BANK

BY:_		, its Managing Directo
	Wes Hardin	

PLAT NOTES:

- 1. Sidewalks shall be constructed in accordance with Chapter 5, Subdivisions & Public Improvements, City of Liberty Hill Unified Development Code. All sidewalks shall be maintained by each of the adjacent property owners.
- No obstructions, including but not limited to fencing or storage, shall be permitted in any drainage easements shown hereon.
- A ten foot (10') Public Utility Easement (PUE) abutting and along the street side property line is hereby dedicated for all street side property lots shown hereon.
- 4. All public water and wastewater collection will be supplied by Georgetown Water and Liberty Hill Wastewater.
- 5. Improvements within the County Road (C.R. 260) Right-of-Way including, but not limited to, landscaping, irrigation lighting, custom signs, is prohibited without first obtaining an executed license agreement with Williamson County.
- All public roadways and easements as shown on this plat are free of liens.
- 7. It is the responsibility of the Owner, not the County or the City of Liberty Hill, to assure compliance with the provisions of all applicable State, Federal, and Local laws and regulations relating the the platting and development of this property.

- representations by other parties in this plat. Floodplain data, in particular, will change over time and the current effective floodplain data takes precedence over floodplain data represented on this plat. It is further understood that the owners of the tract of land covered by this plat must install at their own expense all traffic control devices and 14. signage that may be required before the streets in the subdivision have been accepted for maintenance by the County.
- Maintenance and responsibility for drainage will not be accepted by the County other than that accepted in connection with drainage or protecting the road system. Maintenance responsibility for storm water management controls will remain with the owner.
- THE MINIMUM FFE SHALL BE AT LEAST ONE FOOT ABOVE THE ADJACENT FINISHED GRADE AND BFE. EXCEPTIONS CAN BE MADE AT ENTRANCE AND EGRESS POINTS, WHERE NECESSARY, TO MEET THE AMERICANS WITH DISABILITIES ACT (ADA). RECREATIONAL VEHICHLE PARKING PADS MUST ALSO BE PLACED AT LEAST ONE FOOT ABOVE BFE.
- 11. DRIVEWAY MAINTENANCE WILL BE THE RESPONSIBILITY OF THE PROPERTY OWNER. IF OBSTRUCTIONS OCCUR WITHIN THE DRIVEWAY CULVERT, THE COUNTY RESERVES THE RIGHT TO CLEAR OBSTRUCTIONS THAT ARE CAUSING ADVERSE IMPACTS TO THE ROADWAY.
- 12. NO STRUCTURE OR LAND IN THIS PLAT SHALL HEREAFTER BE LOCATED OR ALTERED WITHOUT FIRST OBTAINING A CERTIFICATE OF COMPLIANCE OR FLOODPLAIN DEVELOPMENT PERMIT FROM THE WILLIAMSON COUNTY FLOODPLAIN ADMINISTRATOR.

- 8. The County assumes no responsibility for the accuracy of 13. THIS SUBDIVISION IS SUBJECT TO STORM-WATER MANAGEMENT CONTROLS AS REQUIRED BY WILLIAMSON COUNTY SUBDIVISION REGULATIONS SECTION B11.1 AND THE FLOOD DAMAGE PREVENTION ORDER, ON NEW DEVELOPMENT THAT WOULD EVOKE SUCH CONTROLS BEYOND EXISTING CONDITIONS.
 - THIS SUBDIVISION WAS EXEMPT FROM PROVIDING STORM-WATER MANAGEMENT CONTROLS (DETENTION) AT THE TIME OF FILING THIS PLAT BASED ON WILLIAMSON COUNTY SUBDIVISION REGULATION B11.1.4. PRIOR TO ANY DEVELOPMENT WITHIN THIS SUBDIVISION, STORM-WATER MANAGEMENT CONTROLS SHALL BE DESIGNED. CONSTRUCTED AND MAINTAINED BY THE OWNER IN ACCORDANCE WITH THE APPLICABLE REGULATIONS IN EFFECT AT THE TIME OF DEVELOPMENT. CONTACT THE WILLIAMSON COUNTY FLOODPLAIN ADMINISTRATOR FOR REVIEW AND APPROVAL OF THE PROPOSED STORMWATER MANAGEMENT CONTROLS PRIOR TO ANY DEVELOPMENT WITHIN THIS SUBDIVISION.

15. UTILITY PROVIDERS

Water: City of Georgetown Wastewater: City of Liberty Hill Electric: Pedernales Electric Cooperative Telecom: AT&T

16. Lot 3 Right-of-Way Reserve to be owned by property owner until such time that the reserve lot is acquired.

THE STATE OF TEXAS

COUNTY OF WILLIAMSON

That I John H. Barton, III, do hereby certify that I prepared this plat from an actual and accurate on the ground survey of the land and that the corner monuments shown thereon were properly placed under my personal supervision, in accordance with Chapter 5, Subdivisions, Public Improvements, City of Liberty Hill Unified Development Code.

John H. Barton III, RPLS# 6737

CERTIFICATE OF APPROVAL

I, Jerry Millard, Director of Planning, designee, of the City of Liberty Hill, Texas, under the authority granted me in Section 3.09.02 of the Unified Development Code, in accordance with the Texas Local Government Code, do hereby certify this plat as approved for filing of record with the County Clerk of Williamson County, Texas.

Jerry Millard, Director of Planning

FLOODPLAIN ADMINISTRATOR APPROVAL

Based upon the representations of the Engineer or Surveyor whose seal is affixed hereto, and after review of the plat as represented by the said Engineer or Surveyor, I find that this plat complies with the Williamson County Floodplain Regulations. This certification is made solely upon such representations and should not be relied upon for verifications of the facts alleged. Williamson County disclaims any responsibility to any member of the public for independent verification of the representations, factual or otherwise, contained in this plat and the documents associated within

J. Terron Evertson, PE, DR, CFM

Road Name and 911 Addressing Approval

Williamson County Engineer

Road name and address assignments verified this the_

Williamson County Addressing Coordinator

THE STATE OF TEXAS COUNTY OF WILLIAMSON That I, Nancy E. Rister, Clerk of the County Court, with and for the County and State aforesaid, do

hereby certify that the foregoing instrument in writing, with its certification of authentication, was filed for record in my office on the_____ day of 20_____, at _____ o'clock___M. and duly recorded on the__ A.D., 20_____, at _____ o'clock____M. in the Plat Records of said ___, WITNESS MY HAND AND SEAL of the County Court of said County, at office in Georgetown, Texas, the last date written above.

Nancy E. Rister Clerk, County Court Williamson County, Texas

MINOR PLAT CHV LIBERTY HILL 29 ADDITION LOTS 1, 2, AND LOT 3 RIGHT-OF-WAY RESERVE, BLOCK B

35.842 ACRES SITUATED IN THE, N. SMITHWICK SURVEY, ABSTRACT #590 E.T.J. OF THE CITY OF LIBERTY HILL, WILLIAMSON COUNTY, TEXAS

ENGINEER/PREPARER



5200 State Highway 121 Colleyville, TX 76034 Phone: 817-488-4960

LAND SURVEYOR

BARTON CHAPA SURVEYING 5200 State Highway 121 Colleyville, TX 76034 Phone: 817-864-1957 info@bcsdfw.com TBPLS Firm #10194474

OWNER/DEVELOPER

CHV LIBERTY HILL 29 PROPERTY OWNER LLC 6601-E Hillcrest Avenue Ste. 212, Dallas, TX 75205

JOB NO. 2021.001.197 DRAWN: BCS CHECKED: JHB **TABLE OF REVISIONS** SUMMARY DATE

MP2.2

SITE BENCHMARKS

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIVE ENTRANCE ON THE EAST SIDE OF COUNTY ROAD 260.

ELEVATION = 1004.02' (NAVD '88)

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ELEVATION = 997.92' (NAVD '88)

PROPOSED FULL DEPTH SAWCUT (PROVIDE CLEAN CONSTRUCTION EDGE)

EXISTING CONTOUR — — — 500 — — — — PROPERTY BOUNDARY

TREE REMOVAL

TREES TO REMAIN

NOTE TO CONTRACTOR

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SHEA O. KIRKMAN

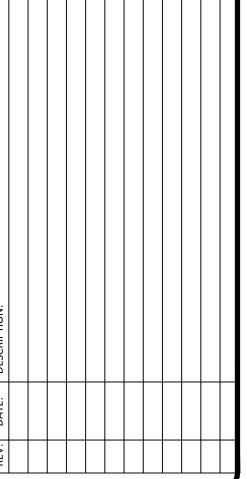
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CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK HILL RANCH
LOTS 1 AND 2, BLOCK A

LOTS 1 AND 2, 1 LOTS 1 AND 2, 1 CITY OF LIBERTY LIBERTY HIL





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

DEMOLITION PLAN I

SHEET:

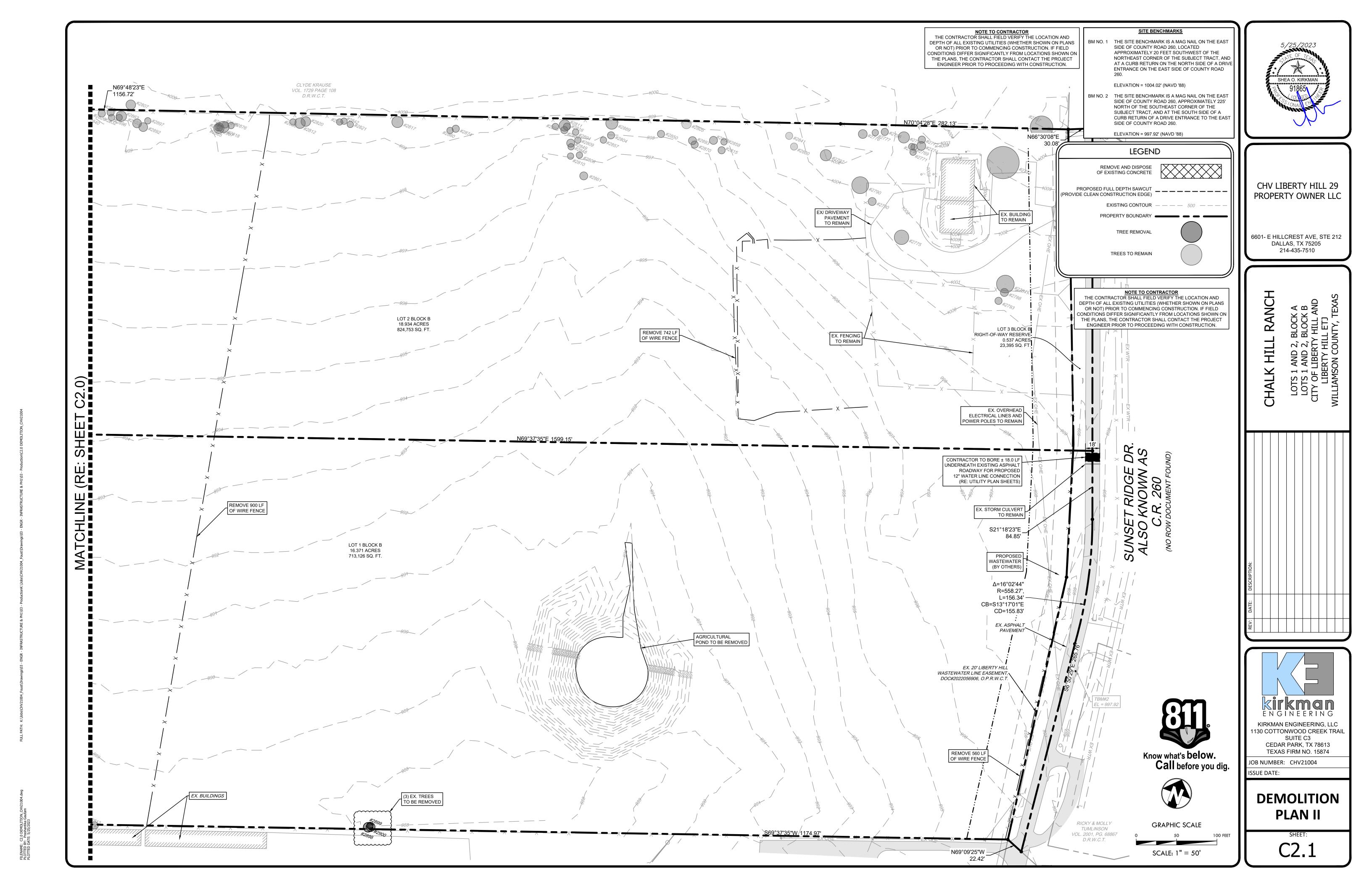
Know what's below.
Call before you dig.

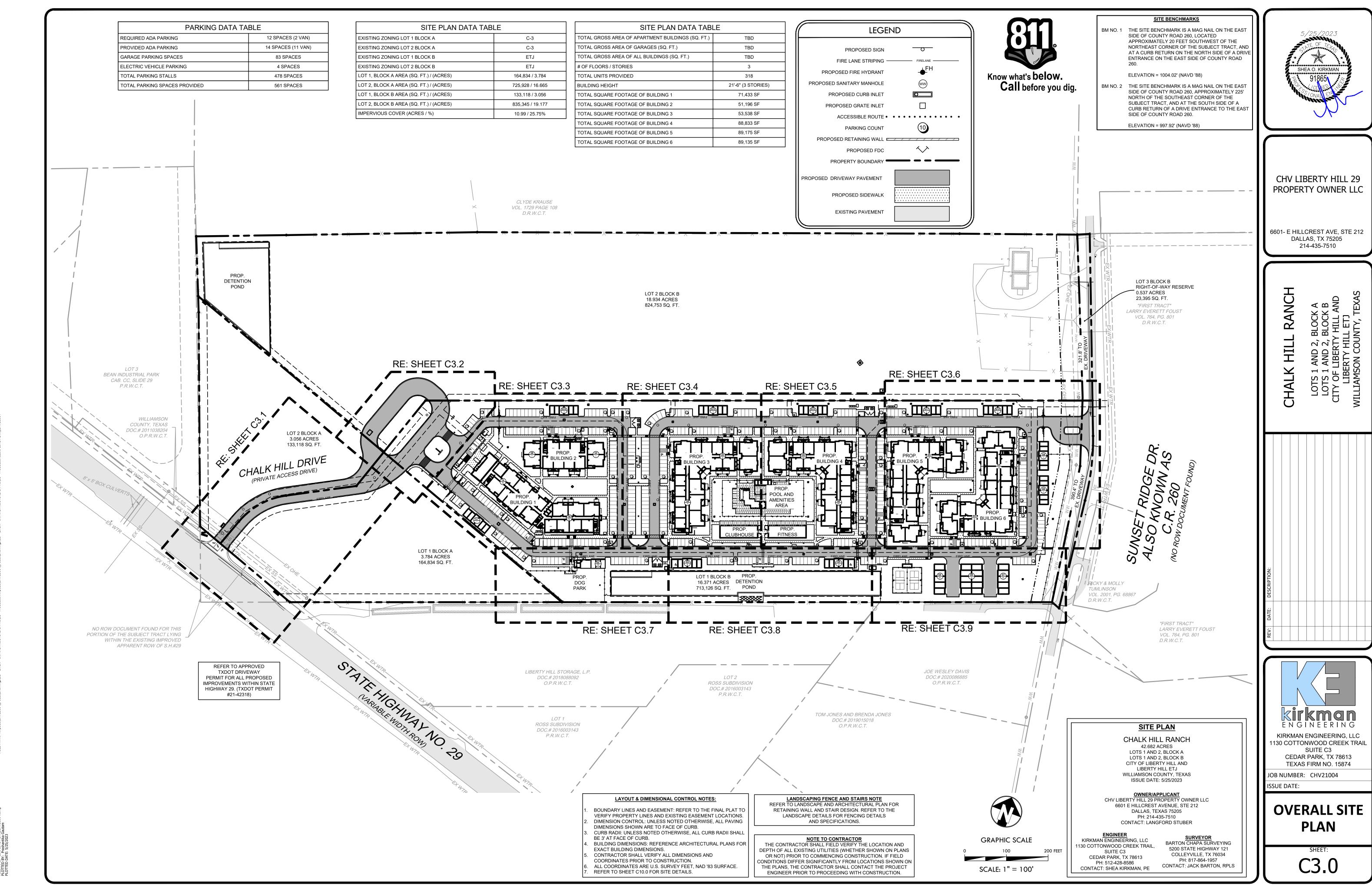
GRAPHIC SCALE

50 100 FEET

SCALE: 1" = 50'

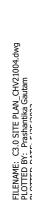
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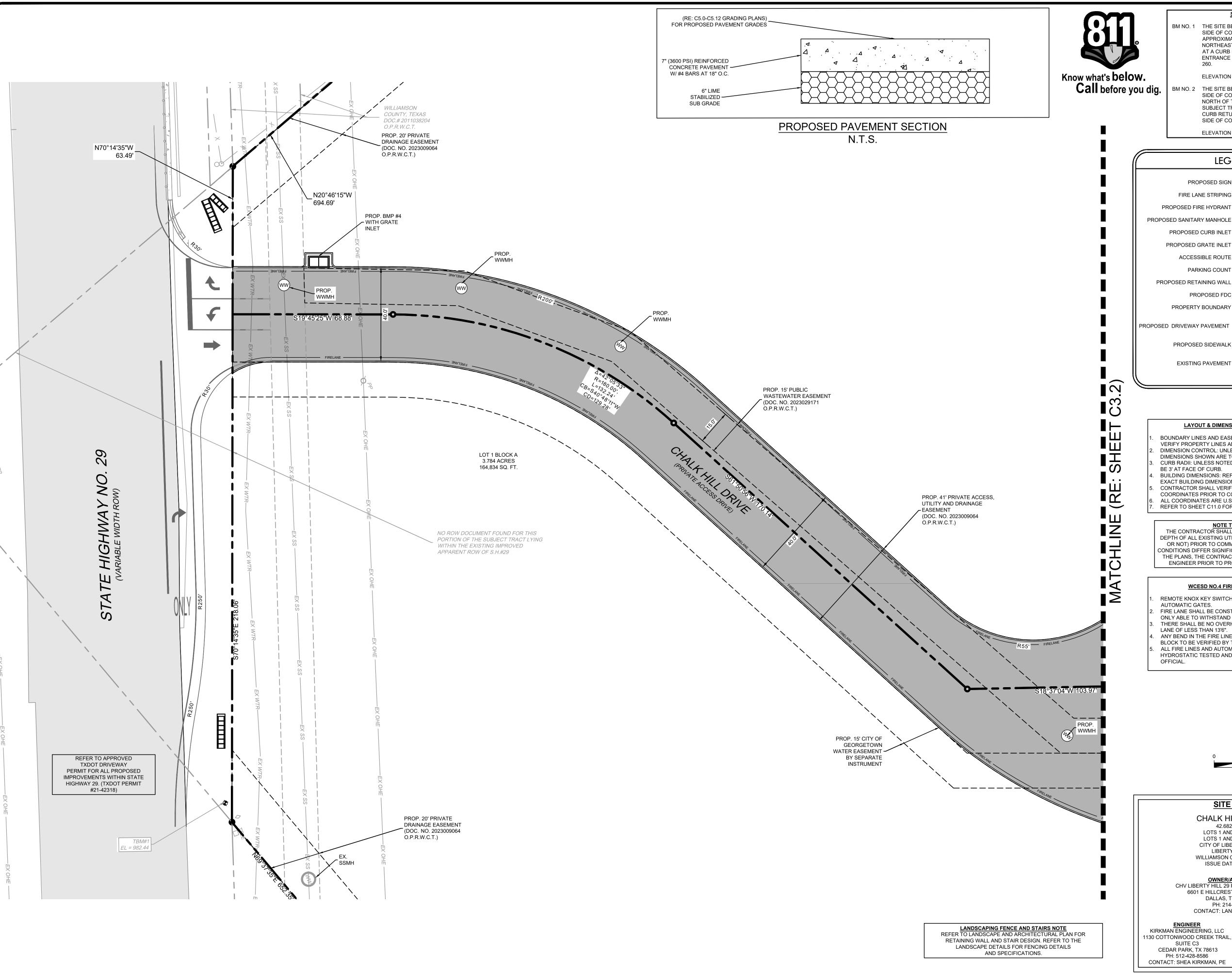




EIII DATH. KAINARACHVOINDA Example Desiminación - ENICD - INEDACTDICTIDE 8. DHIAN2 - Descinction RALAND Francisco (NZ - ENICD - INEDACTDICTIDE 8. DHIAN3 - Descinction CHADINA

FNAME: C3.0 STTF PLAN CHV2





SITE BENCHMARKS

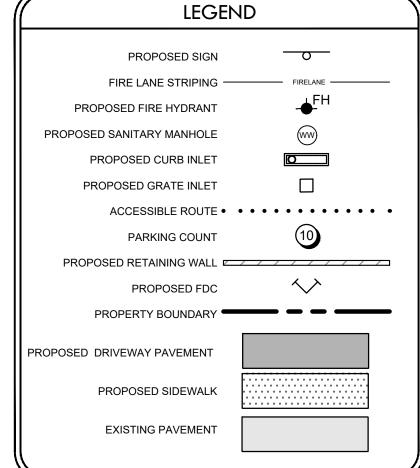
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SCALE: 1" = 20'

CONTACT: JACK BARTON, RPLS

SITE PLAN

CHALK HILL RANCH 42.682 ACRES

LOTS 1 AND 2, BLOCK A LOTS 1 AND 2, BLOCK B CITY OF LIBERTY HILL AND LIBERTY HILL ETJ WILLIAMSON COUNTY, TEXAS

ISSUE DATE: 5/25/2023

CONTACT: LANGFORD STUBER

<u>OWNER/APPLICANT</u> CHV LIBERTY HILL 29 PROPERTY OWNER LLC 6601 E HILLCREST AVENUE, STE 212 DALLAS, TEXAS 75205 PH: 214-435-7510

SURVEYOR
BARTON CHAPA SURVEYING KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL, 5200 STATE HIGHWAY 121 SUITE C3 COLLEYVILLE, TX 76034 CEDAR PARK, TX 78613 PH: 817-864-1957 PH: 512-428-8586

SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

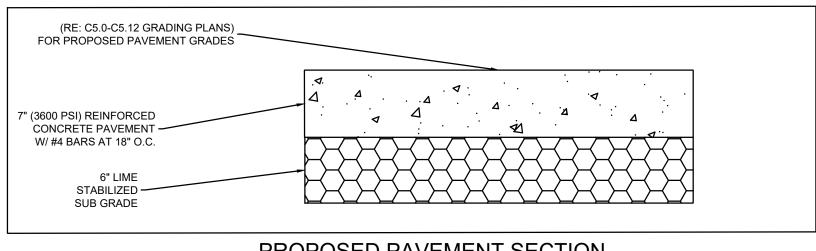
JOB NUMBER: CHV21004

ISSUE DATE:

SITE PLAN I

(RE: C5.0-C5.12 GRADING PLANS) FOR PROPOSED PAVEMENT GRADES 7" (3600 PSI) REINFORCED CONCRETE PAVEMENT -W/ #4 BARS AT 18" O.C. STABILIZED -SUB GRADE

PROPOSED PAVEMENT SECTION N.T.S.





Know what's **below. Call** before you dig.

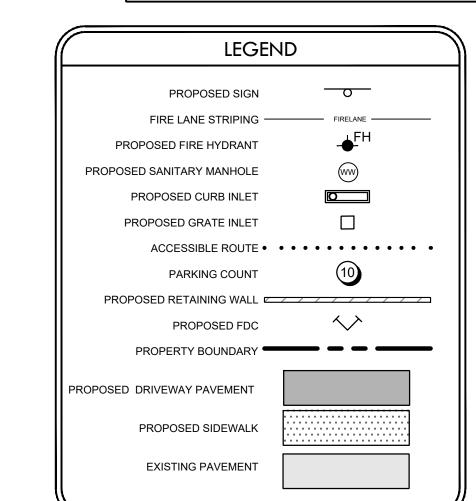
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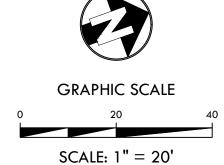
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SITE PLAN

CHALK HILL RANCH

42.682 ACRES LOTS 1 AND 2, BLOCK A LOTS 1 AND 2, BLOCK B CITY OF LIBERTY HILL AND LIBERTY HILL ETJ WILLIAMSON COUNTY, TEXAS ISSUE DATE: 5/25/2023

OWNER/APPLICANT
CHV LIBERTY HILL 29 PROPERTY OWNER LLC 6601 E HILLCREST AVENUE, STE 212 DALLAS, TEXAS 75205 PH: 214-435-7510 CONTACT: LANGFORD STUBER

KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL,

SUITE C3

PH: 512-428-8586

CONTACT: SHEA KIRKMAN, PE

SURVEYOR
BARTON CHAPA SURVEYING 5200 STATE HIGHWAY 121 COLLEYVILLE, TX 76034 CEDAR PARK, TX 78613

PH: 817-864-1957

CONTACT: JACK BARTON, RPLS

SHEA O. KIRKMAN

CHV LIBERTY HILL 29

PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212

DALLAS, TX 75205

214-435-7510

CHALK

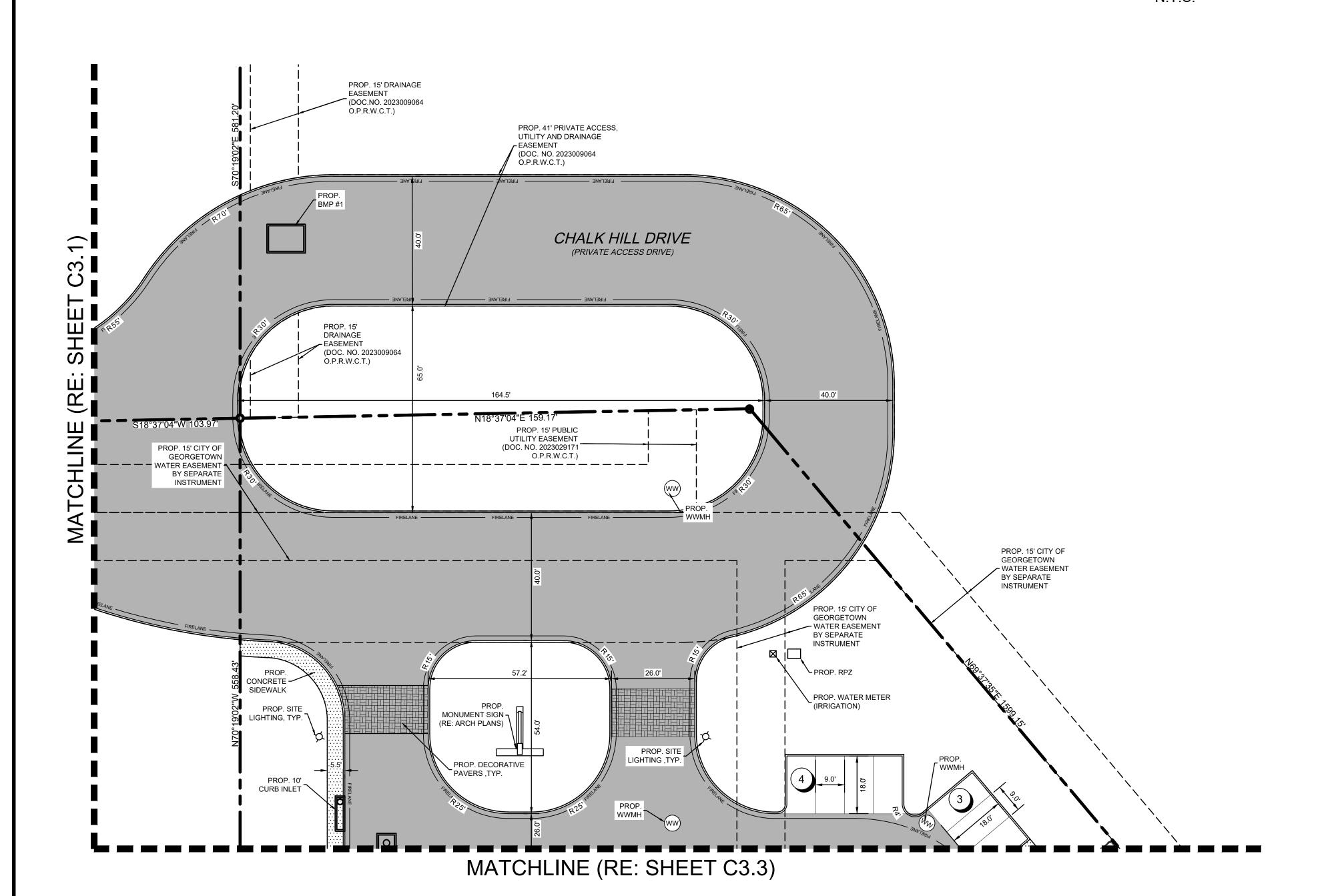
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

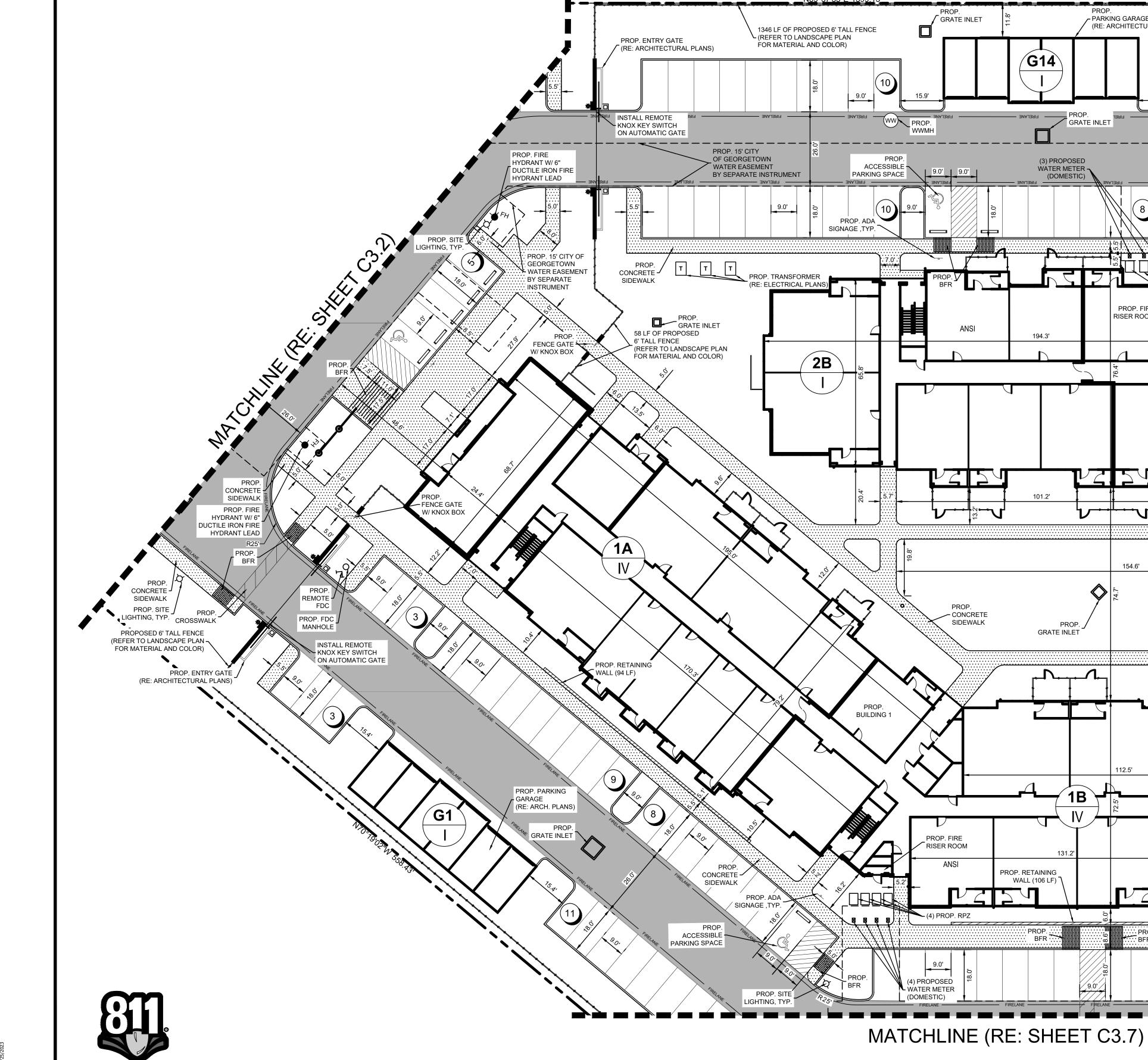
JOB NUMBER: CHV21004

ISSUE DATE:

SITE PLAN II



LANDSCAPING FENCE AND STAIRS NOTE
REFER TO LANDSCAPE AND ARCHITECTURAL PLAN FOR RETAINING WALL AND STAIR DESIGN. REFER TO THE LANDSCAPE DETAILS FOR FENCING DETAILS AND SPECIFICATIONS.



SITE BENCHMARKS

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ELEVATION = 1004.02' (NAVD '88)

OF GEORGETOWN

WATER EASEMENT

PROP FIRE

PROP. SITE

PROP. FDC

MANHOLE

PROP. TRANSFORMER

RE: ELECTRICAL PLANS)

OF GEORGETOWN

WATER EASEMENT

BY SEPARATE INSTRUMENT

PROP.

BFR

HYDRANT W/ 6"

HYDRANT LEAD

LIGHTING, TYP.

PROP.

PROP. STAIR

2A

PROP. **BUILDING 2**

CONCRETE

SIDEWALK

DUCTILE IRON FIRE

BY SEPARATE INSTRUMENT

GRATE INLET

PROP. CROSSWALK

18.0'

18.0'

PROP. STAIR STEPS (3)

18.0'

CROSSWALK

LANDSCAPING FENCE AND STAIRS NOTE
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AND SPECIFICATIONS.

PROP. FIRE

HYDRANT W/6"

HYDRANT LEAD

DUCTILE IRON FIRE

PROP STAIR

PROP.

PROP.

GRATE INLET

_ DETECTA

RPZ

CONCRETE ~ SIDEWALK

> PROP. TRANSFORMER (RE: ELECTRICAL PLANS)

DETECTA RPZ

PROP. RETAINING

WALL (17 LF)

154.6'

~ PARKING GARAGE

GRATE INLET

(3) PROPOSED

WATER METER -

(DOMESTIC)

194.3'

101.2'

PROP.

GRATE INLET

PROP. RETAINING

WALL (106 LF)

∵PROP. <u>.:</u>

PROP.

PROP. FIRE RISER ROOM

(4) PROPOSED

WATER METER

(DOMESTIC)

- CONCRETE

(RE: ARCHITECTURAL PLANS)

PROP. FIRE RISER ROOM L

GRATE INLET

WWMH

G14

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LEGEND

PROPOSED SIGN

FIRE LANE STRIPING -

PROPOSED FIRE HYDRANT

PROPOSED CURB INLET

PROPOSED GRATE INLET

PARKING COUNT

PROPOSED FDC

PROPERTY BOUNDARY

PROPOSED SANITARY MANHOLE



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205

214-435-7510

PROPOSED DRIVEWAY PAVEMENT PROPOSED SIDEWALK

ACCESSIBLE ROUTE • • • • • • • • • • • •

PROPOSED RETAINING WALL

0

10

EXISTING PAVEMENT

LAYOUT & DIMENSIONAL CONTROL NOTES:

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GRAPHIC SCALE

SCALE: 1" = 20'

SITE PLAN

CHALK HILL RANCH 42.682 ACRES

ISSUE DATE: 5/25/2023

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SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

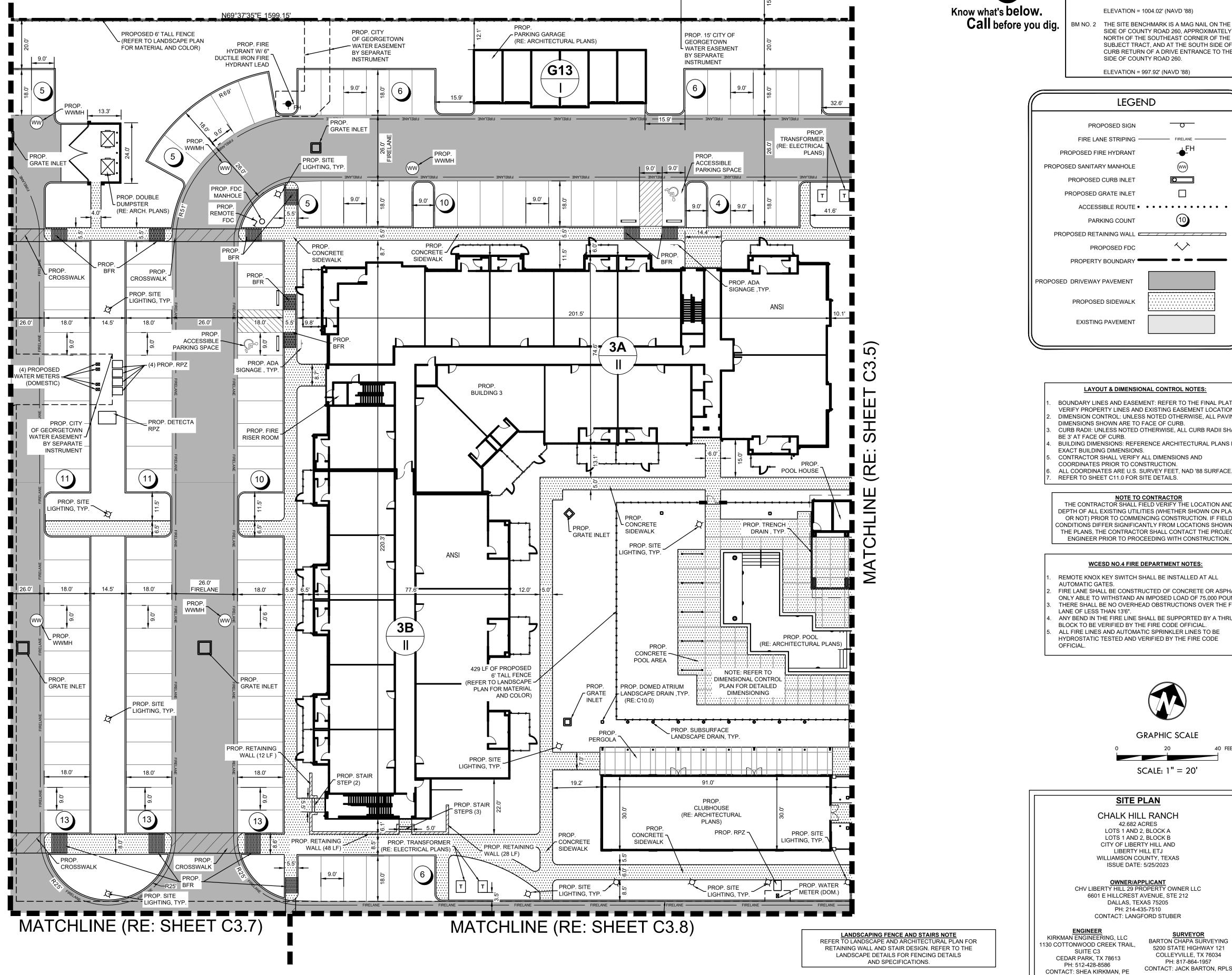
SITE PLAN III

Know what's below.

Call before you dig.

3

SHE



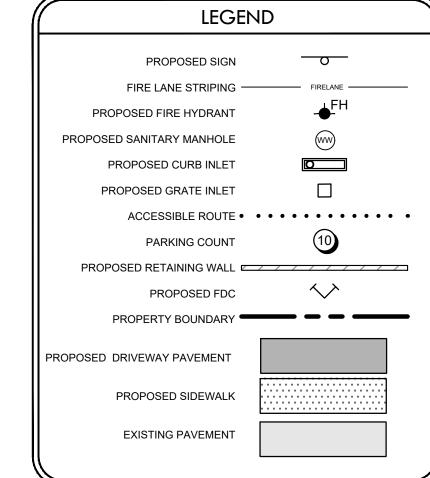
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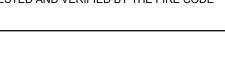
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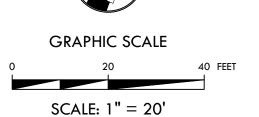
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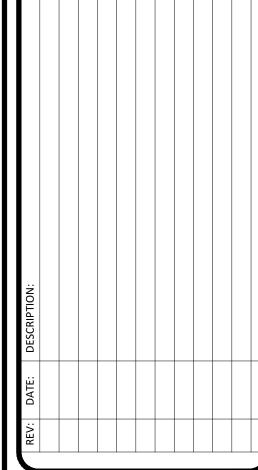
PH: 817-864-1957

CONTACT: JACK BARTON, RPLS



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

SITE PLAN IV



Know what's below. Call before you dig.

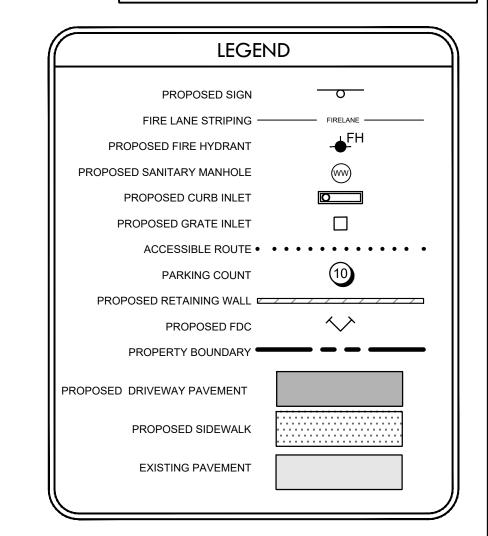
SITE BENCHMARKS

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIV ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

ELEVATION = 997.92' (NAVD '88)



LAYOUT & DIMENSIONAL CONTROL NOTES:

BOUNDARY LINES AND EASEMENT: REFER TO THE FINAL PLAT TO VERIFY PROPERTY LINES AND EXISTING EASEMENT LOCATIONS. DIMENSION CONTROL: UNLESS NOTED OTHERWISE, ALL PAVING DIMENSIONS SHOWN ARE TO FACE OF CURB.

CURB RADII: UNLESS NOTED OTHERWISE, ALL CURB RADII SHALL BE 3' AT FACE OF CURB. BUILDING DIMENSIONS: REFERENCE ARCHITECTURAL PLANS FOR

EXACT BUILDING DIMENSIONS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND COORDINATES PRIOR TO CONSTRUCTION.

ALL COORDINATES ARE U.S. SURVEY FEET, NAD '88 SURFACE. REFER TO SHEET C11.0 FOR SITE DETAILS.

<u>NOTE TO CONTRACTOR</u>
THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON

THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT

ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

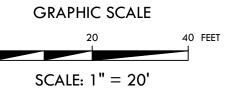
WCESD NO.4 FIRE DEPARTMENT NOTES:

REMOTE KNOX KEY SWITCH SHALL BE INSTALLED AT ALL AUTOMATIC GATES.

FIRE LANE SHALL BE CONSTRUCTED OF CONCRETE OR ASPHALT ONLY ABLE TO WITHSTAND AN IMPOSED LOAD OF 75,000 POUNDS. THERE SHALL BE NO OVERHEAD OBSTRUCTIONS OVER THE FIRE LANE OF LESS THAN 13'6". ANY BEND IN THE FIRE LINE SHALL BE SUPPORTED BY A THRUST

BLOCK TO BE VERIFIED BY THE FIRE CODE OFFICIAL. ALL FIRE LINES AND AUTOMATIC SPRINKLER LINES TO BE HYDROSTATIC TESTED AND VERIFIED BY THE FIRE CODE





SITE PLAN

CHALK HILL RANCH

42.682 ACRES LOTS 1 AND 2, BLOCK A LOTS 1 AND 2, BLOCK B CITY OF LIBERTY HILL AND LIBERTY HILL ETJ WILLIAMSON COUNTY, TEXAS ISSUE DATE: 5/25/2023

<u>OWNER/APPLICANT</u> CHV LIBERTY HILL 29 PROPERTY OWNER LLC 6601 E HILLCREST AVENUE, STE 212 DALLAS, TEXAS 75205 PH: 214-435-7510

CONTACT: LANGFORD STUBER

CEDAR PARK, TX 78613

PH: 512-428-8586

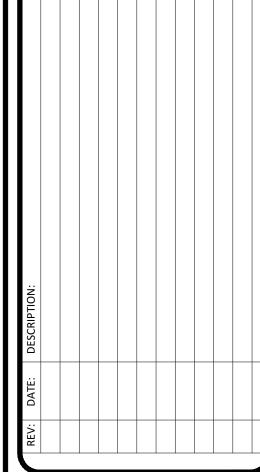
SURVEYOR
BARTON CHAPA SURVEYING KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL, SUITE C3

5200 STATE HIGHWAY 121 COLLEYVILLE, TX 76034 PH: 817-864-1957 CONTACT: JACK BARTON, RPLS CONTACT: SHEA KIRKMAN, PE

SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510





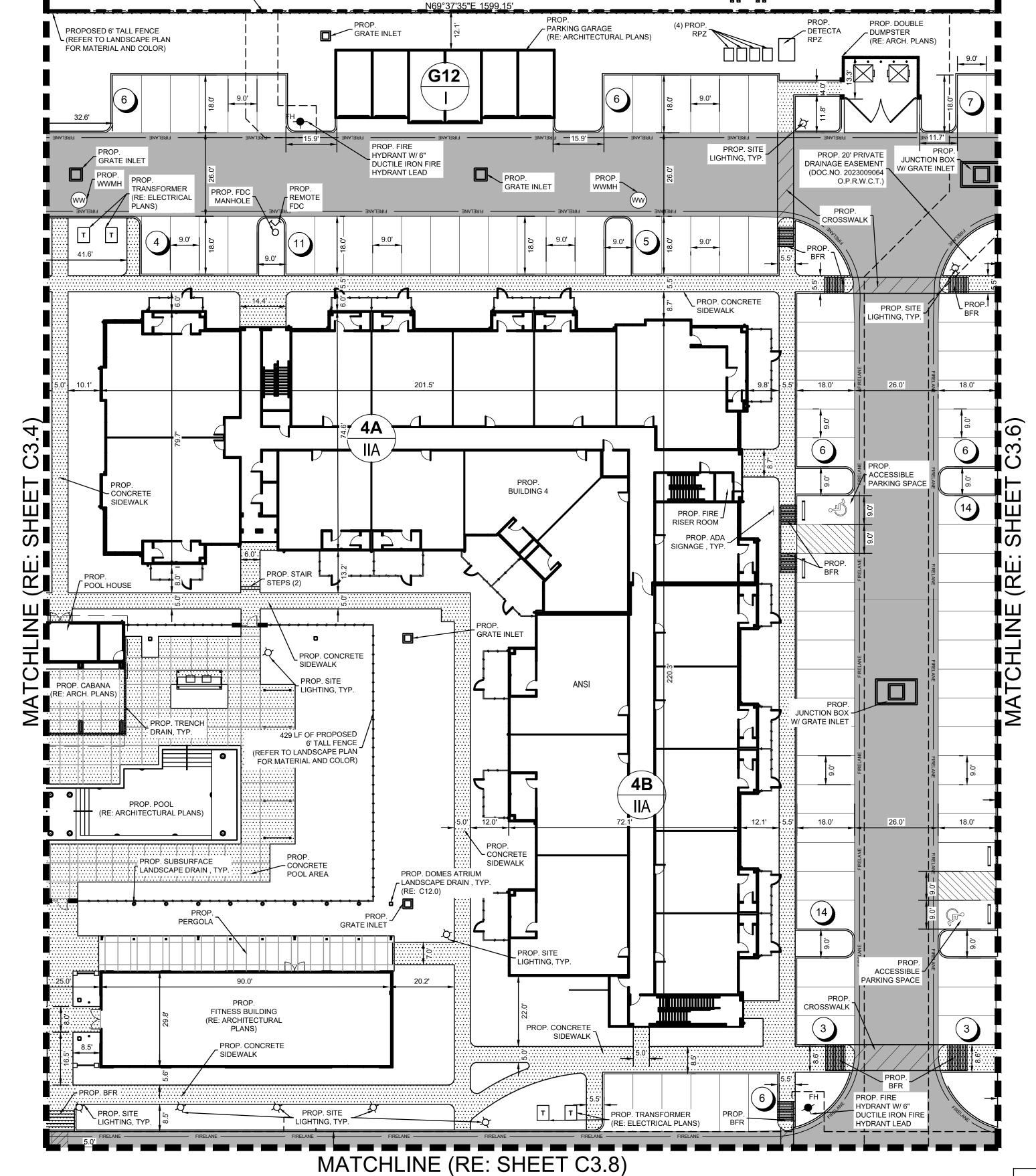
1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

SITE PLAN V



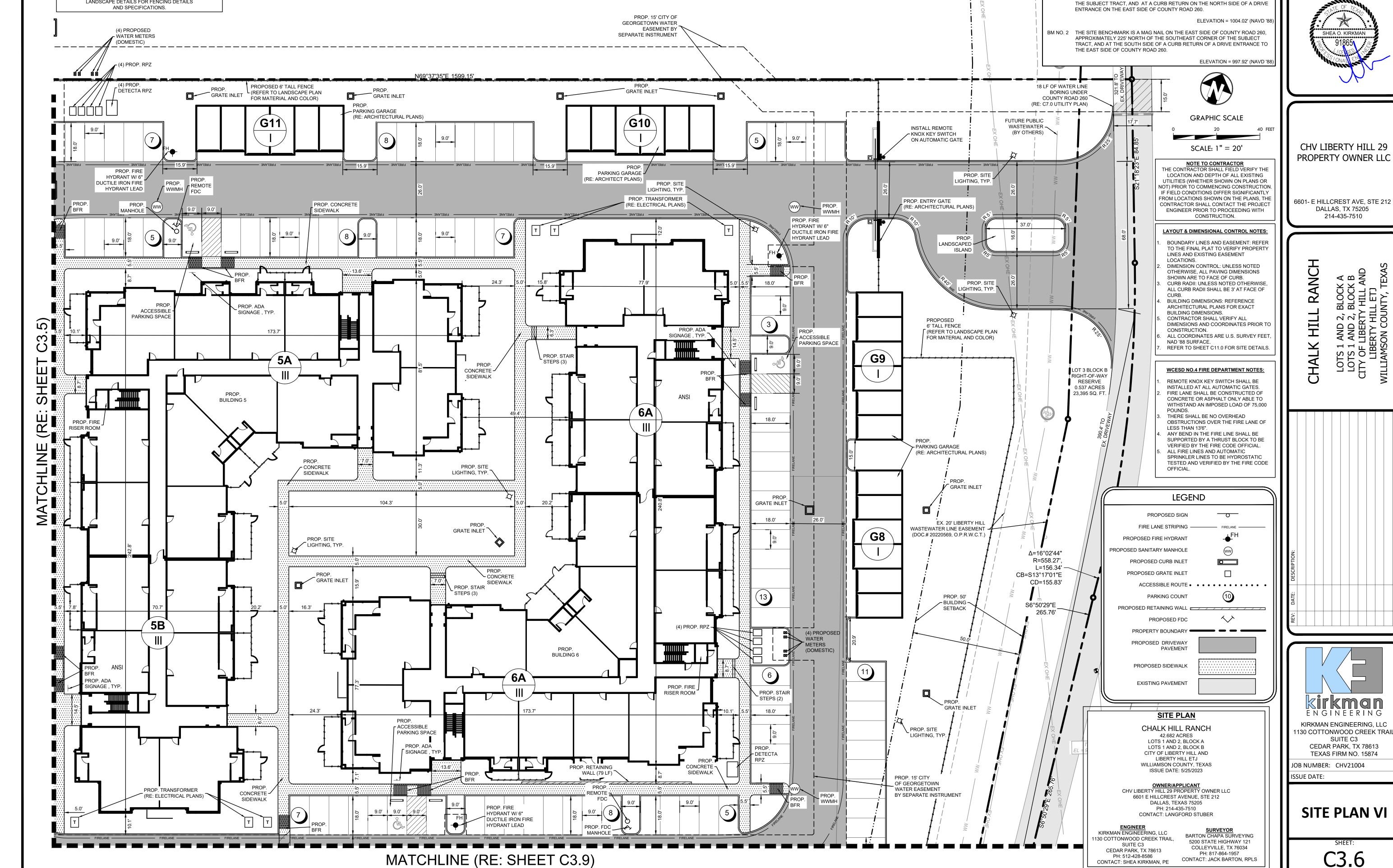
PROP. 15' CITY OF GEORGETOWN

WATER EASEMENT BY SEPARATE

(4) PROPOSED

WATER METERS

LANDSCAPING FENCE AND STAIRS NOTE
REFER TO LANDSCAPE AND ARCHITECTURAL PLAN FOR RETAINING WALL AND STAIR DESIGN. REFER TO THE LANDSCAPE DETAILS FOR FENCING DETAILS



SITE BENCHMARKS

LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260,

RETAINING WALL AND STAIR DESIGN, REFER TO THE

LANDSCAPE DETAILS FOR FENCING DETAILS

Know what's below. Call before you dig.

MATCHLINE (RE: SHEET C3.4)

PROP.

BMP #2

PROP. DOUBLE

RE: ARCH. PLANS)

(0/0 p d 0/ 0/0 p d 0/

648.0'

MAINTENANCE

EX. BUILDING

RAMP

DUMPSTER

PROP. PRIVATE DRAINAGE

EASEMENT

(DOC. NO. 2023009064

O.P.R.W.C.T.)

MATCHLINE (RE: SHEET C3.3)

27.0'

EX. BUILDING

PROP.

ACCESSIBLE

PARKING SPACE

PROP. SITE

PROP. ADA

SIGNAGE, TYP.

130 LF OF PROPOSED

6' TALL FENCE (REFER TO LANDSCAPE PLAN

FOR MATERIAL AND COLOR)

LIGHTING, TYP.

PROP. CONCRETE

567 LF OF PROPOSED 6' TALL FENCE

PROP. 20' PRIVATE

O.P.R.W.C.T.)

N69°37'35"E_652.35'

DRAINAGE EASEMENT

(DOC. NO. 2023009064

(REFER TO LANDSCAPE PLAN

FOR MATERIAL AND COLOR)

EX. BUILDING

PROP. WATER

MAINTENANCE J

PROP. DOG PARK

PROP. TRANSFORMER

G2

PARKING GARAGE -

PROP. SITE

LIBERTY HILL STORAGE, L.P. DOC.# 2018088092 O.P.R.W.C.T.

LIGHTING, TYP.

PROPOSED 6' TALL FENCE

(REFER TO LANDSCAPE PLAN

FOR MATERIAL AND COLOR)

9.0'

13.3'

/ METER (DOM.) (RE: ELECTRICAL PLANS)

PROP.

GRATE INLET

BUILDING (RE: ARCHITECTURAL PLANS)

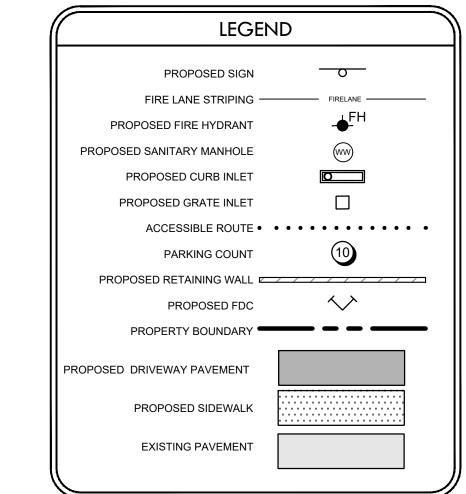
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ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

ELEVATION = 997.92' (NAVD '88)



LAYOUT & DIMENSIONAL CONTROL NOTES:

BOUNDARY LINES AND EASEMENT: REFER TO THE FINAL PLAT TO VERIFY PROPERTY LINES AND EXISTING EASEMENT LOCATIONS. DIMENSION CONTROL: UNLESS NOTED OTHERWISE, ALL PAVING DIMENSIONS SHOWN ARE TO FACE OF CURB.

CURB RADII: UNLESS NOTED OTHERWISE, ALL CURB RADII SHALL BE 3' AT FACE OF CURB. BUILDING DIMENSIONS: REFERENCE ARCHITECTURAL PLANS FOR

EXACT BUILDING DIMENSIONS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND COORDINATES PRIOR TO CONSTRUCTION.

ALL COORDINATES ARE U.S. SURVEY FEET, NAD '88 SURFACE. REFER TO SHEET C11.0 FOR SITE DETAILS.

<u>NOTE TO CONTRACTOR</u>
THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS

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WCESD NO.4 FIRE DEPARTMENT NOTES:

REMOTE KNOX KEY SWITCH SHALL BE INSTALLED AT ALL AUTOMATIC GATES.

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ANY BEND IN THE FIRE LINE SHALL BE SUPPORTED BY A THRUST BLOCK TO BE VERIFIED BY THE FIRE CODE OFFICIAL. ALL FIRE LINES AND AUTOMATIC SPRINKLER LINES TO BE HYDROSTATIC TESTED AND VERIFIED BY THE FIRE CODE OFFICIAL.



SCALE: 1" = 20'

SITE PLAN

CHALK HILL RANCH

42.682 ACRES LOTS 1 AND 2, BLOCK A LOTS 1 AND 2, BLOCK B CITY OF LIBERTY HILL AND LIBERTY HILL ETJ WILLIAMSON COUNTY, TEXAS ISSUE DATE: 5/25/2023

<u>OWNER/APPLICANT</u> CHV LIBERTY HILL 29 PROPERTY OWNER LLC

SUITE C3

CEDAR PARK, TX 78613

CONTACT: SHEA KIRKMAN, PE

PH: 512-428-8586

6601 E HILLCREST AVENUE, STE 212 DALLAS, TEXAS 75205 PH: 214-435-7510 CONTACT: LANGFORD STUBER

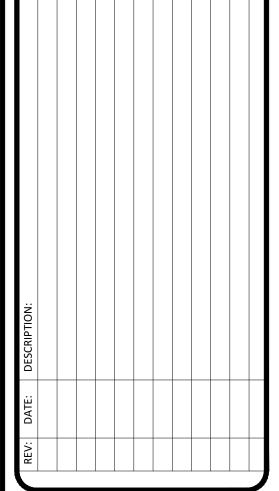
SURVEYOR
BARTON CHAPA SURVEYING KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL,

5200 STATE HIGHWAY 121 COLLEYVILLE, TX 76034 PH: 817-864-1957 CONTACT: JACK BARTON, RPLS SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK





SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

SITE PLAN VII

LANDSCAPING FENCE AND STAIRS NOTE
REFER TO LANDSCAPE AND ARCHITECTURAL PLAN FOR RETAINING WALL AND STAIR DESIGN. REFER TO THE LANDSCAPE DETAILS FOR FENCING DETAILS AND SPECIFICATIONS.

APPROXIMATELY 20 FEET SOUTHWEST OF THE

NORTHEAST CORNER OF THE SUBJECT TRACT, AND

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED

LEGEND

PROPOSED SIGN

 $\overline{}$

CHV LIBERTY HILL 29

6601- E HILLCREST AVE, STE 212

KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

SITE PLAN VIII

Know what's **below. Call** before you dig.

SITE BENCHMARKS

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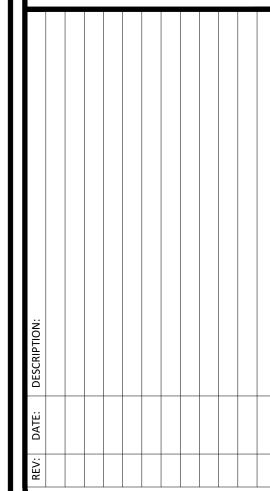
ELEVATION = 997.92' (NAVD '88)



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK





1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

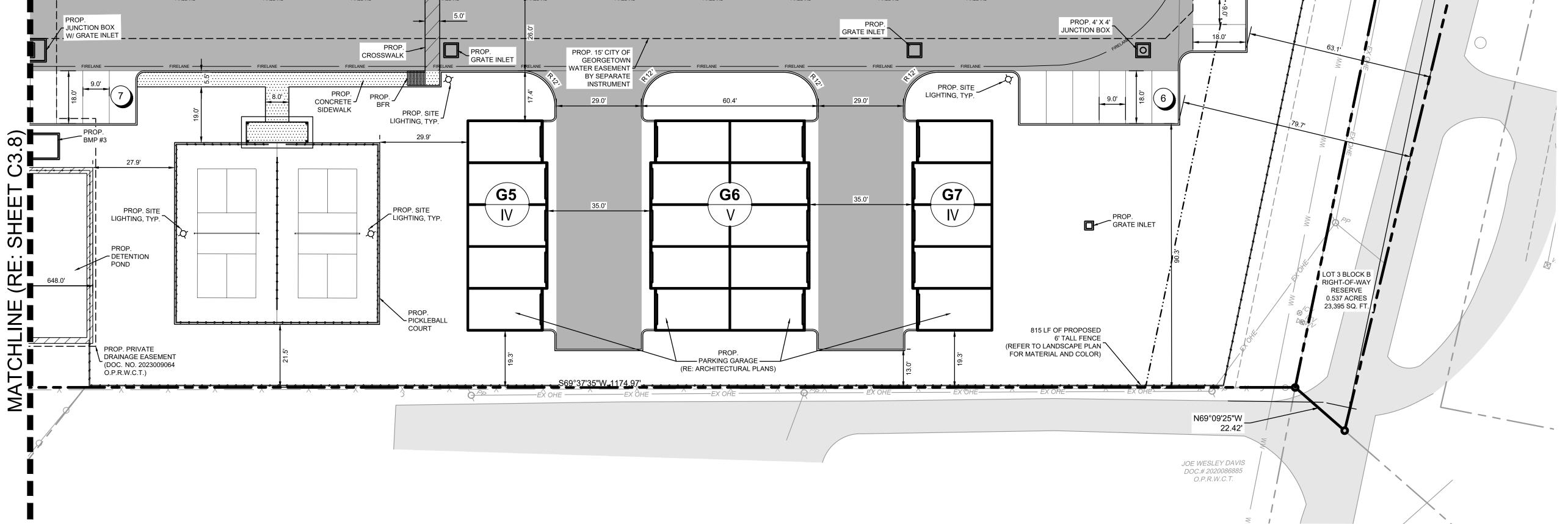
TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

SITE PLAN IX

MATCHLINE (RE: SHEET C3.6)



RETAINING WALL AND STAIR DESIGN. REFER TO THE LANDSCAPE DETAILS FOR FENCING DETAILS
AND SPECIFICATIONS.

NOTE TO CONTRACTOR

THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

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BUILDING DIMENSIONS: REFERENCE ARCHITECTURAL PLANS FOR EXACT BUILDING DIMENSIONS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND

LAYOUT & DIMENSIONAL CONTROL NOTES:

BOUNDARY LINES AND EASEMENT: REFER TO THE FINAL PLAT TO VERIFY PROPERTY LINES AND EXISTING EASEMENT LOCATIONS. DIMENSION CONTROL: UNLESS NOTED OTHERWISE, ALL PAVING DIMENSIONS SHOWN ARE TO FACE OF CURB. CURB RADII: UNLESS NOTED OTHERWISE, ALL CURB RADII SHALL

COORDINATES PRIOR TO CONSTRUCTION.

PROPERTY BOUNDARY PROPOSED DRIVEWAY PAVEMENT PROPOSED SIDEWALK EXISTING PAVEMENT

LEGEND

0

ACCESSIBLE ROUTE • • • • • • • • • • • •

PROPOSED RETAINING WALL

PROPOSED SIGN

FIRE LANE STRIPING -

PROPOSED FIRE HYDRANT

PROPOSED CURB INLET

PROPOSED GRATE INLET

PARKING COUNT

PROPOSED FDC

PROPOSED SANITARY MANHOLE

SITE PLAN CHALK HILL RANCH 42.682 ACRES LOTS 1 AND 2, BLOCK A LOTS 1 AND 2, BLOCK B CITY OF LIBERTY HILL AND LIBERTY HILL ETJ WILLIAMSON COUNTY, TEXAS ISSUE DATE: 5/25/2023 <u>OWNER/APPLICANT</u> CHV LIBERTY HILL 29 PROPERTY OWNER LLC DALLAS, TEXAS 75205

6601 E HILLCREST AVENUE, STE 212 PH: 214-435-7510 CONTACT: LANGFORD STUBER

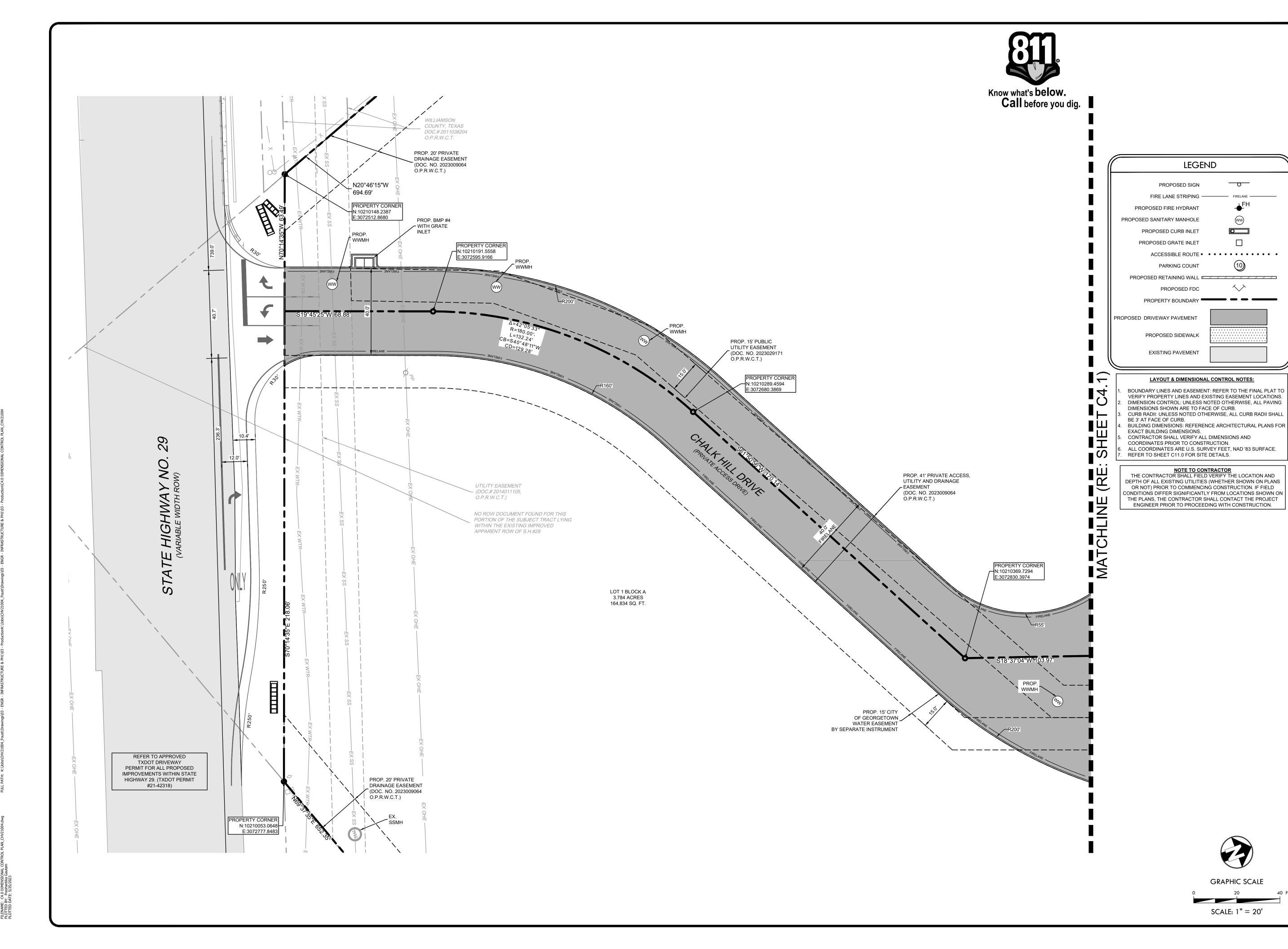
SURVEYOR
BARTON CHAPA SURVEYING KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL, 5200 STATE HIGHWAY 121 SUITE C3 COLLEYVILLE, TX 76034 CEDAR PARK, TX 78613 PH: 817-864-1957 PH: 512-428-8586 CONTACT: JACK BARTON, RPLS

CONTACT: SHEA KIRKMAN, PE

BE 3' AT FACE OF CURB.

ALL COORDINATES ARE U.S. SURVEY FEET, NAD '88 SURFACE. REFER TO SHEET C11.0 FOR SITE DETAILS.

SCALE: 1" = 20'





CHV LIBERTY HILL 29 PROPERTY OWNER LLC

FIRELANE -

ww

0

10)

 $\langle \rangle$

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK

KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

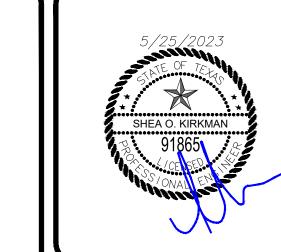
JOB NUMBER: CHV21004

ISSUE DATE:

DIMENSIONAL CONTROL PLAN I

C4.0

GRAPHIC SCALE SCALE: 1" = 20'



CHV LIBERTY HILL 29

PROPERTY OWNER LLC

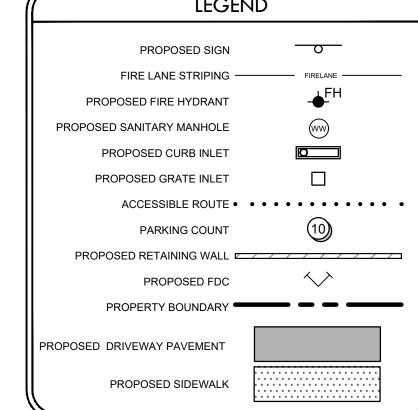
6601- E HILLCREST AVE, STE 212

DALLAS, TX 75205

214-435-7510

RANCH

CHALK



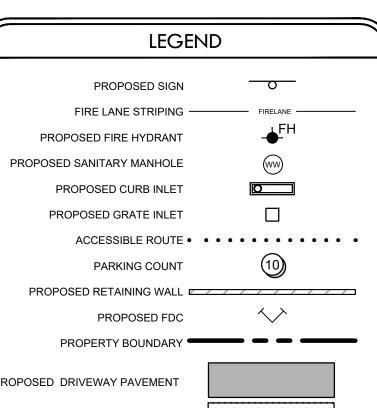
LAYOUT & DIMENSIONAL CONTROL NOTES:

DIMENSION CONTROL: UNLESS NOTED OTHERWISE, ALL PAVING DIMENSIONS SHOWN ARE TO FACE OF CURB. CURB RADII: UNLESS NOTED OTHERWISE, ALL CURB RADII SHALL

EXACT BUILDING DIMENSIONS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND

COORDINATES PRIOR TO CONSTRUCTION.
ALL COORDINATES ARE U.S. SURVEY FEET, NAD '83 SURFACE. REFER TO SHEET C11.0 FOR SITE DETAILS.

DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.



BOUNDARY LINES AND EASEMENT: REFER TO THE FINAL PLAT TO VERIFY PROPERTY LINES AND EXISTING EASEMENT LOCATIONS. BE 3' AT FACE OF CURB. BUILDING DIMENSIONS: REFERENCE ARCHITECTURAL PLANS FOR

NOTE TO CONTRACTOR
THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

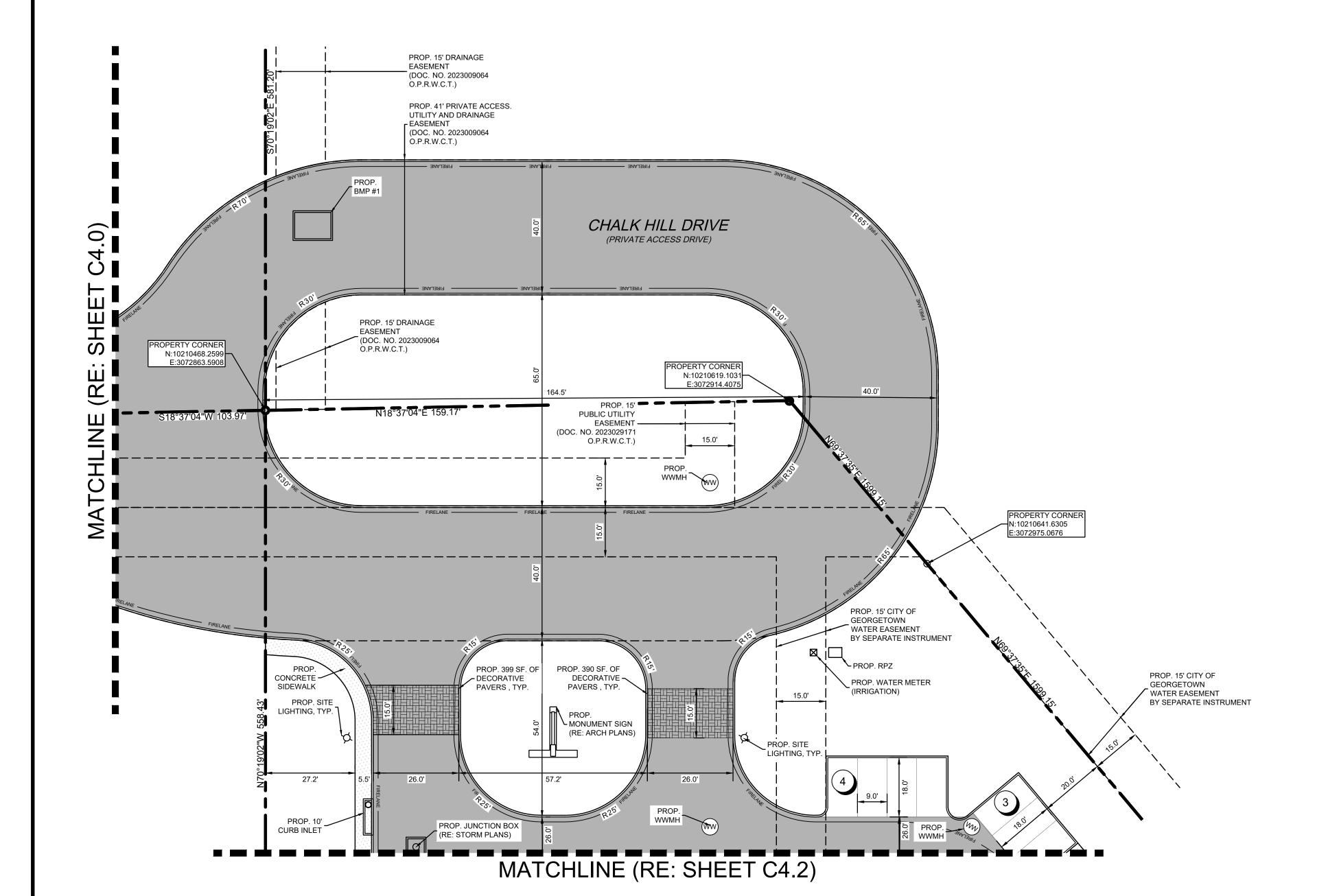
ISSUE DATE:

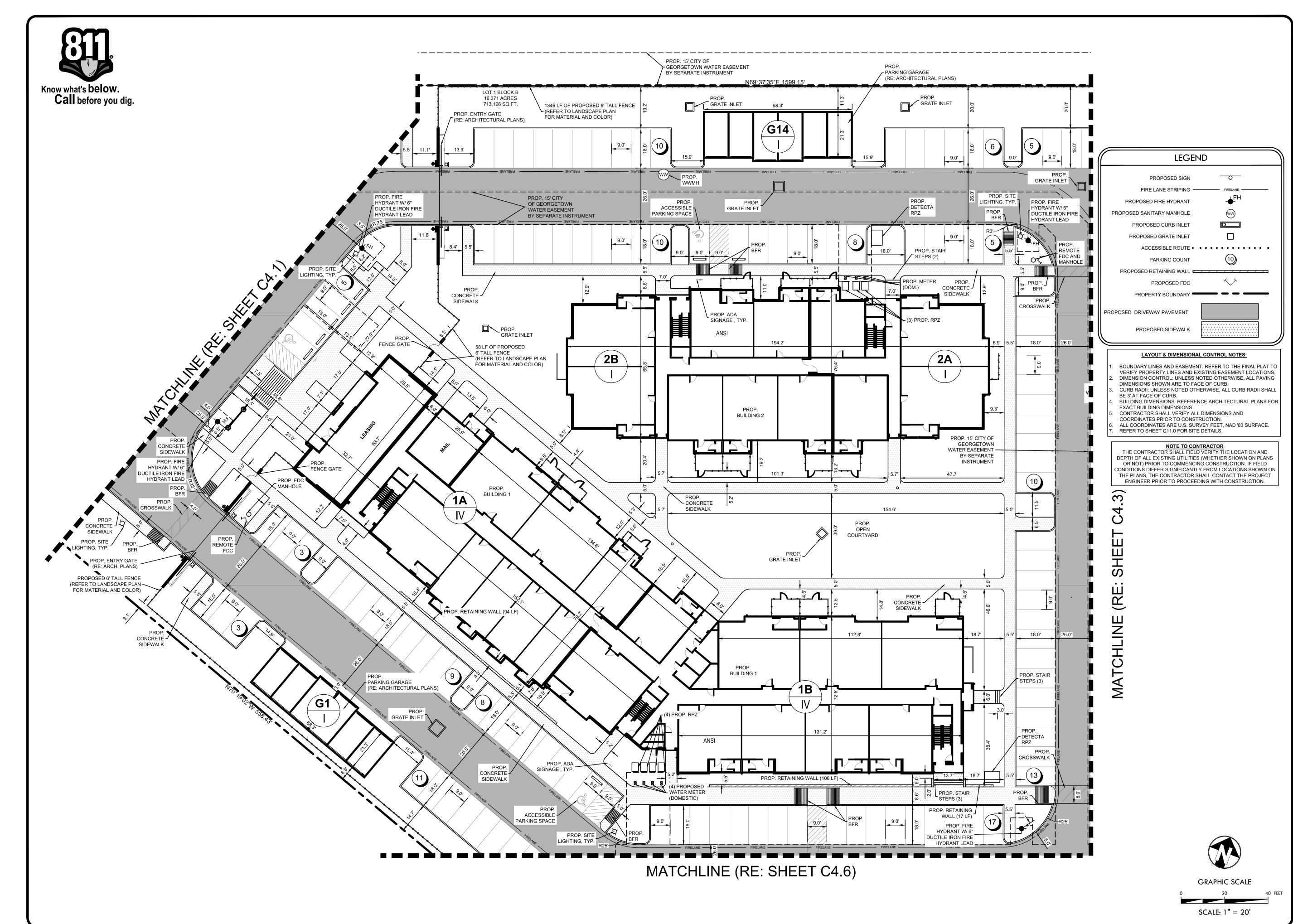
GRAPHIC SCALE

SCALE: 1" = 20'

DIMENSIONAL CONTROL **PLAN II**

C4.1







CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205

214-435-7510

CHALK



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL

SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

DIMENSIONAL CONTROL PLAN III

C4.2

PROP. FIRE

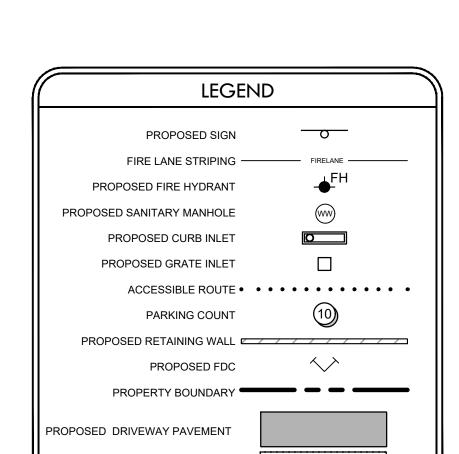
PROPOSED 6' TALL FENCE

PROP. 15' CITY OF GEORGETOWN - WATER EASEMENT

PARKING GARAGE ~

BY SEPARATE INSTRUMENT





LAYOUT & DIMENSIONAL CONTROL NOTES:

- BOUNDARY LINES AND EASEMENT: REFER TO THE FINAL PLAT TO VERIFY PROPERTY LINES AND EXISTING EASEMENT LOCATIONS. DIMENSION CONTROL: UNLESS NOTED OTHERWISE, ALL PAVING DIMENSIONS SHOWN ARE TO FACE OF CURB. CURB RADII: UNLESS NOTED OTHERWISE, ALL CURB RADII SHALL
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ALL COORDINATES ARE U.S. SURVEY FEET, NAD '83 SURFACE.

EXACT BUILDING DIMENSIONS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND COORDINATES PRIOR TO CONSTRUCTION.

PROPOSED SIDEWALK

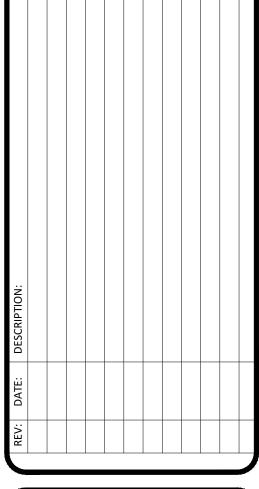
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CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510





SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

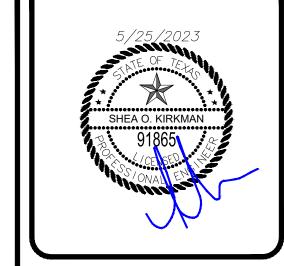
ISSUE DATE:

DIMENSIONAL CONTROL **PLAN IV**

C4.3

GRAPHIC SCALE SCALE: 1" = 20'





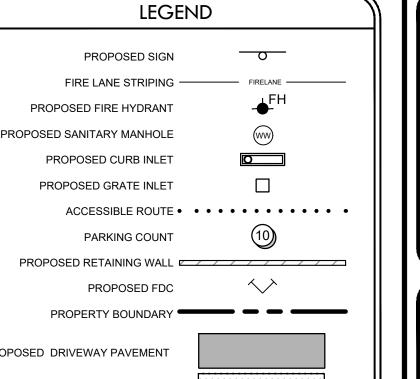
CHV LIBERTY HILL 29

PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212

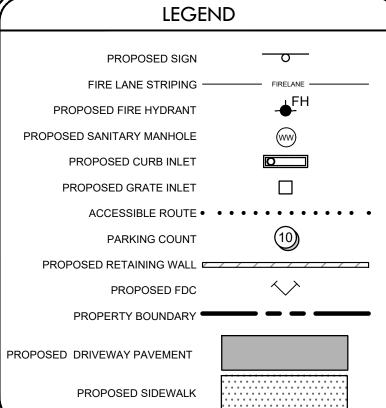
DALLAS, TX 75205

214-435-7510



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- DIMENSIONS SHOWN ARE TO FACE OF CURB.
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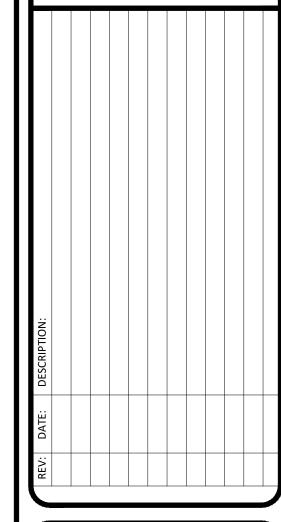
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LAYOUT & DIMENSIONAL CONTROL NOTES:

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- CURB RADII: UNLESS NOTED OTHERWISE, ALL CURB RADII SHALL
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- EXACT BUILDING DIMENSIONS.
- COORDINATES PRIOR TO CONSTRUCTION.
- REFER TO SHEET C11.0 FOR SITE DETAILS.

ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.



CHALK



1130 COTTONWOOD CREEK TRAIL SUITE C3

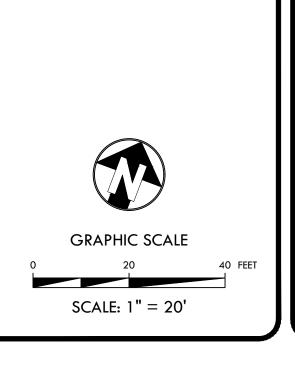
CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

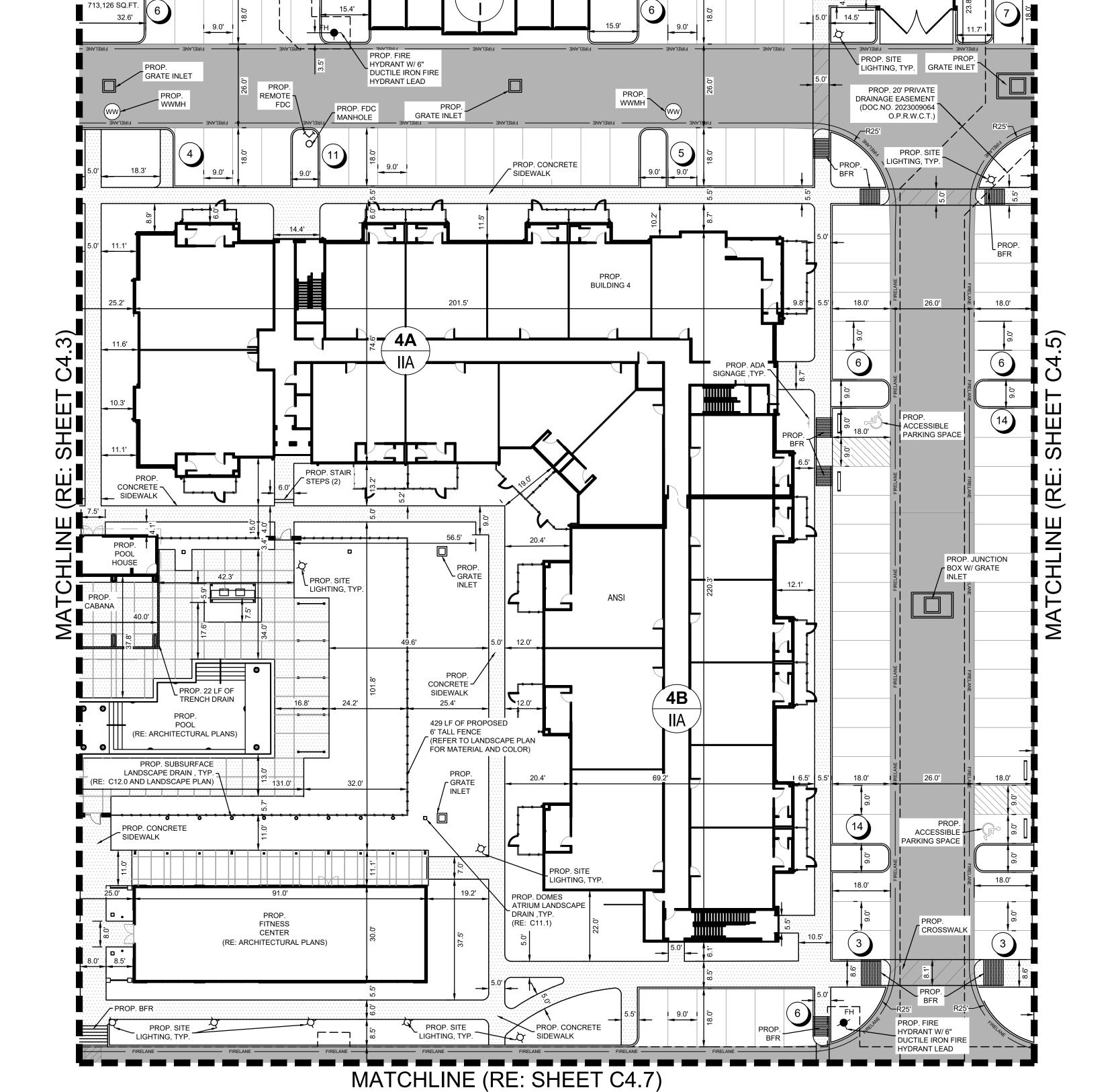
JOB NUMBER: CHV21004

ISSUE DATE:

DIMENSIONAL CONTROL PLAN V

C4.4





PROP. 15' CITY OF

- WATER EASEMENT

GEORGETOWN

BY SEPARATE

INSTRUMENT

GRATE INLET

PARKING GARAGE -

(RE: ARCHITECTURAL PLANS)

G12

GRATE INLET

PROPOSED 6' TALL FENCE

16.371 ACRES

(REFER TO LANDSCAPE PLAN FOR MATERIAL AND COLOR)

(4) PROPOSED

(DOMESTIC)

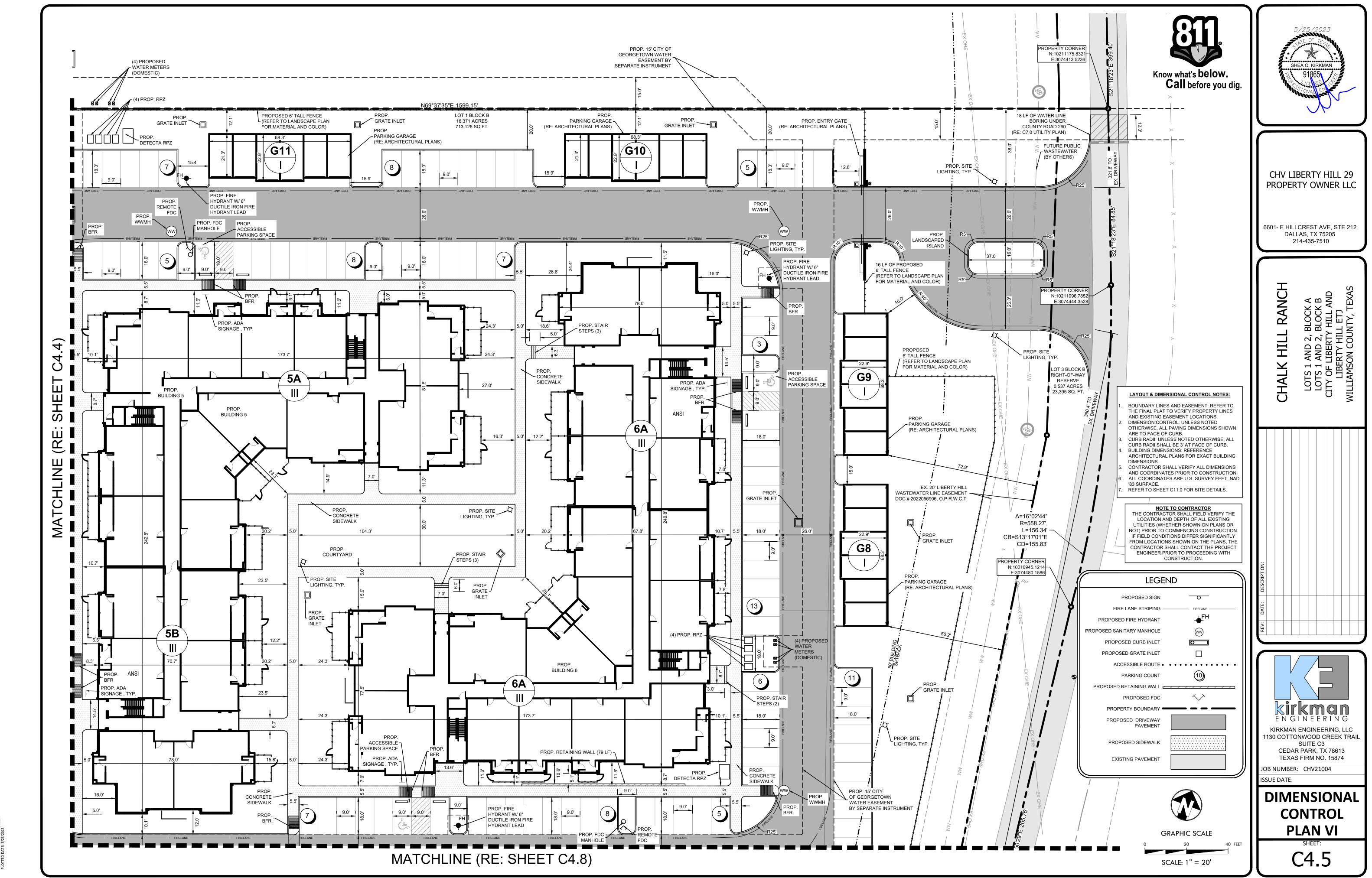
- WATER METERS

DETECTA RPZ

PROP. DOUBLE

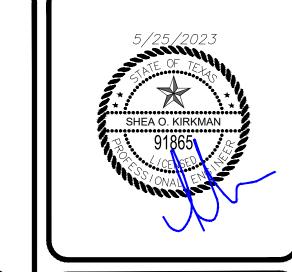
(RE: ARCH. PLANS)

_ DUMPSTER



NITROL PLAN_CHV21004.dwg FULL PATH: K:\Jobs\CHV21004_Foust\Drawings\03 - ENGR - INFRASTRUCTURE & PH1\03 - ProductionK:\Jobs\CHV21004_Foust\Drawings\03 - ENGR - INFRASTRUCTURE & PH1\03 - Production\C4.0 DIMENSIONAL CONTROL PLAN_CHV21004

AME: C4.0 DIMENSIONAL CONTROL PLAN_CHV21004.dwg FULL PA



CHV LIBERTY HILL 29

PROPERTY OWNER LLC

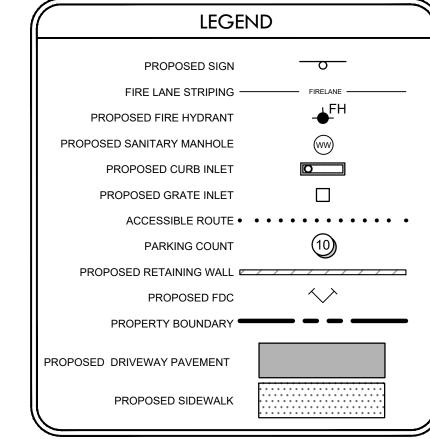
6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205

214-435-7510

RANCH

로

CHALK



LAYOUT & DIMENSIONAL CONTROL NOTES:

BOUNDARY LINES AND EASEMENT: REFER TO THE FINAL PLAT TO VERIFY PROPERTY LINES AND EXISTING EASEMENT LOCATIONS. DIMENSION CONTROL: UNLESS NOTED OTHERWISE, ALL PAVING

CURB RADII: UNLESS NOTED OTHERWISE, ALL CURB RADII SHALL BE 3' AT FACE OF CURB.

BUILDING DIMENSIONS: REFERENCE ARCHITECTURAL PLANS FOR EXACT BUILDING DIMENSIONS.

ALL COORDINATES ARE U.S. SURVEY FEET, NAD '83 SURFACE. REFER TO SHEET C11.0 FOR SITE DETAILS.

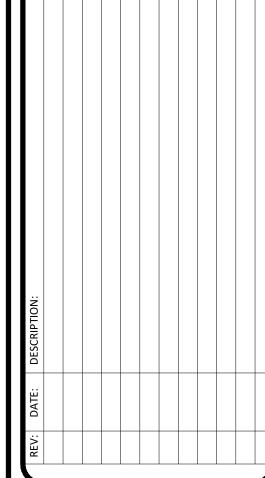
NOTE TO CONTRACTOR

THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON

DIMENSIONS SHOWN ARE TO FACE OF CURB.

CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND COORDINATES PRIOR TO CONSTRUCTION.

THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.





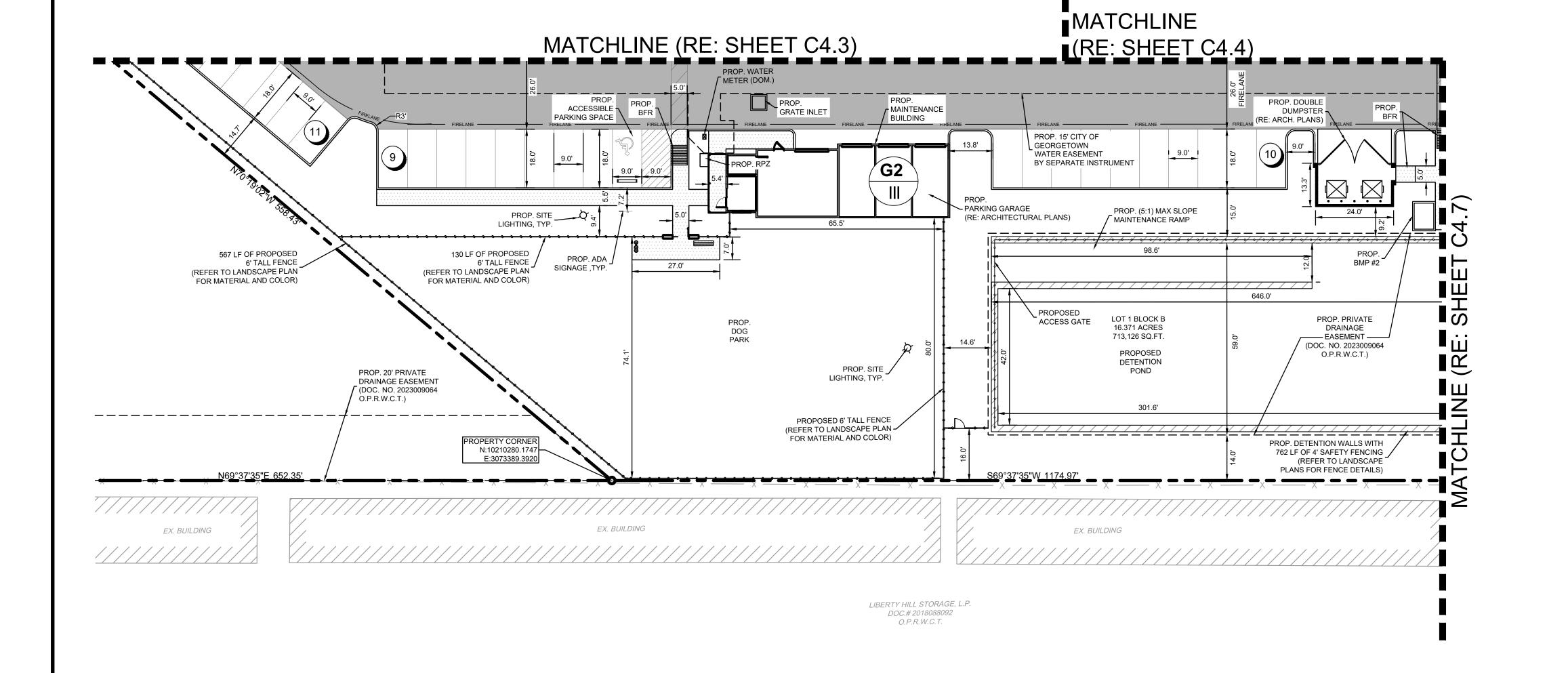
1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

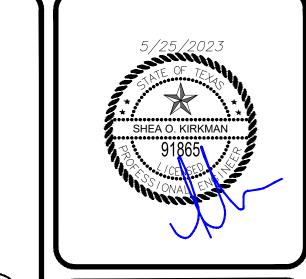
TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

DIMENSIONAL CONTROL PLAN VII

C4.6





CHV LIBERTY HILL 29

PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205

214-435-7510

LEGEND PROPOSED SIGN FIRE LANE STRIPING PROPOSED FIRE HYDRANT PROPOSED SANITARY MANHOLE 0 PROPOSED CURB INLET PROPOSED GRATE INLET ACCESSIBLE ROUTE • • • • • • • • • • • • PARKING COUNT PROPOSED RETAINING WALL PROPOSED FDC PROPERTY BOUNDARY PROPOSED SIDEWALK

RANCH CHALK



SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

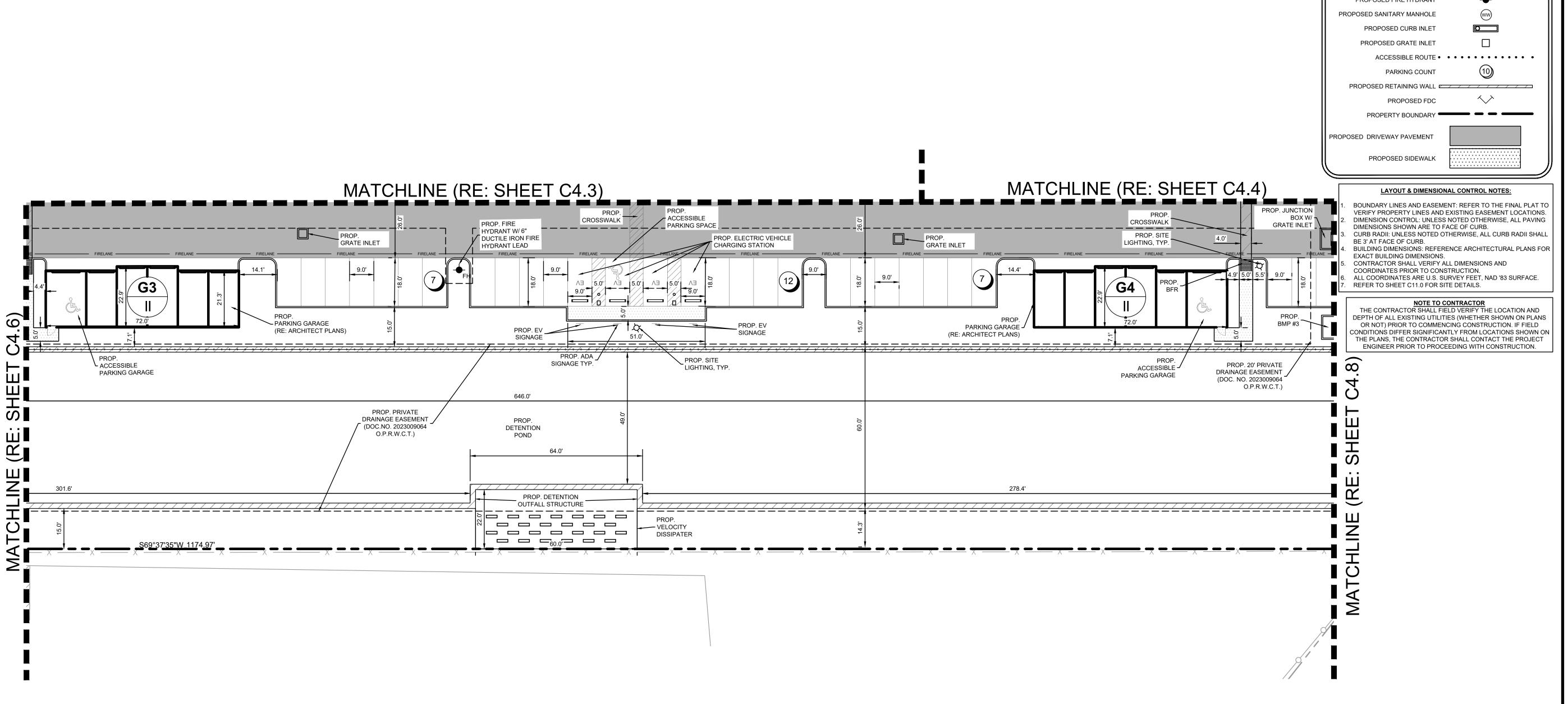
JOB NUMBER: CHV21004

ISSUE DATE:

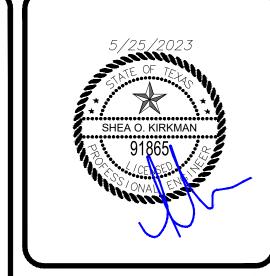
GRAPHIC SCALE

SCALE: 1" = 20'

DIMENSIONAL **CONTROL PLAN VIII**



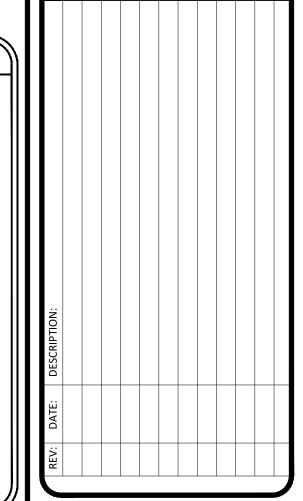




CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

RANCH CHALK HILL



0

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REFER TO SHEET C11.0 FOR SITE DETAILS.

NOTE TO CONTRACTOR

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DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD

ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT

GRAPHIC SCALE

SCALE: 1" = 20'

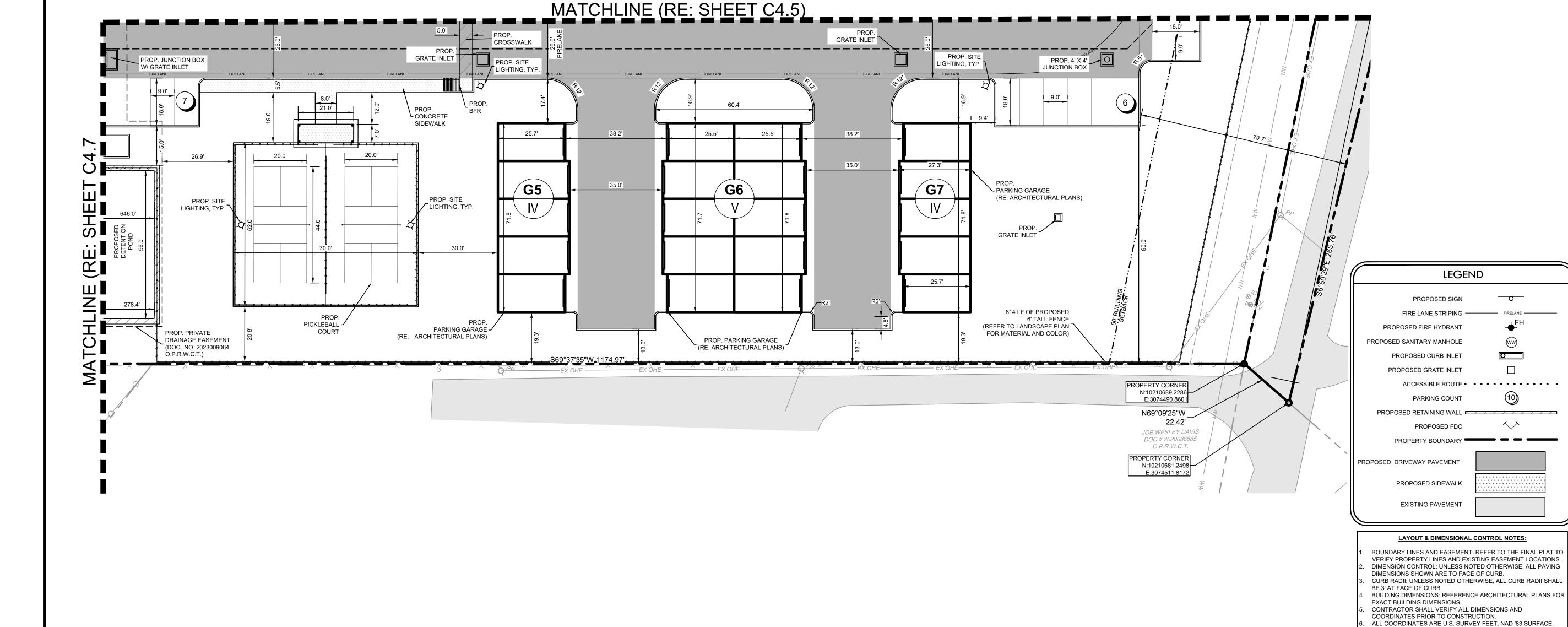


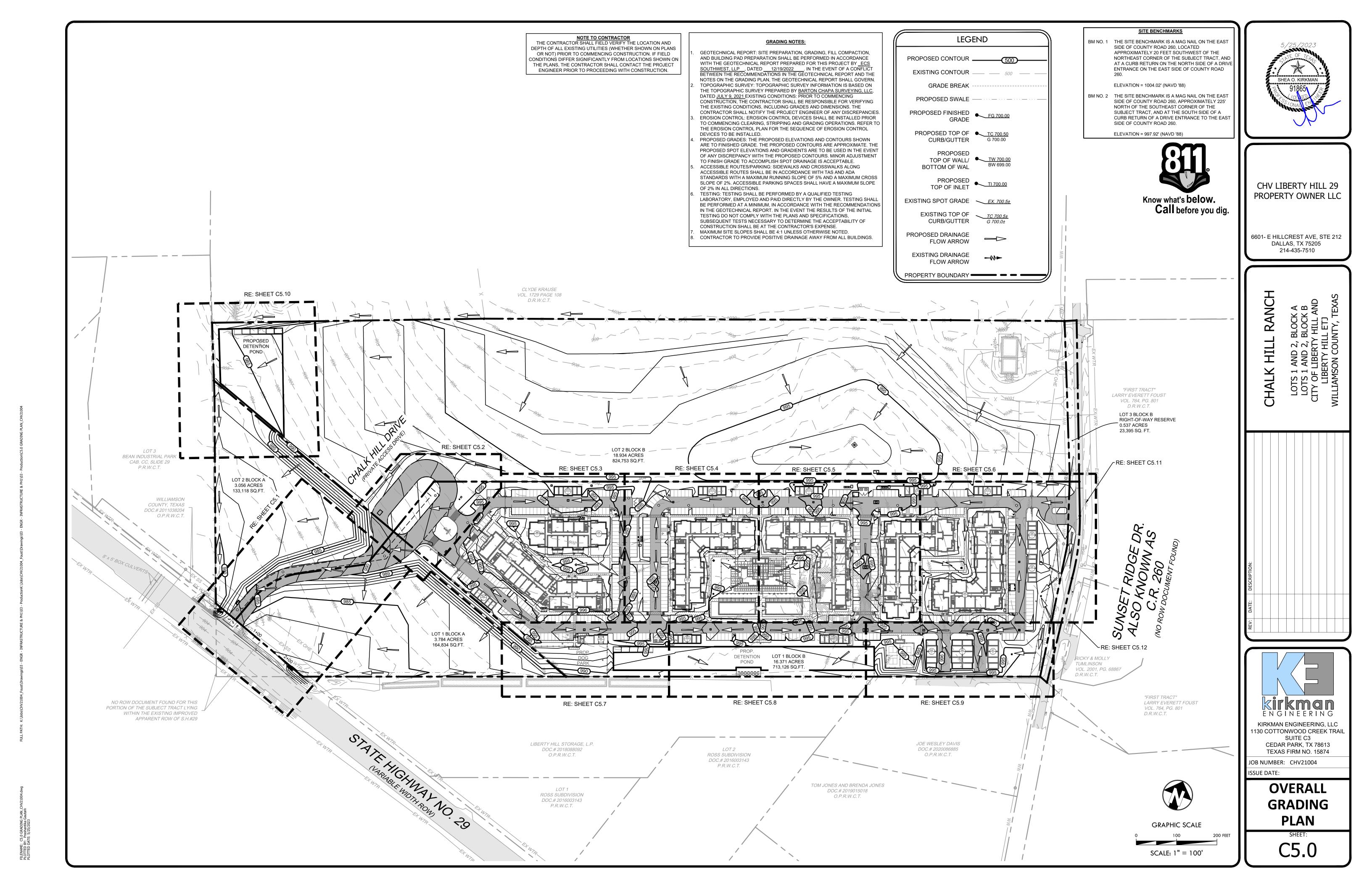
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

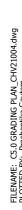
TEXAS FIRM NO. 15874

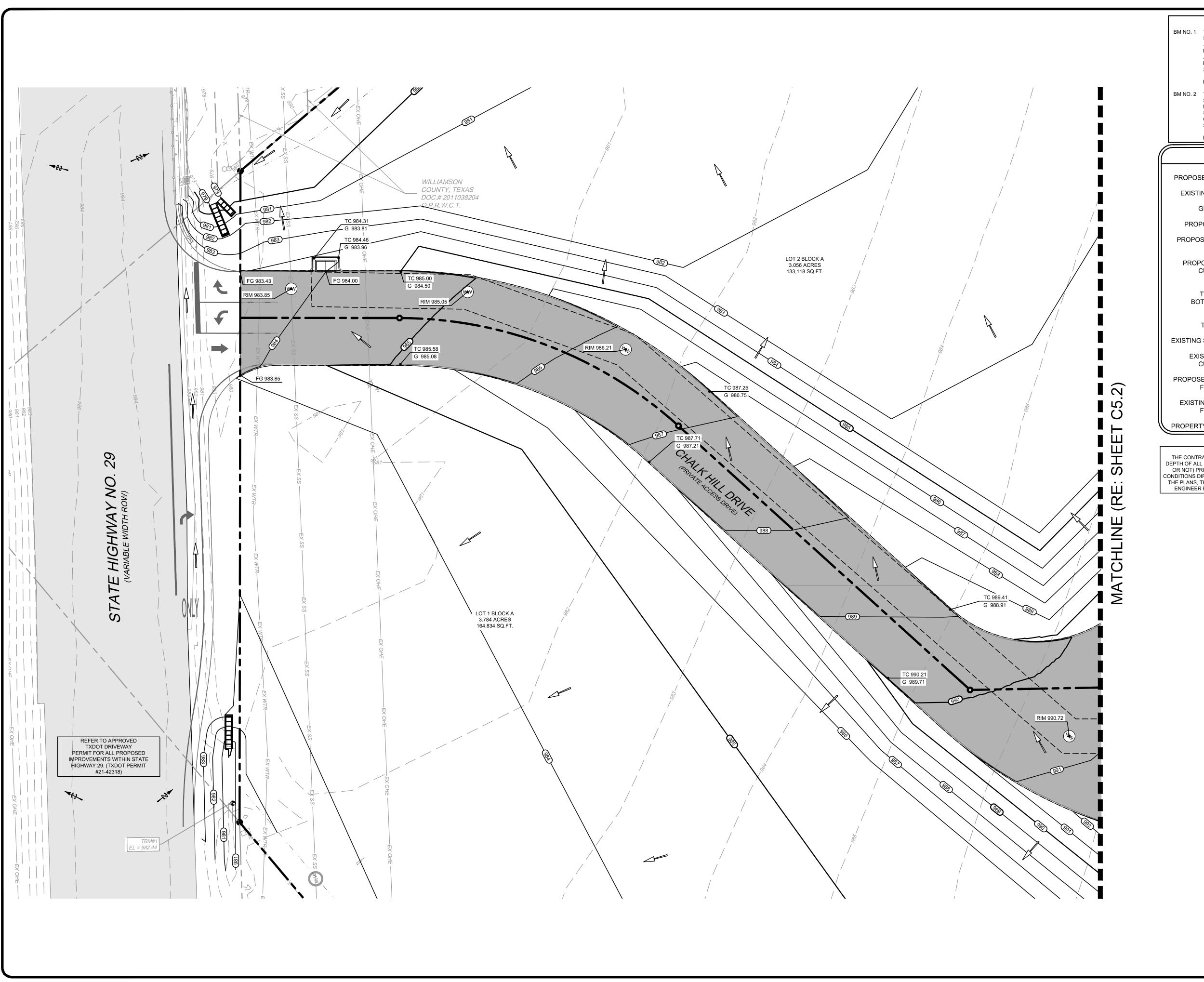
JOB NUMBER: CHV21004 ISSUE DATE:

DIMENSIONAL PLAN IX







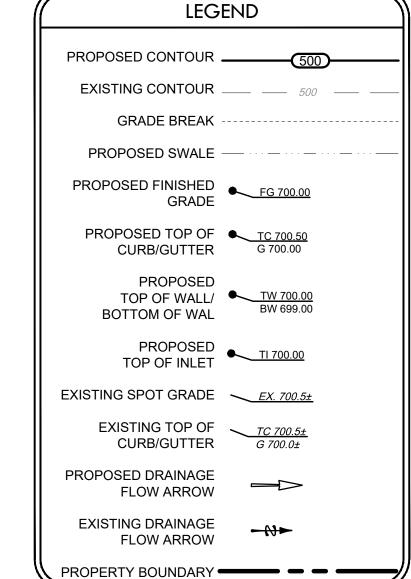


BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIVE ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

ELEVATION = 997.92' (NAVD '88)



NOTE TO CONTRACTOR

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CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

BLOCK A BLOCK B Y HILL AND LL ETJ JNTY, TEXAS

CHALK



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

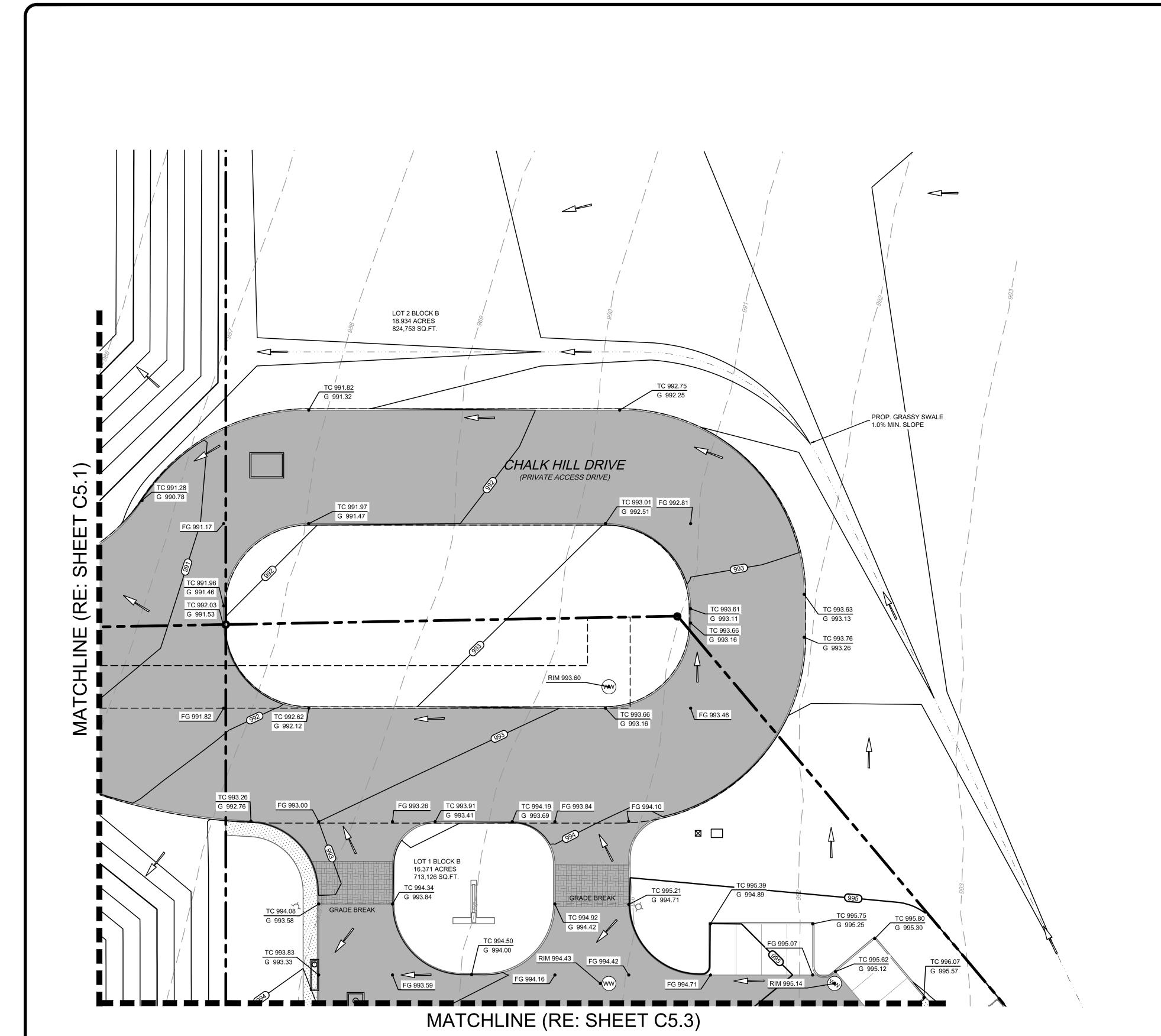
JOB NUMBER: CHV21004 ISSUE DATE:

Know what's **below. Call** before you dig.

GRAPHIC SCALE

SCALE: 1" = 20'

GRADING PLAN I

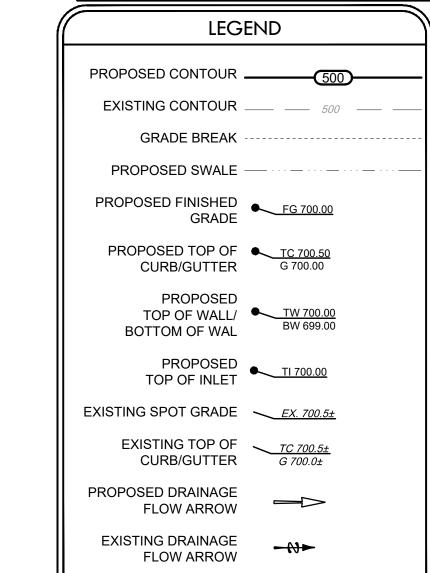


BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIVE ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

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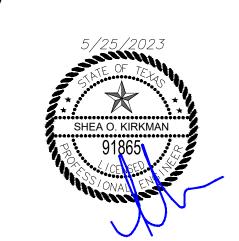
ELEVATION = 997.92' (NAVD '88)



PROPERTY BOUNDARY ———

NOTE TO CONTRACTOR

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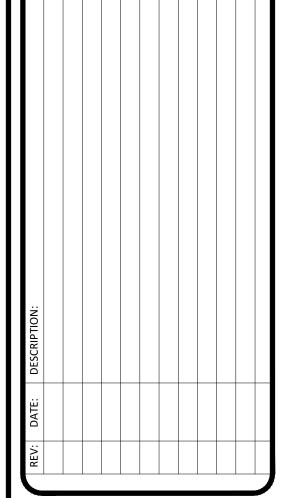


CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

RANCH

CHALK





SUITE C3
CEDAR PARK, TX 78613
TEXAS FIRM NO. 15874

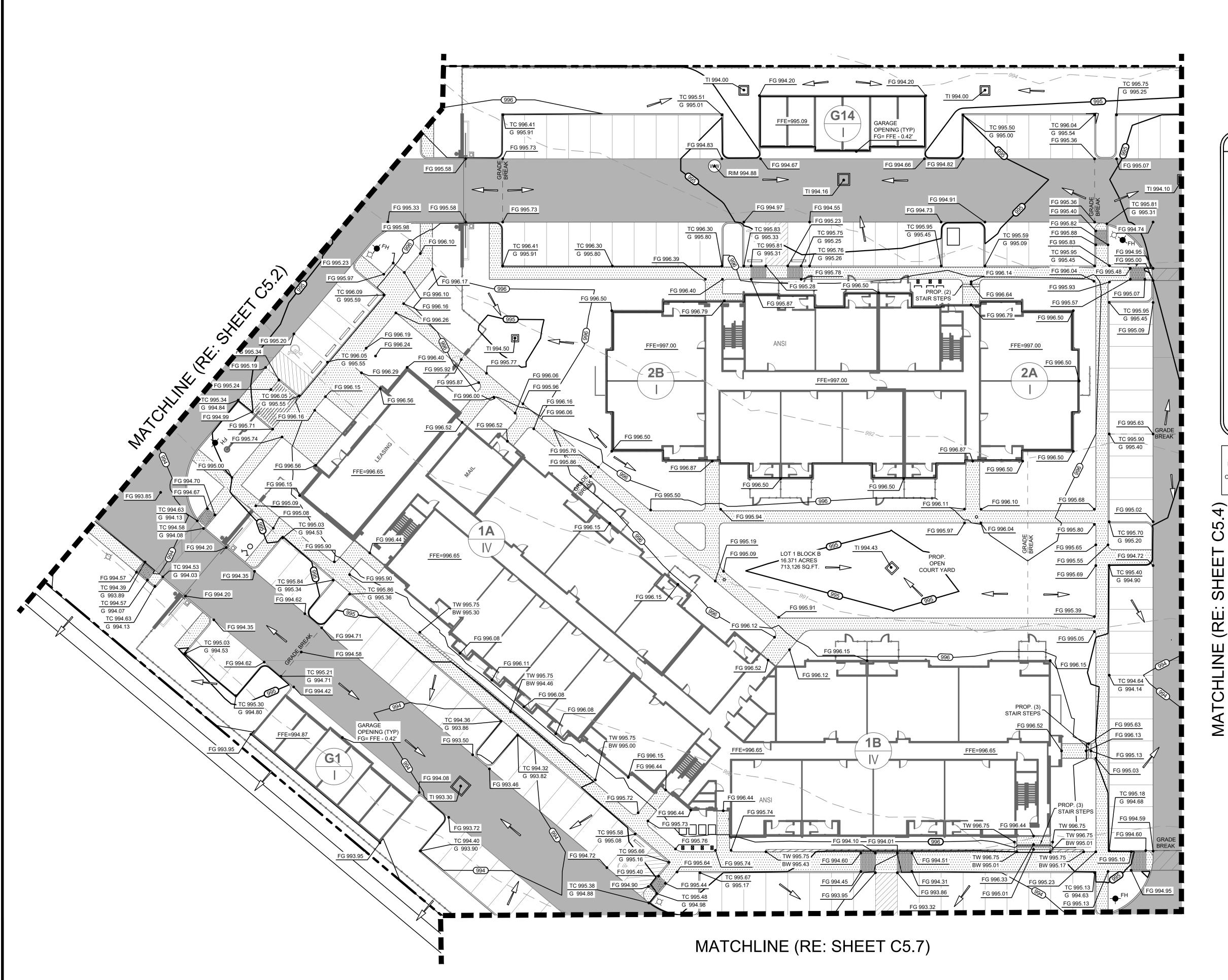
Know what's **below. Call** before you dig.

GRAPHIC SCALE

SCALE: 1" = 20'

GRADING PLAN II

C5.2



BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE SHEA O. KIRKMAN

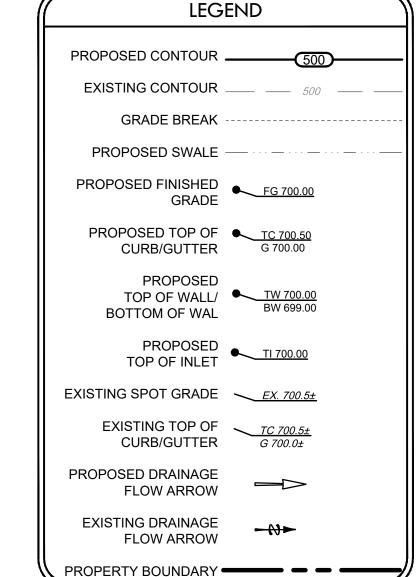
NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIV ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

SITE BENCHMARKS

ELEVATION = 997.92' (NAVD '88)



NOTE TO CONTRACTOR
THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205

214-435-7510

RANCH

CHALK

2, BLOCK A
2, BLOCK B
RTY HILL AND
HILL ETJ



SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

GRADING PLAN

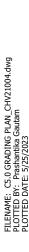
Know what's below.

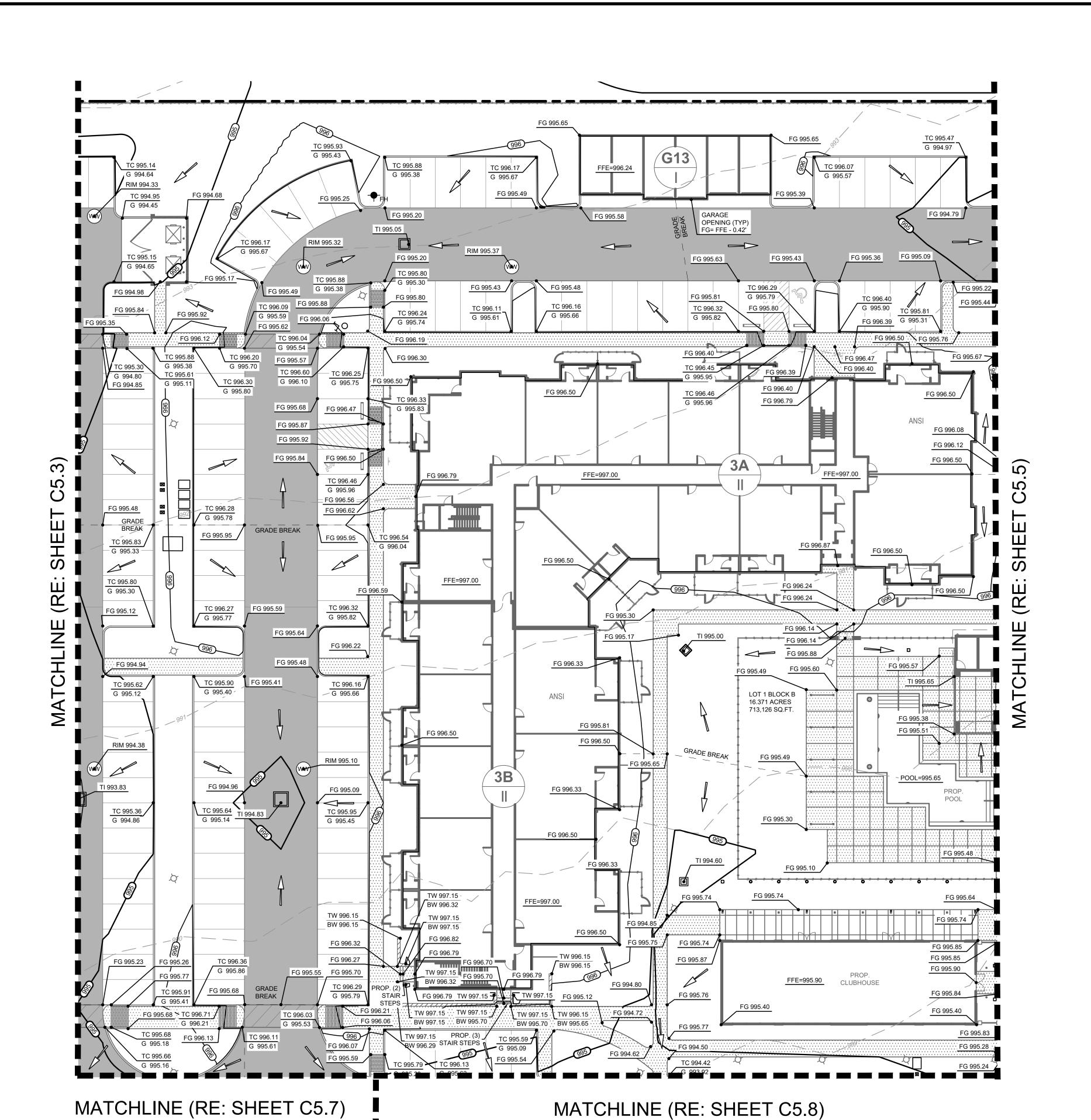
Call before you dig. JOB NUMBER: CHV21004 ISSUE DATE:

GRAPHIC SCALE

SCALE: 1" = 20'

C5.3



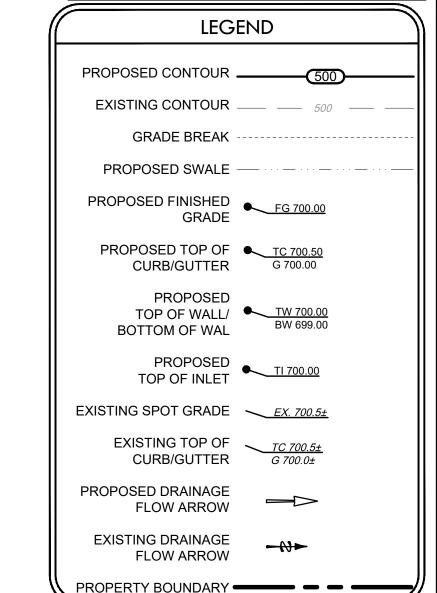


BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIV ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

ELEVATION = 997.92' (NAVD '88)



NOTE TO CONTRACTOR

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SHEA O. KIRKMAN

CHV LIBERTY HILL 29

PROPERTY OWNER LLC



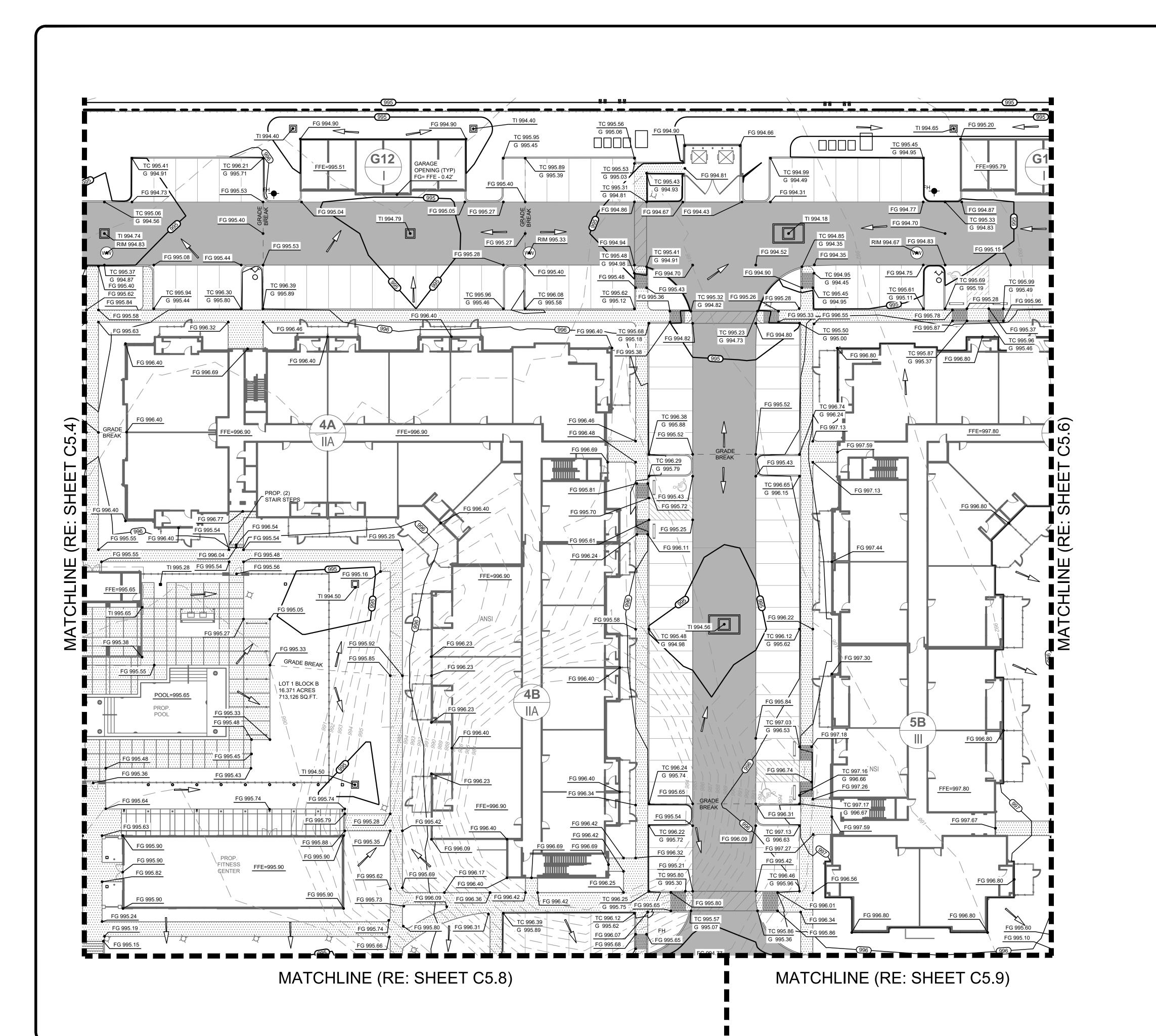
1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874 Know what's below.
Call before you dig.

JOB NUMBER: CHV21004

GRADING PLAN

C5.4

GRAPHIC SCALE SCALE: 1" = 20'



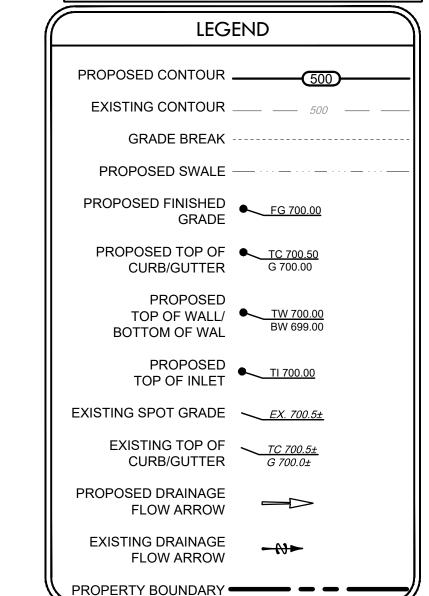
SITE BENCHMARKS BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED

APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIV ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

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CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205

RANCH

CHALK

214-435-7510

BLOCK A BLOCK B Y HILL AND LL ETJ



SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

GRADING PLAN V

C5.5

GRAPHIC SCALE SCALE: 1" = 20'

Know what's **below. Call** before you dig.

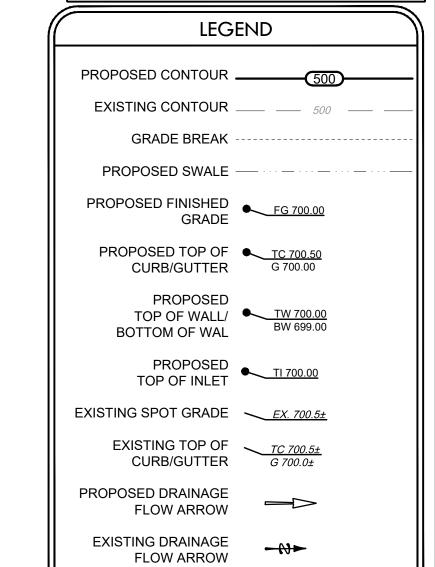
BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIV ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

Call before you dig.

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ELEVATION = 997.92' (NAVD '88)



PROPERTY BOUNDARY -

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SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

RANCH

CHALK

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

2, BLOCK A
2, BLOCK B
RTY HILL AND
HILL ETJ



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAI SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

GRADING PLAN

C5.6

GRAPHIC SCALE

SCALE: 1" = 20'

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIV ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

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ELEVATION = 997.92' (NAVD '88)

PROPOSED TOP OF INLET

EXISTING SPOT GRADE <u>EX. 700.5±</u>

PROPOSED DRAINAGE

EXISTING DRAINAGE FLOW ARROW

PROPERTY BOUNDARY -

FLOW ARROW

EXISTING TOP OF CURB/GUTTER G 700.0±

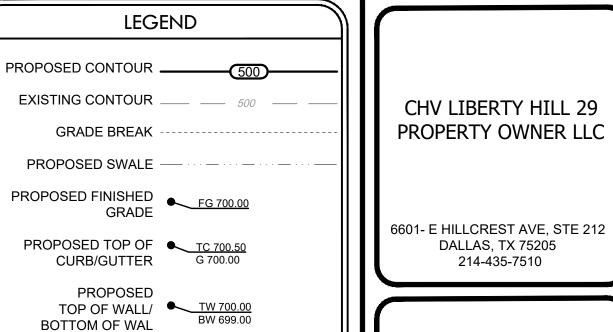
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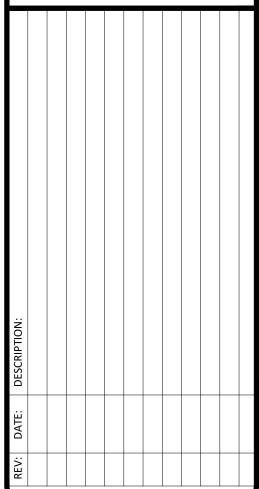
CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON



RANCH

CHALK HILL

SHEA O. KIRKMAN



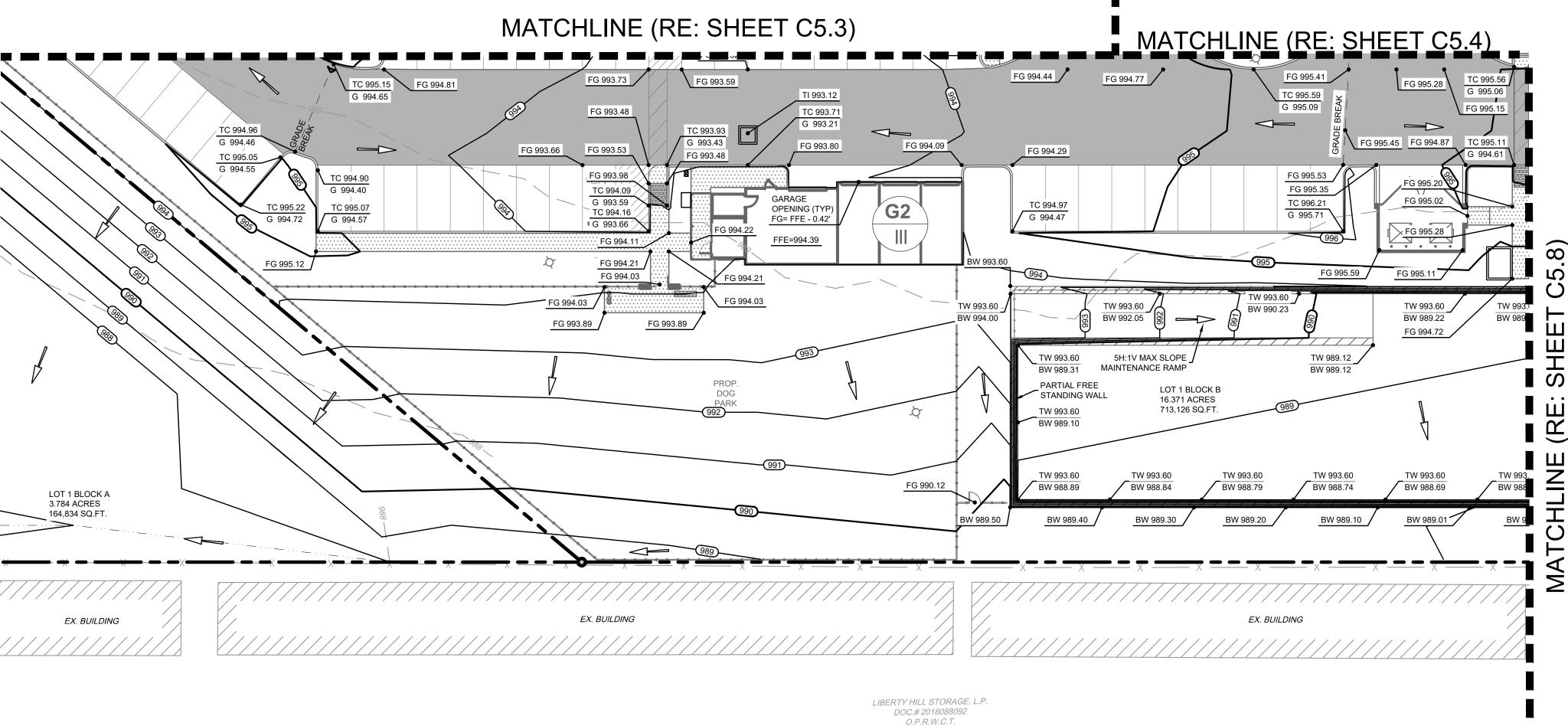


SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

GRADING PLAN



GRAPHIC SCALE

SCALE: 1" = 20'

SHEA O. KIRKMAN

91865

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

A B Q

LOTS 1 AND 2, BLOCK A
LOTS 1 AND 2, BLOCK B
CITY OF LIBERTY HILL AND
LIBERTY HILL ETJ

REV: DATE: DESCRIPTION:

KICKMAN ENGINEERING, LLC

KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

TEXAS FIRM NO. 158

JOB NUMBER: CHV21004

GRADING PLAN

SHEET:

SCALE: 1" = 20'

FILENAME: C5.0 GRADING PLAN_CHV21004.dwg
PLOTTED BY: Prashantika Gautam

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIVE

ELEVATION = 1004.02' (NAVD '88)

Know what's **below. Call** before you dig.

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225'
NORTH OF THE SOUTHEAST CORNER OF THE
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ELEVATION = 997.92' (NAVD '88)



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK

KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

GRADING PLAN

LEGEND

EXISTING CONTOUR ____ GRADE BREAK

PROPOSED SWALE —

PROPOSED CONTOUR ____

PROPOSED FINISHED FG 700.00 GRADE

PROPOSED TOP OF CURB/GUTTER TC 700.50 G 700.00

PROPOSED TOP OF WALL/
BW 699.00 **BOTTOM OF WAL**

PROPOSED TOP OF INLET

EXISTING SPOT GRADE <u>EX. 700.5±</u> EXISTING TOP OF TC 700.5±
CURB/GUTTER G 700.0±

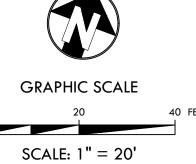
PROPOSED DRAINAGE FLOW ARROW

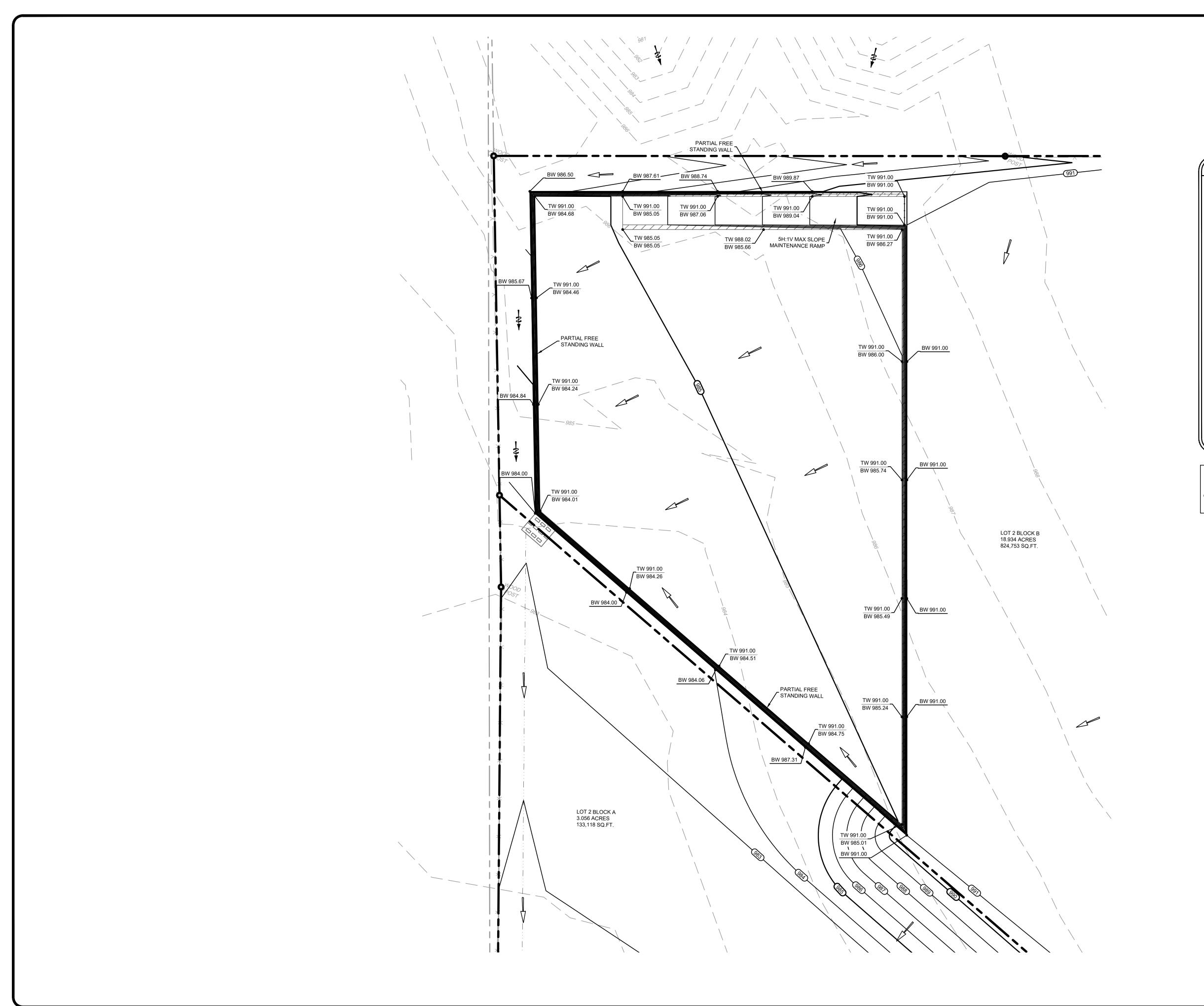
EXISTING DRAINAGE FLOW ARROW

PROPERTY BOUNDARY — — —

NOTE TO CONTRACTOR

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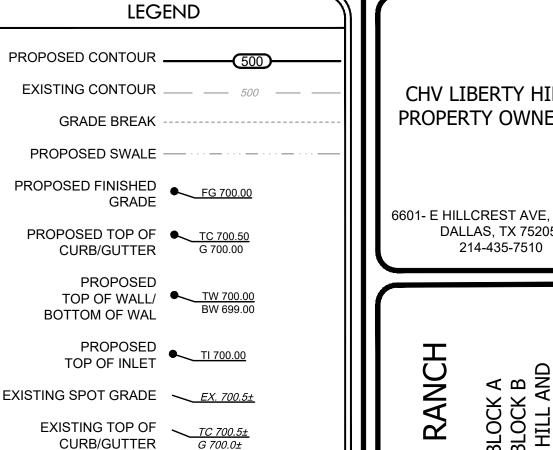


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ELEVATION = 997.92' (NAVD '88)



EXISTING SPOT GRADE <u>EX. 700.5±</u>

PROPOSED DRAINAGE

EXISTING DRAINAGE

PROPERTY BOUNDARY -

FLOW ARROW

FLOW ARROW

EXISTING TOP OF CURB/GUTTER

TC 700.5±
G 700.0±

NOTE TO CONTRACTOR

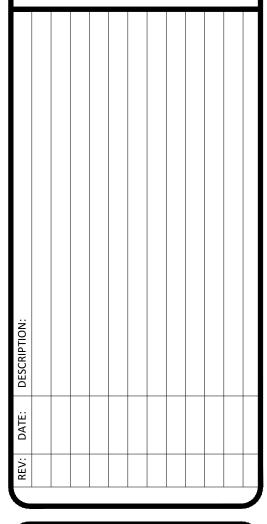
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CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205

CHALK





1130 COTTONWOOD CREEK TRAIL

SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

Know what's **below. Call** before you dig.

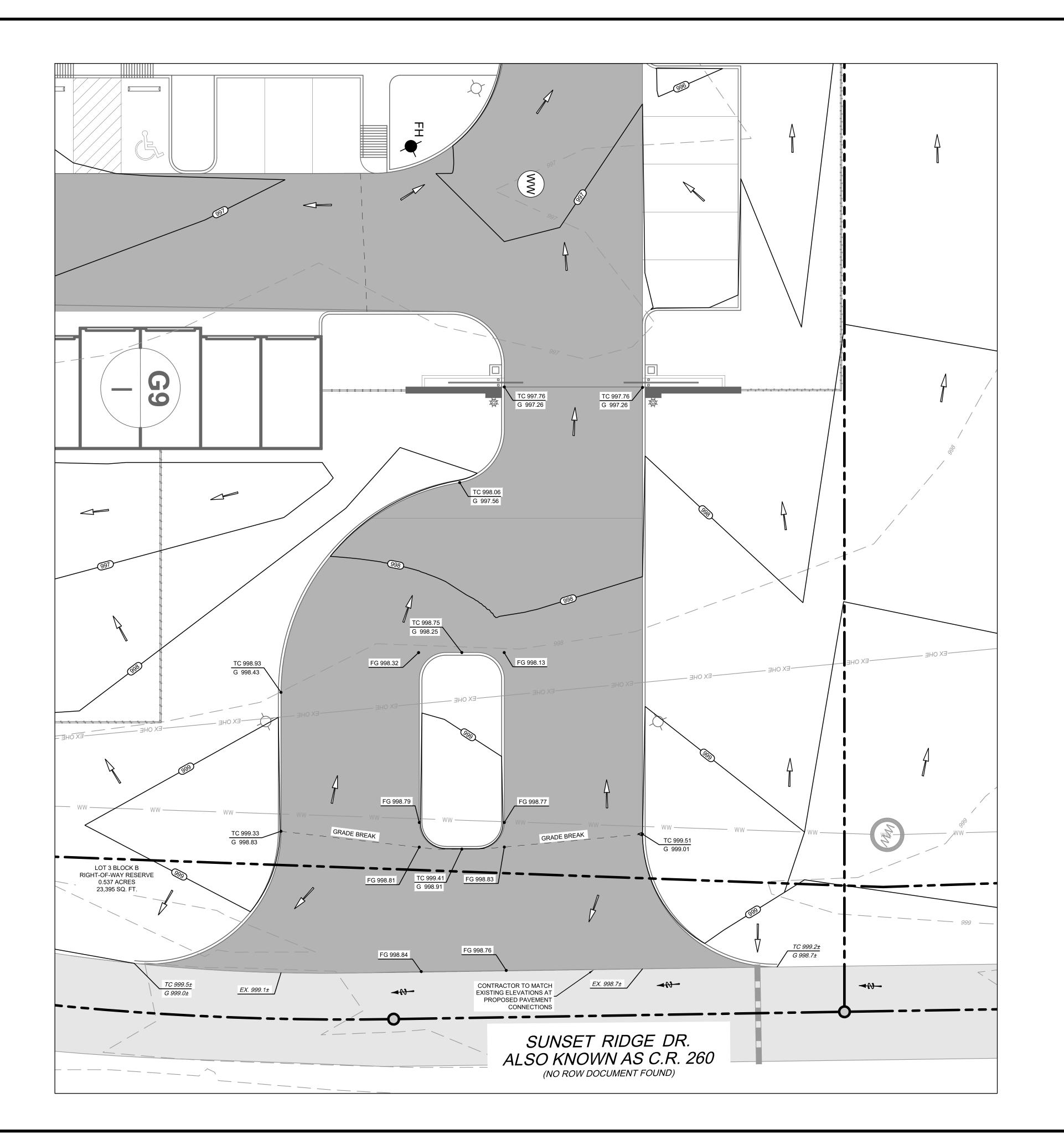
GRAPHIC SCALE

SCALE: 1" = 20'

DETENTION POND GRADING

PLAN

C5.10

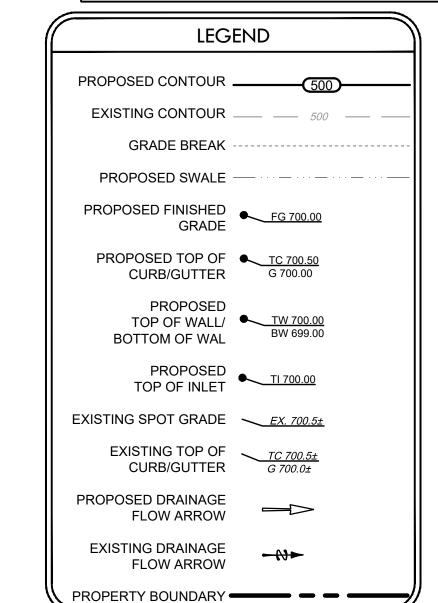


BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIVE ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

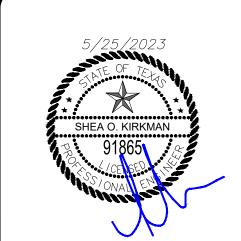
ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

ELEVATION = 997.92' (NAVD '88)



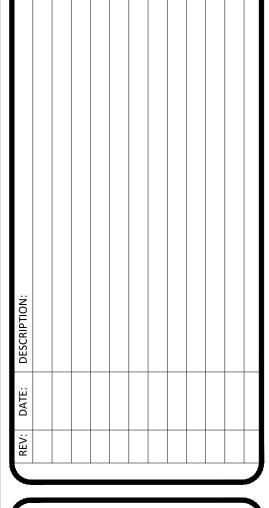
DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

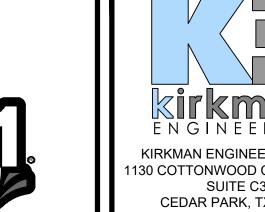


CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK





ISSUE DATE:

Know what's below.
Call before you dig.

GRAPHIC SCALE

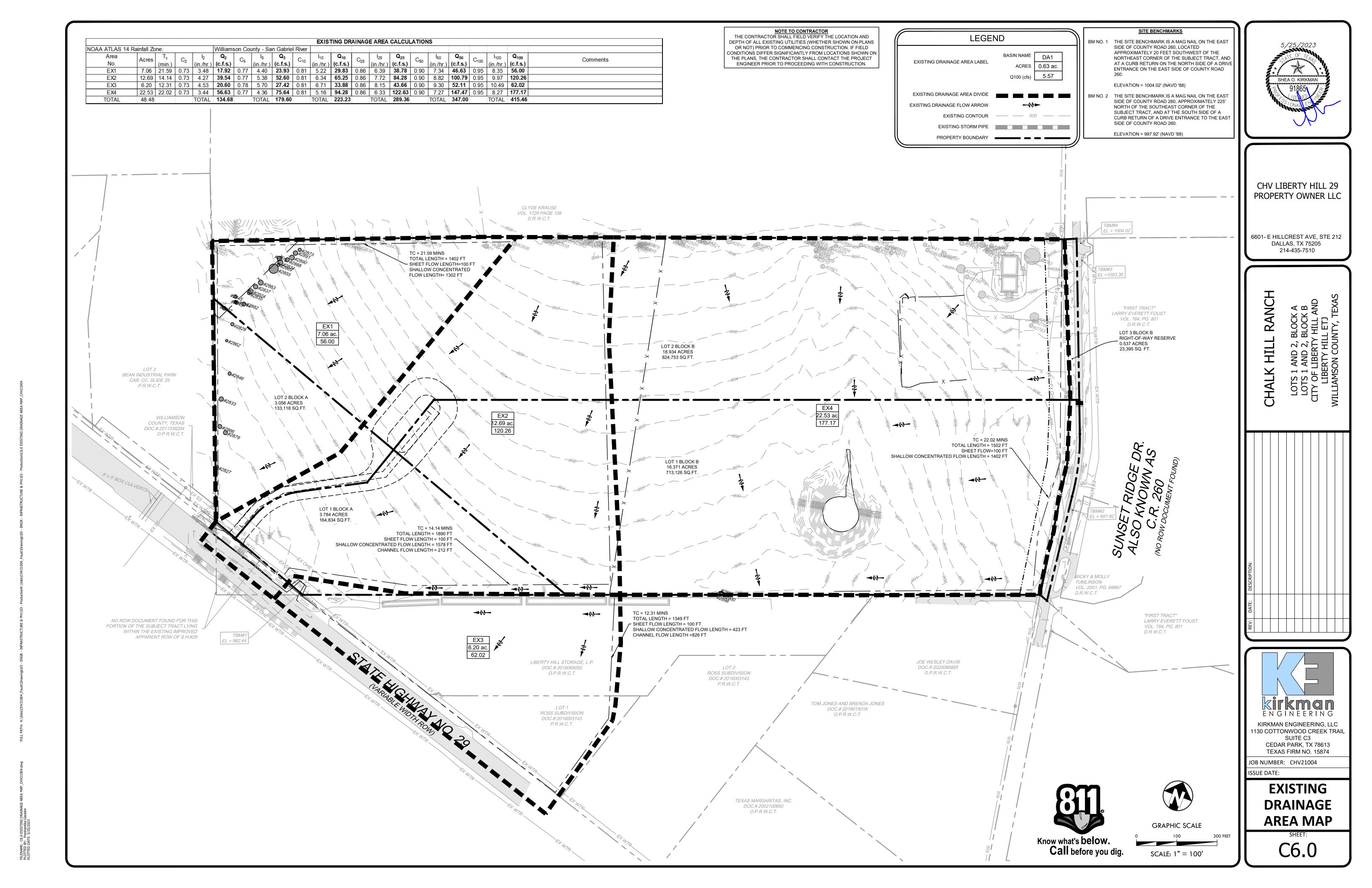
SCALE: 1" = 10'

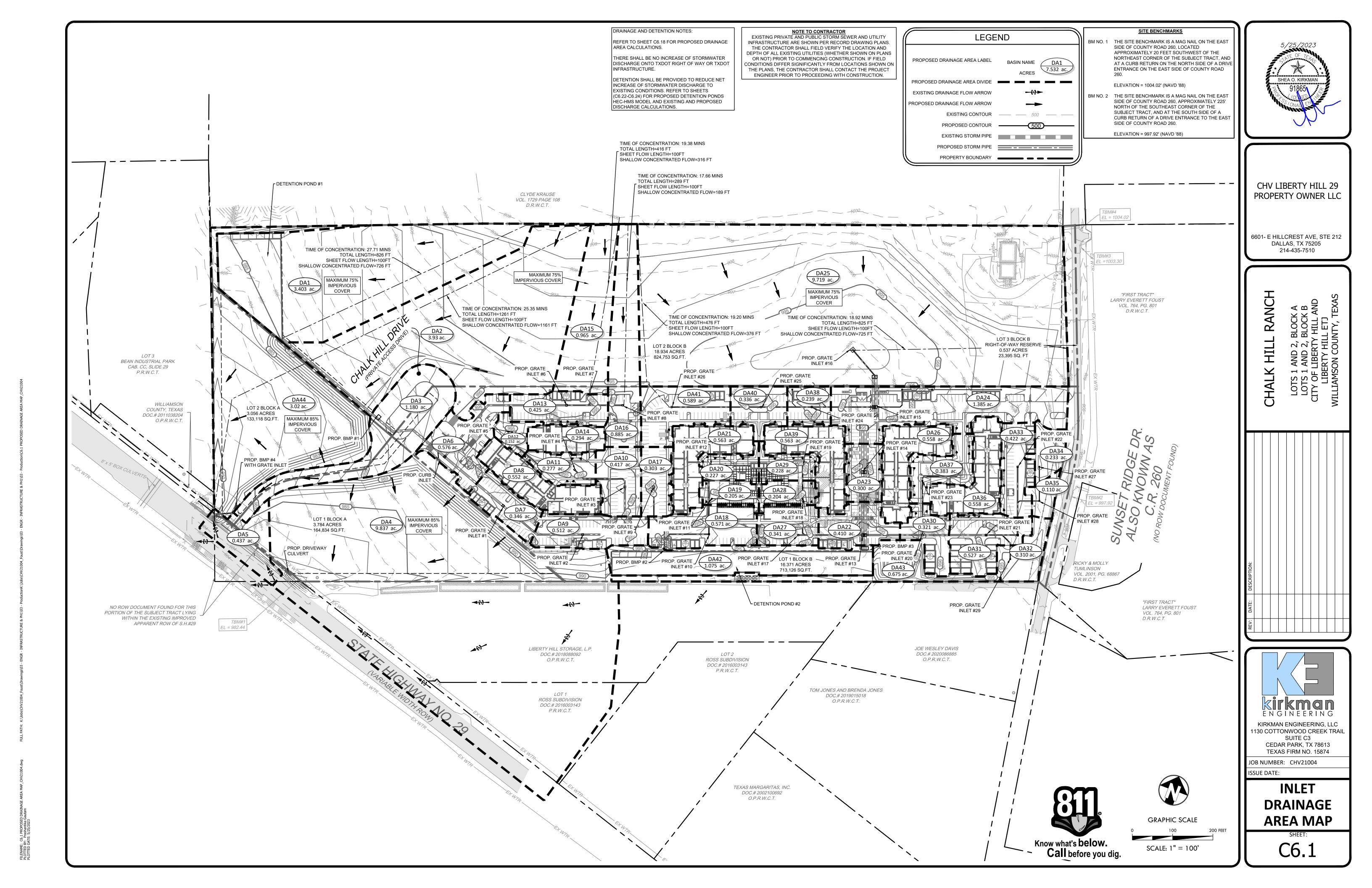
SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

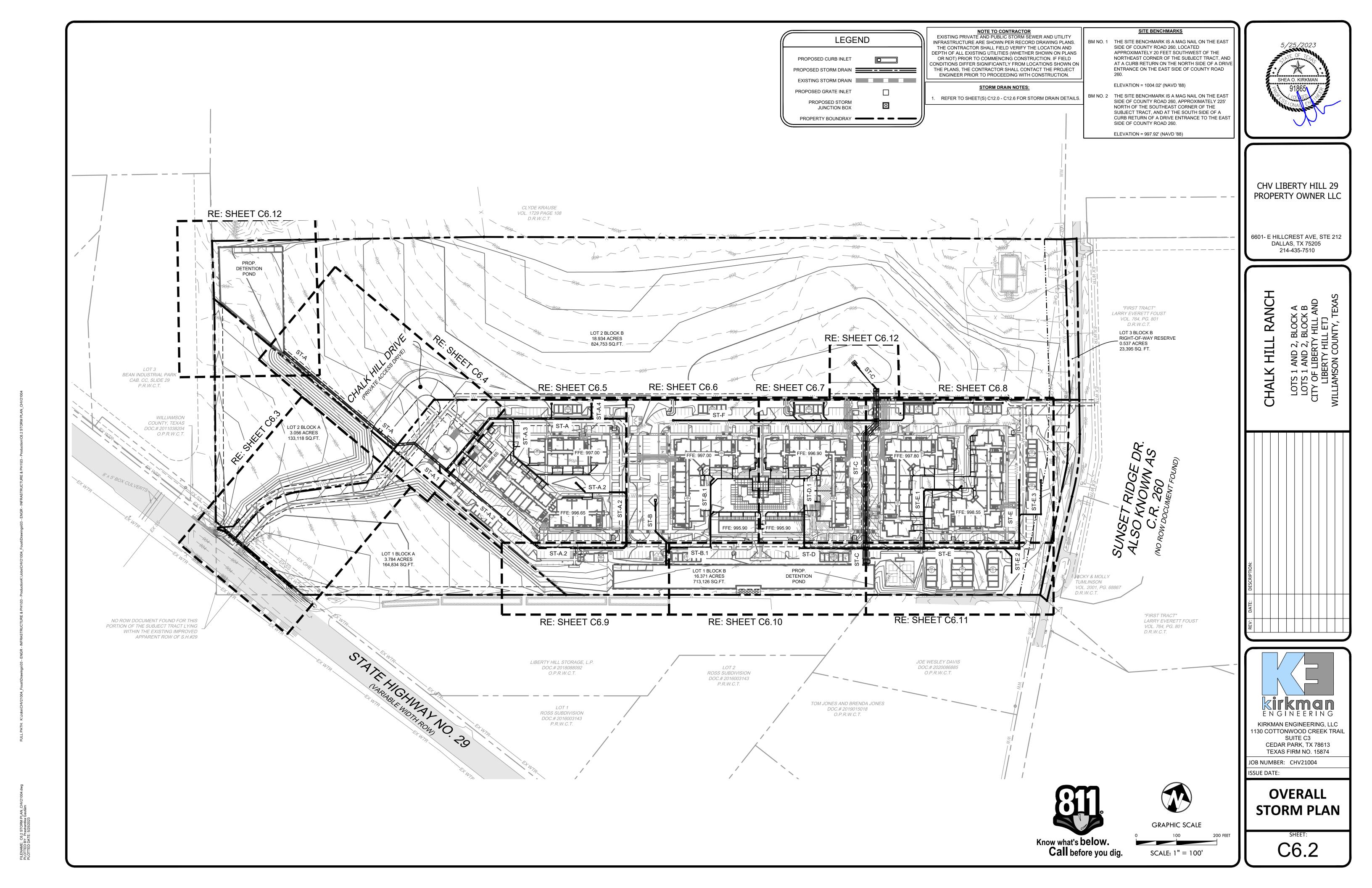
JOB NUMBER: CHV21004

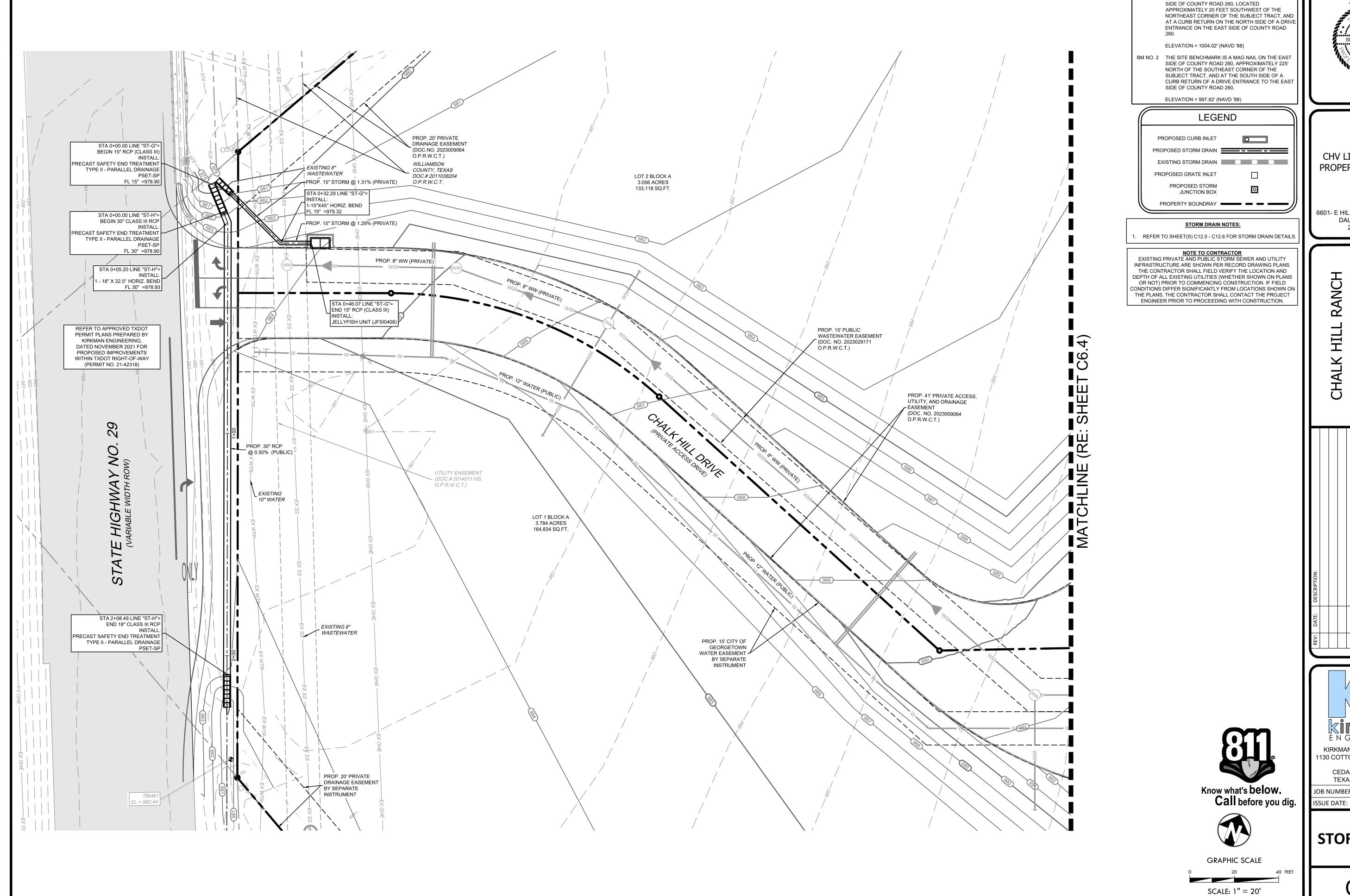
DETAILED GRADING PLAN

C5.11









SHEA O. KIRKMAN

SITE BENCHMARKS BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST

> CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

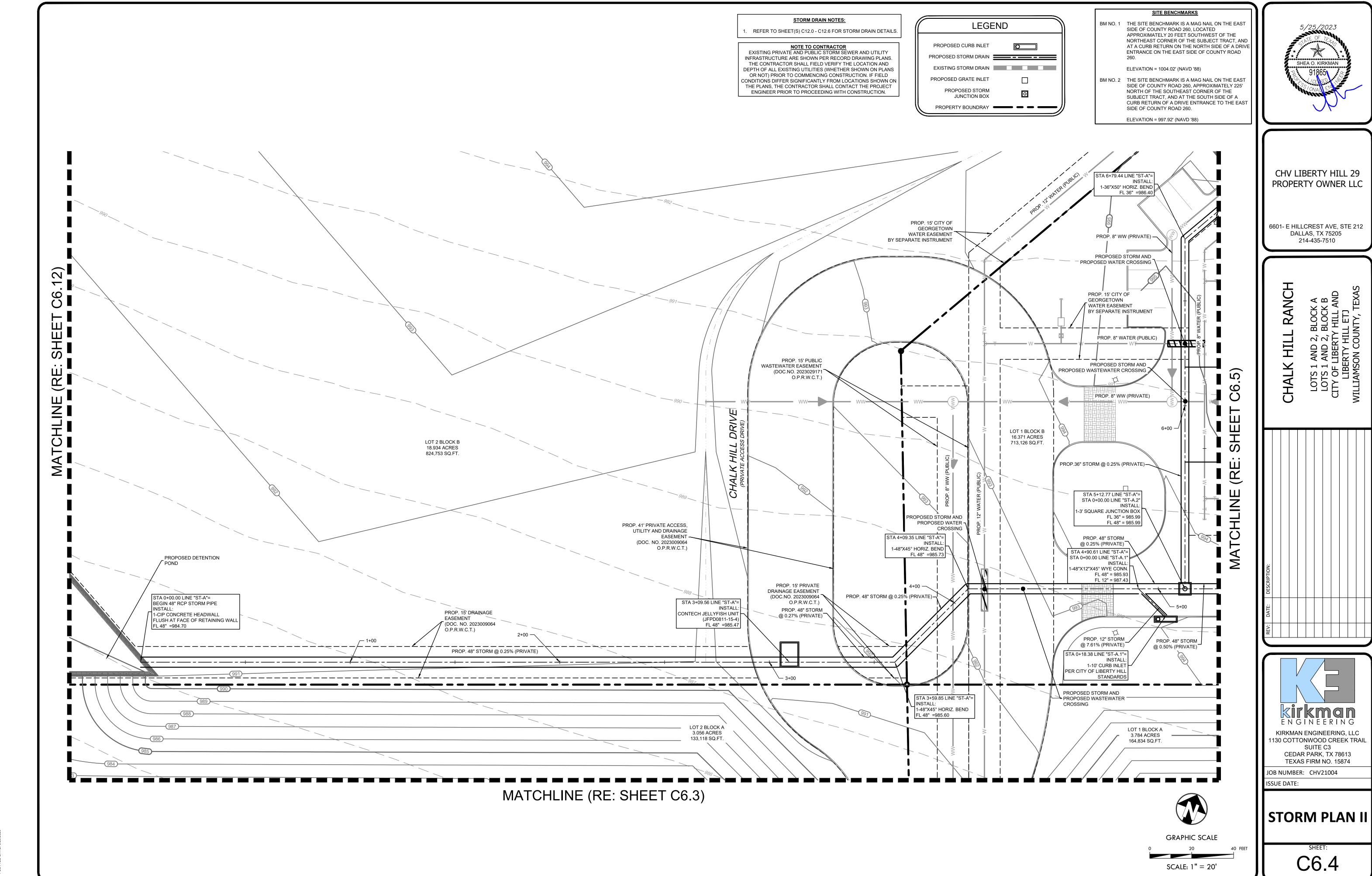


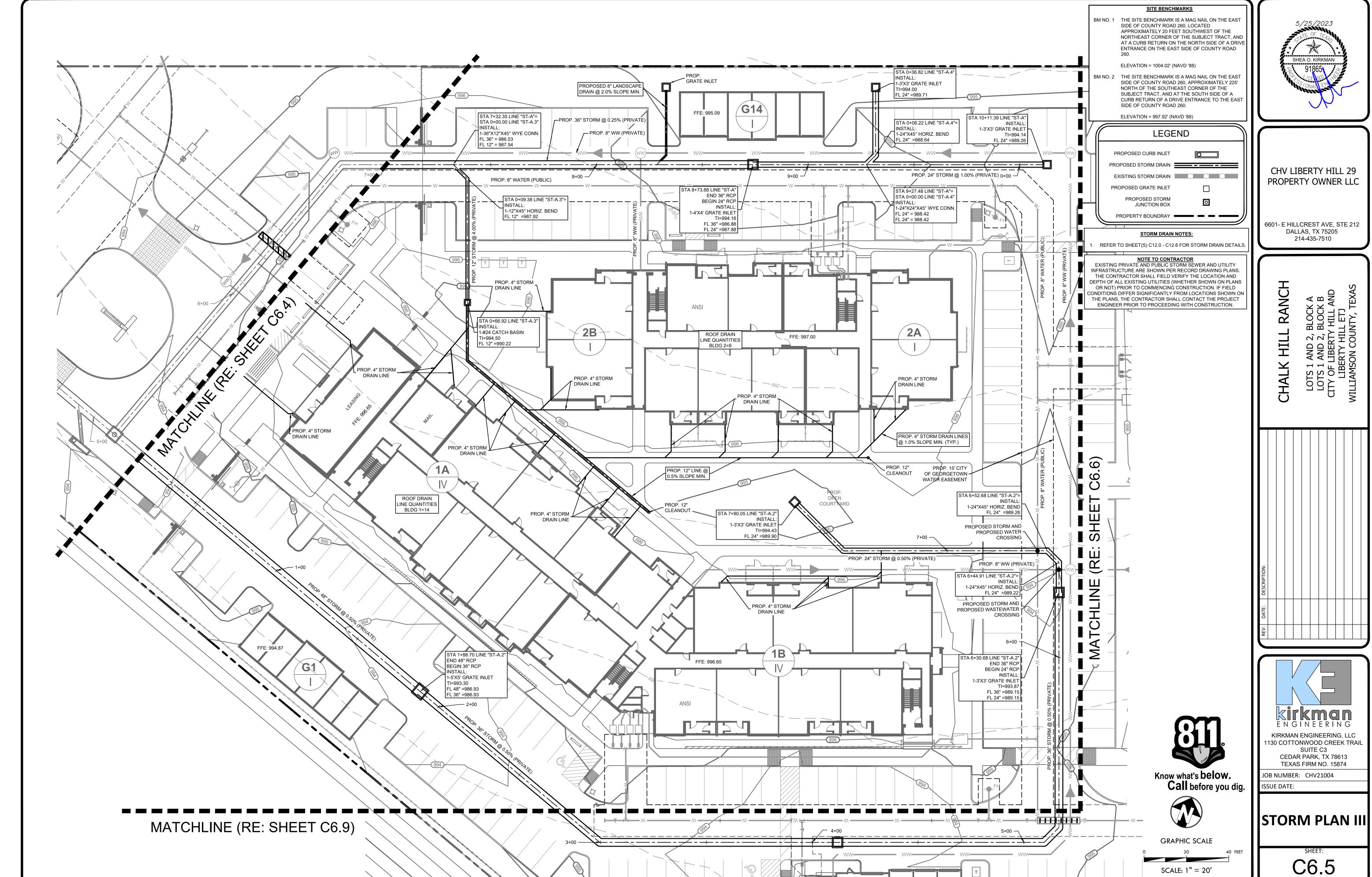
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

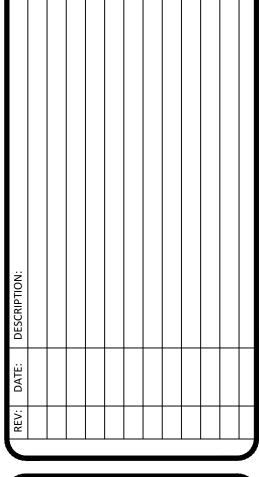
CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

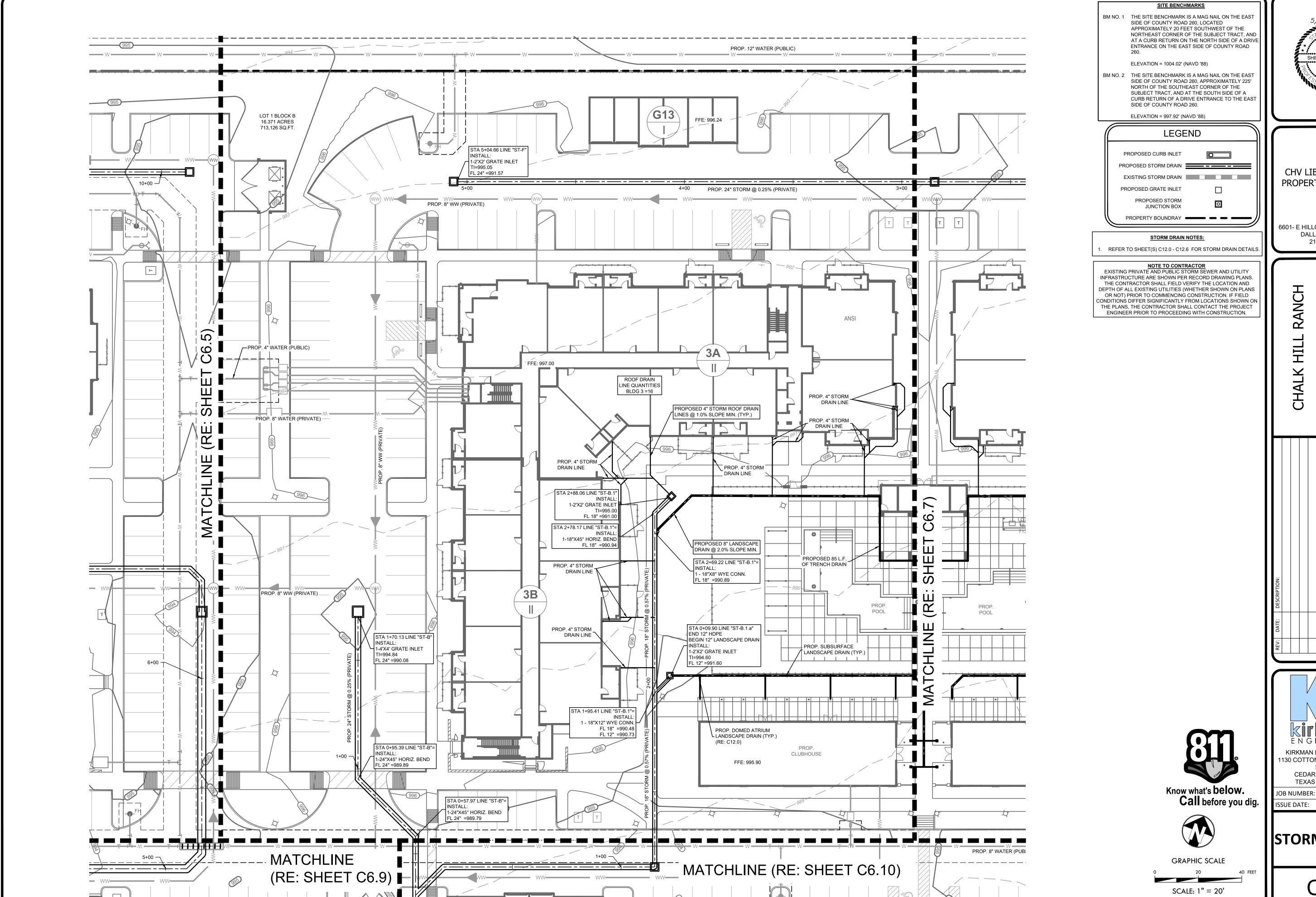
JOB NUMBER: CHV21004

STORM PLAN I







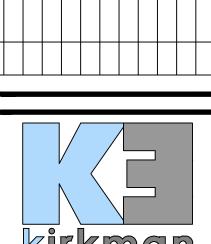


SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

< A < B AND

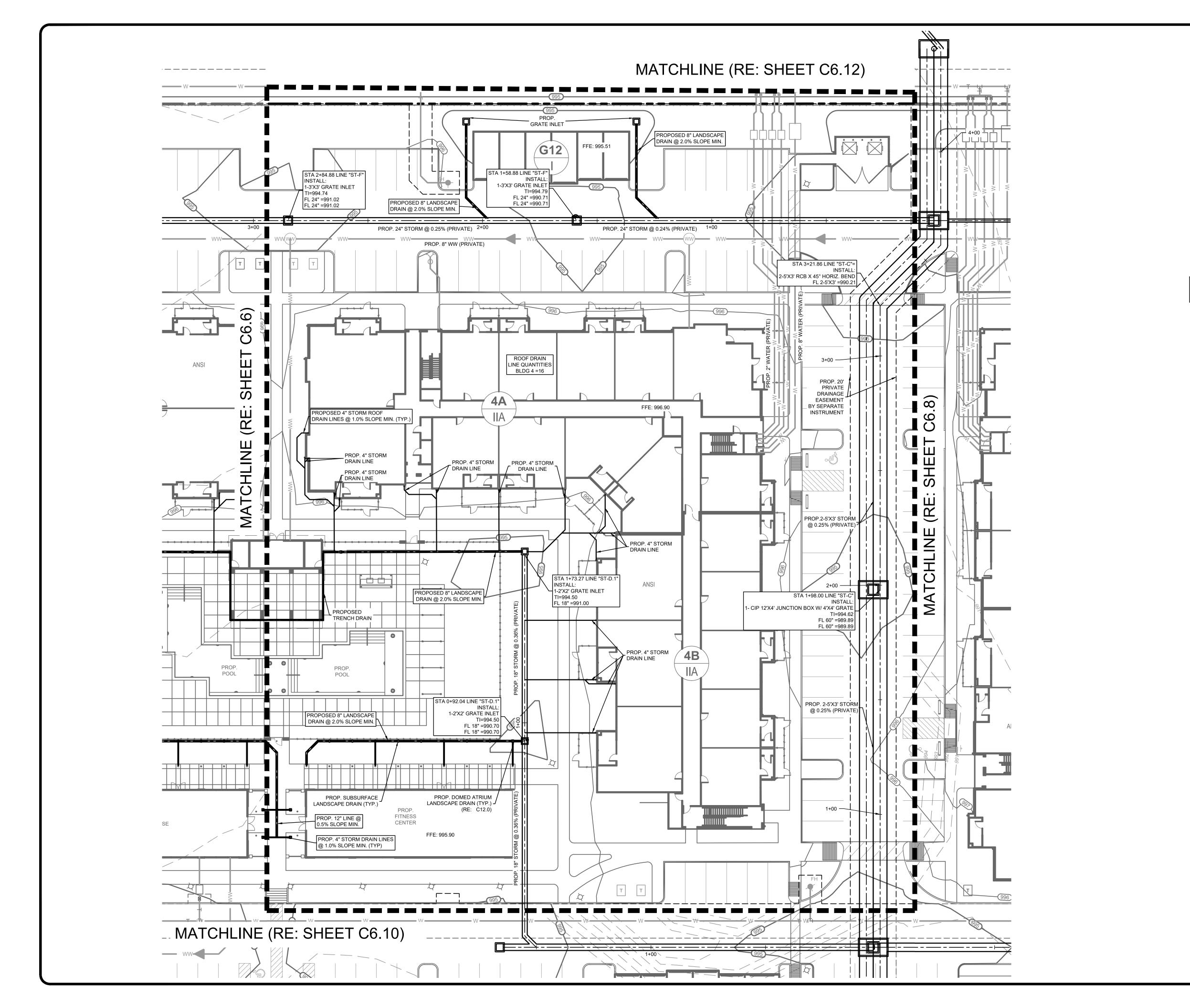


KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAI

SUITE C3
CEDAR PARK, TX 78613
TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

STORM PLAN IV

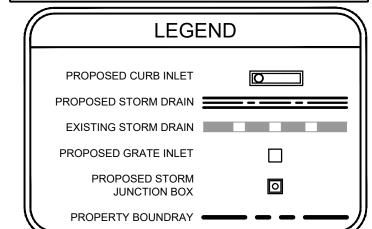


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ELEVATION = 1004.02' (NAVD '88)

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ELEVATION = 997.92' (NAVD '88)



STORM DRAIN NOTES:

REFER TO SHEET(S) C12.0 - C12.6 FOR STORM DRAIN DETAILS

NOTE TO CONTRACTOR

EXISTING PRIVATE AND PUBLIC STORM SEWER AND UTILITY

INFRASTRUCTURE ARE SHOWN PER RECORD DRAWING PLANS. THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

GRAPHIC SCALE

SCALE: 1" = 20'

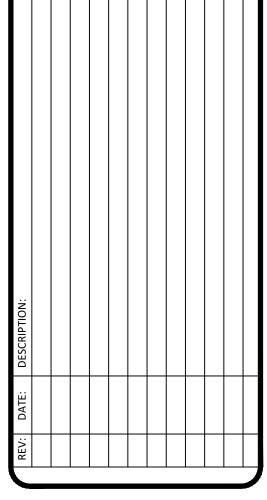
SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

RANCH

CHALK



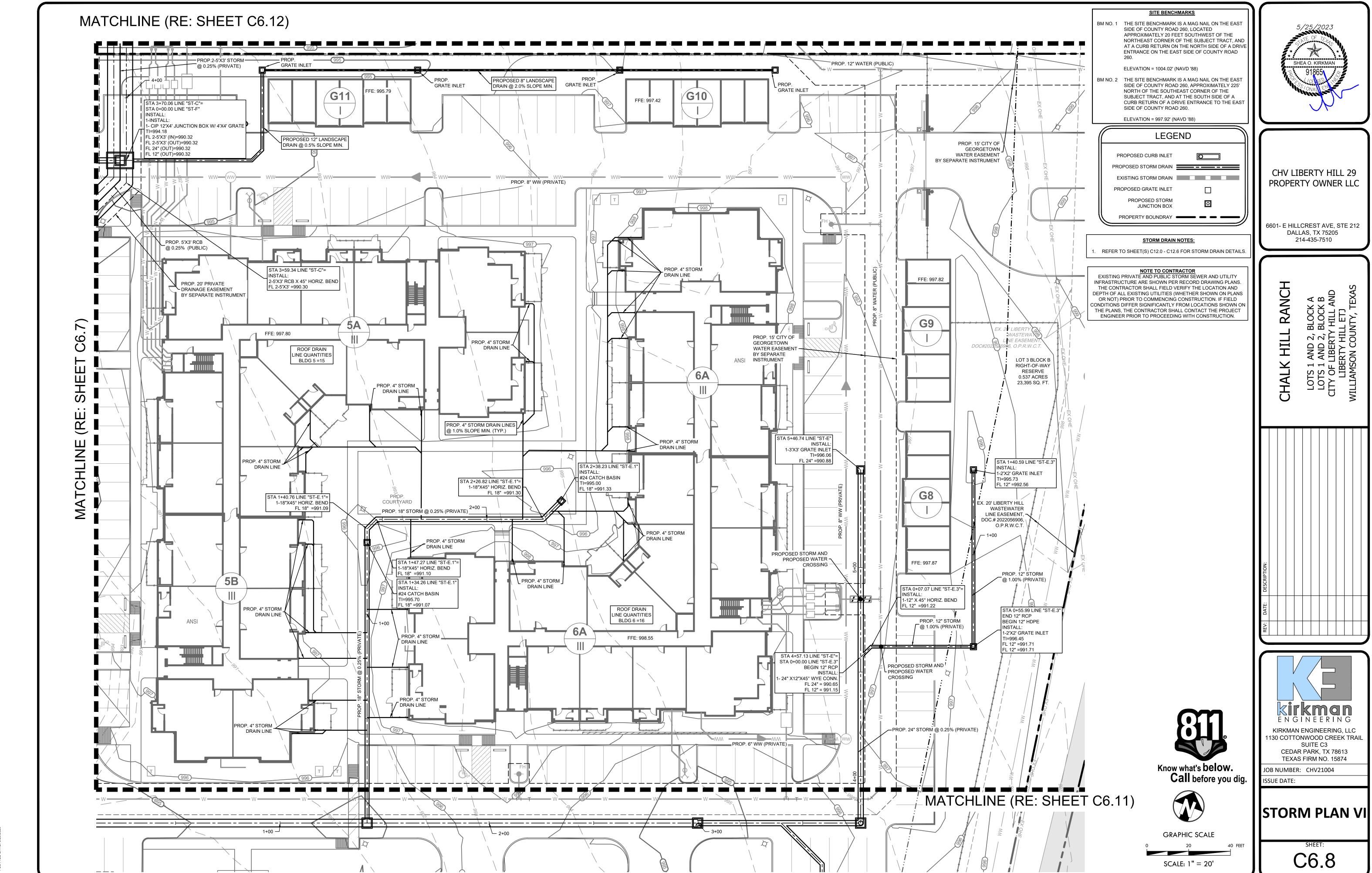


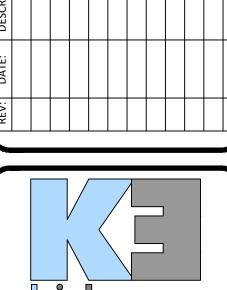
SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

Know what's below.
Call before you dig.

JOB NUMBER:
ISSUE DATE: JOB NUMBER: CHV21004

STORM PLAN V



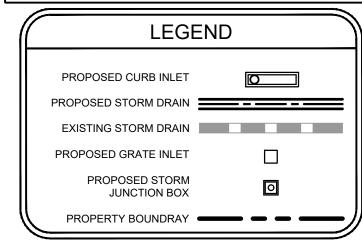


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ELEVATION = 997.92' (NAVD '88)



STORM DRAIN NOTES:

0

PROP. DETENTION POND (REFER TO GRADING PLANS FOR DETENTION WALL ELEVATIONS)

MATCHLINE

➤ PROPOSED WATER

STA 5+19.49 LINE "ST-A.2"=

1-36"X45° HORIZ. BEND

STA 5+09.61 LINE "ST-A.2"=

1-36"X45° HORIZ. BEND

PROP. MAINTENANCE RAMP

(5:1 MAX SLOPE)

644.0'

- CROSSING

FL 36" =988.59

FL 36" =988.54

INSTALL:

(RE: SHEET C6.6)

PROPOSED STORM AND PROP. 8" WATER (PUBLIC)

REFER TO SHEET(S) C12.0 - C12.6 FOR STORM DRAIN DETAILS.

<u>NOTE TO CONTRACTOR</u>
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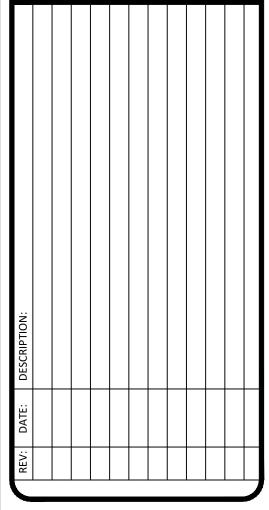
SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

RANCH

CHALK





Know what's below.

Call before you dig.



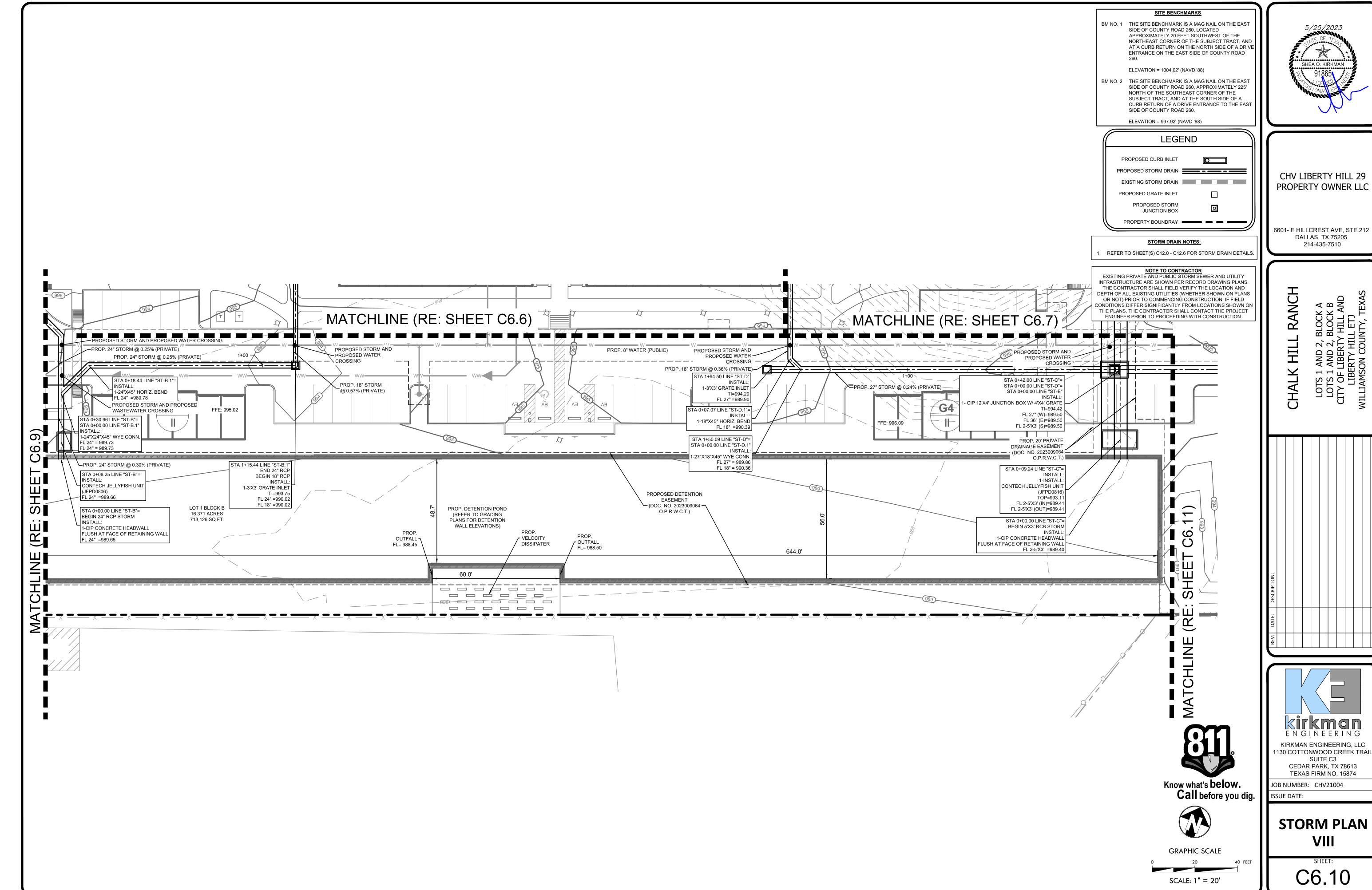
GRAPHIC SCALE

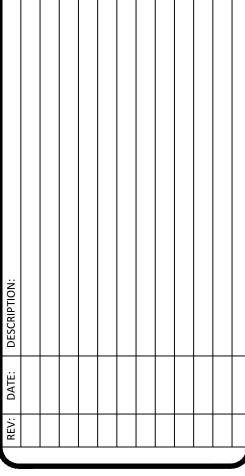
SCALE: 1" = 20'

ISSUE DATE: **STORM PLAN**

JOB NUMBER: CHV21004

1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874





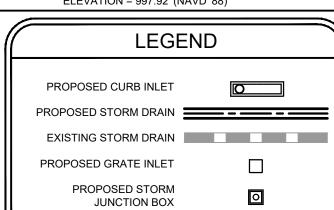
KIRKMAN ENGINEERING, LLC

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STORM DRAIN NOTES:

PROPERTY BOUNDRAY — — — —

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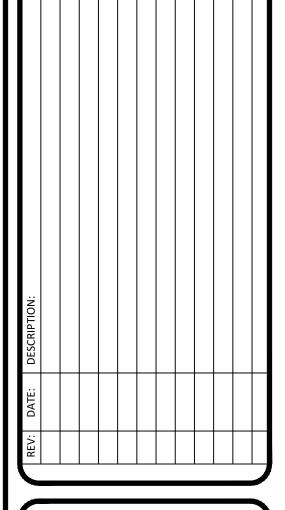


CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

RANCH

CHALK





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

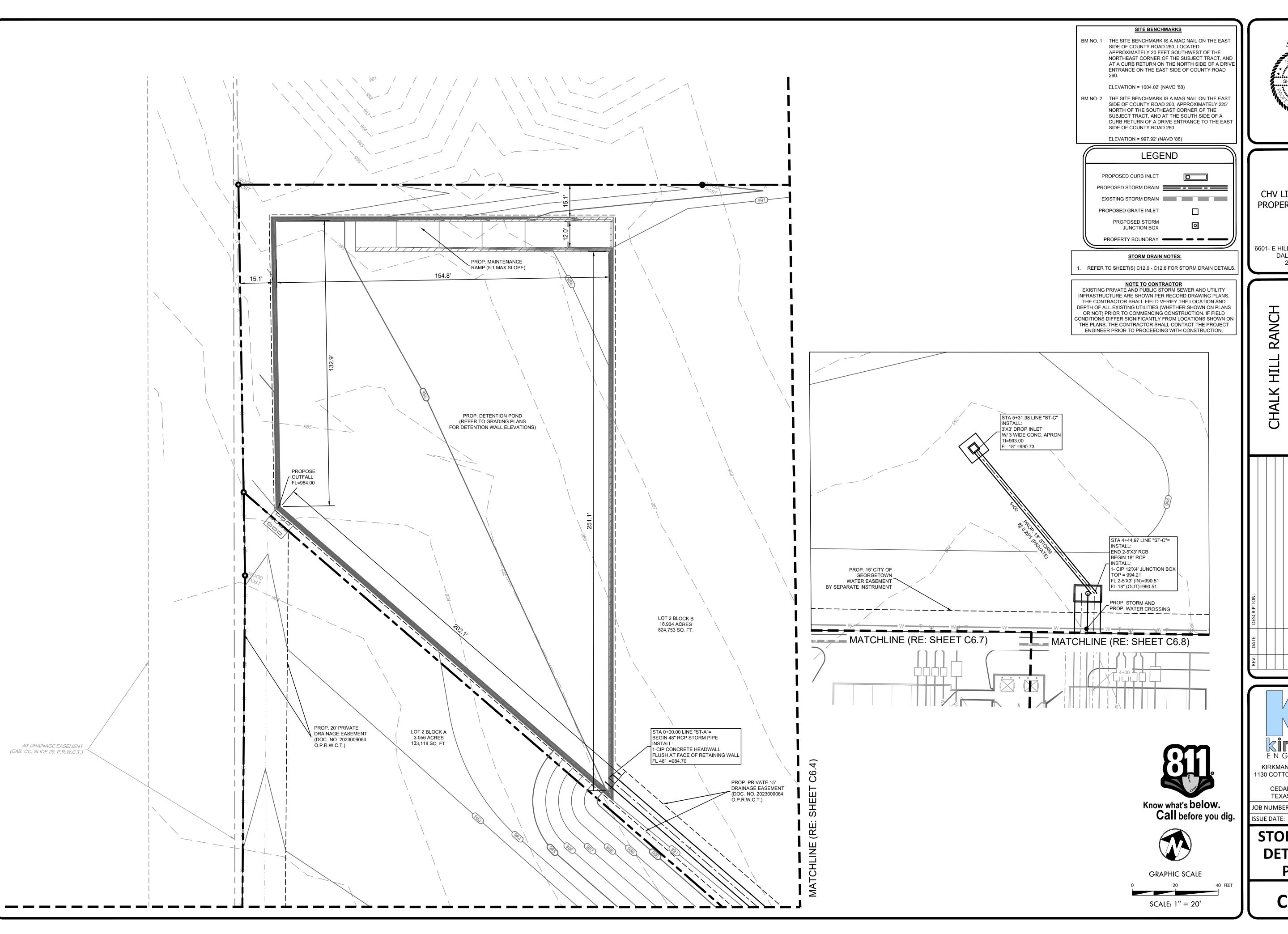
ISSUE DATE:

STORM PLAN IX

C6.11

Know what's **below. Call** before you dig.

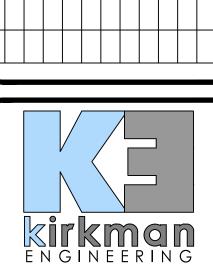
GRAPHIC SCALE SCALE: 1" = 20'



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

STORM PLAN DETENTION POND

PROFILE ST-A

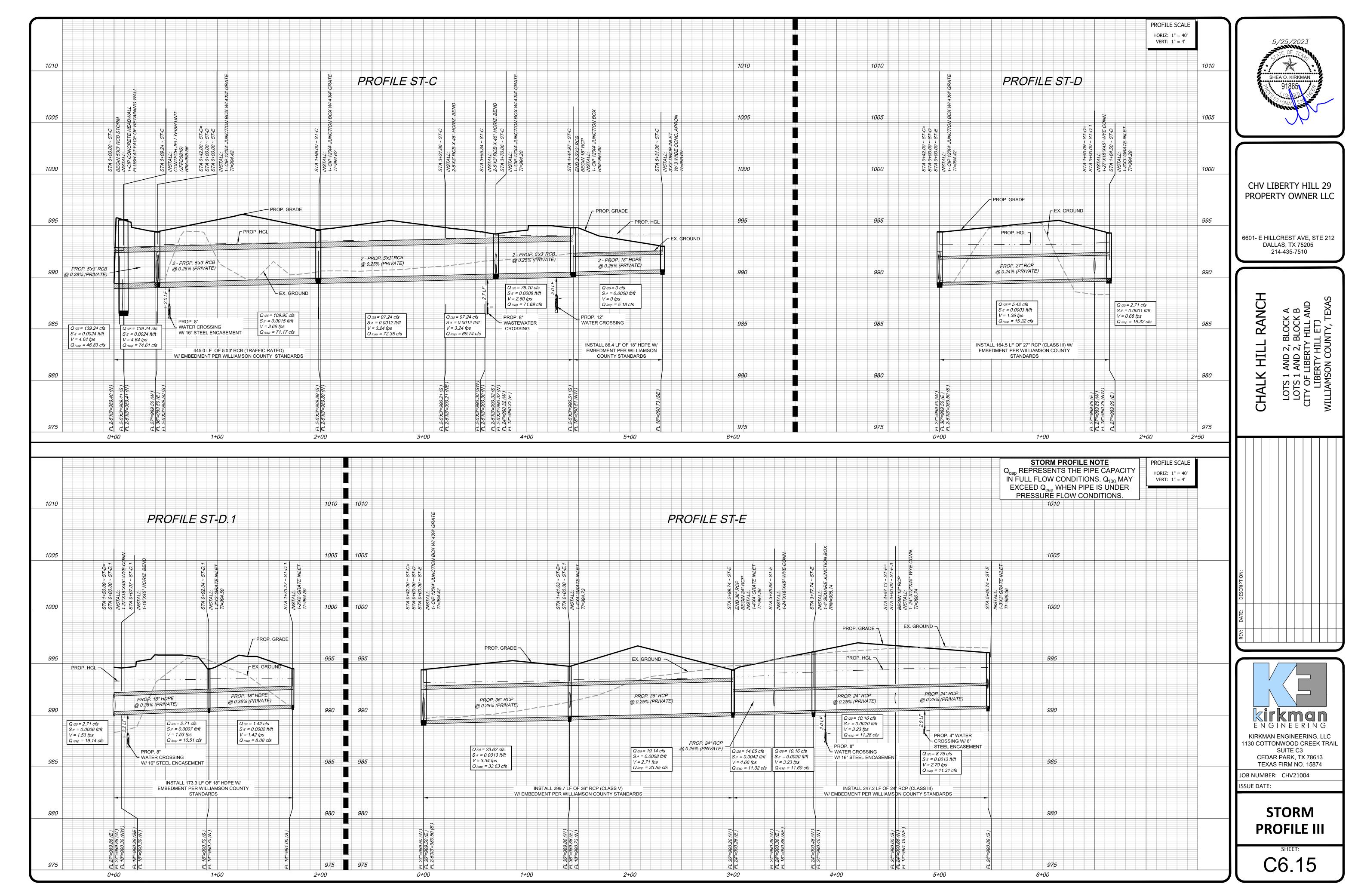
PROFILE SCALE

HORIZ: 1" = 40' VERT: 1" = 4'

KIRKMAN ENGINEERING, LLC

STORM PROFILE NOTE

PROFILE SCALE



STORM PROFILE NOTE

PROFILE SCALE

FILENAME: C6.2 STORM PLAN_CHV21004.dwg PI OTTED RY: Prachantika Gautam

32.29 0.00 32.29

0.00 9.82 0.00 9.82 circular 1

STARTING HGL NOTE: HYDRAULIC CALCULATIONS (FUTURE / ULTIMATE) STARTING HGL TAKEN FROM Runoff Invert Elevation Pipe Drainage Head Loss Calculations Comments 100-YEAR WATER SURFACE Design Point Length Q 25-yr Incremental Uncaptured Q25-yr Shape Upstrm Dwnstrm TC / FG Area Manning Hydr Hydraulic Grade | Head Loss | Design **ELEVATION FROM PROPOSED** Upstrm Dwnstrm Between Runoff Q Bypass in Pipe of No. of n Grade Line Elevation - 25-yr V25-yr at Structure HGL Pipe Box Culvert DETENTION PONDS. Value "Sf" Upstrm Dwnstrm Flow (In) Flow (Out) V1^2/2g Station Station Points "Q" "Q" V2^2/2g Coeff KjV1²/2g "Hk" Elevation Pipe Barrels Diameter Width Heigth (feet) (feet) (feet) (f.p.s.) (f.p.s.) (feet) (c.f.s.) (c.f.s.) (c.f.s.) (Kj) (feet) (feet) (ft) 11 12 14 15 16 4c 4d 5c 5e 6 10 18 13 1011.39 927.48 83.91 4.63 4.63 0.00 4.63 0.013 0.0004 992.94 0.03 0.10 993.04 3.96 0.03 7.81 12.43 0.00 12.43 circular 0.24 0.75 0.03 0.00 12.43 0.00 12.43 circular 1 873.88 873.88 0.00 0.013 0.0030 992.43 992.43 3.96 3.96 0.24 0.24 0.20 0.05 0.10 992.53 987.88 986.88 994.16 2.67 0.24 0.11 0.50 0.12 0.10 992.43 986.88 986.54 994.16 873.88 732.35 141.53 DA 13-14 6.40 18.84 0.00 18.84 circular 0.013 0.0008 992.33 992.21 3.96 2.77 0.11 2.77 0.12 732.35 679.44 52.91 679.44 512.77 166.67
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 994.80
 Future Connection for Lot 2
 0.00 0.00 0.00 circular 78.10 78.10 0.00 78.10 box 444.97 370.06 74.91 DA25 370.06 359.34 10.72 DA24+38+40-41 19.14 97.24 0.00 97.24 box 2 0.015 0.0012 993.92 993.91 2.60 3.24 0.11 0.16 0.50 0.05 0.11 994.03 990.32 990.30 994.20 0.015 0.0012 993.81 993.77 3.24 0.00 97.24 0.00 97.24 box 2 3.24 0.16 0.16 0.35 0.06 0.10 993.91 990.30 990.21 994.35 0.00 97.24 0.00 97.24 box 2 321.85 198.00 123.85 198.00 42.00 156.00 DA23+26+39 12.71 109.95 0.00 109.95 box 2 5 3 0.015 0.0015 993.39 993.16 3.24 3.66 0.16 0.21 0.50 0.08 0.13 993.52 989.89 989.50 994.62 3 0.015 0.0024 992.93 992.85 3.66 4.64 0.21 0.33 0.50 0.10 0.23 993.16 989.50 989.41 994.42 42.00 9.24 32.76 DA22+27-37 29.29 139.24 0.00 139.24 box 0.00 139.24 0.00 139.24 box 2 9.24 0.00 9.24 5 3 0.015 0.0024 992.60 992.58 4.64 4.64 0.33 0.33 0.25 0.08 0.25 992.85 989.41 989.40 994.51 0.013 0.0001 993.31 993.31 0.01 1.25 161.50 147.09 14.41 2.71 2.71 0.00 2.71 circular 0.10 993.41 989.90 989.86 994.29 147.09 0.00 147.09 DA 28-29 2.71 5.42 0.00 5.42 circular 0.10 993.31 989.86 989.50 994.70 0.013 0.0003 993.21 993.16 0.68 1.36 0.01 0.03 0.75 0.01 0.012 0.0002 993.58 993.57 173.27 92.04 81.23 0.10 993.68 991.32 1.42 1.42 0.00 1.42 0.01 1.25 1.53 0.01 92.04 7.07 84.97 1.30 2.71 0.00 2.71 circular 0.013 | 0.0007 | 993.47 | 993.41 | 0.80 0.04 1.25 0.01 0.10 993.57 990.91 990.06 994.50 7.07 0.00 7.07 0.00 2.71 0.00 2.71 circular 1 546.74 457.13 89.61 0.15 994.77 993.82 990.46 996.45 DA33+36 8.75 8.75 0.00 8.75 circular 0.012 0.0013 994.62 994.51 0.12 1.25 457.13 377.74 79.39 3.23 0.12 DA34+35 1.41 10.16 0.00 10.16 circular 0.013 0.0020 994.41 994.25 2.79 0.16 0.75 0.09 0.10 994.51 990.46 990.46 996.80 0.00 10.16 0.00 10.16 circular 3.23 0.16 0.16 0.50 0.08 0.10 994.25 990.46 990.36 996.14 0.013 0.0020 994.15 994.07 3.23 377.74 339.68 38.06 4.66 0.16 0.34 0.75 0.12 0.22 994.07 990.36 990.26 995.40 339.68 299.74 39.94 4.49 14.65 0.00 14.65 circular 0.013 0.0042 993.85 993.69 3.23 2.71 0.34 0.11 0.50 0.17 299.74 141.63 158.11 DA31 4.49 19.14 0.00 19.14 circular 0.013 0.0008 993.59 993.46 4.66 0.10 993.69 990.26 989.86 994.38 141.63 0.00 141.63 DA30+37 4.48 23.62 0.00 23.62 circular 1 0.013 | 0.0013 | 993.34 | 993.16 | 2.71 | 3.34 | 0.11 | 0.17 | 0.50 | 0.06 | 0.12 993.46 989.86 989.50 994.73 238.23 226.82 11.41 1.87 1.87 0.00 1.87 circular 0.02 1.25 0.10 994.02 992.70 992.64 995.00 226.82 147.27 79.55 0.00 1.87 0.00 1.87 circular 18 1.06 0.02 0.02 0.35 0.01 0.10 993.92 992.64 992.25 996.30 0.012 0.0003 993.82 993.80 1.06 147.27 140.76 6.51 0.00 1.87 0.00 1.87 circular 0.012 0.0003 993.70 993.69 1.06 1.06 0.02 0.02 0.35 0.01 0.10 993.80 992.25 992.22 996.10 0.00 1.87 0.00 1.87 circular 140.76 134.26 6.50 18 0.012 0.0003 993.59 993.59 1.06 1.06 0.02 0.02 0.35 0.01 0.10 993.69 992.22 992.18 995.90 134.26 0.00 134.26 0.00 1.87 0.00 1.87 circular 18 1.06 0.02 0.02 0.50 0.01 0.10 993.59 992.18 991.51 995.70 0.013 0.0001 994.35 994.35 0.10 994.45 991.53 991.14 994.60 66.92 27.74 0.01 DA32 1.28 1.28 0.00 1.28 circular 1.25 27.74 0.00 27.74 0.00 1.28 0.00 1.28 circular 18 0.013 | 0.0001 | 994.25 | 994.25 | 0.73 0.73 0.01 0.01 0.35 0.00 0.10 994.35 991.14 990.86 995.80 0.95 0.95 0.00 0.95 circular 0.02 140.59 55.99 0.013 0.0007 994.78 994.78 0.10 994.88 991.53 992.25 995.73 55.99 7.07 48.92 0.46 1.41 0.00 1.41 circular 0.012 0.0013 994.68 994.62 1.21 1.79 0.02 0.05 0.50 0.01 0.10 994.78 992.25 991.14 996.40 7.07 0.00 7.07 0.013 | 0.0016 | 994.52 | 994.51 | 1.79 | 1.79 | 0.05 | 0.05 | 0.35 | 0.02 | 0.10 | 994.62 | 991.14 | 990.86 | 997.00 0.00 1.41 0.00 1.41 circular 1 12 0.10 994.77 991.57 991.02 995.05 0.013 0.0004 994.67 994.59 0.03 1.25 501.66 281.88 219.78 4.51 4.51 0.00 4.51 circular 2.22 0.03 0.08 0.50 0.02 281.88 155.88 126.00 2.45 6.96 0.00 6.96 circular 0.013 0.0009 994.49 994.37 1.44 0.10 994.59 991.02 990.71 994.74 1.83 8.79 0.00 8.79 circular 1 155.88 0.00 155.88 24 DA38 1.24 982.81 979.50 979.32 983.91 0.013 0.0231 981.56 981.25 9.82 9.82 0.00 9.82 circular 46.07 32.29 13.78 8.01 1.00 1.25

0.013 0.0231 980.90 980.15 8.01 8.01 1.00 1.00 0.35 0.35 0.35 981.25 979.32 978.90 984.10



CHV LIBERTY HILL 29
PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK HILL RANCH
LOTS 1 AND 2, BLOCK A
LOTS 1 AND 2, BLOCK B
CITY OF LIBERTY HILL AND
LIBERTY HILL ETJ
WILLIAMSON COUNTY, TEXAS

REV: DATE: DESCRIPTION:							
DATE:							
REV:							
						_	



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004
ISSUE DATE:

HYDRAULIC CALCULATIONS

	×									ULTIN	IATE DR	AINAG	E AREA	CALCU	OITA	IS					
NOAA ATLAS 1	4 Rainfall Zo	ne:			Williams	son Cou	unty - Sa	n Gabriel	River												
Area No.	Acres	T _c (min.)	C ₂	l ₂ (in./hr.)	Q ₂ (c.f.s.)	C ₅	l ₅ (in./hr.)	Q ₅ (c.f.s.)	C ₁₀	l ₁₀ (in./hr.)	Q ₁₀ (c.f.s.)	C ₂₅	l ₂₅ (in./hr.)	Q ₂₅ (c.f.s.)	C ₅₀	l ₅₀ (in./hr.)	Q ₅₀ (c.f.s.)	C ₁₀₀	l ₁₀₀ (in./hr.)	Q ₁₀₀ (c.f.s.)	Comments
DA1	3.40	5.00	0.63	6.06	13.07	0.68	7.58	17.50	0.71	8.85	21.33	0.76	10.61	27.27	0.79	11.97	32.31	0.84	13.34	38.14	ULTIMATE CONDITION: 75% IMPERVIOUS
DA2	3.93	5.00	0.64	6.06	15.14	0.68	7.58	20.28	0.71	8.85	24.71	0.76	10.61	31.59	0.80	11.97	37.43	0.84	13.34	44.18	ULTIMATE CONDITION: 75% IMPERVIOUS
DA3	1.18	5.00	0.66	6.06	4.73	0.71	7.58	6.33	0.74	8.85	7.70	0.79	10.61	9.82	0.83	11.97	11.63	0.87	13.34	13.71	
DA4	9.84	5.00	0.68	6.06	40.63	0.73	7.58	54.31	0.76	8.85	66.00	0.81	10.61	84.16	0.84	11.97	99.54	0.89	13.34	117.24	ULTIMATE CONDITION: 85% IMPERVIOUS
DA5	0.44	5.00	0.67	6.06	1.78	0.71	7.58	2.37	0.75	8.85	2.90	0.79	10.61	3.71	0.83	11.97	4.38	0.88	13.34	5.17	
DA6	0.58	5.00	0.64	6.06	2.25	0.68	7.58	3.01	0.71	8.85	3.67	0.76	10.61	4.69	0.80	11.97	5.55	0.85	13.34	6.55	
DA7	0.35	5.00	0.67	6.06	1.42	0.72	7.58	1.90	0.75	8.85	2.31	0.79	10.61	2.95	0.83	11.97	3.49	0.88	13.34	4.11	
DA8	0.55	5.00	0.73	6.06	2.43	0.77	7.58	3.21	0.81	8.85	3.94	0.86	10.61	5.02	0.90	11.97	5.93	0.95	13.34	6.97	
DA9	0.51	5.00	0.71	6.06	2.20	0.75	7.58	2.93	0.78	8.85	3.56	0.83	10.61	4.54	0.87	11.97	5.36	0.92	13.34	6.31	
DA10	0.42	5.00	0.61	6.06	1.55	0.65	7.58	2.08	0.68	8.85	2.53	0.73	10.61	3.24	0.77	11.97	3.85	0.81	13.34	4.55	
DA11	0.28	5.00	0.37	6.06	0.63	0.41	7.58	0.86	0.44	8.85	1.08	0.48	10.61	1.42	0.51	11.97	1.71	0.55	13.34	2.06	
DA12	0.15	5.00	0.38	6.06	0.35	0.41	7.58	0.47	0.44	8.85	0.59	0.49	10.61	0.77	0.52	11.97	0.93	0.56	13.34	1.12	
DA13	0.43	5.00	0.70	6.06	1.82	0.74	7.58	2.43	0.77	8.85	2.95	0.82	10.61	3.76	0.86	11.97	4.44	0.91	13.34	5.23	
DA14	0.29	5.00	0.73	6.06	1.28	0.77	7.58	1.69	0.81	8.85	2.08	0.86	10.61	2.65	0.90	11.97	3.13	0.95	13.34	3.67	
DA15	0.97	5.00	0.64	6.06	3.74	0.68	7.58	5.01	0.71	8.85	6.11	0.76	10.61	7.81	0.80	11.97	9.25	0.84	13.34	10.92	ULTIMATE CONDITION: 75% IMPERVIOUS
DA16	0.88	5.00	0.39	6.06	2.08	0.42	7.58	2.83	0.45	8.85	3.53	0.50	10.61	4.63	0.53	11.97	5.56	0.57	13.34	6.69	ULTIMATE CONDITION: 75% IMPERVIOUS
DA17	0.30	5.00	0.69	6.06	1.25	0.74	7.58	1.67	0.77	8.85	2.03	0.81	10.61	2.59	0.85	11.97	3.07	0.90	13.34	3.61	
DA18	0.57	5.00	0.64	6.06	2.22	0.69	7.58	2.97	0.72	8.85	3.62	0.77	10.61	4.63	0.80	11.97	5.49	0.85	13.34	6.47	
DA19	0.20	5.00	0.50	6.06	0.60	0.54	7.58	0.81	0.57	8.85	1.00	0.61	10.61	1.30	0.65	11.97	1.54	0.69	13.34	1.84	
DA20	0.23	5.00	0.47	6.06	0.65	0.51	7.58	0.88	0.54	8.85	1.09	0.58	10.61	1.42	0.61	11.97	1.69	0.66	13.34	2.02	
DA21	0.56	5.00	0.73	6.06	2.48	0.77	7.58	3.27	0.81	8.85	4.01	0.86	10.61	5.11	0.90	11.97	6.03	0.95	13.34	7.10	
DA22	0.41	5.00	0.67	6.06	1.67	0.72	7.58	2.23	0.75	8.85	2.71	0.79	10.61	3.46	0.83	11.97	4.09	0.88	13.34	4.82	
DA23	0.30	5.00	0.66	6.06	1.20	0.70	7.58	1.60	0.73	8.85	1.95	0.78	10.61	2.49	0.82	11.97	2.95	0.87	13.34	3.47	
DA24	1.39	5.00	0.59	6.06	4.92	0.63	7.58	6.60	0.66	8.85	8.07	0.70	10.61	10.36	0.74	11.97	12.29	0.79	13.34	14.55	
DA25	9.72	5.00	0.64	6.06	37.43	0.68	7.58	50.13	0.71	8.85	61.08	0.76	10.61	78.10	0.80	11.97	92.53	0.84	13.34	109.23	ULTIMATE CONDITION: 75% IMPERVIOUS
DA26	0.56	5.00	0.73	6.06	2.48	0.77	7.58	3.27	0.81	8.85	4.01	0.86	10.61	5.11	0.90	11.97	6.03	0.95	13.34	7.10	
DA27	0.34	5.00	0.63	6.06	1.30	0.67	7.58	1.73	0.70	8.85	2.12	0.75	10.61	2.71	0.79	11.97	3.21	0.83	13.34	3.79	
DA28	0.20	5.00	0.50	6.06	0.60	0.54	7.58	0.81	0.57	8.85	1.00	0.61	10.61	1.30	0.65	11.97	1.54	0.69	13.34	1.84	
DA29	0.23	5.00	0.47	6.06	0.65	0.51	7.58	0.88	0.54	8.85	1.09	0.58	10.61	1.42	0.61	11.97	1.69	0.66	13.34	2.02	
DA30	0.32	5.00	0.65	6.06	1.26	0.69	7.58	1.68	0.72	8.85	2.05	0.77	10.61	2.61	0.81	11.97	3.10	0.86	13.34	3.65	
DA31	0.53	5.00	0.68	6.06	2.17	0.72	7.58	2.89	0.76	8.85	3.52	0.81	10.61	4.49	0.84	11.97	5.31	0.89	13.34	6.26	
DA32	0.31	5.00	0.29	6.06	0.55	0.32	7.58	0.75	0.35	8.85	0.96	0.39	10.61	1.28	0.42	11.97	1.56	0.46	13.34	1.90	
DA33	0.42	5.00	0.69	6.06	1.76	0.74	7.58	2.35	0.77	8.85	2.86	0.82	10.61	3.65	0.86	11.97	4.31	0.91	13.34	5.08	
DA34	0.23	5.00	0.29	6.06	0.40	0.32	7.58	0.56	0.35	8.85	0.71	0.39	10.61	0.95	0.42	11.97	1.16	0.46	13.34	1.41	
DA35	0.11	5.00	0.29	6.06	0.19	0.32	7.58	0.27	0.35	8.85	0.34	0.39	10.61	0.46	0.42	11.97	0.55	0.46	13.34	0.67	
DA36	0.56	5.00	0.73	6.06	2.48	0.77	7.58	3.27	0.81	8.85	4.01	0.86	10.61	5.11	0.90	11.97	6.03	0.95	13.34	7.10	
DA37	0.38	5.00	0.36	6.06	0.83	0.39	7.58	1.13	0.42	8.85	1.42	0.46	10.61	1.87	0.49	11.97	2.25	0.53	13.34	2.72	
DA38	0.24	5.00	0.60	6.06	0.87	0.65	7.58	1.17	0.68	8.85	1.43	0.73	10.61	1.83	0.76		2.17	0.81	13.34	2.56	
DA39	0.56	5.00	0.73	6.06	2.48	0.77	7.58	3.27	0.81	8.85	4.01	0.86	10.61	5.11	0.90	11.97	6.03	0.95	13.34	7.10	
DA40	0.34	5.00	0.56	6.06	1.16	0.60	7.58	1.55	0.63	8.85	1.90	0.68	10.61	2.45	0.71	11.97	2.91	0.76	13.34	3.45	
DA41	0.59	5.00	0.60	6.06	2.15	0.64	7.58	2.88	0.67	8.85	3.52	0.72	10.61	4.51	0.76	11.97	5.35	0.80	13.34	6.33	
DA42	1.08	5.00	0.29	6.06	1.90	0.32	7.58	2.62	0.35	8.85	3.35	0.39	10.61	4.47	0.42	11.97	5.43	0.46	13.34	6.63	
DA43	0.60	5.00	0.38	6.06	1.39	0.42	7.58	1.89	0.45	8.85	2.37	0.49	10.61	3.11	0.52	11.97	3.74	0.56	13.34	4.50	
DA44	3.20	5.00	0.66	6.06	12.80	0.71	7.58	17.12	0.74	8.85	20.83	0.78	10.61	26.60	0.82	11.97	31.48	0.87	13.34	37.12	ULTIMATE CONDITION: 85% IMPERVIOUS



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK HILL



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

PROPOSED DRAINAGE AREA **CALCULATIONS**

ON COMPOSIT C VALUE CALCULATION

Total / Composite

Developed Concrete

Developed Grass Good (>75% grass cover) 2-7%

Developed Asphalt

Total / Composite

Developed Concrete

Developed Asphalt

Total / Composite

Developed Concrete

Developed Asphalt

Total / Composite

Developed Asphalt

Total / Composite

Developed Asphalt

Total / Composite

Developed Concrete
Developed Asphalt

Total / Composite

Developed Concrete

Developed Asphalt

Total / Composite

Developed Concrete
Developed Asphalt

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Developed Asphalt

Total / Composite

Developed Concrete
Developed Asphalt

Total / Composite

Developed Concrete

Developed Asphalt

Total / Composite

Developed Asphalt

Total / Composite

Developed Asphalt

Total / Composite

Surface Type

Developed Concrete

Total / Composite

Developed Concrete

Developed Asphalt

Total / Composite

Developed Concrete

Developed Asphalt

Total / Composite

Developed Concrete

Developed Asphalt

Surface Type

DA22 Total / Composite

Surface Type

Surface Type

Surface Type
Developed Concrete

Surface Type
Developed Concrete

Surface Type

DA16

Surface Type

Surface Type

Surface Type

Surface Type

Surface Type

Surface Type
Developed Concrete

Surface Type
Developed Concrete

Surface Type

Surface Type

Surface Type

DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	0.00	0.75	0.00	0.80	0.00	0.83	0.00	0.88	0.00	0.92	0.00	0.97	0.00	1.00	0.00
	Developed Asphalt	7.06	0.73	5.15	0.77	5.44	0.81	5.72	0.86	6.07	0.90	6.35	0.95	6.71	1.00	7.06
	Developed Grass Good (>75% grass cover) 2-7%	0.00	0.29	0.00	0.32	0.00	0.35	0.00	0.39	0.00	0.42	0.00	0.46	0.00	0.56	0.00
EX1	Total / Composite	7.06	0.73	5.15	0.77	5.44	0.81	5.72	0.86	6.07	0.90	6.35	0.95	6.71	1.00	7.06
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	0.24	0.75	0.18	0.80	0.19	0.83	0.20	0.88	0.21	0.92	0.22	0.97	0.23	1.00	0.24
	Developed Asphalt	12.45	0.73	9.09	0.77	9.59	0.81	10.08	0.86	10.71	0.90	11.21	0.95	11.83	1.00	12.4
	Developed Grass Good (>75% grass cover) 2-7%	0.00	0.29	0.00	0.32	0.00	0.35	0.00	0.39	0.00	0.42	0.00	0.46	0.00	0.56	0.00
EX2	Total / Composite	12.69	0.73	9.27	0.77	9.78	0.81	10.28	0.86	10.92	0.90	11.43	0.95	12.06	1.00	12.6
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	1.22	0.75	0.92	0.80	0.98	0.83	1.01	0.88	1.07	0.92	1.12	0.97	1.18	1.00	1.22
	Developed Asphalt	4.98	0.73	3.64	0.77	3.83	0.81	4.03	0.86	4.28	0.90	4.48	0.95	4.73	1.00	4.98
	Developed Grass Good (>75% grass cover) 2-7%	0.00	0.29	0.00	0.32	0.00	0.35	0.00	0.39	0.00	0.42	0.00	0.46	0.00	0.56	0.00
EX3	Total / Composite	6.20	0.73	4.55	0.78	4.81	0.81	5.05	0.86	5.36	0.90	5.60	0.95	5.91	1.00	6.20
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	0.44	0.75	0.33	0.80	0.35	0.83	0.37	0.88	0.39	0.92	0.40	0.97	0.43	1.00	0.44
	Developed Asphalt	22.09	0.73	16.13	0.77	17.01	0.81	17.89	0.86	19.00	0.90	19.88	0.95	20.99	1.00	22.0
	Developed Grass Good (>75% grass cover) 2-7%	0.00	0.29	0.00	0.32	0.00	0.35	0.00	0.39	0.00	0.42	0.00	0.46	0.00	0.56	0.00
EX4	Total / Composite	22.53	0.73	16.46	0.77	17.36	0.81	18.26	0.86	19.38	0.90	20.29	0.95	21.41	1.00	22.5

		omposite	C Va	lue Ca	lculati	ons										
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	2.55	0.75	1.91	0.80	2.04	0.83	2.11	0.88	2.24	0.92	2.34	0.97	2.47	1.00	2.55
	Developed Asphalt	0.00	0.73	0.00	0.77	0.00	0.81	0.00	0.86	0.00	0.90	0.00	0.95	0.00	1.00	0.00
	Developed Grass Good (>75% grass cover) 2-7%	0.85	0.29	0.25	0.32	0.27	0.35	0.30	0.39	0.33	0.42	0.36	0.46	0.39	0.56	0.48
DA1	Total / Composite	3.40	0.63	2.16	0.68	2.31	0.71	2.41	0.76	2.57	0.79	2.70	0.84	2.86	0.89	3.02
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	2.95	0.75	2.21	0.80	2.36	0.83	2.45	0.88	2.60	0.92	2.71	0.97	2.86	1.00	2.95
	Developed Asphalt	0.00	0.73	0.00	0.77	0.00	0.81	0.00	0.86	0.00	0.90	0.00	0.95	0.00	1.00	0.00
	Developed Grass Good (>75% grass cover) 2-7%	0.98	0.29	0.28	0.32	0.31	0.35	0.34	0.39	0.38	0.42	0.41	0.46	0.45	0.56	0.55
DA2	Total / Composite	3.93	0.64	2.50	0.68	2.67	0.71	2.79	0.76	2.98	0.80	3.13	0.84	3.31	0.89	3.50
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	0.96	0.75	0.72	0.80	0.76	0.83	0.79	0.88	0.84	0.92	0.88	0.97	0.93	1.00	0.96
	Developed Asphalt	0.00	0.73	0.00	0.77	0.00	0.81	0.00	0.86	0.00	0.90	0.00	0.95	0.00	1.00	0.00
	Developed Grass Good (>75% grass cover) 2-7%	0.22	0.29	0.06	0.32	0.07	0.35	0.08	0.39	0.09	0.42	0.09	0.46	0.10	0.56	0.12
DA3	Total / Composite	1.18	0.66	0.78	0.71	0.83	0.74	0.87	0.79	0.93	0.83	0.97	0.87	1.03	0.92	1.08
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	8.36	0.75	6.27	0.80	6.69	0.83	6.94	0.88	7.36	0.92	7.69	0.97	8.11	1.00	8.36
	Developed Asphalt	0.00	0.73	0.00	0.77	0.00	0.81	0.00	0.86	0.00	0.90	0.00	0.95	0.00	1.00	0.00
	Developed Grass Good (>75% grass cover) 2-7%	1.48	0.29	0.43	0.32	0.47	0.35	0.52	0.39	0.58	0.42	0.62	0.46	0.68	0.56	0.83
DA4	Total / Composite	9.84	0.68	6.70	0.73	7.16	0.76	7.46	0.81	7.93	0.84	8.31	0.89	8.79	0.93	9.19
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete	0.19	0.75	0.14	0.80	0.15	0.83	0.16	0.88	0.17	0.92	0.17	0.97	0.18	1.00	0.19
	Developed Asphalt	0.18	0.73	0.13	0.77	0.14	0.81	0.15	0.86	0.15	0.90	0.16	0.95	0.17	1.00	0.18
	Developed Grass Good (>75% grass cover) 2-7%	0.07	0.29	0.02	0.32	0.02	0.35	0.02	0.39	0.03	0.42	0.03	0.46	0.03	0.56	0.04

0.44 0.67 0.29 0.71 0.31 0.75 0.33 0.79 0.35 0.83 0.37 0.88 0.39 0.93 0.41

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.44 0.75 0.33 0.80 0.35 0.83 0.37 0.88 0.39 0.92 0.40 0.97 0.43 1.00 0.44 0.00 0.73 0.00 0.77 0.00 0.81 0.00 0.86 0.00 0.90 0.00 0.95 0.00 1.00 0.00

0.14 0.29 0.04 0.32 0.04 0.35 0.05 0.39 0.05 0.42 0.06 0.46 0.06 0.56 0.08 0.58 0.64 0.37 0.68 0.40 0.71 0.41 0.76 0.44 0.80 0.46 0.85 0.49 0.89 0.52

 Area
 C2
 C*A
 C5
 C*A
 C10
 C*A
 C25
 C*A
 C50
 C*A
 C100
 C*A
 C500
 C*A

 0.26
 0.75
 0.20
 0.80
 0.21
 0.83
 0.22
 0.88
 0.23
 0.92
 0.24
 0.97
 0.25
 1.00
 0.26

0.03 0.73 0.02 0.77 0.02 0.81 0.02 0.86 0.03 0.90 0.03 0.95 0.03 1.00 0.03

0.06 0.29 0.02 0.32 0.02 0.35 0.02 0.39 0.02 0.42 0.03 0.46 0.03 0.56 0.03

0.35 | 0.67 | 0.23 | 0.72 | 0.25 | 0.75 | 0.26 | 0.79 | 0.28 | 0.83 | 0.29 | 0.88 | 0.31 | 0.92 | 0.32

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₆₀₀ C*A C

0.55 0.73 0.40 0.77 0.42 0.81 0.45 0.86 0.47 0.90 0.50 0.95 0.52 1.00 0.55

0.00 0.29 0.00 0.32 0.00 0.35 0.00 0.39 0.00 0.42 0.00 0.46 0.00 0.56 0.00

0.55 0.73 0.40 0.77 0.42 0.81 0.45 0.86 0.47 0.90 0.50 0.95 0.52 1.00 0.55

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.05 | 0.29 | 0.01 | 0.32 | 0.02 | 0.35 | 0.02 | 0.39 | 0.02 | 0.42 | 0.02 | 0.46 | 0.02 | 0.56 | 0.03

0.51 | 0.71 | 0.36 | 0.75 | 0.39 | 0.78 | 0.40 | 0.83 | 0.43 | 0.87 | 0.45 | 0.92 | 0.47 | 0.96 | 0.49

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

 0.29
 0.75
 0.22
 0.80
 0.23
 0.83
 0.24
 0.88
 0.26
 0.92
 0.27
 0.97
 0.28
 1.00
 0.29

 0.00
 0.73
 0.00
 0.77
 0.00
 0.81
 0.00
 0.86
 0.00
 0.90
 0.00
 0.95
 0.00
 1.00
 0.00

0.13 0.29 0.04 0.32 0.04 0.35 0.05 0.39 0.05 0.42 0.05 0.46 0.06 0.56 0.07 0.42 0.61 0.26 0.65 0.27 0.68 0.29 0.73 0.31 0.77 0.32 0.81 0.34 0.86 0.36

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.05 0.75 0.04 0.80 0.04 0.83 0.04 0.88 0.04 0.92 0.05 0.97 0.05 1.00 0.05

0.00 0.73 0.00 0.77 0.00 0.81 0.00 0.86 0.00 0.90 0.00 0.95 0.00 1.00 0.00 0.23 0.29 0.07 0.32 0.07 0.35 0.08 0.39 0.09 0.42 0.10 0.46 0.11 0.56 0.13

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A C₁₀₀ C*A C₅₀₀ C*A C₁₀₀ C*A C₅₀₀ C*A C

0.01 0.73 0.01 0.77 0.01 0.81 0.01 0.86 0.01 0.90 0.01 0.95 0.01 1.00 0.01

0.12 0.29 0.03 0.32 0.04 0.35 0.04 0.39 0.05 0.42 0.05 0.46 0.06 0.56 0.07 0.15 0.38 0.06 0.41 0.06 0.44 0.07 0.49 0.07 0.52 0.08 0.56 0.08 0.65 0.10

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.34 0.75 0.26 0.80 0.27 0.83 0.28 0.88 0.30 0.92 0.31 0.97 0.33 1.00 0.34

 0.04
 0.73
 0.03
 0.77
 0.03
 0.81
 0.03
 0.86
 0.03
 0.90
 0.04
 0.95
 0.04
 1.00
 0.04

 0.05
 0.29
 0.01
 0.32
 0.02
 0.35
 0.02
 0.39
 0.02
 0.42
 0.02
 0.46
 0.02
 0.56
 0.03

 0.43
 0.70
 0.30
 0.74
 0.32
 0.77
 0.33
 0.82
 0.35
 0.86
 0.37
 0.91
 0.39
 0.95
 0.41

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.00 0.75 0.00 0.80 0.00 0.83 0.00 0.88 0.00 0.92 0.00 0.97 0.00 1.00 0.00 0.29 0.73 0.21 0.77 0.22 0.81 0.23 0.86 0.25 0.90 0.26 0.95 0.28 1.00 0.29

0.00 0.29 0.00 0.32 0.00 0.35 0.00 0.39 0.00 0.42 0.00 0.46 0.00 0.56 0.00 0.29 0.73 0.21 0.77 0.22 0.81 0.23 0.86 0.25 0.90 0.26 0.95 0.28 1.00 0.29

Area C₂ C*A C₅ C*A C₁₀₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A 0.73 0.75 0.55 0.80 0.58 0.83 0.61 0.88 0.64 0.92 0.67 0.97 0.71 1.00 0.73

0.00 | 0.73 | 0.00 | 0.77 | 0.00 | 0.81 | 0.00 | 0.86 | 0.00 | 0.90 | 0.00 | 0.95 | 0.00 | 1.00 | 0.00

 0.24
 0.29
 0.07
 0.32
 0.08
 0.35
 0.08
 0.39
 0.09
 0.42
 0.10
 0.46
 0.11
 0.56
 0.13

 0.97
 0.64
 0.62
 0.68
 0.66
 0.71
 0.69
 0.76
 0.74
 0.80
 0.77
 0.84
 0.82
 0.89
 0.86

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.19 0.75 0.14 0.80 0.15 0.83 0.16 0.88 0.17 0.92 0.17 0.97 0.18 1.00 0.19

0.00 | 0.73 | 0.00 | 0.77 | 0.00 | 0.81 | 0.00 | 0.86 | 0.00 | 0.90 | 0.00 | 0.95 | 0.00 | 1.00 | 0.00

 0.69
 0.29
 0.20
 0.32
 0.22
 0.35
 0.24
 0.39
 0.27
 0.42
 0.29
 0.46
 0.32
 0.56
 0.39

 0.88
 0.39
 0.34
 0.42
 0.37
 0.45
 0.40
 0.50
 0.44
 0.53
 0.46
 0.57
 0.50
 0.66
 0.58

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.26 0.75 0.20 0.80 0.21 0.83 0.22 0.88 0.23 0.92 0.24 0.97 0.25 1.00 0.26 0.00 0.73 0.00 0.77 0.00 0.81 0.00 0.86 0.00 0.90 0.00 0.95 0.00 1.00 0.00

 0.04
 0.29
 0.01
 0.32
 0.01
 0.35
 0.01
 0.39
 0.02
 0.42
 0.02
 0.46
 0.02
 0.56
 0.02

 0.30
 0.69
 0.21
 0.74
 0.22
 0.77
 0.23
 0.81
 0.24
 0.85
 0.26
 0.90
 0.27
 0.94
 0.28

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.38 | 0.75 | 0.29 | 0.80 | 0.30 | 0.83 | 0.32 | 0.88 | 0.33 | 0.92 | 0.35 | 0.97 | 0.37 | 1.00 | 0.38

0.06 0.73 0.04 0.77 0.05 0.81 0.05 0.86 0.05 0.90 0.05 0.95 0.06 1.00 0.06

0.13 0.29 0.04 0.32 0.04 0.35 0.05 0.39 0.05 0.42 0.05 0.46 0.06 0.56 0.07

0.57 | 0.64 | 0.37 | 0.69 | 0.39 | 0.72 | 0.41 | 0.77 | 0.44 | 0.80 | 0.46 | 0.85 | 0.49 | 0.90 | 0.51

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₆₀₀ C*A C

0.00 0.73 0.00 0.77 0.00 0.81 0.00 0.86 0.00 0.90 0.00 0.95 0.00 1.00 0.00 0.11 0.29 0.03 0.32 0.04 0.35 0.04 0.39 0.04 0.42 0.05 0.46 0.05 0.56 0.06

0.20 0.50 0.10 0.54 0.11 0.57 0.11 0.61 0.12 0.65 0.13 0.69 0.14 0.76 0.15

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.08 | 0.75 | 0.06 | 0.80 | 0.06 | 0.83 | 0.07 | 0.88 | 0.07 | 0.92 | 0.07 | 0.97 | 0.08 | 1.00 | 0.08

0.01 0.73 0.01 0.77 0.01 0.81 0.01 0.86 0.01 0.90 0.01 0.95 0.01 1.00 0.01

0.14 0.29 0.04 0.32 0.04 0.35 0.05 0.39 0.05 0.42 0.06 0.46 0.06 0.56 0.08

0.23 0.47 0.11 0.51 0.12 0.54 0.12 0.58 0.13 0.61 0.14 0.66 0.15 0.73 0.17

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.00 0.75 0.00 0.80 0.00 0.83 0.00 0.88 0.00 0.92 0.00 0.97 0.00 1.00 0.00 0.56 0.73 0.41 0.77 0.43 0.81 0.45 0.86 0.48 0.90 0.50 0.95 0.53 1.00 0.56

0.00 0.29 0.00 0.32 0.00 0.35 0.00 0.39 0.00 0.42 0.00 0.46 0.00 0.56 0.00

0.56 | 0.73 | 0.41 | 0.77 | 0.43 | 0.81 | 0.45 | 0.86 | 0.48 | 0.90 | 0.50 | 0.95 | 0.53 | 1.00 | 0.56

Area C₂ C*A C₅ C*A C₁₀ C*A C₂₅ C*A C₅₀ C*A C₁₀₀ C*A C₅₀₀ C*A

0.31 0.75 0.23 0.80 0.25 0.83 0.26 0.88 0.27 0.92 0.29 0.97 0.30 1.00 0.31

0.03 0.73 0.02 0.77 0.02 0.81 0.02 0.86 0.03 0.90 0.03 0.95 0.03 1.00 0.03

0.07 | 0.29 | 0.02 | 0.32 | 0.02 | 0.35 | 0.02 | 0.39 | 0.03 | 0.42 | 0.03 | 0.46 | 0.03 | 0.56 | 0.04

0.41 0.67 0.27 0.72 0.29 0.75 0.31 0.79 0.33 0.83 0.34 0.88 0.36 0.92 0.38

COMPOSITE "C" VALUE CALCULATION
PROPOSED

				-03E												
DA	Surface Type	Composite Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
DA23	Developed Concrete Developed Asphalt Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.24 0.00 0.06 0.30	0.75 0.73 0.29 0.66	0.18 0.00 0.02 0.20	0.80 0.77 0.32 0.70	0.19 0.00 0.02 0.21	0.83 0.81 0.35 0.73	0.20 0.00 0.02 0.22	0.88 0.86 0.39 0.78	0.21 0.00 0.02 0.23	0.92 0.90 0.42 0.82	0.22 0.00 0.03 0.25	0.97 0.95 0.46 0.87	0.23 0.00 0.03 0.26	1.00 1.00 0.56 0.91	0.24 0.00 0.03 0.27
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.82 0.07	0.75	0.62 0.05	0.80	0.66 0.05	0.83	0.68	0.88	0.73 0.06	0.92	0.76 0.06	0.97	0.80 0.07	1.00	0.82
DA24	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.49	0.29	0.14	0.32	0.16 0.87	0.35	0.17	0.39	0.19	0.42	0.21	0.46	0.23	0.56	0.28
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	7.29 0.00	0.75	5.47	0.80	5.83	0.83	6.05	0.88	6.42 0.00	0.92	6.71 0.00	0.97	7.07	1.00	7.29 0.00
DA25	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	2.43 9.72	0.29	0.70	0.32	0.78	0.35	0.85	0.39	0.95 7.36	0.42	1.02 7.73	0.46	1.12	0.56	1.36 8.65
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
- DA	Developed Concrete Developed Asphalt	0.00 0.56	0.75	0.00	0.80	0.00	0.83	0.00	0.88	0.00	0.92	0.00	0.97	0.00	1.00	0.00
DA26	Developed Asphalt Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.00	0.29	0.00	0.32	0.00	0.35	0.00	0.39	0.00	0.42	0.00	0.46 0.95	0.00	0.56	0.00 0.56
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
DA.	Developed Concrete Developed Asphalt	0.19 0.06	0.75	0.14 0.05	0.80	0.15 0.05	0.83	0.16	0.88	0.17 0.05	0.92	0.17 0.06	0.97	0.18	1.00	0.19
DA27	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.09	0.29	0.03	0.32	0.03	0.35	0.03	0.39	0.03	0.42	0.04	0.46	0.04	0.56	0.05
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.09	0.75	0.07	0.80	0.07	0.83	0.07	0.88	0.08	0.92	0.08	0.97	0.09	1.00	0.09
DA28	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.11	0.29	0.03	0.32	0.04	0.35	0.04	0.39	0.04	0.42	0.05	0.46	0.05	0.56	0.06
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.08 0.01	0.75 0.73	0.06 0.01	0.80 0.77	0.06 0.01	0.83 0.81	0.07 0.01	0.88 0.86	0.07 0.01	0.92 0.90	0.07 0.01	0.97 0.95	0.08 0.01	1.00	0.08
DA29	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.14 0.23	0.29	0.04	0.32	0.04	0.35	0.05	0.39	0.05		0.06	0.46 0.66	0.06	0.56 0.73	0.08
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
E	Developed Concrete Developed Asphalt	0.21 0.04	0.75	0.16	0.80	0.17	0.83	0.17	0.88	0.18	0.92	0.19	0.97 0.95	0.20	1.00	0.21
DA30	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.07	0.29	0.02	0.32	0.02	0.35 0.72	0.02	0.39	0.03 0.25	0.42	0.03	0.46 0.86	0.03	0.56	0.04
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.32 0.13	0.75 0.73	0.24 0.09	0.80 0.77	0.26 0.10	0.83 0.81	0.27 0.11	0.88	0.28 0.11	0.92 0.90	0.30 0.12	0.97	0.31 0.12	1.00 1.00	0.32 0.13
DA31	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.07 0.53	0.29	0.02	0.32	0.02	0.35	0.03	0.39	0.03	0.42	0.03	0.46	0.03	0.56	0.04
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.00	0.75 0.73	0.00	0.80 0.77	0.00	0.83	0.00	0.88 0.86	0.00	0.92	0.00	0.97 0.95	0.00	1.00	0.00
DA32	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.31	0.29	0.09	0.32	0.10	0.35 0.35	0.11	0.39	0.12	0.42	0.13 0.13	0.46 0.46	0.14	0.56 0.56	0.17 0.17
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.30	0.75 0.73	0.23 0.05	0.80 0.77	0.24 0.05	0.83	0.25	0.88	0.26 0.06	0.92 0.90	0.28 0.06	0.97 0.95	0.29	1.00	0.30
DA33	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.05 0.42	0.29 0.69	0.01 0.29	0.32 0.74	0.02 0.31	0.35	0.02	0.39 0.82	0.02 0.34	0.42 0.86	0.02 0.36	0.46 0.91	0.02 0.38	0.56 0.95	0.03 0.40
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.00	0.75 0.73	0.00	0.80 0.77	0.00	0.83 0.81	0.00	0.88	0.00	0.92 0.90	0.00	0.97 0.95	0.00	1.00	0.00
DA34	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.23 0.23	0.29 0.29	0.07 0.07	0.32 0.32	0.07 0.07	0.35 0.35	0.08	0.39 0.39	0.09	0.42 0.42	0.10 0.10	0.46 0.46	0.11 0.11	0.56 0.56	0.13 0.13
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.00	0.75 0.73	0.00	0.80 0.77	0.00	0.83 0.81	0.00	0.88 0.86	0.00	0.92 0.90	0.00	0.97 0.95	0.00	1.00 1.00	0.00
DA35	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.11 0.11	0.29	0.03	0.32 0.32	0.04 0.04	0.35 0.35	0.04 0.04	0.39 0.39	0.04 0.04	0.42 0.42	0.05 0.05	0.46 0.46	0.05 0.05	0.56 0.56	0.06
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.00 0.56	0.75 0.73	0.00 0.41	0.80	0.00	0.83	0.00 0.45	0.88	0.00	0.92	0.00	0.97 0.95	0.00	1.00	0.00
DA36	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.00 0.56	0.29 0.73	0.00 0.41	0.32 0.77	0.00 0.43	0.35 0.81	0.00 0.45	0.39 0.86	0.00 0.48	0.42 0.90	0.00 0.50	0.46 0.95	0.00 0.53	0.56 1.00	0.00 0.56
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.06	0.75 0.73	0.04	0.80 0.77	0.04	0.83 0.81	0.05	0.88	0.05	0.92 0.90	0.05	0.97 0.95	0.05	1.00	0.06
DA37	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.33 0.38	0.29 0.36	0.09 0.14	0.32 0.39	0.10 0.15	0.35 0.42	0.11 0.16	0.39 0.46	0.13 0.18	0.42 0.49	0.14 0.19	0.46 0.53	0.15 0.20	0.56 0.62	0.18 0.24
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.13 0.03	0.75 0.73	0.10 0.02	0.80 0.77	0.10 0.03	0.83 0.81	0.11 0.03	0.88 0.86	0.11 0.03	0.92 0.90	0.12 0.03	0.97 0.95	0.13 0.03	1.00 1.00	0.13 0.03
DA38	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.07 0.24	0.29 0.60	0.02 0.14	0.32 0.65	0.02 0.15	0.35 0.68	0.03 0.16	0.39 0.73	0.03 0.17	0.42 0.76	0.03 0.18	0.46 0.81	0.03 0.19	0.56 0.86	0.04 0.21
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.00 0.56	0.75 0.73	0.00 0.41	0.80 0.77	0.00 0.43	0.83 0.81	0.00 0.45	0.88 0.86	0.00 0.48	0.92 0.90	0.00 0.50	0.97 0.95	0.00 0.53	1.00	0.00 0.56
DA39	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.00 0.56	0.29 0.73	0.00 0.41	0.32 0.77	0.00 0.43	0.35 0.81	0.00 0.45	0.39 0.86	0.00 0.48	0.42 0.90	0.00 0.50	0.46 0.95	0.00 0.53	0.56 1.00	0.00 0.56
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.20 0.00	0.75 0.73	0.15 0.00	0.80 0.77	0.16 0.00	0.83 0.81	0.17 0.00	0.88 0.86	0.18 0.00	0.92 0.90	0.18 0.00	0.97 0.95	0.19 0.00	1.00 1.00	0.20 0.00
DA40	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.14 0.34	0.29 0.56	0.04 0.19	0.32 0.60	0.04 0.20	0.35 0.63	0.05 0.22	0.39 0.68	0.05 0.23	0.42 0.71	0.06 0.24	0.46 0.76	0.06 0.26	0.56 0.82	0.08 0.28
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.37 0.03	0.75 0.73	0.28 0.02	0.80 0.77	0.30 0.02	0.83 0.81	0.31	0.88 0.86	0.33	0.92 0.90	0.34 0.03	0.97 0.95	0.36	1.00	0.37
DA41	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.19 0.59	0.29 0.60	0.06 0.35	0.32 0.64	0.06 0.38	0.35 0.67	0.07 0.40	0.39 0.72	0.07 0.43	0.42 0.76	0.08 0.45	0.46 0.80	0.09 0.47	0.56 0.86	0.11 0.51
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	0.00	0.75 0.73	0.00	0.80 0.77	0.00	0.83	0.00	0.88 0.86	0.00	0.92 0.90	0.00	0.97 0.95	0.00	1.00	0.00
DA42	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	1.08 1.08	0.29 0.29	0.31 0.31	0.32 0.32	0.35 0.35	0.35 0.35	0.38 0.38	0.39 0.39	0.42 0.42	0.42 0.42	0.45 0.45	0.46 0.46	0.50 0.50	0.56 0.56	0.60 0.60
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
100 100	Developed Concrete Developed Asphalt	0.12 0.00	0.75 0.73	0.09	0.80 0.77	0.10	0.83 0.81	0.10	0.88	0.11	0.92	0.11	0.97	0.12	1.00	0.12
DA43	Developed Grass Good (>75% grass cover) 2-7% Total / Composite	0.48 0.60	0.29	0.14	0.32	0.15 0.25	0.35 0.45	0.17 0.27	0.39	0.19	0.42 0.52	0.20	0.46 0.56	0.22	0.56 0.65	0.27 0.39
DA	Surface Type	Area	C ₂	C*A	C ₅	C*A	C ₁₀	C*A	C ₂₅	C*A	C ₅₀	C*A	C ₁₀₀	C*A	C ₅₀₀	C*A
	Developed Concrete Developed Asphalt	2.57 0.00	0.75	1.93	0.80 0.77	2.06	0.83	2.13	0.88	2.26 0.00	0.92		0.97	2.49	1.00	2.57 0.00
	Developed Grass Good (>75% grass cover) 2-7%		0.29													

0.63 0.29 0.18 0.32 0.20 0.35 0.22 0.39 0.25 0.42 0.26 0.46 0.29 0.56 0.35 3.20 0.66 2.11 0.71 2.26 0.74 2.35 0.78 2.51 0.82 2.63 0.87 2.78 0.91 2.92



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

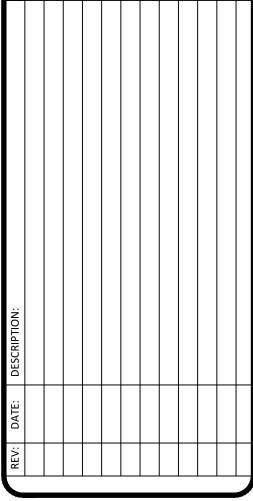
RANCH

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) 2, BLOCK A) 2, BLOCK B RTY HILL AND HILL ETJ SOUNTY, TEXAS

LOTS 1 AND 2, BLOC LOTS 1 AND 2, BLOC CITY OF LIBERTY HILL LIBERTY HILL ETJ WILLIAMSON COUNTY,





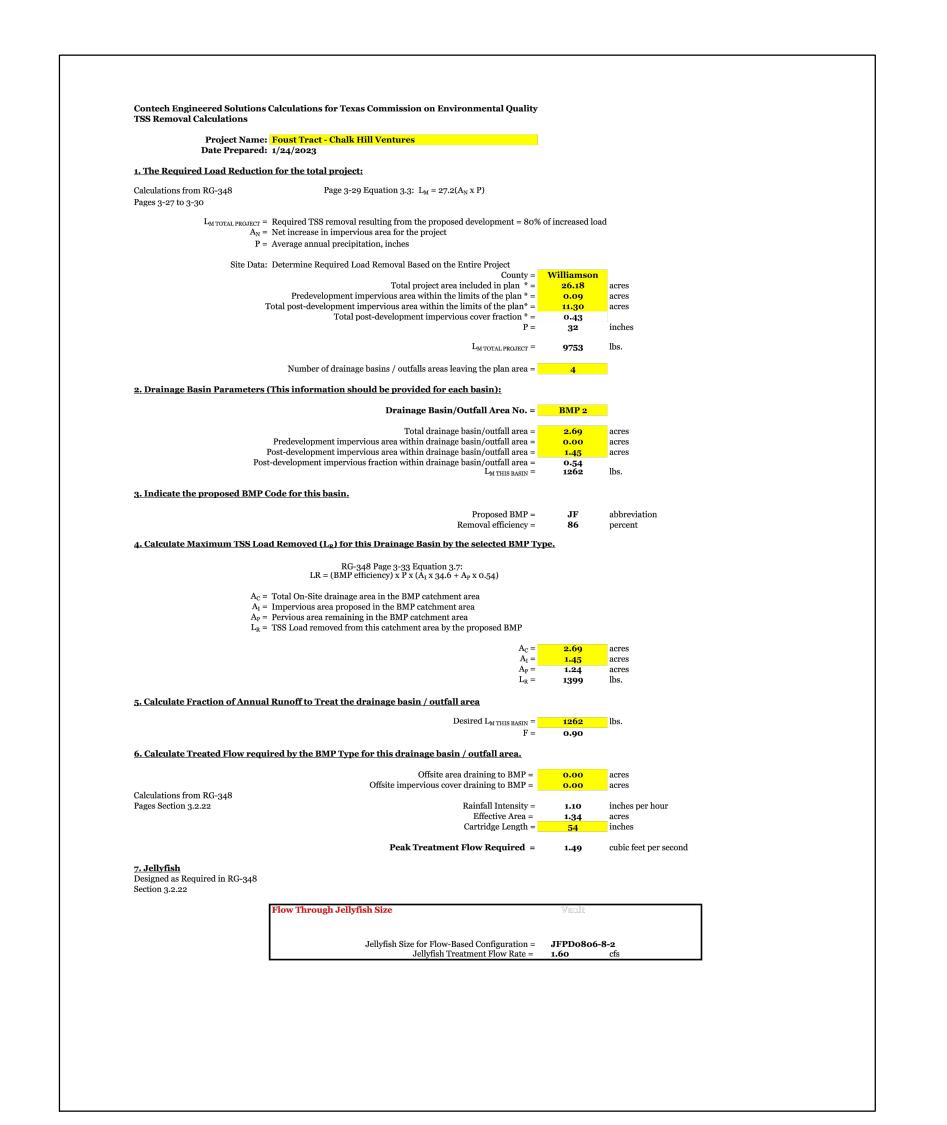
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

JOB NUMBER:

COMPOSITE C CALCULATIONS

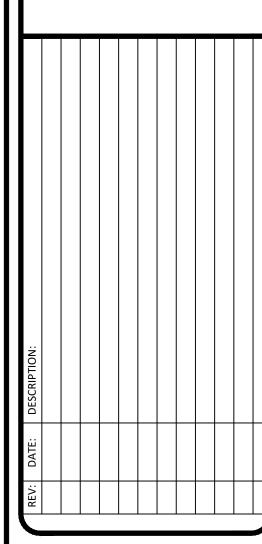




CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK HILL RANC





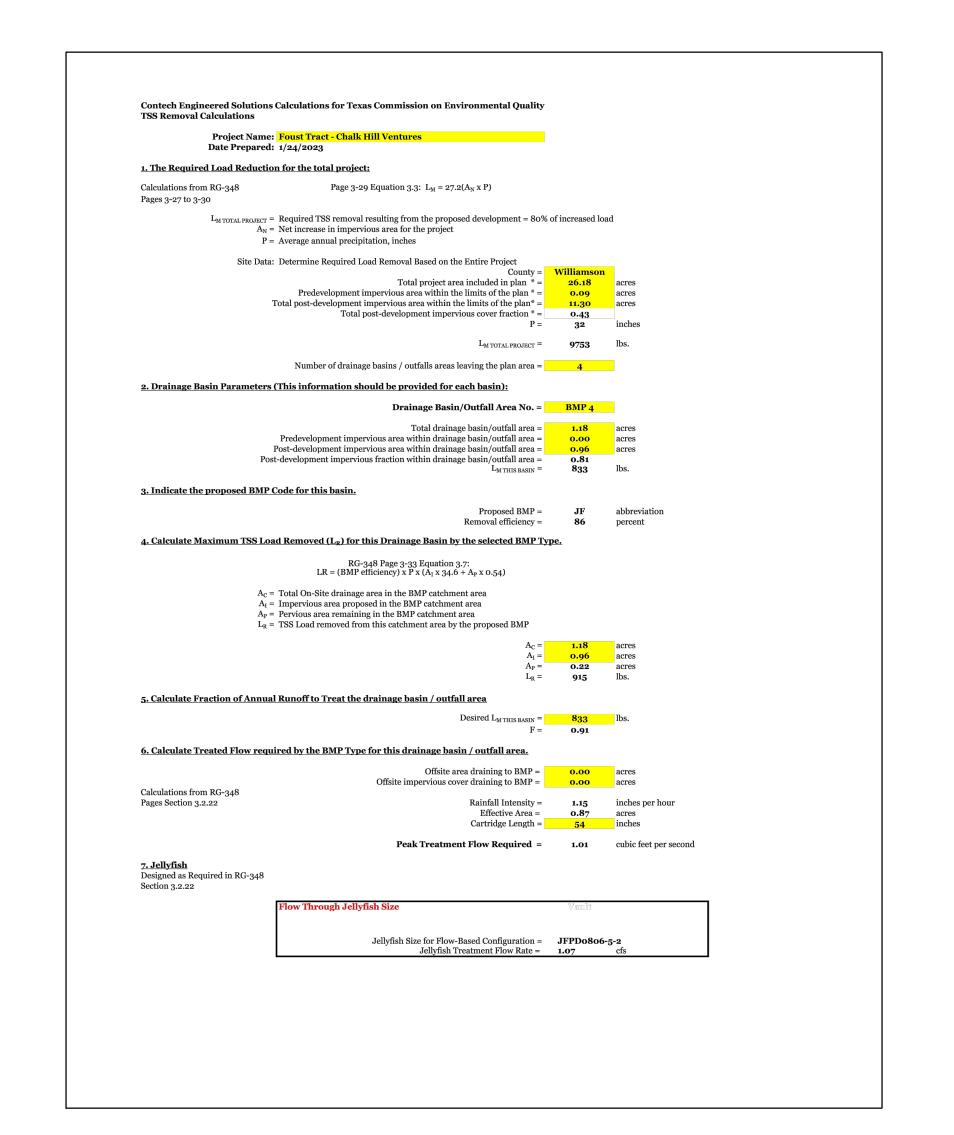
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

WATER QUALITY CALCULATIONS I

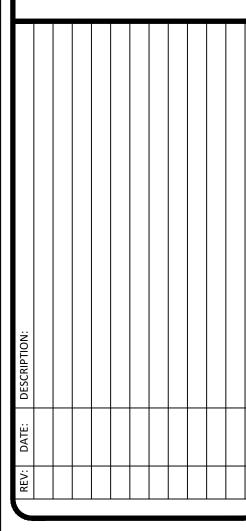




CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK HILL RANCH





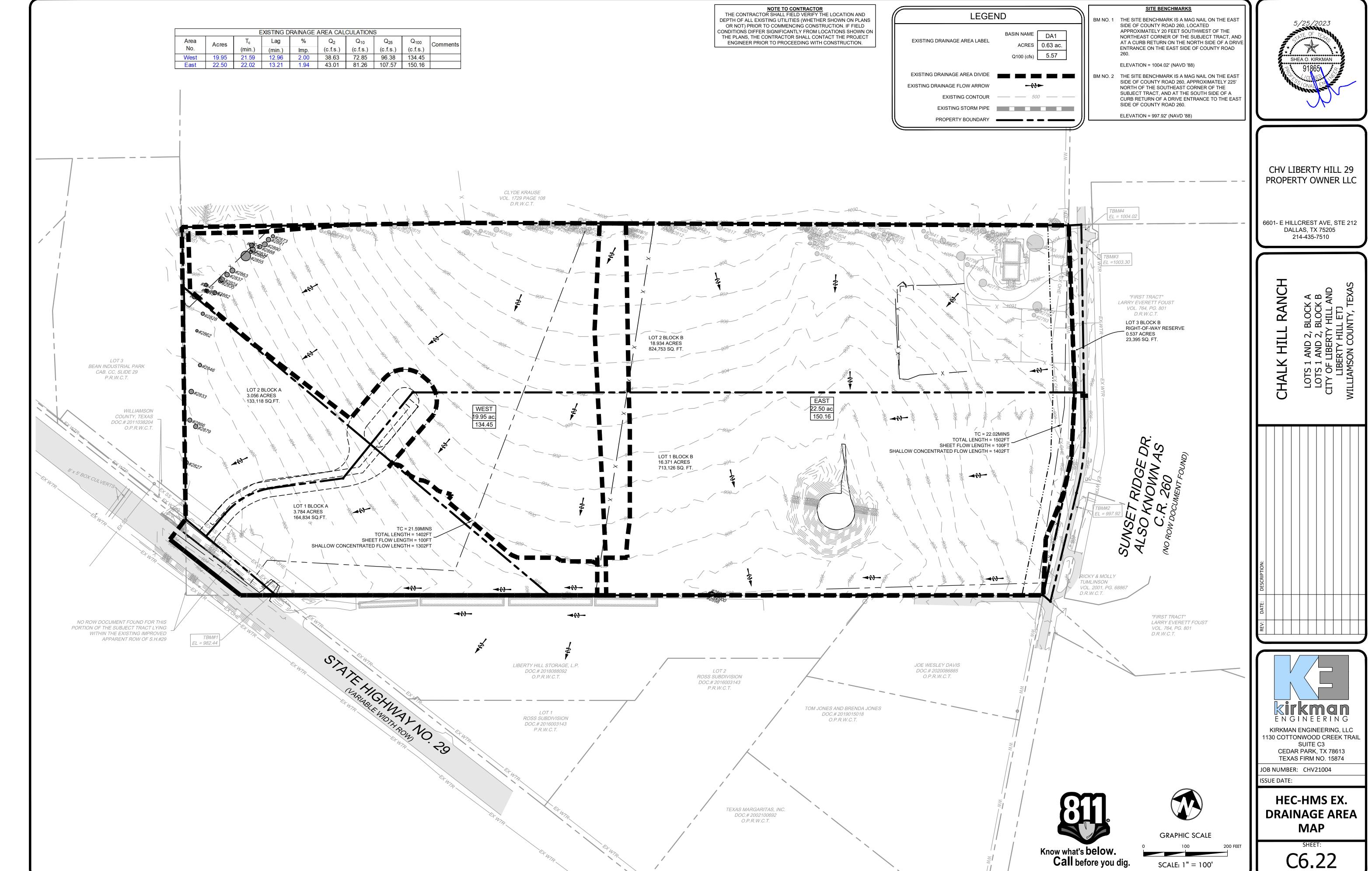
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK TX 78613

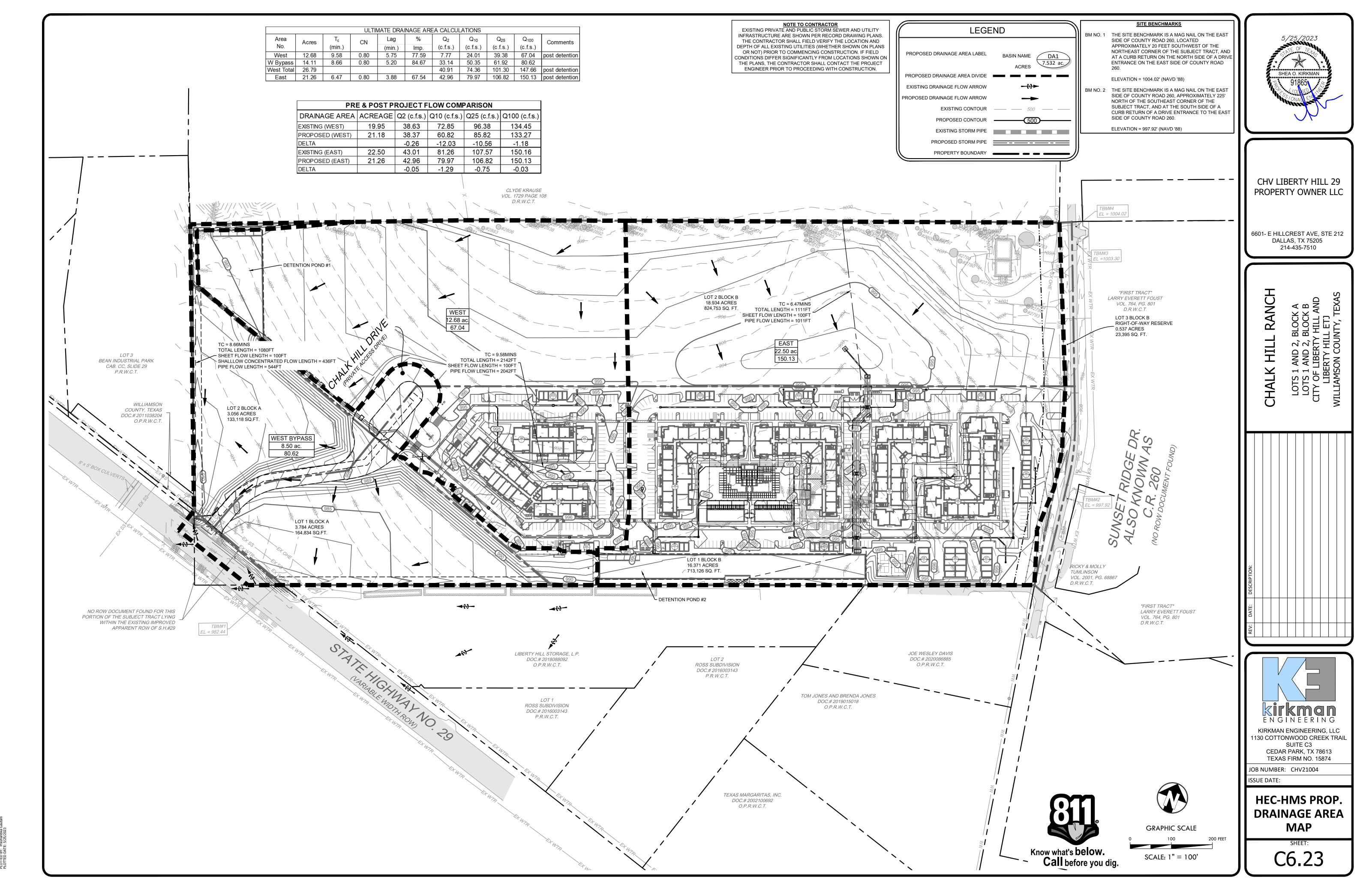
CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

WATER QUALITY CALCULATIONS II





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FILENAME: HMS CALCS.dwg	PLOTTED BY: Prashantika Gautan	PLOTTED DATE: 5/25/2023	

	EXISTING DRAINAGE AREA CALCULATIONS												
Area	Acres	T _c	CN	Lag	%	Q_2	Q ₁₀	Q ₂₅	Q ₁₀₀	Comments			
No.	710100	(min.)		(min.)	lmp.	(c.f.s.)	(c.f.s.)	(c.f.s.)	(c.f.s.)	Sommone			
West	25.95	21.59	0.80	12.96	5.64	51.52	95.90	126.39	175.72				
East	22.50	22.02	0.80	13.21	1.94	43.01	81.26	107.57	150.16				

			ULTIN	MATE DRAI	inage are	ea calcul	_ATIONS			
Area	Acres	T _c	CN	Lag	%	Q_2	Q ₁₀	Q ₂₅	Q ₁₀₀	Comments
No.	710100	(min.)	011	(min.)	lmp.	(c.f.s.)	(c.f.s.)	(c.f.s.)	(c.f.s.)	Commente
West	12.68	9.58	0.80	5.75	77.59	7.77	24.01	39.38	67.04	post detention
W Bypass	14.11	8.66	0.80	5.20	84.67	33.14	50.35	61.92	80.62	
West Total	26.79					40.91	74.36	101.30	147.66	post detention
East	21.26	6.47	0.80	3.88	67.54	42.96	79.97	106.82	150.13	post detention

DDI	PRE & POST PROJECT FLOW COMPARISON												
PRI	E & PUSTP	RUJECTF	LOW COM	ARISON									
DRAINAGE AREA	ACREAGE	Q2 (c.f.s.)	Q10 (c.f.s.)	Q25 (c.f.s.)	Q100 (c.f.s								
EXISTING (WEST)	19.95	38.63	72.85	96.38	134.45								
PROPOSED (WEST)	21.18	38.37	60.82	85.82	133.27								
DELTA		-0.26	-12.03	-10.56	-1.18								
EXISTING (EAST)	22.50	43.01	81.26	107.57	150.16								
PROPOSED (EAST)	21.26	42.96	79.97	106.82	150.13								
DELTA		-0.05	-1.29	-0.75	-0.03								

	WEST POND			EAST POND	
STAGE-STO	RAGE-DISCHA	ARGE TABLE	STAGE-STO	RAGE-DISCHA	ARGE TABLE
ELEVATION	STORAGE	DISCHARGE	ELEVATION	STORAGE	DISCHARGE
(FT)	(AC-FT)	(CFS)	(FT)	(AC-FT)	(CFS)
984.00	0.000000	0.00	988.45	0.000000	0.00
984.10	0.000081	0.04	988.50	0.000019	0.15
984.20	0.000756	0.11	988.60	0.001512	0.76
984.30	0.002696	0.20	988.70	0.007903	1.63
984.40	0.006564	0.31	988.80	0.024477	2.70
984.50	0.013017	0.43	988.90	0.050188	3.94
984.60	0.022729	0.57	989.00	0.083789	5.32
984.70	0.036353	0.72	989.10	0.125318	6.84
984.80	0.054452	0.88	989.20	0.174793	8.48
984.90	0.077355	1.05	989.30	0.231793	10.23
985.00	0.105353	1.23	989.40	0.294707	12.08
985.10	0.138756	1.42	989.50	0.361729	14.04
985.20	0.177452	1.62	989.60	0.431783	16.09
985.30	0.220996	1.82	989.70	0.504248	18.24
985.40	0.268934	2.04	989.80	0.578498	20.47
985.50	0.320820	2.26	989.90	0.654291	22.79
985.60	0.376202	2.49	990.00	0.731436	25.18
985.70	0.434640	2.73	990.10	0.809709	27.66
985.80	0.495676	2.97	990.20	0.888843	30.21
985.90	0.558874	3.22	990.30	0.968585	32.84
986.00	0.623771	3.48	990.40	1.048661	35.54
986.10	0.689926	3.74	990.50	1.128855	38.30
986.20	0.756886	4.01	990.60	1.209124	41.14
986.30	0.824219	4.29	990.70	1.289473	44.06
986.40	0.891626	4.57	990.80	1.369897	47.58
986.50	0.959107	4.86	990.90	1.450395	51.55
986.60	1.026645	5.16	991.00	1.530961	55.84
986.70	1.094250	5.46	991.10	1.611607	60.41
986.80	1.161917	5.76	991.20	1.692329	65.22
986.90	1.229647	6.07	991.30	1.773124	70.25
987.00	1.297444	6.39	991.40	1.853988	75.49
987.10	1.365298	6.71	991.50	1.934932	80.93
987.20	1.433219	7.04	991.60	2.015944	86.55
987.30	1.501202	7.37	991.70	2.097025	92.36
987.40	1.569248	7.71	991.80	2.178186	98.33
987.50	1.637349	8.05	991.90	2.259409	104.47
987.60	1.705519	8.76	992.00	2.340713	110.77
987.70	1.773750	9.77	992.10	2.422079	117.22
987.80	1.842043	10.97	992.20	2.503525	123.82
987.90	1.910393	12.34	992.30	2.585033	130.57
988.00	1.978810	13.85	992.40	2.666616	137.47
988.10	2.047283	15.48	992.50	2.748267	144.50
988.20	2.115818	17.23	992.60	2.829986	151.66
988.30	2.184415	19.08	992.70	2.911773	151.00
988.40	2.253068	21.03	992.80	2.993634	166.40
988.50	2.321789	23.08	992.90	3.075558	173.95
988.60	2.390560	25.22	993.00	3.157438	181.64
988.70	2.459399	27.44	333.00	J. 1J/4J0	101.04
988.80	2.528293	29.74			
988.90	2.597250	32.13			
989.00	2.666262	34.58			
080.10	2.735337	37.12			

 989.10
 2.735337
 37.12

 989.20
 2.804467
 39.72

 989.30
 2.873659
 42.39

 989.40
 2.942907
 45.13

 989.50
 3.012217
 47.94

 989.60
 3.081583
 50.81

 989.70
 3.151004
 53.74

 989.80
 3.220481
 56.74

 989.90
 3.290021
 59.79

 990.00
 3.359616
 62.90

 990.10
 3.429267
 66.07

 990.20
 3.498979
 69.30

 990.30
 3.568742
 72.58

990.40 3.638566 75.92

990.50 3.708446 79.31

990.60 3.778376 82.75

990.70 3.848368 86.25

 990.80
 3.918415
 89.79

 990.90
 3.988519
 93.39

991.00 4.058671 97.03

 991.10
 4.128791
 100.73

 991.20
 4.198919
 104.47

 991.30
 4.269048
 108.26

991.40 4.339176 112.10

 991.90
 4.689816
 131.97

 992.00
 4.759944
 136.08

NOTE: Calculations extend above top of pond

991.60 4.479432

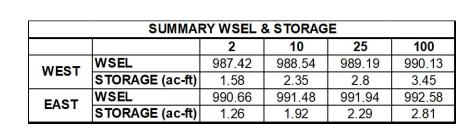
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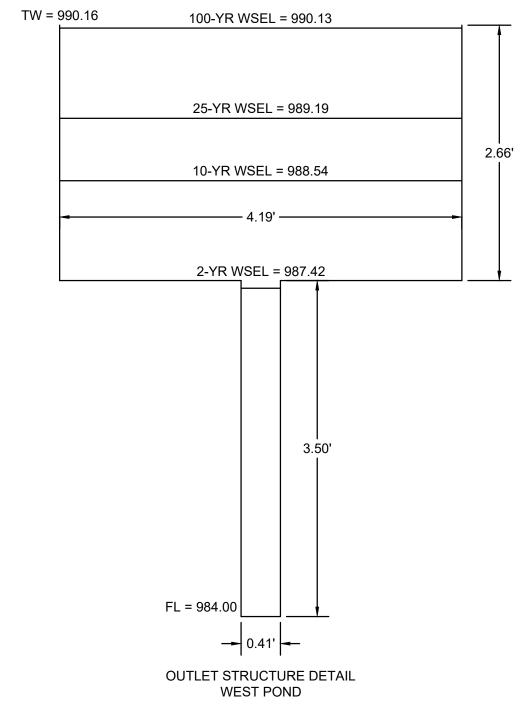
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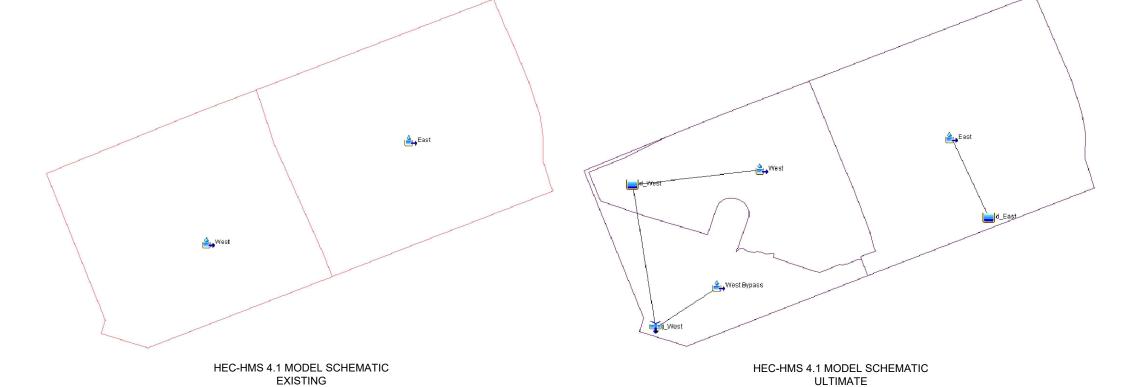
for modeling purposes

4.409304 115.98

	OUTLET STRUCTURE DETAIL WEST POND	
TW = 993.00		. —
	100-YR WSEL = 992.58	1
	25-YR WSEL = 991.94	
	10-YR WSEL = 991.48	2.31
-	9.57'	
	2-YR WSEL = 990.66	
	2.24' FL = 988.45	
	4.35'	
	OUTLET STRUCTURE DETAIL EAST POND	







TIME OF CON	TRATIO	TIME OF CONCENTRATION						
CALCULATIONS (T	R-55	5) - EXIS	TING	CALCULATIONS (TR-55) - EXISTING EAST For Sheet Flow of less than 300 ft				
WE	ST	-						
For Sheet Flow of	les	s than 3	00 ft					
$T_t = (0.007*(n*L)^{0.8} / P_2*0.5$	*S ^{0.4})*	[*] 60		$T_t = (0.007*(n*L)^{0.8} / P_2*0.5$	*S ^{0.4})*	[*] 60		
Travel Time	T _t	9.97	min	Travel Time	T _t	7.46	min	
Manning's 'n'	n	0.130		Manning's 'n'	n	0.130		
Length	L	100	feet	Length	L	100	feet	
2-year, 24-hour rainfall	P ₂	3.94	inches	2-year, 24-hour rainfall	P ₂	3.94	inches	
Slope	S	0.0113	ft/ft	Slope	S	0.0233	ft/ft	
Shallow Conce	ntra	ted Flo	w	Shallow Conce	entra	ted Flo	<i>N</i>	
Unpaved	V = 1	6.13*(S) ^{0.5}		Unpaved	V = 1	V = 16.13*(S) ^{0.5}		
Paved		0.33*(S) ^{0.5}		Paved		0.33*(S) ^{0.5}		
Paved? (Y/N)		No		Paved? (Y/N)	Τ	No		
Slope	S	0.0134	(ft/ft)	Slope	S	0.0099	(ft/ft)	
 Time	t	11.62	min	Time	t	14.56	min	
Distance	d	1,302		Distance	d	1,402		
Velocity	V	1.87		Velocity	V	1.60		
Channel or	Pipe	Flow		Channel or	Pipe	Flow		
Time	t		min	Time	t	0.00	min	
Distance	d	0	feet	Distance	d	0	feet	
Velocity	v	1.00	C. 1	Velocity	V	1.00	c. 1	

NOTE: Minimum TC value is 5 minutes and minimum	NOTE: Minimum TC value is 5 minutes and minimum	
corresponding Lag time is 3 minutes.	corresponding Lag time is 3 minutes.	

21.59 min

12.96 min

Total Time

22.02 min

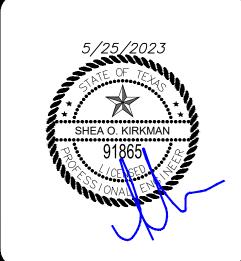
13.21 min

Total Time

TIME OF CONCENTRATION			TIME OF CON	TIME OF CONCENTRATION			TIME OF CONCENTRATION					
CALCULATIONS (T	R-55	5) - ULTII	MATE	CALCULATIONS (T	R-55	5) - ULTII	MATE	CALCULATIONS (T	R-55) - ULTII	MAT	
WE		-		WEST B		•		EA		-		
For Sheet Flow of		s than 1	50 ft	For Sheet Flow of			50 ft	For Sheet Flow of less than 150			50 ft	
$T_t = (0.007*(n*L)^{0.8} / P_2*0.5)$			30 10	$T_t = (0.007*(n*L)^{0.8} / P_2*0.5)$			30 10	$T_t = (0.007*(n*L)^{0.8} / P_2*0.5)$, , , , , , , , , , , , , , , , , , , 	
I _t = (0.007 (II L) / P ₂ 0.5	.3)	-60		$I_t = (0.007^{\circ}(11^{\circ}L)^{\circ})^{\circ} P_2^{\circ} 0.5$	3)	160		$I_t = (0.007 \cdot (11 \cdot L) - 7 \cdot P_2 \cdot 0.5)$.3).	60		
Travel Time	T _t	3.91	min	Travel Time	T _t	3.82	min	Travel Time	T _t	3.66	min	
Manning's 'n'	n	0.035		Manning's 'n'	n	0.035		Manning's 'n'	n	0.035		
Length	L	100	feet	Length	L	100	feet	Length	L	100	feet	
2-year, 24-hour rainfall	P ₂	3.94	inches	2-year, 24-hour rainfall	P ₂	3.94	inches	2-year, 24-hour rainfall	P ₂	3.94	inches	
Slope	S	0.0085	ft/ft	Slope	S	0.0090	ft/ft	Slope	S	0.0100	ft/ft	
	•											
Shallow Conce	entra	ated Flo	w	Shallow Conce	ntra	ated Flo	w	Shallow Conce	ntra	ted Flo	N	
Unpaved	V = 1	.6.13*(S) ^{0.5}		Unpaved	V = 1	.6.13*(S) ^{0.5}		Unpaved	V = 16	5.13*(S) ^{0.5}		
Paved	V = 2	.0.33*(S) ^{0.5}		Paved	V = 2	0.33*(S) ^{0.5}		Paved		0.33*(S) ^{0.5}		
						• • •	•					
Paved? (Y/N)		No		Paved? (Y/N)		No		Paved? (Y/N)	Ь,	No		
Slope	S	0.0100	(ft/ft)	Slope	S	0.0183	(ft/ft)	Slope	S	0.0100	(ft/ft)	
 Time	t	0.00	min	Time	t	3 33	min	Time	t	0.00	min	
Distance	d		feet	Distance	d		feet	Distance	d		feet	
Velocity	V	1.61		Velocity	V		ft/s	Velocity	V	1.61		
		The state of the s		,				,				
Channel or	Pip	e Flow		Channel or	Pip	e Flow		Channel or	Pipe	Flow		
Time	t	5.67	min	Time	t	1.51	min	Time	t	2.81	min	
Distance	d	2,042	feet	Distance	d	544	feet	Distance	d	1,011	feet	
Velocity	V	6.00	ft/s	Velocity	٧	6.00	ft/s	Velocity	V	6.00	ft/s	
Total	1			Total	1		1	Total		1		
Time	TC	9.58		Time	TC		min	Time	TC	6.47		
Lag	L	5.75	min	Lag	L	5.20	min	Lag	L I	3.88	min	

NOTE: Minimum TC value is 5 minutes and minimum NOTE: Minimum TC value is 5 minutes and minimum NOTE: Minimum TC value is 5 minutes and minimum corresponding Lag time is 3 minutes.

NOTE: Minimum TC value is 5 minutes and minimum corresponding Lag time is 3 minutes.

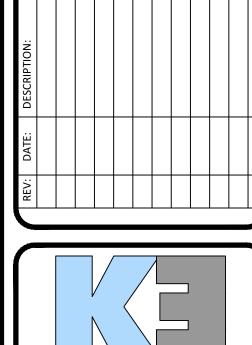


CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

> LOTS 1 AND 2, BLOCK A LOTS 1 AND 2, BLOCK B CITY OF LIBERTY HILL AND LIBERTY HILL ETJ

CHALK

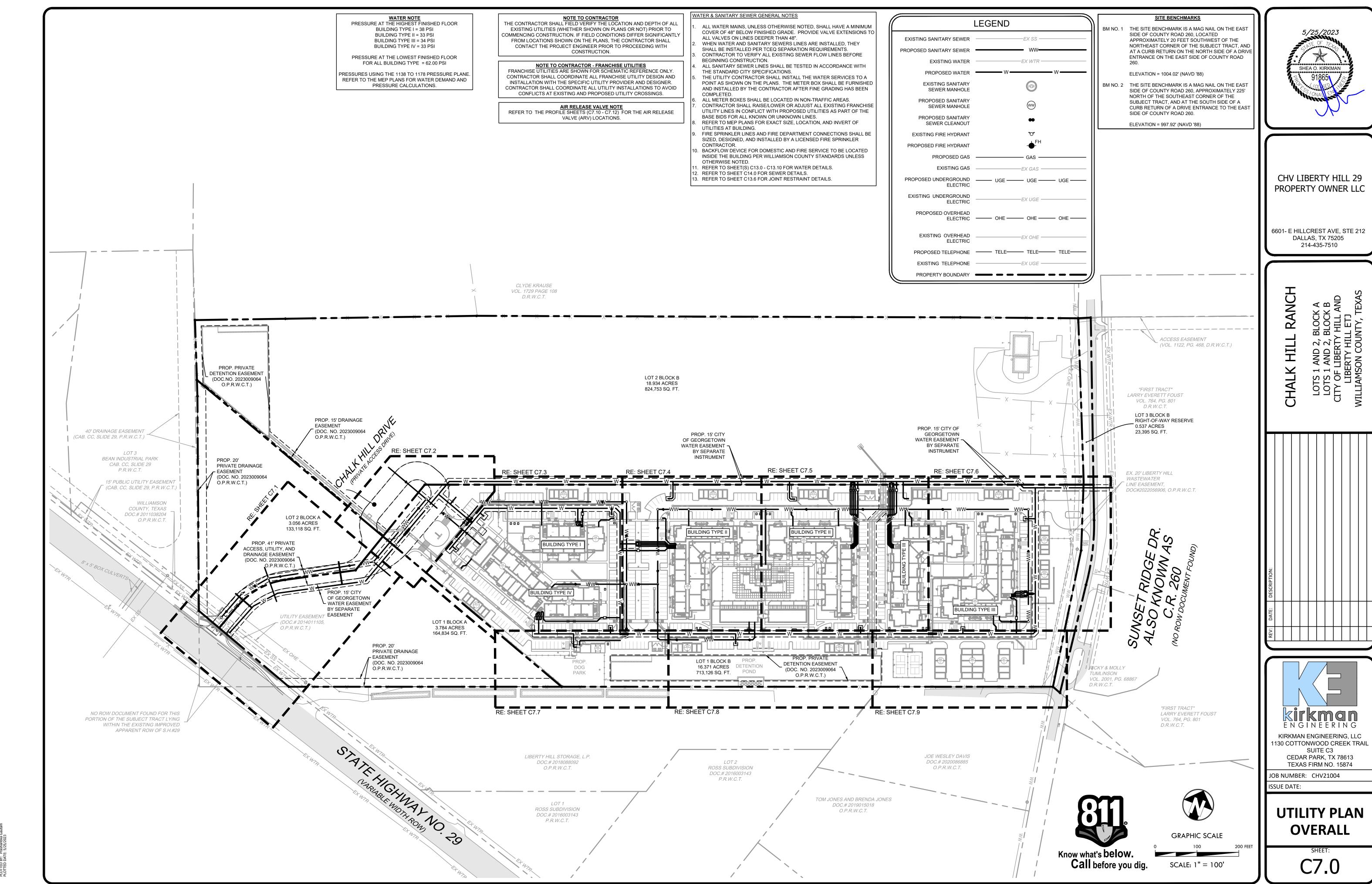




JOB NUMBER: CHV21004

HEC-HMS

CALCULATIONS



FULL PATH: K:\Jobs\CHV21004 Foust\Drawings\03 - ENGR - INFRASTRUCTURE & PH1\03 - ProductionK:\Jobs\CHV21004 Foust\Drawings\03 - ENGR - INFRASTRUCTURE & PH1\03 - Production\C7.0 UTILITY PLAN CHV210

IAME: C7.0 UTILITY PLAN_CHV21004.dv

SITE BENCHMARKS BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED

CHV LIBERTY HILL 29

6601- E HILLCREST AVE, STE 212

KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

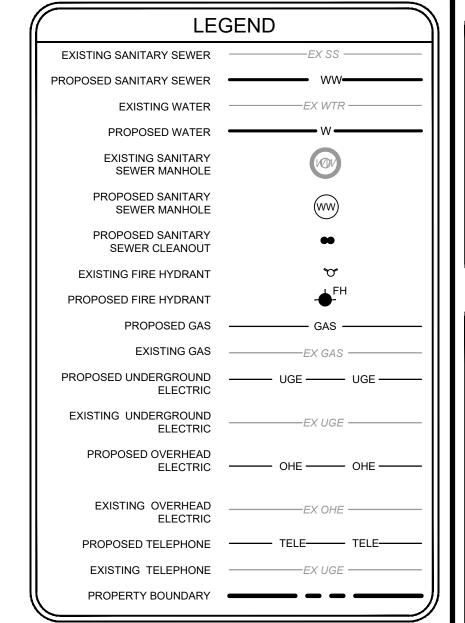
REFER TO THE PROFILE SHEETS (C7.10 - C7.12) FOR THE AIR RELEASE VALVE (ARV) LOCATIONS. SITE BENCHMARKS

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT. AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIVE ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

ELEVATION = 997.92' (NAVD '88)



WATER & SANITARY SEWER GENERAL NOTES

- ALL WATER MAINS, UNLESS OTHERWISE NOTED, SHALL HAVE A MINIMUM COVER OF 48" BELOW FINISHED GRADE. PROVIDE VALVE EXTENSIONS TO ALL VALVES ON LINES DEEPER THAN 48". WHEN WATER AND SANITARY SEWERS LINES ARE INSTALLED, THEY SHALL BE INSTALLED PER TCEQ SEPARATION REQUIREMENTS.
- ALL WASTEWATER LINE TO BE SDR-26. CONTRACTOR TO VERIFY ALL EXISTING SEWER FLOW LINES
- BEFORE BEGINNING CONSTRUCTION. ALL SANITARY SEWER LINES SHALL BE TESTED IN ACCORDANCE WITH THE STANDARD CITY SPECIFICATIONS.
- THE UTILITY CONTRACTOR SHALL INSTALL THE WATER SERVICES TO A POINT AS SHOWN ON THE PLANS. THE METER BOX SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR AFTER FINE GRADING HAS BEEN COMPLETED.
- ALL METER BOXES SHALL BE LOCATED IN NON-TRAFFIC AREAS. CONTRACTOR SHALL RAISE/LOWER OR ADJUST ALL EXISTING FRANCHISE UTILITY LINES IN CONFLICT WITH PROPOSED UTILITIES AS PART OF THE BASE BIDS FOR ALL KNOWN OR UNKNOWN LINES. REFER TO MEP PLANS FOR EXACT SIZE, LOCATION, AND INVERT OF UTILITIES AT BUILDING.
- FIRE SPRINKLER LINES AND FIRE DEPARTMENT CONNECTIONS SHALL BE SIZED, DESIGNED, AND INSTALLED BY A LICENSED FIRE SPRINKLER CONTRACTOR.
- BACKFLOW DEVICE FOR DOMESTIC AND FIRE SERVICE TO BE LOCATED INSIDE THE BUILDING PER WILLIAMSON COUNTY STANDARDS UNLESS OTHERWISE NOTED.
- REFER TO SHEET(S) C13.0 C13.10 FOR WATER DETAILS. . REFER TO SHEET C14.0 FOR SEWER DETAILS.

. REFER TO SHEET C13.6 FOR JOINT RESTRAINT TABLE.

NOTE TO CONTRACTOR

THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER SIGNIFICANTLY FROM LOCATIONS SHOWN ON THE PLANS. THE CONTRACTOR SHALL CONTACT THE PROJECT ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

CONTRACTOR SHALL COORDINATE ALL FRANCHISE UTILITY DESIGN AND INSTALLATION WITH THE SPECIFIC UTILITY PROVIDER AND DESIGNER. CONTRACTOR SHALL COORDINATE ALL UTILITY INSTALLATIONS TO AVOID CONFLICTS AT EXISTING AND PROPOSED UTILITY CROSSINGS.



Know what's **below. Call** before you dig.



GRAPHIC SCALE

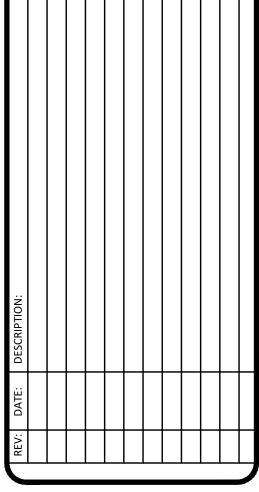
SCALE: 1" = 20'

SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHAL





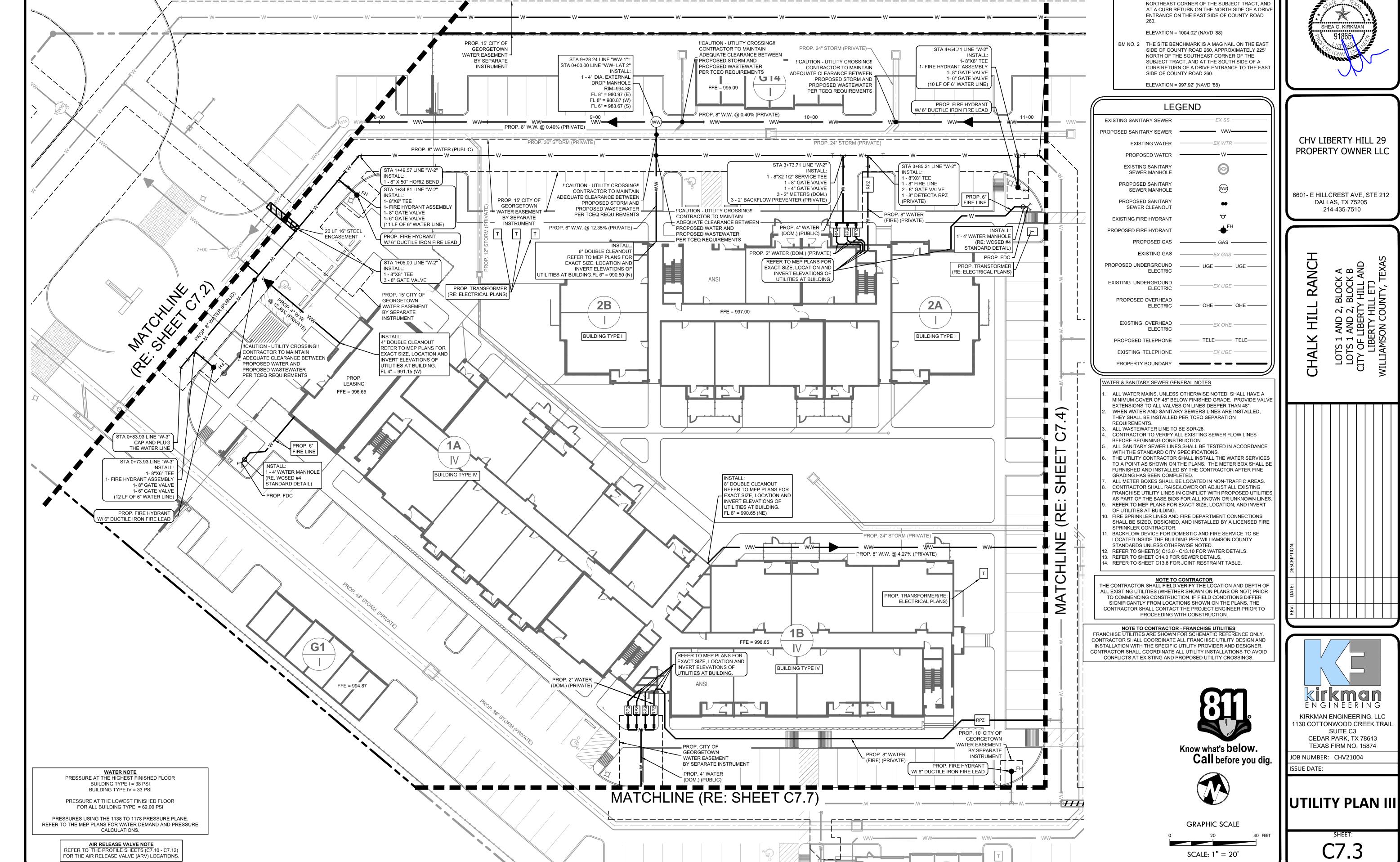
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874 JOB NUMBER: CHV21004

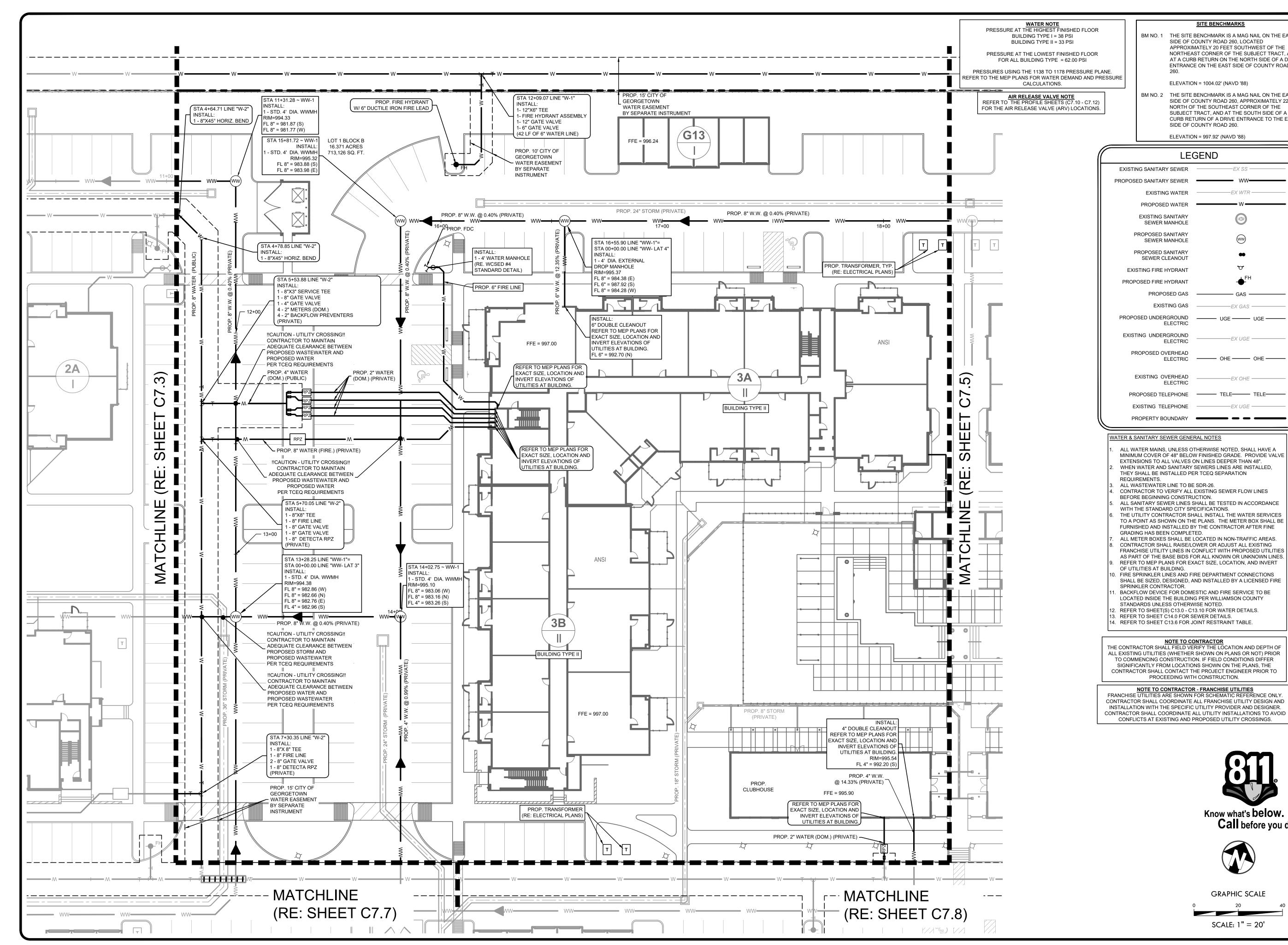
ISSUE DATE:

UTILITY PLAN II



SITE BENCHMARKS BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED

APPROXIMATELY 20 FEET SOUTHWEST OF THE



SITE BENCHMARKS

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIV ENTRANCE ON THE EAST SIDE OF COUNTY ROAD

ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

—EX GAS —

——— UGE ——— UGE ———

EX OHE

ELECTRIC — OHE — OHE —

ELEVATION = 997.92' (NAVD '88)

LEGEND

EXISTING WATER

PROPOSED WATER

EXISTING SANITARY SEWER MANHOLE

SEWER MANHOLE

SEWER CLEANOUT

PROPOSED GAS

EXISTING GAS

ELECTRIC

ELECTRIC

PROCEEDING WITH CONSTRUCTION.

Know what's below.

Call before you dig.

GRAPHIC SCALE

SCALE: 1'' = 20'



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205

214-435-7510

CHAL

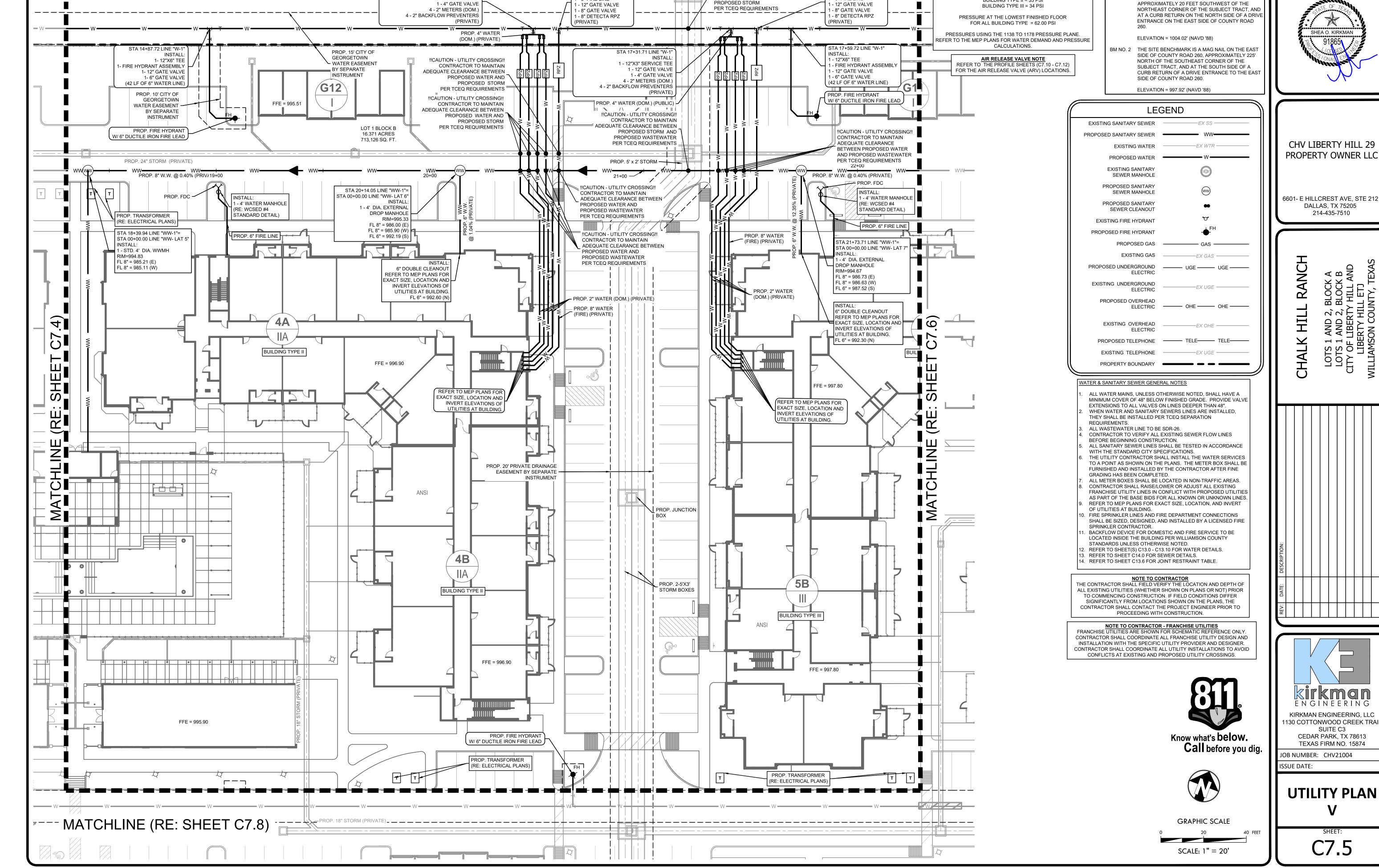
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAI SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

UTILITY PLAN IV



!!CAUTION - UTILITY CROSSING!!

ADEQUATE CLEARANCE BETWEEN

CONTRACTOR TO MAINTAIN

PROPOSED WATER AND

PROPOSED STORM

STA 16+51.12 LINE "W-1"

INSTALL:

1 - 12"X8" TEE

1 - 8" FIRE LINE

STA 17+43.72 LINE "W-1"

PRESSURE AT THE HIGHEST FINISHED FLOOR

BUILDING TYPE II = 33 PSI

INSTALL:

1 - 12"X8" TEE

1 - 8" FIRE LINE

STA 16+39.12 LINE "W-1"

1 - 12"X3" SERVICE TEE

1 - 12" GATE VALVE

INSTAL

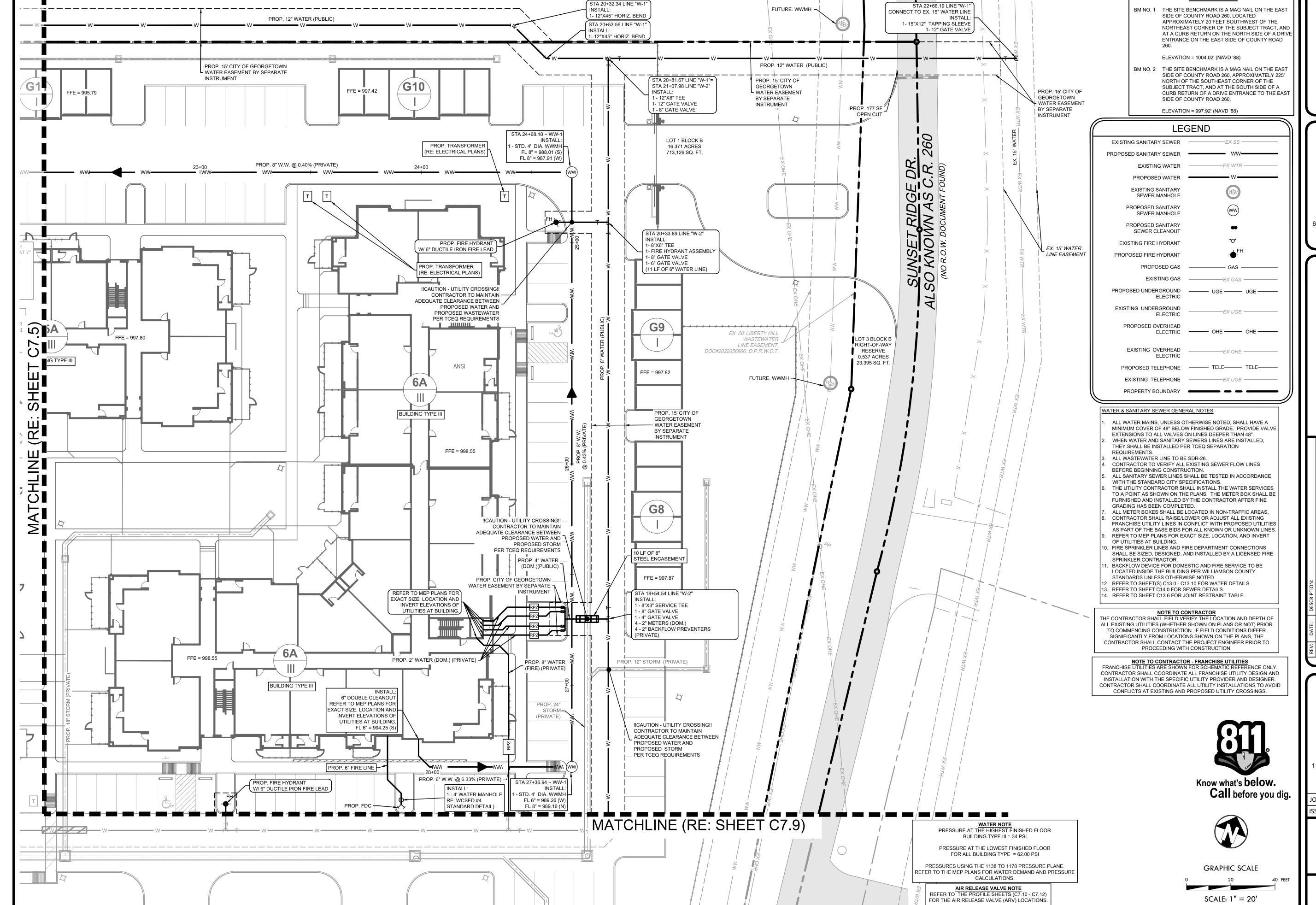
SITE BENCHMARKS

BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST

SIDE OF COUNTY ROAD 260, LOCATED

CHV LIBERTY HILL 29

KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAI



SHEA O. KIRKMAN

SITE BENCHMARKS

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

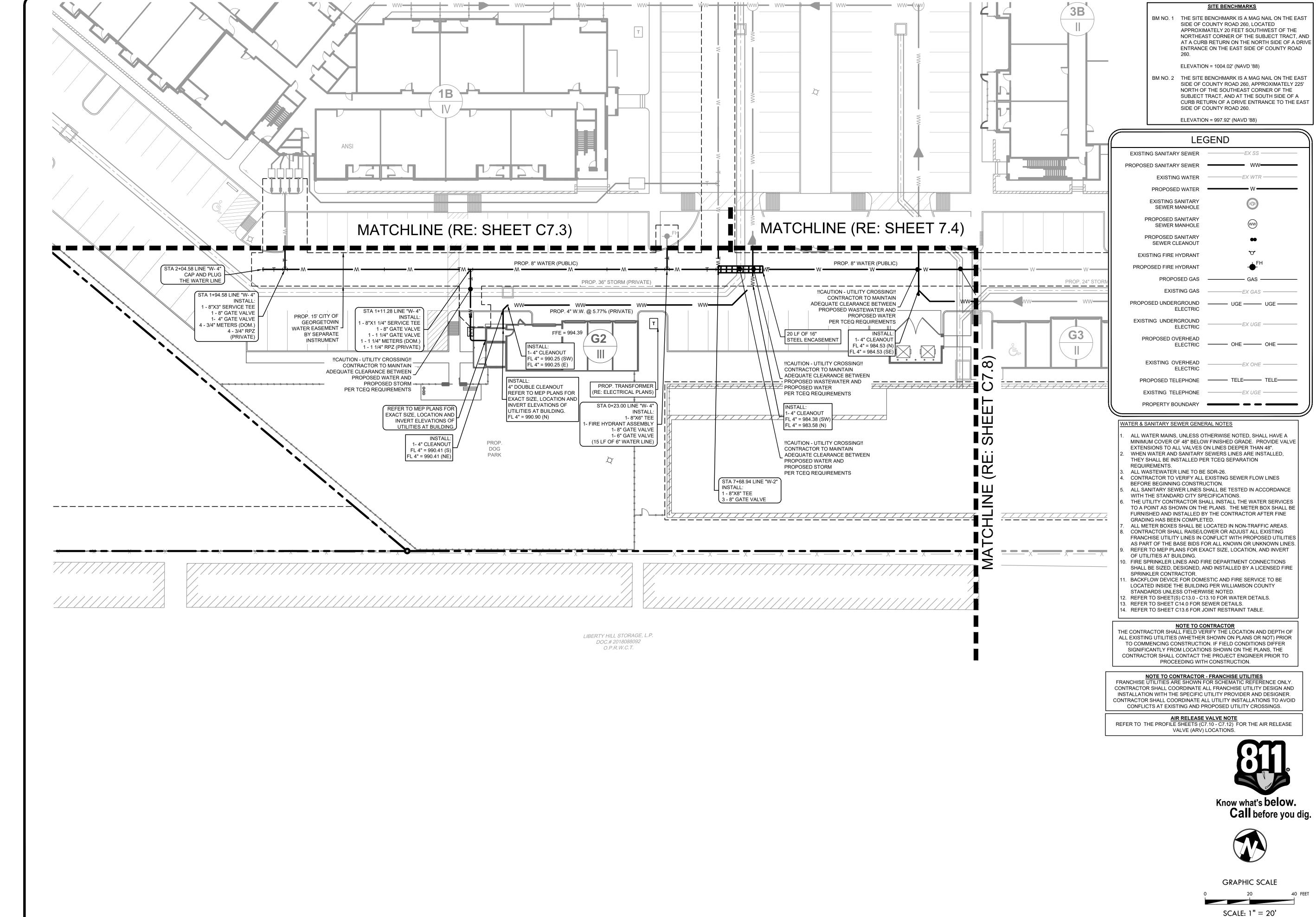
6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHAL

KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAI SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:



SHEA O. KIRKMAN

CHV LIBERTY HILL 29 PROPERTY OWNER LLC

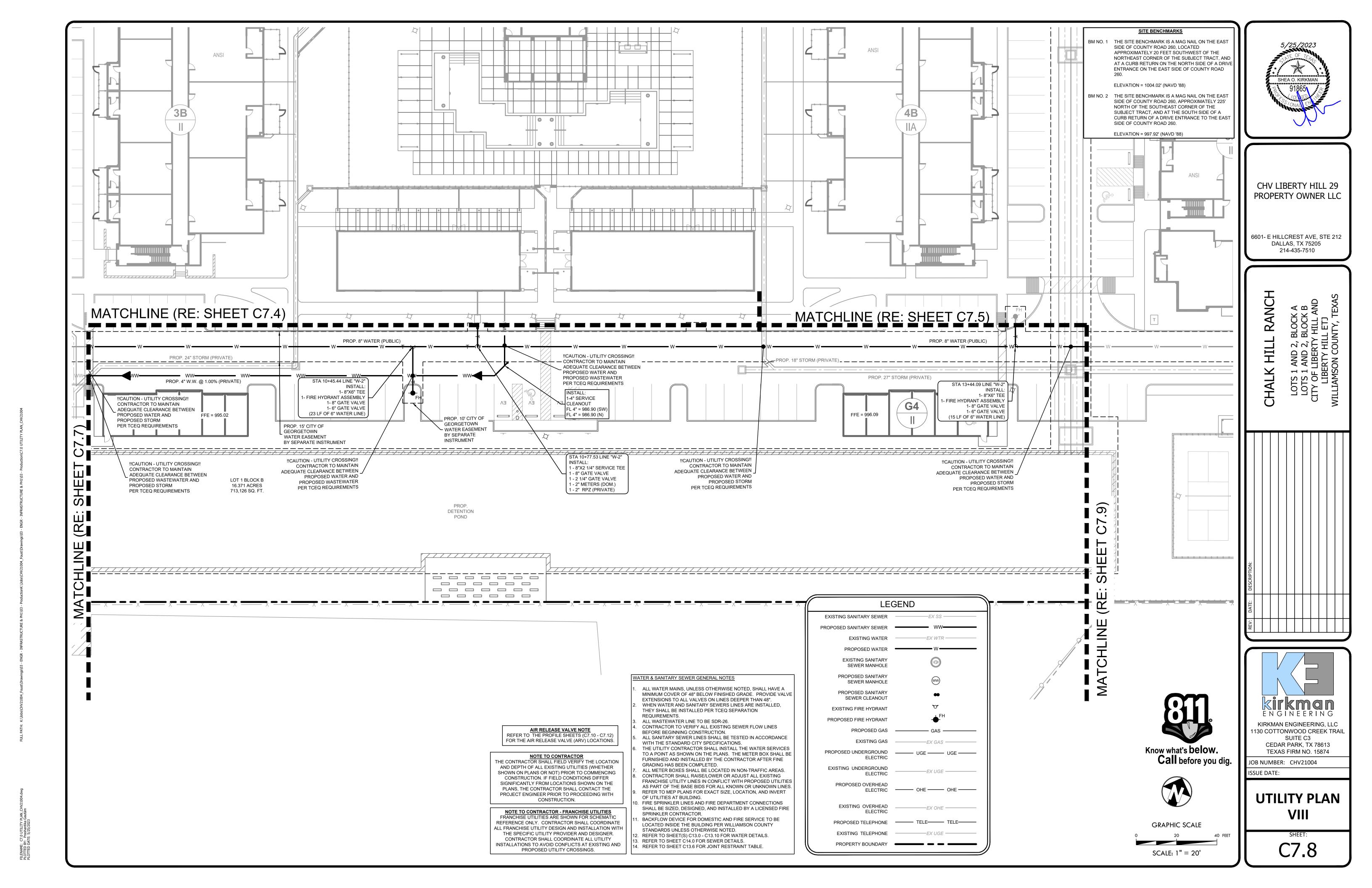
6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

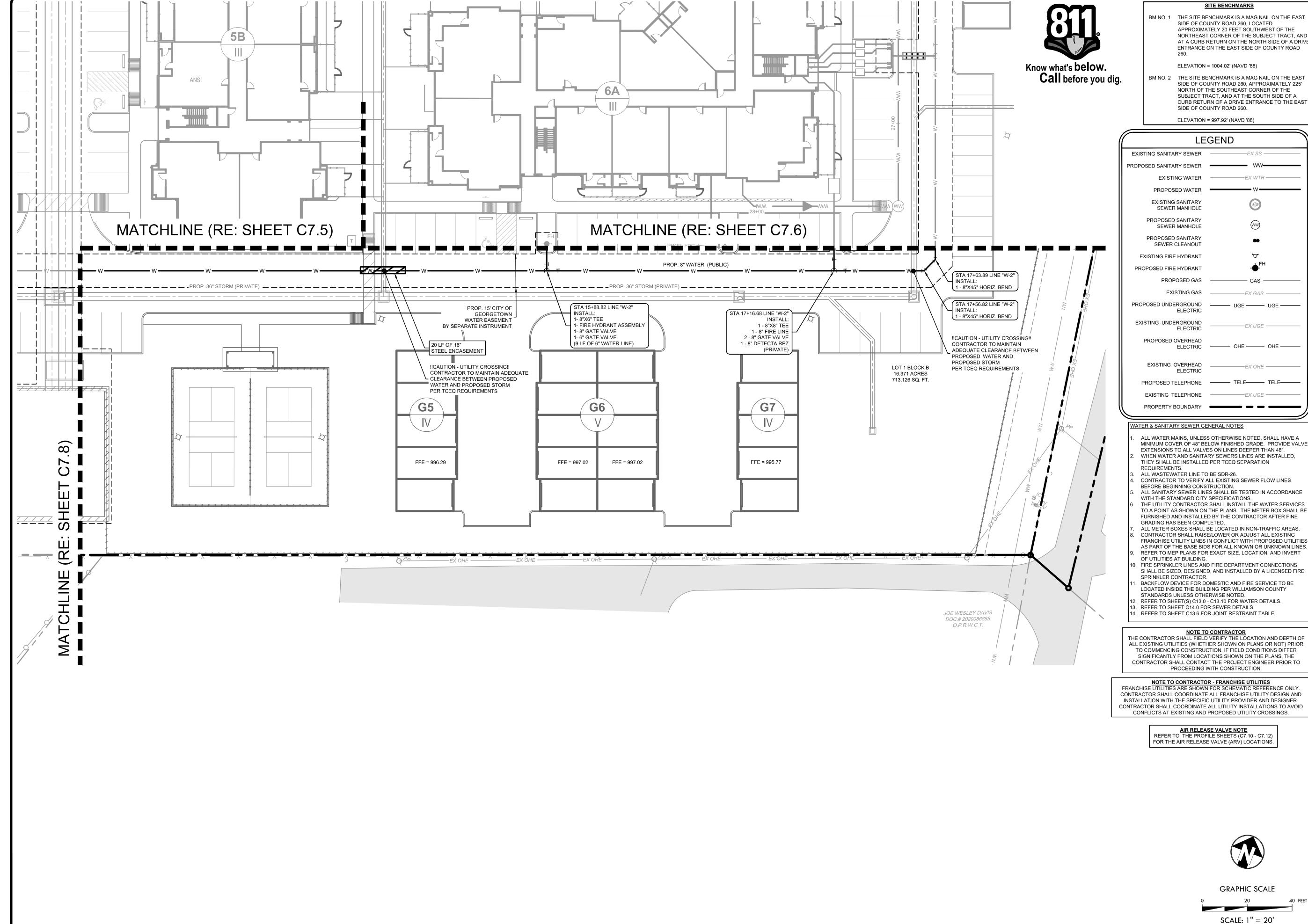


KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:





BM NO. 1 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, LOCATED APPROXIMATELY 20 FEET SOUTHWEST OF THE NORTHEAST CORNER OF THE SUBJECT TRACT, AND AT A CURB RETURN ON THE NORTH SIDE OF A DRIV

ELEVATION = 1004.02' (NAVD '88)

BM NO. 2 THE SITE BENCHMARK IS A MAG NAIL ON THE EAST SIDE OF COUNTY ROAD 260, APPROXIMATELY 225' NORTH OF THE SOUTHEAST CORNER OF THE SUBJECT TRACT, AND AT THE SOUTH SIDE OF A CURB RETURN OF A DRIVE ENTRANCE TO THE EAST SIDE OF COUNTY ROAD 260.

ELEVATION = 997.92' (NAVD '88)



- ALL WATER MAINS, UNLESS OTHERWISE NOTED, SHALL HAVE A MINIMUM COVER OF 48" BELOW FINISHED GRADE. PROVIDE VALVE EXTENSIONS TO ALL VALVES ON LINES DEEPER THAN 48". WHEN WATER AND SANITARY SEWERS LINES ARE INSTALLED, THEY SHALL BE INSTALLED PER TCEQ SEPARATION
- CONTRACTOR TO VERIFY ALL EXISTING SEWER FLOW LINES
- ALL SANITARY SEWER LINES SHALL BE TESTED IN ACCORDANCE WITH THE STANDARD CITY SPECIFICATIONS. THE UTILITY CONTRACTOR SHALL INSTALL THE WATER SERVICES TO A POINT AS SHOWN ON THE PLANS. THE METER BOX SHALL BE
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- REFER TO MEP PLANS FOR EXACT SIZE, LOCATION, AND INVERT . FIRE SPRINKLER LINES AND FIRE DEPARTMENT CONNECTIONS SHALL BE SIZED, DESIGNED, AND INSTALLED BY A LICENSED FIRE
- BACKFLOW DEVICE FOR DOMESTIC AND FIRE SERVICE TO BE
- LOCATED INSIDE THE BUILDING PER WILLIAMSON COUNTY
- REFER TO SHEET(S) C13.0 C13.10 FOR WATER DETAILS. . REFER TO SHEET C14.0 FOR SEWER DETAILS.
- 14. REFER TO SHEET C13.6 FOR JOINT RESTRAINT TABLE.

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NOTE TO CONTRACTOR - FRANCHISE UTILITIES
FRANCHISE UTILITIES ARE SHOWN FOR SCHEMATIC REFERENCE ONLY. CONTRACTOR SHALL COORDINATE ALL FRANCHISE UTILITY DESIGN AND INSTALLATION WITH THE SPECIFIC UTILITY PROVIDER AND DESIGNER. CONTRACTOR SHALL COORDINATE ALL UTILITY INSTALLATIONS TO AVOID CONFLICTS AT EXISTING AND PROPOSED UTILITY CROSSINGS.

> AIR RELEASE VALVE NOTE
> REFER TO THE PROFILE SHEETS (C7.10 - C7.12) FOR THE AIR RELEASE VALVE (ARV) LOCATIONS.



CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

PROFILE SCALE

PROFILE SCALE

PROFILE W-2

STA 13+44.09 ~ INSTALL: 1. 8'X6", TEE 1. FIRE HYDRAN 1. 6" GATE VALV 1. 6" GATE VALV (15 LF OF 6" WA

1000

PROFILE SCALE

HORIZ: 1" = 40' VERT: 1" = 4'

1000

SHEA O. KIRKMAN

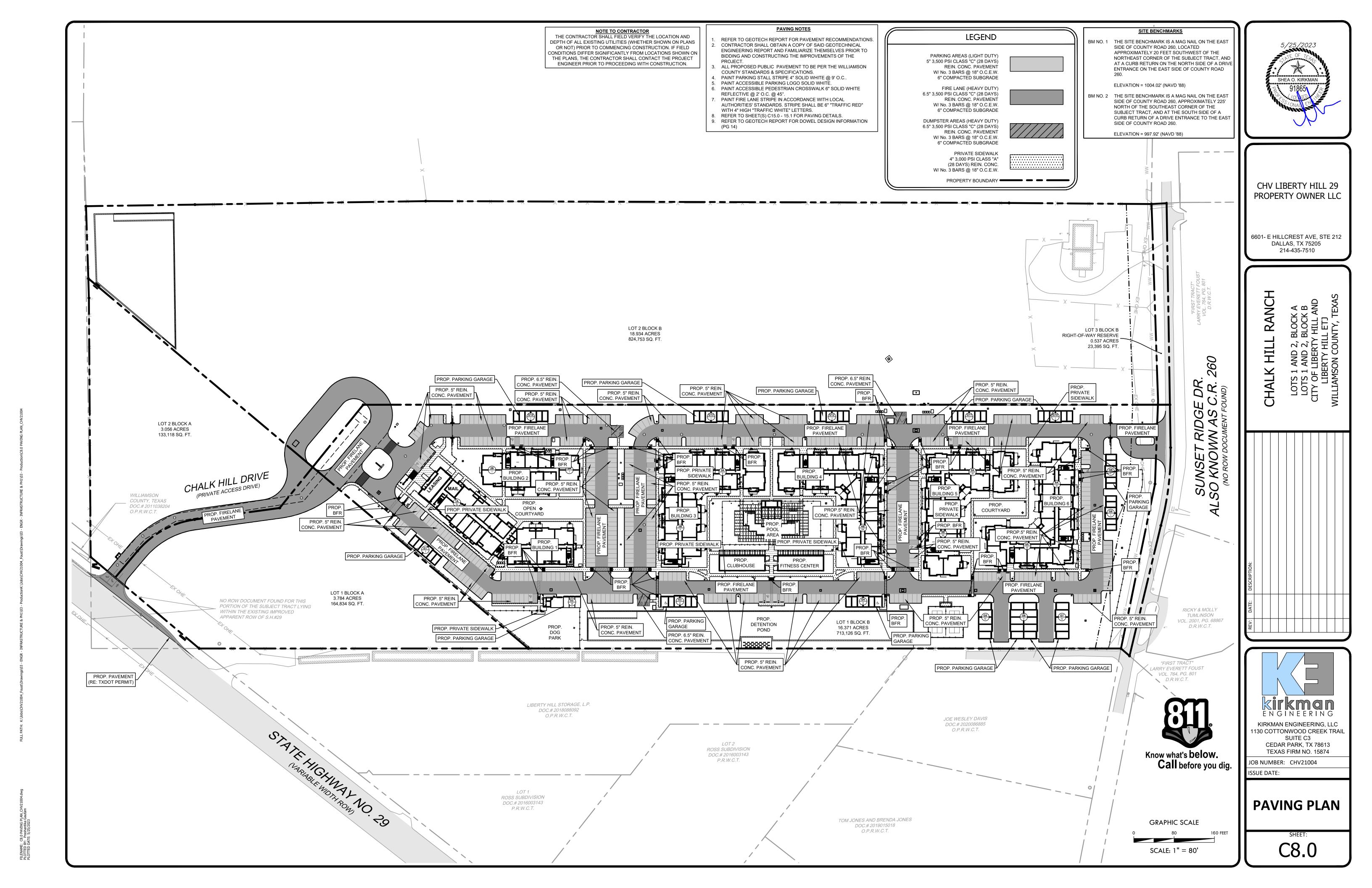
PROFILE SCALE

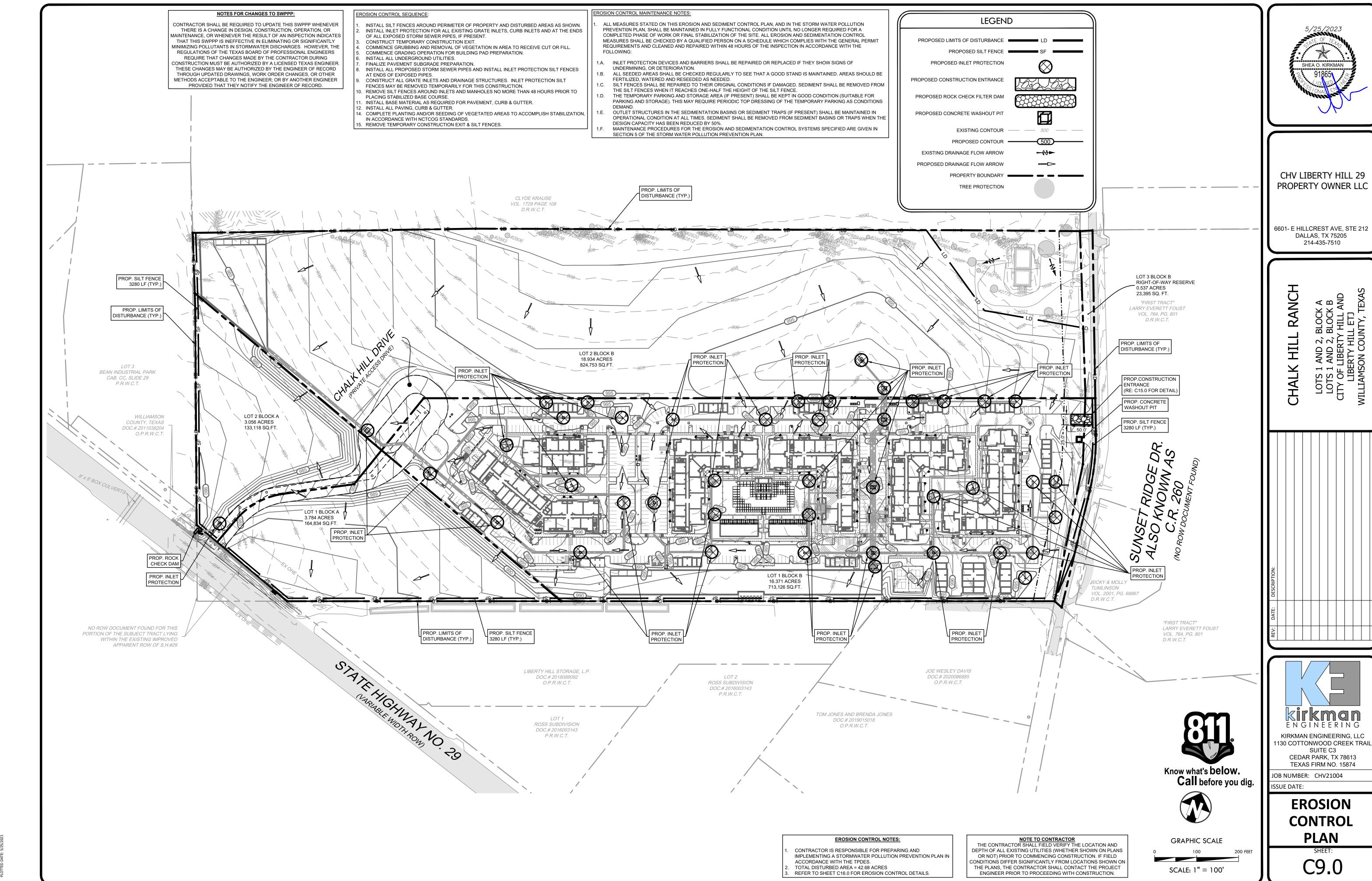
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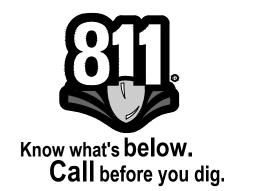
NAME: C7.0 UTILITY PLAN_CHV21004.dwg TTED BY: Prashantika Gautam

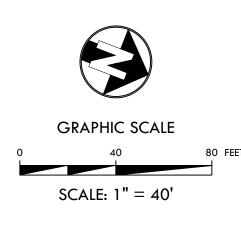


PROFILE SCALE











CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK HILL	1 C AINA 1 STOIL	LOIS 1 AND 2, I	CITY OF LIBERTY	I IBFRTY HII	WILLIAMSON COOL	



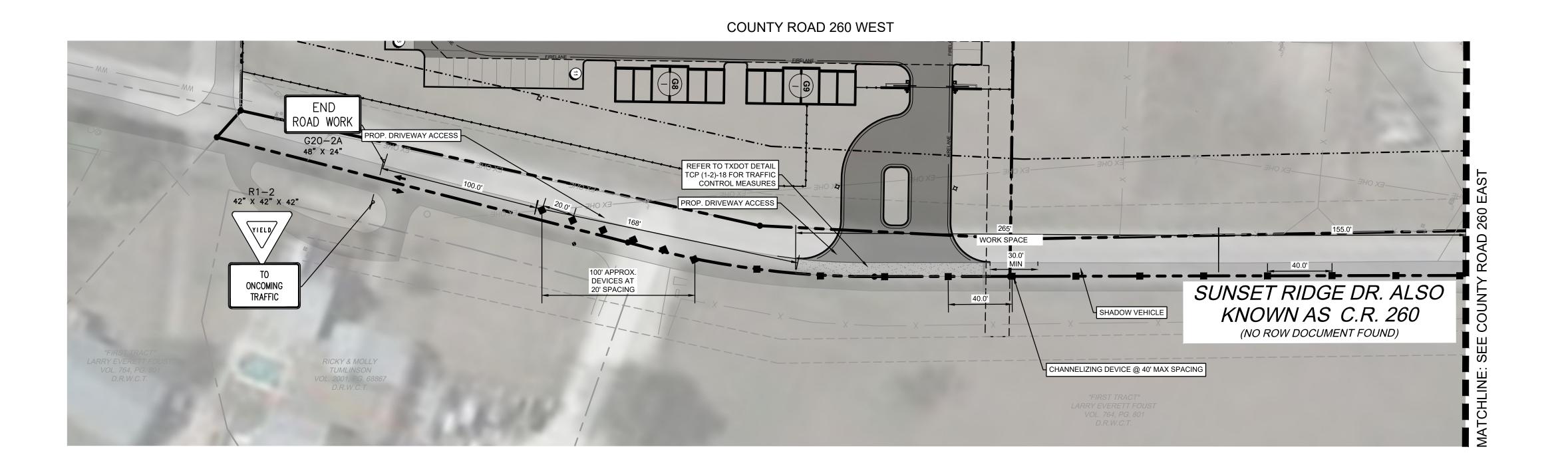
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

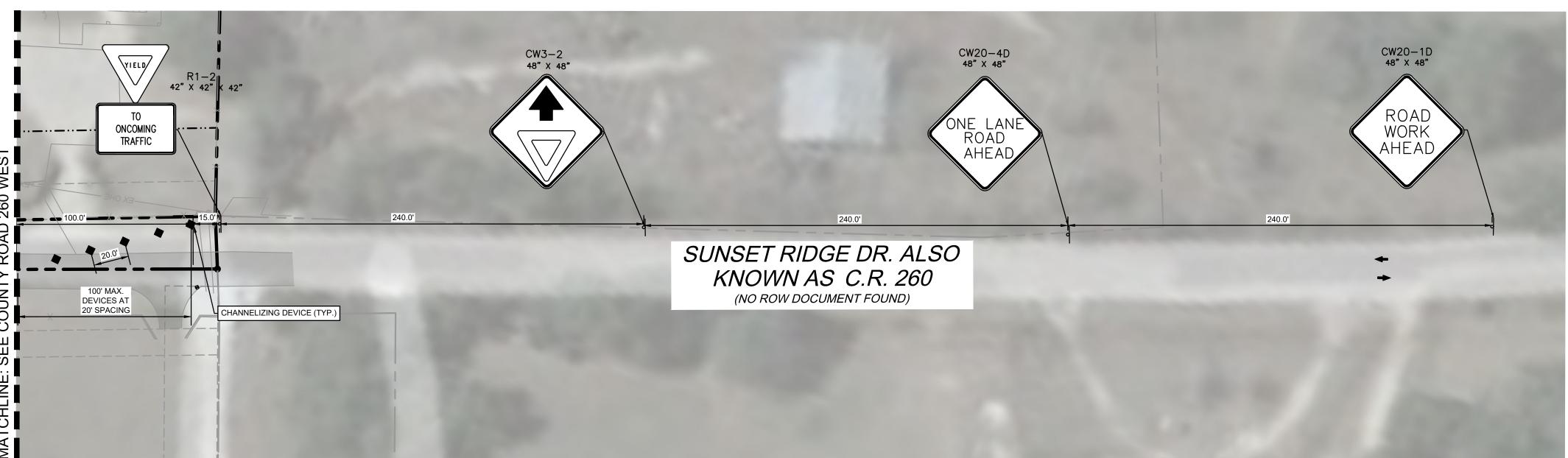
LANE CLOSURE

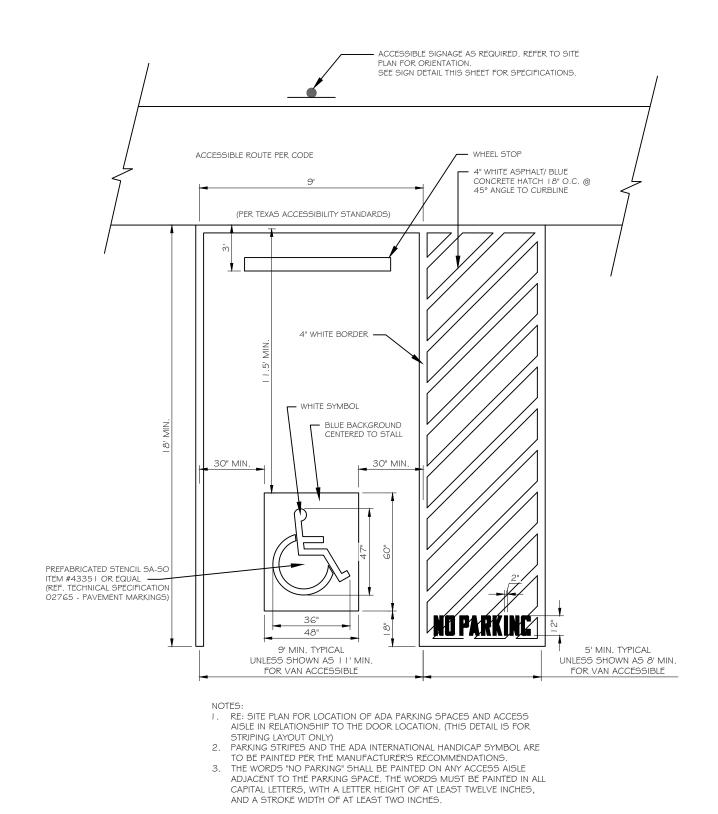
C10.0

PLAN

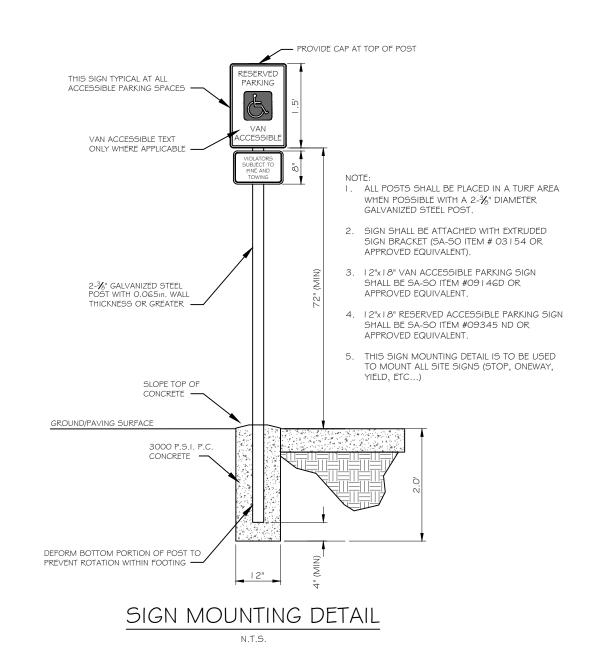


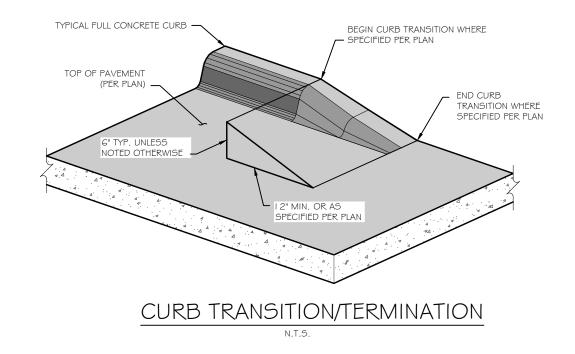
COUNTY ROAD 260 EAST

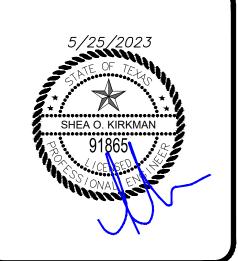




ACCESSIBLE PARKING SPACE







CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK HILL RANCH

LOTS 1
LOTS 1
CITY OF I
LIBE
WILLIAMS(



KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

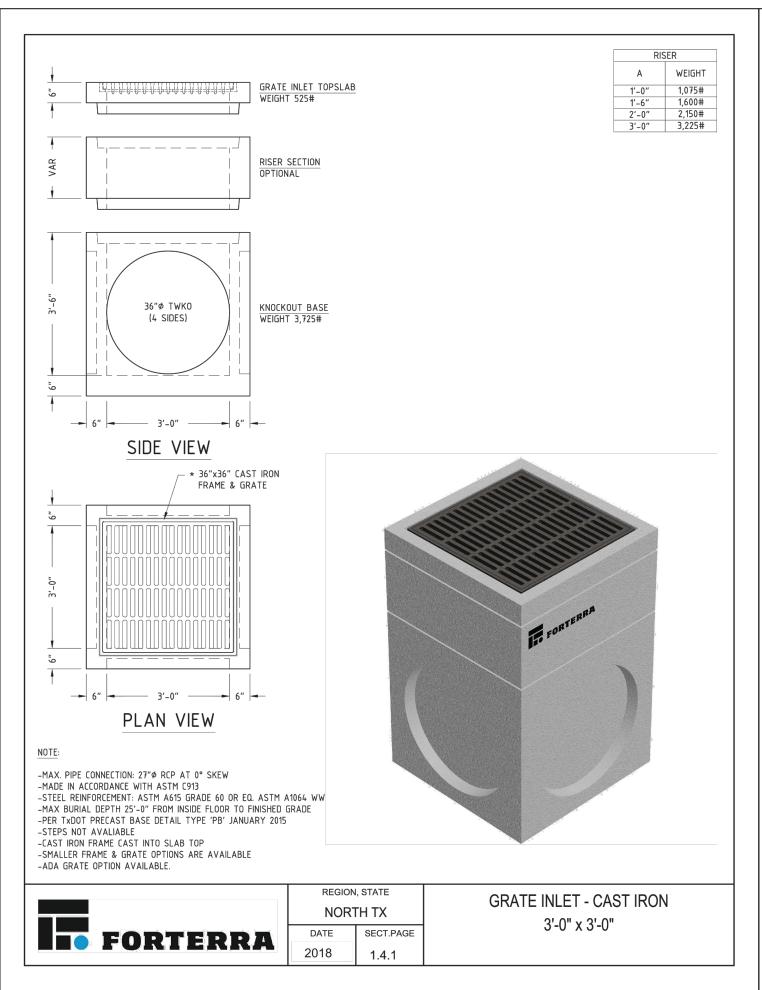
JOB NUMBER: CHV21004

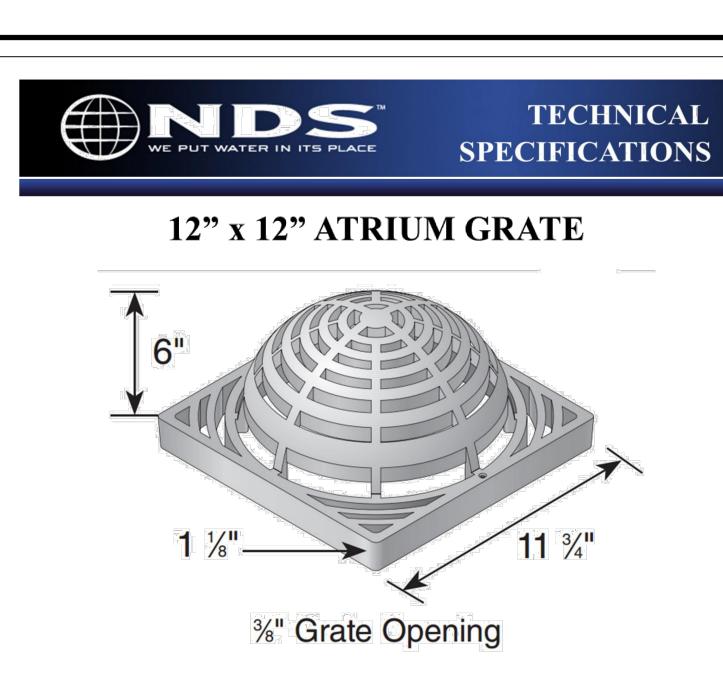
ISSUE DATE:

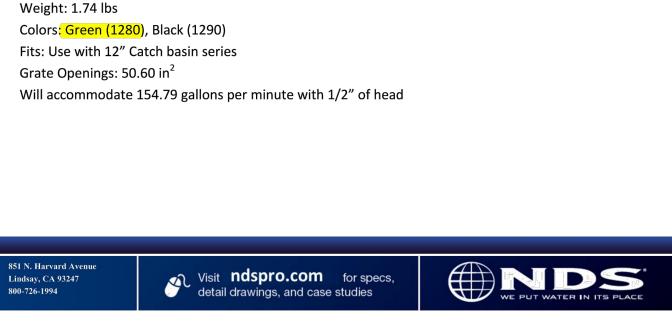
SITE DETAILS

C11.0

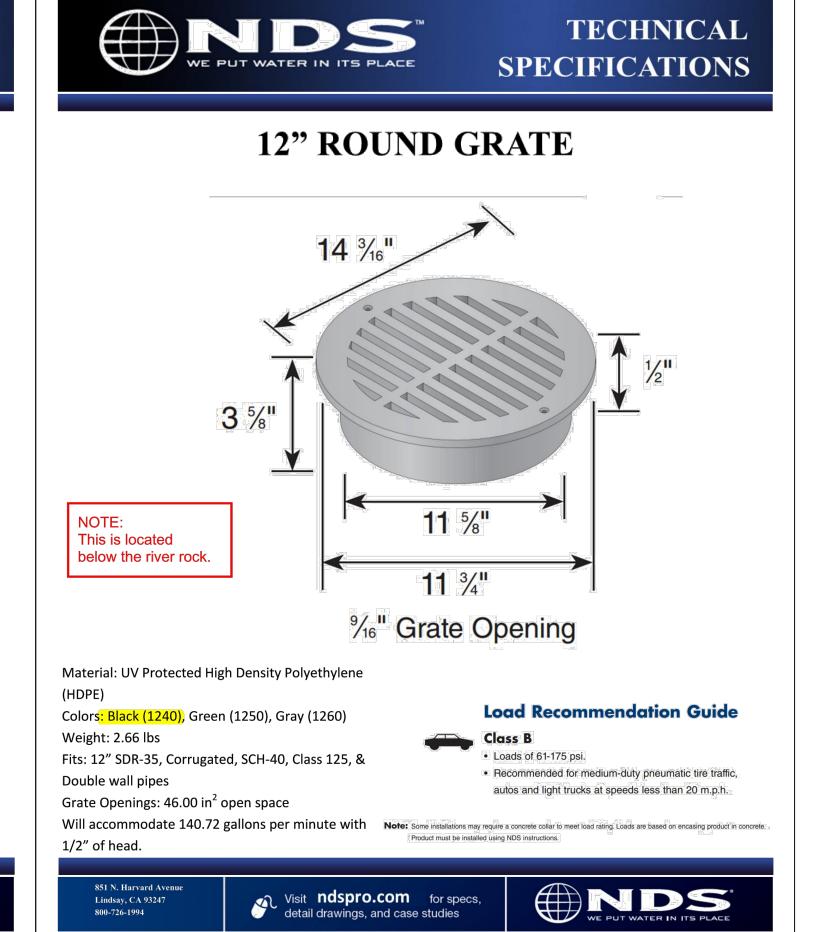




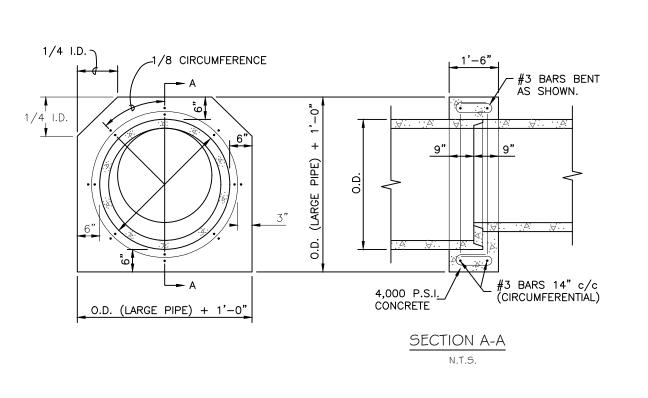




Material: UV Protected High Density Polyethylene (HDPE)







CONCRETE COLLAR DETAIL (PIPE)



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK

1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

> **DRAINAGE DETAILS I**

C12.0

SELECT BACKFILL

(6") MIN.

+ 300 mm (12")

PIPE SUPPORT

(APPROPRIATE

CONCRETE ENCASEMENT

505S-1

1 OF 1

THE ARCHITECT/ENGINEER ASSUMES

RESPONSIBILITY FOR APPROPRIATE USE OF THIS STANDARD.

PIPE O.D. + 300 mm

(12") MIN PIPE O.D. + 600 mm FOR TYPE AND SIZE OF

PIPE) AS SHOWN ON THE

DRAWING OR APPROVED BY THE ENGINEER

UNDISTURBED EARTH -

CLASS "D" CONCRETE

CENTER PIPE IN TRENCH

STRAPS AND ANCHORS, OR OTHER DEVICES APPROVED BY THE ENGINEER, SHALL BE INSTALLED TO PREVENT

FLOTATION OF THE PIPE.

AND LOCATION.

SEE DRAWINGS FOR NUMBER

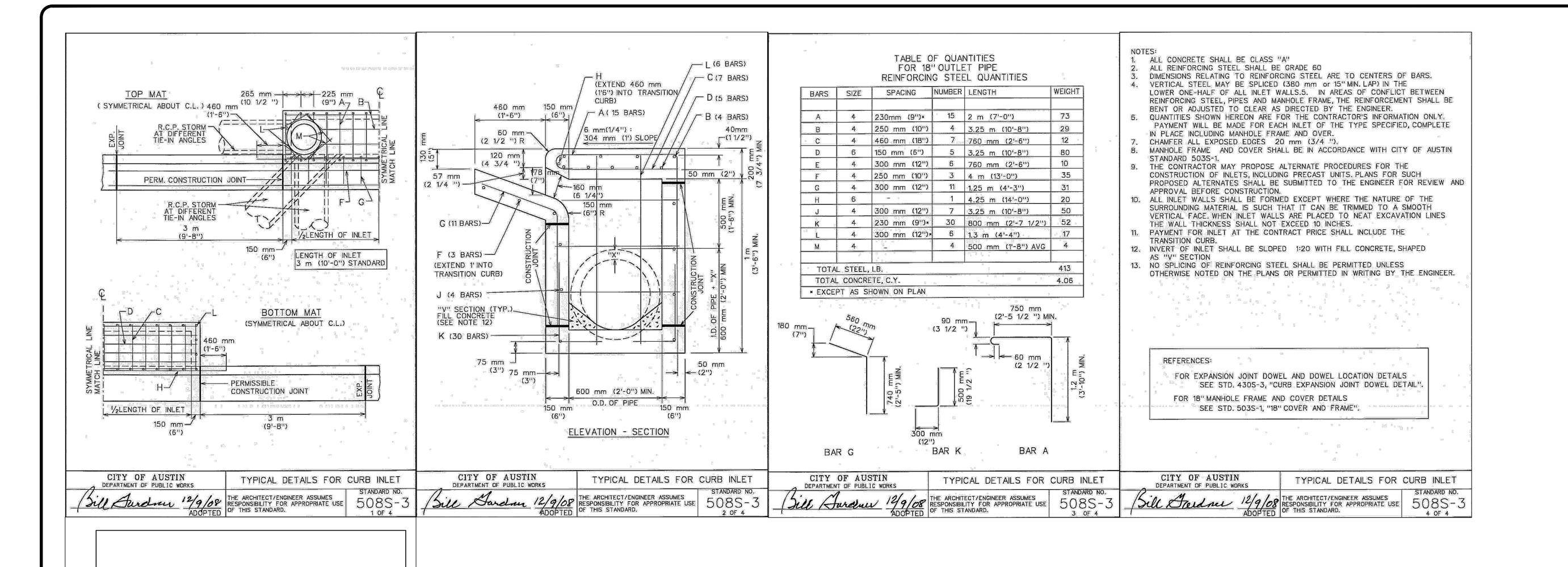
CITY OF AUSTIN

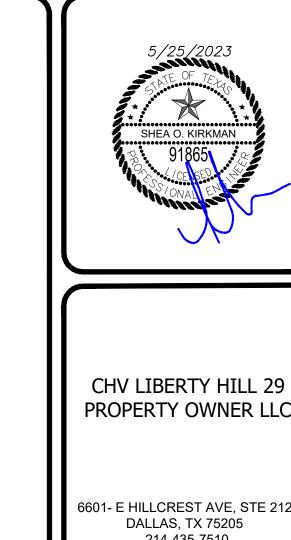
WATER AND WASTEWATER UTILITY

ADOPTED

ITEM 403



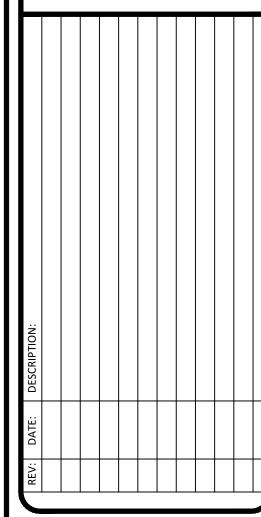




PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

> **RANCH** BLOCK A BLOCK B Y HILL AND LL ETJ CHALK





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

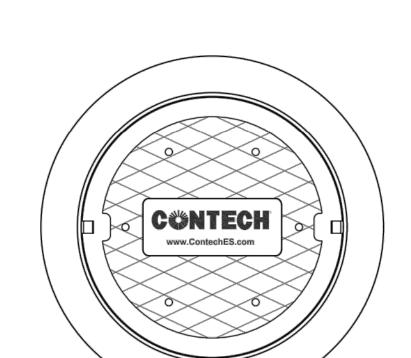
ISSUE DATE:

DRAINAGE DETAILS II

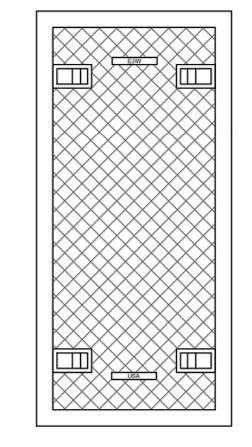
JELLYFISH DESIGN NOTES

CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD PEAK DIVERSION STYLE WITH PRECAST TOP SLAB IS SHOWN. ALTERNATE OFFLINE VAULT AND/OR SHALLOW ORIENTATIONS ARE AVAILABLE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD

CARTRIDGE LENGTH	54"
OUTLET INVERT TO STRUCTURE INVERT (A)	6'-6"
FLOW RATE HI-FLO / DRAINDOWN (CFS) (PER CART)	0.178 / 0.089
MAX. TREATMENT (CFS)	1.96
DECK TO INSIDE TOP (MIN) (B)	5.00



N.T.S.



STRUCTURE	ID				ST-B	
WATER QUA	LITY FLO	W RATE (cfs)		1.50	
PEAK FLOW	RATE (cfs	;)			*	
RETURN PER	RIOD OF F	PEAK FLO	W (yrs)		*	
# OF CARTRIDGES REQUIRED (HF / DD) 8/2						
CARTRIDGE	LENGTH				54	
					_	
PIPE DATA:	I.E.	MAT'L	DIA	SLOPE %	HGL	
INLET #1	989.66	RCP	24	*	*	
INLET #2	*	*	*	*	*	
OUTLET 989.66 RCP 24 * *						
SEE GENERAL NOTES 6-7 FOR INLET AND OUTLET HYDRAULIC AND SIZING REQUIREMENTS.						

SITE SPECIFIC

RIM ELEVATION		994.90
KIWI ELEVATION		001.00
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT
	*	*
NOTES/SPECIAL REQUIREMEN	NTS:	

FRAME AND COVER (DIAMETER VARIES)

TRENCH COVER (LENGTH VARIES)

N.T.S.

* PER ENGINEER OF RECORD

GENERAL NOTES:

INLET

TRANSFER

OUTLET TRANSFER

OPENING

PLAN VIEW

(TOP SLAB NOT SHOWN FOR CLARITY)

OPENING

HI FLO

STEPS (LOCATION MAY VARY)

DECK **WEIR**

FRAME AND COVER

TOP OF STRUCTURE)

TRANSFER

CARTRIDGE

- CARTRIDGE

— TRANSFER OPENING

ELEVATION VIEW

DECK

OPENING

(TRENCH COVER FLUSH WITH

RIM

ELEV. = 994.90'

ELEV. = 994.90'

TOP OF STRUCTURE

WEIR ELEV. =991.16'

STRUCTURE INV.

BOTTOM OF STRUCTURE

Jellyfish® Filter

FOLLOWING: U.S. PATENT NO. 8,287,726; 8,221,618; US 8,123,935;

OTHER INTERNATIONAL PATENTS PENDING

ELEV. = 983.16'

ELEV. = 982.49'

INLET INV. ELEV. = 989.66'

OUTLET INV. ELEV. = 989.66'

BLANK HI FLO CARTRIDGE

DRAINDOWN

CARTRIDGE

CARTRIDGE

INLET

FLOATABLES

Ø36" OPENING FOR Ø24" RCP INLET PIPE

BYPASS WEIR

Ø36" OPENING

FOR Ø24" RCP

OUTLET PIPE

BOTTOM OF

FLOATABLES

BAFFLE

TOP OF

BAFFLE

BYPASS WEIR

BAY

1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.

- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS REPRESENTATIVE. www.ContechES.com
- 3. JELLYFISH WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT
- 4. STRUCTURE SHALL MEET AASHTO HS-20 OR PER APPROVING JURISDICTION REQUIREMENTS, WHICHEVER IS MORE STRINGENT, ASSUMING EARTH COVER OF 0' - 10', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 LOAD RATING AND BE CAST WITH THE CONTECH LOGO.
- 5. STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C-857, ASTM C-918, AND AASHTO LOAD FACTOR DESIGN METHOD.
- 6. OUTLET PIPE INVERT IS EQUAL TO THE CARTRIDGE DECK ELEVATION.
- 7. THE OUTLET PIPE DIAMETER FOR NEW INSTALLATIONS IS RECOMMENDED TO BE ONE PIPE SIZE LARGER THAN THE INLET PIPE AT EQUAL OR GREATER SLOPE.
- 8. NO PRODUCT SUBSTITUTIONS SHALL BE ACCEPTED UNLESS SUBMITTED 10 DAYS PRIOR TO PROJECT BID DATE, OR AS DIRECTED BY THE ENGINEER OF RECORD.

INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STRUCTURE.
- C. CONTRACTOR WILL INSTALL AND LEVEL THE STRUCTURE, SEALING THE JOINTS, LINE ENTRY AND EXIT POINTS (NON-SHRINK GROUT WITH APPROVED WATERSTOP OR FLEXIBLE BOOT).
- D. CARTRIDGE INSTALLATION, BY CONTECH, SHALL OCCUR ONLY AFTER SITE HAS BEEN STABILIZED AND THE JELLYFISH UNIT IS CLEAN AND FREE OF DEBRIS. CONTACT CONTECH TO COORDINATE CARTRIDGE INSTALLATION WITH SITE STABILIZATION.



9025 Centre Pointe Dr., Suite 400, West Chester, OH 45069

513-645-7993 FAX

800-338-1122 513-645-7000

PROJECT NAME: FOUST TRACT - CHALK HILL VENTURES LOCATION: LIBERT HILL, TX SITE DESIGNATION: ST-B

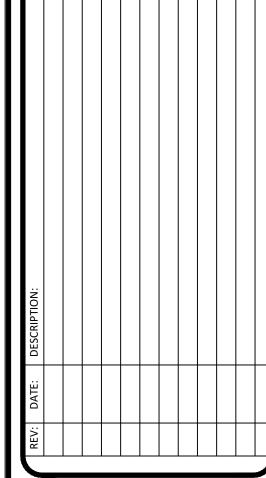
8' x 6' JELLYFISH - 715260- 020



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

WATER QUALITY DETAILS I

C12.2

Ø64" OPENING

FOR Ø48" RCP

OUTLET PIPE

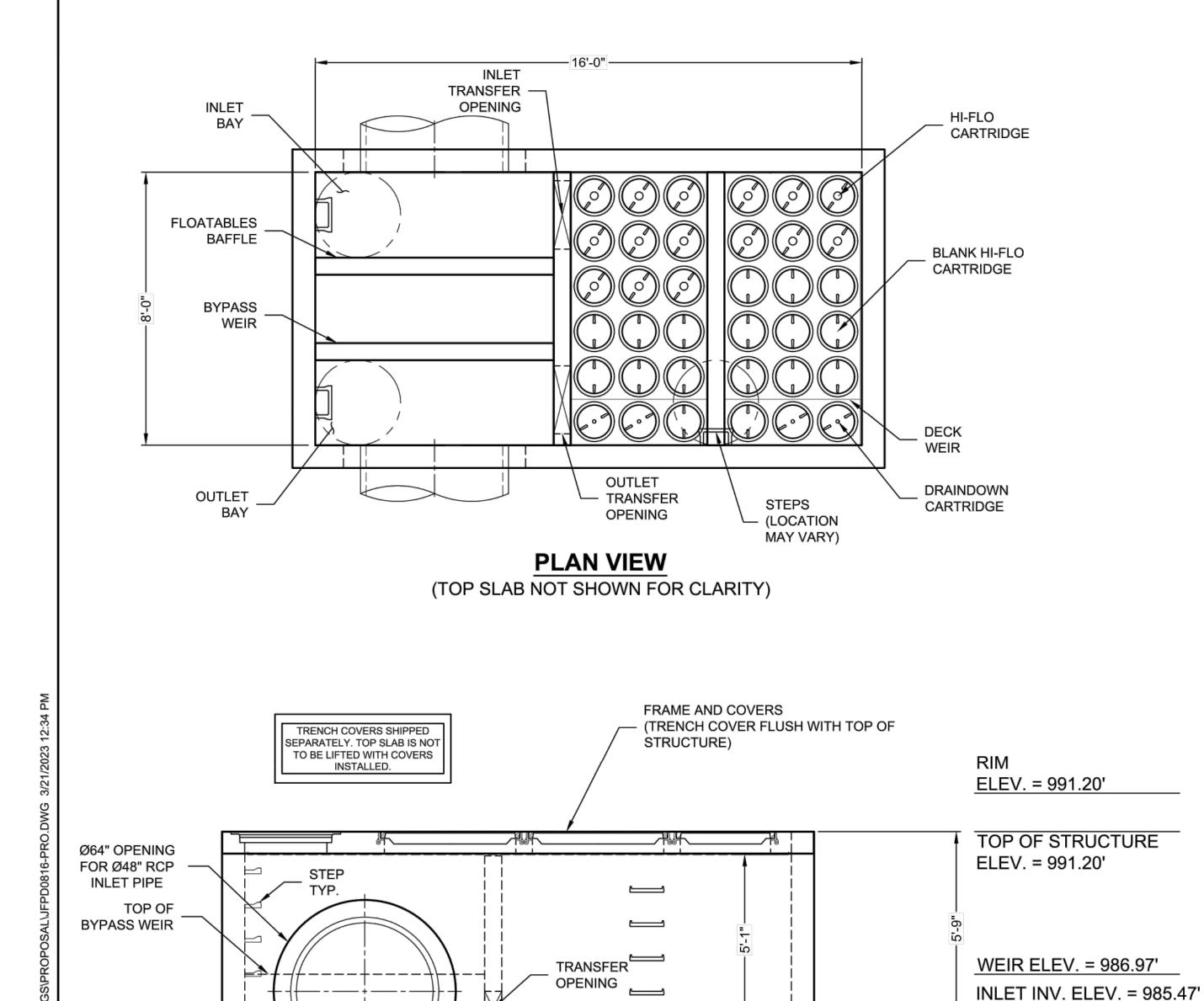
BOTTOM OF FLOATABLES

BAFFLE

TRANSFER OPENING

ELEVATION VIEW





CARTRIDGE

- CARTRIDGE

STRUCTURE INV.

BOTTOM OF STRUCTURE

ELEV. = 978.97'

ELEV. = 978.30'

FOLLOWING: U.S. PATENT NO. 8,287,726; 8,221,618; US 8,123,935;

OTHER INTERNATIONAL PATENTS PENDING

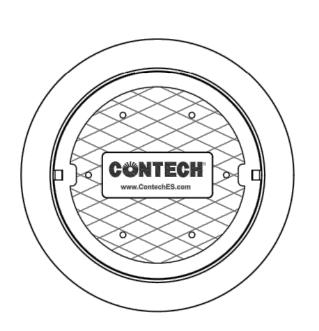
DECK

JELLYFISH DESIGN NOTES

JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD PEAK DIVERSION STYLE WITH PRECAST TOP SLAB IS SHOWN. ALTERNATE OFFLINE VAULT AND/OR SHALLOW ORIENTATIONS ARE AVAILABLE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD

CARTRIDGE SELECTION

O/INTRIBUL GELECTION	
CARTRIDGE LENGTH	54"
OUTLET INVERT TO STRUCTURE INVERT (A)	6'-6"
FLOW RATE HI-FLO / DRAINDOWN (CFS) (PER CART)	0.178 / 0.089
MAX. TREATMENT (CFS)	7.84
DECK TO INSIDE TOP (MIN) (B)	5.00



FRAME AND COVER (DIAMETER VARIES) N.T.S.

SITE SPECIFIC
DATA REQUIREMENTS
TRUCTURE ID

	STRUCTURE		ST-A					
	WATER QUA		2.94					
	PEAK FLOW		*					
	RETURN PER		*					
	# OF CARTR		15/4					
	CARTRIDGE		54					
	PIPE DATA:	I.E.	MAT'L	DIA	SLOPE %	6 HGL		
	INLET #1	985.47	RCP	48"	*	*		
	INLET #2	*	*	*	*	*		

SEE GENERAL NOTES 6-7 FOR INLET AND OUTLET HYDRAULIC AND SIZING REQUIREMENTS.

985.47 RCP 48"

RIM ELEVATION		991.20
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT
	*	*

NOTES/SPECIAL REQUIREMENTS: UNIT IS SIZED AS JFPD0811-15-4. VAULT SIZE IS 8X16 TO ACCOMMODATE 48" RCP INLET/OUTLET. * PER ENGINEER OF RECORD

GENERAL NOTES:

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED
- SOLUTIONS REPRESENTATIVE. www.ContechES.com
- 3. JELLYFISH WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING.
- CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
- 4. STRUCTURE SHALL MEET AASHTO HS-20 OR PER APPROVING JURISDICTION REQUIREMENTS, WHICHEVER IS MORE STRINGENT, ASSUMING EARTH COVER OF 0' - 10', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 LOAD RATING AND BE CAST WITH THE CONTECH LOGO.
- 5. STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C-857, ASTM C-918, AND AASHTO LOAD FACTOR DESIGN METHOD.
- OUTLET INV. ELEV. = 985.47' 6. OUTLET PIPE INVERT IS EQUAL TO THE CARTRIDGE DECK ELEVATION.
 - 7. THE OUTLET PIPE DIAMETER FOR NEW INSTALLATIONS IS RECOMMENDED TO BE ONE PIPE SIZE LARGER THAN THE INLET PIPE AT EQUAL OR GREATER SLOPE.
 - 8. NO PRODUCT SUBSTITUTIONS SHALL BE ACCEPTED UNLESS SUBMITTED 10 DAYS PRIOR TO PROJECT BID DATE, OR AS DIRECTED BY THE ENGINEER OF RECORD.

INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STRUCTURE.
- C. CONTRACTOR WILL INSTALL AND LEVEL THE STRUCTURE, SEALING THE JOINTS, LINE ENTRY AND EXIT POINTS (NON-SHRINK GROUT WITH APPROVED WATERSTOP OR FLEXIBLE BOOT).
- D. CARTRIDGE INSTALLATION, BY CONTECH, SHALL OCCUR ONLY AFTER SITE HAS BEEN STABILIZED AND THE JELLYFISH UNIT IS CLEAN AND FREE OF DEBRIS. CONTACT CONTECH TO COORDINATE CARTRIDGE INSTALLATION WITH SITE STABILIZATION.



www.ContechES.com 9100 Centre Pointe Dr., Suite 400, West Chester, OH 45069 800-338-1122 513-645-7000 513-645-7993 FAX 8' x 16' JELLYFISH - 715260 - 010 FOUST TRACK - CHALK HILL VENTURES LIBERTY HILL, TX SITE DESIGNATION: ST-A

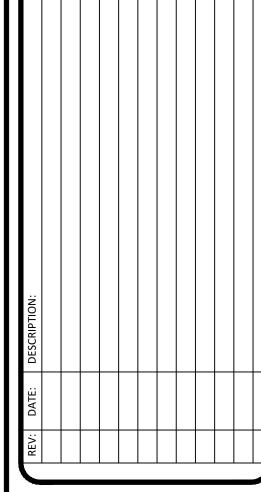


CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

LOTS 1 AND 2, BLOCK A
LOTS 1 AND 2, BLOCK B
CITY OF LIBERTY HILL AND
LIBERTY HILL ETJ

CHALK





KIRKMAN ENGINEERING, LLC 130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

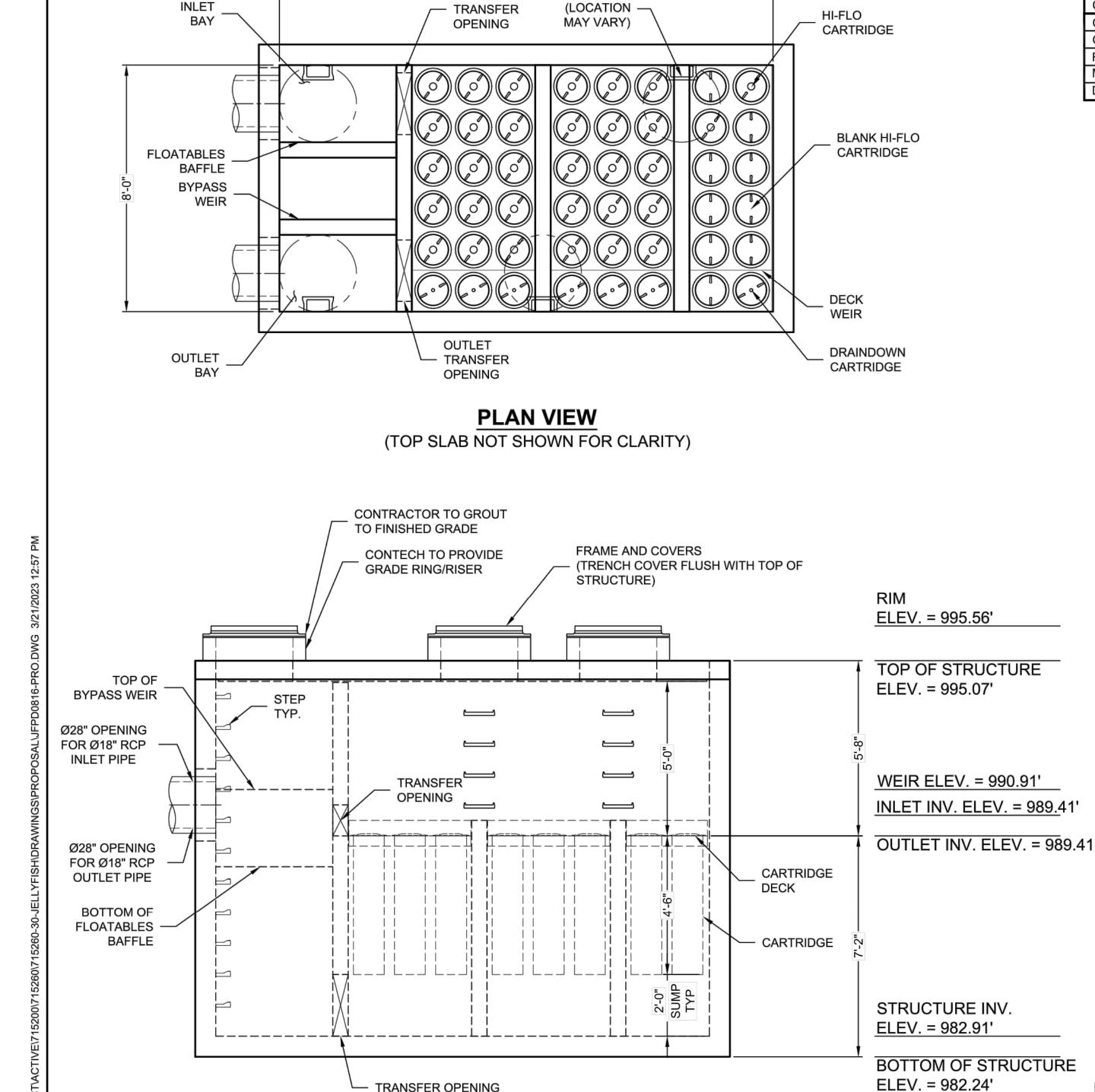
TEXAS FIRM NO. 158

JOB NUMBER: CHV21004

ISSUE DATE:

WATER QUALITY DETAILS II

~1**)** 3



TRANSFER OPENING

ELEVATION VIEW

INLET

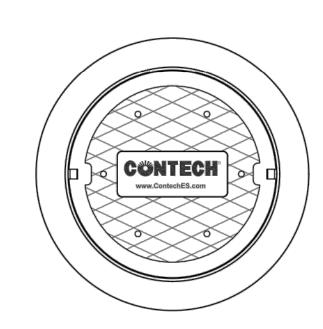
STEPS

JELLYFISH DESIGN NOTES

JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD PEAK DIVERSION STYLE WITH PRECAST TOP SLAB IS SHOWN. ALTERNATE OFFLINE VAULT AND/OR SHALLOW ORIENTATIONS ARE AVAILABLE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD

CARTRIDGE SELECTION

54"
6'-6"
0.178 / 0.089
7.84
5.00



FRAME AND COVER (DIAMETER VARIES) N.T.S.

SITE SPECIFIC **DATA REQUIREMENTS**

<u> </u>						
STRUCTURE ID					ST-C	
WATER QUALITY FLOW RATE (cfs)					6.32	
PEAK FLOW RATE (cfs)						
RETURN PERIOD OF PEAK FLOW (yrs)						
# OF CARTR		32/7				
CARTRIDGE		54				
PIPE DATA:	I.E.	MAT'L	DIA	SLOPE %	HGL	
INLET #1	989.41	RCP	18	*	*	
INI FT #2	*	*	*	*	*	

SEE GENERAL NOTES 6-7 FOR INLET AND OUTLET HYDRAULIC AND SIZING REQUIREMENTS

989.41 RCP

RIM ELEVATION	995.56	
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT
	*	*

NOTES/SPECIAL REQUIREMENTS: UNIT IS PLACED OFFLINE AND PAIRED WITH CONTECH STORMGATE DUE TO (2) 5'x3' RCB MAINLINE. PER ENGINEER OF RECORD

GENERAL NOTES:

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED
- SOLUTIONS REPRESENTATIVE. www.ContechES.com
- 3. JELLYFISH WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
- 4. STRUCTURE SHALL MEET AASHTO HS-20 OR PER APPROVING JURISDICTION REQUIREMENTS, WHICHEVER IS MORE STRINGENT, ASSUMING EARTH COVER OF 0' - 10', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 LOAD RATING AND BE CAST WITH THE CONTECH LOGO.
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- OUTLET INV. ELEV. = 989.41' 6. OUTLET PIPE INVERT IS EQUAL TO THE CARTRIDGE DECK ELEVATION
 - 7. THE OUTLET PIPE DIAMETER FOR NEW INSTALLATIONS IS RECOMMENDED TO BE ONE PIPE SIZE LARGER THAN THE INLET PIPE AT EQUAL OR
 - 8. NO PRODUCT SUBSTITUTIONS SHALL BE ACCEPTED UNLESS SUBMITTED 10 DAYS PRIOR TO PROJECT BID DATE, OR AS DIRECTED BY THE ENGINEER OF RECORD.

INSTALLATION NOTES

FOLLOWING: U.S. PATENT NO. 8,287,726; 8,221,618; US 8,123,935;

OTHER INTERNATIONAL PATENTS PENDING

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STRUCTURE.
- C. CONTRACTOR WILL INSTALL AND LEVEL THE STRUCTURE, SEALING THE JOINTS, LINE ENTRY AND EXIT POINTS (NON-SHRINK GROUT WITH
- APPROVED WATERSTOP OR FLEXIBLE BOOT). D. CARTRIDGE INSTALLATION, BY CONTECH, SHALL OCCUR ONLY AFTER SITE HAS BEEN STABILIZED AND THE JELLYFISH UNIT IS CLEAN AND FREE OF DEBRIS. CONTACT CONTECH TO COORDINATE CARTRIDGE INSTALLATION WITH SITE STABILIZATION.



www.ContechES.com 9100 Centre Pointe Dr., Suite 400, West Chester, OH 45069 800-338-1122 513-645-7000 513-645-7993 FAX

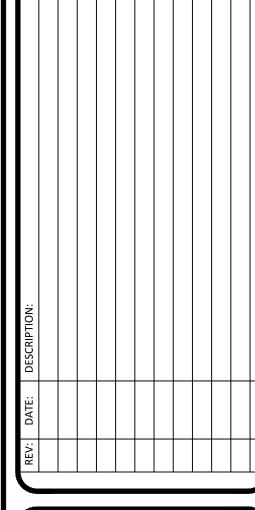
8' x 16' JELLYFISH - 715260 - 030 FOUST TRACT - CHALK HILL VENTURES LIBERTY HILL, TX SITE DESIGNATION: ST-C



CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

CHALK



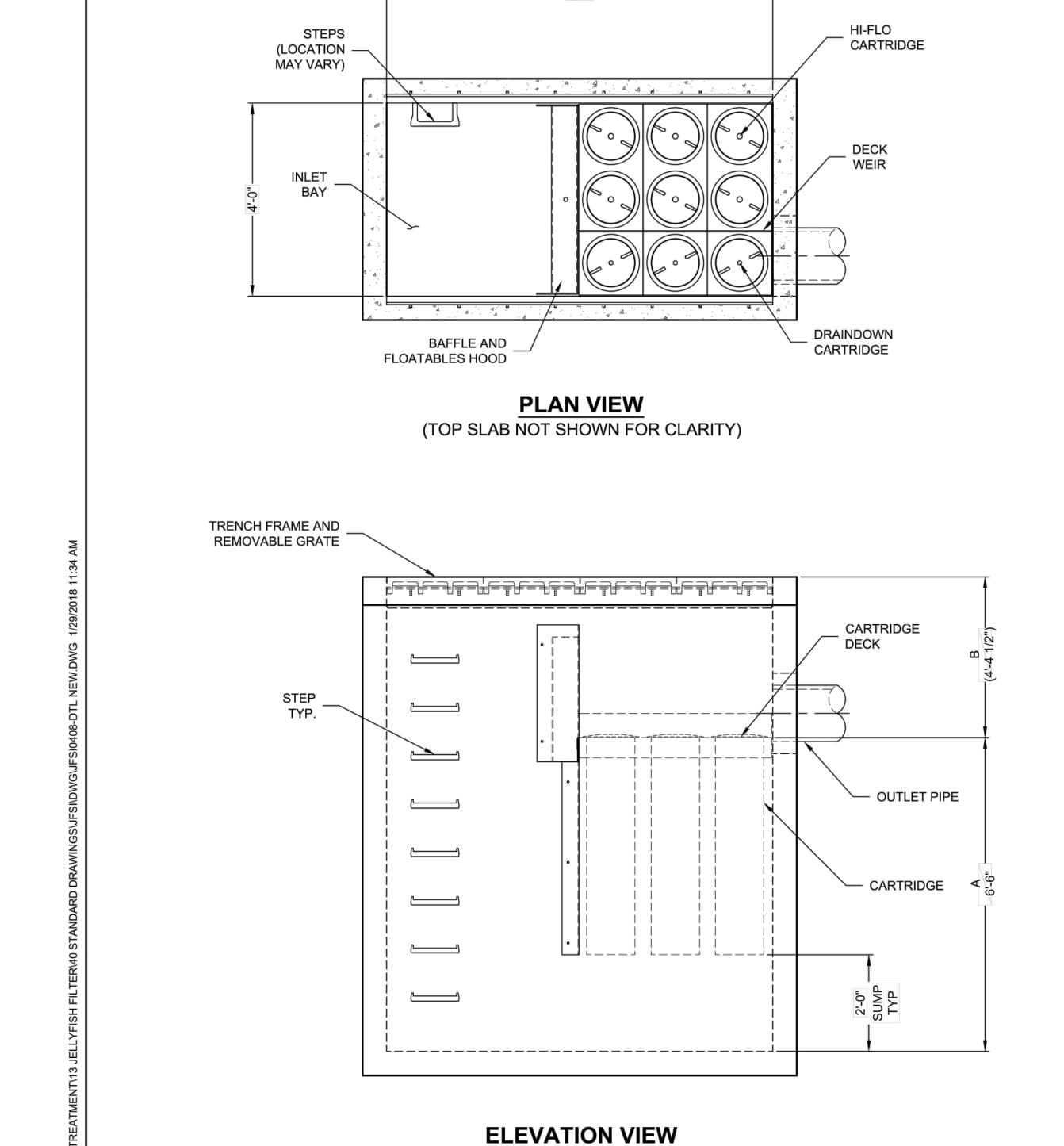


KIRKMAN ENGINEERING, LLC 130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 SSUE DATE:

> **WATER QUALITY DETAILS III**

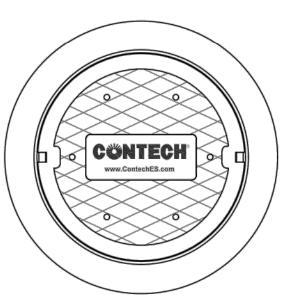


JELLYFISH DESIGN NOTES

JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE LENGTH AND THE NUMBER OF CARTRIDGES. THE STANDARD SURFACE INLET STYLE WITH TRENCH GRATE AND COVER IS SHOWN. ALTERNATE CURB INLET, PIPE INLET, OR SLAB TOP WITH EARTH COVER OPTIONS ARE AVAILABLE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD

CARTRIDGE SELECTION

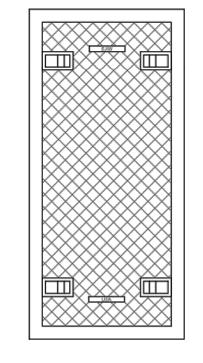
CARTRIDGE SELECTION				
CARTRIDGE LENGTH	54"	40"	27"	15"
OUTLET INVERT TO STRUCTURE INVERT (A)	6'-6"	5'-4"	4'-3"	3'-3"
FLOW RATE HIGH-FLO / DRAINDOWN (CFS) (PER CART)	0.178 / 0.089	0.133 / 0.067	0.089 / 0.045	0.049 / 0.025
MAX. TREATMENT (CFS)	1.34	1.00	0.67	0.37
OUTLET INVERT TO RIM (MIN) (B)	3'-4"	3'-4"	3'-4"	3'-4"

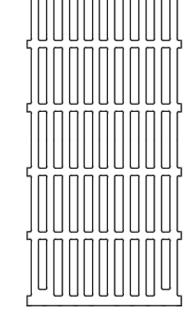


FRAME AND COVER

(DIAMETER VARIES)

N.T.S.





24" TRENCH GRATE N.T.S.

DATA REQUIREMENTS						
STRUCTURE	: ID				ST-G	
WATER QUA	LITY FLO	W RATE (cfs)		0.99	
PEAK FLOW	RATE (cfs	s)			*	
RETURN PER	RIOD OF F	PEAK FLO	W (yrs)		*	
# OF CARTR	IDGES RE	QUIRED	(HF / DD)		5/2	
CARTRIDGE LENGTH 54						
PIPE DATA:	I.E.	MAT'L	DIA	SLOPE '	% HGL	
INLET #1	*	*	*	*	*	
INLET #2	*	*	*	*	*	
OUTLET 979.50 RCP 15" * *						
SEE GENER HYDRAULIC					LET	

SITE SPECIFIC

RIM ELEVATION		983.89
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT
	*	*
NOTES/SPECIAL REQUIREMENTS:		

TOP OF BAFFLE WALL ELEV. = 981.83'

* PER ENGINEER OF RECORD

GENERAL NOTES:

1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.

TRENCH COVER

- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS REPRESENTATIVE. www.ContechES.com
- 3. JELLYFISH WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT
- 4. STRUCTURE SHALL MEET AASHTO HS-20 OR PER APPROVING JURISDICTION REQUIREMENTS, WHICHEVER IS MORE STRINGENT, ASSUMING EARTH COVER OF 0' TO 10', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 LOAD RATING AND BE CAST WITH THE CONTECH LOGO.
- 5. STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C-857, ASTM C-918, AND AASHTO LOAD FACTOR DESIGN METHOD.
- 6. OUTLET PIPE INVERT IS EQUAL TO THE CARTRIDGE DECK ELEVATION.

513-645-7993 FAX

- 7. THE OUTLET PIPE DIAMETER FOR NEW INSTALLATIONS IS RECOMMENDED TO BE ONE PIPE SIZE LARGER THAN THE INLET PIPE (WHERE APPLICABLE) AT EQUAL OR GREATER SLOPE.
- 8. NO PRODUCT SUBSTITUTIONS SHALL BE ACCEPTED UNLESS SUBMITTED 10 DAYS PRIOR TO PROJECT BID DATE, OR AS DIRECTED BY THE ENGINEER OF RECORD.

Jellyfish® Filter

FOLLOWING: U.S. PATENT NO. 8,287,726; 8,221,618; US 8,123,935;

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STRUCTURE.
- C. CONTRACTOR WILL INSTALL AND LEVEL THE STRUCTURE, SEALING THE JOINTS, LINE ENTRY AND EXIT POINTS (NON-SHRINK GROUT WITH APPROVED WATERSTOP OR FLEXIBLE BOOT).
- D. CARTRIDGE INSTALLATION, BY CONTECH, SHALL OCCUR ONLY AFTER SITE HAS BEEN STABILIZED AND THE JELLYFISH UNIT IS CLEAN AND FREE OF DEBRIS. CONTACT CONTECH TO COORDINATE CARTRIDGE INSTALLATION WITH SITE STABILIZATION.



800-338-1122 513-645-7000

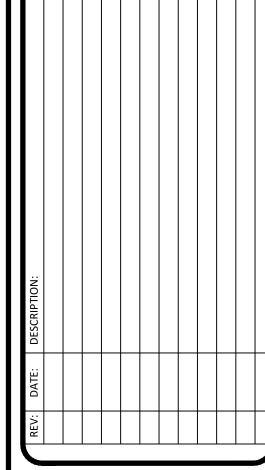
JELLYFISH JFSI0408 STANDARD DETAIL SURFACE INLET CONFIGURATION



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CHALK





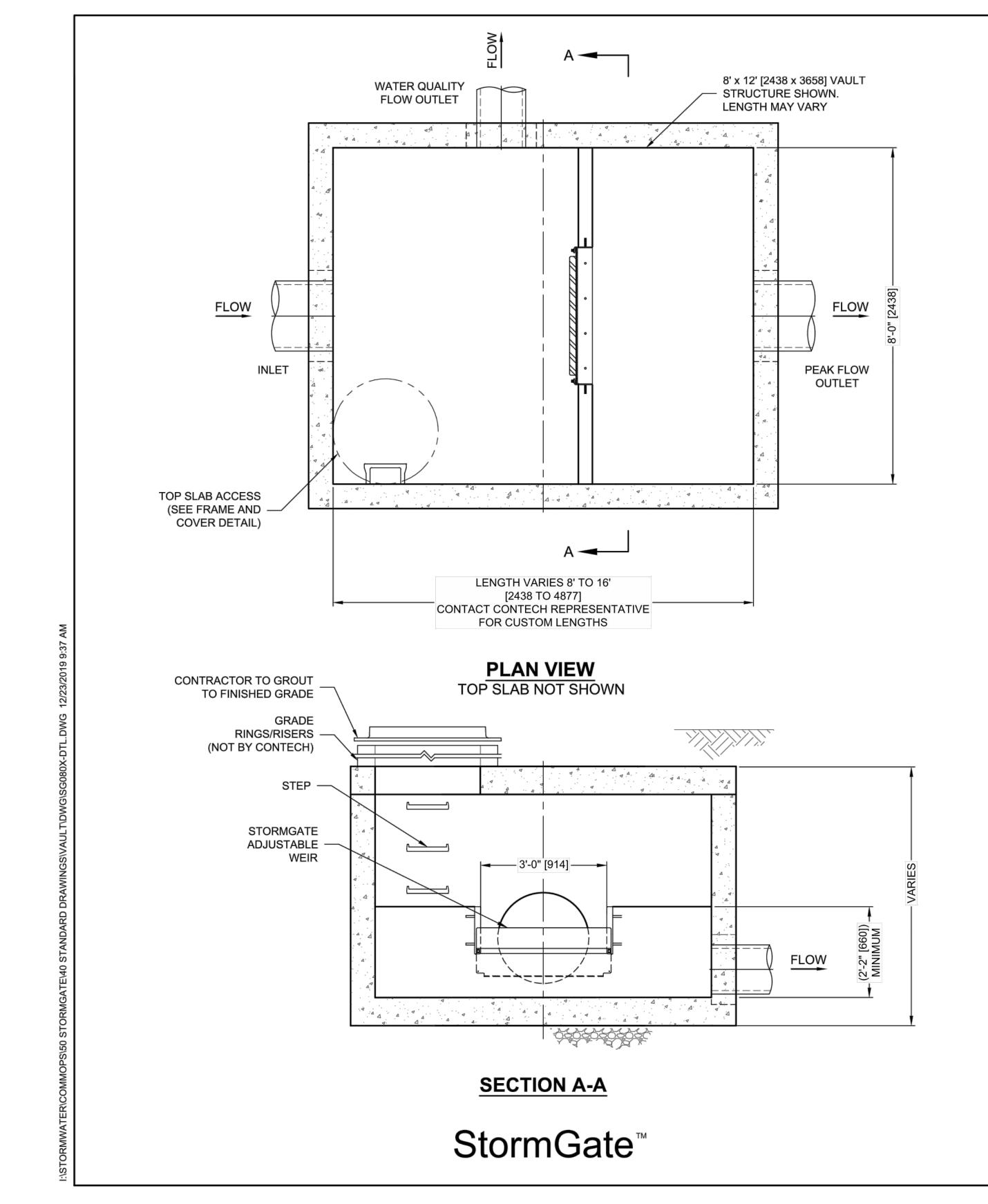
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

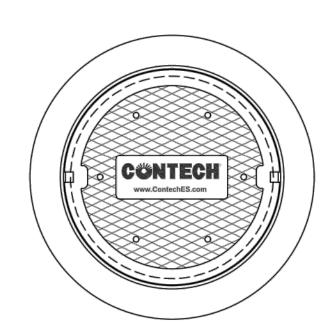
JOB NUMBER: CHV21004

WATER **QUALITY DETAILS IV**



STORMGATE DESIGN NOTES

THE STORMGATE IS A WEIR STYLE DIVERSION STRUCTURE. A 8' [2438] WIDE VAULT CONFIGURATION IS SHOWN. THE UNIT IS DESIGNED SUCH THAT THE TREATMENT FLOW IS DIVERTED TO THE OFFLINE WATER QUALITY DEVICE. THE HIGH FLOWS ARE BYPASSED OVER THE WEIR DIRECTLY TO THE DOWNSTREAM CONVEYANCE. THE STORMGATE TYPICALLY HAS ONE INLET, A LOW FLOW OUTLET FOR TREATMENT FLOW, AND A HIGH FLOW BYPASS AFTER INSTALLATION. THE CAPACITY OF THE STORMGATE IS A FUNCTION OF THE AVAILABLE HEIGHT ABOVE THE WEIR (HEAD) AND UNIT GEOMETRY PLEASE CONTACT YOUR LOCAL CONTECH DESIGN ENGINEER FOR SIZING ASSISTANCE



FRAME AND COVER (DIAMETER VARIES) NOT TO SCALE

STRUCTURE ID				ST-C BYPAS	
WATER QUALITY	(WQ) FLOW	RATE (cfs [L/s])		6.32 cfs	
PEAK FLOW RATE	(cfs [L/s])			*	
RIM ELEVATION				995.56	
PIPE DATA:	INVERT	ORIENTATION	MATERIAL	DIAMETER	
INLET PIPE	989.41	16' SIDE	RCB	(2) 5'x3'	
WQ FLOW OUTLET PIPE	989.41	8' SIDE	RCP	18"	
PEAK FLOW OUTLET PIPE	989.41	16' SIDE	RCB	(2) 5'x3'	
ORIFICE TYPE (PIPE, PLATE)					
ORIFICE DIAMETER (in. [mm])					
WEIR CREST ELEVATION 990.51					
WEIR WALL ELEVATION 991.01					
HEAD OVER WEIF	R, H (ft. [mm])			
WSE AT PEAK FLO	OW RATE				
WEIR ORIENTATION					
FLOOR ELEVATION					
NOTES / SPECIAL REQUIREMENTS:					
UNIT IS 8'x16'					

GENERAL NOTES

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.ContechES.com
- 3. STORMGATE WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS THE REQUIREMENTS OF THE PROJECT.
- 4. STORMGATE STRUCTURE SHALL MEET AASHTO HS-20 LOAD RATING, ASSUMING EARTH COVER OF 0' 5' [1524] AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.
- 5. STORMGATE STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C857, ASTM C918 AND ACI-318 LOAD FACTOR DESIGN METHOD.
- 6. WEIR FRAME AND ADJUSTABLE WEIR PLATE SHALL BE FABRICATED OF ALUMINUM ALLOY ASTM B209, T-5052, T-6061, PROVIDED AND INSTALLED BY CONTECH.
- 7. ALTERNATE UNITS ARE SHOWN IN [mm].

INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMGATE STRUCTURE.
- C. CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
- D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET AND OUTLET PIPE(S). MATCH PIPE INVERTS WITH ELEVATIONS SHOWN ON SITE SPECIFIC DRAWINGS. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES.
- E. CONTRACTOR TO INSTALL GRADE RINGS/RISERS OR BLOCK REQUIRED BETWEEN THE TOP OF THE STRUCTURE AND THE BASE OF THE MANHOLE FRAMES.
- F. CONTRACTOR TO ADJUST STORMGATE WEIR TO DESIGN ELEVATION SPECIFIED IN DRAWING. DO NOT EXCEED 5.0 FT-LBS TORQUE WHEN TIGHTENING SCREWS ON WEIR FRAME. SEAL WEIR TO FRAME WITH SIKAFLEX 1A SEALANT AFTER FINAL ADJUSTMENT.



www.ContechES.com 9025 Centre Pointe Dr., Suite 400, West Chester, OH 45069 800-338-1122 513-645-7000 513-645-7993 FAX

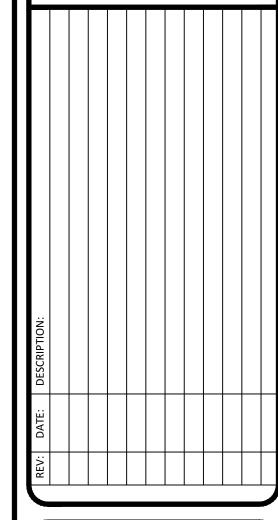
8' WIDTH x VARYING LENGTH STORMGATE VAULT STANDARD DETAIL



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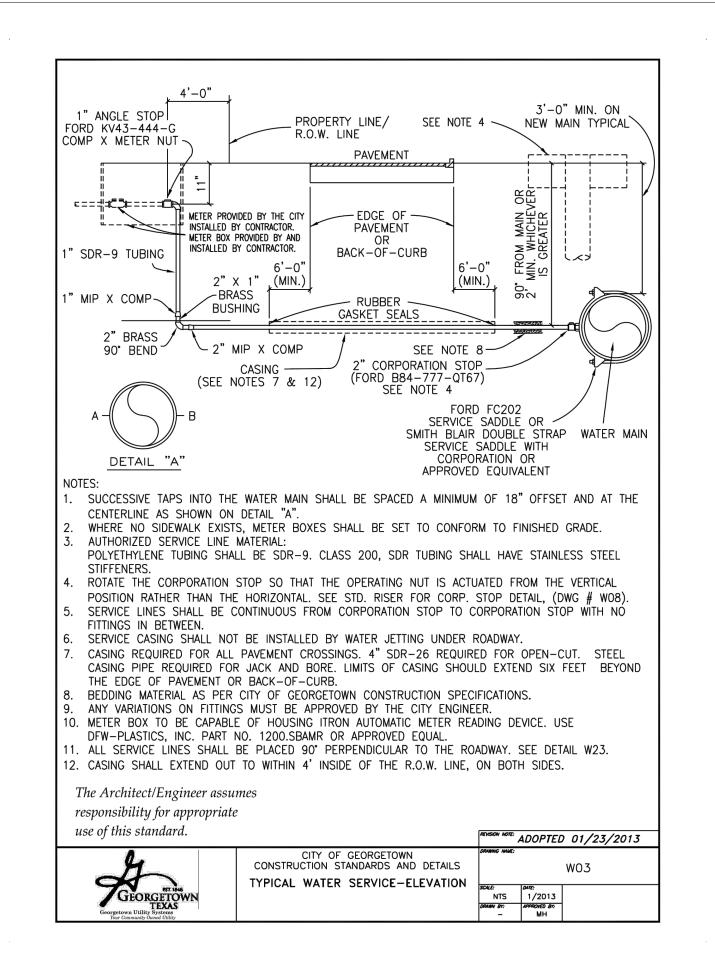
CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

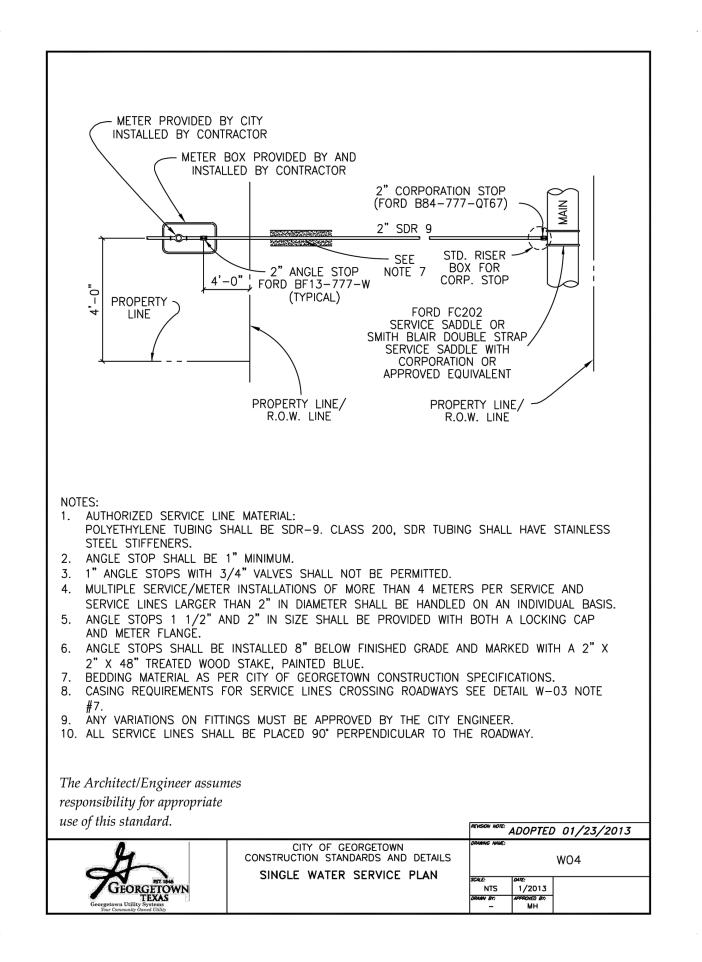
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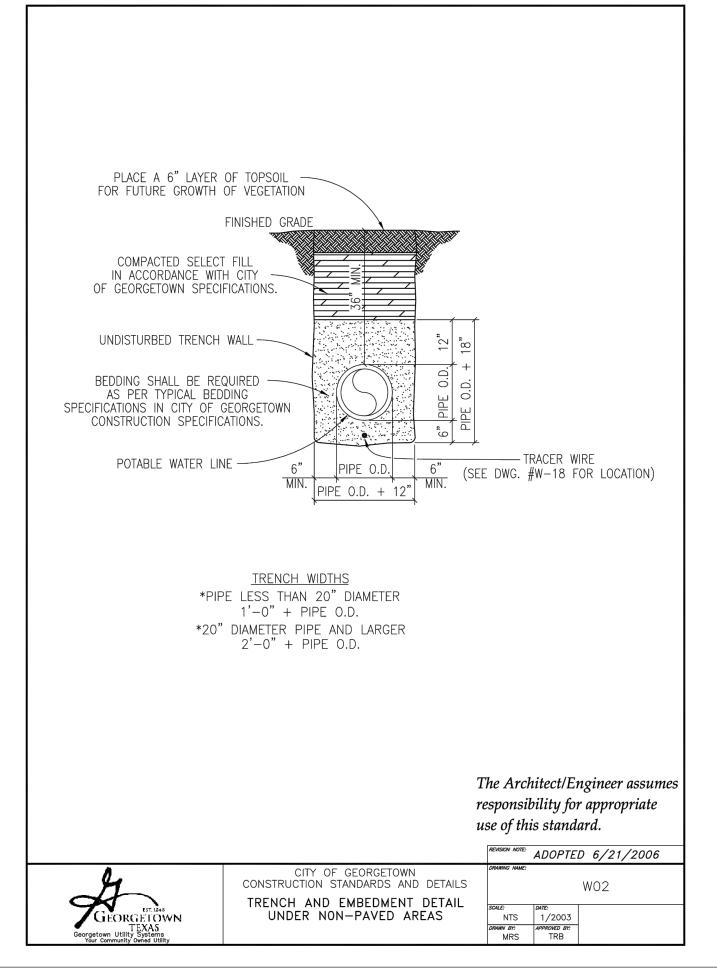
> **WATER QUALITY DETAILS V**

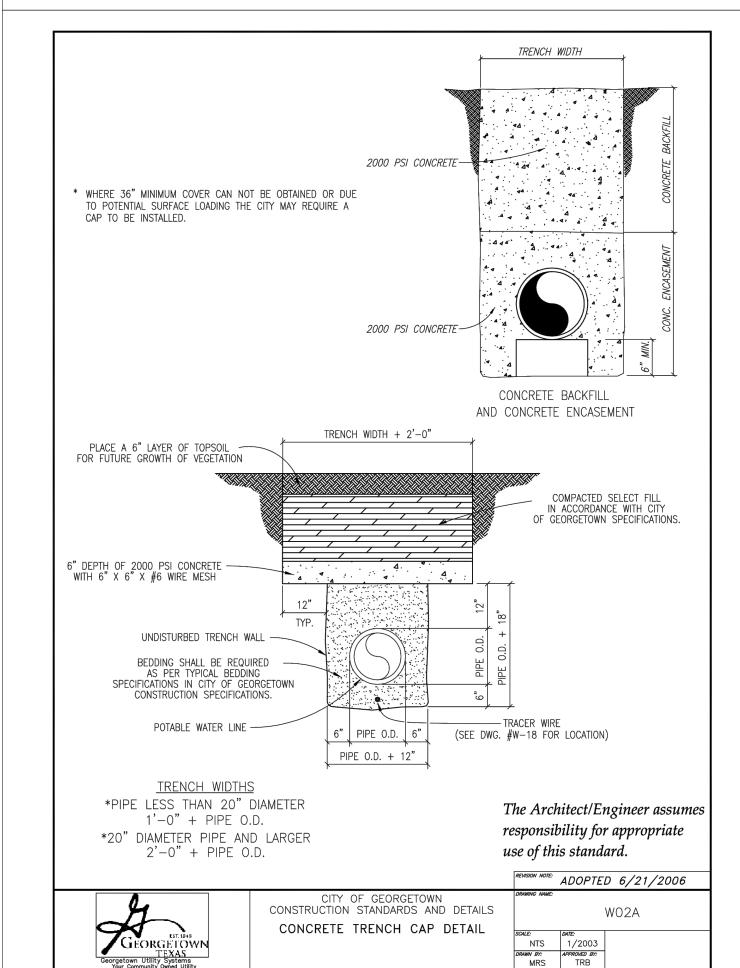
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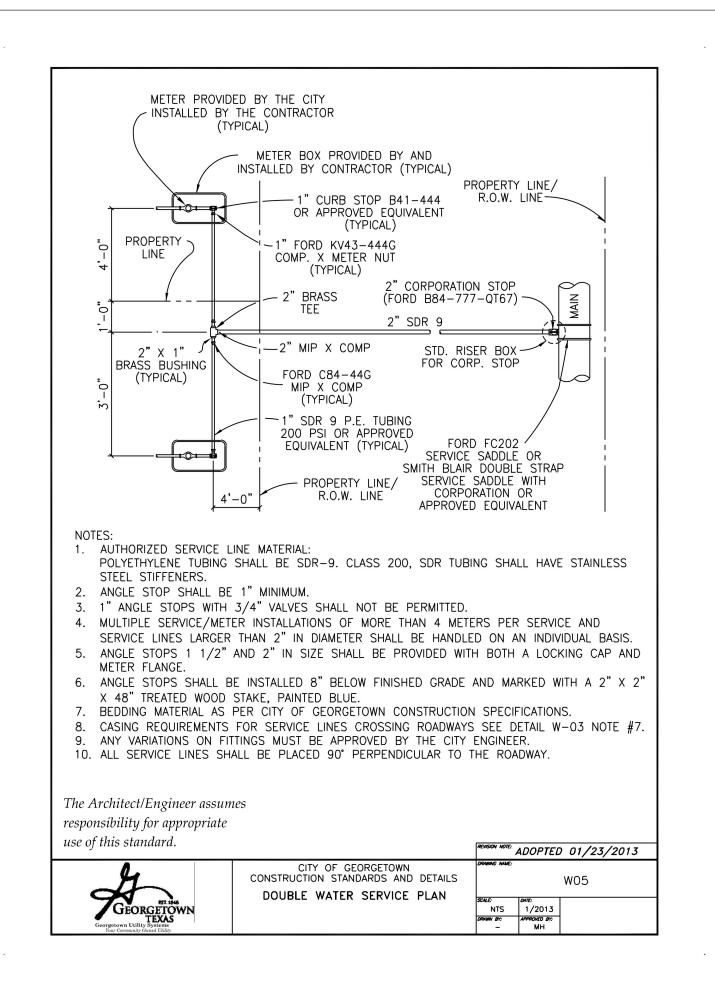


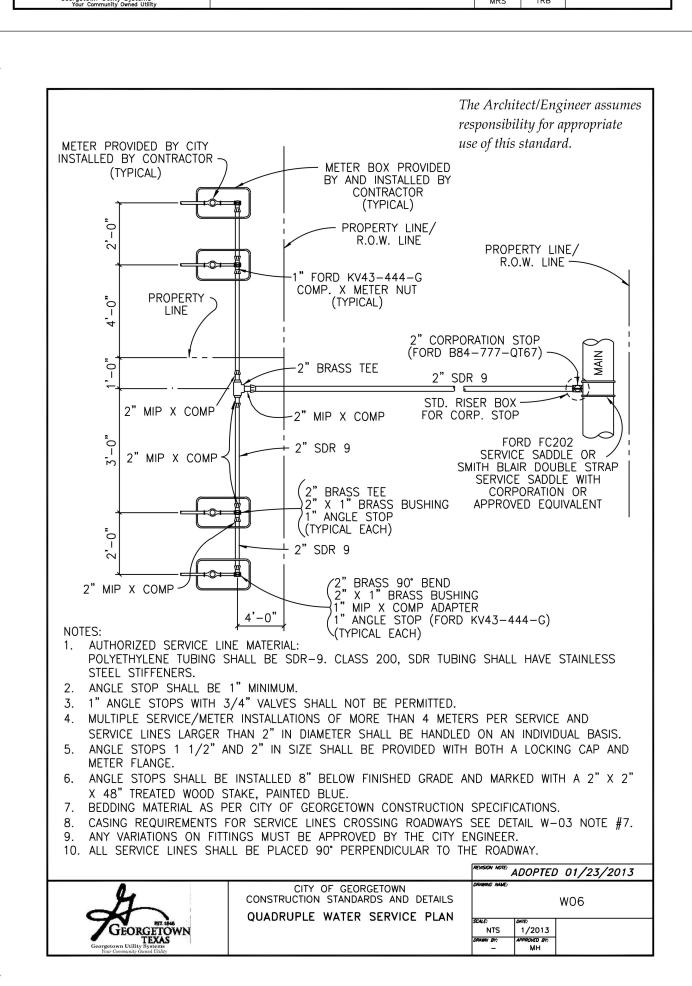


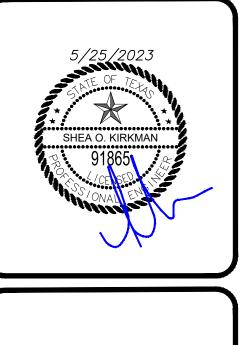












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PROPERTY OWNER LLC

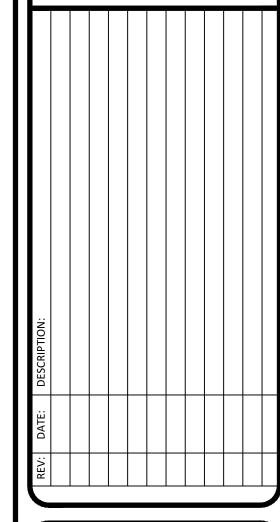
6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

> 2, BLOCK A 2, BLOCK B TY HILL AND HILL ETJ

RANCH

CHALK

LOTS 1 AND 2, BLOCH LOTS 1 AND 2, BLOCH CITY OF LIBERTY HILL LIBERTY HILL ETJ





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

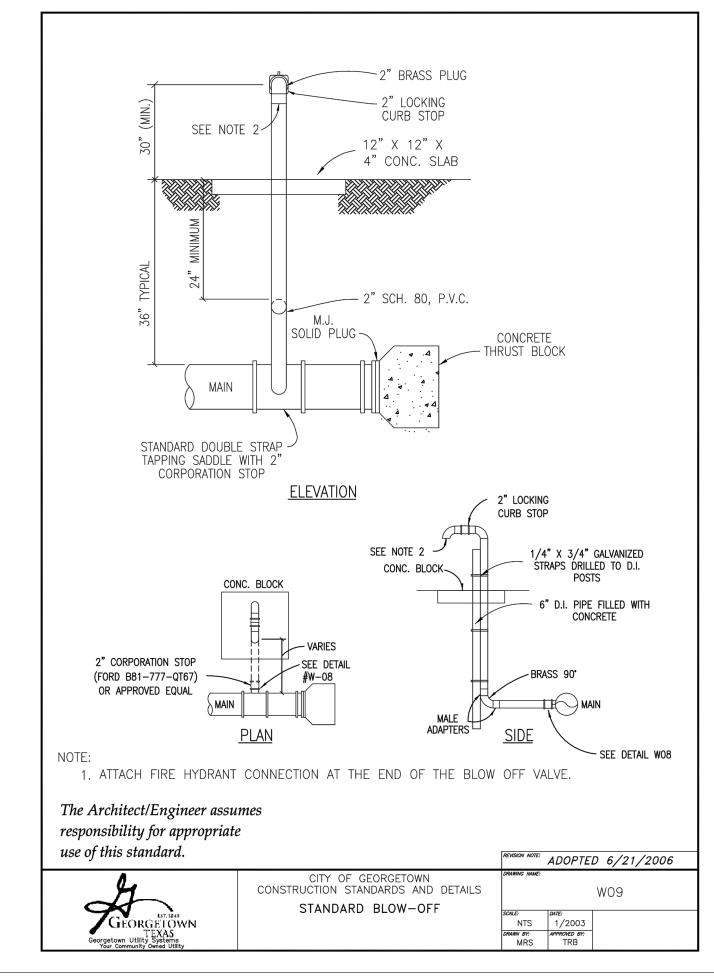
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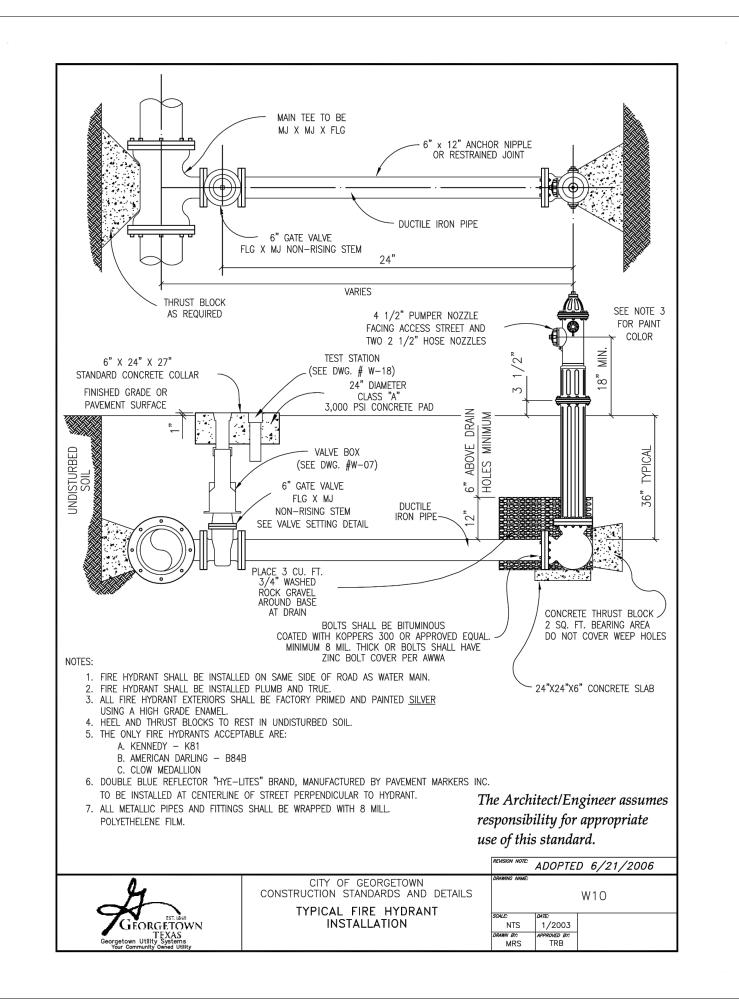
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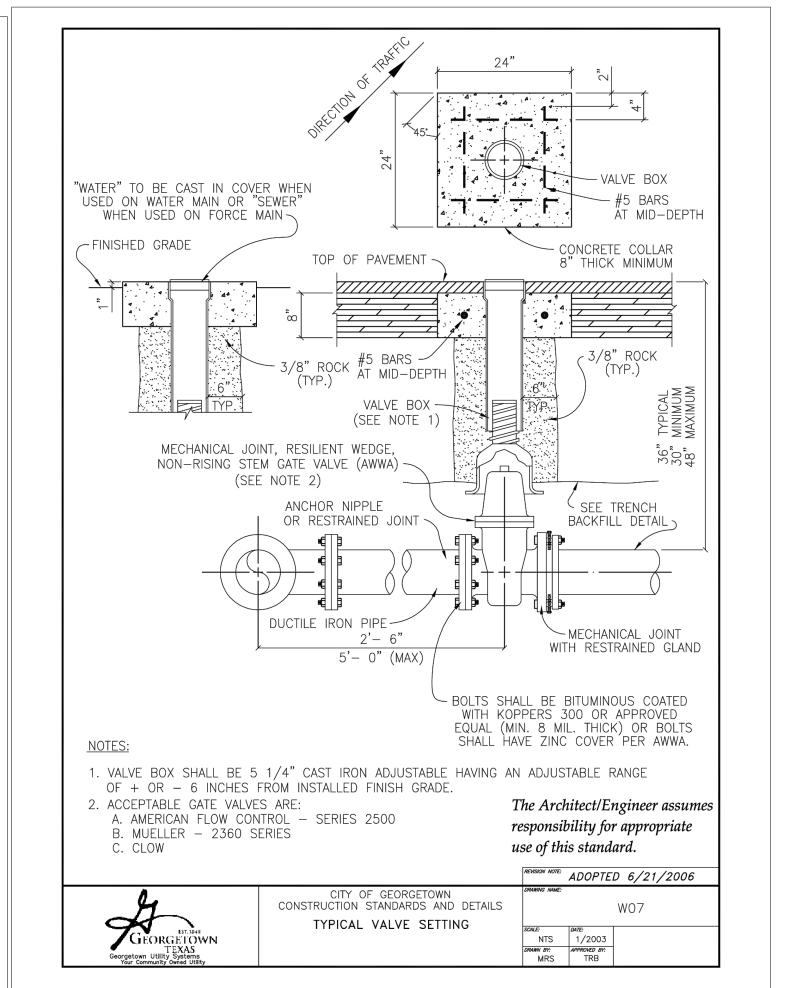
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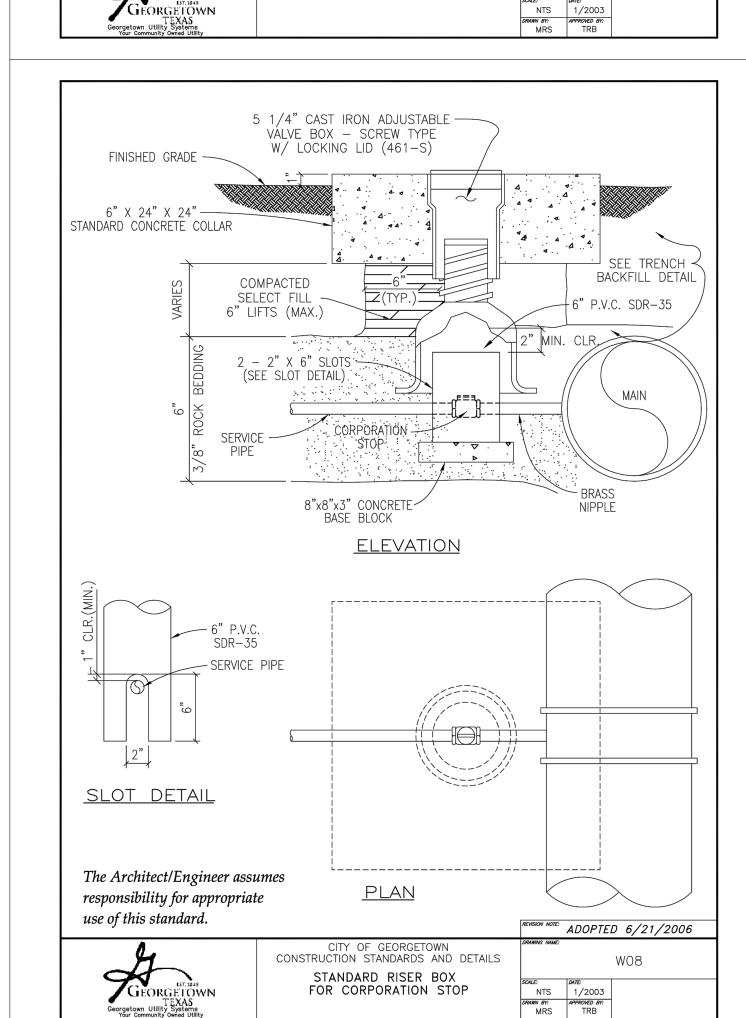
WATER DETAILS I









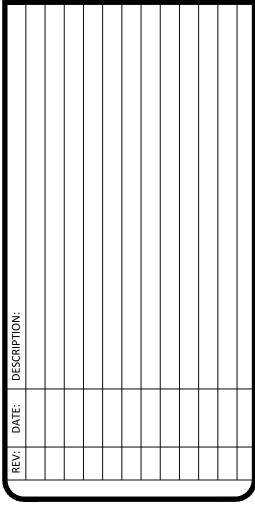




CHV LIBERTY HILL 29 PROPERTY OWNER LLC

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CHALK HILL RANCH
LOTS 1 AND 2, BLOCK A
LOTS 1 AND 2, BLOCK B
CITY OF LIBERTY HILL AND





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

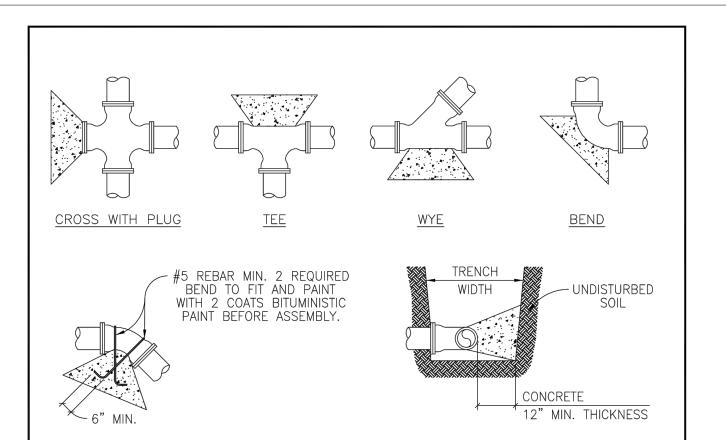
CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

WATER DETAILS II





*DEADMAN

ALL THRUST BLOCKS SHALL BE FORMED. LAID FORMS SHALL BE INSPECTED BY THE CITY OF GEORGETOWN PRIOR TO THE POURING OF CONCRETE AND SHALL ALSO BE INSPECTED BY THE CITY OF GEORGETOWN PRIOR TO COVERING. TYPICAL LOCATIONS WHICH REQUIRE CONCRETE REACTION (THRUST) BLOCKS, FOR PRESSURE MAINS FOUR INCHES (4") AND GREATER. CONCRETE SHALL HAVE 2,500 P.S.I. MINIMUM STRENGTH AT TWENTY EIGHT (28) DAYS AND BEAR AGAINST UNDISTURBED STABLE SOILS, AREA OF CONTACT SHALL BE GOVERNED BY PIPE SIZE, MAXIMUM PRESSURE IN PIPE, AND BEARING CAPACITY OF SOIL. PROTECT FITTINGS, BOLTS, ETC. BY COVERING WITH VISQUEEN OR OTHER ACCEPTABLE MATERIAL. CONCRETE SHALL BE A MINIMUM OF TWELVE INCHES (12") THICK.

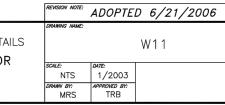
	PIPE SIZE	THRUST BLOCK AREA REQUIRED		THRUST BLOCK AREA REQUIRED	REMARKS	
١	4"	2.0 SQ. FT.	18"	30.0 SQ. FT.		
١	6"	4.0 SQ. FT.	20"	37.0 SQ. FT.	VALUES ARE FOR 90° BENDS, BASED ON	
1	8"	6.6 SQ. FT.	24"	53.0 SQ. FT.	2000 P.S.F. SAFE BEARING LOAD AND PIPE PRESSURE OF 150 P.S.I. PLUS 33% SAFETY	
1	10"	10.0 SQ. FT.	27"	80.0 SQ. FT.	FACTOR FOR OTHER SOILS AND PRESSURES,	
1	12"	14.0 SQ. FT.	30"	98.0 SQ. FT.	THE AREA REQUIRED IS IN DIRECT	
-	14"	18.0 SQ. FT.	36"	127.0 SQ. FT.	PROPORTION.	
-	16"	24.0 SQ. FT.				
	* THE ENGINEER OF RECORD SHALL CALCULATE THE SIZE OF THE DEADMAN REQUIRED AS WELL AS ANY INSTALLATION WHICH IS NOT COVERED BY THE ABOVE.					

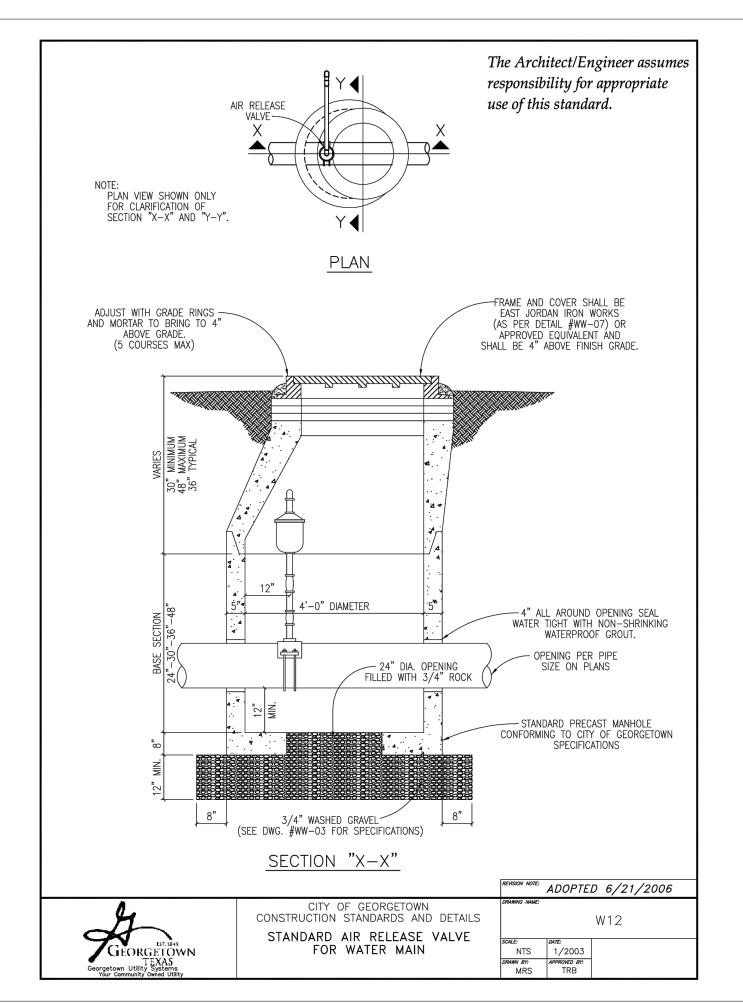
The Architect/Engineer assumes

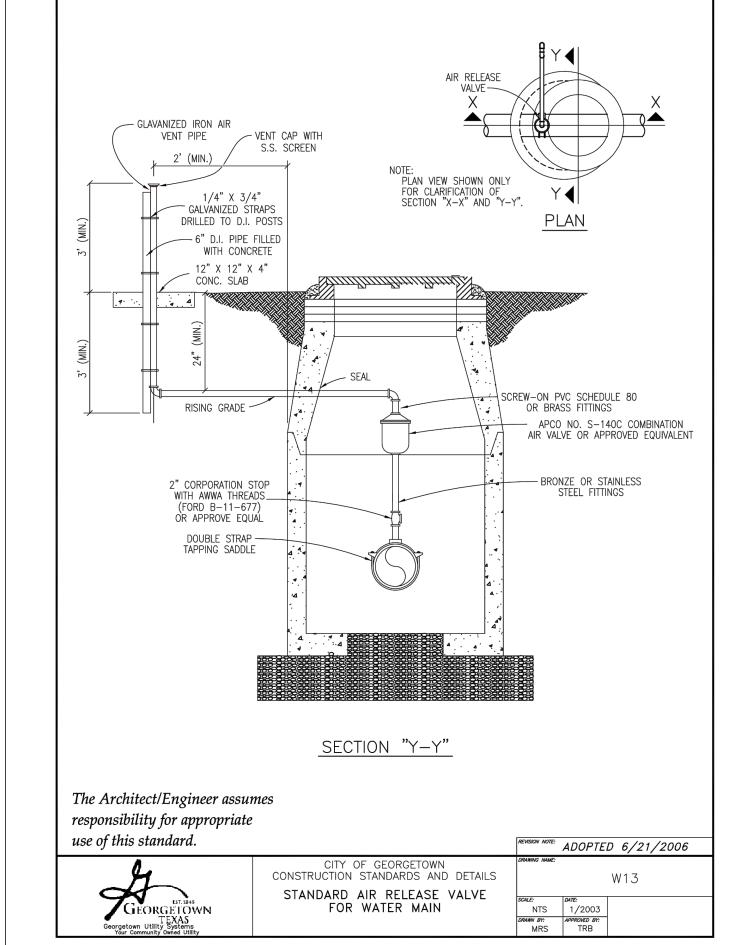
responsibility for appropriate use of this standard.

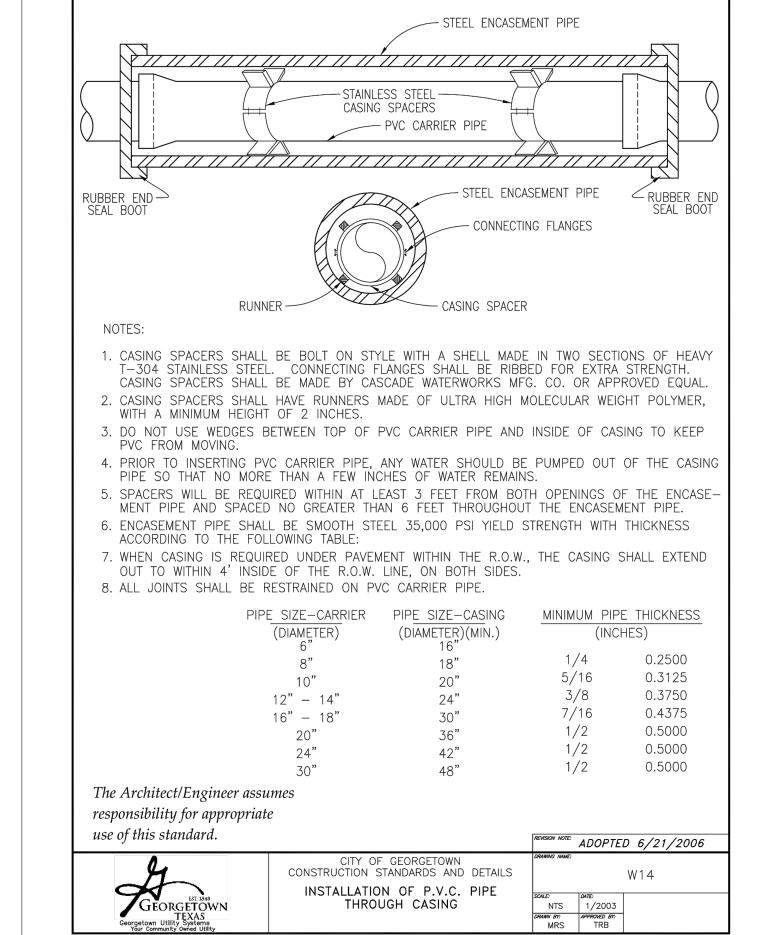
GEORGETOWN

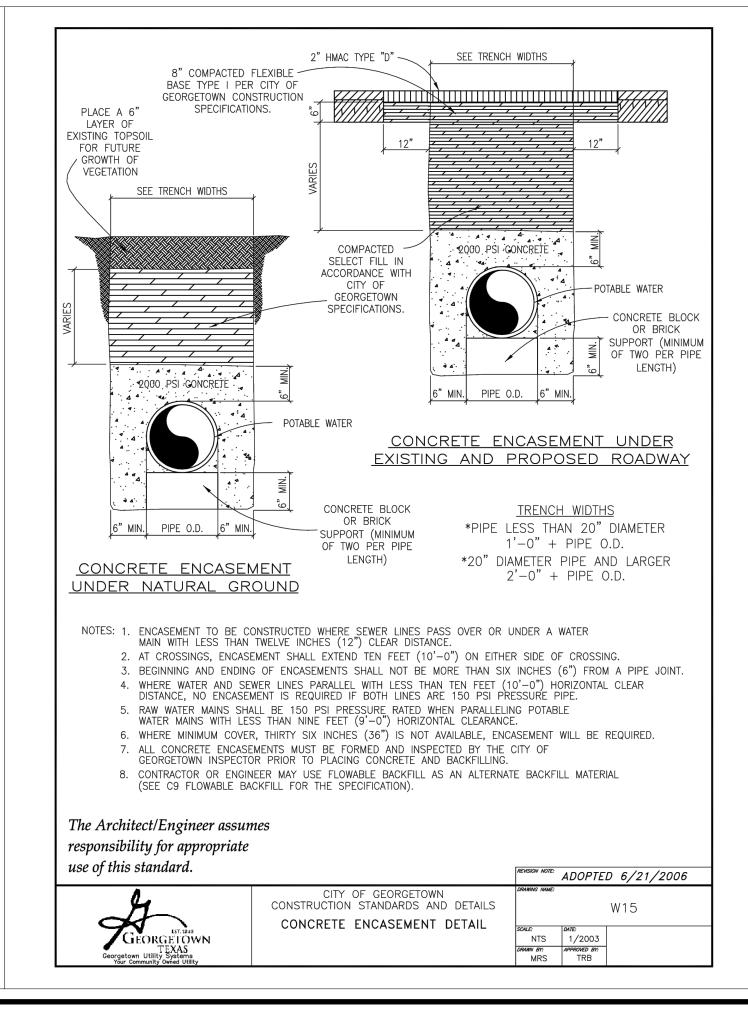
CITY OF GEORGETOWN
CONSTRUCTION STANDARDS AND DETAILS
TYPICAL THRUST BLOCKS FOR
WATER AND FORCE MAIN

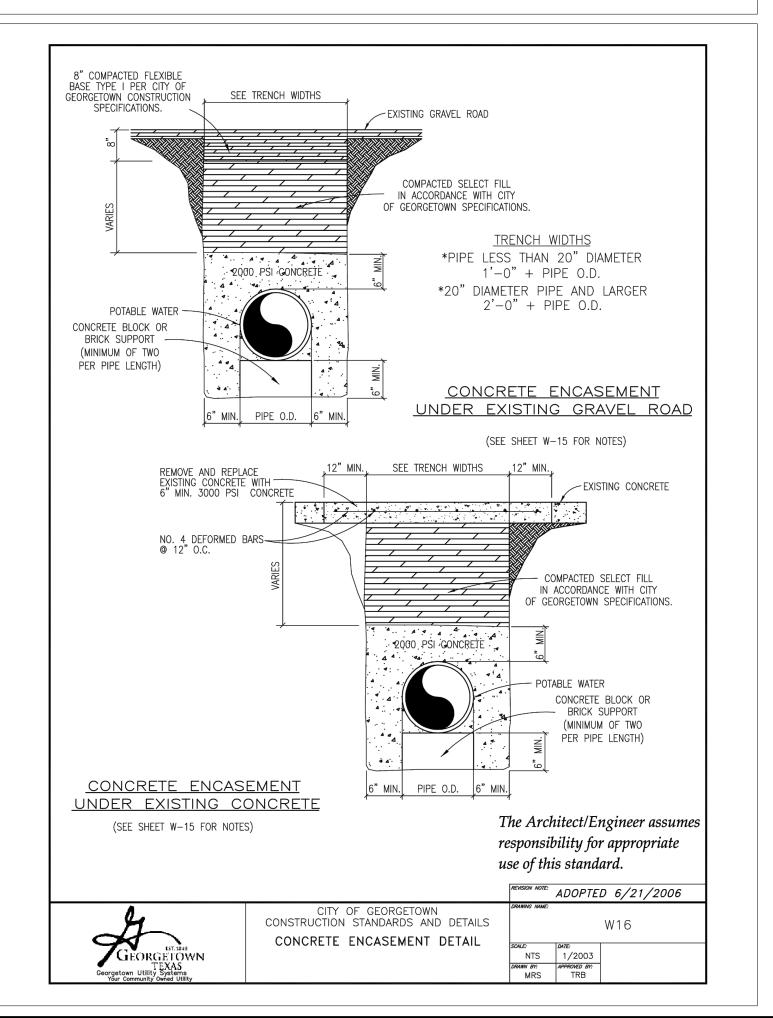














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PROPERTY OWNER LLC

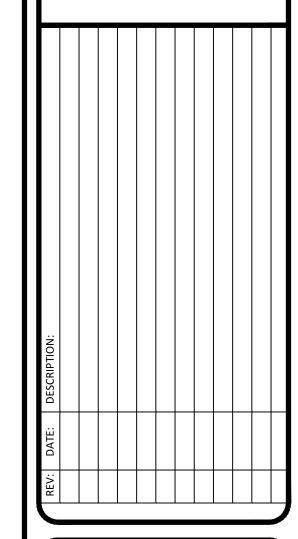
6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

RANCH

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CHALK

LOTS 1 AND 2, BLOCK A
LOTS 1 AND 2, BLOCK B
ITY OF LIBERTY HILL AND
LIBERTY HILL ETJ





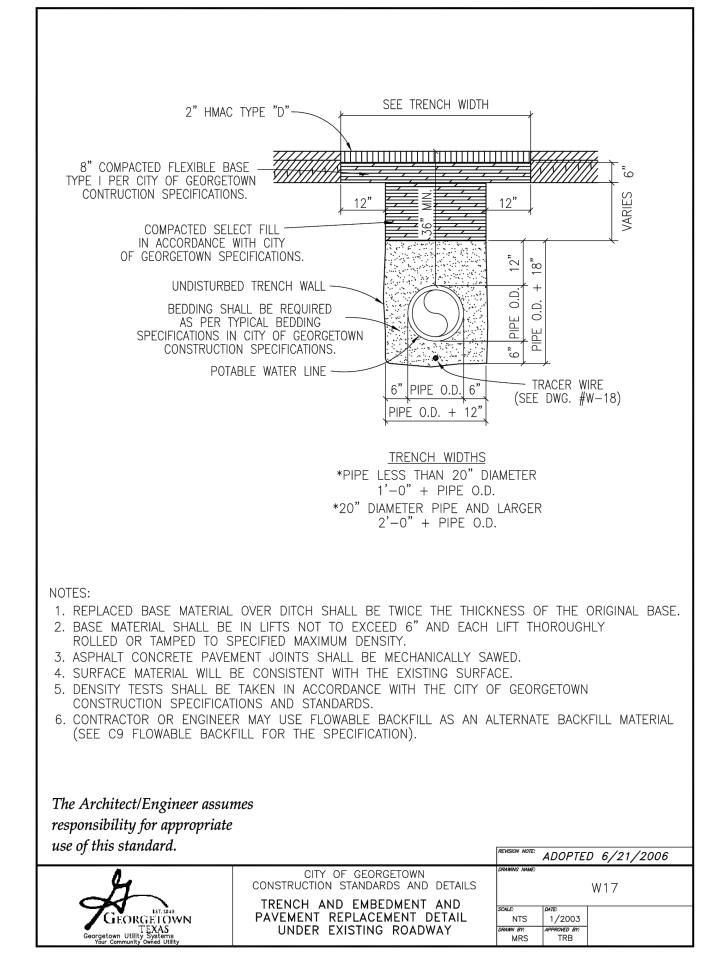
KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK TX 78613

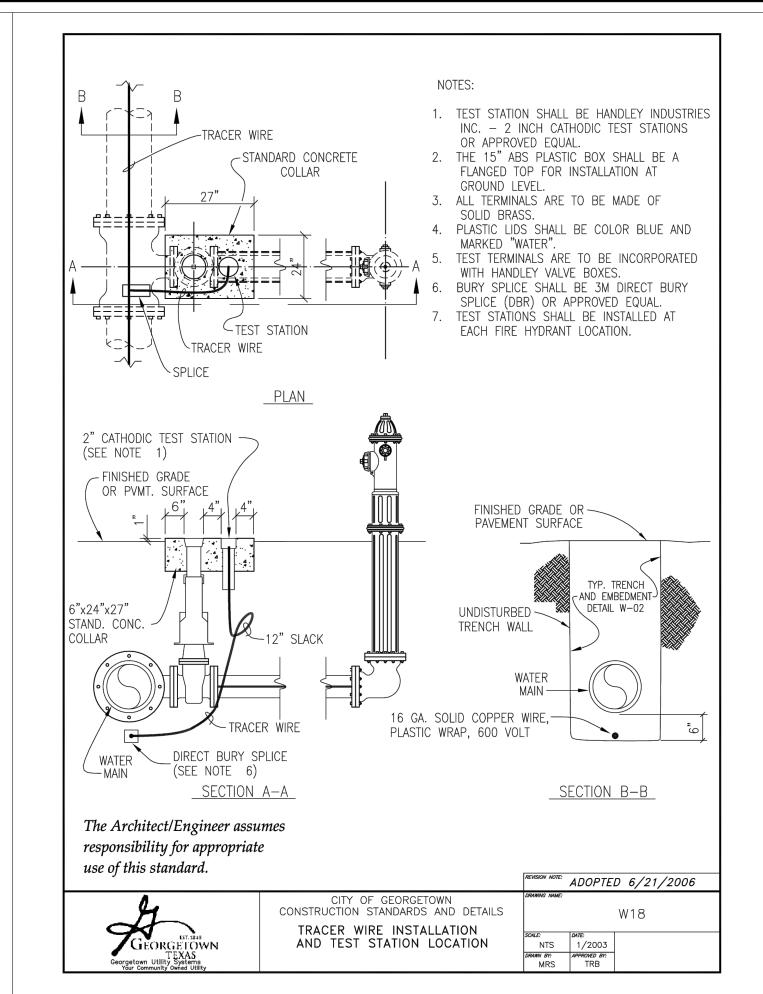
CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

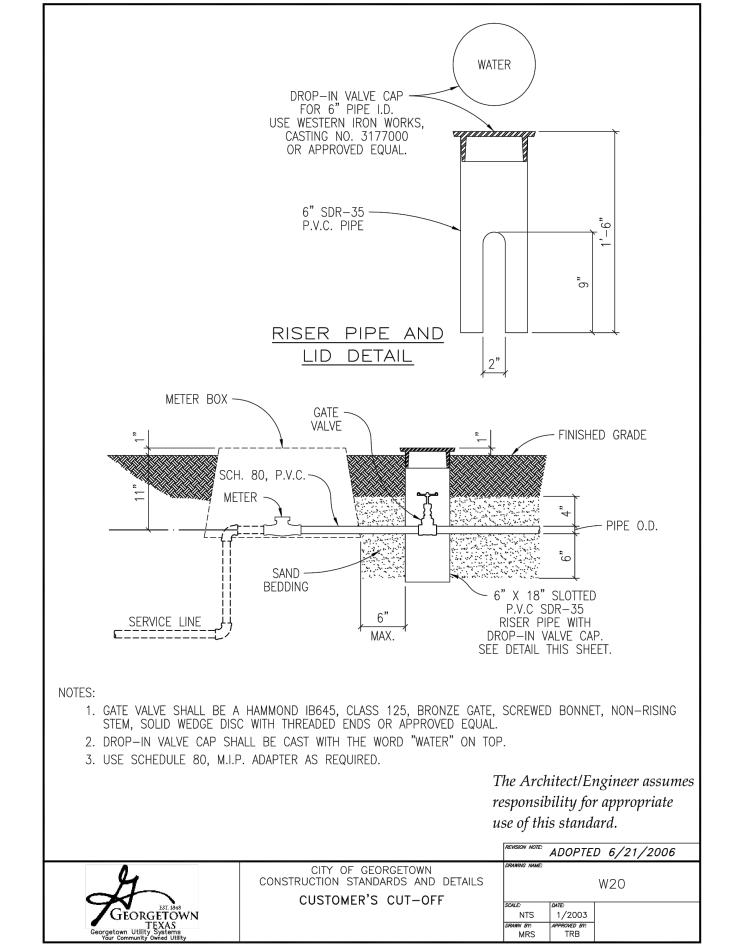
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ISSUE DATE:

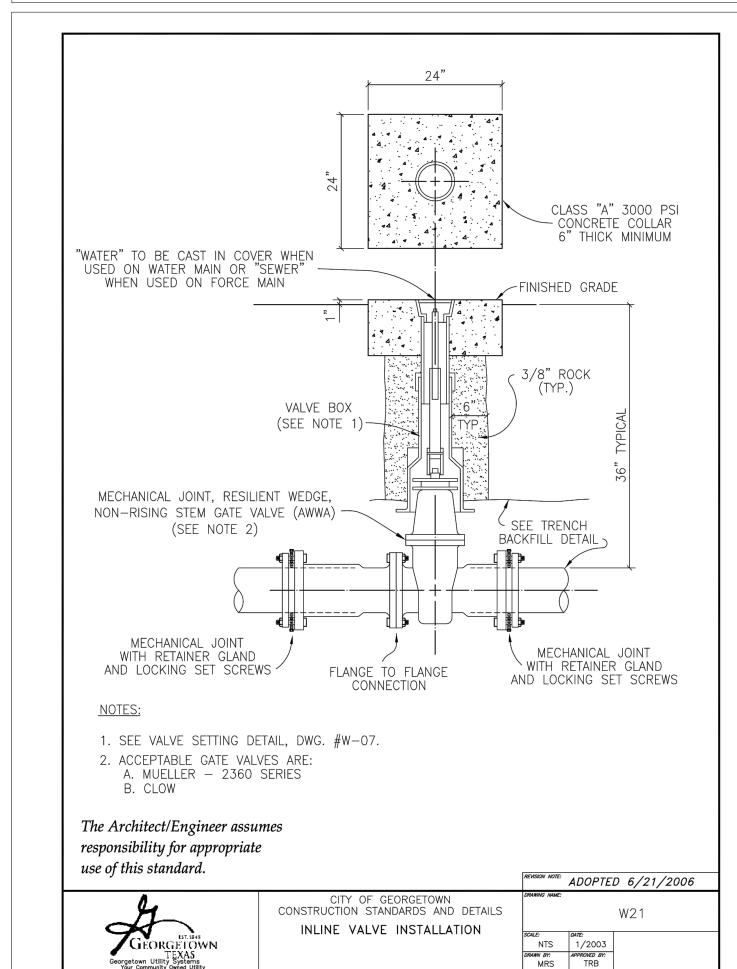
WATER DETAILS III

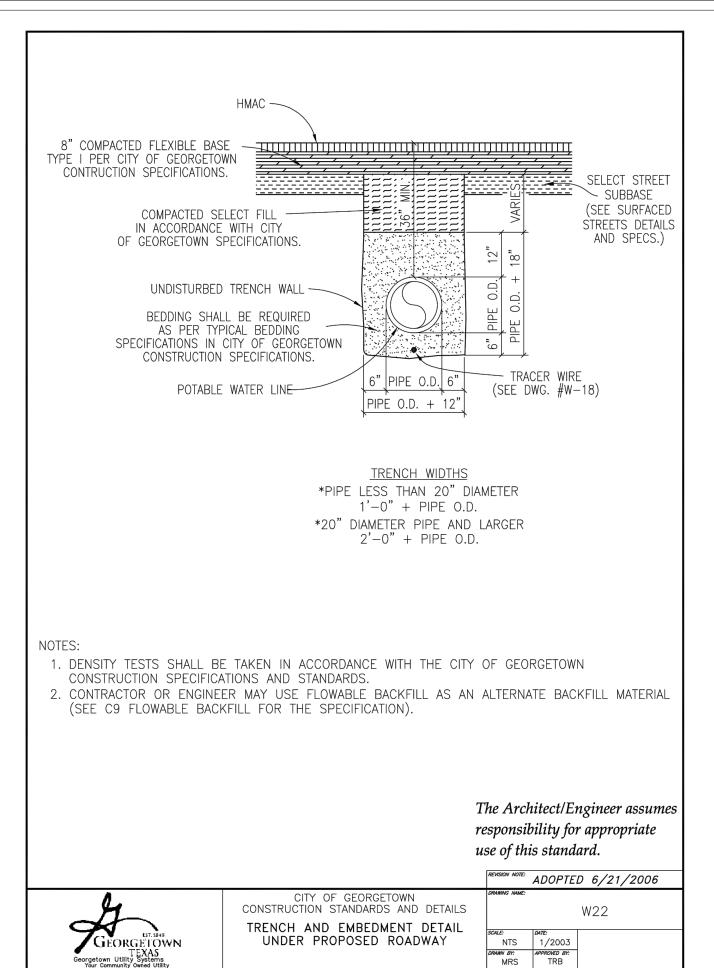


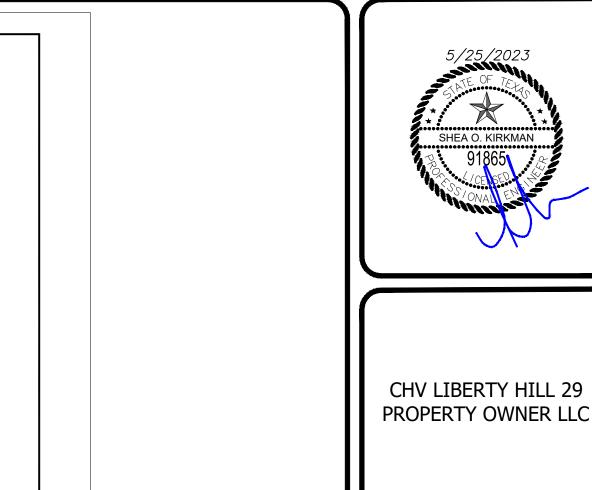






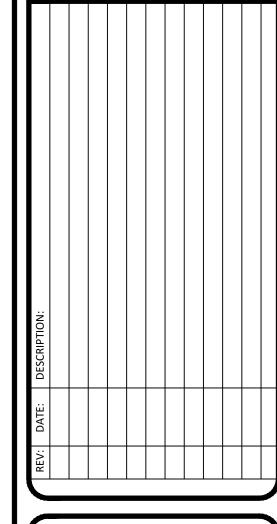






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CHALK HILL RANCH
LOTS 1 AND 2, BLOCK A
LOTS 1 AND 2, BLOCK B
CITY OF LIBERTY HILL AND
LIBERTY HILL ETJ





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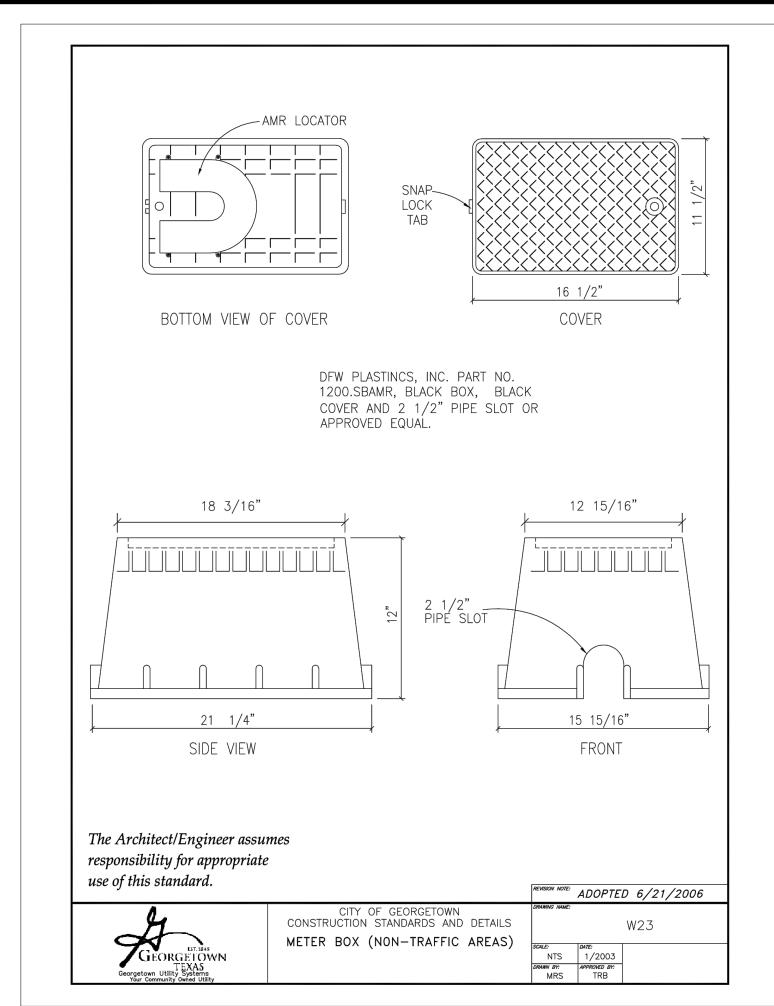
CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

WATER DETAILS IV





TITLE/DESCRIPTION

WT-17 (1 of 9) - INDEX AND GENERAL NOTES WT-17 (2 of 9) - VAULT ARRANGEMENT SCHEMATICS WT-17 (3 of 9) - VAULT SECTIONAL PLAN VIEW

WT-17 (6 of 9) - VAULT SECTIONS "B" AND "C"

WT-17 (9 of 9) - VAULT DIMENSIONS SCHEDULE

BE INCLUDED ON THE CONSTRUCTION PLANS.

A. THE VALVE SIZE AND LAYOUT SCHEME SHALL BE DETERMINED BY THE OWNER; APPROVAL WILL BE BY THE UTILITIES DEPARTMENT. PLANS MUST BE PREPARED, SEALED AND SIGNED BY A

C. THE CONTRACTOR SHALL PROVIDE SUBMITTALS FOR APPROVAL BY THE OWNER FOR ALL VAULT,

D. FIELD VERIFY THE DEPTH OF THE ADJACENT PIPING TO BE CONNECTED TO AND ADJUST THE

ELEVATIONS OF ANY EXISTING UTILITIES, STRUCTURES AND EQUIPMENT WHICH PERTAIN TO

H. THE LOCATION AND ORIENTATION OF THE VAULT SHALL BE APPROVED BY THE OWNER PRIOR

I. DIMENSIONS AND ELEVATIONS SHOWN WITH AN ASTERISK (*) SHALL BE DETERMINED AND/OR

L. ALL SITE AND VAULT PIPING SHALL BE DUCTILE IRON PIPE (D.I.P.) WITH BITUMINOUS COATING

INSTALLED TO A MINIMUM DISTANCE OUTSIDE OF THE VAULT WALL AS SHOWN FOR NOTE NO.

O. THE CONTRACTOR SHALL PROVIDE COUPLINGS, EXPANSION JOINTS AND THRUST RESTRAINTS AS

CITY OF ROUND ROCK | DRAWING NO. WT-17 (1 of 9

PRESSURE REDUCING VALVE

VAULT LAYOUT

INDEX AND GENERAL NOTES

P. ALL VALVES, FITTINGS AND PIPE NOT DESIGNATED OTHERWISE SHALL BE EPOXY LINED AND

K. ALL AREAS DISTURBED DURING CONSTRUCTION SHALL BE REVEGETATED TO A MINIMUM OF

M. ALL BURIED PIPING SHALL BE POLYWRAPPED, AND SHALL ALSO HAVE RESTRAINED JOINTS

N. ALL BURIED PIPING SHALL BE INSTALLED WITH 4-FEET MINIMUM COVER.

SHALL BE NATIONAL SANITATION FOUNDATION (NSF) STANDARD 61 CERTIFIED. Q. CONFINED SPACE ENTRY PLAQUE REQUIRED ON OUTSIDE OF ACCESS HATCH.

G. ANY EXISTING UTILITIES, PAVEMENT, FENCING, CURBS, SIDEWALKS, STRUCTURES, ETC., THAT ARE

E. FIELD VERIFY THE MATERIAL AND PRESSURE CLASS OF THE ADJACENT PIPING TO BE CONNECTED TO AND PROVIDE APPROPRIATE PIPE CONNECTION FITTINGS AS REQUIRED. F. PRIOR TO BEGINNING CONSTRUCTION, THE CONTRACTOR SHALL FIELD VERIFY LOCATIONS AND

DAMAGED OR REMOVED SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR.

VERIFIED AFTER FINAL EQUIPMENT SELECTION AND LOCATION HAVE BEEN MADE. J. VERIFY ALL PIPING DIMENSIONS AND ELEVATIONS FOR EQUIPMENT AND PIPING MATERIALS

WT-17 (7 of 9) - PIPING AND EQUIPMENT SCHEDULE WT-17 (8 of 9) - SPECIFIC EQUIPMENT NOTES

WT-17 (4 of 9) - COVER SLAB PLAN VIEW WT-17 (5 of 9) - VAULT SECTION "A"

ALL NINE (9) PRV DRAWINGS SHALL

REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF TEXAS. B. THE VAULT PIPING LAYOUT SHALL BE SHOWN IN DETAIL WT-17 (2 OF 9).

PIPING, EQUIPMENT AND MATERIALS PRIOR TO ANY CONSTRUCTION.

VERTICAL DIMENSIONS OF THE VAULT AS REQUIRED.

AND/OR AFFECT THE CONSTRUCTION OF THE VAULT.

ACTUALLY FURNISHED FOR THIS PROJECT.

PRE-CONSTRUCTION CONDITIONS.

"29" ON DETAIL WT-17 (7 of 9).

REQUIRED FOR ALL PIPING.

ON FILE AT U&ES DEPARTME

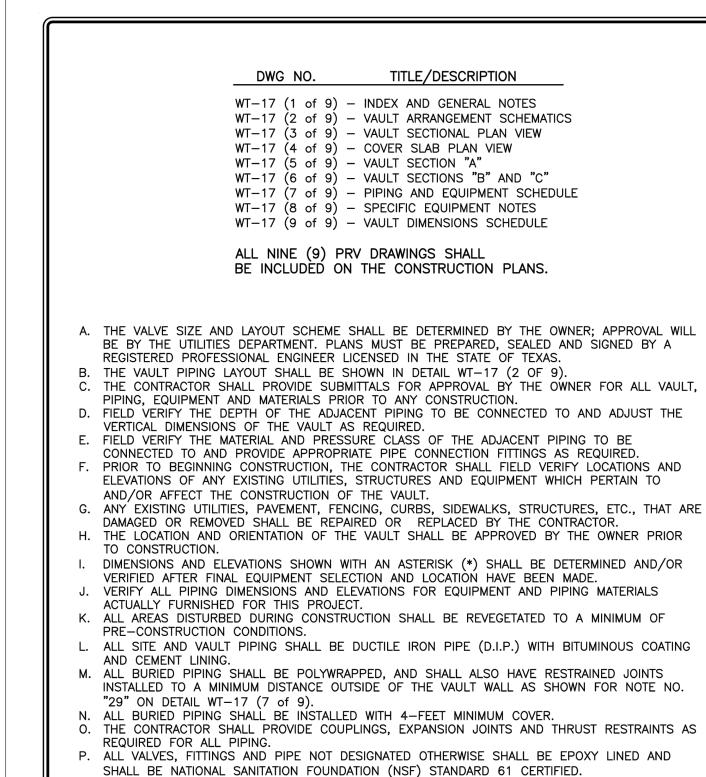
APPROVED

03-01-18

DATE

USE OF THIS DETAIL. (NOT TO SCALE

TO CONSTRUCTION.



Q. CONFINED SPACE ENTRY PLAQUE REQUIRED ON OUTSIDE OF ACCESS HATCH.

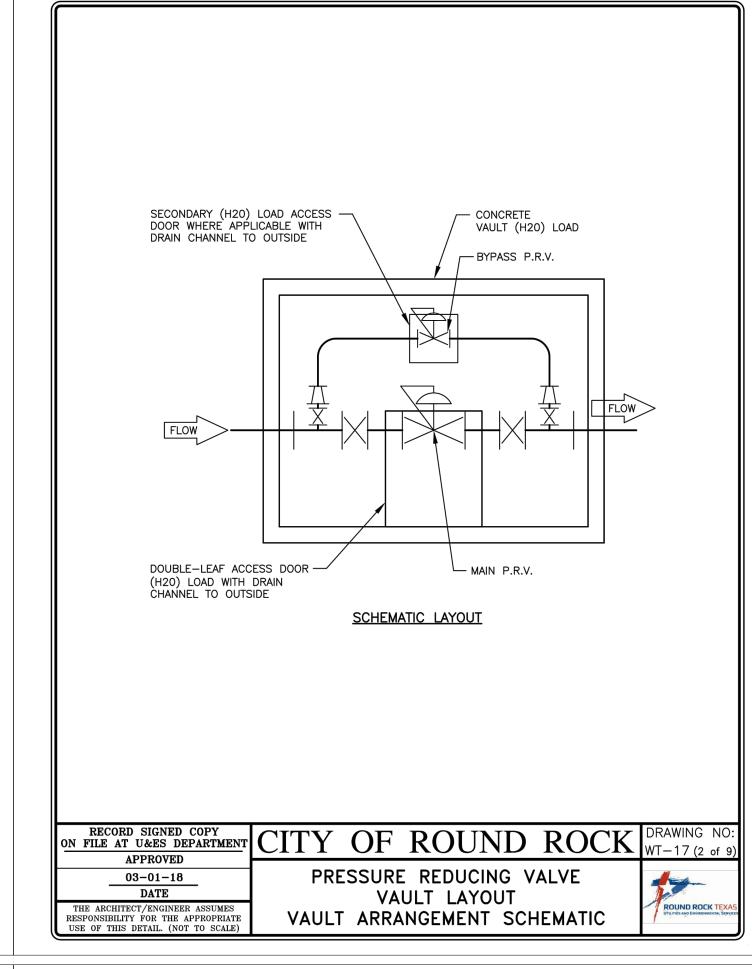
CITY OF ROUND ROCK WT-17 (1 of 9)

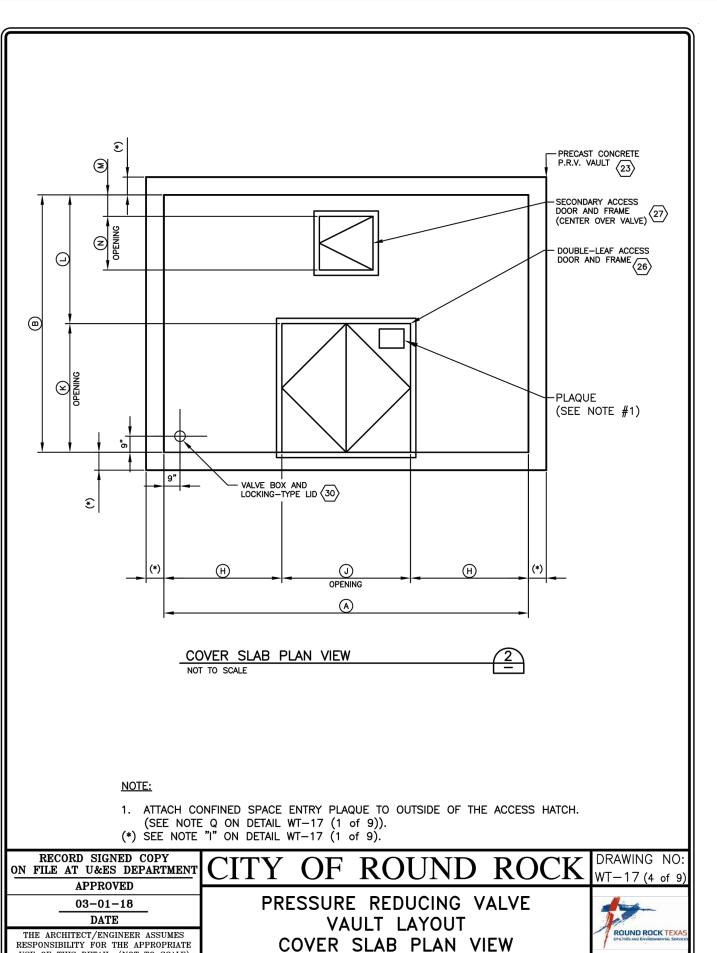
ROUND ROCK

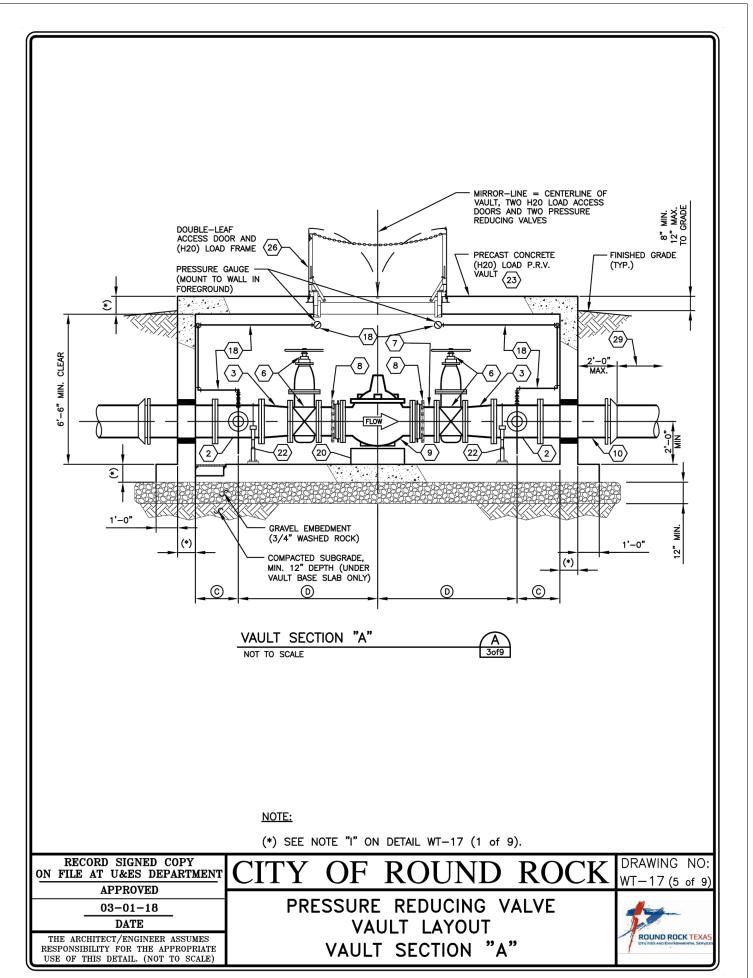
PRESSURE REDUCING VALVE

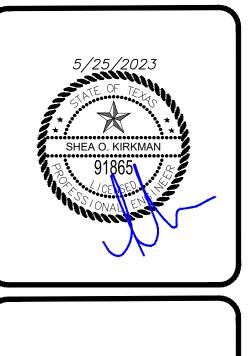
VAULT LAYOUT

INDEX AND GENERAL NOTES





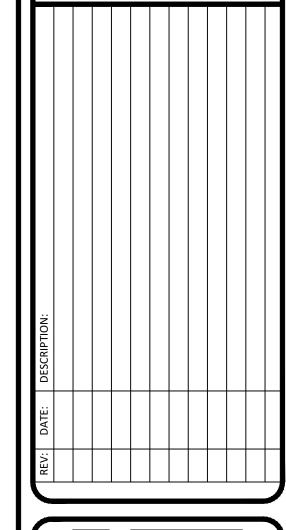




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> RANCH BLOCK A BLOCK B Y HILL AND LL ETJ CHALK



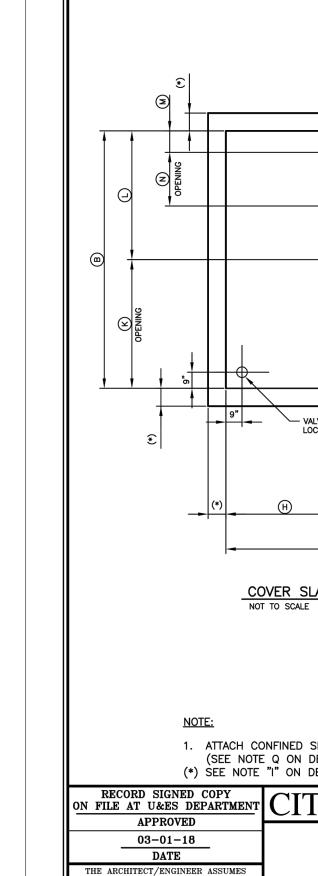


KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

> **WATER DETAILS V**



BILITY FOR THE APPROPRIA

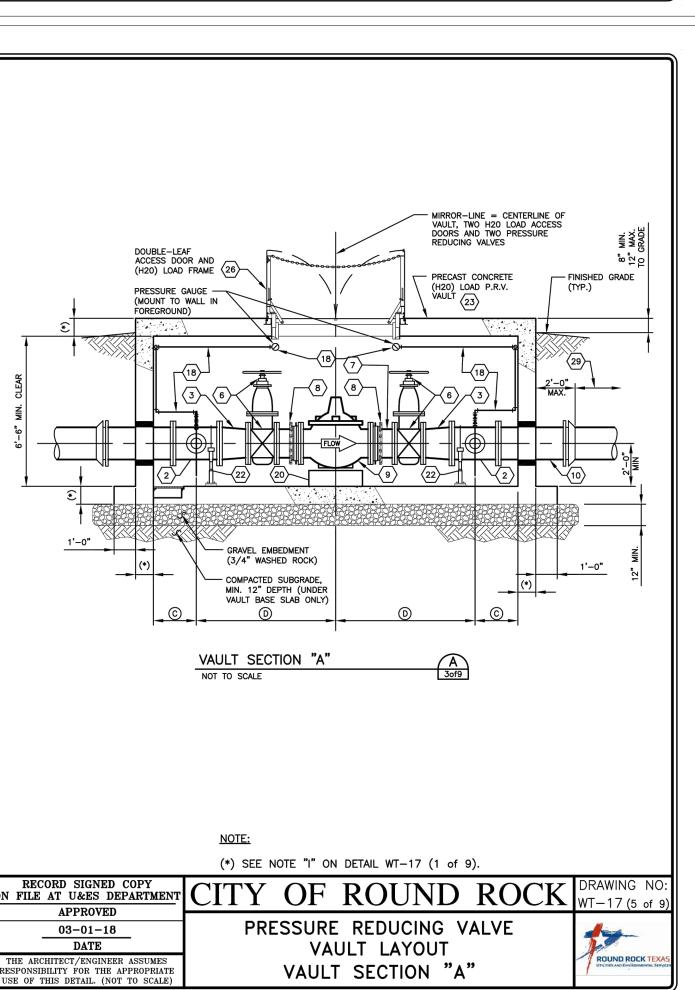
JSE OF THIS DETAIL. (NOT TO SCALE)

ON FILE AT U&ES DEPARTMENT

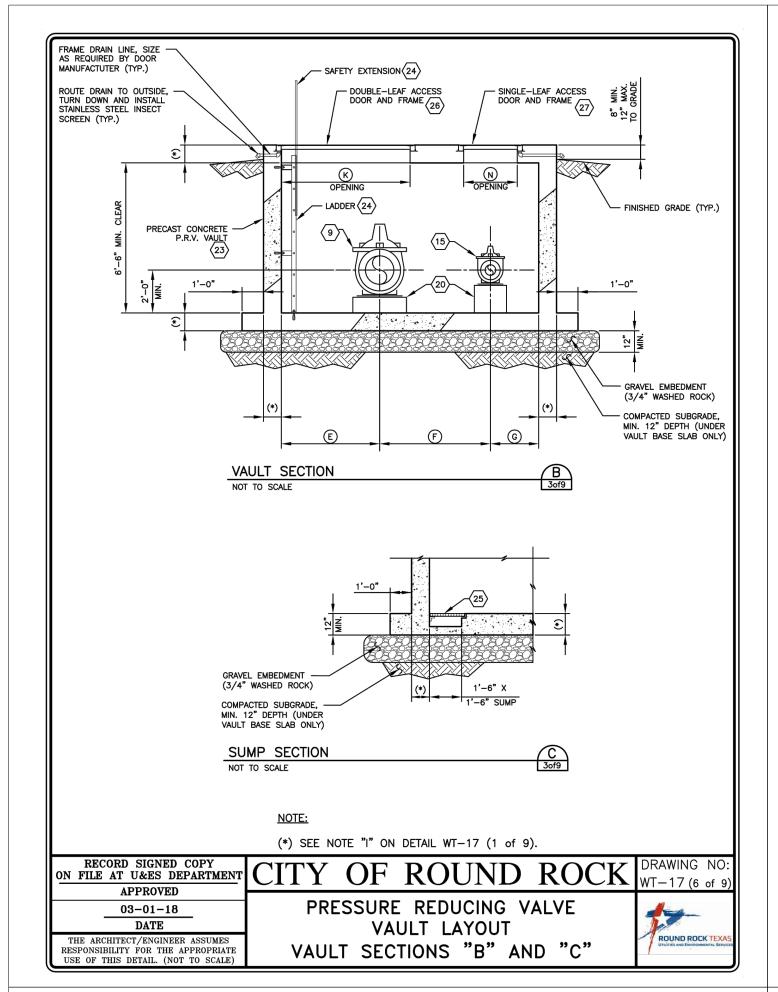
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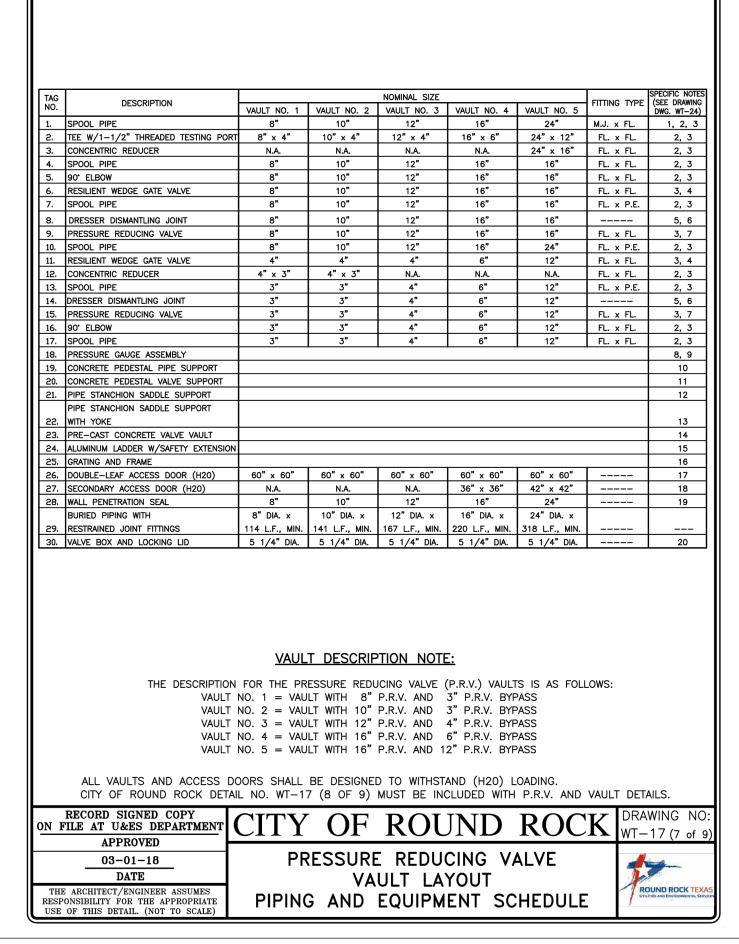
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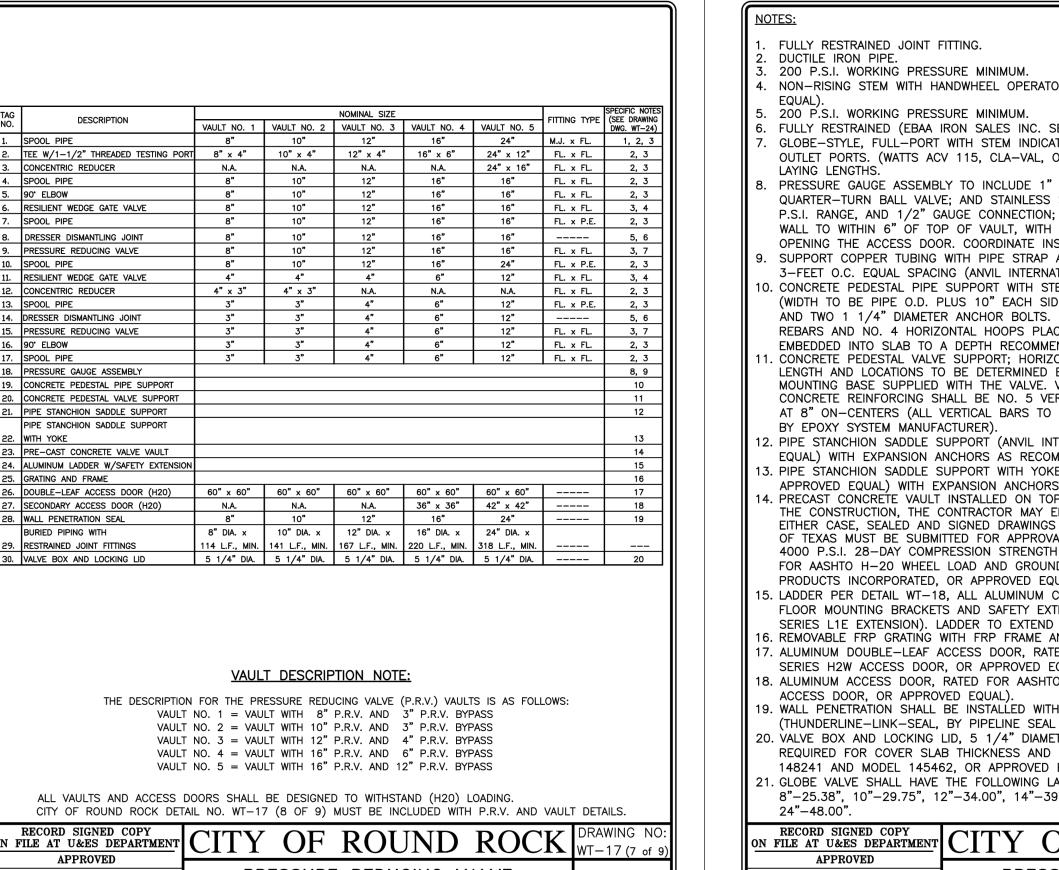
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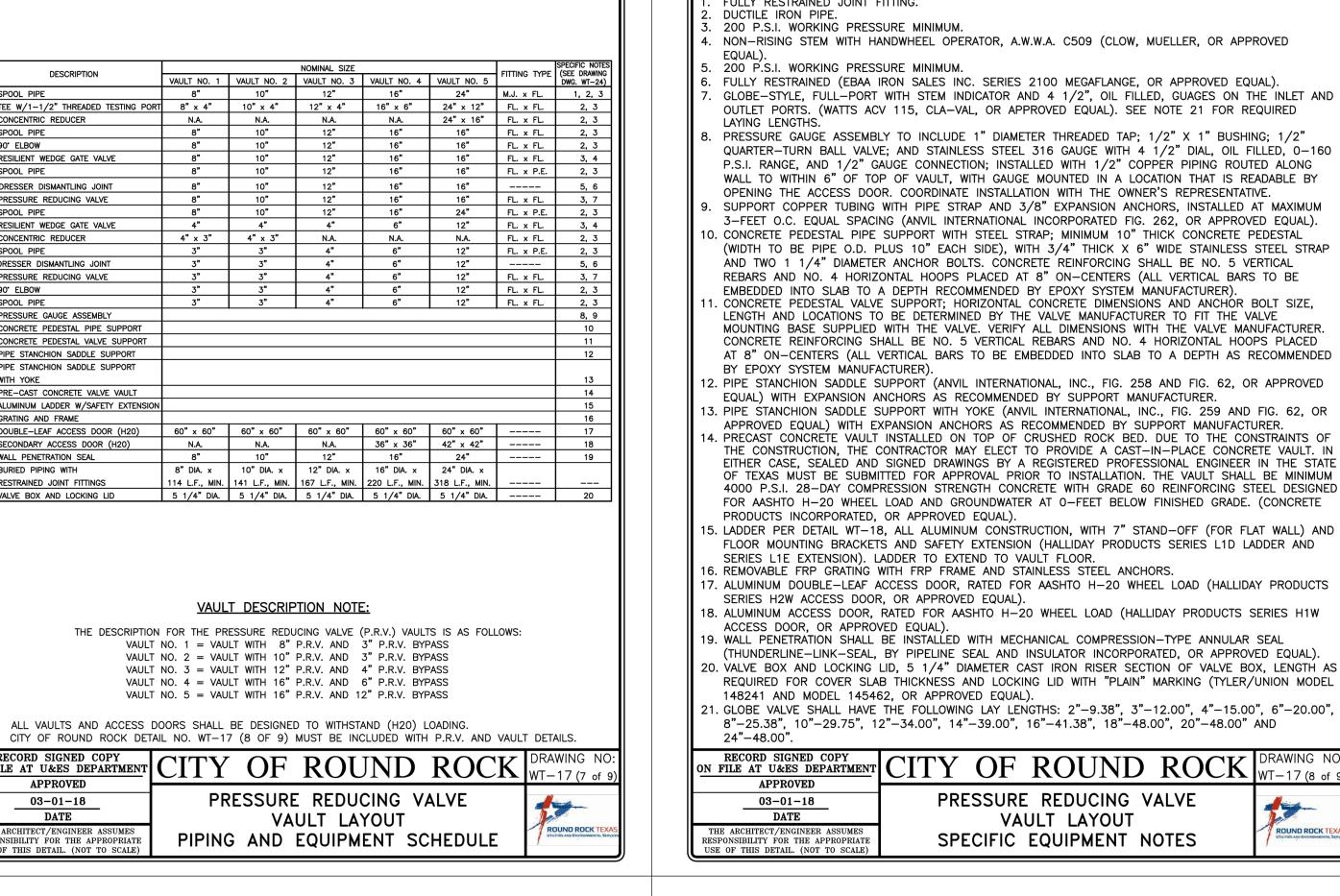


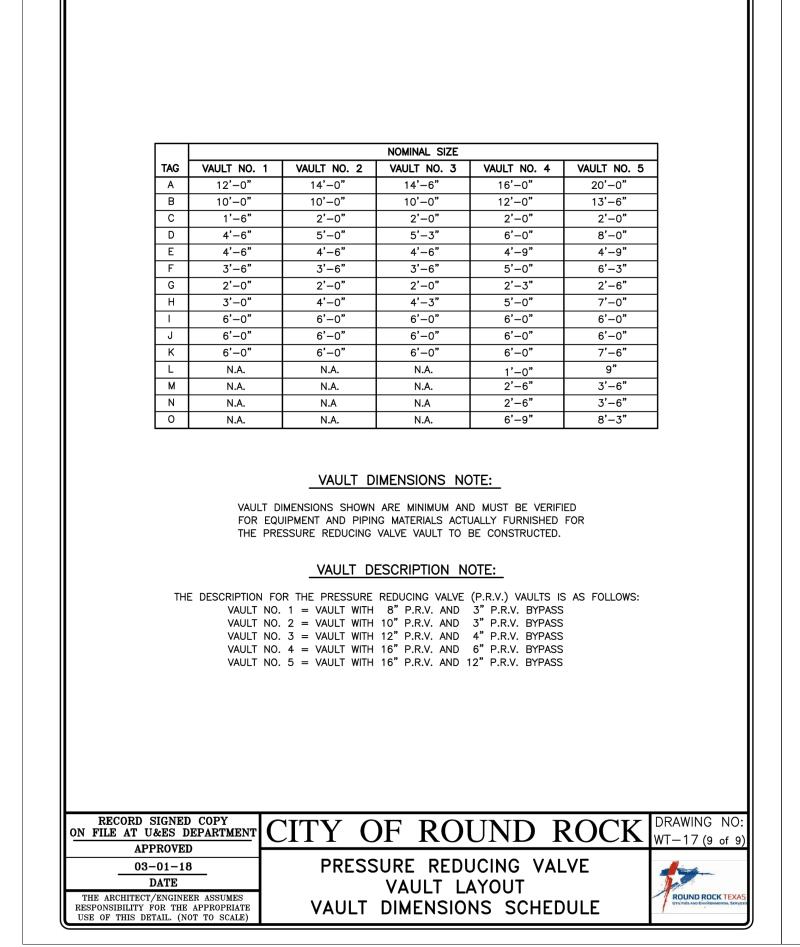


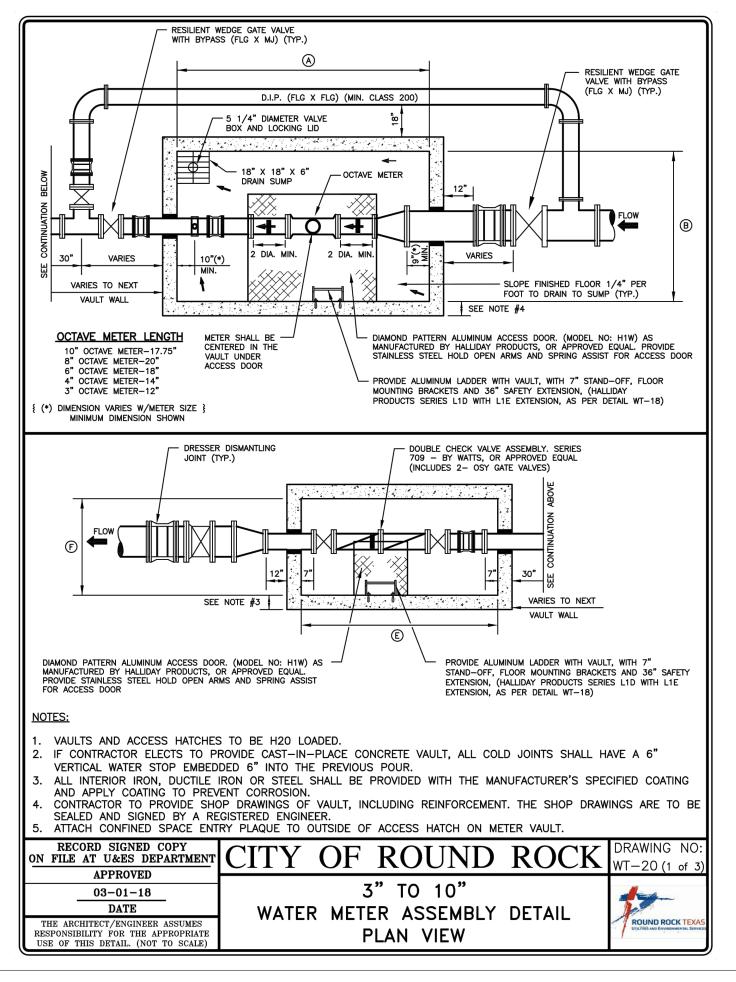


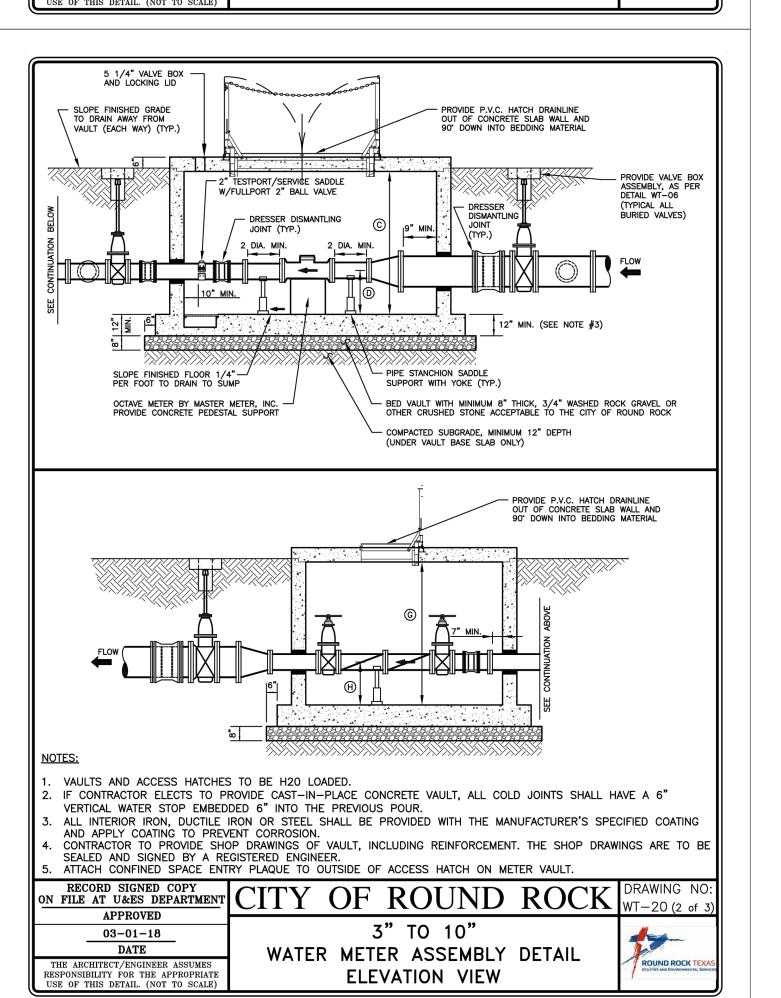














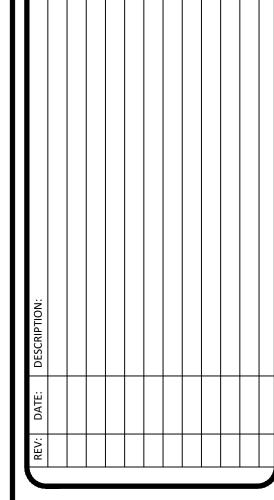
CHV LIBERTY HILL 29 PROPERTY OWNER LLC

6601- E HILLCREST AVE, STE 212 DALLAS, TX 75205 214-435-7510

RANCH

CHALK

BLOCK A BLOCK B Y HILL AND LL ETJ





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

WATER **DETAILS VI**

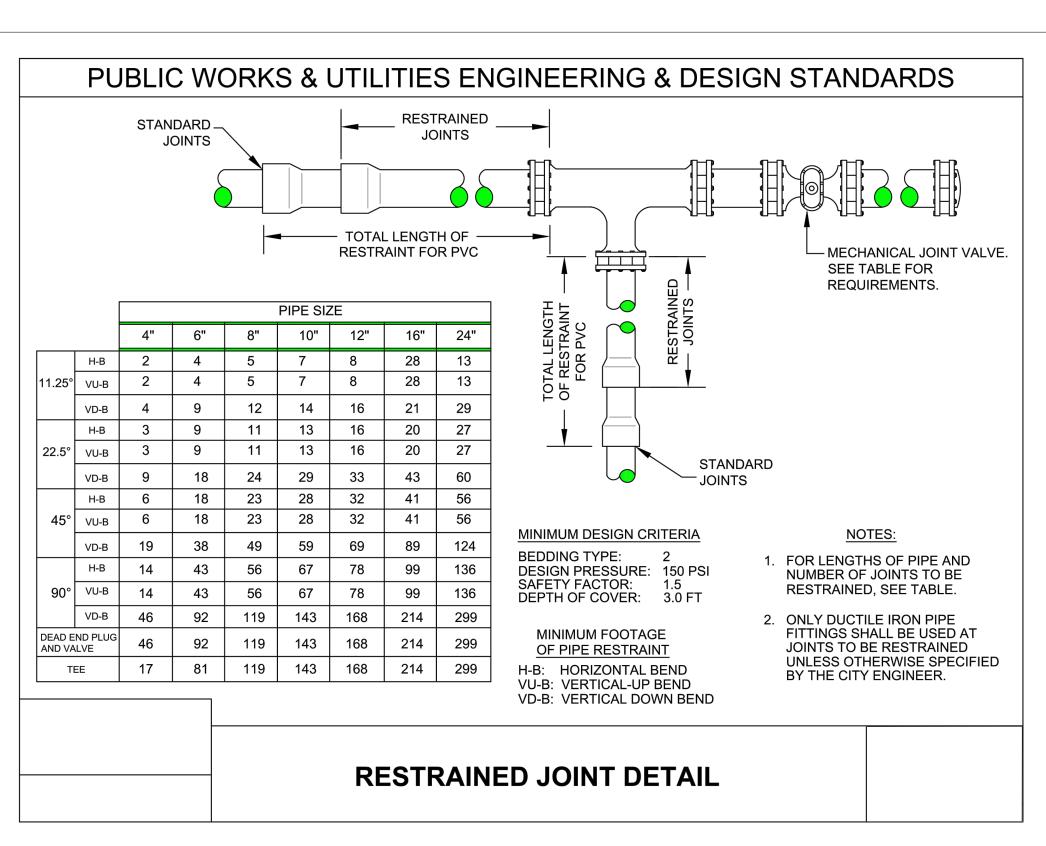
	WATE	R METER	VAULT S	CHEDULE		
TAG NO.	DESCRIPTION	VAULT NO. 1 (3" METER)	VAULT NO. 2 (4" METER)	VAULT NO. 3 (6" METER)	VAULT NO. 4 (8" METER)	VAULT NO. 5 (10" METER)
Α	MINIMUM VAULT LENGTH	6'-0"	6'-0"	8'-0"	9'-0"	12'-0"
В	MINIMUM VAULT WIDTH	4'-0"	4'-0"	6'-0"	6'-0"	6'-0"
С	MINIMUM VAULT HEIGHT	4'-0"	4'-0"	6'-6"	6'-6"	6'-6"
D	MINIMUM PIPE HEIGHT	18"	18"	24"	24"	24"
_	OCTAVE METER SIZE	3"	4"	6"	8"	10"
_	METER LAY LENGTH	12"	14"	18"	20"	17.75"
_	DOUBLE-LEAF ACCESS DOOR	42" × 42"	42" x 42"	42" x 42"	60" x 60"	60" x 60"

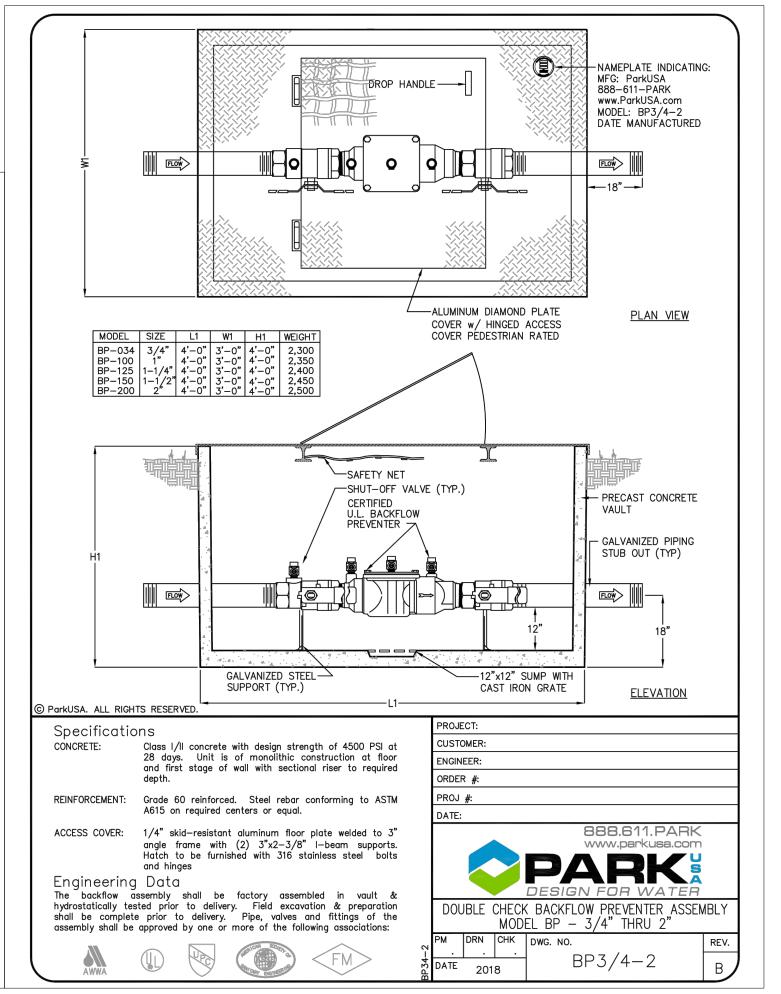
	BACKFLOW PREVENTOR VAULT SCHEDULE					
TAG NO.	DESCRIPTION	VAULT NO. 1 (3" BACKFLOW)	VAULT NO. 2 (4" BACKFLOW)	VAULT NO. 3 (6" BACKFLOW)	VAULT NO. 4 (8" BACKFLOW)	VAULT NO. 5 (10" BACKFLOW)
E	MINIMUM VAULT LENGTH	6'-0"	7'-0"	8'-0"	9'-0"	10'-0"
F	MINIMUM VAULT WIDTH	4'-0"	5'-0"	5'-0"	5'-0"	5'-0"
G	MINIMUM VAULT HEIGHT	4'-0"	4'-0"	6'-6"	6'-6"	6'-6"
н	MINIMUM PIPE HEIGHT	18"	18"	24"	24"	24"
-	BACKFLOW PREVENTOR SIZE	3"	4"	6"	8"	10"
_	DOUBLE-LEAF ACCESS DOOR	42" v 42"				

VAULT DIMENSIONS NOTE:

VAULT DIMENSIONS SHOWN ARE MINIMUM AND MUST BE VERIFIED FOR EQUIPMENT AND PIPING MATERIALS ACTUALLY FURNISHED FOR THE WATER METER AND BACKFLOW VAULT TO BE CONSTRUCTED.

RECORD SIGNED COPY ON FILE AT U&ES DEPARTMENT APPROVED	CITY OF ROUND ROCK	DRAWING NO WT-20 (3 of
03-01-18 DATE	3" TO 10" WATER METER ASSEMBLY DETAIL	1
THE ARCHITECT/ENGINEER ASSUMES RESPONSIBILITY FOR THE APPROPRIATE USE OF THIS DETAIL. (NOT TO SCALE)	VAULT SCHEDULE	ROUND ROCK TE



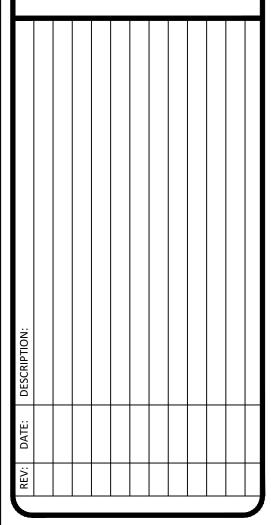




CHV LIBERTY HILL 29 PROPERTY OWNER LLC

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CHALK HILL RANCH
LOTS 1 AND 2, BLOCK A
LOTS 1 AND 2, BLOCK B
CITY OF LIBERTY HILL AND
LIBERTY HILL ETJ
WILLIAMSON COUNTY, TEXAS





KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3

CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

WATER DETAILS VII

C13.6

<u>SECTION W1 – DUCTILE IRON PIPE AND FITTINGS</u> W1.01SCOPE OF WORK This specification covers the requirements to furnish and install ductile iron pipe and ductile iron pipe fittings including bracing, pipe laying, jointing, testing, blocking, and any other work that is required or necessary to complete the installation as shown on the Plans and as specified herein. W1.02 **SUBMITTALS** A. Within 30 days of the Notice to Proceed, the Contractor shall submit to the Engineer for approval, technical product literature including the name of the pipe and fitting suppliers and a list of materials to be furnished, completely detailed working drawings and schedules of all ductile-iron pipe and fittings required, prior to each shipment of pipe, submit certified test reports that the pipe for this Contract was manufactured and tested in accordance with the ASTM and ANSI/AWWA Standards specified herein. REFERENCE STANDARDS A. American Society for Testing and Materials (ASTM) ASTM A307 - Standard Specification for Carbon Steel Bolts and Studs 60,000 PSI Tensile American Water Works Association (AWWA) AWWA C104 - Cement-Mortar Lining for Ductile-Iron Pressure Pipe and Fittings. AWWA C105 - Polyethylene Encasement for Ductile-Iron Piping for Water and Other Liquids. AWWA C110 - Ductile-Iron and Gray-Iron Fittings, 3-inch Through 48-in for Water and Other AWWA C111 - Rubber-Gasket Joints for Ductile-Iron Pressure Pipe and Fittings. AWWA C115 - Standard for Flanged Ductile-Iron Pipe with Threaded Flanges. AWWA C151 - Ductile-Iron Pipe, Centrifugally Cast in Metal Molds or Sand-Lined Molds for Water or Other Liquids. AWWA C153 - Ductile-Iron Compact Fittings, 3-inch Through 16-inch for Water and Other AWWA C600 - Standard for Installation of Ductile-Iron Water Mains and Their Appurtenances. AWWA C651 - Disinfection Water Mains C. <u>American National Standards Institute (ANSI)</u> ANSI B16.1 - Cast Iron Pipe Flanges and Flanged Fittings. Where reference is made to one of the above standards, the revision in effect at the time of bid opening **QUALITY ASSURANCE** RESTRAINED JOINTS Restrained joints shall be installed where shown on the Plans. The joint assemblies shall be made in accordance with the Manufacturer's recommendations. W1.11 SLEEVE TYPE COUPLINGS Couplings shall be installed where shown on the Plans. Couplings shall not be assembled until adjoining push-on joints have been assembled. After installation, apply a heavy bitumastic coating to bolts and nuts. W1.12 POLYETHYLENE ENCASEMENT A. The polyethylene encasement shall be installed in accordance with either method specified in AWWA W1.13 CONCRETE AND BLOCKING 2,500 psi concrete shall be placed for blocking at each change in direction in the pipeline, in such manner as will substantially brace the pipe against undisturbed trench walls. Concrete blocking, made from Type

A.	All ductile-iron pipe and fittings shall be from a single Manufacturer. All ductile-iron pipe to be installed under this Contract may be inspected at the foundry for compliance with these Specifications by an independent testing laboratory provided by the City. The Contractor shall require the Manufacturer's cooperation in these inspections. The cost of foundry inspection of all pipe approved for this Contract, plus the cost of inspection of disapproved pipe will be borne by the Contractor.
B.	Inspection of the pipe will be made by the Engineer or other representatives of the City after delivery. The pipe shall be subject to rejection at any time on account of failure to meet any of the Specification requirements, even though pipes may have been accepted as satisfactory at the place of manufacture. Pipe

- rejected after delivery shall be marked for identification and shall immediately be removed from the job.
- Ductile iron pipe shall conform to AWWA C151, be manufactured from metal having a minimum tensile strength of 60,000 psi, a minimum yield strength of 42,000 psi, and a minimum elongation of 10 percent (60–42–10), and be provided in the following minimum pressure classes:
 - 12-inch and smaller, Class 350.
 - 14-inch through 20-inch, Class 250.
 - 24-inch, Class 200.
 - 4. 30-inch and larger, Class 150.
- Ductile iron fittings shall conform to AWWA C110 or C153.
- All pipe and fittings shall have a bituminous outside coating in accordance with AWWA C151 and C110, respectively. All pipe and fittings shall be cement-mortar lined and seal coated in accordance with AWWA C104. Cement mortar lining shall be double thickness.
- Ductile iron pipe with push-on or mechanical joints shall be centrifugally cast pipe in accordance with AWWA C150 and C151.
- Restrained joints shall be restrained push-on joints, TR Flex by U.S. Pipe and Foundry; Lok-Fast by American Cast Iron Pipe Company, or equal. Joints shall be suitable for 250 psi working pressure and be fabricated of heavy section ductile iron casting. Bolts and nuts shall be low carbon steel conforming to
- Sleeve type couplings shall be of steel and shall be Style 38 by Dresser Manufacturing Division, Smith-Blair or equal. Couplings shall be furnished with black steel bolts and nuts and with pipe stop removed. Gaskets shall be of a material suitable for exposure to liquid within the pipe.

POLYETHYLENE ENCASEMENT W1.06

MATERIALS

W1.05

- All buried ductile iron pipe and metallic fittings shall be encased with 8 mil, Type I, Grade E-1, polyethylene film conforming to AWWA C105. Class usage shall be:
 - Class A Natural Color where exposure to weather (including sunlight) is less than 48 hours
 - Class C Black where exposure to weather (including sunlight) may be more than 48 hours.
- Exposure to weather shall be kept to a minimum, and in no case shall it exceed 10 days. The Class of polyethylene used shall be approved by the Engineer.
- Polyethylene encasement shall not be paid for separately, but the cost there of shall be included in he appropriate item of the Proposal and Bid Schedule.

LAYING DUCTILE IRON PIPE AND FITTINGS

W1-2 DUCTILE IRON PIPE AND FITTINGS

- Care shall be taken in loading, transporting and unloading to prevent injury to the pipe, lining or coatings. Pipe or fittings shall not be dropped. All pipe or fittings shall be examined before laying and no piece shall be installed which is found to be defective. Any damage to the pipe linings or coatings shall be repaired as directed by the Engineer. Handling and laying of pipe and fittings shall be in accordance with the Manufacturer's instruction and as specified herein.
- All pipe and fittings shall be thoroughly cleaned before laying, shall be kept clean until they are used in the work, and when laid, shall conform to the lines and grades required. Pipe shall not be laid unless the subgrade is free of water and in a satisfactory condition. Ductile iron pipe and fittings shall be installed in accordance with the requirements of AWWA C600 except as otherwise provided herein. All piping on this project regardless of size or class shall be placed in the embedment as shown on the detail sheets in the Plans. If any defective pipe is discovered after it has been laid, it shall be removed and replaced with a sound pipe in a satisfactory manner by the Contractor, at his own expense.
- All pipe shall be sound and clean before laying. When laying is not in progress, including lunchtime, the open ends of the pipe shall be closed by watertight plugs or other approved means. Good alignment shall be preserved in laying. The deflection at joints shall not exceed that recommended by the Manufacturer. Fittings, in addition to those shown on the Plans, shall be provided, if required, for crossing utilities which may be encountered upon opening the trench. Solid sleeves shall be used only where approved by the Engineer and the City.
- When cutting pipe is required, the cutting shall be done by machine, leaving a smooth cut at right angles to the axis of the pipe. Cut ends of pipe to be joined with a bell shall be beveled to conform to the manufactured spigot end. Cement lining shall be undamaged.
- Joints shall be protected by eight (8) mil. Polyethylene film prior to placing concrete. Concrete shall be placed against undisturbed material, and shall not cover joints, bolts or nuts, or interfere with the removal of any joint.

PUSH-ON JOINTS

Push-on joints shall be made in accordance with AWWA C111 and the Manufacturer's instructions. Pipe shall be laid with bell ends in the direction of trenching. A rubber gasket shall be inserted in the groove of the bell end of the pipe, and the joint surfaces cleaned and lubricated. The plain end of the pipe to be laid shall then be aligned and inserted in the bell of the pipe to which it is to be joined and pushed home with a jack or by other means. After joining the pipe, a metal feeler shall be used to make certain that the rubber gasket is correctly located.

MECHANICAL JOINTS W1.09

Mechanical joints shall be made in accordance with Appendix "A" of AWWA C111 and the Manufacturer's instructions. Thoroughly clean and lubricate the joint surfaces and rubber gasket with soapy water before assembly. Bolts shall be tightened to the specified torque. Under no conditions shall extension wrenches or pipe over the handle of ordinary ratchet wrench be used to secure greater leverage.

> DUCTILE IRON PIPE AND FITTINGS

PAYMENT

- The pipeline, complete in place, including cleanup, will be measured for payment in linear feet along the centerline of the pipe actually installed. Measurement shall be through all fittings, specials, valves, etc., and no deduction in length shall be made for such appurtenances. Installation of the pipeline will be paid
- Payment of the unit contract price for the items of work performed shall be the total compensation for furnishing all labor, materials, tools, equipment and incidentals and performing all work that is necessary for the installation, testing, and sterilization of the pipe, fittings, connections, blocking, embedment or placing in encasement pipe and all other appurtenances in accordance with the Plans and the provisions of

TECHNICAL SPECIFICATIONS

SECTION W2 - POLYVINYL CHLORIDE (PVC) PIPE-WATER

SCOPE OF WORK

This specification covers the requirements to install polyvinyl chloride (PVC) water pipe and ductile iron fittings for the water line, including excavation, sheeting, shoring, dewatering, pipe laying, jointing, testing, backfilling and any other work that is required or necessary to complete the installation as shown on the Plans and as specified herein.

W2.02 **SUBMITTALS**

Within 30 days of the Notice to Proceed, the Contractor shall submit to the Engineer or the City for approval, technical product literature including the name of the pipe and fittings suppliers, a list of materials to be furnished, shop drawings and schedules of all PVC pipe and fittings required, prior to each shipment of pipe, submit certified test reports that the pipe for this Contract was Manufactured and tested in accordance with the ASTM Standards specified herein.

QUALITY ASSURANCE W2.03

- A. All PVC pipe and fittings shall be from a single Manufacturer. The supplier shall be responsible for the provisions of all test requirements specified in ASTM D3034 or ASTM F789 and/or ASTM F758 as applicable. In addition, all PVC pipe to be installed under this Contract may be inspected at the plant for compliance with these specifications by an independent testing laboratory provided by the City. The Contractor shall require the Manufacturer's cooperation in these inspections. The cost of plant inspection of all pipe approved for this Contract, plus the cost of inspection of disapproved pipe, will be borne by
- Inspections of the pipe may also be made by the Engineer or other representatives of the City after delivery. The pipe shall be subject to rejection at any time on account of failure to meet any of the Specification requirements, even though sample pipes may have been accepted as satisfactory at the place of manufacture. Pipe rejected after delivery shall be marked for identification and shall be removed from the job at once.

POLYVINYL CHLORIDE (PVC) PIPE AND FITTINGS W2.04

- Polyvinyl chloride pipe for water lines, unless otherwise specifically shown on the Plans, or approved in writing, shall be AWWA C900, C905, or C909 Class 150 psi with a dimension ratio of 18 (DR-18), for water lines and shall be extruded, be of rubber gasket type, and be furnished in 20-foot nominal laying lengths. All such pipe shall bear a mark denoting approval by the Underwriters' Laboratories of Chicago, Illinois, so that it will be acceptable to the Texas State Fire Insurance Commission for use in fire protection lines without penalty. All joints shall be of the type which provides a recession in the bell for the employment of a single rubber gasket to be placed before the insertion of the succeeding spigot. Each size of polyvinyl chloride pipe shall have the same outside diameter as the corresponding size of castiron
- Fittings shall be ductile iron, mechanical joint or flanged type and shall be Class 250 in accordance with AWWA Specifications C110-77, C-111-80, and C115-75. Flanges shall be faced and drilled in accordance with ASA Standard B16.1, Class 125 unless otherwise shown on the Plans or in the Special Conditions. All fittings shall be tar coated on the outside surface and shall have an interior cement lining with seal coat per AWWA Specifications C104-80 unless otherwise shown or specified.
- The Contractor shall obtain installation instructions, including support spacing and solvent welding, from the supplying Manufacturer, shall comply with the instructions, and shall meet the requirements of ASTM D-2855, Standard Recommended Practice for making Solvent Cemented Joints with PVC Pipe and Fittings. The PVC solvent cement shall comply with ASTM D-2564 and shall be furnished by the pipe and fitting Manufacturer for the class and type of pipe supplied to the project.



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KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004 ISSUE DATE:

> **WATER NOTES I**

construction period. If obstructions remain after this cleaning, the obstructions shall be removed. **CONNECTIONS AND APPURTENANCES**

may be made in two (2) days after completion of blocking if Type III cement is used.

I cement, shall have been in place four (4) days prior to testing the pipeline as hereinafter specified. Test

At all points where wet connections are made to existing lines, the existing lines shall be adequately

C. Concrete blocking will not be measured or paid for as a separate item but the cost thereof shall be

A. At the conclusion of the work thoroughly clean all of the new pipelines by flushing with water or other

blocked and the tapping connection fittings shall be supported by blocking up to the spring line with 2,500

means to remove all dirt, stones, pieces of wood or other material which may have entered during the

case, when the work is started, it shall be prosecuted expeditiously and continuously until completed.

A. The Contractor shall make the alterations and the necessary connections to existing water mains as shown on the Plans. Such connection shall be made at such time and in a manner approved by the City; in each

included in the proper items listed in the Proposal and Bid Schedule.

TECHNICAL SPECIFICATIONS

Fittings, bends, plugs and valves and shall be of standard manufacture and mechanical joint type to fit AWWA pipe specifications in Classes A, B, C and D, unless otherwise shown on the Plans.

Payment for fittings and gate valves shall be restrained and shall be made separately under the appropriate bid items listed in the Proposal and Bid Schedule.

LEAKAGE TESTING AND STERILIZATION

CLEANING

W1.14

W1.15

A. All Ductile Iron Pipe shall be leak tested and sterilized according to Section CIP12 – TESTING OF PIPELINES.

for at the unit contract price per linear foot as provided in the Proposal and Bid Schedule.

the Specifications.

END OF SECTION

DUCTILE IRON PIPE

W1-5

DUCTILE IRON PIPE AND FITTINGS

W2-1

(PVC) PIPE-WATER

AND FITTINGS

DUCTILE IRON PIPE

AND FITTINGS

POLYVINYL CHLORIDE

PAYMENT HANDLING AND CUTTING PIPE pipe until sufficient select material backfill has been placed to ensure that such compaction equipment W2.10 will not have a damaging effect on the pipe. Equipment used in compacting the varying depths of select Pipe and fittings are slightly brittle. Care shall be taken in shipping, handling and laying to avoid material backfill shall be approved by the pipe Manufacturer's representative prior to use. Adjustments of The pipeline, complete in place, including cleanup, will be measured for payment in linear feet along the centerline of the pipe actually installed. Measurement shall be through all fittings, specials, valves, etc., damaging the pipe and fittings. Extra care will be necessary during cold weather construction. the pipe to line and grade shall be made by scraping away or filling in with granular material, and not by wedging or blocking up the bell. and no deduction in length shall be made for such appurtenances. Installation of the pipeline will be paid Any pipe or fitting showing a crack or which has received a blow that may have caused an incipient for at the unit contract price per linear foot as provided in the Proposal and Bid Schedule. Perforated PVC Pipe and fittings shall be installed in accordance with the instructions of the fracture, even though no such fracture can be seen, shall be marked as rejected and removed at once from Manufacturer, ASTM F758 and as specified herein. As soon as the excavation for the trench is complete Payment of the unit contract price for the items of work performed shall be the total compensation for to normal grade of the bottom of the trench, geotextile fabric shall be laid and then the pea gravel bedding furnishing all labor, materials, tools, equipment and incidentals and performing all work that is necessary C. All pipe ends shall be square after cutting. shall be carefully placed (so not to damage the geotextile fabric) and graded to provide uniform and for the installation, testing, and sterilization of the pipe, fittings, connections, blocking, embedment or continuous support for the pipe. Bell holes shall be excavated so that only the barrel of the pipe bears placing in encasement pipe and all other appurtenances in accordance with the Plans and the provisions of While stored, pipe shall be adequately supported from below at not more than three (3) foot intervals to upon the bedding. Before the perforated pipe is laid on the trench, the perforated pipe shall be wrapped the Specifications. prevent deformation. Pipe shall not be stacked higher than six (6) feet. Pipe and fittings shall be stored in around and closed according to the Manufacturer's closure recommendations with the geotextile fabric. The pipe shall be laid accurately to the lines and grades indicated on the Plans. Blocking under the a manner which will keep them at ambient outdoor temperatures and out of direct sunlight. Temporary END OF SECTION shading as required to meet this requirement shall be provided. Simple covering of the pipe and fittings perforated PVC pipe will not be permitted. Pea gravel shall be placed evenly on each side of the pipe to mid-diameter and hand tools shall be used to gently place the pea gravel under the haunches of the pipe which allows temperature buildup when exposed to direct sunlight will not be permitted. and into the bell holes to give firm continuous support for the pipe. Making sure not to damage the W2.06 JOINTING POLYVINYL CHLORIDE (PVC) PIPE AND FITTINGS geotextile fabric, pea gravel shall then be carefully placed above the top of the perforated pipe varying from two to three (2-3) feet depending on the Plans. Once the remaining pea gravel has been placed, PVC pipe and fittings shall be jointed in accordance with the recommendations of the latest ASTM overlap or close the geotextile fabric according to the Manufacturer's recommendations or six (6) inches Standards and detailed instructions of the Manufacturer. minimum overlap. Then one (1) foot of topsoil shall be placed over the pea gravel to the ground level with proper grass sodding on top. INSTALLING POLYVINYL CHLORIDE (PVC) PIPE AND FITTINGS Joints shall not be "pulled" or "cramped". Each joint of pipe shall be completed in compliance with Unless otherwise specified on the Plans, polyvinyl chloride pipe shall be installed to clear all utility lines Manufacturer's recommendations. and shall have three (3) feet minimum cover. For water lines to be constructed under a future roadway, the cover may be increased to allow for future paving grades. The depth of cover, where shown on the Plans, Before any joint is made, the pipe shall be checked to assure that a close joint with the next adjoining pipe is that distance from the top of the pipe to the approximate proposed grade line. has been maintained and that the inverts are matched and conform to the required grade. The pipe shall not be driven down to grade by striking it. No single piece of pipe shall be laid unless it is generally straight. The centerline of the pipe shall not deviate from a straight line drawn between the centers of the openings at the ends of the pipe by more than Precautions shall be taken to prevent flotation of the pipe in the trench. ¹/₁₆-inch per foot of length. If a piece of pipe fails to meet this requirement check for straightness, it shall When moveable trench bracing such as trench boxes, moveable sheeting, shoring or plates are used to be rejected and removed from the site. Laying instructions of the Manufacturer shall be explicitly support the sides of the trench, care shall be taken in placing and moving the boxes or supporting bracing to prevent movement of the pipe, or disturbance of the pipe bedding and backfill or embedment material. Any pipe or fittings discovered to be defective after laying shall be removed and replaced with a sound Trench boxes, moveable sheeting, shoring or plates shall not be allowed to extend below mid-diameter of the pipe. As trench boxes, moveable sheeting, shoring or plates are moved, embedment material shall be placed to fill any voids created and the embedment material and backfill shall be recompacted to provide The Engineer or the City may examine each bell and spigot end to determine whether any preformed joint uniform side support for the pipe. has been damaged prior to installation. Any pipe having defective joint surfaces shall be rejected, marked CONCRETE AND BLOCKING as such, and immediately removed from the job site. 2,500 psi concrete shall be placed for blocking at each change in direction in the pipeline, as shown in the All pipe shall be sound and clean before laying. When laying is not in progress, including lunch time, the Standard Details and in such manner as will substantially brace the pipe against undisturbed trench walls. open ends of the pipe shall be closed by watertight plugs or other approved means. Good alignment shall In no event shall this quantity of concrete blocking be less than those shown in the Plans. Concrete be preserved in laying. blocking, made from Type I cement, shall have been in place four (4) days prior to testing the pipeline as hereinafter specified. Tests may be made in two (2) days after completion of blocking if Type III cement Pipe and fittings shall be installed in accordance with the instructions of the Manufacturer, ASTMD2321 and as specified herein. As soon as the excavation is complete to normal grade of the bottom of the trench, embedment material shall be placed, compacted and graded to provide firm, uniform and At all points where wet connections are made to existing lines, the existing lines shall be adequately continuous support for the pipe. Bell holes shall be excavated so that only the barrel of the pipe bears blocked and the tapping connection fittings shall be supported by blocking up to the spring line with 2,500 upon the bedding. The pipe shall be laid accurately to the lines and grades indicated on the Plans. The specified embedment shall be accurately shaped and trimmed to receive the pipe barrel and each pipe section, when in place, shall have a uniform bearing on the subgrade for the full length of the pipe barrel. Concrete blocking will not be measured or paid for as a separate item but the cost thereof shall be Pipe shall not be laid unless the subgrade is free of water and in a satisfactory condition. Embedment included in the various items listed in the Proposal and Bid Schedule. material shall be placed evenly on each side of the pipe to mid-diameter and hand tools shall be used to force the embedment material under the haunches of the pipe and into the bell holes to give firm TESTING AND ALLOWABLE LEAKAGE continuous support for the pipe. Embedment material shall then be placed to 12-inches above the top of the pipe. Next, the varying depths of select material backfill above the embedment material backfill shall All PVC pipe and fittings shall be leak tested and sterilized according to Section CIP12.05- TEST be placed according to the Plan Details and carefully compacted. Generally, the compaction shall be PROCEDURES FOR PRESSURE PIPELINES. done evenly on each side of the pipe and compaction equipment shall not be operated directly over the POLYVINYL CHLORIDE POLYVINYL CHLORIDE POLYVINYL CHLORIDE W2-2 W2-3 W2-4 (PVC) PIPE-WATER (PVC) PIPE-WATER (PVC) PIPE-WATER TECHNICAL SPECIFICATIONS Where reference is made to one (1) of the above standards, the revision in effect at the time of bid opening Extension shafts shall be steel and the operating nut shall be two (2) inches square. Shafts shall SECTION W3 – VALVES, HYDRANTS AND APPURTENANCES be designed to provide a factor of safety of not less than four (4). Operating nuts shall be pinned shall apply W3.04 DELIVERY, STORAGE AND HANDLING Valve boxes shall be a heavy-pattern cast iron, three (3) piece, telescoping type box with dome W3.01 SCOPE OF WORK Deliver materials to the site to ensure uninterrupted progress of the work. This specification covers the requirements to provide all buried valves, valves in manholes and Barrel length shall be adapted to the depth of cover, with a lap of at least six (6) inches when in the most extended position. Covers shall be cast iron with integrally-cast direction-to-open Protect threads and seats from corrosion and damage. Rising stems and exposed stem valves shall be underground vaults, hydrants and appurtenances complete with actuators and all accessories as shown on coated with a protective oil film which shall be maintained until time of use. arrow and "WATER" shall be cast in the cover when used on a water line or "SEWER" when the Plans and as specified herein. used on a wastewater force main. Aluminum or plastic are not acceptable. A means of lateral support for the valve extension shafts shall be provided in the top portion of the valve box. The W3.02 **SUBMITTALS** C. Provide covers for all openings. valve box lid shall be furnished with a pentagon-head bolt for locking. All valves three (3) inches and larger shall be shipped and stored on site until time of use with Within 30 days of the Notice to Proceed, the Contractor shall submit to the Engineer or the City for The upper section of each box shall have a bottom flange of sufficient bearing area to prevent approval, technical product literature including Manufacturer's literature, illustrations, specifications and wood or plywood covers on each valve end. settling. The bottom of the lower section shall enclose the stuffing box and operating nut of the engineering data which includes dimensions, size, materials of construction, weight, protection coating, and all other pertinent data to illustrate conformance to the specification found within. The Contractor All valves smaller than three (3) inches shall be shipped and stored as above except that heavy valve and shall be oval. card board covers may be furnished instead of wood. shall also submit four (4) copies of all certified shop test results specified herein, complete operation and 4. An approved operating key or wrench shall be provided. maintenance manuals including all copies of all approved shop drawings, and certificates of compliance Store equipment to permit easy access for inspection and identification. Any corrosion in evidence at the where required by referenced standards: For each valve specified to be manufactured and/or installed in time of City acceptance shall be removed, or the valve shall be removed from the job. All fasteners shall be Type 304 stainless steel. accordance with AWWA and other standards, submit an affidavit of compliance with the appropriate standards, including certified results of required tests, and certification of proper installation. E. Store all equipment in covered storage off the ground. RESILIENT SEATED GATE VALVES W3.03 REFERENCE STANDARDS W3.05 Valves shall be manufactured in accordance with AWWA C509. COORDINATION Comply with applicable provisions and recommendations of the following, except as otherwise shown or Acceptable Gate Valves are: Review installation procedures under other Sections and coordinate with the work which is related to this Section including buried piping installation and site utilities. American Flow Control – Series 2500 American Water Works Association (AWWA) Mueller – 2360 Series B. Contractor shall coordinate the location and placement of concrete thrust blocks when required. Clow AWWA C515 - Gate Valves, three (3)-inch through 48-inch NPS, for Water and Sewage W3.06 **GENERAL** Valves shall be provided with a minimum of two (2) O-ring stem seals. A. All valves shall open counter-clockwise. AWWA C502 - Dry-Barrel Fire Hydrants. Bonnet and gland bolts and nuts shall be either fabricated from a low alloy-steel for corrosion resistance or electroplated with zinc or cadmium. The hot-dip process in accordance with ASTM A153 is not The use of a Manufacturer's name and/or model or catalog number is for the purpose of establishing the AWWA C509 - Resilient-Seated Gate Valves, three (3) inch through 12-inch NPS, for Water acceptable. standard of quality and general configuration desired. and Sewage Systems. Wedges shall be totally encapsulated with rubber. Valves shall be of the size shown on the Plans or as noted, and as far as possible equipment of the same C. <u>American National Standards Institute (ANSI)</u> type shall be identical and from one Manufacturer. Units shall be, in addition, UL and FM approved. ANSI B16.1 - Cast-Iron Pipe Flanges and Flanged Fittings Valves shall have the name of the maker, nominal size, flow directional arrows, working pressure for Resilient wedge gate valves shall be furnished and installed in sizes and shall be manufactured in which they are designed and standard to which they are manufactured cast in raised letters on some ANSI C111 - Rubber-Gasket Joints for Ductile-Iron and Gray-Iron Pressure Pipe and Fittings. accordance with the latest AWWA C-509 and cast iron shall conform to the latest ASTM A-126 appropriate part of the body. standards. Gate valves furnished under these specifications shall be of the solid wedge, resilient seat type American Society for Testing and Materials (ASTM) with cast iron/ductile iron body and bronze stem designed for 250 pounds per square inch working Unless otherwise noted, valves shall have a minimum working pressure of 200 psi or be of the same pressure. All gate valves shall be tested hydrostatically to 400 pounds per square inch. Gate valves shall working pressure as the pipe they connect to, whichever is higher, and suitable for the pressures noted 1. ASTM A48 - Gray Iron Castings. meet the latest AWWA standard specifications (C-509). where they are installed. ASTM A126 - Gray Iron Castings for Valves, Flanges and Pipe Fittings The seat shall be made of Styrene Butadiene rubber and provide a positive water tight seal. The seat Valves shall be of the same nominal diameter as the pipe or fittings they are connected to. Except as shall be permanently bonded or mechanically attached to the wedge with stainless steel screws. If ASTM A153 - Zinc Coating (Hot-Dip) on Iron and Steel Hardware otherwise noted, joints shall be mechanical joints, with joint restraint where the adjacent piping is bonded, ASTM P-429 requirements shall be followed. Non-rising stem gate valves shall be equipped required to be restrained. with "O" ring type packing gland consisting of at least two (2) "O" rings. The thrust collar shall work in ASTM A276 - Standard Specification for Stainless and Heat Resisting Steel Bars and Shapes. an "O" ring seal lubricant reservoir or against bearings or washers, above and below constructed of Valves shall be constructed for buried service. Delrin or approved equal material. Gate valve stems, shall be fabricated from solid bronze rod havinga 5. ASTM A536 - Ductile Iron Castings. tensile strength of not less than 60,000 pounds per square inch, and a minimum yield strength of 30,000 W3.07 VALVE BOXES pounds per square inch. E. <u>Steel Structures Painting Council (SSPC)</u> A. All gate valves shall be provided with extension shafts (where the operating nut is greater than five (5) feet below grade), operating nuts and valve boxes as follows: SSPC SP-6 - Commercial Blast Cleaning VALVES, HYDRANTS VALVES, HYDRANTS VALVES, HYDRANTS W3-2 W3-3 W3-1

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KIRKMAN ENGINEERING, LLC 1130 COTTONWOOD CREEK TRAIL SUITE C3 CEDAR PARK, TX 78613

TEXAS FIRM NO. 15874

JOB NUMBER: CHV21004

ISSUE DATE:

AND APPURTENANCES

WATER NOTES II

Cast iron body shall be of iron with an even grain and shall possess a tensile strength of not less than 32,000 pounds per square inch. All bronze castings, except the stem, shall have a tensile strength of not less than 30,000 pounds per square inch. The entire internal valve body surfaces shall be coated with a factory applied two (2) component epoxy system or approved equal. The seating surface shall be machined or otherwise constructed to provide a smooth, even surface for the resilient seat. All valves shall open left (counter clockwise) and have a two (2) inch square wrench nut unless specified otherwise. TAPPING SLEEVES AND TAPPING VALVES

Tapping sleeves shall be of cast iron epoxy coated, designated for working pressure not less than 200 psi. Armored end gaskets shall be provided for the full area of the sleeve flanges. Sleeves shall be as manufactured by A.P. Smith Division of U.S. Pipe, Mueller, Clow, or equal. Nuts and bolts shall be Type 304 stainless steel.

Size-on-Size tapping sleeve shall be ductile iron or cast iron.

Tapping valves shall conform to the requirements specified above for gate valves except that one (1) end shall be flanged and one (1) mechanical. Tapping valves shall be provided with an oversized opening to permit the use of full size cutters. Tapping valves shall be Ford B81-777 or equal.

W3.10 CHECK VALVES

A. Controlled Closing Swing Check Valves (lever & weight)

Check valves shall be of the controlled closing swing type. The controlled closing swing check valves shall be guaranteed to operate under severe conditions as check valves. The valve shall be designed to open smoothly, provide full pipe line flow, permit minimum head loss and close at a controlled rate of speed for the final predetermined portion of its stroke. All bolts and nuts used in the assembly shall be steel, commercial.

The valve body shall be Cast Iron ASTM A126-B/ductile iron ASTM A536. The disc arm and chamber level shall be of heavy steel construction and keyed to the hinge shaft. The hinge shaft shall be of 18-8 stainless steel and of adequate diameter to withstand a complete hydraulic unbalance pressure of 125 psi on the valve disc. A single cushioning device mounted on the external side of the valve shall control the valve closure by way of the interchange of oil to and from an oil reservoir. The use of air or gas pressurized oil reservoir shall not be permitted. The oil plunger assembly shall be rigidly attached to the valve body by shoulder bolts or dowel pins to prevent fretting.

The Manufacturer, if required by the Engineer or the City, shall submit design calculations of principle component stresses to substantiate the integrity of the valve for the working pressure

The valve when closed shall be tight seating by way of a resilient replaceable seat against a bronze seat ring in the body.

defective material shall be replaced and made good by the Manufacturer. Under these specifications, any valve stuffing box that leaks for any reason or because of excessive wear or deterioration of packing, shall be reason for classification as defective material.

B. Slanted / Tilted Check Valves

Slanted or tilted check valves shall be furnished and installed where shown on the Plans.

Valves shall be as manufactured by GA Industries or Series 6000 as manufactured by APCO.

The City reserves the right to inspect all valves before shipment is made. Any failure of valves

to operate satisfactorily during the first year of installation due to faulty workmanship or

The body of the valve shall be ductile iron or cast iron with access ports to the disc. The disc shall be cast iron. The seat and disc rings shall be bronze. Pivot pins and bushings shall be bronze or stainless steel. The valve shall include a localized indicator of the position of the

The valves shall include a top mounted oil dash pot to prevent slamming of the disc. The dash pot shall control the last 10% of closure of the disc. The speed of closure within this 10% shall be adjustable.

Valves shall be APCO Slanting Disc, Valmatic or Golden Anderson Tilted Disc or approved

FLANGES

Flanges shall be cast solid and faced accurately at right angles to the axis of the casting. Dimensions and drilling of flanges shall be in accordance with the American Standard Association for a working pressure of 125 pounds per square inch. Special drilling shall be provided where necessary.

W3.12 FIRE HYDRANTS

Fire hydrants shall be dry-barrel type conforming to the requirements of the latest revision of AWWA C502. Hydrants shall be designed such that the hydrant valve closes with line pressure preventing loss of water and consequent flooding in the event of traffic damage.

Hydrants shall have six (6)-inch mechanical joint inlet connections, two 2½-inch hose connections and one $4^{1}/_{2}$ -inch pumper connection. Threads for the hose and pumper connections shall be in accordance with National Standard Thread. Hydrants shall be according to Manufacturer's standard pattern. Hydrants shall be equipped with "O" ring packing. Each nozzle cap shall be provided with a Buna-N rubber

Hydrants shall be so arranged that the direction of outlets may be turned 90 degrees without interference with the drip mechanism or obstructing the discharge from any outlet. The body of the hydrant shall be equipped with a breakable flange, or breakable cast iron flange bolts, just above the grade line.

D. A bronze or rustproof steel nut and check nut shall be provided to hold the main hydrant valve on its stem.

Hydrant valve opening shall have an area at least equal to that area of a $4^{1}/_{2}$ -inch minimum diameter circle and be obstructed only by the valve rod. Each hydrant shall be able to deliver 500 gallons minimum through its two 2½-inch hose nozzles when opened together with a loss of not more than two (2) psi in the

Hydrants shall be designed for installation in a trench that will provide minimum cover as noted on Plans and for the flange to be 3 \(^1\),-inches above ground surface. Hydrant extensions shall be as manufactured by the company furnishing the hydrants and of a style appropriate for the hydrants as furnished.

Hydrants shall be provided with an automatic and positively operating, non-corrodible drain or drip valve so as to drain the hydrant completely when the main valve is shut. A drain valve operating by

> VALVES, HYDRANTS W3-5 AND APPURTENANCES

springs or gravity is not acceptable.

Operating stems whose threads are located in the barrel or waterway shall be of manganese bronze, everdur, or other high-quality non-corrodible metal, and all working parts in the waterway shall be bronze

Hydrants shall open by turning operating nut to left (counter-clockwise) and shall be marked with a raised arrow and the word "open" to indicate the direction to turn stem to open hydrant.

Hydrants shall be furnished with caps, double galvanized steel hose cap chain, galvanized steel pumper hose cap chain, a galvanized steel chain holder and any other hooks and/or appurtenances required for

Hydrant operating nut shall be AWWA Standard pentagonal type measuring 1½-inch point to flat.

Hydrants shall be hydrostatically tested as specified in AWWA C502.

Hydrants shall be of the following:

1. Kennedy – K81

American Darling – B84B

2. Clow Medallion

All iron work to be set below ground, after being thoroughly cleaned, shall be painted with two (2) coats of asphalt varnish specified in AWWA C502. Iron work to be left above ground shall be factory primed and painted silver using a high grade enamel paint of quality and color to correspond to the present standard of the City.

Fire hydrants shall be installed on the same side of the street or roadway as the water main and shall be

P. Heel and thrust blocks shall be placed in undisturbed soil as shown in the details of the Plans.

Double blue reflector "HYE-LITES" brand as manufactured by pavement markers ink shall be installed at the centerline of the street or roadway perpendicular to the hydrant.

W3.13 CORPORATION STOPS

A. Corporation stops shall be brass, not less than 1-inch in diameter and shall be installed where shown, specified or required.

B. <u>Provide corporation stops as manufactured by the following:</u>

1. Ford Company

W3.14 **COMBINATION AIR-VACUUM RELIEF VALVES**

The air-vacuum release valves shall be installed as shown on the Plans. The valve body shall be of cast iron ASTM A126-B; the floats, float guide, and stem shall be of Type 316 stainless steel. The resilient seat shall be of Buna N. The valve shall be suitable for 150 psig working pressure. Valve shall have standard NPT inlets and outlet ports with diameters as indicated on the Plans. Valve shall be Model 200A Series by APCO Valve and Primer Corporation, Schaumburg, IL, or approved equal.

SURFACE PREPARATION AND SHOP COATINGS

A. The interior ferrous metal surfaces, except finished or bearing surfaces, shall be blast cleaned in accordance with SSPC SP-6 and painted with two (2) coats of an approved two (2) component coal tar

> VALVES, HYDRANTS AND APPURTENANCES

epoxy coating specifically formulated for potable water use. The coating used must appear on the current edition of the United States Environmental Protection Agency's list entitled "Accepted Categories and Subcategories of Coatings, Liners and Paints for Potable Water Usage."

VALVES, HYDRANTS

AND APPURTENANCES

B. Exterior ferrous metal surfaces of all buried valves and hydrants shall be blast cleaned in accordance with SSPC SP-6 and given two (2) shop coats of a heavy coat tar enamel or an approved two (2)

INSPECTION AND PREPARATION W3.16

- During installation of all valves and appurtenances, the Contractor shall verify that all items are clean, free of defects in material and workmanship and function properly.
- B. All valves shall be closed and kept closed until otherwise directed by the Engineer or the City.

W3-4

W3.17 INSTALLATION OF BURIED VALVES AND VALVE BOXES

- Buried valves shall be cleaned and manually operated before installation. Buried valves and valve boxes shall be set with the stem vertically aligned in the center of the valve box. Valves shall be set on a firm foundation and supported by tamping pipe bedding material under the sides of the valve. The valve box shall be supported during backfilling and maintained in vertical alignment with the top flush with finish grade. The valve box shall be set so as not to transmit traffic loads to the valve.
- Before backfilling, all exposed portions of any bolts shall be coated with two (2) coats of bituminous

INSTALLATION OF TAPPING SLEEVES AND VALVES W3.18

closer than three (3) feet from a pipe joint.

- The City of Georgetown shall be contacted and their permission granted prior to tapping a line. The required procedures and time table shall be followed exactly.
- Installation shall be made under pressure and flow shall be maintained. The diameters of the tap shall be a minimum of ¹/₄-inch less than the inside diameter of the branch line.
- The entire operation shall be conducted by workers experienced in the installation of tapping sleeves and valves. The tapping machine shall be furnished by the Contractor. Determine the location of the line to be tapped to confirm that the proposed location will be satisfactory
- A tapping sleeve and valve with boxes shall be set squarely centered on the line to be tapped. Adequate support shall be provided under the sleeve and valve during the tapping operation. Thrust blocks or other permanent restraint acceptable to the Engineer and the City shall be provided behind all tapping sleeves. Proper tamping of supporting pipe bedding material around and under the valve and sleeve is mandatory for buried installations.

and that no interference will be encountered such as joints or fittings. No tap or sleeve will be made

After completing the tap, the valve shall be flushed to ensure that the valve seat is clean. All proper regulatory procedures (including disinfection) shall be followed exactly.

W3.19 **INSTALLATION OF FIRE HYDRANTS**

Fire hydrants shall be set at the locations as shown on the Plans and bedded on a firm foundation. Hydrants and connecting pipe shall have at least the same depth of cover as the distributing pipe. A drainage pit as detailed on the Plans shall be filled with 3/4-inch washed rock gravel and compacted. The hydrants shall be set upon a slab of concrete not less than four (4)-inches thick and 15-inches square. During backfilling, additional screened gravel shall be brought up around and six (6) inches over the drain port. Each hydrant shall be set in true vertical alignment and properly braced.

2,500 psi concrete thrust blocks shall be placed between the back of the hydrant inlet and undisturbed soil at the end of the trench. Minimum bearing area shall be as shown on the Plans. Eight (8) mil. Povethlene film shall be placed around the hydrant elbow before placing concrete. CARE SHALL BE TAKEN TO ENSURE THAT CONCRETE DOES NOT PLUG THE DRAIN PORTS.

All connections from the main to the fire hydrants shall be anchoring mechanical joints designed to prevent movement due to thrust or pressure.

The hydrant shall be tied to the pipe with suitable rods or clamps, and shall be coated with Koppers 300 or approved equal at a minimum of 8 mil. thick. Bolts shall have a zinc bolt cover per AWWA. Hydrant paint shall be touched up as required after installation.

Fire hydrants shall be factory primed and painted silver using a high grade enamel.

Fire sprinkler lines shall be protected by a reduced pressure zone (RPZ). All fire lines shall be ductile iron pipe. All private fire lines shall be separated by double detecta check.

FIELD TESTS AND ADJUSTMENTS W3.20

Conduct a functional field test of each valve, including actuators and valve control equipment, inpresence of Engineer or the Representative of the City to demonstrate that each part and all components together function correctly. All testing equipment required shall be provided by the Contractor at his/her sole

W3.21 **PAYMENT**

Gate valves, tapping sleeves and tapping valves, fire hydrants, and air and vacuum relief valves complete in place as shown on the Plans and as specified, will be paid for at the unit contract price per each as provided in the Proposal and Bid Schedule.

The unit price per each installation shall be the total compensation for furnishing all labor, materials, tools, equipment and incidentals necessary to complete the work including excavation, base blocking, disposal of surplus materials and backfill in conformance with the Plans and these specifications. The six (6) inch connection pipe, six (6) inch gate valve, test station, concrete collar, thrust block, drain pit, concrete pad, rods, bolts, paint, protective coatings, and fittings for fire hydrants shall not be paid for

Fire hydrants shall be furnished with the proper length of barrel to comply with these specifications. Barrel extensions will not be measured and paid for separately.

No separate payment shall be made for work performed in accordance with this specification, other than that listed in Parts A-C of this subsection, and the cost thereof shall be included in the proper items of the Proposal and Bid Schedule.

END OF SECTION

TECHNICAL SPECIFICATIONS

<u>SECTION W4 – ENCASEMENT PIPE</u>

SCOPE OF WORK

A. This specification covers the requirements for furnishing and installing encasement pipe complete inplace including any required spacers and end plugs as shown on the plans and specified herein.

W4.02 **SUBMITTALS**

Within 30 days after the Notice to Proceed, the Contractor shall submit to the Engineer for approval, technical product literature including type and Manufacturer of pipe, spacers, and end plugs, and all other pertinent data to illustrate conformance to the specification found within.

W4.03 **GENERAL**

Where pipe is required to be installed under highways, streets, or other facilities, construction shall be made in such a manner that will not interfere with the operation of the street, highway, or other facility, and shall not weaken or damage any embankment or structure.

All carrier pipe shall be laid to the required line and grade within the specified limits through the encasement pipe. Carrier pipe shall be handled and placed in the encasement pipe by use of proper skids, wedges, guide fails or other approved means. Care shall be taken that once the pipe is in place to line and grade, it shall not be disturbed or become displaced. All carrier pipe shall have restrained joints.

W4.04 **MATERIALS**

Encasement pipe shall be smooth steel 35,000 psi yield strength with thickness according to the following

Carrier Size	Minimum Encasement Steel Casing Size	Minimum Casing Thickness
(Inner Diameter)	(Inner Diameter)	(Inches)
4"	14"	0.2500
6"	16"	0.2500
8"	18"	0.2500
10"	20"	0.3125
12"	24"	0.3175
14"	24"	0.375
16"	30"	0.4375
18"	30"	0.4375
20"	36"	0.500
24"	42"	0.500
27"	42"	0.500
30"	48"	0.500
33"	48"	0.500
36"	54"	0.500
39"	60"	0.500
42"	60"	0.500

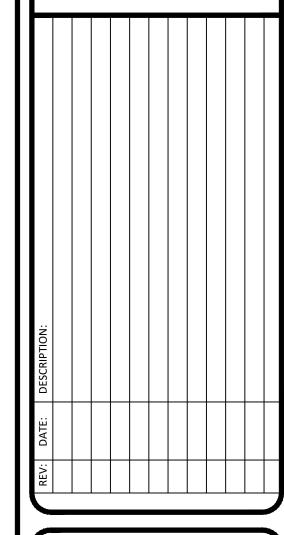
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W4-1 ENCASEMENT PIPE

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JOB NUMBER: CHV21004 ISSUE DATE:

> WATER **NOTES III**

VALVES, HYDRANTS

C. End Plugs shall be provided as required and as specified by the pipe manufacturer.

W4.05 **PAYMENT**

- Separate payment will be made for Steel Encasement Pipe per linear foot as called for on the Plans and set forth in the Proposal and Bid Schedule.
- All costs incurred for furnishing and installing encasement pipe shall include all labor, materials, tools, equipment and incidentals necessary to perform all work or whatever nature required to complete the specific operation.

END OF SECTION



Georgetown Utility Systems City of Georgetown

Construction General Notes

- 1. These construction plans were prepared, sealed, signed and dated by a Texas Licensed Professional Engineer. Therefore, based on the engineer's concurrence of compliance, the construction plans for construction of the proposed project are hereby approved subject to the standard Construction Specifications and Details Manual and all other applicable City, State and Federal Requirements and Codes.
- 2. This project is subject to all City Standard Specifications and Details in effect at the time of submittal of the project to the City.
- 3. The site construction plans shall meet all requirements of the approved site plan.
- 4. Wastewater mains and service lines shall be SDR 26 PVC.
- 5. Wastewater mains shall be installed without horizontal or vertical bends.
- 6. Maximum distance between wastewater manholes is 500 feet.
- 7. Wastewater mains shall be low pressure air tested and mandrel tested by the contractor according to City of Georgetown and TCEQ requirements.
- 8. Wastewater manholes shall be vacuum tested and coated by the contractor according to City of Georgetown and TCEQ requirements.
- 9. Wastewater mains shall be camera tested by the contractor and submitted to the City on DVD format prior to paving the streets.
- 10. Private water system fire lines shall be tested by the contractor to 200 psi for 2 hours.
- 11. Private water system fire lines shall be ductile iron piping from the water main to the building sprinkler system, and 200 psi C900 PVC for all others.
- 12. Public water system mains shall be 150 psi C900 PVC and tested by the contractor at 200 psi for 15 minutes and 150 psi for 2 hours.
- 13. All bends and changes in direction on water mains shall be restrained and thrust blocked.
- 14. Long fire hydrant leads shall be restrained.
- 15. All water lines are to be bacteria tested by the contractor according to the City standards and specifications.

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- 16. Water and Sewer main crossings shall meet all requirements of the TCEQ and the City.
- 17. Flexible base material for public streets shall be TXDOT Type A Grade 1.
- 18. Hot mix asphaltic concrete pavement shall be Type D unless otherwise specified and shall be a minimum of 2 inches thick on public streets and roadways.
- 19. All sidewalk ramps are to be installed with the public infrastructure.
- 20. A maintenance bond is required to be submitted to the City prior to acceptable of the public improvements. This bond shall be established for 2 years in the amount of 10% of the cost of the public improvements and shall follow the City format.
- 21. Record drawings of public improvements shall be submitted to the City by the design engineer prior to acceptance of the project. These drawings shall be a pdf emailed to the City Development engineer.

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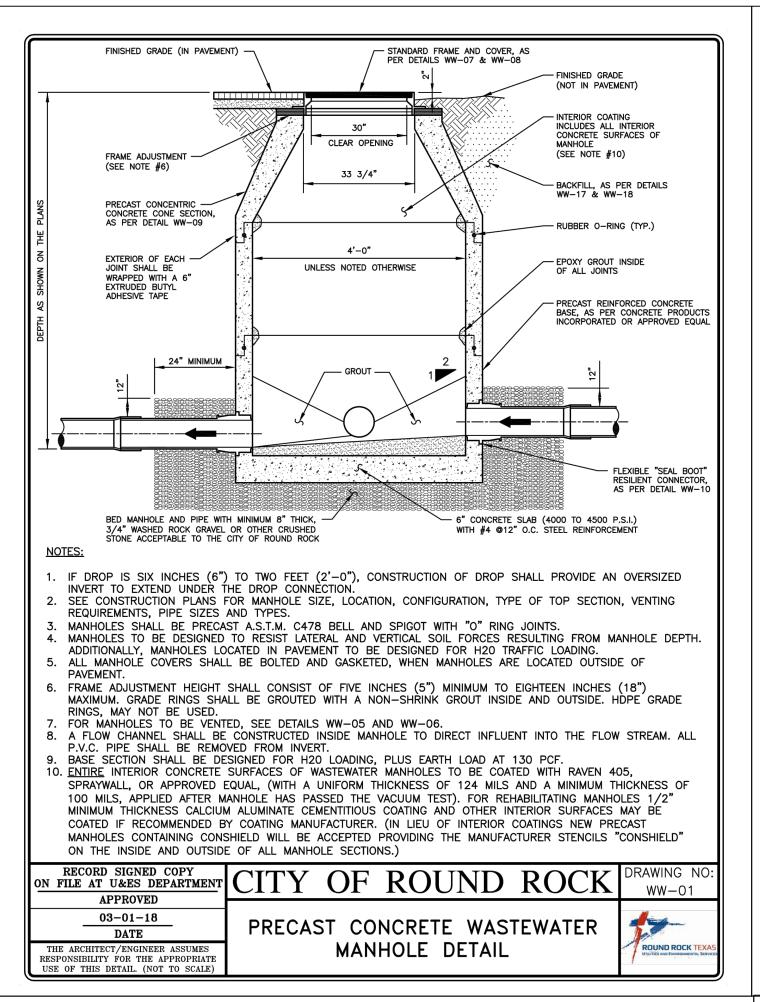
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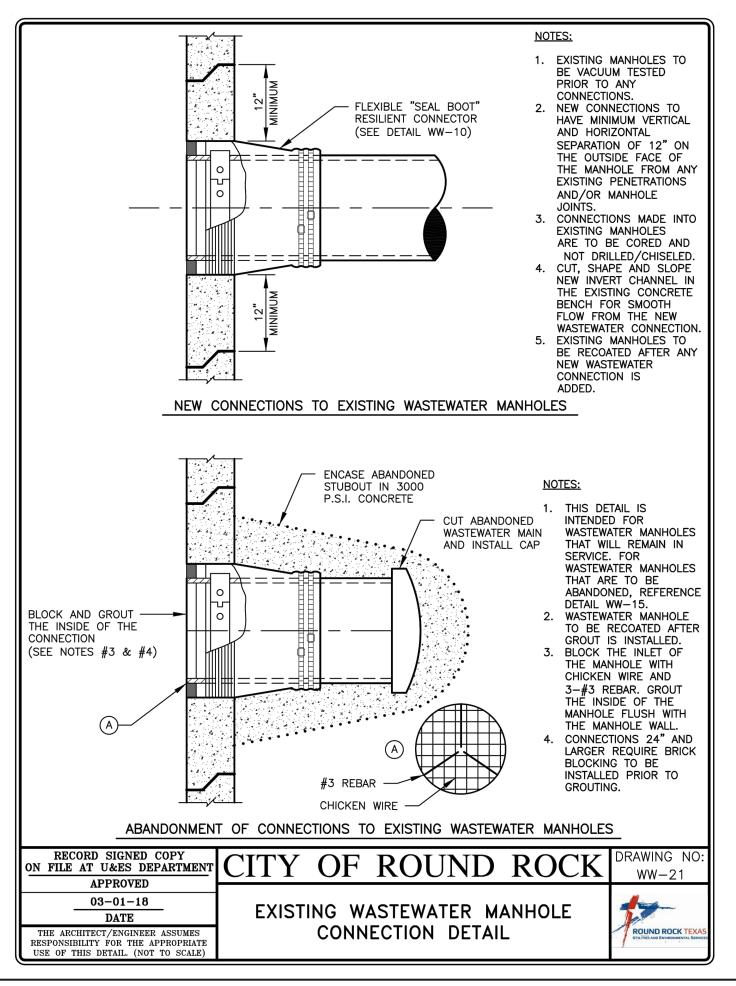
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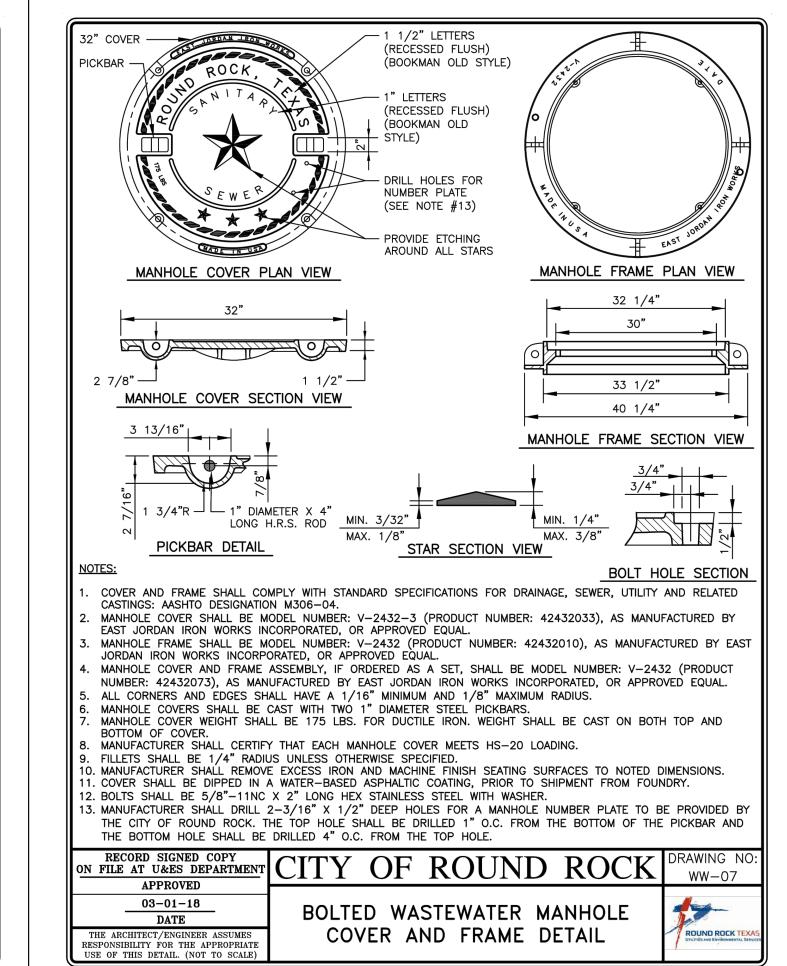
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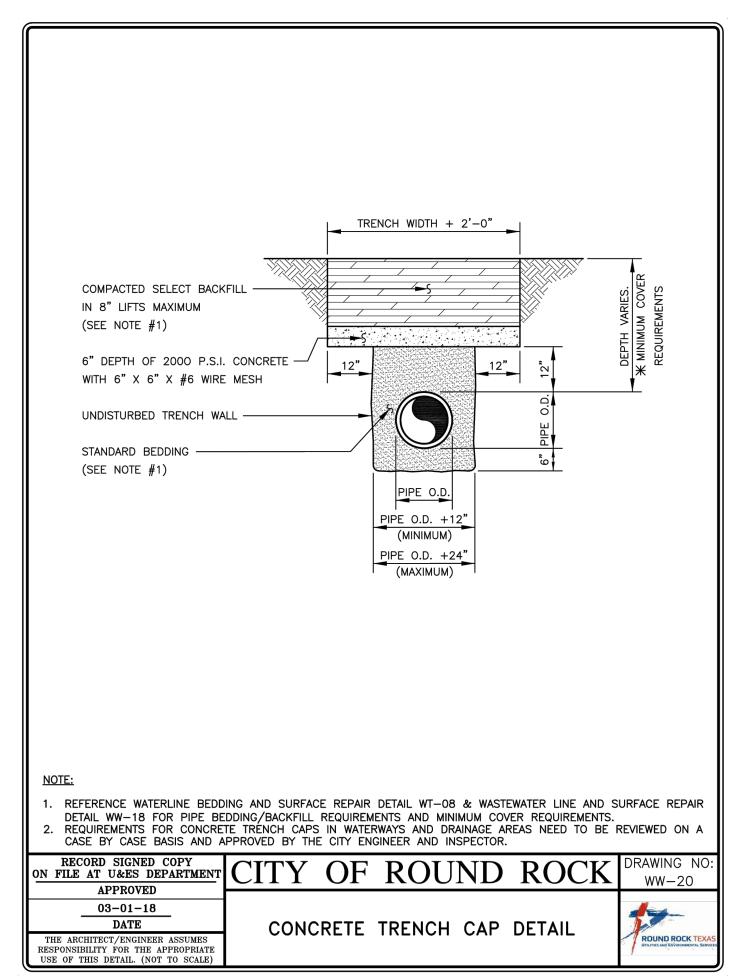
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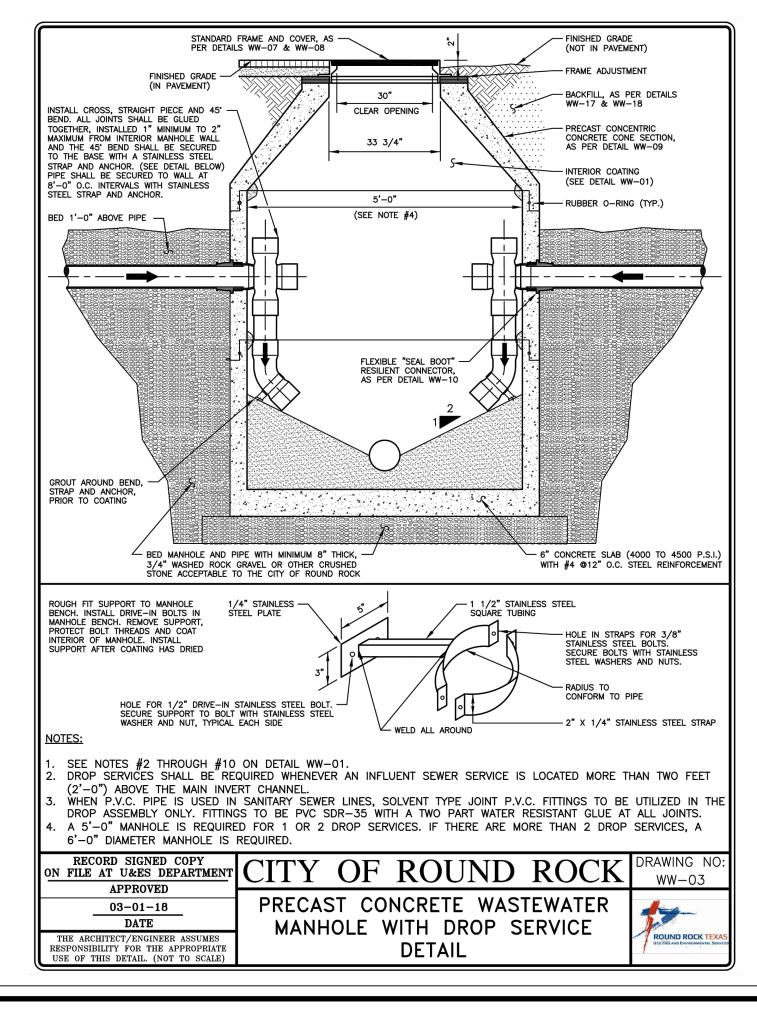


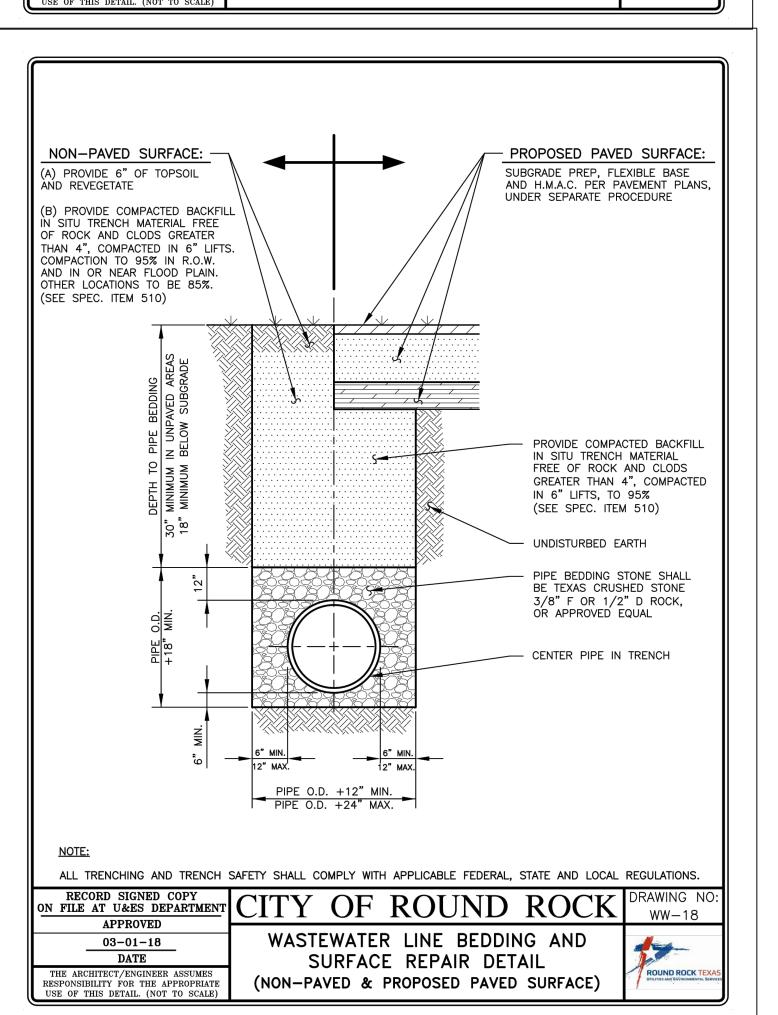














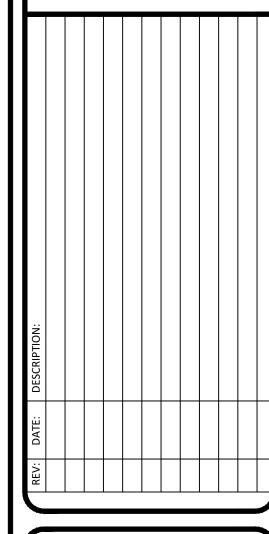
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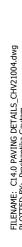
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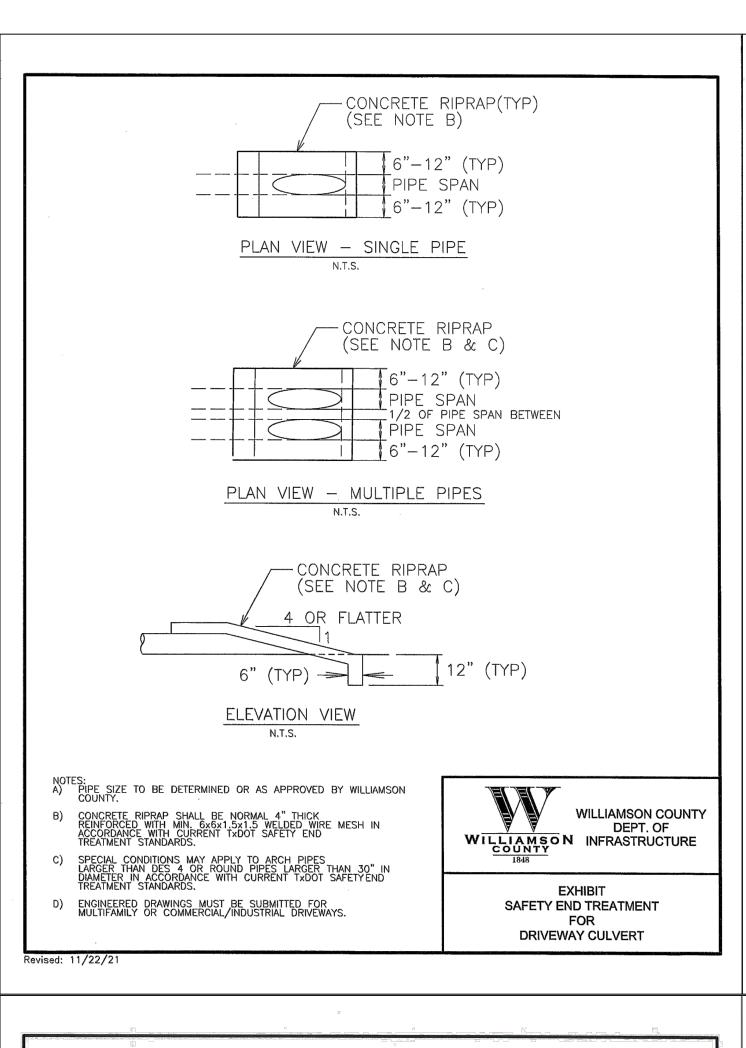
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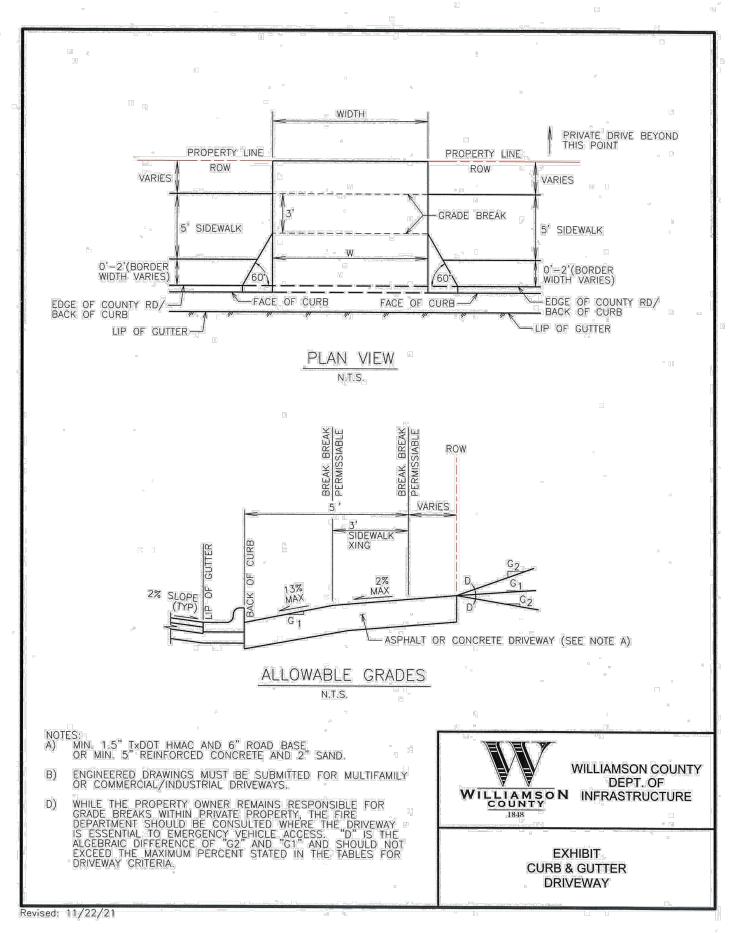
WASTE WATER DETAILS

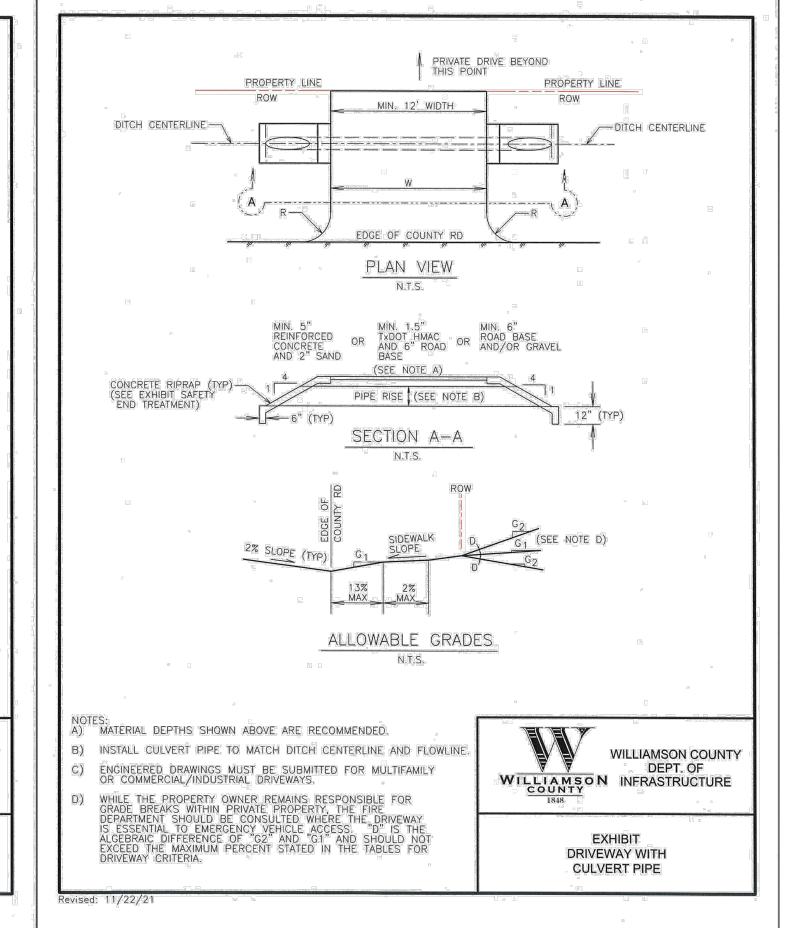
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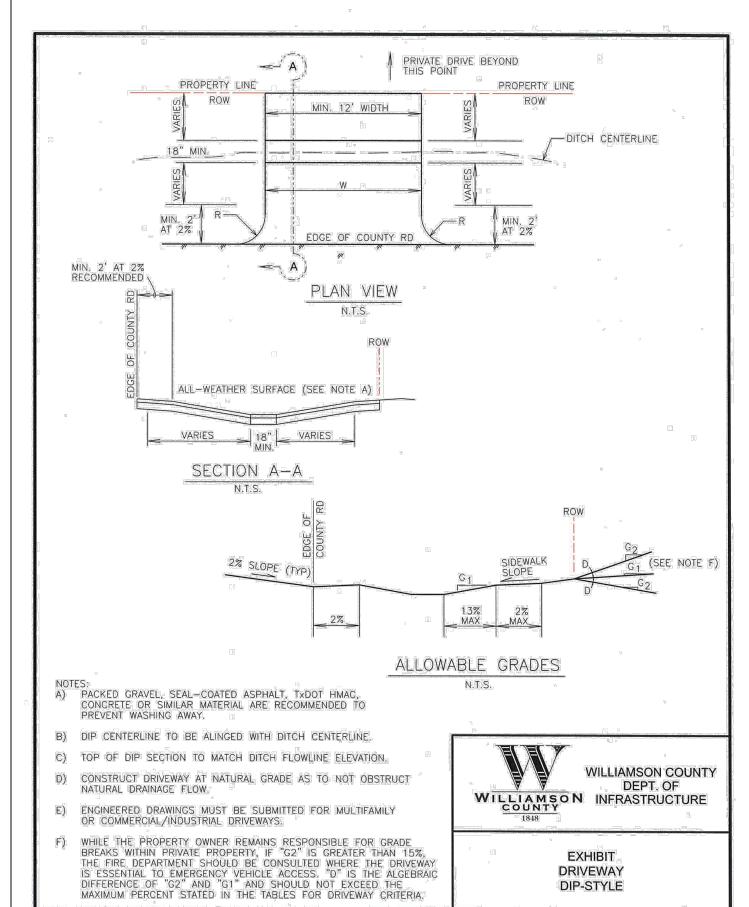


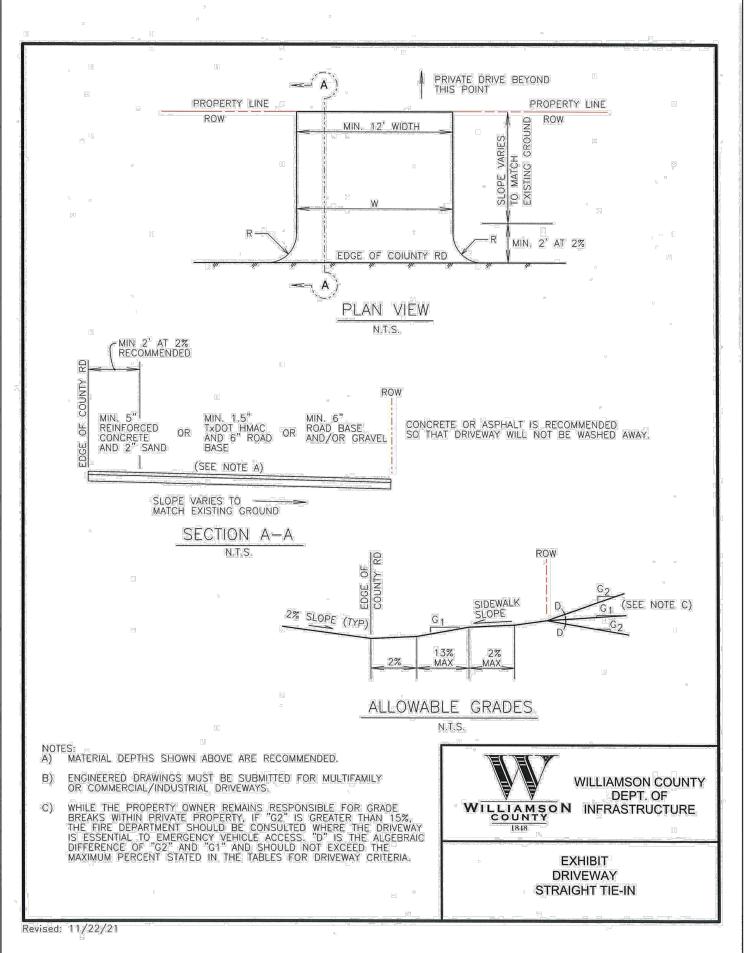










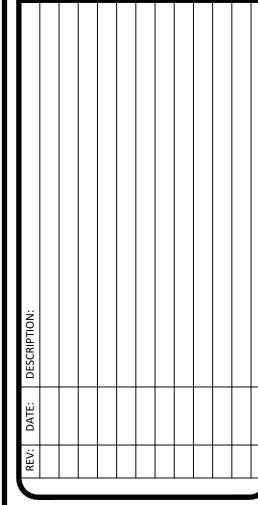




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CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

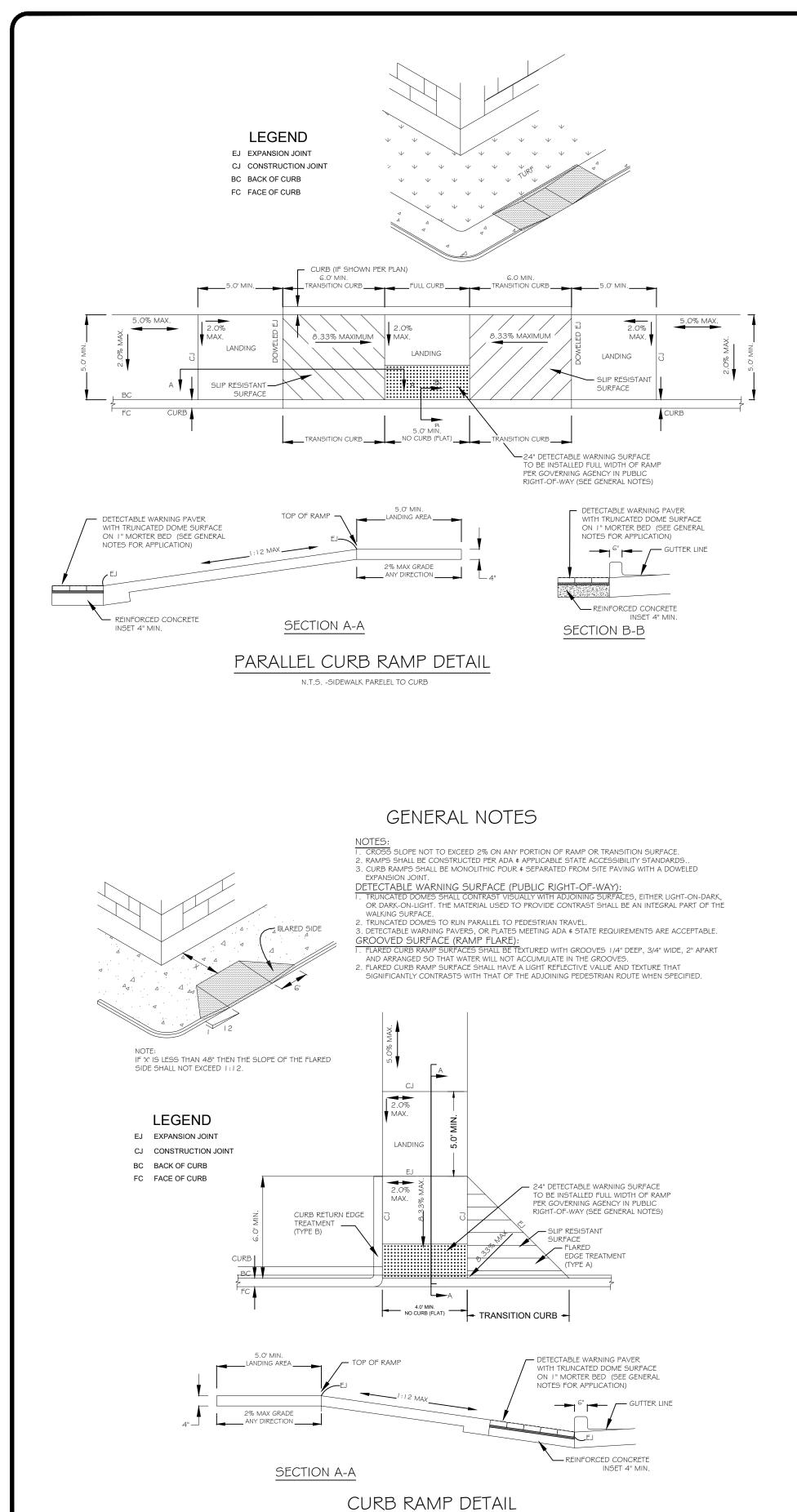
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PAVING DETAILS I

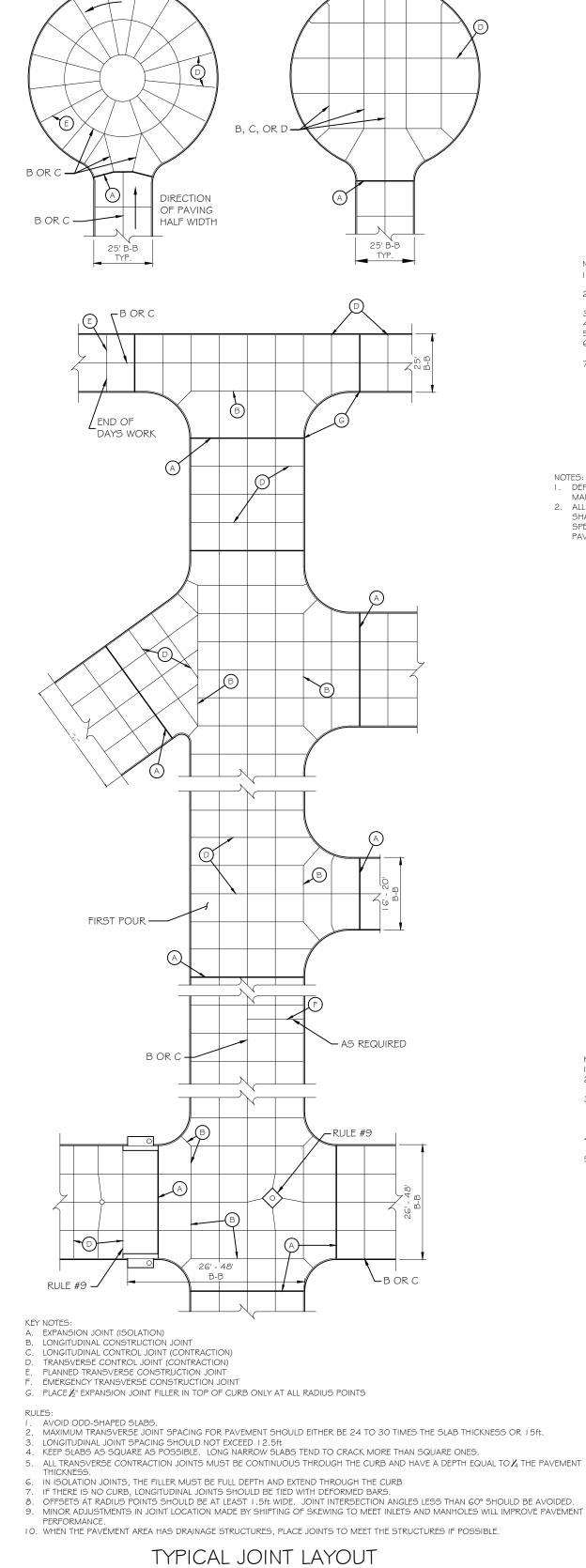
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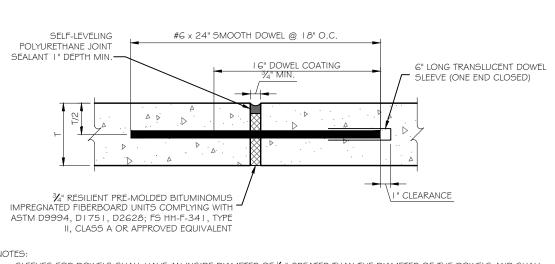






N.T.S. - FLARED OR RETURN CURB



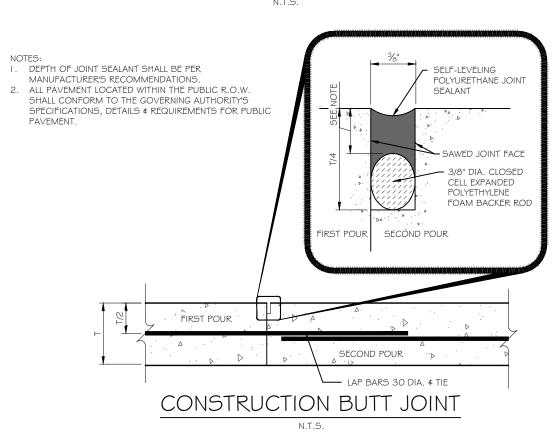


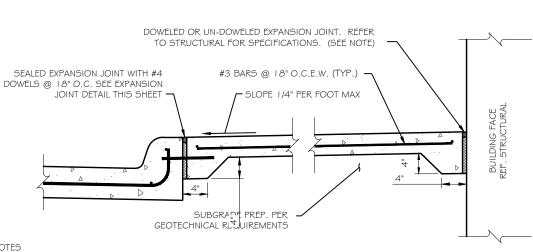
3. DOWEL COATING SHALL BE ASPHALTIC COATING.

DOWELS SHALL NOT BE TIED TO OTHER REINFORCEMENT.
 REFER TO SIDEWALK DETAILS THIS SHEET FOR EXPANSION JOINTS IN SIDEWALK AREAS.

DETAILS & REQUIREMENTS FOR PUBLIC PAYEMENT. 7. FINISHED SURFACES SHALL BE INSTALLED FLUSH WITH A DIFFERENTIAL ELEVATION NOT TO EXCEED \(\begin{align*} \lambda_n^* \\ \ellin \right* \rig

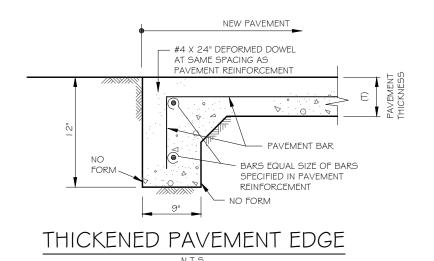
EXPANSION JOINT (ISOLATION)

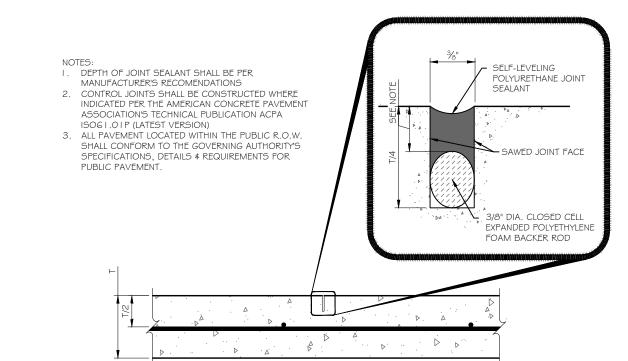




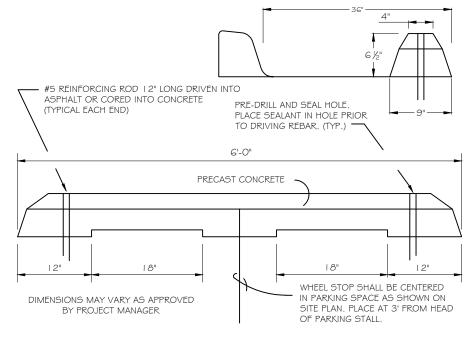
- ALL SIDEWALK JOINTS LOCATED WITHIN 25' OF A BUILDING FACE OR ADJACENT TO ANY STRUCTURE SHALL BE SEALED. REFER TO STRUCTURAL PLANS/DETAILS FOR DOWEL/HINGE JOINT AT ALL CONNECTIONS BETWEEN FOUNDATIONS/STOOPS AND FLATWORK.
- 3. THE SUBGRADE PREPARATION FOR ANY FLATWORK OR SIDEWALK PAVEMENT WITHIN 25' FROM ANY BUILDING FACE OR ADJACENT TO ANY STRUCTURE SHALL AS SPECIFIED BY THE GEOTECHNICAL REPORT. IN THE EVENT THAT THE GEOTECHNICAL REPORT DOES NOT CONTAIN A RECOMMENDATION THE CONTRACTOR SHALL NOTIFY THE ENGINEER
- 4. ALL PAVEMENT LOCATED WITHIN THE PUBLIC R.O.W. SHALL CONFORM TO THE GOVERNING AUTHORITY'S SPECIFICATIONS, DETAILS & REQUIREMENTS FOR PUBLIC PAVEMENT. 5. FINISHED SURFACES SHALL BE INSTALLED FLUSH WITH A DIFFERENTIAL ELEVATION NOT TO EXCEED 1/8".

SIDEWALK/CURB/BUILDING DETAIL

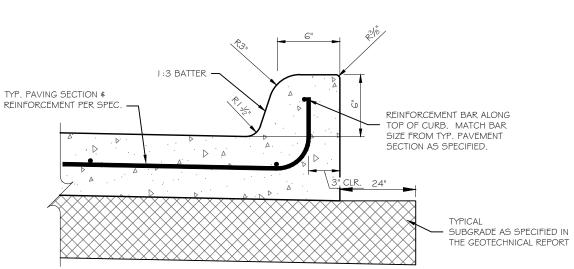






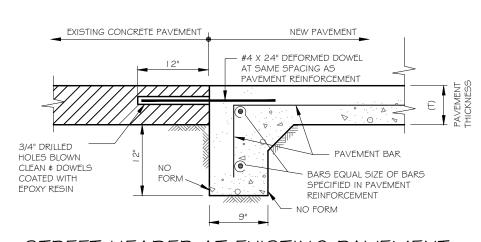


WHEEL STOP DETAIL

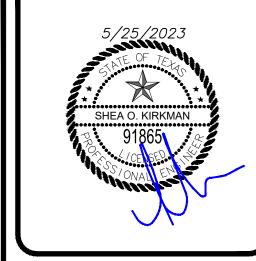


- ALL CURBS ARE CONSTRUCTED OF PORTLAND CEMENT CONCRETE UNLESS OTHERWISE SHOWN. GRADES SHALL BE MEASURED AT BACK OF CURB UNLESS OTHERWISE SPECIFIED
- ALL PAVEMENT LOCATED WITHIN THE PUBLIC R.O.W. SHALL CONFORM TO THE GOVERNING AUTHORITY'S SPECIFICATIONS, DETAILS & REQUIREMENTS FOR PUBLIC PAVEMENT. 4. CONTROL JOINTS THROUGH CURB SHALL BE SEALED IN THE PAVEMENT AND TERMINATE AT THE GUTTER.

MONOLITHIC CURB DETAIL



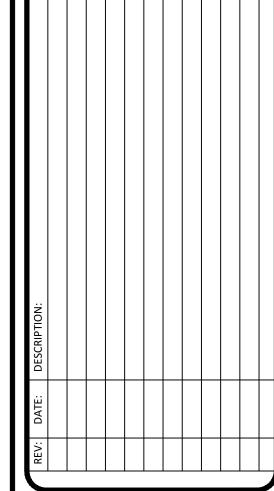
STREET HEADER AT EXISTING PAVEMENT



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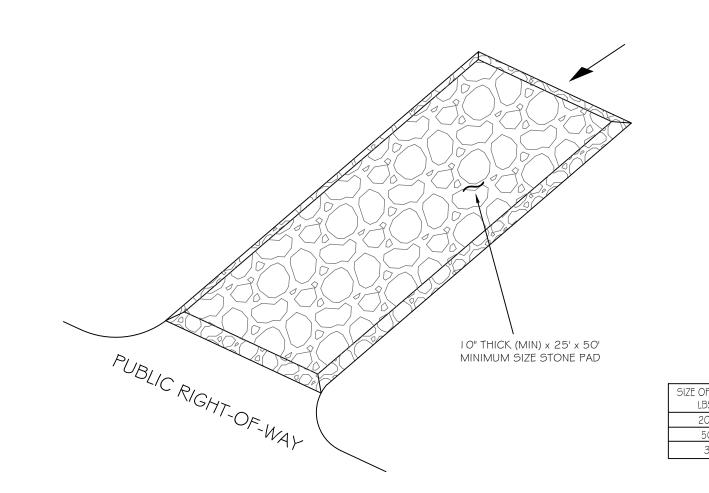


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ISSUE DATE:

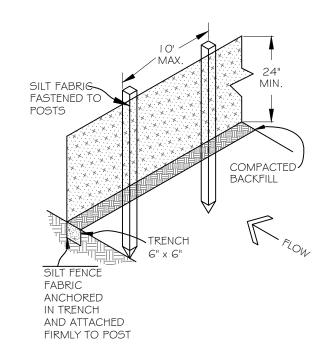
PAVING DETAILS II

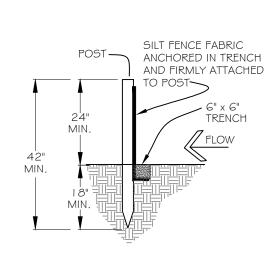


NOTES:

- I. THE ENTRANCE SHALL BE MAINTAINED TO PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAY. THIS MAY REQUIRE DRESSING WITH ADDITIONAL STONE AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT.
- 2. WHEN NECESSARY, WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE INTO PUBLIC RIGHT-OF-WAY. WASHING SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE WHICH DRAINS INTO AN APPROVED SEDIMENT CONTROLLING STRUCTURE. USE SAND BAGS, GRAVEL, BOARDS OR OTHER APPROVED METHODS TO PREVENT SEDIMENT FROM ENTERING ANY STORM DRAIN, DITCH, OR
- 3. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHT-OF-WAY MUST BE REMOVED IMMEDIATELY.

TEMPORARY CONSTRUCTION EXIT POINT





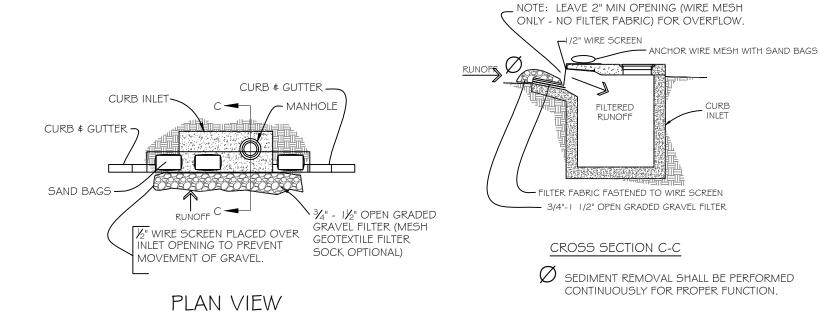
SILT FENCE GENERAL NOTES:

- I. STEEL POSTS WHICH SUPPORT THE SILT FENCE SHALL BE INSTALLED ON A SLIGHT ANGLE TOWARD THE ANTICIPATED RUNOFF SOURCE. POST MUST
- BE EMBEDDED A MINIMUM OF ONE FOOT. 2. THE TOE OF THE SILT FENCE SHALL BE TRENCHED IN WITH A SPADE OR MECHANICAL TRENCHER, SO THAT THE DOWNSLOPE FACE OF THE TRENCH IS FLAT AND PERPENDICULAR TO THE LINE OF FLOW. WHERE FENCE CANNOT BE TRENCHED IN (e.g. PAVEMENT), WEIGHT FABRIC FLAP WITH ROCK ON UPHILL SIDE TO PREVENT FLOW FROM SEEPING UNDER FENCE.
- 3. THE TRENCH MUST BE A MINIMUM OF G INCHES DEEP AND G INCHES WIDE TO ALLOW FOR THE SILT FENCE FABRIC TO BE LAID IN THE GROUND AND BACKFILLED WITH COMPACTED MATERIAL.
- 4. SILT FENCE SHOULD BE SECURELY FASTENED TO EACH STEEL SUPPORT POST OR TO WOVEN WIRE, WHICH IN TURN IS ATTACHED TO THE STEEL FENCE POST. THERE SHALL BE A 3 FOOT OVERLAP, SECURELY FASTENED WHERE ENDS OF FABRIC MEET.
- 5. INSPECTION SHALL BE MADE IN ACCORDANCE WITH PERMIT REQUIREMENTS. REPAIR OR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED.
- 6. SILT FENCE SHALL BE REMOVED WHEN THE SITE IS COMPLETELY STABILIZED SO AS NOT TO BLOCK OR IMPEDE STORM WATER FLOW OR DRAINAGE.

 7. ACCUMULATED SILT SHALL BE REMOVED WHEN IT REACHES A DEPTH OF HALF THE HEIGHT OF THE FENCE. THE SILT SHALL BE DISPOSED OF AT AN APPROVED SITE AND IN SUCH A MANNER AS TO NOT CONTRIBUTE TO ADDITIONAL SILTATION.

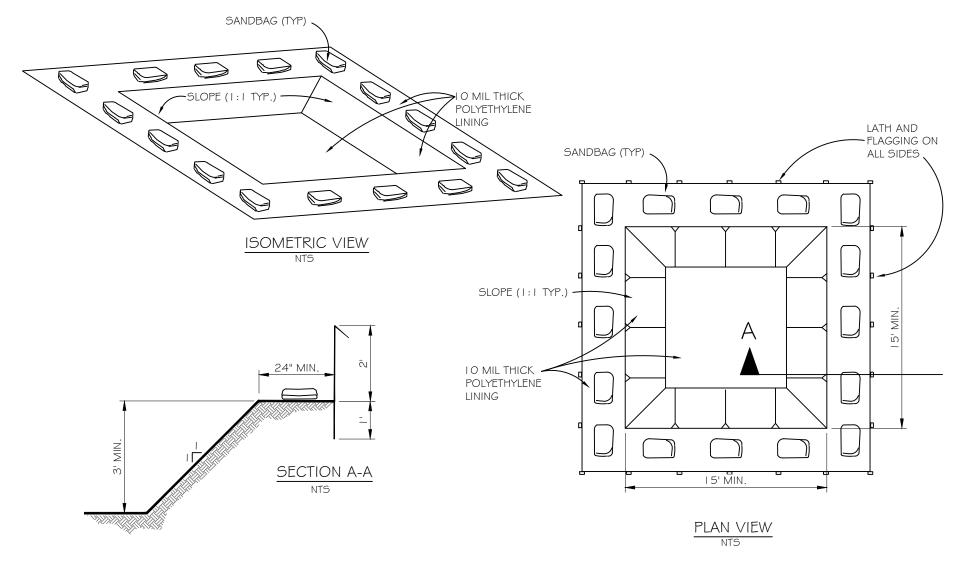
SILT FENCE

N.T.S.



(HIGHER VOLUME TRAFFIC AREAS)

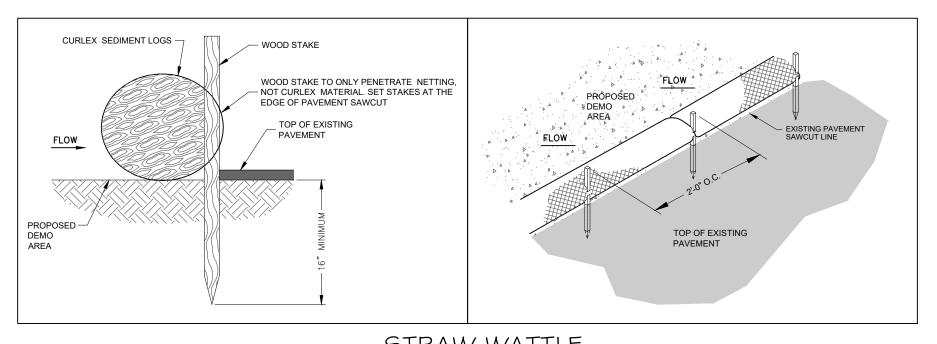
INLET PROTECTION N.T.S.



I. Actual layout, size and location to be determined by Contractor.

- The concrete washout sign shall be installed within 30 ft. of the temporary concrete washout facility. 3. Once concrete wastes are allowed to harden, the concrete should be broken up, removed and disposed of
- properly. dispose of hardened concrete on a regular basis.

TEMPORARY CONCRETE WASHOUT AREA



STRAW WATTLE

N.T.S.

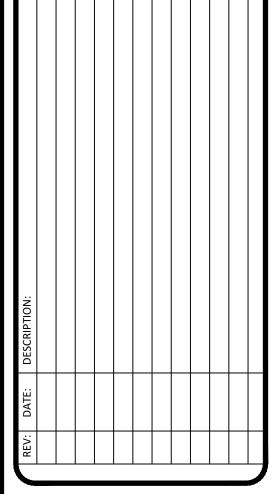


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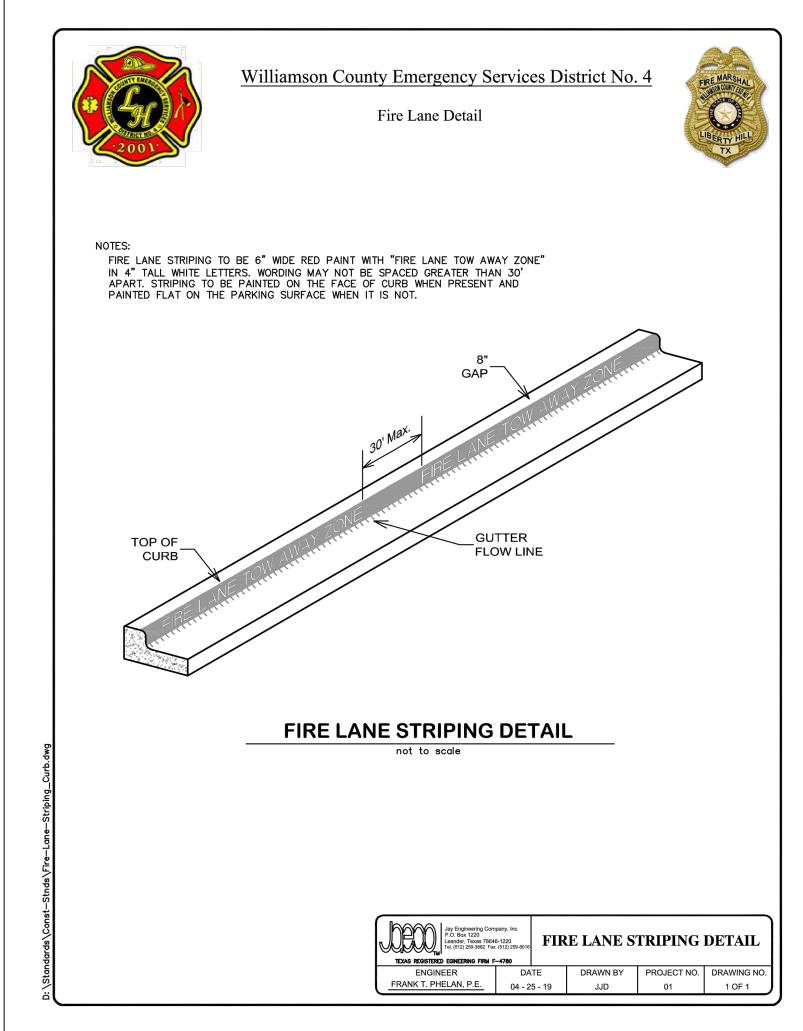
CEDAR PARK, TX 78613 TEXAS FIRM NO. 15874

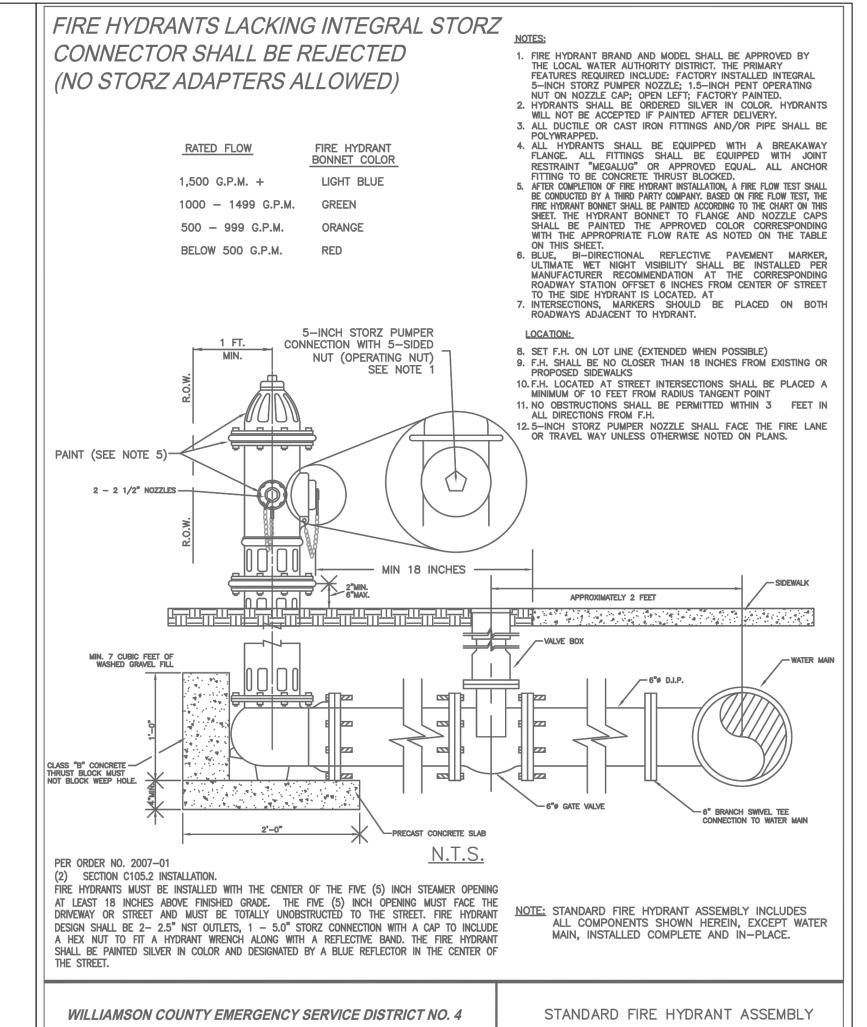
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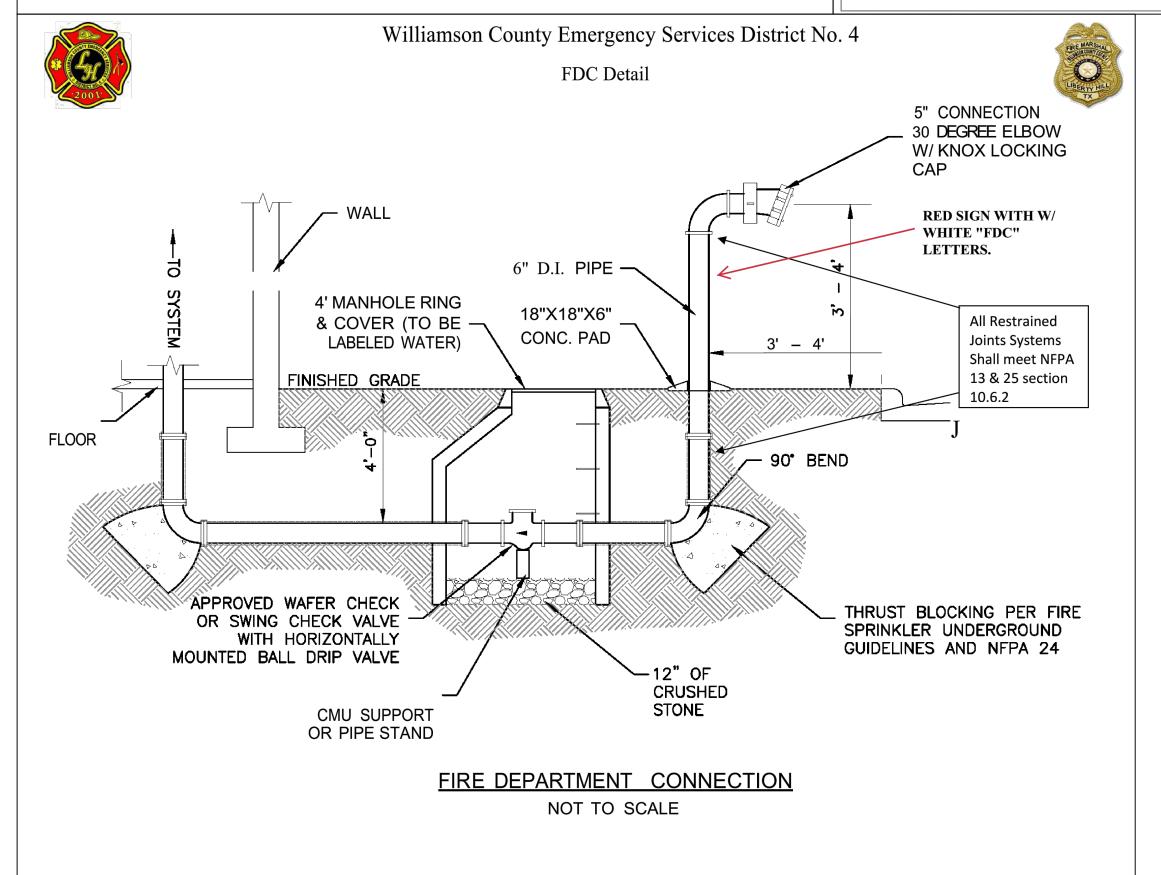
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DETAILS





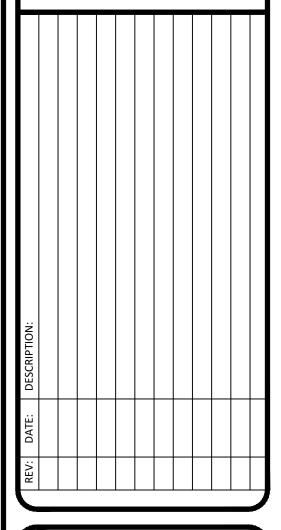




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JOB NUMBER: CHV21004
ISSUE DATE:

WCESD NO.4 DETAILS

SHEET: **17** (

ATTACHMENT N INSPECTION, MAINTENANCE, REPAIR AND RETROFIT FOR PERMANENT BMPs



The following details the owner's maintenance manual and worksheets for inspection and maintenance of the permanent BMPs proposed onsite.



Jellyfish® Filter Owner's Manual



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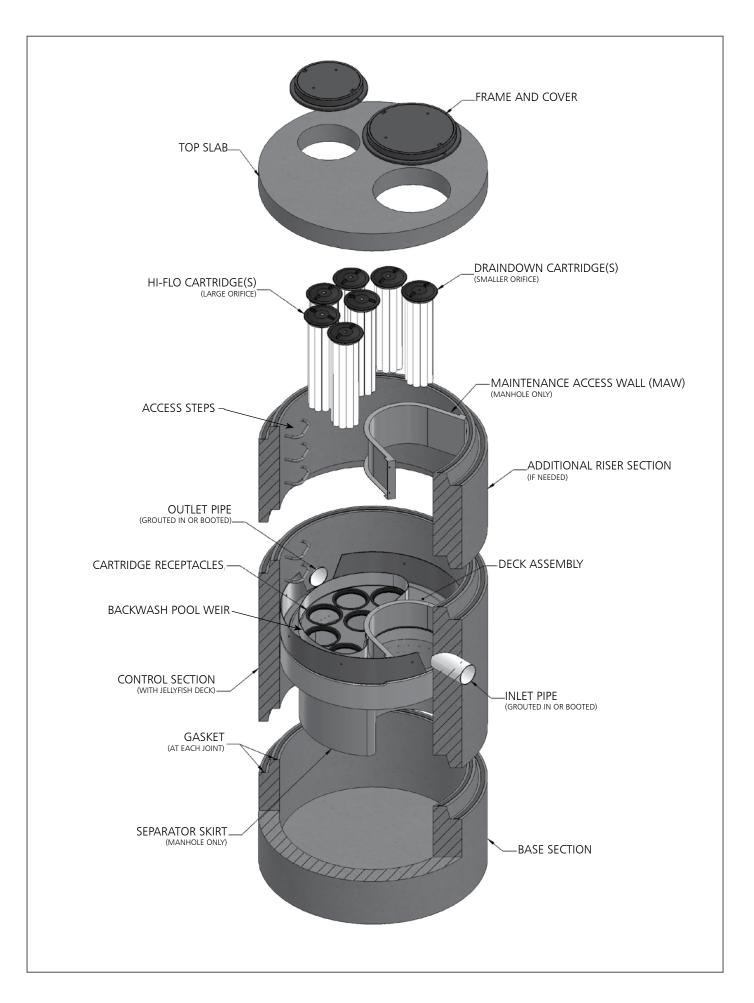
THANK YOU FOR PURCHASING THE JELLYFISH® FILTER!

Contech Engineered Solutions would like to thank you for selecting the Jellyfish Filter to meet your project's stormwater treatment needs. With proper inspection and maintenance, the Jellyfish Filter is designed to deliver ongoing, high levels of stormwater pollutant removal.

If you have any questions, please feel free to call us or e-mail us:

Contech Engineered Solutions

9025 Centre Pointe Drive, Suite 400 | West Chester, OH 45069 513-645-7000 | 800-338-1122 www.ContechES.com info@conteches.com



WARNINGS / CAUTION

- 1. FALL PROTECTION may be required.
- 2. WATCH YOUR STEP if standing on the Jellyfish Filter Deck at any time; Great care and safety must be taken while walking or maneuvering on the Jellyfish Filter Deck. Attentive care must be taken while standing on the Jellyfish Filter Deck at all times to prevent stepping onto a lid, into or through a cartridge hole or slipping on the deck.
- 3. The Jellyfish Filter Deck can be SLIPPERY WHEN WET.
- 4. If the Top Slab, Covers or Hatches have not yet been installed, or are removed for any reason, great care must be taken to NOT DROP ANYTHING ONTO THE JELLYFISH FILTER DECK. The Jellyfish Filter Deck and Cartridge Receptacle Rings can be damaged under high impact loads. This type of activity voids all warranties. All damaged items to be replaced at owner's expense.
- 5. Maximum deck load 2 persons, total weight 450 lbs.

Safety Notice

Jobsite safety is a topic and practice addressed comprehensively by others. The inclusions here are intended to be reminders to whole areas of Safety Practice that are the responsibility of the Owner(s), Manager(s) and Contractor(s). OSHA and Canadian OSH, and Federal, State/Provincial, and Local Jurisdiction Safety Standards apply on any given site or project. The knowledge and applicability of those responsibilities is the Contractor's responsibility and outside the scope of Contech Engineered Solutions.

Confined Space Entry

Secure all equipment and perform all training to meet applicable local and OSHA regulations regarding confined space entry. It is the Contractor's or entry personnel's responsibility to proceed safely at all times.

Personal Safety Equipment

Contractor is responsible to provide and wear appropriate personal protection equipment as needed including, but not limited to safety boots, hard hat, reflective vest, protective eyewear, gloves and fall protection equipment as necessary. Make sure all equipment is staffed with trained and/or certified personnel, and all equipment is checked for proper operation and safety features prior to use.

- Fall protection equipment
- Eye protection
- Safety boots
- Ear protection
- Gloves
- Ventilation and respiratory protection
- Hard hat
- Maintenance and protection of traffic plan

Chapter 1

1.0 - Owner Specific Jellyfish Filter Product Information

Below you will find a reference page that can be filled out according to your Jellyfish Filter specification to help you easily inspect, maintain and order parts for your system.

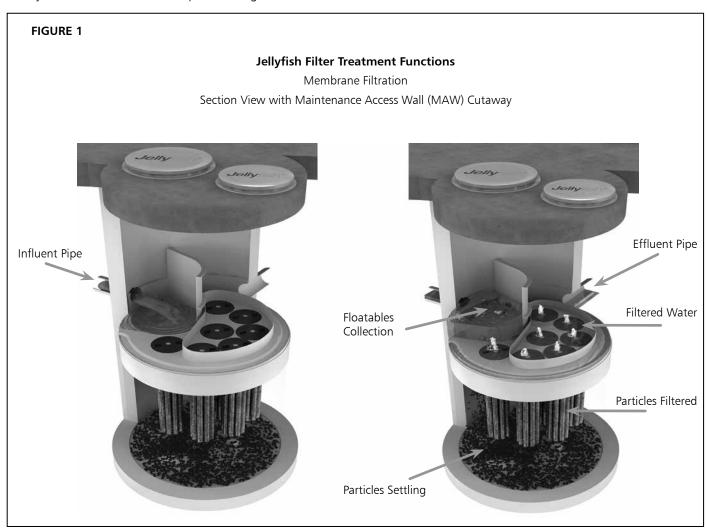
Owner Name:	
Phone Number:	
Site Address:	
Site GPS Coordinates/unit location:	
Unit Location Description:	
Jellyfish Filter Model No.:	
Contech Project & Sequence Number	
No. of Hi-Flo Cartridges	
No. of Cartridges:	
Length of Draindown Cartridges:	
No. of Blank Cartridge Lids:	
Bypass Configuration (Online/Offline):	
Notes:	

Chapter 2

2.0 - Jellyfish Filter System Operations and Functions

The Jellyfish Filter is an engineered stormwater quality treatment technology that removes a high level and wide variety of stormwater pollutants. Each Jellyfish Filter cartridge consists of eleven membrane - encased filter elements ("filtration tentacles") attached to a cartridge head plate. The filtration tentacles provide a large filtration surface area, resulting in high flow and high pollutant removal capacity.

The Jellyfish Filter functions are depicted in Figure 1 below.

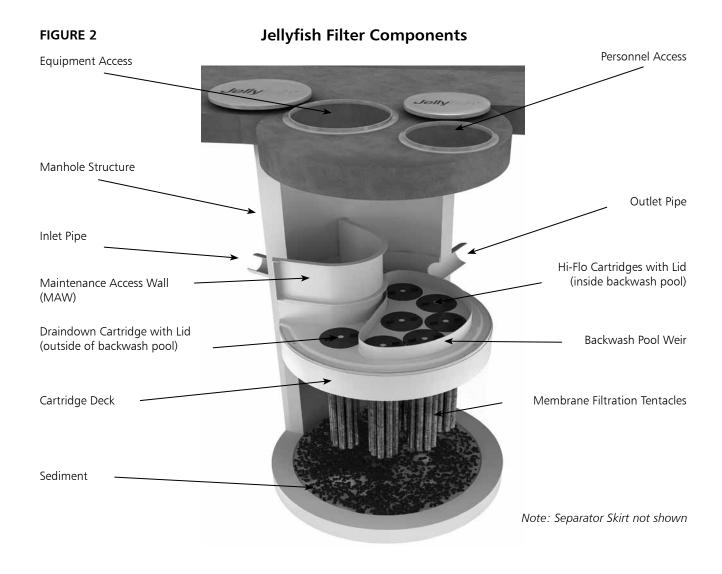


Jellyfish Filter cartridges are backwashed after each peak storm event, which removes accumulated sediment from the membranes. This backwash process extends the service life of the cartridges and increases the time between maintenance events.

For additional details on the operation and pollutant capabilities of the Jellyfish Filter please refer to additional details on our website at www.ContechES.com.

2.1 - Components and Cartridges

The Jellyfish Filter and components are depicted in Figure 2 below.



Tentacles are available in various lengths as depicted in Table 1 below.

Table 1 – Cartridge Lengths / Weights and Cartridge Lid Orifice Diameters

Cartridge Lengths	Dry Weight	Hi-Flo Orifice Diameter	Draindown Orifice Diameter
15 inches (381 mm)	10 lbs (4.5 kg)	35 mm	20 mm
27 inches (686 mm)	14.5 lbs (6.6 kg)	45 mm	25 mm
40 inches (1,016 mm)	19.5 lbs (8.9 kg)	55 mm	30 mm
54 inches (1,372 mm)	25 lbs (11.4 kg)	70 mm	35 mm

2.2 - Jellyfish Membrane Filtration Cartridge Assembly

The Jellyfish Filter utilizes multiple membrane filtration cartridges. Each cartridge consists of removable cylindrical filtration "tentacles" attached to a cartridge head plate. Each filtration tentacle has a threaded pipe nipple and o-ring. To attach, insert the top pipe nipples with the o-ring through the head plate holes and secure with locking nuts. Hex nuts to be hand tightened and checked with a wrench as shown below.

2.3 – Jellyfish Membrane Filtration Cartridge Installation

- Cartridge installation will be performed by trained individuals and coordinated with the installing site Contractor. Flow diversion devices are required to be in place until the site is stabilized (final paving and landscaping in place). Failure to address this step completely will reduce the time between required maintenance.
- Descend to the cartridge deck (see Safety Notice and page 3).
- Refer to Contech's submittal drawings to determine proper quantity and placement of Hi-Flo, Draindown and Blank cartridges with appropriate lids. Lower the Jellyfish membrane filtration cartridges into the cartridge receptacles within the cartridge deck. It is possible that not all cartridge receptacles will be filled with a filter cartridge. In that case, a blank headplate and blank cartridge lid (no orifice) would be installed.



Cartridge Assembly

Do not force the tentacles down into the cartridge receptacle, as this may damage the membranes. Apply downward pressure on the cartridge head plate to seat the lubricated rim gasket (thick circular gasket surrounding the circumference of the head plate) into the cartridge receptacle. (See Figure 3 for details on approved lubricants for use with rim gasket.)

- Examine the cartridge lids to differentiate lids with a small orifice, a large orifice, and no orifice.
 - Lids with a <u>small orifice</u> are to be inserted into the <u>Draindown cartridge receptacles</u>, outside of the backwash pool weir.
 - Lids with a <u>large orifice</u> are to be inserted into the <u>Hi-Flo cartridge receptacles</u> within the backwash pool weir.
 - Lids with <u>no orifice</u> (blank cartridge lids) and a <u>blank headplate</u> are to be inserted into unoccupied cartridge receptacles.
- To install a cartridge lid, align both cartridge lid male threads with the cartridge receptacle female threads before rotating approximately 1/3 of a full rotation until firmly seated. Use of an approved rim gasket lubricant may facilitate installation.

3.0 Inspection and Maintenance Overview

The primary purpose of the Jellyfish® Filter is to capture and remove pollutants from stormwater runoff. As with any filtration system, these pollutants must be removed to maintain the filter's maximum treatment performance. Regular inspection and maintenance are required to insure proper functioning of the system.

Maintenance frequencies and requirements are site specific and vary depending on pollutant loading. Additional maintenance activities may be required in the event of non-storm event runoff, such as base-flow or seasonal flow, an upstream chemical spill or due to excessive sediment loading from site erosion or extreme runoff events. It is a good practice to inspect the system after major storm events.

Inspection activities are typically conducted from surface observations and include:

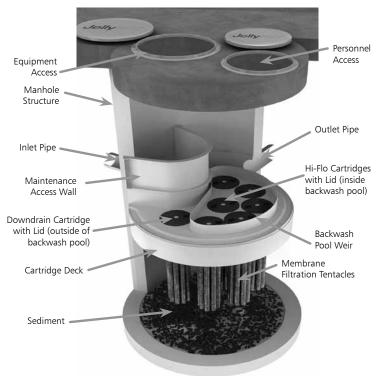
- Observe if standing water is present
- Observe if there is any physical damage to the deck or cartridge lids
- Observe the amount of debris in the Maintenance Access Wall (MAW) or inlet bay for vault systems

Maintenance activities include:

- Removal of oil, floatable trash and debris
- Removal of collected sediments
- Rinsing and re-installing the filter cartridges
- Replace filter cartridge tentacles, as needed

4.0 Inspection Timing

Inspection of the Jellyfish Filter is key in determining the maintenance requirements for, and to develop a history of, the site's pollutant loading characteristics. In general, inspections should be performed at the times indicated below; or per the approved project stormwater quality documents (if applicable), whichever is more frequent.



Note: Separator Skirt not shown

- A minimum of quarterly inspections during the first year of operation to assess the sediment and floatable pollutant accumulation, and to ensure proper functioning of the system.
- 2. Inspection frequency in subsequent years is based on the inspection and maintenance plan developed in the first year of operation. Minimum frequency should be once per year.
- 3. Inspection is recommended after each major storm event.
- Inspection is required immediately after an upstream oil, fuel or other chemical spill.

5.0 Inspection Procedure

The following procedure is recommended when performing inspections:

- 1. Provide traffic control measures as necessary.
- 2. Inspect the MAW or inlet bay for floatable pollutants such as trash, debris, and oil sheen.
- Measure oil and sediment depth in several locations, by lowering a sediment probe until contact is made with the floor of the structure. Record sediment depth, and presences of any oil layers.
- Inspect cartridge lids. Missing or damaged cartridge lids to be replaced.
- Inspect the MAW (where appropriate), cartridge deck and receptacles, and backwash pool weir, for damaged or broken components.

5.1 Dry weather inspections

- Inspect the cartridge deck for standing water, and/or sediment on the deck.
- No standing water under normal operating conditions.
- Standing water inside the backwash pool, but not outside the backwash pool indicates, that the filter cartridges need to be rinsed.





Inspection Utilizing Sediment Probe

- Standing water outside the backwash pool is not anticipated and may indicate a backwater condition caused by high water elevation in the receiving water body, or possibly a blockage in downstream infrastructure.
- Any appreciable sediment (≥1/16") accumulated on the deck surface should be removed.

5.2 Wet weather inspections

- Observe the rate and movement of water in the unit.
 Note the depth of water above deck elevation within the MAW or inlet bay.
- Less than 6 inches, flow should be exiting the cartridge lids of each of the draindown cartridges (i.e. cartridges located outside the backwash pool).
- Greater than 6 inches, flow should be exiting the cartridge lids of each of the draindown cartridges and each of the hi-flo cartridges (i.e. cartridges located inside the backwash pool), and water should be overflowing the backwash pool weir.
- 18 inches or greater and relatively little flow is exiting the cartridge lids and outlet pipe, this condition indicates that the filter cartridges need to be rinsed.

6.0 Maintenance Requirements

Required maintenance for the Jellyfish Filter is based upon results of the most recent inspection, historical maintenance records, or the site specific water quality management plan; whichever is more frequent. In general, maintenance requires some combination of the following:

- Sediment removal for depths reaching 12 inches or greater, or within 3 years of the most recent sediment cleaning, whichever occurs sooner.
- 2. Floatable trash, debris, and oil removal.
- 3. Deck cleaned and free from sediment.
- 4. Filter cartridges rinsed and re-installed as required by the most recent inspection results, or within 12 months of the most recent filter rinsing, whichever occurs sooner.
- Replace tentacles if rinsing does not restore adequate hydraulic capacity, remove accumulated sediment, or if damaged or missing. It is recommended that tentacles should remain in service no longer than 5 years before replacement.
- Damaged or missing cartridge deck components must be repaired or replaced as indicated by results of the most recent inspection.
- The unit must be cleaned out and filter cartridges inspected immediately after an upstream oil, fuel, or chemical spill.
 Filter cartridge tentacles should be replaced if damaged or compromised by the spill.

7.0 Maintenance Procedure

The following procedures are recommended when maintaining the Jellyfish Filter:

- Provide traffic control measures as necessary.
- Open all covers and hatches. Use ventilation equipment as required, according to confined space entry procedures. Caution: Dropping objects onto the cartridge deck may cause damage.
- 3. Perform Inspection Procedure prior to maintenance activity.

- 4. To access the cartridge deck for filter cartridge service, descend into the structure and step directly onto the deck. Caution: Do not step onto the maintenance access wall (MAW) or backwash pool weir, as damage may result. Note that the cartridge deck may be slippery.
- 5. Maximum weight of maintenance crew and equipment on the cartridge deck not to exceed 450 lbs.

7.1 Filter Cartridge Removal

- 1. Remove a cartridge lid.
- Remove cartridges from the deck using the lifting loops in the cartridge head plate. Rope or a lifting device (available from Contech) should be used. Caution: Should a snag occur, do not force the cartridge upward as damage to the tentacles may result. Wet cartridges typically weigh between 100 and 125 lbs.
- 3. Replace and secure the cartridge lid on the exposed empty receptacle as a safety precaution. Contech does not recommend exposing more than one empty cartridge receptacle at a time.

7.2 Filter Cartridge Rinsing

- Remove all 11 tentacles from the cartridge head plate. Take care not to lose or damage the O-ring seal as well as the plastic threaded nut and connector.
- 2. Position tentacles in a container (or over the MAW), with the



threaded connector (open end) facing down, so rinse water is flushed through the membrane and captured in the container.

3. Using the Jellyfish rinse tool (available from Contech) or a low-pressure garden hose sprayer, direct water spray onto the tentacle membrane, sweeping from top to bottom along the length of the tentacle. Rinse until all sediment is removed from the membrane. Caution: Do not use a high pressure sprayer or focused stream of water on the membrane. Excessive water pressure may damage the membrane.

5. Reassemble cartridges as detailed later in this document. Reuse O-rings and nuts, ensuring proper placement on each tentacle.

7.3 Sediment and Flotables Extraction

- 1. Perform vacuum cleaning of the Jellyfish Filter only after filter cartridges have been removed from the system. Access the lower chamber for vacuum cleaning only through the maintenance access wall (MAW) opening. Be careful not to damage the flexible plastic separator skirt that is attached to the underside of the deck on manhole systems. Do not lower the vacuum wand through a cartridge receptacle, as damage to the receptacle will result.
- Vacuum floatable trash, debris, and oil, from the MAW opening or inlet bay. Alternatively, floatable solids may be removed by a net or skimmer.
- 3. Pressure wash cartridge deck and receptacles to remove all



Rinsing Cartridge with Contech Rinse Tool

sediment and debris. Sediment should be rinsed into the sump area. Take care not to flush rinse water into the outlet pipe.

- Remove water from the sump area. Vacuum or pump equipment should only be introduced through the MAW or inlet bay.
- 5. Remove the sediment from the bottom of the unit through the MAW or inlet bay opening.
- 6. For larger diameter Jellyfish Filter manholes (≥8-ft) and some



Vacuuming Sump Through MAW

vaults complete sediment removal may be facilitated by removing a cartridge lid from an empty receptacle and inserting a jetting wand (not a vacuum wand) through the receptacle. Use the sprayer to rinse loosened sediment toward the vacuum hose in the MAW opening, being careful not to damage the receptacle.

7.4 Filter Cartridge Reinstallation and Replacement

- Cartridges should be installed after the deck has been cleaned.
 It is important that the receptacle surfaces be free from grit and debris.
- 2. Remove cartridge lid from deck and carefully lower the filter cartridge into the receptacle until head plate gasket is seated squarely in receptacle. Caution: Do not force the cartridge downward; damage may occur.
- Replace the cartridge lid and check to see that both male threads are properly seated before rotating approximately 1/3 of a full rotation until firmly seated. Use of an approved rim gasket lubricant may facilitate installation. See next page for additional details.
- 4. If rinsing is ineffective in removing sediment from the tentacles, or if tentacles are damaged, provisions must be made to replace the spent or damaged tentacles with new tentacles. Contact Contech to order replacement tentacles.

7.5 Chemical Spills

Caution: If a chemical spill has been captured, do not attempt maintenance. Immediately contact the local hazard response agency and contact Contech.

7.6 Material Disposal

The accumulated sediment found in stormwater treatment and conveyance systems must be handled and disposed of in accordance with regulatory protocols. It is possible for sediments to contain measurable concentrations of heavy metals and organic chemicals (such as pesticides and petroleum products). Areas with the greatest potential for high pollutant loading include industrial areas and heavily traveled roads. Sediments and water must be disposed of in accordance with all applicable waste disposal regulations. When scheduling maintenance, consideration must be made for the disposal of solid and liquid wastes. This typically requires coordination with a local landfill for solid waste disposal. For liquid waste disposal a number of options are available including a municipal vacuum truck decant facility, local waste water treatment plant or on-site treatment and discharge.

Jellyfish Filter Components & Filter Cartridge Assembly and Installation

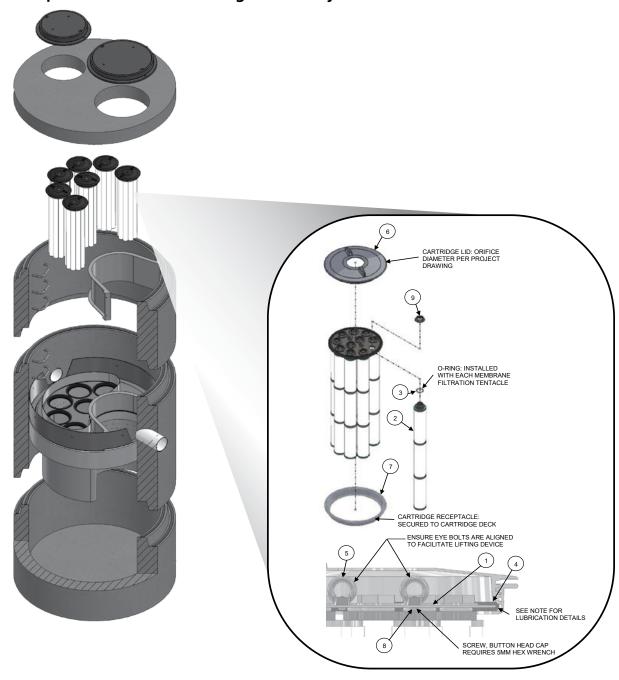


TABLE 1: BOM

TABLE I. DOM				
ITEM NO.	DESCRIPTION			
1	JF HEAD PLATE			
2	JF TENTACLE			
3	JF O-RING			
4	JF HEAD PLATE GASKET			
5	JF CARTRIDGE EYELET			
6	JF 14IN COVER			
7	JF RECEPTACLE			
	BUTTON HEAD CAP			
8	SCREW M6X14MM SS			
9	JF CARTRIDGE NUT			

TABLE 2: APPROVED GASKET LUBRICANTS

PART NO.	MFR	DESCRIPTION
78713	LA-CO	LUBRI-JOINT
40501	HERCULES	DUCK BUTTER
30600	OATEY	PIPE LUBRICANT
PSI UBXI 10	PROSELECT	PIPE JOINT LUBRICANT

NOTES:

Head Plate Gasket Installation:

Install Head Plate Gasket (Item 4) onto the Head Plate (Item 1) and liberally apply a lubricant from Table 2: Approved Gasket Lubricants onto the gasket where it contacts the Receptacle (Item 7) and Cartridge Lid (Item 6). Follow Lubricant manufacturer's instructions.

Lid Assembly:

Rotate Cartridge Lid counter-clockwise until both male threads drop down and properly seat. Then rotate Cartridge Lid clock-wise approximately one-third of a full rotation until Cartridge Lid is firmly secured, creating a watertight seal.

Jellyfish Filter Inspection and Maintenance Log

Owner:			Jellyfish Model No.:			_
Location:			GPS Coordinate	es:		_
Land Use:	Commercial:	Industrial:	Servic	e Station:		
	Road/Highway:	Airport:	Reside	ential:	Parking L	ot:
Date/Time:						
Inspector:						
Maintenance	Contractor:					
Visible Oil Pre	esent: (Y/N)					
Oil Quantity F	Removed					
Floatable Deb	oris Present: (Y/N)					
Floatable Deb	oris removed: (Y/N)					
Water Depth	in Backwash Pool					
Cartridges ext	ternally rinsed/re-commission	oned: (Y/N)				
New tentacles	s put on Cartridges: (Y/N)					
Sediment Dep	oth Measured: (Y/N)					
Sediment Dep	oth (inches or mm):					
Sediment Ren	moved: (Y/N)					
Cartridge Lids	s intact: (Y/N)					
Observed Dar	mage:					
Comments:						

Maintenance Levels				
Model	Oil	Sediment		
Number	Depth	Depth		
SWAQ-05	12"	12"		
SWAQ-10	12"	12"		
SWAQ-20	12"	12"		
SWAQ-25	12"	12"		
SWAQ-40	12"	12"		
SWAQ-70	12"	12"		
SWAQ-110	12"	12"		

It is very useful to keep a record of each inspection.

<u>Inspection Procedures</u>

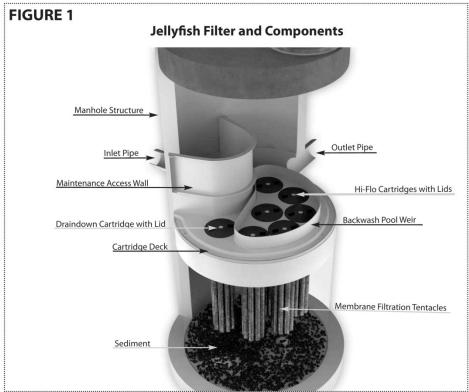
- 1. Easiest observation and maintenance is best accomplished during non-flow (dry weather) conditions 3-4 days after the most recent rain.
- 2. Remove interceptor covers or open hatchway to observe conditions. Remove hatchway safety net ("EnterNet"). Observe for trash and debris and remove if necessary. This is the most important maintenance requirement. If absorbent pillows are utilized, observe their condition. Uniform browning or gray color of the pillow means they should be replaced. Observe baffle debris screen and clean if necessary.
- 3. Coalescing plates are self-cleaning and seldom require maintenance unless damaged. Do not walk on or stand on plate packs. Call ParkUSA (888-611-PARK) for replacement parts.
- 4. Check of the depth (level) of oil and sediment with a tank sampler device designed for this purpose.

3.2.22 Jellyfish® Filter

The Jellyfish® Filter is an engineered stormwater quality treatment technology featuring unique membrane filtration in a compact stand-alone treatment system that removes a wide variety of stormwater pollutants. The Jellyfish® Filter integrates pre-treatment and filtration with passive self-cleaning mechanisms. The system utilizes membrane filtration cartridges with very high filtration surface area and flow capacity, which provide the advantages of high sediment capacity and low filtration flux rate (flow per unit surface area) at relatively low driving head compared to conventional filter systems.

Each lightweight Jellyfish® Filter cartridge consists of multiple detachable membrane-encased filter elements ("filtration tentacles") attached to a cartridge head plate. The Jellyfish® Filter and components are depicted in Figure 1 (6-ft diameter system shown).

New Innovative Technology



Note: Separator Skirt Not Shown

The Jellyfish® Filter can be used as a stand-alone device to treat stormwater or in a treatment train with other BMPs. Field testing of the Jellyfish® Filter has demonstrated capture of high levels of stormwater pollutants, including:

- 86% of the total suspended solids (TSS) load, including particles less than 5 microns;
- Some Phosphorus Nitrogen; and
- Metals, and additional particulate-bound pollutants such as hydrocarbons, and bacteria, free oil and floatable trash and debris.

Selection Criteria

- Use when space constraints make installation of a surface treatment system infeasible
- Appropriate for space-limited areas
- Appropriate for various size drainage basins
- Requires a minimal amount of land since underground
- Appropriate for retrofits and new development
- Appropriate to combine with low impact development (LID) applications and Green Infrastructure

Limitations

- Typically requires 18 inches of drop across the system (can be as low as 9 inches)
- Requires regular (minimum annually) inspection and/or maintenance

Cost Considerations

Cost of the Jellyfish® Filter is generally equal to or less than that of a sand filter and other granular media-filled cartridge systems, particularly when installation and maintenance costs are included.

Performance Claim

"The Jellyfish® Filter with standard membrane filtration cartridges designed for a maximum treatment flow rate consistent with a filtration flux rate (flow per unit surface area) of 0.21 gpm/ft2 (0.14 Lps/m2) for the hi-flo cartridge and 0.11 gpm/ft2 (0.07 Lps/m2) for the draindown cartridge, demonstrated removal of 86% of TSS, 99% of SSC, 59% of Total Phosphorus, 51% of Total Nitrogen, and greater than 50% of Total Copper and Total Zinc from urban rainfall-runoff, based on median pollutant removal efficiencies developed from the TARP and VTAP field monitoring study with a duration from 28 May 2010 through 27 June 2011."

3.4.20 <u>Design Criteria</u>

Design Rainfall Depth – The design rainfall depth is dependent on the characteristics of the contributing drainage area. The method for calculation of the fraction of annual rainfall to be treated and the design rainfall depth is specified in Section 3.3 of this manual.

Standard length (54 inches) Jellyfish membrane filtration cartridges have a design treatment flow rate of 80 gpm for the hi-flo cartridge and 40 gpm for the draindown cartridge.

A high-flow bypass located upstream of the Jellyfish® Filter is recommended to divert flows in excess of the design storm around the filtration system. A weir 18 inches higher than the outlet pipe invert or deck elevation of the Jellyfish is typically installed in the diversion structure to provide 18 inches of driving head to the treatment unit. In-line systems are also available. Table 1 shows standard manhole configurations and flow rates. Rectangular catch basin models with top inlet or curb inlets are available for small drainage areas as well as large rectangular vaults for drainage areas that exceed the capacity of largest standard manhole model listed. Standard cartridges lengths are 54 inches, 40 inches, 27 inches and 15 inches.

Table 1 Design Flow Capacities of the Jellyfish Filter

Table 1 Design Flow Capacities Standard Jellyfish Filter Manhole Configurations					
Manhole Diameter (ft/m)¹	Model No.	Hi-Flo Cartridges ² 54 in / 1372 mm	Draindown Cartridges² 54 in / 1372 mm	Treatment Flow Rate (gpm / cfs)	Treatment Flow Rate (L/S)
4 / 1.2	JF4-2-1	2	1	200 / 0.45	12.6
6 / 1.8	JF6-3-1	3	1	280 / 0.62	17.7
	JF6-4-1	4	1	360 / 0.80	22.7
	JF6-5-1	5	1	440 / 0.98	27.8
	JF6-6-1	6	1	520 / 1.16	32.8
8/2.4	JF8-6-2	6	2	560 / 1.25	35.3
	JF8-7-2	7	2	640 / 1.43	40.4
	JF8-8-2	8	2	720 / 1.60	45.
	JF8-9-2	9	2	800 / 1.78	50.5
	JF8-10-2	10	2	880 / 1.96	55.5
10/3.0	JF10-11-3	11	3	1000 / 2.23	63.1
	JF10-12-3	12	3	1080 / 2.41	68.1
	JF10-12-4	12	4	1120 / 2.50	70.7
	JF10-13-4	13	4	1200 / 2.67	75.7
	JF10-14-4	14	4	1280 / 2.85	80.8
	JF10-15-4	15	4	1360 / 3.03	85.8
	JF10-16-4	16	4	1440 / 3.21	90.8
	JF10-17-4	17	4	1520 / 3.39	95.9
	JF10-18-4	18	4	1600/3.56	100.9
	JF10-19-4	19	4	1680 / 3.74	106
12 / 3.6	JF12-20-5	20	5	1800 / 4.01	113.6
	JF12-21-5	21	5	1880 / 4.19	118.6
	JF12-22-5	22	5	1960 / 4.37	123.7
	JF12-23-5	23	5	2040 / 4.54	128.7
	JF12-24-5	24	5	2120 / 4.72	133.8
	JF12-25-5	25	5	2200 / 4.90	138.8
	JF12-26-5	26	5	2280 / 5.08	143.8
	JF12-27-5	27	5	2360 / 5.26	148.9

¹Smaller and larger systems may be custom designed

Jellyfish Cartridges and Membrane Properties

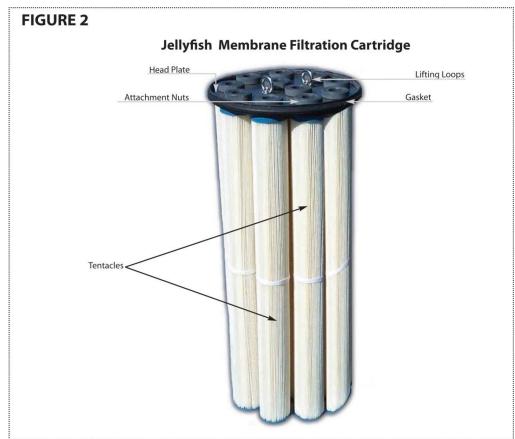
The filtration tentacle membranes provide a large amount of surface area, resulting in superior flow capacity and suspended sediment removal capacity. A typical Jellyfish cartridge with eleven 54-inch (1372 mm) long filtration tentacles has 381 ft2 (35.4 m2) of membrane surface area. Hydraulic testing on a clean 54-inch (1372 mm) filter cartridge has demonstrated a flow rate of 180 gpm (11.3 L/s) at 18 inches (457 mm) of driving head. In addition, the filtration tentacle membrane has anti-microbial characteristics that inhibit the growth of bio-film that might otherwise prematurely occlude the pores of the membrane and restrict hydraulic conductivity.

The cylindrical membrane filtration tentacle has a threaded pipe nipple at the top and is

² Shorter length cartridge configurations are available

sealed at the bottom with an end cap. A cluster of tentacles is attached to a head plate by inserting the top pipe nipples through the head plate holes and securing with removable nuts. A removable oil-resistant polymeric rim gasket is attached to the head plate to impart a watertight seal when the cartridge is secured into the cartridge receptacle with the cartridge lid. A Jellyfish membrane filtration cartridge is depicted in Figure 2.

The dry weight of a new cartridge is less than 20 pounds (9 kg), and the wet weight of a used cartridge is less than 50 pounds (23 kg), making a cartridge easy to install and remove by hand. No heavy lifting equipment is required.



3.5.25 <u>Jellyfish® Filter Inspection and Maintenance</u>

Jellyfish cartridges are passively backwashed automatically after each storm event, which removes accumulated sediment from the membranes and significantly extends the service life of the cartridges and the maintenance interval. If required, the cartridges can be easily manually backwashed without removing the cartridges. Additionally, the lightweight cartridges can be removed by hand and externally rinsed, and rinsed cartridges then re-installed. These simple maintenance options allow for cartridge regeneration, thereby minimizing cartridge replacement costs and life-cycle treatment costs while ensuring long-term treatment performance.

Regular inspection and maintenance are proven, cost-effective ways to maximize water resource protection for all stormwater pollution control practices, and are required to insure proper functioning of the Jellyfish® Filter. Inspection of the Jellyfish® Filter is performed from the surface, while proper maintenance requires a combination of

procedures conducted from the surface and with worker entry into the structure.

Please refer to the following information and guidelines before conducting inspection and maintenance activities:

When is inspection needed?

Post-construction inspection is required prior to putting the Jellyfish Filter into service.

Routine inspections are recommended quarterly during the first year of operation to accurately assess the sediment and floatable pollutant accumulation, and to ensure that the automatic backwash feature is functioning properly.

Inspection frequency in subsequent years is based on the maintenance plan developed in the first year, but must occur annually at a minimum.

Inspections should also be performed immediately after oil, fuel or other chemical spill.

When is maintenance service needed?

The unit must be cleaned annually. This cleaning includes removal and appropriate disposal of all water, sediment, oil and grease, and debris that has accumulated within the unit. The Jellyfish Filter is inspected and maintained by professional vacuum cleaning service providers with experience in the maintenance of underground tanks, sewers and catch basins. Since some of the maintenance procedures require manned entry into the Jellyfish structure, only professional maintenance service providers trained in confined space entry procedures should enter the vessel. Service provider companies typically have personnel who are trained and certified in confined space entry procedures according to local, state, and federal standards.

Filter cartridges should be tested for adequate flow rate, every 12 months and cleaned and re-commissioned, or replaced if necessary. A manual backflush must be performed on a single draindown cartridge using a Jellyfish Cartridge Backflush Pipe (described in the Jellyfish® Filter Owner's Manual). If the time required to drain 14 gallons of backflush water from the Backflush Pipe (from top of pipe to the top of the open flapper valve) exceeds 15 seconds, it is recommended to perform a manual backflush on each of the cartridges. After the manual backflush, the draindown test should be repeated on a single cartridge to determine if the cartridge can drain 14 gallons of water in 15 seconds. If the cartridge still does not achieve the design flow rate, it must be replaced.

The unit should be cleaned out immediately after an oil, fuel or chemical spill.

External Rinsing

This cartridge cleaning procedure is performed by removing the cartridge from the cartridge deck and externally rinsing the filtration tentacles using a low-pressure water sprayer, as described in the Jellyfish® Filter Owner's Manual. If this procedure is performed within the structure, the cartridge or individual filtration tentacles should be rinsed while safely suspended over the maintenance access wall opening in the cartridge deck, such that rinsate flows into the lower chamber of the Jellyfish® Filter. If the rinsing procedure is performed outside the structure, the cartridge or individual filtration tentacles should be rinsed in a suitable basin such as a plastic barrel or tub, and rinsate subsequently poured into the maintenance access wall opening in the cartridge deck. Sediment is subsequently removed from the lower chamber by standard vacuum service.

Inspection / Maintenance Completion - Summary												
Company Name:												
Company Address:												
City/State/Zip:												
Phone:												
Engineer:												
Engineers Address:												
City/State/Zi	City/State/Zip:											
Phone:												
Property Owner:												
*Jellyfish Mo	del											
Monitoring /	' Maint	enance	e Table	<u> </u>								
	Jan	Feb	Mar	Apr	May	Jun	July	Aug	Sep	Oct	Nov	Dec
Oil Depth (inches)												
Sediment Depth (inches)												
Completed By												
Date												
Floatables (optional)												
I hereby cert completed in plan.												ance
					(9	Signed	by pro	perty	owner	or des	ignee)	

Langford Stuber

Temporary Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

	uifer. This Temporary Stormwater Section is hereby submitted for TCEQ review and ecutive director approval. The application was prepared by:
Pri	nt Name of Customer/Agent: LANGFORD STUBER
Da	te: <u>5/25/20</u> 23
Sig	nature of Customer/Agent:
<u></u>	angford Stuber
	gulated Entity Name: Chalk Hill Ranch
Pi	roject Information
P	otential Sources of Contamination
	amples: Fuel storage and use, chemical storage and use, use of asphaltic products, nstruction vehicles tracking onto public roads, and existing solid waste.
1.	Fuels for construction equipment and hazardous substances which will be used during construction:
	☐ The following fuels and/or hazardous substances will be stored on the site:
	These fuels and/or hazardous substances will be stored in:
	Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

- Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year.
 Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
- X Fuels and hazardous substances will not be stored on the site.
- 2. X Attachment A Spill Response Actions. A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
- 3. X Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
- 4. X Attachment B Potential Sources of Contamination. A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.

Sequence of Construction

- 5. X Attachment C Sequence of Major Activities. A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
 - X For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given.
 - X For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
- 6. X Name the receiving water(s) at or near the site which will be disturbed or which will receive discharges from disturbed areas of the project: South Fork San Gabriel River

Temporary Best Management Practices (TBMPs)

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

7. X Attachment D – Temporary Best Management Practices and Measures. TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

	X A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.
	X A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.
	X A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer.
	X A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.
8. 🗹	The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
	Attachment E - Request to Temporarily Seal a Feature. A request to temporarily seal a feature is attached. The request includes justification as to why no reasonable and practicable alternative exists for each feature.
	X There will be no temporary sealing of naturally-occurring sensitive features on the site.
9. X	Attachment F - Structural Practices . A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.
10. X	Attachment G - Drainage Area Map . A drainage area map supporting the following requirements is attached:
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided.
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used.
	X For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area.
	There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.

- ☐ There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. Erosion and sediment controls other than sediment basins or sediment traps within each disturbed drainage area will be used.
 11. ☐ Attachment H Temporary Sediment Pond(s) Plans and Calculations. Temporary sediment pond or basin construction plans and design calculations for a proposed temporary BMP or measure have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information must be signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed temporary BMPs and measures are attached.
 X N/A
- 12. Attachment I Inspection and Maintenance for BMPs. A plan for the inspection of each temporary BMP(s) and measure(s) and for their timely maintenance, repairs, and, if necessary, retrofit is attached. A description of the documentation procedures, recordkeeping practices, and inspection frequency are included in the plan and are specific to the site and/or BMP.
- 13. X All control measures must be properly selected, installed, and maintained in accordance with the manufacturer's specifications and good engineering practices. If periodic inspections by the applicant or the executive director, or other information indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations.
- 14. X If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain).
- 15. X Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50%. A permanent stake will be provided that can indicate when the sediment occupies 50% of the basin volume.
- 16. X Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source for stormwater discharges (e.g., screening outfalls, picked up daily).

Soil Stabilization Practices

Examples: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

17. X Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices. A schedule of the interim and permanent soil stabilization practices for the site is attached.

- 18. X Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 19. X Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

Administrative Information

- 20. X All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
- 21. X If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
- 22. X Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

ATTACHMENT A SPILL RESPONSE ACTIONS



General

- An effective spill and leak response depends on proper recognition and response practices by construction workers and supervisors. Key elements are education and training.
- Records of releases that exceed the Reportable Quantity (RQ) for oil and hazardous substances should be maintained in accordance with the Federal and State regulations.
- Emergency contact information and spill response procedures shall be posted in a readily available area for access by all employees and subcontractors.
- Spill containment kits should be maintained for petroleum products and other chemicals that are regularly onsite. Materials in kits should be based on containment guidelines in the Material Safety and Data Sheets (MSDSs) for the substance most frequently onsite.
- Spill kits are intended for response to small spills, typically less than 5 gallons, of substances that are not extremely hazardous.
- Significant spills or other releases warrant immediate response by trained professionals.
- Suspected job-site contamination should be immediately reported to regulatory authorities and protective actions taken.

Coordinator

- The contractor should be required to designate a site superintendent, foreman, safety officer, or other senior person who is onsite daily to be the Spill and Leak Response Coordinator.
- The coordinator must have knowledge of and be trained in correct spill and leak response procedures.
- The coordinator shall be responsible for implementing the spill and leak procedures and training all employees and sub-contractors on the site-specific spill and leak procedures. The training should include their responsibility to immediately notify the coordinator if a spill or leak occurs.

Spill Response

- Upon discovery of a spill, employees and subcontractors shall implement the following procedures:
 - o Immediately stop work and clear the area by moving upwind of the spill.
 - Remove all ignition sources.
 - Notify the Spill and Leak Response Coordinator.
 - If there is an immediate danger to health or life, contact 911.
- The Spill and Leak Response Coordinator shall perform the following when the spill is not immediately dangerous to health and safety:
 - Consult the MSDS for safety and response procedures.
 - If it can be done safely, use onsite spill kits and soil to contain the spill.
 - Notify a hazardous response company to remove and properly dispose of the spilled material and the contaminated containment materials.

Spill Reporting

- The Spill and Leak Response Coordinator is responsible for notifying authorities of spills and leaks. Notification requirements are based on Reportable Quantities as established by the type or material, quantity and location (onto land or into water in the state) of the release. https://www.tceq.texas.gov/response/spills/spill_rq.html
- Reportable Quantities (RQ) in the State of Texas are established by the TCEQ in Texas Administrative Code Title 30, Chapter 327 (30 TAC 327) Spill Prevention and Control.
- The Texas RQ for petroleum products and used oil is 25 gallons released onto land or any amount that causes sheen on water.
- Reportable Quantities for all other substances are listed in 30 TAC 327.4, which references the EPA List of Lists (EPA 550-B-01-003) available at: http://www.epa.gov/ceppo/pubs/title3.pdf
- The Spill and Leak Response Coordinator shall notify the following:
 - The municipality that operates the local Municipal Separate Storm Sewer System (MS4) if a spill or leak enters public rights-of-way or any type of drainage way or drainage infrastructure within the jurisdiction of the municipality.
 - State of Texas Spill Report Hotline at 1-800-832-8224 if the spill or leak exceeds the RQ.
 - National Spill Response Center at 1-800-424-8802 if the spill or leak exceeds the RQ.

ATTACHMENT B POTENTIAL SOURCES OF CONTAMINATION



Pollutant	Activity	Response		
Fuels, grease, and oils	Trucks, generators, machinery	Secondary containment will be		
	used during construction	established around all above-		
	activity	ground storage tanks. Any		
		emergency maintenance will		
		utilize drip pans, and no		
		scheduled maintenance will		
		occur onsite.		
Sediment	General construction activity	Sediment and erosion control		
		measures will be established		
		and operating prior to any soil		
		disturbance		
Sanitary/septic systems	General construction activity	Portable toilets will be located		
		in designated sites within the		
		construction site. Licensed		
		sanitary sewer services will		
		ensure facilities are in working		
		order at all times.		

ATTACHMENT C SEQUENCE OF CONSTRUCTION



- 1. Install Construction Entrance/Exit per Erosion Control Plans in the Construction Drawings.
- 2. Install tree protection (as needed).
- 3. Begin silt fence installation.
- 4. Strip topsoil and organic materials and store onsite in stockpiles for later use. All stockpiles shall be temporarily seeded and mulched to prevent loss due to erosion and encircled with a silt fence or filter tube. (41 acres)
- 5. Begin site grading.
- 6. Install utilities and underground stormwater system.
- 7. As an area reaches final grade, install permanent stabilization as soon as possible but in no case longer than 14 days from reaching final grade.
- 8. Any area that will remain undisturbed for 14 days or more shall be temporarily stabilized.
- 9. Construct buildings.
- 10. Finish grading of all areas.
- 11. Complete permanent stabilization of all disturbed areas. Once a minimum of 70 percent of the vegetated areas have been stabilized, remove the erosion control BMPs.

ATTACHMENT D TEMPORARY BEST MANAGEMENT PRACTICES AND MEASURES



The following temporary BMPs have been identified on the Erosion Control Plan and the Water Quality Plan in Attachment M – Construction Plans.

- 1. Silt Fence Protects downstream property from sediment due to storm events.
- 2. Stabilized Construction Exit Minimizes sediment and aggregate from exiting the project site from equipment trucks and construction vehicles.
- 3. Concrete Washout Pit To capture the concrete and debris from concrete vehicles and equipment.
- 4. Inlet Protection To capture sedimentation and debris from entering the underground storm system.

ATTACHMENT F STRUCTURAL PRACTICES

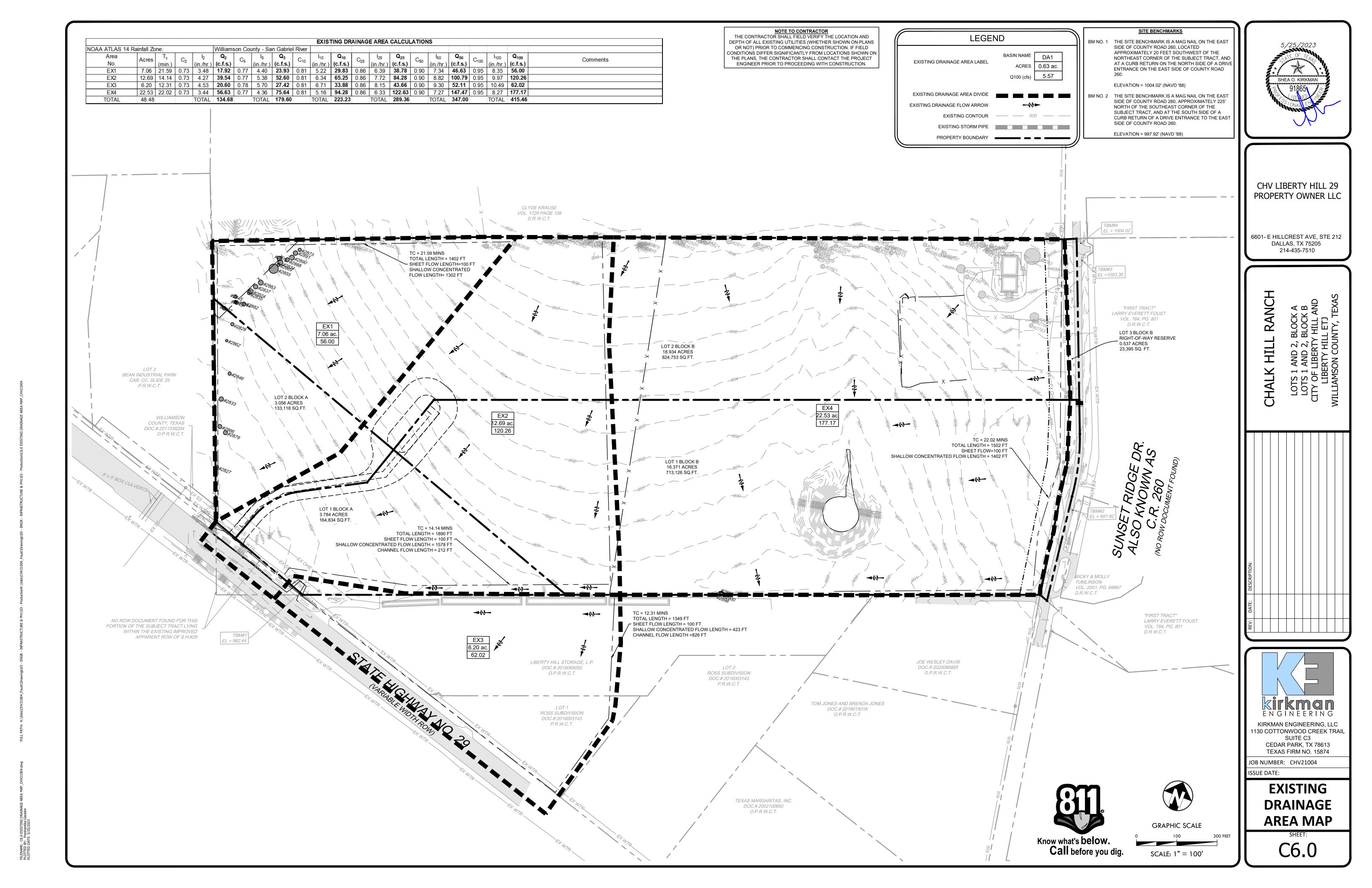


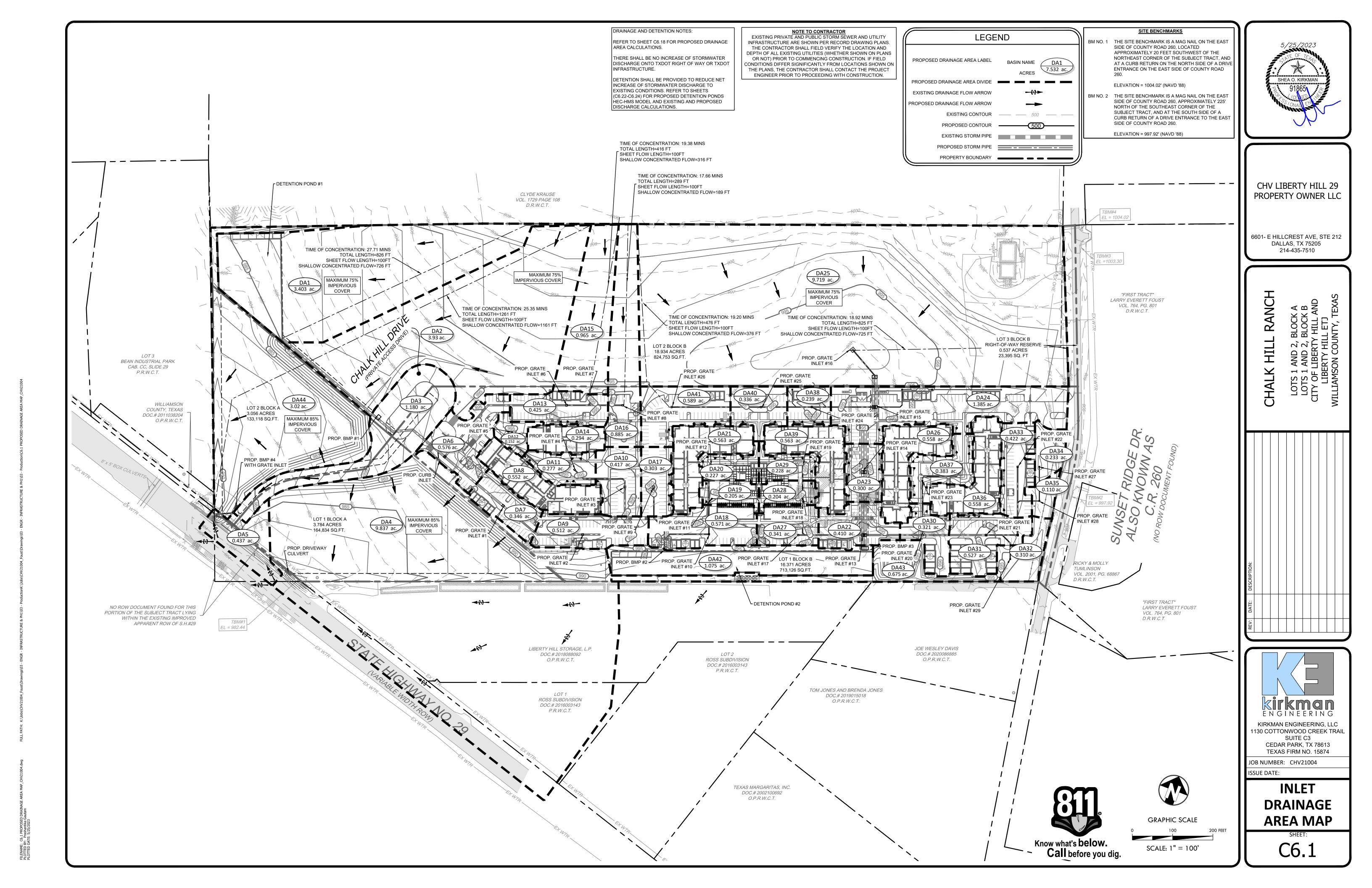
Underground storm system has been in place to catch the water from site through inlets, which then will be directed to the outfall locations at the detention ponds.

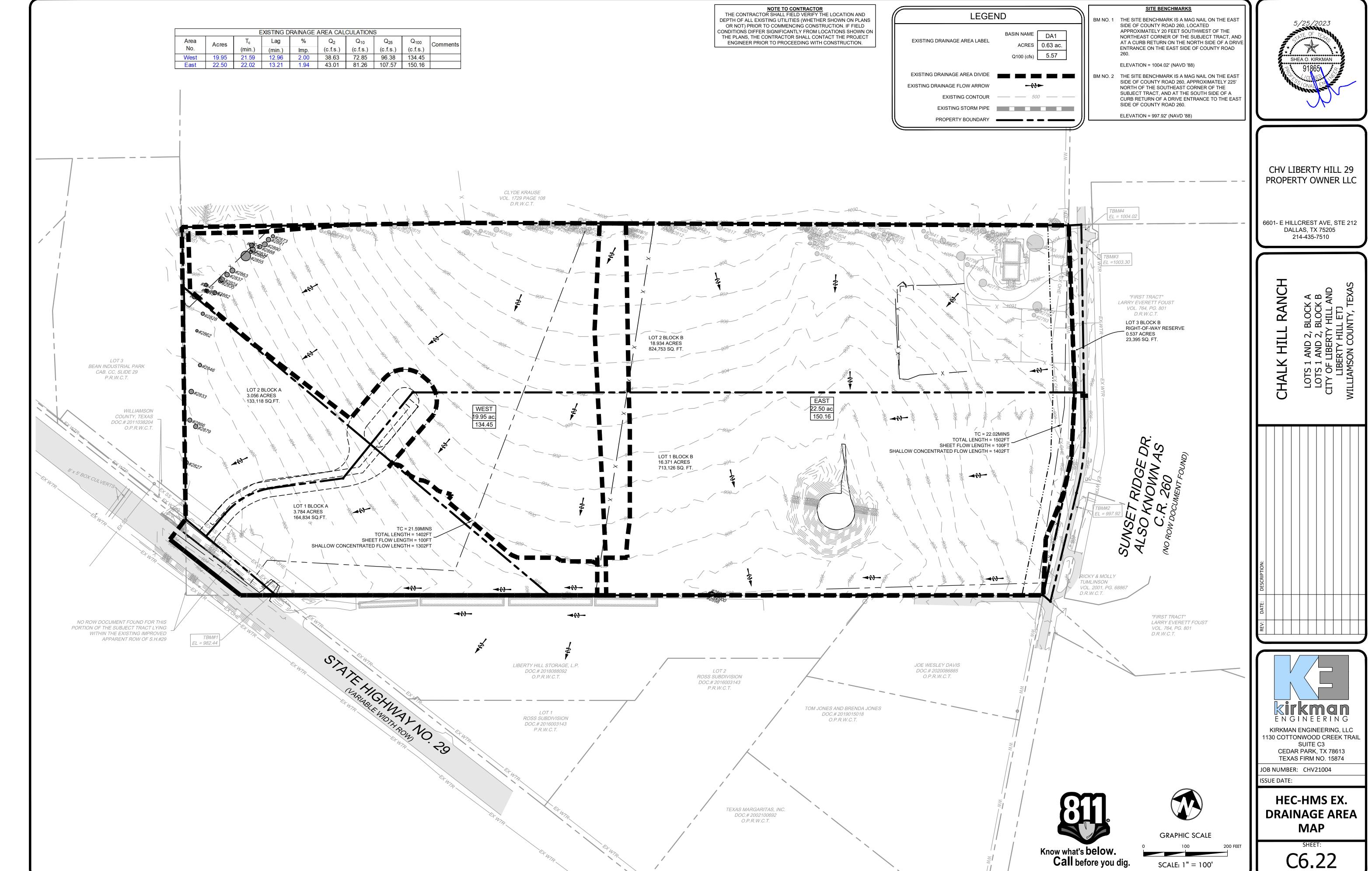
BMP's will collect sedimentation and debris prior to discharge.

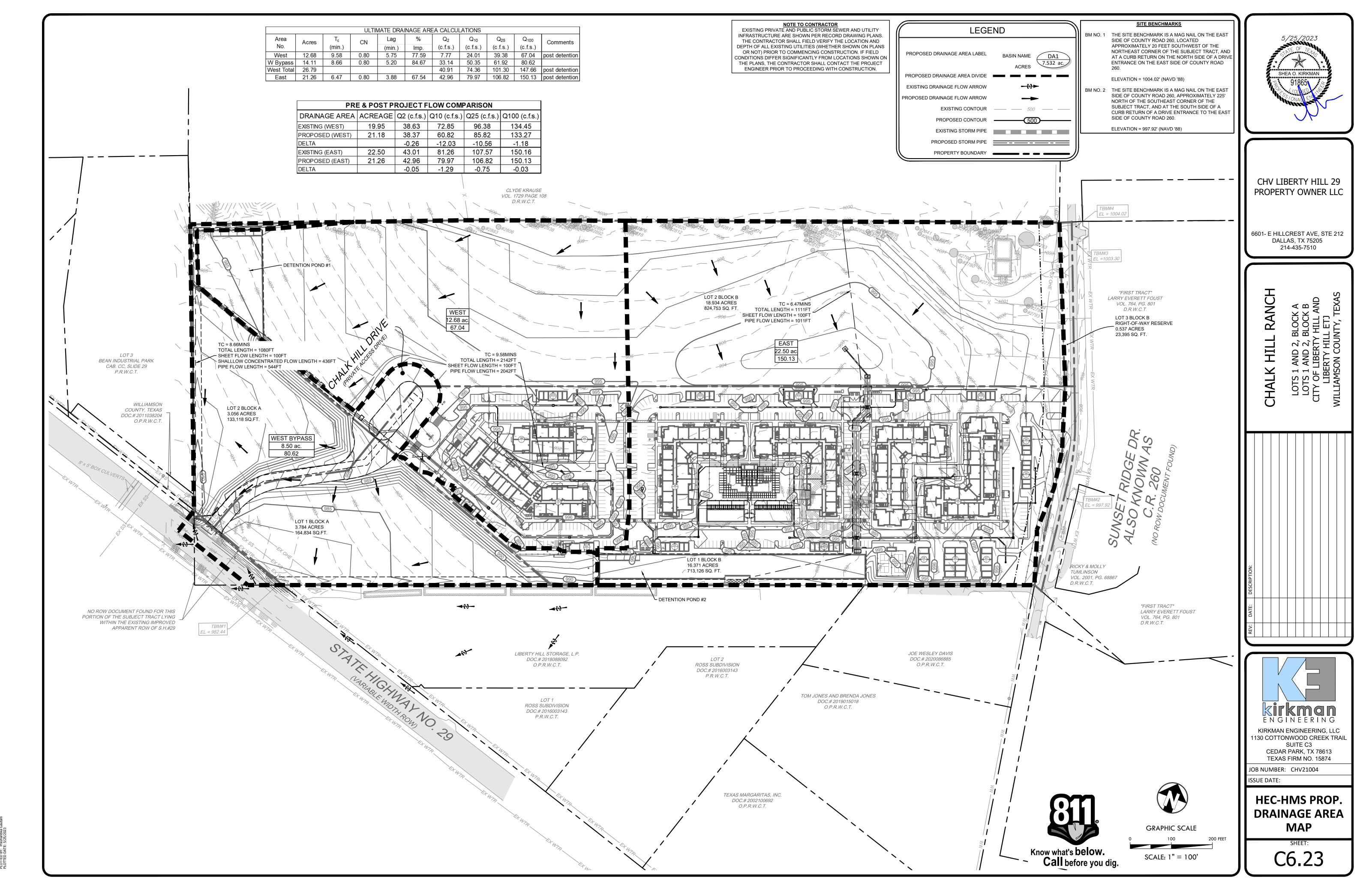
ATTACHMENT G DRAINAGE AREA MAP











FULL PATH: K:\Jobs\CHV21004_Foust\Drawings\03 - ENGR - INFRASTRUCTURE & PH1\03 - ProductionK:\Jobs\CHV21004_Foust\Drawings\03 - ENGR - INFRASTRUCTURE & PH1\03 - Production\HMS DAM ULT

ENAME: HMS DAM ULT.dwg

ATTACHMENT I INSPECTION AND MAINTENANCE FOR BMPS

Inspector (name/title):



am/pm

Complete this worksheet every seven days; OR, every 14 days and within 24 hours of a 0.5 inch rainfall event, and retain in your SWP3.

Inspection Date:

Day:

Time:

Scope of inspection (circle	one):	14 Day Inspection or Weekly Inspection				
Day of week normally con	ducted:		0.5 inch Rainfall Event: y			
Туре		Inspected?		Areas of Concern		
		(Y/N)		_		
Disturbed Soil Areas						
Sediment & Erosion Cont	rols					
Entrance(s) and Exit(s)						
BMP & Location	OK? (Y/N)	BMP Failur	BMP Failure? (describe)		BMP Failure? (describe)	
"I certify that the facility opermit."	r site is in complia	nce with the stormw	ater pollutior	າ prevention p	olan and this	
I further certify that I am a Signatories to Reports)	authorized to sign	this report under TC	EQ rules at 30	TAC 305.128	(relating to	
Name/Title:			Date	:		

ATTACHMENT J SCHEDULE OF INTERIM AND PERMANENT SOIL STABILIZATION PRACTICES



The following soil stabilization practices shall be used:

- 1. All stockpiles shall be temporarily seeded and mulched to prevent loss due to erosion and encircled with a silt fence or filter tube.
- 2. As an area reaches final grade, install permanent stabilization as soon as possible but in no case longer than 14 days from reaching final grade.
- 3. Any area that will remain undisturbed for 14 days or more shall be temporarily stabilized.
- 4. Complete permanent stabilization of all disturbed areas. Once a minimum of 70 percent of the vegetated areas have been stabilized, remove the erosion control BMPs.