



## Water Pollution Abatement Plan (WPAP)

### Parkside on the River Sections 9A & 10A

CITY OF GEORGETOWN  
WILLIAMSON COUNTY, TEXAS

December 19, 2023

HR Green Project No: 2303295

Prepared For:  
HM Parkside, LP  
1011 North Lamar Boulevard  
Austin, Texas 78703

Prepared By:  
HR Green Development TX, LLC  
5508 Highway 290 West, Suite 150  
Austin, Texas 78735  
TBPE Firm No. F-16384

12/19/2023



*Christine Campbell*



## TABLE OF CONTENTS

Edwards Aquifer Application Cover Page (TCEQ-20705).....	1
General Information Form (TCEQ-0587).....	2
Attachment A - Road Map	
Attachment B - USGS Quadrangle Map	
Attachment C - Project Narrative	
Geologic Assessment Form (TCEQ-0585).....	3
Attachment A – Project Figures: Stratigraphic Column (Figure 5)	
Attachment B – Site Geologic Map	
Attachment C – Geologic Assessment Table (TCEQ-0585-Table)	
Attachment D – Site Photographs	
Water Pollution Abatement Plan Application Form (TCEQ-0584).....	4
Attachment A – Factors Affecting Water Quality	
Attachment B – Volume and Character of Stormwater	
Temporary Stormwater Section (TCEQ-0602).....	5
Attachment A – Spill Response Actions	
Attachment B – Potential Sources of Contamination	
Attachment C – Sequence of Major Activities	
Attachment D – Temporary Best Management Practices and Measures	
Attachment F – Structural Practices	
Attachment G – Drainage Area Map	
Attachment H – Temporary Sediment Pond(s) Plans and Calculations	
Attachment I – Inspection and Maintenance for BMPs	
Attachment J – Schedule of Interim and Permanent Soil Stabilization Practices	
Permanent Stormwater Section (TCEQ-0600).....	6
Attachment B – BMPs for Upgradient Stormwater	
Attachment C – BMPs for On-site Stormwater	
Attachment D – BMPs for Surface Streams	
Attachment F – Construction Plans	
Attachment I – Measures for Minimizing Surface Stream Contamination	
Agent Authorization Form (TCEQ-0599) – Authorizing HR Green Development TX, LLC.....	7
Application Fee Form (TCEQ-0574).....	8
Core Data Form (TCEQ-10400).....	9

# Texas Commission on Environmental Quality

## Edwards Aquifer Application Cover Page

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### Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with [30 TAC 213](#).

### Administrative Review

1. [Edwards Aquifer applications](#) must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.

To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: <http://www.tceq.texas.gov/field/eapp>.

2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.

An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.

5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
6. If the geologic assessment was completed before October 1, 2004 and the site contains “possibly sensitive” features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

### Technical Review

1. When an application is deemed administratively complete, the technical review period begins. The regional office will distribute copies of the application to the identified affected city, county, and groundwater conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.

3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited**.
4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

### Mid-Review Modifications

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a “Mid-Review Modification”. Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ’s Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ’s San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

<b>1. Regulated Entity Name: Parkside on the River Sections 9A &amp; 10A</b>						<b>2. Regulated Entity No.:</b>			
<b>3. Customer Name: HM Parkside, LP</b>						<b>4. Customer No.: CN605721653</b>			
<b>5. Project Type: (Please circle/check one)</b>	New <input checked="" type="checkbox"/> X		Modification			Extension		Exception	
<b>6. Plan Type: (Please circle/check one)</b>	WPAP <input checked="" type="checkbox"/> X	CZP	SCS	UST	AST	EXP	EXT	Technical Clarification	Optional Enhanced Measures
<b>7. Land Use: (Please circle/check one)</b>	Residential <input checked="" type="checkbox"/> X		Non-residential			<b>8. Site (acres):</b>		34.42 (LOC = 33.14) Legal Boundary = 75.68	
<b>9. Application Fee:</b>	\$6,500		<b>10. Permanent BMP(s):</b>				Batch Detention Pond		
<b>11. SCS (Linear Ft.):</b>	N/A		<b>12. AST/UST (No. Tanks):</b>				N/A		
<b>13. County:</b>	Williamson County		<b>14. Watershed:</b>				South Fork San Gabriel River		

# Application Distribution

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the “Texas Groundwater Conservation Districts within the EAPP Boundaries” map found at:

[http://www.tceq.texas.gov/assets/public/compliance/field\\_ops/eapp/EAPP%20GWCD%20map.pdf](http://www.tceq.texas.gov/assets/public/compliance/field_ops/eapp/EAPP%20GWCD%20map.pdf)

For more detailed boundaries, please contact the conservation district directly.

Austin Region			
County:	Hays	Travis	Williamson
Original (1 req.)	—	—	<u>  X  </u>
Region (1 req.)	—	—	<u>  X  </u>
County(ies)	—	—	<u>  X  </u>
Groundwater Conservation District(s)	<u>  </u> Edwards Aquifer Authority <u>  </u> Barton Springs/ Edwards Aquifer <u>  </u> Hays Trinity <u>  </u> Plum Creek	<u>  </u> Barton Springs/ Edwards Aquifer	NA
City(ies) Jurisdiction	<u>  </u> Austin <u>  </u> Buda <u>  </u> Dripping Springs <u>  </u> Kyle <u>  </u> Mountain City <u>  </u> San Marcos <u>  </u> Wimberley <u>  </u> Woodcreek	<u>  </u> Austin <u>  </u> Bee Cave <u>  </u> Pflugerville <u>  </u> Rollingwood <u>  </u> Round Rock <u>  </u> Sunset Valley <u>  </u> West Lake Hills	<u>  </u> Austin <u>  </u> Cedar Park <u>  </u> Florence <u>  X  </u> Georgetown <u>  </u> Jerrell <u>  </u> Leander <u>  </u> Liberty Hill <u>  </u> Pflugerville <u>  </u> Round Rock

San Antonio Region					
County:	Bexar	Comal	Kinney	Medina	Uvalde
Original (1 req.)	—	—	—	—	—
Region (1 req.)	—	—	—	—	—
County(ies)	—	—	—	—	—
Groundwater Conservation District(s)	<u>  </u> Edwards Aquifer Authority <u>  </u> Trinity-Glen Rose	<u>  </u> Edwards Aquifer Authority	<u>  </u> Kinney	<u>  </u> EAA <u>  </u> Medina	<u>  </u> EAA <u>  </u> Uvalde
City(ies) Jurisdiction	<u>  </u> Castle Hills <u>  </u> Fair Oaks Ranch <u>  </u> Helotes <u>  </u> Hill Country Village <u>  </u> Hollywood Park <u>  </u> San Antonio (SAWS) <u>  </u> Shavano Park	<u>  </u> Bulverde <u>  </u> Fair Oaks Ranch <u>  </u> Garden Ridge <u>  </u> New Braunfels <u>  </u> Schertz	NA	<u>  </u> San Antonio ETJ (SAWS)	NA

I certify that to the best of my knowledge, that the application is complete and accurate. This application is hereby submitted to TCEQ for administrative review and technical review.

Christine Campbell

Print Name of Customer/Authorized Agent

*Christine Campbell*

12/19/2023

Signature of Customer/Authorized Agent

Date

**\*\*FOR TCEQ INTERNAL USE ONLY\*\***

Date(s) Reviewed:		Date Administratively Complete:	
Received From:		Correct Number of Copies:	
Received By:		Distribution Date:	
EAPP File Number:		Complex:	
Admin. Review(s) (No.):		No. AR Rounds:	
Delinquent Fees (Y/N):		Review Time Spent:	
Lat./Long. Verified:		SOS Customer Verification:	
Agent Authorization Complete/Notarized (Y/N):		Fee Check:	Payable to TCEQ (Y/N):
Core Data Form Complete (Y/N):			Signed (Y/N):
Core Data Form Incomplete Nos.:			Less than 90 days old (Y/N):

# General Information Form

## Texas Commission on Environmental Quality

For Regulated Activities on the Edwards Aquifer Recharge and Transition Zones and Relating to 30 TAC §213.4(b) & §213.5(b)(2)(A), (B) Effective June 1, 1999

***To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.***

***Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.***


## Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **General Information Form** is hereby submitted for TCEQ review. The application was prepared by:

Print Name of Customer/Agent: Christine Campbell, P.E.

Date: 12/19/2023

Signature of Customer/Agent:



## Project Information

1. Regulated Entity Name: Parkside on the River Sections 9A & 10A

2. County: Williamson

3. Stream Basin: Brazos River Basin

4. Groundwater Conservation District (If applicable): N/A

5. Edwards Aquifer Zone:

☒ Recharge Zone

☐ Transition Zone

6. Plan Type:

☒ WPAP

☐ SCS

☐ Modification

☐ AST

☐ UST

☐ Exception Request

7. Customer (Applicant):

Contact Person: Blake Magee

Entity: HM Parkside, LP

Mailing Address: 1011 North Lamar Boulevard

City, State: Austin, TX

Zip: 78703

Telephone: 512-481-0303

FAX: \_\_\_\_\_

Email Address: Blake@blakemageeco.com

8. Agent/Representative (If any):

Contact Person: Christine Campbell

Entity: HR Green Development TX, LLC

Mailing Address: 5508 US Highway 290 West Suite #150

City, State: Austin, TX

Zip: 78735

Telephone: 512-872-6696

FAX: \_\_\_\_\_

Email Address: christine.campbell@hrgreen.com

9. Project Location:

- ☐ The project site is located inside the city limits of \_\_\_\_\_.
- ☒ The project site is located outside the city limits but inside the ETJ (extra-territorial jurisdiction) of Georgetown.
- ☐ The project site is not located within any city's limits or ETJ.

10. ☒ The location of the project site is described below. The description provides sufficient detail and clarity so that the TCEQ's Regional staff can easily locate the project and site boundaries for a field investigation.

Located east of Parkside Parkway. East of Parkside on the River Phase 3 Sections 4, 7A & 7B. Property ID R574025, R312360, R574027, R500990, R500991, R500992, R501370

11. ☒ **Attachment A – Road Map.** A road map showing directions to and the location of the project site is attached. The project location and site boundaries are clearly shown on the map.
12. ☒ **Attachment B - USGS / Edwards Recharge Zone Map.** A copy of the official 7 ½ minute USGS Quadrangle Map (Scale: 1" = 2000') of the Edwards Recharge Zone is attached. The map(s) clearly show:
- ☒ Project site boundaries.
- ☒ USGS Quadrangle Name(s).
- ☒ Boundaries of the Recharge Zone (and Transition Zone, if applicable).
- ☒ Drainage path from the project site to the boundary of the Recharge Zone.
13. ☒ **The TCEQ must be able to inspect the project site or the application will be returned.** Sufficient survey staking is provided on the project to allow TCEQ regional staff to locate the boundaries and alignment of the regulated activities and the geologic or manmade features noted in the Geologic Assessment.

☒ Survey staking will be completed by this date: November 17, 2023

14. ☒ **Attachment C – Project Description.** Attached at the end of this form is a detailed narrative description of the proposed project. The project description is consistent throughout the application and contains, at a minimum, the following details:

- ☒ Area of the site
- ☒ Offsite areas
- ☒ Impervious cover
- ☒ Permanent BMP(s)
- ☒ Proposed site use
- ☒ Site history
- ☒ Previous development
- ☒ Area(s) to be demolished

15. Existing project site conditions are noted below:

- ☐ Existing commercial site
- ☐ Existing industrial site
- ☐ Existing residential site
- ☐ Existing paved and/or unpaved roads
- ☒ Undeveloped (Cleared)
- ☒ Undeveloped (Undisturbed/Uncleared)
- ☐ Other: \_\_\_\_\_

### ***Prohibited Activities***

16. ☒ I am aware that the following activities are prohibited on the Recharge Zone and are not proposed for this project:

- (1) Waste disposal wells regulated under 30 TAC Chapter 331 of this title (relating to Underground Injection Control);
- (2) New feedlot/concentrated animal feeding operations, as defined in 30 TAC §213.3;
- (3) Land disposal of Class I wastes, as defined in 30 TAC §335.1;
- (4) The use of sewage holding tanks as parts of organized collection systems; and
- (5) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41(b), (c), and (d) of this title (relating to Types of Municipal Solid Waste Facilities).
- (6) New municipal and industrial wastewater discharges into or adjacent to water in the state that would create additional pollutant loading.

17. ☒ I am aware that the following activities are prohibited on the Transition Zone and are not proposed for this project:

- (1) Waste disposal wells regulated under 30 TAC Chapter 331 (relating to Underground Injection Control);



- (2) Land disposal of Class I wastes, as defined in 30 TAC §335.1; and
- (3) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41 (b), (c), and (d) of this title.

### ***Administrative Information***

18. The fee for the plan(s) is based on:

- ☒ For a Water Pollution Abatement Plan or Modification, the total acreage of the site where regulated activities will occur.
- ☐ For an Organized Sewage Collection System Plan or Modification, the total linear footage of all collection system lines.
- ☐ For a UST Facility Plan or Modification or an AST Facility Plan or Modification, the total number of tanks or piping systems.
- ☐ A request for an exception to any substantive portion of the regulations related to the protection of water quality.
- ☐ A request for an extension to a previously approved plan.

19. ☒ Application fees are due and payable at the time the application is filed. If the correct fee is not submitted, the TCEQ is not required to consider the application until the correct fee is submitted. Both the fee and the Edwards Aquifer Fee Form have been sent to the Commission's:

- ☐ TCEQ cashier
- ☒ Austin Regional Office (for projects in Hays, Travis, and Williamson Counties)
- ☐ San Antonio Regional Office (for projects in Bexar, Comal, Kinney, Medina, and Uvalde Counties)

20. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

21. ☒ No person shall commence any regulated activity until the Edwards Aquifer Protection Plan(s) for the activity has been filed with and approved by the Executive Director.

SCALE: 1"=4000'



5508 HIGHWAY 290 WEST  
SUITE 150  
AUSTIN, TX 78735  
512. 872. 6696  
HRGREEN.COM

TBPE NO: 16384  
TBPLS NO: 10194101

# PARKSIDE ON THE RIVER SECTIONS 9A & 10A LOCATION MAP

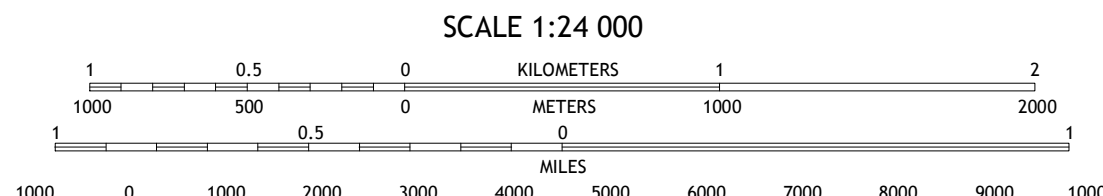
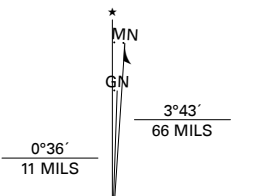




Produced by the United States Geological Survey

North American Datum of 1983 (NAD83)  
World Geodetic System of 1984 (WGS84). Projection and  
1 000-meter grid/Universal Transverse Mercator, Zone 14R.  
This map is not a legal document. Boundaries may be  
generalized for this map scale. Private lands within government  
reservations may not be shown. Obtain permission before  
entering private lands.

Imagery.....NAIP, September 2016 - November 2016  
Roads.....U.S. Census Bureau, 2015  
Names.....GNS, 1979 - 2018  
Hydrography.....National Hydrography Dataset, 2002 - 2018  
Contours.....National Elevation Dataset, 2002  
Boundaries.....Multiple sources; see metadata file 2017  
Wetlands.....FWS National Wetlands Inventory 1982



1	2	3
4	5	6
7	8	9

ADJOINING QUADRANGLES

1 Liberty Hill  
2 Leander NE  
3 Georgetown  
4 Nameless  
5 Round Rock  
6 Mansfield Dam  
7 Jollyville  
8 Pflugerville West

ROAD CLASSIFICATION	
Expressway	Local Connector
Secondary Hwy	Local Road
Ramp	4WD
Interstate Route	US Route
	State Route





## ATTACHMENT C – PROJECT NARRATIVE

The Parkside on the River Sections 9A & 10A development is a proposed single-family residential development tract, including associated right-of-way, drainage, and utilities located in the City of Georgetown and Williamson County. The project site is located within the Edwards Aquifer Recharge Zone, the Edwards Aquifer Contributing Zone, and within the San Gabriel River watershed. The overall project site encompasses a 34.42-acre tract of land located east of Parkside Parkway and east of Parkside on the River Phase 3 Sections 4, 7A & 7B. There will be roughly 33.14-acres of disturbed land and a 75.68-acre legal boundary for application fee purposes.

The project site is primarily undeveloped wooded land with grass. Runoff flows towards the South Fork San Gabriel River. No portion of the project site is located within the 100-year floodplain as defined by FEMA FIRM Panel No. 48491C0460F, dated December 20, 2019.

The proposed development results in an impervious cover of approximately 47.6% and will have the associated runoff treated by the batch detention pond associated with Parkside on the River Section 8. Of the 34.42 acres of the proposed Parkside on the River Section 9A & 10A property, there is approximately 16.28 acres of impervious cover. There is an additional 0.11 acres of impervious cover proposed for a temporary turnaround, resulting in a total of approximately 16.39 acres of proposed impervious cover for this project. Based on the 80% TSS removal requirement by TCEQ we need to provide 14,266 lbs of TSS removal for the proposed development. As shown in the calculations, the Section 8 pond satisfies the TSS removal requirement. The 85% TSS removal requirement by the City of Georgetown is also satisfied by the Section 8 batch detention pond.

The proposed conditions for the overall area propose approximately 30.56 acres of post-development impervious cover, of which approximately 1.60 acres are existing from Parkside on the River Phase 3 Sections 4, 7A & 7B, 12.57 acres from Parkside on the River Section 8, and 16.39 acres proposed with Parkside on the River Sections 9A & 10A. Based on the 80% TSS removal requirement by TCEQ we need to provide 25,207 lbs of TSS removal in the proposed case. As shown in the calculations, the Section 8 pond and vegetative filter strip satisfy this requirement. The 85% TSS removal requirement by the City of Georgetown is also satisfied for the Section 8 batch detention pond. In the proposed condition, the Section 8 batch detention pond (BDP-01) will treat a total of 25.88 acres of impervious cover (1.60 acres of existing impervious cover from Sections 4, 7A & 7B, 10.02 acres of impervious cover from Section 8, and 14.26 acres of proposed impervious cover from Sections 9A & 10A) and provide 23,750 lbs of TSS removal. Approximately 2.13 acres of impervious cover proposed with Sections 9A & 10A is bypassing treatment. The BMPs are overtreating to account for the bypass impervious cover.

Refer to the construction plans for the water quality calculations and the attached Parkside on the River Section 8 plans for the batch detention pond design. Refer to the table below for the proposed sedimentation treatment breakdown provided.

A tree demolition schedule is included in the construction plans.

The associated combination of roadway, drainage, water quality, water, and wastewater improvements will be designed and built to serve this residential development.

PARKSIDE ON THE RIVER SECTIONS 9A & 10A - TSS REMOVAL SUMMARY - PROPOSED													
DRAINAGE AREA	BMP TYPE	MAX TSS REMOVAL EFFICIENCY	BASIN AREA	PRE- DEVELOPMENT I.C.	PROPOSED I.C.		POST-DEVELOPMENT I.C.		TCEQ REQUIRED 80% TSS LOAD REMOVAL	CITY OF GEORGETOWN REQUIRED 85% POND TSS LOAD REMOVAL	PROVIDED TSS LOAD REMOVAL	VOLUME REQUIRED	VOLUME PROVIDED
					SECTION 8	SECTION 9A 10A							
			AC	AC	AC	AC	AC	%	LB	LB	LB	CF	CF
BDP-01	BATCH DETENTION POND	91%	57.62	1.60	10.02	14.26	25.88	45%	21,133	22,454	23,750	132,879	137,853
VFS-01	VEGETATIVE FILTER STRIP	85%	4.00	0.00	1.92		1.92	48%	1,671		1,838		
BP-01	BY-PASS	0%	1.17	0.00	0.63		0.63	54%	548				
BP-02	BY-PASS	0%	2.90	0.00		1.48	1.48	51%	1,288				
BP-03	BY-PASS	0%	1.33	0.00		0.65	0.65	49%	566				
TOTAL:			67.02	1.60	12.57	16.39	30.56	46%	25,207		25,588		
1 - FOR THE GEORGETOWN TSS REMOVAL REQUIREMENT, WE CONSIDER 85% OF TSS REMOVAL FOR THE DRAINAGE AREA THAT DRAINS TOWARD THE BATCH DETENTION PONDS.													



**Narrative Description of Site-Specific Geology for the  
Parkside on the River Property (Phase 3, Sections 8,  
9A, & 10A) Located in Georgetown, Williamson  
County, Texas**

Prepared for:

**HM PARKSIDE DEVELOPMENT, INC**

Prepared by:

**CAMBRIAN ENVIRONMENTAL**

October 18<sup>th</sup>, 2023

**NARRATIVE DESCRIPTION OF SITE-SPECIFIC GEOLOGY FOR THE PARKSIDE  
ON THE RIVER PROPERTY (PHASE 3, SECTIONS 8, 9A, & 10A) LOCATED IN  
GEORGETOWN, WILLIAMSON COUNTY, TEXAS**

Prepared for:

**HM Parkside Development, Inc.**  
**Blake Magee Co.**  
1011 North Lamar Blvd.  
Austin, Texas 78703

Prepared by:

Craig Crawford, P.G.  
TX Geoscience License #10791

**Cambrian Environmental**  
4422 Pack Saddle Pass  
Suite 204  
Austin, Texas 78745

TX Geoscience Firm Registration #50484

As a licensed professional geoscientist, I attest that the  
contents of this report are complete and accurate to the  
best of my knowledge.



# Geologic Assessment

## Texas Commission on Environmental Quality

For Regulated Activities on The Edwards Aquifer Recharge/transition Zones and Relating to 30 TAC §213.5(b)(3), Effective June 1, 1999

*To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.*

*Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.*

## Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

Print Name of Geologist: Craig Crawford, PG

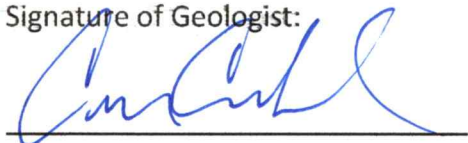
Telephone: 512.705.5541

Date: 18 October 2023

Fax: \_\_\_\_\_

Representing: Cambrian Environmental (TBPG Firm # 50484) (Name of Company and TBPE registration number)

Signature of Geologist:



Regulated Entity Name: HM Parkside Development, Inc.(Parkside on the River - Ph. 3, Sec. 8, 9A, & 10A)

## Project Information

1. Date(s) Geologic Assessment was performed: August 30<sup>th</sup> through September 12<sup>th</sup> 2023

2. Type of Project:

☒ WPAP  
☐ SCS

☐ AST  
☐ UST

3. Location of Project:

☒ Recharge Zone  
☐ Transition Zone  
☐ Contributing Zone within the Transition Zone



4. ☒ **Attachment A - Geologic Assessment Table.** Completed Geologic Assessment Table (Form TCEQ-0585-Table) is attached.
5. ☒ Soil cover on the project site is summarized in the table below and uses the SCS Hydrologic Soil Groups\* (Urban Hydrology for Small Watersheds, Technical Release No. 55, Appendix A, Soil Conservation Service, 1986). If there is more than one soil type on the project site, show each soil type on the site Geologic Map or a separate soils map.

**Table 1 - Soil Units, Infiltration Characteristics and Thickness**

Soil Name	Group*	Thickness(feet)
Denton (DnB)	D	< 3.5
Eckrant (EeB,ErE,ErG)	D	< 2

\* Soil Group Definitions (Abbreviated)

- A. Soils having a high infiltration rate when thoroughly wetted.
- B. Soils having a moderate infiltration rate when thoroughly wetted.
- C. Soils having a slow infiltration rate when thoroughly wetted.
- D. Soils having a very slow infiltration rate when thoroughly wetted.

6. ☒ **Attachment B – Stratigraphic Column.** A stratigraphic column showing formations, members, and thicknesses is attached. The outcropping unit, if present, should be at the top of the stratigraphic column. Otherwise, the uppermost unit should be at the top of the stratigraphic column.
7. ☒ **Attachment C – Site Geology.** A narrative description of the site specific geology including any features identified in the Geologic Assessment Table, a discussion of the potential for fluid movement to the Edwards Aquifer, stratigraphy, structure(s), and karst characteristics is attached.
8. ☒ **Attachment D – Site Geologic Map(s).** The Site Geologic Map must be the same scale as the applicant's Site Plan. The minimum scale is 1": 400'  
 Applicant's Site Plan Scale: 1" = 100'  
 Site Geologic Map Scale: 1" = 100'  
 Site Soils Map Scale (if more than 1 soil type): 1" = 400'
9. Method of collecting positional data:
  - ☒ Global Positioning System (GPS) technology.
  - ☐ Other method(s). Please describe method of data collection: \_\_\_\_\_
10. ☒ The project site and boundaries are clearly shown and labeled on the Site Geologic Map.
11. ☒ Surface geologic units are shown and labeled on the Site Geologic Map.

12. ☒ Geologic or manmade features were discovered on the project site during the field investigation. They are shown and labeled on the Site Geologic Map and are described in the attached Geologic Assessment Table.
- ☐ Geologic or manmade features were not discovered on the project site during the field investigation.
13. ☒ The Recharge Zone boundary is shown and labeled, if appropriate.
14. All known wells (test holes, water, oil, unplugged, capped and/or abandoned, etc.): If applicable, the information must agree with Item No. 20 of the WPAP Application Section.
- ☐ There are \_\_\_\_\_ (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply.)
- ☐ The wells are not in use and have been properly abandoned.
- ☐ The wells are not in use and will be properly abandoned.
- ☐ The wells are in use and comply with 16 TAC Chapter 76.
- ☒ There are no wells or test holes of any kind known to exist on the project site.

### ***Administrative Information***

15. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.



## **NARRATIVE DESCRIPTION OF SITE-SPECIFIC GEOLOGY FOR THE PARKSIDE ON THE RIVER PROPERTY (PHASE 3, SECTIONS 8, 9A, & 10A) LOCATED IN GEORGETOWN, WILLIAMSON COUNTY, TEXAS**

### **INTRODUCTION**

This narrative Geologic Assessment accompanies the Texas Commission on Environmental Quality (TCEQ) Geologic Assessment Form TCEQ-0585 completed for the Parkside on the River property in Georgetown, Williamson County, Texas (see Site Location Map). This assessment covers a portion of Phase 3, and includes Sections 8, 9A, and 10A. The project area is located on the north side of Leander Road (FM 2243), approximately 5.25 miles west of the intersection with Interstate Highway (IH) 35.

### **METHODOLOGY**

A Cambrian Environmental Registered Professional Geoscientist (Texas License #10791) and several karst technicians conducted a field survey for a TCEQ Geologic Assessment on various dates between August 30<sup>th</sup> and September 12<sup>th</sup> 2023. The pedestrian survey was completed by walking parallel transects spaced approximately 50 feet apart as directed by the TCEQ in the Instructions to Geologists for Geologic Assessments on the Edwards Aquifer Recharge/Transition Zones (Rev. 10-01-04). Closer spacing was used where vegetation inhibited clear observation. The project site was thoroughly examined for the presence of potential karst features, including depressions, holes, and animal burrows. A number of techniques can be used for this effort, including probing with a digging implement to determine the thickness and consistency of fill material and feeling for the presence of air flow, which may indicate the presence of a sub-surface void space. Other techniques include making observations of any notable characteristics of the feature site such as the presence of various types of vegetation or a semi-circular burrow mound produced by the activities of small mammals.

### **RESULTS**

#### **Soils**

Soils mapped within the project area consist of the Denton silty clay (DnB), Eckrant extremely stony clay (EeB), Eckrant-Rock outcrop (ErE, ErG) series soils<sup>1</sup> (see Site Soils Map). The Denton and Eckrant series soils are within the “D” classification of the hydrologic soil groups. Type “D” soils have a very slow infiltration rate (very high runoff potential) when thoroughly wet.

#### **Geology**

The mapped bedrock lithology underlying the majority of the project area consists of the Edwards Limestone (Ked), with the Comanche Peak Limestone (Kc) present in the lower elevation areas. The Comanche Peak Limestone serves as the lower confining unit of the Edwards Aquifer. The western portion of this tract is mapped as being within the Edwards Aquifer Recharge Zone, and the eastern portion is within the Contributing Zone (see Site Geologic Map). The portion of the tract mapped as Contributing Zone coincides with areas where topography drops off towards a drainage, and also where the Comanche Peak Limestone is present. Based on topographic and geologic maps, the Edwards outcrop present on this

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<sup>1</sup> United States Department of Agriculture, Natural Resource Conservation Service. Online Web Soil Survey, Williamson County, Texas. <http://websoilsurvey.sc.egov.usda.gov/>

property is likely no more than 50 to 60 feet thick in the areas of highest elevation. The geology of the property has been mapped most recently at a useful scale by Collins (2005) and we find his interpretation of the geology to be generally accurate.<sup>2</sup> Bedrock outcrops were common in some areas, while other areas seemed to have relatively thick soil cover. No faults are mapped within the project limits, and none were observed during the pedestrian survey.

Recharge into the aquifer primarily occurs in areas where the Edwards Group and upper confining units are exposed at the surface. Most recharge is from direct infiltration via precipitation and streamflow loss. Recharge occurs predominantly along secondary porosity features such as faults, fractures, and karst features (caves, solution cavities, sinkholes, etc.); and these types of karst features are commonly formed along joints, fractures, and bedding plane surfaces formed within the Edwards Group Limestone.

### **Site Hydrogeologic Assessment**

One sensitive feature was identified during the pedestrian survey (feature “F-2”). Recharge to the aquifer on this property has the greatest potential to occur in the immediate vicinity of this feature. Other areas of the property had a very low density of discovered features and thick soil cover, and the potential for recharge to occur is thought to be low in these areas. Additionally, should any karst features be discovered during the construction phase of the project, they should be reported to TCEQ to determine the appropriate mitigation measures.

### **Feature Descriptions**

- F-1** The feature consists of a non-karst closed depression that measures approximately 8 feet by 10 feet by 2.5 feet deep. The depression is located near the top of drainage, and appears to be the result of bedrock scour and headward erosional processes. There are no signs of any portals, or any other indicators that this feature contributes to subsurface infiltration. The feature is lined with cobbles, soil, and grassy vegetation. The feature is ranked as “non-sensitive”.
- F-2** The feature consists of small sinkhole that measures approximately 4 feet in diameter by at least 2 feet deep. Within the bowl of the sinkhole there is an opening that measures 14 inches by 8 inches. No airflow was detected during this investigation, however the rocks surrounding the opening were covered in green moss, which can be an indicator that subsurface airflow occurs periodically. Small persimmon trees are present around the bowl of the sinkhole, and the feature is lined with loose organic debris and cobbles. The feature is ranked as “sensitive” and Cambrian recommends a minimum of a 50-foot radius protective buffer around this feature.

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<sup>2</sup> E.W. Collins, 2005, Geologic Map of the West Half of the Taylor Texas 30x60 Quadrangle: Central Texas Urban Corridor Encompassing Round Rock, Georgetown, Salado, Briggs, Liberty Hill, and Leander, Bureau of Economic Geology, University of Texas at Austin. Scale 1:100,000

### **City of Georgetown Salamander Ordinance**

No springs were identified within the interior of the property during the pedestrian survey, and therefore no occupied site protection, or spring buffer protection measures will be required for the property. A mapped stream is present on the property (flowing from west to east, see Site Geologic Map), but it appears to only flow during heavy rain when there is high runoff potential. This mapped stream consists of a shallow and gently sloping drainage that did not have any water present, even after a moderate precipitation event that occurred during the course of the pedestrian survey. The catchment area of this mapped stream is less than 64 acres, and therefore no stream protection buffer will be required. A second and larger mapped stream is present along the southeastern boundary of the property, and it was actively flowing at the time of the pedestrian survey. This stream is present in the lowest elevation portion of this property and is within the Edwards Aquifer Contributing Zone, and therefore will not require a stream protection buffer. The 100-year floodplain is present along this channel, with a small portion being within the bounds of the limits of the project area included in this assessment.

Additionally, all regulated activities within the Recharge Zone must follow water quality best management practices, and development of the property will need to comply with the water quality protection measures as outlined in Section 8 of the Ordinance.

## Stratigraphic Column

\*Area shaded gray represents the lithology directly underlying the project site

Period	Group	Stratigraphic Unit	Hydrologic Unit	Maximum Thickness (Feet)
Quaternary to Tertiary		Stream and river alluvium (Qal)	Overlying Units	70
		Terrace alluvium (Qt)		
		Older alluvium (QTa)		
Upper Cretaceous (Gulf Series)	Taylor	Taylor Clay (Ktl)	Confining Units	300
	Austin	Austin Chalk (Kau)		400
	Eagle Ford	Eagle Ford Shale (Kef)		60
	Washita	Buda Limestone (Kbu)		20
		Del Rio Clay (Kdr)		60
Lower Cretaceous (Comanche Series)	Fredericksburg	Georgetown Limestone (Kgt)	Edwards Aquifer	100
		Edwards Limestone (Ked)		120
		Comanche Peak Formation (Kc)		50
	Trinity	Walnut Formation (Kw)	Confining Unit	140
		Upper Glen Rose Limestone (Kgru)	Upper Trinity Aquifer	200





**Photo 1.** View of feature F-1

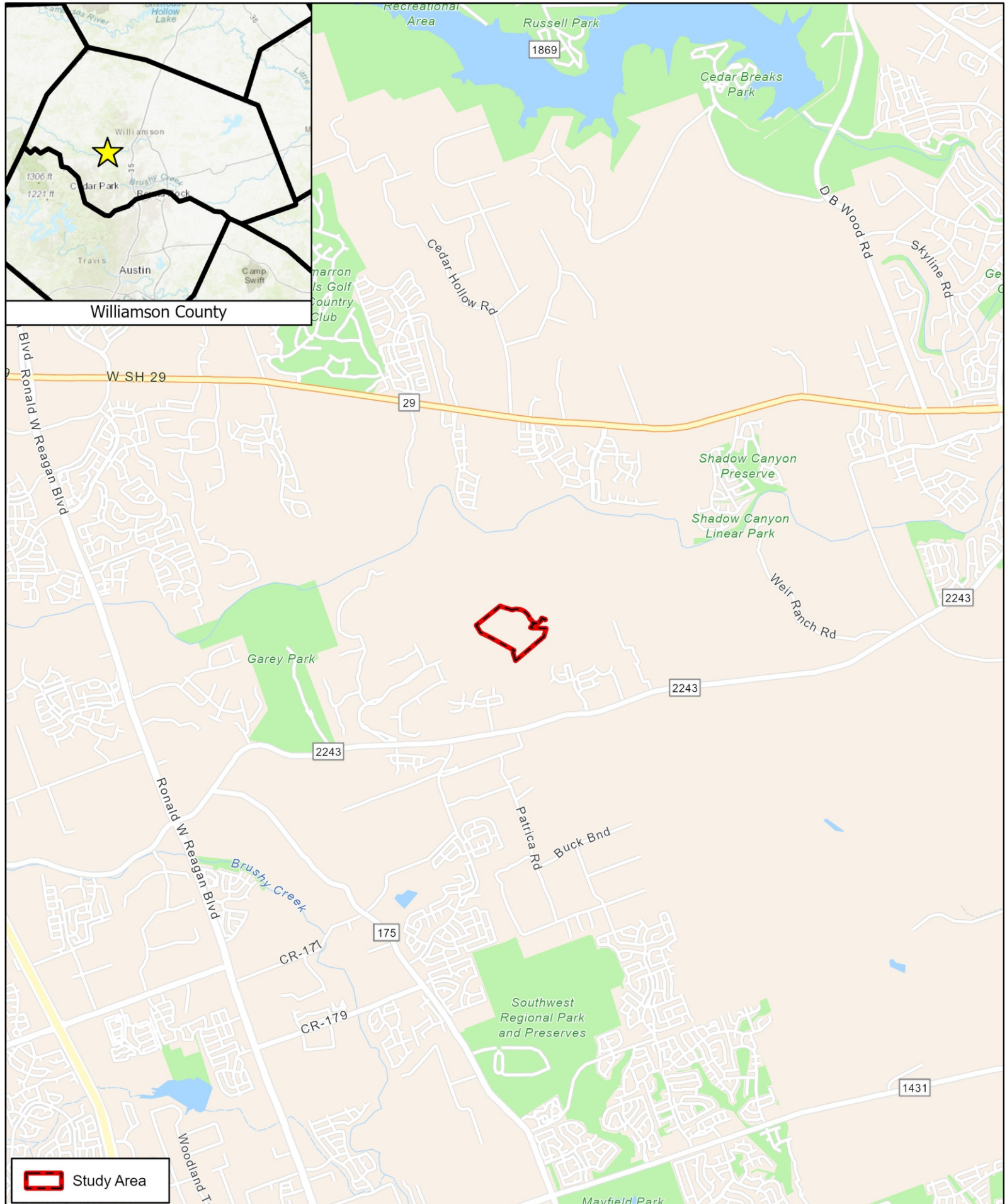



**Photo 2.** View of feature F-2









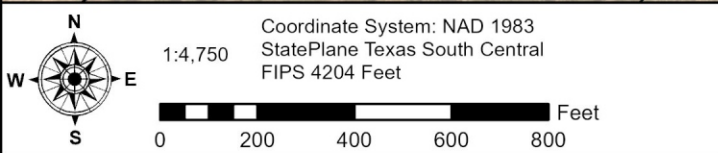
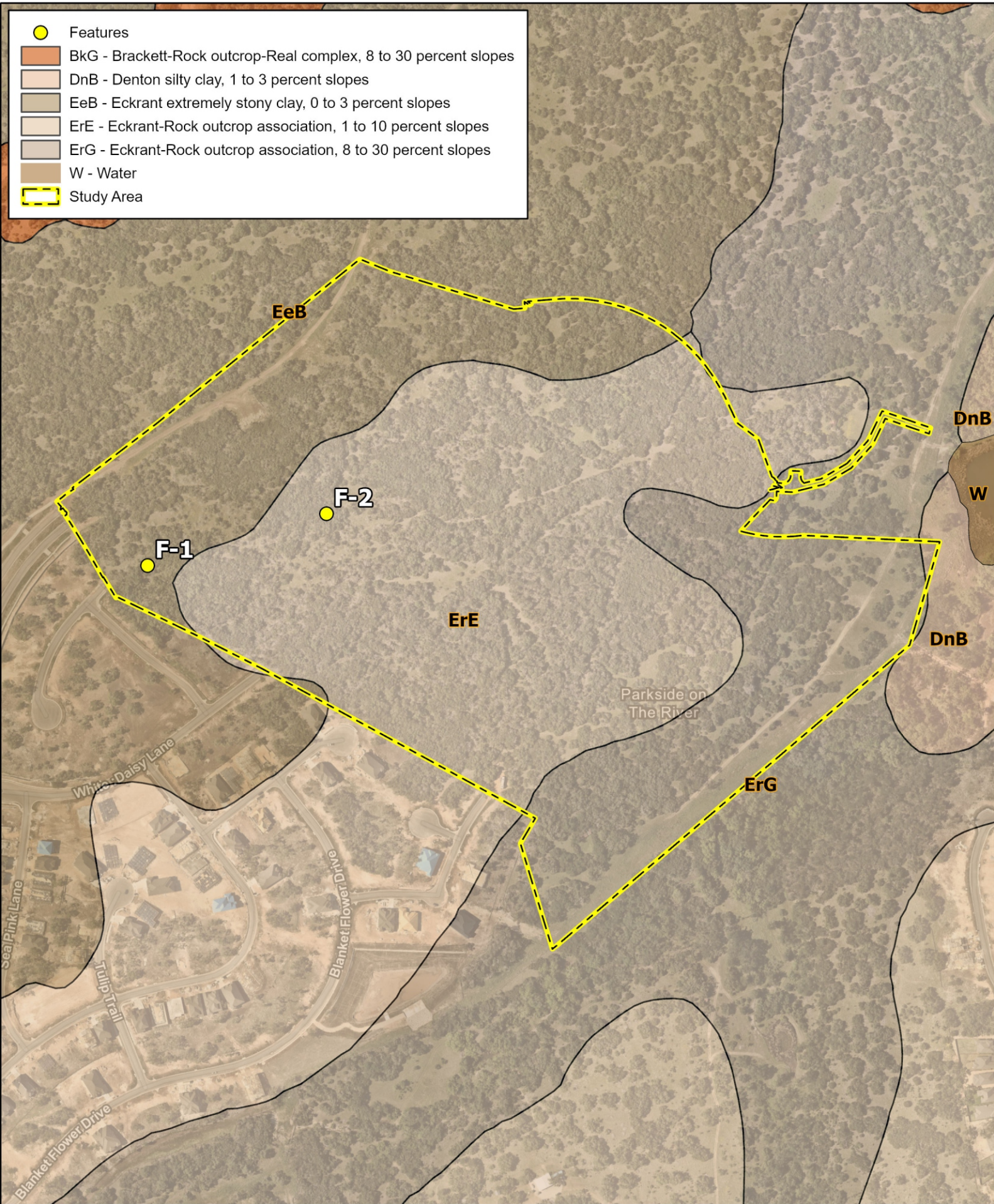


1:60,000

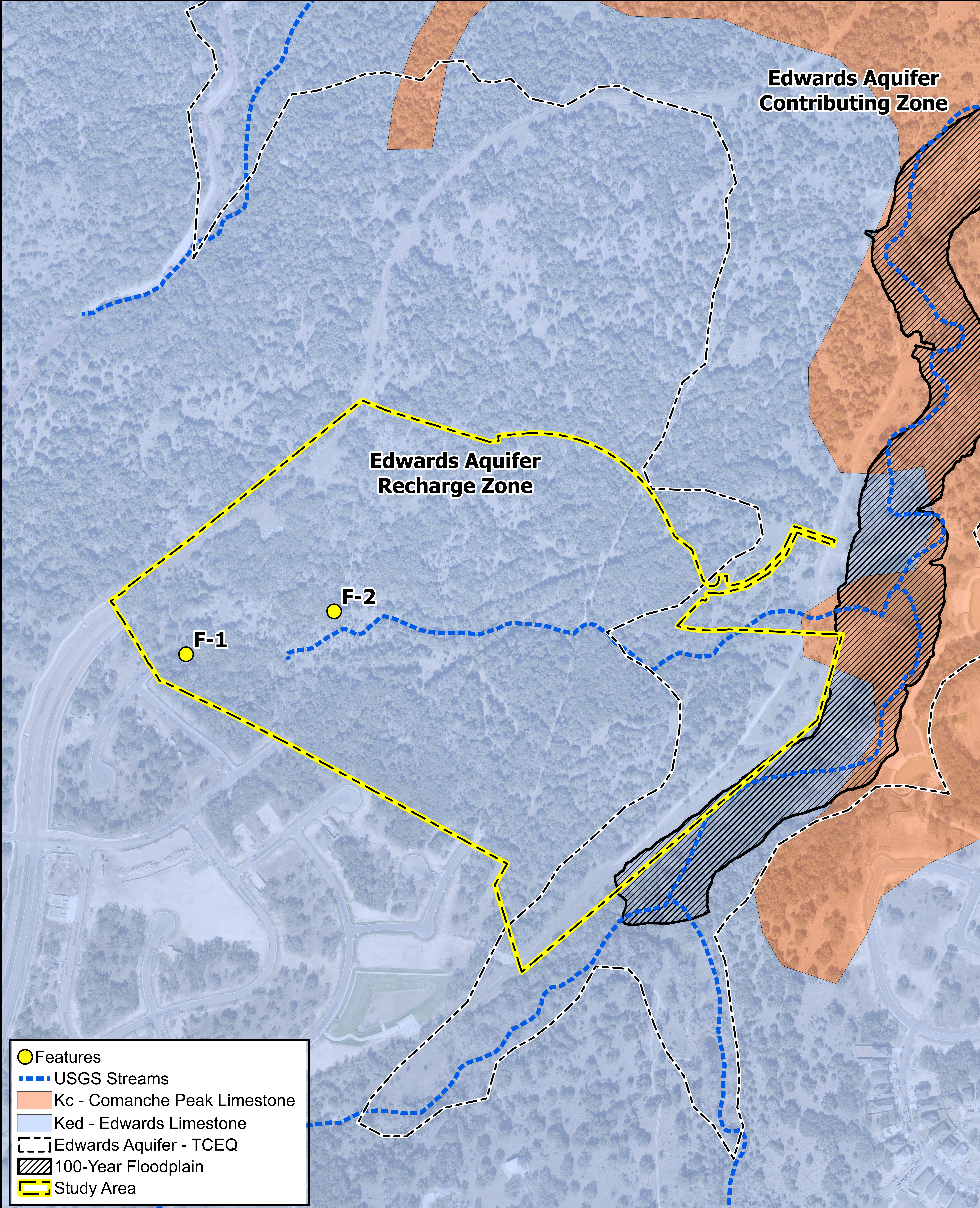
0 2,500 5,000 7,500 10,000 Feet

Coordinate System: NAD  
1983 StatePlane Texas South  
Central FIPS 4204 Feet









- Features
- USGS Streams
- Kc - Comanche Peak Limestone
- Ked - Edwards Limestone
- Edwards Aquifer - TCEQ
- ▨ 100-Year Floodplain
- Study Area

1 inch = 100 feet

0 250 500 750 1,000 Feet

Coordinate System: NAD 1983  
StatePlane Texas South Central  
FIPS 4204 Feet

Site Geologic Map





# Water Pollution Abatement Plan Application

## Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b), Effective June 1, 1999

*To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.*

*Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.*


## Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Water Pollution Abatement Plan Application Form** is hereby submitted for TCEQ review and Executive Director approval. The form was prepared by:

Print Name of Customer/Agent: Christine Campbell, P.E.

Date: 12/19/2023

Signature of Customer/Agent:



Regulated Entity Name: Parkside on the River Sections 9A & 10A

## Regulated Entity Information

1. The type of project is:

- ☒ Residential: Number of Lots: 103
- ☐ Residential: Number of Living Unit Equivalents:
- ☐ Commercial
- ☐ Industrial
- ☐ Other:

2. Total site acreage (size of property): 34.42 (LOC - 33.14 acres)

3. Estimated projected population: 103 units \* 3.5 people / unit = 360.5 people

4. The amount and type of impervious cover expected after construction are shown below:

**Table 1 - Impervious Cover Table**

Impervious Cover of Proposed Project	Sq. Ft.	Sq. Ft./Acre	Acres
Structures/Rooftops	428,400	$\div 43,560 =$	9.83
Parking	-	$\div 43,560 =$	-
Other paved surfaces	285,550	$\div 43,560 =$	6.56
Total Impervious Cover	713,950	$\div 43,560 =$	16.39

**Total Impervious Cover 16.39  $\div$  Total Acreage 34.42 X 100 = 47.6% Impervious Cover**

5. ☒ **Attachment A - Factors Affecting Surface Water Quality.** A detailed description of all factors that could affect surface water and groundwater quality that addresses ultimate land use is attached.
6. ☒ Only inert materials as defined by 30 TAC §330.2 will be used as fill material.

### ***For Road Projects Only***

**Complete questions 7 - 12 if this application is exclusively for a road project.**

7. Type of project:

- ☐ TXDOT road project.
- ☐ County road or roads built to county specifications.
- ☐ City thoroughfare or roads to be dedicated to a municipality.
- ☐ Street or road providing access to private driveways.

8. Type of pavement or road surface to be used:

- ☐ Concrete
- ☐ Asphaltic concrete pavement
- ☐ Other: \_\_\_\_\_

9. Length of Right of Way (R.O.W.): \_\_\_\_\_ feet.

Width of R.O.W.: \_\_\_\_\_ feet.

$L \times W =$  \_\_\_\_\_  $\text{Ft}^2 \div 43,560 \text{ Ft}^2/\text{Acre} =$  \_\_\_\_\_ acres.

10. Length of pavement area: \_\_\_\_\_ feet.

Width of pavement area: \_\_\_\_\_ feet.

$L \times W =$  \_\_\_\_\_  $\text{Ft}^2 \div 43,560 \text{ Ft}^2/\text{Acre} =$  \_\_\_\_\_ acres.

Pavement area \_\_\_\_\_ acres  $\div$  R.O.W. area \_\_\_\_\_ acres  $\times 100 =$  \_\_\_\_\_ % impervious cover.

11. ☐ A rest stop will be included in this project.

☐ A rest stop will not be included in this project.

12. ☐ Maintenance and repair of existing roadways that do not require approval from the TCEQ Executive Director. Modifications to existing roadways such as widening roads/adding shoulders totaling more than one-half (1/2) the width of one (1) existing lane require prior approval from the TCEQ.

### ***Stormwater to be generated by the Proposed Project***

13. ☒ **Attachment B - Volume and Character of Stormwater.** A detailed description of the volume (quantity) and character (quality) of the stormwater runoff which is expected to occur from the proposed project is attached. The estimates of stormwater runoff quality and quantity are based on the area and type of impervious cover. Include the runoff coefficient of the site for both pre-construction and post-construction conditions.

### ***Wastewater to be generated by the Proposed Project***

14. The character and volume of wastewater is shown below:

<u>100%</u> Domestic	<u>25,750</u> Gallons/day
<u>      </u> % Industrial	<u>      </u> Gallons/day
<u>      </u> % Commingled	<u>      </u> Gallons/day
TOTAL gallons/day <u>25,750</u>	

15. Wastewater will be disposed of by:

☐ On-Site Sewage Facility (OSSF/Septic Tank):

☐ **Attachment C - Suitability Letter from Authorized Agent.** An on-site sewage facility will be used to treat and dispose of the wastewater from this site. The appropriate licensing authority's (authorized agent) written approval is attached. It states that the land is suitable for the use of private sewage facilities and will meet or exceed the requirements for on-site sewage facilities as specified under 30 TAC Chapter 285 relating to On-site Sewage Facilities.

☐ Each lot in this project/development is at least one (1) acre (43,560 square feet) in size. The system will be designed by a licensed professional engineer or registered sanitarian and installed by a licensed installer in compliance with 30 TAC Chapter 285.

☒ Sewage Collection System (Sewer Lines):

☐ Private service laterals from the wastewater generating facilities will be connected to an existing SCS.

☐ Private service laterals from the wastewater generating facilities will be connected to a proposed SCS.

☐ The SCS was previously submitted on       .

☒ The SCS was submitted with this application.

☐ The SCS will be submitted at a later date. The owner is aware that the SCS may not be installed prior to Executive Director approval.

☒ The sewage collection system will convey the wastewater to the Dove Springs (name) Treatment Plant. The treatment facility is:

☒ Existing.

☐ Proposed.

16. ☒ All private service laterals will be inspected as required in 30 TAC §213.5.

## **Site Plan Requirements**

**Items 17 – 28 must be included on the Site Plan.**

17. ☒ The Site Plan must have a minimum scale of 1" = 400'.

Site Plan Scale: 1" = 100'.

18. 100-year floodplain boundaries:

☐ Some part(s) of the project site is located within the 100-year floodplain. The floodplain is shown and labeled.

☒ No part of the project site is located within the 100-year floodplain.

The 100-year floodplain boundaries are based on the following specific (including date of material) sources(s): FEMA FIRM Panel No. 48491C0460F, 12/20/2019

19. ☒ The layout of the development is shown with existing and finished contours at appropriate, but not greater than ten-foot contour intervals. Lots, recreation centers, buildings, roads, open space, etc. are shown on the plan.

☐ The layout of the development is shown with existing contours at appropriate, but not greater than ten-foot intervals. Finished topographic contours will not differ from the existing topographic configuration and are not shown. Lots, recreation centers, buildings, roads, open space, etc. are shown on the site plan.

20. All known wells (oil, water, unplugged, capped and/or abandoned, test holes, etc.):

☐ There are \_\_\_\_\_ (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply)

☐ The wells are not in use and have been properly abandoned.

☐ The wells are not in use and will be properly abandoned.

☐ The wells are in use and comply with 16 TAC §76.

☒ There are no wells or test holes of any kind known to exist on the project site.

21. Geologic or manmade features which are on the site:

☒ All sensitive geologic or manmade features identified in the Geologic Assessment are shown and labeled.

☐ No sensitive geologic or manmade features were identified in the Geologic Assessment.

☐ **Attachment D - Exception to the Required Geologic Assessment.** A request and justification for an exception to a portion of the Geologic Assessment is attached.

- 22. ☒ The drainage patterns and approximate slopes anticipated after major grading activities.
- 23. ☒ Areas of soil disturbance and areas which will not be disturbed.
- 24. ☒ Locations of major structural and nonstructural controls. These are the temporary and permanent best management practices.
- 25. ☒ Locations where soil stabilization practices are expected to occur.
- 26. ☐ Surface waters (including wetlands).  
☒ N/A
- 27. ☐ Locations where stormwater discharges to surface water or sensitive features are to occur.  
☒ There will be no discharges to surface water or sensitive features.
- 28. ☒ Legal boundaries of the site are shown.

### ***Administrative Information***

- 29. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
- 30. ☒ Any modification of this WPAP will require Executive Director approval, prior to construction, and may require submission of a revised application, with appropriate fees.



## ATTACHMENT A – FACTORS AFFECTING WATER QUALITY

Potential sources of pollution that may be expected to affect the quality of the storm water discharges from the construction site include the following:

- Soil erosion due to the clearing of the site for wastewater improvements.
- Oil, grease, fuel and hydraulic fluid contamination from construction equipment and vehicle drippings.
- Hydrocarbons from asphalt paving operations.
- Miscellaneous trash and litter from construction.

Potential sources of pollution that may be expected to affect the quality of the storm water discharges from the site after construction is completed include the following:

- Oil, grease, fuel and hydraulic fluid contamination from vehicle drippings.
- Dirt and dust from vehicles.
- Trash and litter.

## ATTACHMENT B – VOLUME AND CHARACTER OF STORMWATER

The project site is primarily undeveloped wooded land with grass. Runoff flows towards the South Fork San Gabriel River. No portion of the project site is located within the 100-year floodplain as defined by FEMA FIRM Panel No. 48491C0460F, dated December 20, 2019.

The proposed development results in an impervious cover of approximately 47.6% and will have the associated runoff treated by the batch detention pond associated with Parkside on the River Section 8. Of the 34.42 acres of the proposed Parkside on the River Section 9A & 10A property, there is approximately 16.28 acres of impervious cover. There is an additional 0.11 acres of impervious cover proposed for a temporary turnaround, resulting in a total of approximately 16.39 acres of proposed impervious cover for this project. Based on the 80% TSS removal requirement by TCEQ we need to provide 14,266 lbs of TSS removal for the proposed development. As shown in the calculations, the Section 8 pond satisfies the TSS removal requirement. The 85% TSS removal requirement by the City of Georgetown is also satisfied by the Section 8 batch detention pond.

The proposed conditions for the overall area propose approximately 30.56 acres of post-development impervious cover, of which approximately 1.60 acres are existing from Parkside on the River Phase 3 Sections 4, 7A & 7B, 12.57 acres from Parkside on the River Section 8, and 16.39 acres proposed with Parkside on the River Sections 9A & 10A. Based on the 80% TSS removal requirement by TCEQ we need to provide 25,207 lbs of TSS removal in the proposed case. As shown in the calculations, the Section 8 pond and vegetative filter strip satisfy this requirement. The 85% TSS removal requirement by the City of Georgetown is also satisfied for the Section 8 batch detention pond. In the proposed condition, the Section 8 batch detention pond (BDP-01) will treat a total of 25.88 acres of impervious cover (1.60 acres of existing impervious cover from Sections 4, 7A & 7B, 10.02 acres of impervious cover from Section 8, and 14.26 acres of proposed impervious cover from Sections 9A & 10A) and provide 23,750 lbs of TSS removal. Approximately 2.13 acres of impervious cover proposed with Sections 9A & 10A is bypassing treatment. The BMPs are overtreating to account for the bypass impervious cover.

Refer to the construction plans for the water quality calculations and the attached Parkside on the River Section 8 plans for the batch detention pond design. Refer to the table below for the proposed sedimentation treatment breakdown provided.

Detailed existing and proposed flow data for the points of interest are provided on the drainage plan as part of the construction documents submitted with this application.

Storm drainage will be captured in the proposed curb inlets and drain to the Section 8 batch detention pond.

PARKSIDE ON THE RIVER SECTIONS 9A & 10A - TSS REMOVAL SUMMARY - PROPOSED													
DRAINAGE AREA	BMP TYPE	MAX TSS REMOVAL EFFICIENCY	BASIN AREA	PRE- DEVELOPMENT I.C.	PROPOSED I.C.		POST-DEVELOPMENT I.C.		TCEQ REQUIRED 80% TSS LOAD REMOVAL	CITY OF GEORGETOWN REQUIRED 85% POND TSS LOAD REMOVAL	PROVIDED TSS LOAD REMOVAL	VOLUME REQUIRED	VOLUME PROVIDED
					SECTION 8	SECTION 9A 10A							
			AC	AC	AC	AC	AC	%	LB	LB	LB	CF	CF
BDP-01	BATCH DETENTION POND	91%	57.62	1.60	10.02	14.26	25.88	45%	21,133	22,454	23,750	132,879	137,853
VFS-01	VEGETATIVE FILTER STRIP	85%	4.00	0.00	1.92		1.92	48%	1,671		1,838		
BP-01	BY-PASS	0%	1.17	0.00	0.63		0.63	54%	548				
BP-02	BY-PASS	0%	2.90	0.00		1.48	1.48	51%	1,288				
BP-03	BY-PASS	0%	1.33	0.00		0.65	0.65	49%	566				
TOTAL:			67.02	1.60	12.57	16.39	30.56	46%	25,207		25,588		
1 - FOR THE GEORGETOWN TSS REMOVAL REQUIREMENT, WE CONSIDER 85% OF TSS REMOVAL FOR THE DRAINAGE AREA THAT DRAINS TOWARD THE BATCH DETENTION PONDS.													

# Temporary Stormwater Section

## Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

***To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.***

***Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.***


## Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: Christine Campbell, P.E.

Date: 12/19/2023

Signature of Customer/Agent:



Regulated Entity Name: Parkside on the River Sections 9A & 10A

## Project Information

### Potential Sources of Contamination

*Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.*

1. Fuels for construction equipment and hazardous substances which will be used during construction:

☐ The following fuels and/or hazardous substances will be stored on the site: \_\_\_\_\_

These fuels and/or hazardous substances will be stored in:

- ☐ Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

- ☐ Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year.
- ☐ Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
- ☒ Fuels and hazardous substances will not be stored on the site.
- 2. ☒ **Attachment A - Spill Response Actions.** A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
- 3. ☒ Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
- 4. ☒ **Attachment B - Potential Sources of Contamination.** A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.

### ***Sequence of Construction***

- 5. ☒ **Attachment C - Sequence of Major Activities.** A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
  - ☒ For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given.
  - ☐ For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
- 6. ☒ Name the receiving water(s) at or near the site which will be disturbed or which will receive discharges from disturbed areas of the project: San Gabriel River

### ***Temporary Best Management Practices (TBMPs)***

*Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.*

- 7. ☒ **Attachment D – Temporary Best Management Practices and Measures.** TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

- ☒ A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.
  - ☒ A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.
  - ☒ A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer.
  - ☒ A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.
8. ☒ The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
- ☐ **Attachment E - Request to Temporarily Seal a Feature.** A request to temporarily seal a feature is attached. The request includes justification as to why no reasonable and practicable alternative exists for each feature.
- ☒ There will be no temporary sealing of naturally-occurring sensitive features on the site.
9. ☒ **Attachment F - Structural Practices.** A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.
10. ☒ **Attachment G - Drainage Area Map.** A drainage area map supporting the following requirements is attached:
- ☒ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided.
  - ☐ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used.
  - ☐ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area.
  - ☐ There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.

- ☐ There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. Erosion and sediment controls other than sediment basins or sediment traps within each disturbed drainage area will be used.
11. ☒ **Attachment H - Temporary Sediment Pond(s) Plans and Calculations.** Temporary sediment pond or basin construction plans and design calculations for a proposed temporary BMP or measure have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information must be signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed temporary BMPs and measures are attached.
- ☐ N/A
12. ☒ **Attachment I - Inspection and Maintenance for BMPs.** A plan for the inspection of each temporary BMP(s) and measure(s) and for their timely maintenance, repairs, and, if necessary, retrofit is attached. A description of the documentation procedures, recordkeeping practices, and inspection frequency are included in the plan and are specific to the site and/or BMP.
13. ☒ All control measures must be properly selected, installed, and maintained in accordance with the manufacturer's specifications and good engineering practices. If periodic inspections by the applicant or the executive director, or other information indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations.
14. ☒ If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain).
15. ☒ Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50%. A permanent stake will be provided that can indicate when the sediment occupies 50% of the basin volume.
16. ☒ Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source for stormwater discharges (e.g., screening outfalls, picked up daily).

## ***Soil Stabilization Practices***

*Examples: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.*

17. ☒ **Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices.** A schedule of the interim and permanent soil stabilization practices for the site is attached.



18. ☒ Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
19. ☒ Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

### ***Administrative Information***

20. ☒ All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
21. ☒ If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
22. ☒ Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

## ATTACHMENT A – SPILL RESPONSE ACTIONS

The objective of this section is to describe measures to prevent or reduce the discharge of pollutants to drainage systems or watercourses. Measures include reducing the chance for spills, stopping the source of spills, containing and cleaning up spills, properly disposing of spill materials, and training employees.

### The following practices will be followed for spill prevention and cleanup:

- Manufacturers' recommended methods for spill cleanup will be clearly posted and site personnel will be made aware of the procedures and the location of the information and cleanup supplies.
- Materials and equipment necessary for spill cleanup will be kept in the material storage area onsite. Equipment and materials will include but not be limited to brooms, dustpans, mops, rags, gloves, goggles, kitty litter, sand, sawdust, and plastic and metal trash containers specifically for this purpose.
- All spills will be cleaned up immediately after discovery.
- The spill area will be kept well ventilated and personnel will wear appropriate protective clothing to prevent injury from contact with a hazardous substance.
- Spills of toxic or hazardous material will be reported to the Owner and to the appropriate State or local government agency, regardless of the size.
- The spill prevention plan will be adjusted to include measures to prevent this type of spill from reoccurring and how to clean up the spill if there is another one. A description of the spill, what caused it, and the cleanup measures will also be included.
- The site superintendent responsible for the day-to-day site operations will be the spill prevention and cleanup coordinator. He will designate at least three other site personnel who will receive spill prevention and cleanup training. These individuals will each become responsible for a particular phase of prevention and cleanup. The names of responsible spill personnel will be posted in the material storage area and in the office trailer onsite.
- Any reportable quantity hydrocarbon or hazardous material spill should be reported to the TCEQ at the following 24-hour toll free number 1-800-832-8224.

### For a spill of Reportable Quantity:

- Initial notification. Upon the determination that a reportable discharge or spill has occurred, the responsible person shall notify the agency as soon as possible but not later than 24 hours after the discovery of the spill or discharge.
- Method of notification. The responsible person shall notify the agency in any reasonable manner including by telephone, in person, or by any other method approved by the agency. In all cases, the initial notification shall provide, to the extent known, the information listed in subsection (d) of Title 30, Part I, Chapter 327, Rule §327.3. Notice provided under this section satisfies the federal requirement to notify the State Emergency Response Commission in the State of Texas.
- Notification of local government authorities. If the discharge or spill creates an imminent health threat, the responsible person shall immediately notify and cooperate with local emergency authorities. The responsible party will cooperate with the local emergency authority in providing support to implement appropriate notification and response actions. The local emergency authority, as necessary, will implement its emergency management plan, which may include notifying and evacuating affected persons. In the absence of a local emergency authority, the responsible person shall take reasonable measures to notify potentially affected persons of the imminent health threat.
- As soon as possible, but no later than two (2) weeks after discovery of the spill or discharge, the Contractor shall reasonably attempt to notify the Owner (if identifiable) or Occupant of the property upon which the discharge or spill occurred as well as the occupants of any property that the Contractor believes is adversely affected.

More information on spill rules and appropriate responses is available on the TCEQ website at:  
<http://www.tceq.texas.gov/response/>

#### Vehicle and Equipment Maintenance:

- If maintenance must occur onsite, use a designated area and a secondary containment, located away from drainage courses, to prevent the runoff of stormwater and the runoff of spills.
- Regularly inspect onsite vehicles and equipment for leaks and repair immediately.
- Check incoming vehicles and equipment (including delivery trucks, and employee and subcontractor vehicles) for leaking oil and fluids. Do not allow leaking vehicles or equipment onsite.
- Always use secondary containment, such as drain pan or drop cloth, to catch spills or leaks when removing or changing fluids.
  - Place drip pans or absorbent materials under paving equipment when not in use.
  - Use absorbent materials on small spills rather than hosing down or burying the spill. Remove the absorbent materials promptly and dispose of properly.
  - Promptly transfer used fluids to the proper waste or recycling drums. Do not leave full drip pans or other containers lying around.
  - Oil filters disposed of in trashcans or dumpsters can leak oil and pollute stormwater. Place the oil filter in a funnel over the waste oil-recycling drum to drain excess oil before disposal. Oil filters can also be recycled. Ask the oil supplier or recycler about recycling oil filters.
  - Store cracked batteries in a non-leaking secondary container. Do this with all cracked batteries even if you think all of the acid has drained out. If you drop a battery, treat it as if it is cracked. Put it into the containment area until you are sure it is not leaking.

### **ATTACHMENT B – POTENTIAL SOURCES OF CONTAMINATION**

Once grading activities begin, erosion of bare soil during rainfall events is the most common source of contamination. Silt fences will be installed at the beginning of the grading operation to minimize the potential for transport of the soil offsite.

Asphalt products will be used on this project. After placement of asphalt, emulsion, or coatings, the applicant will be responsible for immediate cleanup should an unexpected rain occur. For the duration of the asphalt curing time, the applicant should maintain standby personnel and equipment to contain any asphalt wash-off should an unexpected rain occur.

During construction activities, potential sources of contamination would include petroleum products leaking from construction equipment. The contractor will be advised to keep the equipment in working order and report any spills per the spill response plan.

Other potential sources of contamination include hydraulic fluid and diesel fuel from mechanical equipment and vehicles, as well as paints and chemicals used on site. Any spills shall be handled according to the Spill Response Actions in Attachment A.

### **ATTACHMENT C – SEQUENCE OF MAJOR ACTIVITIES**

The first activity of construction will be to install the erosion control measures, consisting of silt fences, tree protection, storm drains, inlet protection, rock berm, and a stabilized construction entrance. Temporary erosion control measures will remain in place throughout the duration of construction and will be required to be maintained by the contractor to ensure proper functionality, especially after storm events. All disturbed areas to remain pervious will be vegetated using the procedures detailed in the construction plans and all temporary erosion control measures will be removed upon revegetation. Construction activities associated with this application is expected to disturb 33.14 acres of the site.

#### Major Construction Activities and Sequencing:

The major construction activities for this project will include and be sequenced as follows:

1. Established Best Management Practices shall consist of the following: silt fencing, temporary spoils areas, concrete truck washout pits, and a temporary construction entrance (Estimated area to be disturbed = 0.58 Acres). These items are to remain and be maintained throughout all construction activities.

2. Initial site mass grading operation including right-of-way and first grading. (Estimated area to be disturbed = 11.10 Acres)
3. Installation of utilities including storm, water, and wastewater (Estimated area to be disturbed = 0.91 Acres)
4. Construction of street/driveway pavement including backfill behind curbs (estimated area to be disturbed = 5.72 Acres)
5. Total Construction (estimated area to be disturbed = 33.14 Acres)
6. Final soil stabilization for the site and removal of temporary BMPs once the soil has been stabilized.

The contractor is responsible for implementing and maintaining the storm water pollution prevention plan which includes maintaining all the necessary erosion controls throughout construction.

#### **ATTACHMENT D – TEMPORARY BEST MANAGEMENT PRACTICES AND MEASURES**

As shown on the Construction Erosion Control Plans, temporary BMP practices and measures will include installing silt fences, inlet protection, rock berm, a stabilized construction entrance, a concrete truck washout, and a temporary spoils area prior to beginning grading operations on the site. Temporary measures are intended to provide a method of slowing the upgradient flow, onsite flow or runoff from the construction site in order to allow sediment and suspended solids to settle out of the water. By containing the sediment and solids within the site, they will not enter surface streams and/or sensitive features. As a temporary BMP, a silt fence will be installed to reduce pollutants. BMP measures utilized in this plan are intended to allow storm water to continue downstream after passing through for treatment.

##### Site Preparation:

The methodology for pollution prevention of all on-site stormwater will include a) the erection of silt fences along the downgradient boundary of the construction activities, b) installation of inlet protection at all inlets, c) installation of a stabilized construction entrance to reduce the dispersion of sediment from the site, and d) installation of a construction staging area.

##### Construction:

All installed erosion control measure will be inspected, and if necessary, repaired before any additional construction begins, as well as periodically throughout the construction process. The contractor will be responsible for all maintenance of erosion control measures, as well as the installation of all remaining on-site control measures, including the concrete truck washout, as necessary.

#### **ATTACHMENT E – REQUEST TO TEMPORARILY SEAL A FEATURE**

There is one sensitive feature on-site within Parkside on the River Sections 9A & 10A as shown in the geologic assessment and construction plans. Sensitive feature F-2 is located on site and has a buffer extending 50' in all directions. There will be no sealing of sensitive features on the site.

#### **ATTACHMENT F – STRUCTURAL PRACTICES**

Most of the site flows and upgradient run off will encounter the Section 8 batch detention pond. There is roughly 2.13 acres of impervious cover in Parkside on the River Sections 9A & 10A that will bypass treatment. The BMPs are overtreating to account for the bypass impervious cover.

#### **ATTACHMENT G – DRAINAGE AREA MAPS**

Refer to the construction plans attached.

#### **ATTACHMENT H – TEMPORARY SEDIMENT POND(S) PLANS AND CALCULATIONS**

The batch detention pond will act as a temporary and permanent sedimentation pond. The Section 8 pond (BDP-01) provides 137,853 CF of water quality volume.

The calculated temporary sedimentation pond volume required is calculated below.

Calculation: Required Volume = (Rainfall Depth\*Runoff Coefficient\*Drainage Area\*120%)  
= 1.60 in. \* 0.33 \* 57.62 acres \* 120%  
= 132,524 CF

## ATTACHMENT I – INSPECTION AND MAINTENANCE FOR BMPs

See construction plans included with this application submittal.

Temporary Best Management Practices (BMPs) and measures will be used during construction to prevent pollution of groundwater, surface water and naturally occurring environmental features. Silt fence, inlet protection, stabilized construction entrance, tree protection, concrete washout area, and a temporary spoils area will be installed prior to beginning construction and prior to commencement of any of the activities defined in the sequence of construction as Attachment C. Inspection and maintenance of the on-site controls shall be performed during the site clearing and rough grading process. Weekly inspections will be documented in an inspection report. The inspection reports will document maintenance activities, sediment removal, and any modifications to the erosion and sedimentation controls. The perimeter fence shall be regularly monitored to ensure that the buffers remain no-construction zones until the site work has been completed and authorization has been granted by the engineer. Refer to the construction plans attached for specific controls and details.

BMPs and measures will prevent pollution of surface water or groundwater that originates on site or flows off-site, including pollution caused by contaminated stormwater run-off from the site, and through the use of silt fences placed immediately downstream of disturbed areas and inlet protection at all inlets. To minimize destruction to any portion of the Recharge Zone, on-site perimeter silt fence will also be implemented for pertinent areas throughout the entirety of construction. The Contractor is expected to inspect the controls weekly and after significant rainfalls to ensure proper function. When silt accumulates six (6) inches in depth the Contractor shall promptly remove the silt from the controls.

BMPs and measures will prevent pollutants from entering surface streams or the aquifer by intercepting stormwater potentially carrying sediment and other pollutants. BMPs and measures will implement a stabilized construction entrance, a construction stockpiling/staging area, and a concrete washout area to help minimize pollutant run-off and erosion generated during construction. Paved streets and driveways adjacent to these sites will be cleaned regularly to remove excess mud, dirt or rock tracked from the site. Sedimentation will be concentrated only in these areas for efficient maintenance. Water trucks will be on-site as necessary to aid in cleaning regularly to remove excess mud, dirt or rock tracked from the site. Sedimentation will be concentrated only in these areas for efficient maintenance. Water trucks will be on-site as necessary to aid in controlling dust. BMPs will be implemented to limit/prevent contaminated inflow from entering surface streams or the aquifer. These practices are to include the following measures: the use of silt fence and inlet protection. The fabricated silt fence barricade will provide help to reduce the likelihood of contaminated runoff from entering the aquifer. If any sensitive features are identified by TCEQ inspections, or during excavation or construction, measures appropriate to the sensitivity of the discovered feature will be enacted. No blasting is proposed.

### Temporary Erosion and Sedimentation Notes:

1. The Contractor shall maintain, install erosion/sedimentation controls and tree/natural protective fencing prior to any site preparation work (clearing, grubbing or excavation).
2. The placement of erosion/sedimentation controls and tree/natural area protective fencing shall be in accordance with the TCEQ Technical Guidance Manual and the approved Erosion and Sedimentation Control Plan. No erosion controls shall be placed beyond the property lines of the site unless written permission has been obtained from adjacent property owners.
3. A pre-construction conference shall be held on-site with the Contractor, design engineer/permit applicant and Environmental Inspector after installation of the erosion/sedimentation and tree/natural area protection measures and prior to beginning any site preparation work. The Contractor shall notify the Environmental Inspector at least three (3) days prior to the meeting date.

4. Any major variation in materials or locations of controls or fences from those shown on the approved plans will require a revision and must be approved by the reviewing engineer, environmental specialist or city arborist as appropriate. Minor changes to be made as field revisions to the Erosion and Sedimentation Control Plan may be required by the Environmental Inspector during the course of construction to correct control inadequacies.
5. The Contractor is required to inspect the controls at weekly intervals and after significant rainfall events to ensure that they are functioning properly. The person(s) responsible for maintenance of controls shall immediately make any necessary repairs to damaged areas. Silt accumulation at controls must be removed when the depth reaches six (6) inches.
6. Prior to final acceptance by the City, haul roads and waterway crossing constructed for temporary Contractor access must be removed, accumulated sediment removed from the waterway and the area restored to the original grade and revegetated. All land clearing debris shall be disposed of in approved soil disposal sites.
7. All work must stop if a void in the rock substrate is discovered, which is one (1) square foot in total area, blows air from within the substrate, and/or consistently received water during any rain event. At this time it is the responsibility of the project manager to immediately contact an Environmental Inspector for further investigation.
8. All slopes shall be sodded or seeded with approved grass, grass mixtures or ground cover suitable to the area and season in which they are applied.
9. Silt fences, rock berms, sedimentation basins and similarly recognized techniques and materials shall be employed during construction to prevent point source sedimentation loading of downstream facilities. Such installation shall be regularly inspected for effectiveness. Additional measures may be required if, in the opinion of the City Engineer, they are warranted.
10. All temporary erosion control measures shall not be removed until final inspection and approval of the project by the engineer. It shall be the responsibility of the Contractor to maintain all temporary erosion control structures and to remove each structure as approved by the engineer.
11. Any dirt, mud, rocks, debris, etc., that is spilled, tracked, or otherwise deposited on any existing paved street shall be cleaned up immediately.

#### Dewatering Operations

1. Inspect and verify that activity-based BMPs are in place prior to the commencement of associated activities. While activities associated with the BMP area under way, inspect weekly to verify continued BMP implementation.
2. Inspect BMPs subject to non-stormwater discharges daily while non-stormwater discharges occur.
3. Unit-specific maintenance requirements are included with the description of each technology.
4. Sediment removed during the maintenance of a dewatering device may be either spread onsite and stabilized, or disposed of at a disposal site.
5. Sediment that is commingled with other pollutants must be disposed of in accordance with all applicable laws and regulations.

#### **ATTACHMENT J – SCHEDULE OF INTERIM AND PERMANENT SOIL STABILIZATION PRACTICES**

Contractors will ensure that existing vegetation is preserved where attainable and that disturbed portions of the site will be stabilized. Stabilization practices may include but are not limited to temporary seeding, permanent seeding, mulching, geotextiles, sodding, tree protection, preservation of natural vegetation and other appropriate measures. All slopes shall be sodded or seeded with approved grass, grass mixtures or ground cover suitable to the area and season in which they are applied. Except as noted below, stabilization shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than 14 days after the activity has temporarily or permanently ceased. Refer to the construction plans attached for the TCEQ Notes, the Existing Conditions & Tree Survey, and the Erosion & Sedimentation Control Plan.



# Permanent Stormwater Section

## Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(C), (D)(li), (E), and (5), Effective June 1, 1999

***To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.***

***Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.***

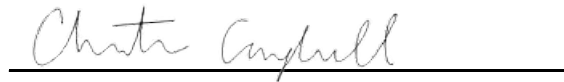
## Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Permanent Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: Christine Campbell, P.E.

Date: 12/19/2023

Signature of Customer/Agent



Regulated Entity Name: Parkside on the River Sections 9A & 10A

## Permanent Best Management Practices (BMPs)

***Permanent best management practices and measures that will be used during and after construction is completed.***

1. ☒ Permanent BMPs and measures must be implemented to control the discharge of pollution from regulated activities after the completion of construction.  
☐ N/A
2. ☒ These practices and measures have been designed, and will be constructed, operated, and maintained to insure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity is removed. These quantities have been calculated in accordance with technical guidance prepared or accepted by the executive director.  
☒ The TCEQ Technical Guidance Manual (TGM) was used to design permanent BMPs and measures for this site.

- ☐ A technical guidance other than the TCEQ TGM was used to design permanent BMPs and measures for this site. The complete citation for the technical guidance that was used is: \_\_\_\_\_
- ☐ N/A
3. ☒ Owners must insure that permanent BMPs and measures are constructed and function as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the appropriate regional office within 30 days of site completion.
- ☐ N/A
4. Where a site is used for low density single-family residential development and has 20 % or less impervious cover, other permanent BMPs are not required. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
- ☐ The site will be used for low density single-family residential development and has 20% or less impervious cover.
- ☐ The site will be used for low density single-family residential development but has more than 20% impervious cover.
- ☒ The site will not be used for low density single-family residential development.
5. The executive director may waive the requirement for other permanent BMPs for multi-family residential developments, schools, or small business sites where 20% or less impervious cover is used at the site. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
- ☐ **Attachment A - 20% or Less Impervious Cover Waiver.** The site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is attached.
- ☐ The site will be used for multi-family residential developments, schools, or small business sites but has more than 20% impervious cover.
- ☒ The site will not be used for multi-family residential developments, schools, or small business sites.
6. ☒ **Attachment B - BMPs for Upgradient Stormwater.**

- ☒ A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is attached.
  - ☐ No surface water, groundwater or stormwater originates upgradient from the site and flows across the site, and an explanation is attached.
  - ☐ Permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site, and an explanation is attached.
7. ☒ **Attachment C - BMPs for On-site Stormwater.**
- ☒ A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is attached.
  - ☐ Permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, and an explanation is attached.
8. ☒ **Attachment D - BMPs for Surface Streams.** A description of the BMPs and measures that prevent pollutants from entering surface streams, sensitive features, or the aquifer is attached. Each feature identified in the Geologic Assessment as sensitive has been addressed.
- ☐ N/A
9. ☒ The applicant understands that to the extent practicable, BMPs and measures must maintain flow to naturally occurring sensitive features identified in either the geologic assessment, executive director review, or during excavation, blasting, or construction.
- ☒ The permanent sealing of or diversion of flow from a naturally-occurring sensitive feature that accepts recharge to the Edwards Aquifer as a permanent pollution abatement measure has not been proposed.
  - ☐ **Attachment E - Request to Seal Features.** A request to seal a naturally-occurring sensitive feature, that includes, for each feature, a justification as to why no reasonable and practicable alternative exists, is attached.
10. ☒ **Attachment F - Construction Plans.** All construction plans and design calculations for the proposed permanent BMP(s) and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer, and are signed, sealed, and dated. The plans are attached and, if applicable include:
- ☒ Design calculations (TSS removal calculations)
  - ☒ TCEQ construction notes
  - ☒ All geologic features
  - ☒ All proposed structural BMP(s) plans and specifications
- ☐ N/A

11. ☒ **Attachment G - Inspection, Maintenance, Repair and Retrofit Plan.** A plan for the inspection, maintenance, repairs, and, if necessary, retrofit of the permanent BMPs and measures is attached. The plan includes all of the following:
- ☒ Prepared and certified by the engineer designing the permanent BMPs and measures
  - ☒ Signed by the owner or responsible party
  - ☒ Procedures for documenting inspections, maintenance, repairs, and, if necessary retrofit
  - ☒ A discussion of record keeping procedures
- ☐ N/A
12. ☐ **Attachment H - Pilot-Scale Field Testing Plan.** Pilot studies for BMPs that are not recognized by the Executive Director require prior approval from the TCEQ. A plan for pilot-scale field testing is attached.
- ☒ N/A
13. ☐ **Attachment I - Measures for Minimizing Surface Stream Contamination.** A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity, which increase erosion that results in water quality degradation.
- ☒ N/A

## ***Responsibility for Maintenance of Permanent BMP(s)***

***Responsibility for maintenance of best management practices and measures after construction is complete.***

14. ☒ The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
- ☐ N/A
15. ☒ A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development, or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.
- ☐ N/A

### **ATTACHMENT B – BMP’S FOR UPGRADIENT STORMWATER**

There is no upgradient, offsite flow that will be captured in the proposed storm infrastructure and routed to the BMPs.

### **ATTACHMENT C – BMP’S FOR ON-SITE STORMWATER**

Onsite areas considered in this description are all part of the overall Parkside on the River development. The water flows towards the South Fork San Gabriel River. The proposed infrastructure is sized to treat a minimum 80% of the TSS as defined by the TCEQ and 85% of the batch detention pond TSS as defined by the City of Georgetown. In the proposed condition, the Section 8 batch detention pond (BDP-01) is estimated to treat a total of 25.88 acres of impervious cover (1.60 acres of existing impervious cover from Sections 4, 7A & 7B, 10.02 acres of impervious cover from Section 8, and 14.26 acres of proposed impervious cover from Sections 9A & 10A) and provide 23,750 lbs of TSS removal. Approximately 2.13 acres of impervious cover proposed with Sections 9A & 10A is bypassing treatment. The BMPs are overtreating to account for the bypass impervious cover.

Refer to the Construction Plans for the sediment treatment details.

### **ATTACHMENT D – BMP’S FOR SURFACE STREAMS**

There is a mapped stream south of the property, in Parkside on the River Section 8. This stream has a catchment area of less than 64 acres and appears to only flow during heavy rain. No portion of the project site is located within the 100-year floodplain as defined by FEMA FIRM Panel No. 48491C0460F, dated December 20, 2019. Buffer zones are provided as permanent BMPs for sensitive environmental features. Sensitive feature F-2 is located on site and has a buffer extending 50’ in all directions. Proposed impervious cover associated with the Parkside on the River Sections 9A & 10A development is routed to the Section 8 BMPs which are sized to treat a minimum 80% of the TSS as defined by TCEQ. Refer to the Geologic Assessment and Proposed Conditions Plan.

### **ATTACHMENT F – CONSTRUCTION PLANS**

Construction plans are attached.

### **ATTACHMENT I – MEASURES OF MINIMIZING SURFACE STREAM CONTAMINATION**

There is a mapped stream south of the property, in Parkside on the River Section 8. This stream has a catchment area of less than 64 acres and appears to only flow during heavy rain. Proposed impervious cover associated with the Parkside on the River Sections 9A & 10A development is routed to the Section 8 BMPs which are sized to treat a minimum 80% of the TSS as defined by TCEQ.

## ATTACHMENT G – INSPECTION, MAINTENANCE, REPAIR, AND RETROFIT PLAN

### Batch Detention Pond

1. Inspections should take place a minimum of twice a year and be documented in inspection reports. Inspection reports should include a field logbook documenting date, location, and action items. One inspection should take place during wet weather to determine if the basin is meeting the target detention time of 12 hours and a drawdown time of no more than 48 hours. The remaining inspections should occur between storm events so that manual operation of the valve and controller can be verified. The level sensor in the basin should be inspected and any debris or sediment in the area should be removed. The outlet structure and the trash screen should be inspected for signs of clogging. Debris and sediment should be removed from the orifice and outlet(s) as described in previous sections. Debris obstructing the valve should be removed. During each inspection, erosion areas inside and downstream of this BMP should be identified and repaired/revegetated immediately.
2. The basin, basin side-slopes, and embankment of the basin must be mowed to prevent woody growth and control weeds. A mulching mower should be used, or the grass clippings should be caught and removed. Mowing should take place at least twice a year, or more frequently if vegetation exceeds 18 inches in height. More frequent mowing to maintain aesthetic appeal may be necessary in landscaped areas.
3. Litter and debris removal should take place at least twice a year, as part of the periodic mowing operations and inspections. Debris and litter should be removed from the surface of the basin. Particular attention should be paid to floatable debris around the outlet structure. The outlet should be checked for possible clogging or obstructions and any debris removed.
4. The basin side slopes and embankment all may periodically suffer from slumping and erosion. To correct these problems, corrective action, such as regrading and revegetation, may be necessary. Correction of erosion control should take place whenever required based on the periodic inspections.
5. Standing water or soggy conditions may occur in the basin. Some standing water may occur after a storm event since the valve may close with 2 to 3 inches of water in the basin. Some flow into the basin may also occur between storms due to spring flow and residential water use that enters the storm sewer system. Twice a year, the facility should be evaluated in terms of nuisance control (insects, weeds, odors, algae, etc.).
6. With each inspection, any damage to structural elements of the basin (pipes, concrete drainage structures, retaining walls, etc.) should be identified and repaired immediately. An example of this type of repair can include patching of cracked concrete, sealing of voids, removal of vegetation from cracks and joints. The various inlet/outlet structures in a basin will eventually deteriorate and must be replaced.
7. A properly designed batch detention basin will accumulate quantities of sediment over time. The accumulated sediment can detract from the appearance of the facility and reduce the pollutant removal performance of the facility. The sediment also tends to accumulate near the outlet structure and can interfere with the level sensor operation. Sediment shall be removed from the basin at least every 5 years, when sediment depth exceeds 6 inches, when the sediment interferes with the level sensor or when the basin does not drain within 48 hours. Care should be taken not to compromise the basin lining during maintenance.
8. The Logic Controller should be inspected as part of the twice-yearly investigations. Verify that the external indicators (active, cycle in progress) are operating properly by turning the controller off and on, and by initiating a cycle by triggering the level sensor in the basin. The valve should be manually opened and closed using the open/close switch to verify valve operation and to assist in inspecting the valve for debris. The solar panel should be inspected and any dust or debris on the panel should be carefully removed. The controller and all other circuitry and wiring should be inspected for signs of corrosion, damage from insects, water leaks, or other damage. At the end of the inspection, the controller should be reset.

### Vegetative Filter Strips

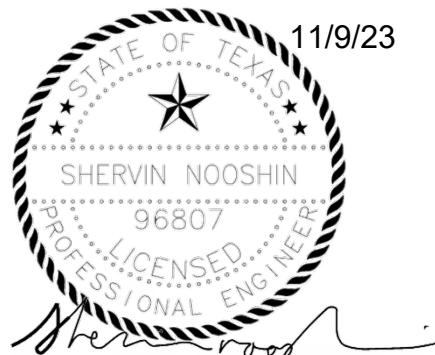
1. Seasonal Mowing and Lawn Care. If the filter strip is made up of turf grass, it should be mowed as needed to limit vegetation height to 18 inches, using a mulching mower (or removal of clippings). If native grasses are used, the filter may require less frequent mowing, but a minimum of twice annually. Grass clippings and brush debris should not be deposited on vegetative filter strip areas.

2. **Inspection.** Inspect filter strips at least twice annually for erosion or damage to vegetation; however, additional inspection after periods of heavy runoff is most desirable. Inspections should be documented in inspection reports. Inspection reports should include a field logbook documenting date, location, and action items. The strip should be checked for uniformity of grass cover, debris and litter, and areas of sediment accumulation. More frequent inspections of the grass cover during the first few years after establishment will help to determine if any problems are developing, and to plan for long-term restorative maintenance needs. Bare spots and areas of erosion identified during semi-annual inspections.
3. **Debris and Litter Removal.** Trash tends to accumulate in vegetated areas, particularly along highways. Any filter strip structures (i.e. level spreaders) should be kept free of obstructions to reduce floatables being flushed downstream, and for aesthetic reasons. The need for this practice is determined through periodic inspection, but should be performed no less than 4 times per year.
4. **Sediment Removal.** Sediment may accumulate along the upstream boundary of the strip preventing uniform overland flow. Excess sediment should be removed by hand or with flatbottomed shovels.
5. **Grass Reseeding and Mulching.** A healthy dense grass should be maintained on the filter strip. If areas are eroded, they should be filled, compacted, and reseeded so that the final grade is level. Grass damaged during the sediment removal process should be promptly replaced using the same seed mix used during filter strip establishment. If possible, flow should be diverted from the damaged areas until the grass is firmly established. Bare spots and areas of erosion identified during semi-annual inspections must be replanted and restored to meet specifications. Corrective maintenance, such as weeding or replanting should be done more frequently in the first two to three years after installation to ensure stabilization. Dense vegetation may require irrigation immediately after planting, and during particularly dry periods, particularly as the vegetation is initially established.

An amended copy of this document will be provided to the TCEQ within thirty days of any changes in the following information.

Responsible Party for Maintenance: HM Parkside, LP  
Address: 1101 North Lamar Boulevard  
City, State, Zip: Austin, TX 78703  
Telephone Number: (512) 481-0303

Signature of Responsible Party





**Agent Authorization Form**  
For Required Signature  
Edwards Aquifer Protection Program  
Relating to 30 TAC Chapter 213  
Effective June 1, 1999

Blake Magee  
Print Name

\_\_\_\_\_  
Title - Owner/President/Other

of HM Parkside, LP  
Corporation/Partnership/Entity Name

have authorized Christine Campbell, P.E.  
Print Name of Agent/Engineer

of HR Green Development TX, LLC  
Print Name of Firm

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:

Blake Magee  
Applicant's Signature

11-13-23  
Date

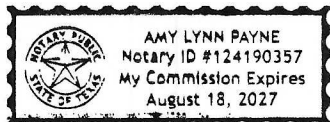
THE STATE OF Texas §

County of Travis §

BEFORE ME, the undersigned authority, on this day personally appeared Blake Magee known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this 13th day of November, 2023.

Amy Lynn Payne  
NOTARY PUBLIC



Amy Lynn Payne  
Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 11-13-23

# Application Fee Form

## Texas Commission on Environmental Quality

Name of Proposed Regulated Entity: Parkside on the River Sections 9A & 10A

Regulated Entity Location: Located east of Parkside Parkway. East of Parkside on the River Phase 3 Sections 4, 7A & 7B. Property ID R574025, R312360, R574027, R500990, R500991, R500992, R501370

Name of Customer: HM Parkside, LP

Contact Person: Blake Magee

Phone: 512-481-0303

Customer Reference Number (if issued): CN 605721653

Regulated Entity Reference Number (if issued): RN \_\_\_\_\_

### Austin Regional Office (3373)

☐ Hays

☐ Travis

☒ Williamson

### San Antonio Regional Office (3362)

☐ Bexar

☐ Medina

☐ Uvalde

☐ Comal

☐ Kinney

Application fees must be paid by check, certified check, or money order, payable to the **Texas Commission on Environmental Quality**. Your canceled check will serve as your receipt. **This form must be submitted with your fee payment.** This payment is being submitted to:

☒ Austin Regional Office

☐ San Antonio Regional Office

☐ Mailed to: TCEQ - Cashier

☐ Overnight Delivery to: TCEQ - Cashier

Revenues Section

Mail Code 214

P.O. Box 13088

Austin, TX 78711-3088

12100 Park 35 Circle

Building A, 3rd Floor

Austin, TX 78753

(512)239-0357

### Site Location (Check All That Apply):

☒ Recharge Zone

☐ Contributing Zone

☐ Transition Zone

<i>Type of Plan</i>	<i>Size</i>	<i>Fee Due</i>
Water Pollution Abatement Plan, Contributing Zone Plan: One Single Family Residential Dwelling	Acres	\$
Water Pollution Abatement Plan, Contributing Zone Plan: Multiple Single Family Residential and Parks	Legal boundary = 75.68 Acres	\$ 6,500.00
Water Pollution Abatement Plan, Contributing Zone Plan: Non-residential	Acres	\$
Sewage Collection System	L.F.	\$
Lift Stations without sewer lines	Acres	\$
Underground or Aboveground Storage Tank Facility	Tanks	\$
Piping System(s)(only)	Each	\$
Exception	Each	\$
Extension of Time	Each	\$

Signature: Chitra Campbell

Date: 12/19/2023

## Application Fee Schedule

Texas Commission on Environmental Quality

Edwards Aquifer Protection Program 30 TAC Chapter 213 (effective 05/01/2008)

### ***Water Pollution Abatement Plans and Modifications***

#### ***Contributing Zone Plans and Modifications***

<b><i>Project</i></b>	<b><i>Project Area in Acres</i></b>	<b><i>Fee</i></b>
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5	\$1,500
	5 < 10	\$3,000
	10 < 40	\$4,000
	40 < 100	\$6,500
	100 < 500	\$8,000
	≥ 500	\$10,000
Non-residential (Commercial, industrial, institutional, multi-family residential, schools, and other sites where regulated activities will occur)	< 1	\$3,000
	1 < 5	\$4,000
	5 < 10	\$5,000
	10 < 40	\$6,500
	40 < 100	\$8,000
	≥ 100	\$10,000

#### ***Organized Sewage Collection Systems and Modifications***

<b><i>Project</i></b>	<b><i>Cost per Linear Foot</i></b>	<b><i>Minimum Fee- Maximum Fee</i></b>
Sewage Collection Systems	\$0.50	\$650 - \$6,500

#### ***Underground and Aboveground Storage Tank System Facility Plans and Modifications***

<b><i>Project</i></b>	<b><i>Cost per Tank or Piping System</i></b>	<b><i>Minimum Fee- Maximum Fee</i></b>
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500

#### ***Exception Requests***

<b><i>Project</i></b>	<b><i>Fee</i></b>
Exception Request	\$500

***Extension of Time Requests***

<b><i>Project</i></b>	<b><i>Fee</i></b>
Extension of Time Request	\$150



TCEQ Use Only

# TCEQ Core Data Form

For detailed instructions regarding completion of this form, please read the Core Data Form Instructions or call 512-239-5175.

## SECTION I: General Information

<b>1. Reason for Submission</b> (If other is checked please describe in space provided.)		
<input checked="" type="checkbox"/> New Permit, Registration or Authorization (Core Data Form should be submitted with the program application.)		
<input type="checkbox"/> Renewal (Core Data Form should be submitted with the renewal form)		<input type="checkbox"/> Other
<b>2. Customer Reference Number (if issued)</b>		<b>3. Regulated Entity Reference Number (if issued)</b>
CN 605721653		RN

[Follow this link to search for CN or RN numbers in Central Registry\\*\\*](#)

## SECTION II: Customer Information

<b>4. General Customer Information</b>		<b>5. Effective Date for Customer Information Updates</b> (mm/dd/yyyy)					
<input type="checkbox"/> New Customer		<input type="checkbox"/> Update to Customer Information					
<input type="checkbox"/> Change in Legal Name (Verifiable with the Texas Secretary of State or Texas Comptroller of Public Accounts)		<input type="checkbox"/> Change in Regulated Entity Ownership					
<b>The Customer Name submitted here may be updated automatically based on what is current and active with the Texas Secretary of State (SOS) or Texas Comptroller of Public Accounts (CPA).</b>							
<b>6. Customer Legal Name</b> (If an individual, print last name first: eg: Doe, John)		If new Customer, enter previous Customer below:					
HM Parkside, LP							
<b>7. TX SOS/CPA Filing Number</b>	<b>8. TX State Tax ID</b> (11 digits)	<b>9. Federal Tax ID</b> (9 digits)	<b>10. DUNS Number</b> (if applicable)				
0803154683	32068805335						
<b>11. Type of Customer:</b>	<input type="checkbox"/> Corporation	<input type="checkbox"/> Individual	Partnership: <input type="checkbox"/> General <input checked="" type="checkbox"/> Limited				
Government: <input type="checkbox"/> City <input type="checkbox"/> County <input type="checkbox"/> Federal <input type="checkbox"/> State <input type="checkbox"/> Other	<input type="checkbox"/> Sole Proprietorship	<input type="checkbox"/> Other:					
<b>12. Number of Employees</b>		<b>13. Independently Owned and Operated?</b>					
<input checked="" type="checkbox"/> 0-20 <input checked="" type="checkbox"/> 21-100 <input type="checkbox"/> 101-250 <input type="checkbox"/> 251-500 <input type="checkbox"/> 501 and higher		<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No					
<b>14. Customer Role</b> (Proposed or Actual) – as it relates to the Regulated Entity listed on this form. Please check one of the following:							
<input checked="" type="checkbox"/> Owner <input type="checkbox"/> Operator <input type="checkbox"/> Owner & Operator							
<input type="checkbox"/> Occupational Licensee <input type="checkbox"/> Responsible Party <input type="checkbox"/> Voluntary Cleanup Applicant <input type="checkbox"/> Other:							
<b>15. Mailing Address:</b>	1011 North Lamar Boulevard						
	City	Austin	State	TX	ZIP	78703	ZIP + 4
<b>16. Country Mailing Information</b> (if outside USA)				<b>17. E-Mail Address</b> (if applicable)			
				Blake@blakemageeco.com			
<b>18. Telephone Number</b>		<b>19. Extension or Code</b>		<b>20. Fax Number</b> (if applicable)			
( 512 ) 481-0303				( ) -			

## SECTION III: Regulated Entity Information

<b>21. General Regulated Entity Information</b> (If 'New Regulated Entity' is selected below this form should be accompanied by a permit application)	
<input checked="" type="checkbox"/> New Regulated Entity <input type="checkbox"/> Update to Regulated Entity Name <input type="checkbox"/> Update to Regulated Entity Information	
<b>The Regulated Entity Name submitted may be updated in order to meet TCEQ Agency Data Standards (removal of organizational endings such as Inc, LP, or LLC.)</b>	
<b>22. Regulated Entity Name</b> (Enter name of the site where the regulated action is taking place.)	
Parkside on the River Sections 9A & 10A	

23. Street Address of the Regulated Entity: (No PO Boxes)	Located east of Parkside Parkway. East of Parkside on the River Phase 3 Sections 4, 7A & 7B.							
	City	Georgetown	State	TX	ZIP	78628	ZIP + 4	
24. County	Williamson County							
Enter Physical Location Description if no street address is provided.								
25. Description to Physical Location:	Located east of Parkside Parkway. East of Parkside on the River Phase 3 Sections 4, 7A & 7B. Property ID R574025, R312360, R574027, R500990, R500991, R500992, R501370							
26. Nearest City	Georgetown				State	TX	Nearest ZIP Code	
								78628
27. Latitude (N) In Decimal:	30.609739			28. Longitude (W) In Decimal:	-97.766178			
Degrees	Minutes	Seconds		Degrees	Minutes	Seconds		
30	36	35.06N		97	45	58.24W		
29. Primary SIC Code (4 digits)	30. Secondary SIC Code (4 digits)		31. Primary NAICS Code (5 or 6 digits)		32. Secondary NAICS Code (5 or 6 digits)			
1521			236115					
33. What is the Primary Business of this entity? (Do not repeat the SIC or NAICS description.)								
Land Development - Single Family Residential								
34. Mailing Address:	1011 North Lamar Boulevard							
	City	Austin	State	TX	ZIP	78703	ZIP + 4	
35. E-Mail Address:	blake@blakemageeco.com							
36. Telephone Number	37. Extension or Code		38. Fax Number (if applicable)					
( 512 ) 481-0303			( ) -					

39. TCEQ Programs and ID Numbers Check all Programs and write in the permits/registration numbers that will be affected by the updates submitted on this form. See the Core Data Form instructions for additional guidance.


<input type="checkbox"/> Dam Safety	<input type="checkbox"/> Districts	<input checked="" type="checkbox"/> Edwards Aquifer	<input type="checkbox"/> Emissions Inventory Air	<input type="checkbox"/> Industrial Hazardous Waste
<input type="checkbox"/> Municipal Solid Waste	<input type="checkbox"/> New Source Review Air	<input type="checkbox"/> OSSF	<input type="checkbox"/> Petroleum Storage Tank	<input type="checkbox"/> PWS
<input type="checkbox"/> Sludge	<input type="checkbox"/> Storm Water	<input type="checkbox"/> Title V Air	<input type="checkbox"/> Tires	<input type="checkbox"/> Used Oil
<input type="checkbox"/> Voluntary Cleanup	<input type="checkbox"/> Waste Water	<input type="checkbox"/> Wastewater Agriculture	<input type="checkbox"/> Water Rights	<input type="checkbox"/> Other:

#### SECTION IV: Preparer Information

40. Name:	Christine Campbell	41. Title:	Project Engineer
42. Telephone Number	43. Ext./Code	44. Fax Number	45. E-Mail Address
( 512 ) 872-6696		( ) -	christine.campbell@hrgreen.com

#### SECTION V: Authorized Signature

46. By my signature below, I certify, to the best of my knowledge, that the information provided in this form is true and complete, and that I have signature authority to submit this form on behalf of the entity specified in Section II, Field 6 and/or as required for the updates to the ID numbers identified in field 39.

Company:	HR Green Development TX, LLC	Job Title:	Project Engineer
Name(In Print) :	Christine Campbell	Phone:	( 512 ) 872-6696
Signature:		Date:	12/19/2023



**NOTICE OF CONFIDENTIALITY RIGHTS: IF YOU ARE A NATURAL PERSON, YOU MAY REMOVE OR STRIKE ANY OR ALL OF THE FOLLOWING INFORMATION FROM ANY INSTRUMENT THAT TRANSFERS AN INTEREST IN REAL PROPERTY BEFORE IT IS FILED FOR RECORD IN THE PUBLIC RECORDS: YOUR SOCIAL SECURITY NUMBER OR YOUR DRIVER'S LICENSE NUMBER.**

**SPECIAL WARRANTY DEED**

This Special Warranty Deed (this **Deed**) is made as of December 28, 2018, by **HCB LAREDO TEXAS, LLC**, a Colorado limited liability company (**Grantor**), to **HM PARKSIDE, LP**, a Texas limited partnership (**Grantee**).

For other valuable consideration to Grantor paid by the Grantee, the receipt of which is acknowledged, Grantor and Grantee agree as follows:

1. Conveyance and Warranty of Title.

Grantor GRANTS, SELLS, and CONVEYS to Grantee, subject to the Permitted Exceptions (defined below), all of the real property (the **Real Property**) more particularly described on Exhibit A attached to this Deed, together with all interest of Grantor in:

- any easements, rights-of-way, and rights of ingress or egress that benefit the Real Property;
- any dedicated highway, avenue, street, or alley, in, on, across, in front of, abutting, or adjoining the Real Property or any land lying in or under the bed of any of the foregoing; and
- any strips or gores of land adjoining the Real Property and abutting properties, whether owned or claimed by deed, limitations, or otherwise, and whether or not located inside or outside of the Real Property;

(collectively, the **Property**).

TO HAVE AND TO HOLD the Property, subject to the Permitted Exceptions, together with all and singular the rights and appurtenances thereto in anywise belonging, to Grantee, its successors and assigns, forever; and Grantor binds itself, its successors and assigns, to WARRANT AND FOREVER DEFEND all and singular the Property to Grantee, its successors and assigns, against every person whomsoever lawfully claiming or to claim the same or any part thereof, by, through, or under Grantor, but not otherwise.

2. Permitted Exceptions.

This Deed is made, and is accepted by Grantee, subject to the restrictions, easements, covenants, encumbrances, and liens described on Exhibit B attached to this Deed, but only to the extent that same are in existence and affect the Property (the **Permitted Exceptions**).

EXECUTED as of the date first above written.

**GRANTOR:**

**HCB LAREDO TEXAS, LLC,**  
a Colorado limited liability company

By: [Signature]  
Name: Michael D. Balsbaugh  
Title: Executive Vice President

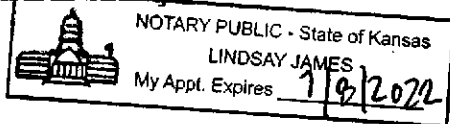
**Address of Grantee:**

HM Parkside LP  
1011 N. Lamar Blvd.  
Austin, Texas 78703

THE STATE OF KANSAS       §  
                                          §  
COUNTY OF JOHNSON       §

This instrument was acknowledged before me on December 21, 2018, by Michael D. Balsbaugh, Executive Vice President of HCB LAREDO TEXAS, LLC, a Colorado limited liability company, on behalf of said limited liability company.

[NOTARIAL SEAL]



[Signature]  
Notary Public in and for The State of Kansas  
Print Name: LINDSAY JAMES  
My Commission Expires: 7/9/2022

**Exhibit A**

**Real Property**

**Tract 1:**

1,143.511 acres of land in Williamson County, Texas, being more particularly described as 1,156.001 acres described on Exhibit A-1 attached hereto and incorporated herein, SAVE AND EXCEPT 9.410 acres described on Exhibit A-2 attached hereto and incorporated herein and SAVE AND EXCEPT 3.080 acres described on Exhibit A-3 attached hereto and incorporated herein and

**Tract 2:**

Lot 2, Block G, Water Oak North Section 1, a subdivision in Williamson County, Texas, according to the map or plat thereof recorded under Document No. 2013033404, Official Public Records of Williamson County, Texas.

## EXHIBIT A-1

County: Williamson  
 Project: Water Oak South  
 Job No.: A180801  
 MBS No.: 18-005

## FIELD NOTES FOR 1156.001 ACRES

Being a tract containing 1,156.001 acres of land located in the I. Donagan Survey, Abstract Number 178, the J. Thompson Survey, Abstract Number 608, the Key West Irrigation Survey, Abstract Number 711, the I.&G.N. R.R. Survey, Abstract number 744, the J.D. Johns Survey, Abstract Number 365, the W.E. Pate Survey, Abstract Number 836, the D. Medlock Survey, Abstract Number 839, in Williamson County, Texas; Said 1,156.001 acre tract being a call 195.193 acre tract of land recorded in the name of Laredo Wo, Ltd. in Williamson County Clerk's File (W.C.C.F.) Number 2007014280, a call 71.001 acre tract of land recorded in the name of Laredo Wo, Ltd. in W.C.C.F. Number 2007014281, call 77.399 acre, 44.314 acre, and 203.137 acre tracts of land recorded in the name of Laredo Wo, Ltd. in W.C.C.F. Number 2007014282, call 330.24 acre and 15.56 acre tracts of land recorded in the name of Laredo Wo, Ltd. in W.C.C.F. Number 2007014285, a call 0.368 acre tract of land recorded in the name of Laredo Wo, Ltd. in W.C.C.F. Number 2008039394, and a call 6.190 acre tract of land recorded in the name of Laredo Wo, Ltd. in W.C.C.F. Number 2009022803, and a portion of a call 192.314 acre tract of land recorded in the name of Laredo Wo, Ltd. in W.C.C.F. Number 2007014289, a call 3.080 acre tract of land recorded in the name of Austin WO, LLC in W.C.C.F. Number 2014011207 and a call 324.00 acre tract of land recorded in the name of Laredo Wo, Ltd. in W.C.C.F. Number 2007014278; Said 1,156.001 acres being more particularly described by metes and bounds descriptions as follows (bearings are referenced to the Texas Coordinate System, NAD 1983, Central Zone):

**Beginning** at a 1/2-inch iron rod found at the southwesterly corner of said 6.190 acre tract, the southeasterly corner of a call 47.420 acre tract of land recorded in the name of Georgetown Properties II, LLC in W.C.C.F. Number 2012043969 and the northerly Right-of-Way (R.O.W.) line of F.M. 2243 (80-foot width);

Thence, with the easterly line of said 47.420 acre tract, the following sixteen (16) courses:

1. North 28 degrees 25 minutes 04 seconds East, a distance of 160.70 feet to a 1/2-inch iron rod found;
2. 155.33 feet along the arc of a curve to the left, said curve having a central angle of 16 degrees 06 minutes 30 seconds, a radius of 552.50 feet and a chord which bears North 18 degrees 24 minutes 54 seconds West, a distance of 154.82 feet to a 1/2-inch iron rod found;
3. North 26 degrees 28 minutes 10 seconds West, a distance of 157.44 feet to a 1/2-inch iron rod found;

4. 38.91 feet along the arc of a curve to the left, said curve having a central angle of 89 degrees 10 minutes 31 seconds, a radius of 25.00 feet and a chord which bears North 71 degrees 03 minutes 54 seconds West, a distance of 35.10 feet to a 1/2-inch iron rod set;
5. North 27 degrees 14 minutes 19 second West, a distance of 65.03 feet to a 1/2-iron rod set;
6. 39.79 feet along the arc of a curve to the left, said curve having a central angle of 91 degrees 11 minutes 17 seconds, a radius of 25.00 feet and a chord which bears North 19 degrees 07 minutes 36 seconds East, a distance of 35.72 feet to a 1/2-inch iron rod set;
7. North 26 degrees 28 minutes 10 seconds West, a distance of 150.25 feet to a 1/2-inch iron rod set;
8. 674.40 feet along the arc of a curve to the right, said curve having a central angle of 45 degrees 58 minutes 22 seconds, a radius of 840.50 feet and a chord which bears North 03 degrees 28 minutes 59 seconds West, a distance of 656.45 feet to a 1/2-inch iron rod set;
9. 203.98 feet along the arc of a curve to the left, said curve having a central angle of 22 degrees 04 minutes 18 seconds, a radius of 529.52 feet and a chord which bears South 77 degrees 26 minutes 54 seconds West, a distance of 202.72 feet to a 1/2-inch iron rod set;
10. North 32 degrees 58 minutes 10 seconds West, a distance of 44.22 feet to a 1/2-iron rod set;
11. North 34 degrees 39 minutes 43 seconds West, a distance of 239.78 feet to a 1/2-inch iron rod found;
12. North 55 degrees 20 minutes 17 seconds East, a distance of 450.00 feet, from which a 1/2-inch iron rod found, bears South 61 degrees East a distance of 0.49 feet;
13. North 34 degrees 39 minutes 43 seconds West, a distance of 97.07 feet to a 1/2-inch iron rod set;
14. 124.70 feet along the arc of a curve to the left, said curve having a central angle of 119 degrees 05 minutes 02 seconds, a radius of 60.00 feet and a chord which bears North 24 degrees 52 minutes 55 seconds East, a distance of 103.44 feet, from which a 1/2-inch iron rod found, bears South 68 degrees East, a distance of 0.55 feet;
15. North 55 degrees 20 minutes 17 seconds East, a distance of 120.00 feet to a 1/2-inch iron rod found;

16. North 34 degrees 39 minutes 43 seconds West, a distance of 126.11 feet to an easterly line of The Preserve Phase 1, a subdivision recorded in Cabinet EE, Slide Number 310-316 of the Williamson County Plat Records (W.C.P.R.), from which a 1/2-inch iron rod found, bears South 67 degrees East, a distance of 0.66 feet;

Thence, with the easterly line of said The Preserve Phase 1, the following twelve (12) courses:

1. North 80 degrees 20 minutes 05 seconds East, a distance of 307.48 feet to a 1/2-inch iron rod set;
2. North 23 degrees 41 minutes 11 seconds West, a distance of 279.38 feet to a 1/2-inch iron rod set;
3. 31.65 feet along the arc of a curve to the left, said curve having a central angle of 72 degrees 13 minutes 47 seconds, a radius of 25.11 feet and a chord which bears North 63 degrees 28 minutes 50 seconds West, a distance of 29.60 feet to a 1/2-inch iron rod set;
4. North 09 degrees 39 minutes 51 seconds West, a distance of 50.00 feet to a 1/2-inch iron rod set;
5. North 80 degrees 20 minutes 05 seconds East, a distance of 155.74 feet to a 1/2-inch iron rod found;
6. North 21 degrees 06 minutes 30 seconds West, a distance of 186.45 feet to a 1/2-inch iron rod set;
7. North 30 degrees 29 minutes 37 seconds West, a distance of 233.35 feet to a 1/2-inch iron rod found;
8. North 23 degrees 41 minutes 11 seconds West, a distance of 528.84 feet to a cotton spindle found;
9. South 66 degrees 44 minutes 24 seconds West, a distance of 125.00 feet to a 1/2-inch iron rod set;
10. North 23 degrees 41 minutes 11 seconds West, a distance of 409.01 feet to a 1/2-inch iron rod found;
11. North 68 degrees 45 minutes 39 seconds East, a distance of 108.54 feet to a 1/2-inch iron rod found;
12. North 21 degrees 14 minutes 21 seconds West, a distance of 714.47 feet to the easterly line of a call 60.5184 acre tract of land recorded in the name of AVP Ranch, Ltd. in W.C.C.F. Number 2011081794, from which a 1/2-inch iron rod found, bears North 27 degrees West, a distance of 0.68 feet;

Thence, with said easterly line, North 14 degrees 11 minutes 42 seconds East, a distance of 1,508.94 feet to a 1/2-inch iron rod set at the southwesterly corner of a call 314.00 acre tract of land recorded in the name of Georgetown Properties II in W.C.C.F. Number 2012043969;

Thence, with the southerly line of said 314.00 acre tract, the following ten (10) courses:

1. South 75 degrees 48 minutes 18 seconds East, a distance of 431.73 feet to a 1/2-inch iron rod found;
2. 326.94 feet along the arc of a curve to the right, said curve having a central angle of 32 degrees 24 minutes 32 seconds, a radius of 578.00 feet and a chord which bears South 59 degrees 36 minutes 01 seconds East, a distance of 322.60 feet to a 1/2-inch iron rod found;
3. South 43 degrees 23 minutes 44 seconds East, a distance of 1,170.13 feet to a 1/2-inch iron rod found;
4. 175.01 feet along the arc of a curve to the right, said curve having a central angle of 09 degrees 18 minutes 07 seconds, a radius of 1078.00 feet and a chord which bears North 55 degrees 24 minutes 17 seconds East, a distance of 174.82 feet to a 1/2-inch iron rod found;
5. North 60 degrees 03 minutes 21 seconds East, a distance of 538.21 feet, from which a 1/2-inch iron rod found, bears South 23 degrees West, a distance of 0.50 feet;
6. 839.65 feet along the arc of a curve to the left, said curve having a central angle of 52 degrees 10 minutes 41 seconds, a radius of 922.00 feet and a chord which bears North 33 degrees 58 minutes 00 seconds East, a distance of 810.93 feet to a 1/2-inch iron rod found;
7. North 07 degrees 52 minutes 40 seconds East, a distance of 108.32 feet to a 1/2-inch iron rod set;
8. 1,349.11 feet along the arc of a curve to the right, said curve having a central angle of 79 degrees 02 minutes 14 seconds, a radius of 978.00 feet and a chord which bears North 47 degrees 23 minutes 47 seconds East, a distance of 1,244.66 feet to a 1/2-inch iron rod found;
9. North 86 degrees 54 minutes 53 seconds East, a distance of 321.28 feet to a 1/2-inch iron rod found;
10. 75.21 feet along the arc of a curve to the right, said curve having a central angle of 03 degrees 59 minutes 50 seconds, a radius of 1078.00 feet and a chord which bears North 88 degrees 54 minutes 08 seconds East, a distance of 75.19 feet to a 1/2-inch iron rod set at the southeasterly corner of said 314.00 acre tract and the westerly line of aforesaid 203.137 acre tract;

Thence, with the easterly line of said 314.00 acre tract, the following two (2) courses:



1. North 22 degrees 05 minutes 52 seconds West, a distance of 1596.68 feet to a 1-inch iron pipe found;
2. North 22 degrees 18 minutes 08 seconds West, a distance of 624.71 feet to the northeasterly corner of said 314.00 acre tract, the northwesterly corner of aforesaid 324.00 acre tract, a southerly corner of aforesaid 192.314 acre tract, and the centerline of South San Gabriel River;

Thence, with a northerly line of said 314.00 acre tract and the meanders of said centerline, South 68 degrees 48 minutes 05 seconds West, a distance of 57.92 feet to the southeasterly corner of a call 168.62 acre tract of land recorded in the name of Zamin, L.P. in W.C.C.F. Number 201403274 and the most southerly corner of said 192.314 acre tract;

Thence, with the easterly line of said 168.62 acre tract, the following ten (10) courses:

1. North 00 degrees 10 minutes 15 seconds West, a distance of 94.12 feet to a 1/2-inch iron rod set;
2. North 00 degrees 06 minutes 25 seconds East, a distance of 765.27 feet to a 1/2-inch iron rod (1847 cap) found;
3. North 00 degrees 15 minutes 54 seconds West, a distance of 374.43 feet to a nail in fence post found;
4. North 04 degrees 32 minutes 45 seconds East, a distance of 49.08 feet to a 1/2-inch iron rod set;
5. North 02 degrees 05 minutes 56 seconds East, a distance of 31.02 feet to a 1/2-inch iron rod set;
6. North 00 degrees 04 minutes 52 seconds East, a distance of 74.51 feet to a 1/2-inch iron rod set;
7. North 02 degrees 25 minutes 02 seconds West, a distance of 79.29 feet to a 1/2-inch iron rod (1847 cap) found;
8. North 00 degrees 29 minutes 19 seconds West, a distance of 311.09 feet to a 26-inch pine tree;
9. North 01 degrees 10 minutes 38 seconds West, a distance of 96.13 feet to a nail in a 30-inch cedar tree found;

10. North 02 degrees 08 minutes 59 seconds East, a distance of 140.61 feet to a nail in a 28-inch oak tree found at the southwesterly corner of a call 106.00 acre tract of land recorded in the name of Zamin, L.P. in W.C.C.F. Number 2010065268;

Thence, with the southerly line of said 106.00 acre tract, the following thirteen (13) courses:

1. South 36 degrees 25 minutes 52 seconds East, a distance of 145.97 feet to a 1/2-inch iron rod found;
2. South 40 degrees 04 minutes 40 seconds East, a distance of 159.64 feet to a 1/2-inch iron rod found;
3. South 65 degrees 38 minutes 47 seconds East, a distance of 83.14 feet to a 1/2-inch iron rod found;
4. North 88 degrees 53 minutes 22 seconds East, a distance of 622.87 feet to a cotton spindle found;
5. North 69 degrees 06 minutes 39 seconds East, a distance of 153.64 feet to a cotton spindle found;
6. North 67 degrees 02 minutes 44 seconds East, a distance of 133.64 feet to a 1/2-inch iron rod found;
7. South 27 degrees 21 minutes 25 seconds East, a distance of 172.95 feet to a 1/2-inch iron rod found;
8. South 36 degrees 36 minutes 32 seconds East, a distance of 272.53 feet to a 1/2-inch iron rod found;
9. South 82 degrees 53 minutes 15 seconds East, a distance of 115.61 feet to a 1/2-inch iron rod set;
10. North 56 degrees 07 minutes 11 seconds East a distance of 186.34 feet to a 1/2-inch iron rod found;
11. North 07 degrees 51 minutes 19 seconds West, a distance of 67.58 feet to a to a 1/2-inch iron rod found;
12. North 34 degrees 57 minutes 21 seconds West, a distance of 1007.97 feet to a 1/2-inch iron rod found;
13. North 55 degrees 43 minutes 32 seconds East, a distance of 579.96 feet to a 1/2-inch iron rod found at a westerly corner of a call 26.673 acre tract of land recorded in the name of Chesmar Homes Austin LLC in W.C.C.F. Number 2013095985;

Thence, with the southerly line of said 26.673 acre tract and the southerly line of a call 21.255 acre tract of land recorded in the name of Chesmar Homes Austin LLC. in W.C.C.F. Number 2018039081, the following nineteen (19) courses:

1. South 32 degrees 47 minutes 04 seconds East, a distance of 44.07 feet to a 1/2-inch iron rod set;
2. South 57 degrees 12 minutes 56 seconds West, a distance of 102.66 feet to a 1/2-inch iron rod set;
3. South 14 degrees 17 minutes 30 seconds East, a distance of 224.58 feet to a 1/2-inch iron rod set;
4. South 65 degrees 02 minutes 43 seconds East, a distance of 102.90 feet to a 1/2-inch iron rod set;
5. South 31 degrees 01 minutes 16 seconds East, a distance of 404.11 feet to a 1/2-inch iron rod set;
6. South 04 degrees 15 minutes 14 seconds West, a distance of 202.68 feet to a 1/2-inch iron rod set;
7. South 04 degrees 50 minutes 11 seconds West, a distance of 99.31 feet to a 1/2-inch iron rod set;
8. South 41 degrees 38 minutes 10 seconds East, a distance of 114.53 feet to a 1/2-inch iron rod set;
9. South 55 degrees 58 minutes 17 seconds East, a distance of 65.00 feet to a 1/2-inch iron rod set;
10. 49.41 feet along the arc of a curve to the right, said curve having a central angle of 14 degrees 31 minutes 09 seconds, a radius of 194.99 feet and a chord which bears South 84 degrees 07 minutes 03 seconds East, a distance of 49.28 feet to a 1/2-inch iron rod set;
11. South 80 degrees 23 minutes 52 seconds East, a distance of 35.39 feet to a 1/2-inch iron rod set;
12. South 83 degrees 07 minutes 59 seconds East, a distance of 260.77 feet to a 1/2-inch iron rod set;
13. South 73 degrees 37 minutes 51 seconds East, a distance of 287.96 feet to a 1/2-inch iron rod set;
14. North 83 degrees 40 minutes 45 seconds East, a distance of 84.78 feet to a 1/2-inch iron rod set;

15. North 06 degrees 19 minutes 15 seconds West, a distance of 176.09 feet to a 1/2-inch iron rod set;
16. 60.98 feet along the arc of a curve to the left, said curve having a central angle of 15 degrees 01 minutes 43 seconds, a radius of 232.50 feet and a chord which bears North 68 degrees 07 minutes 46 seconds East, a distance of 60.81 feet to a 1/2-inch iron rod set;
17. North 60 degrees 36 minutes 55 seconds East, a distance of 246.19 feet to a 1/2-inch iron rod set;
18. 39.28 feet along the arc of a curve to the right, said curve having a central angle of 90 degrees 01 minutes 54 seconds, a radius of 25.00 feet and a chord which bears South 74 degrees 23 minutes 05 seconds East, a distance of 35.36 feet to a 1/2-inch iron rod set;
19. North 60 degrees 36 minutes 55 seconds East, a distance of 55.00 feet to a 1/2-inch iron rod set at a westerly corner of a call 24.958 acre tract of land recoded in the name of ABG Water Oak Partners, Ltd. in W.C.C.F. Number 2014071868;

Thence, with the southerly line of said 24.958 acre tract, the following ten (10) courses:

1. South 03 degrees 29 minutes 46 seconds East, a distance of 31.45 feet to a 1/2-inch iron rod set;
2. South 03 degrees 43 minutes 00 seconds East, a distance of 299.26 feet to a 1/2-inch iron rod set;
3. 90.18 feet along the arc of a curve to the right, said curve having a central angle of 05 degrees 53 minutes 18 seconds, a radius of 877.50 feet and a chord which bears South 00 degrees 46 minutes 21 seconds East, a distance of 90.14 feet to a 1/2-inch iron rod set;
4. South 01 degrees 24 minutes 06 seconds East, a distance of 233.95 feet to a 1/2-inch iron rod set;
5. South 73 degrees 49 minutes 36 seconds East, a distance of 545.48 feet to a 1/2-inch iron rod set;
6. South 89 degrees 06 minutes 15 seconds East, a distance of 70.34 feet to a 1/2-inch iron rod set;
7. South 83 degrees 26 minutes 51 seconds East, a distance of 1532.87 feet to a 1/2-inch iron rod set;
8. North 85 degrees 29 minutes 19 seconds East, a distance of 278.11 feet to a 1/2-inch iron rod set;

9. North 04 degrees 30 minutes 41 seconds West, a distance of 130.00 feet to a 1/2-inch iron rod set;
10. North 85 degrees 29 minutes 19 seconds East, a distance of 160.52 feet to a 1/2-inch iron rod set on the westerly line of a call 32.61 acre tract of land recorded in the name of William Charles Bagwell, Et Ux in Volume 2438, Page 0499 of the Williamson County Deed Records (W.C.D.R.);

Thence, with the westerly line of said 32.61 acre tract, the following two (2) courses:

1. South 09 degrees 08 minutes 19 seconds East, a distance of 233.24 feet to a 1/2-inch iron rod found;
2. South 00 degrees 25 minutes 18 seconds East, a distance of 188.62 feet to the northerly line of a call 190.40 acre tract of land recorded in the name of Texas Crushed Stone Company in Volume 743, Page 47 of the W.C.D.R. and the said centerline of the South San Gabriel River;

Thence, with the meanders of said centerline, the following seven (7) courses:

1. South 87 degrees 44 minutes 31 seconds West, a distance of 362.99 feet;
2. North 78 degrees 02 minutes 28 seconds West, a distance of 85.59 feet;
3. South 80 degrees 19 minutes 11 seconds West, a distance of 148.88 feet;
4. South 65 degrees 08 minutes 13 seconds West, a distance of 207.18 feet;
5. North 66 degrees 16 minutes 04 seconds West, a distance of 40.94 feet;
6. North 89 degrees 30 minutes 57 seconds West, a distance of 541.24 feet;
7. North 79 degrees 08 minutes 16 seconds West, a distance of 180.05 feet to a 5/8-inch iron rod set for the northwesterly corner of said 190.40 acre tract;

Thence, leaving said centerline, with the westerly line of said 190.40 acre tract, the following seven (7) courses:

1. South 01 degrees 52 minutes 12 seconds East, a distance of 1026.81 feet to a 1/2-inch iron rod (1847 cap) found;
2. South 01 degrees 10 minutes 35 seconds East, a distance of 167.70 feet to a 1/2-inch iron rod found;
3. South 00 degrees 03 minutes 35 seconds West, a distance of 341.80 feet to a 1-inch iron pipe found;

4. South 06 degrees 25 minutes 15 seconds East, a distance of 359.37 feet to a 5/8-inch iron rod set;
5. South 01 degrees 45 minutes 07 seconds East, a distance of 480.85 feet to a 1/2-inch iron rod found;
6. South 02 degrees 48 minutes 39 seconds East, a distance of 258.38 feet to a nail found;
7. South 02 degrees 30 minutes 15 seconds East, a distance of 1139.73 feet to a 1/2-inch iron rod found at a northerly corner of a call 77.902 acre tract of land recorded in the name of Edwin H. Vale, Jr in W.C.C.F. Number 2017014736;

Thence, with the northerly and westerly line of said 77.902 acre tract, the following seven (7) courses:

1. South 68 degrees 13 minutes 42 seconds West, a distance of 128.79 feet to a cotton spindle found;
2. North 36 degrees 37 minutes 28 seconds West, a distance of 381.75 feet to a 1/2-inch iron rod found;
3. North 68 degrees 46 minutes 05 seconds West, a distance of 137.51 feet to a 1/2-inch iron rod found;
4. South 84 degrees 17 minutes 41 seconds West, a distance of 214.68 feet to a 1/2-inch iron rod found;
5. South 71 degrees 34 minutes 53 seconds West, a distance of 180.12 feet to a 1/2-inch iron rod found;
6. South 75 degrees 44 minutes 55 seconds West, a distance of 433.46 feet to a cotton spindle found;
7. South 80 degrees 42 minutes 01 seconds West, a distance of 377.54 feet to a 1/2-inch iron rod found, said iron rod being the most northerly northeast corner of a called 3.080 acre tract of land called Road Easement in W.C.C.F. No. 2014011208;

Thence, through and across aforesaid 77.902 acre tract and with the easterly line of said road easement, 764.64 feet along the arc of a curve to the right, said curve having a central angle of 49 degrees 52 minutes 11 seconds, a radius of 878.50 feet and a chord which bears South 46 degrees 35 minutes 20 seconds East, a distance of 740.73 feet to a 5/8-inch iron rod set on the southerly line of said 77.902 acre tract, also being on the northerly line of aforesaid 195.193 acre tract;

Thence, with a southerly of said 77.902 acre tract, North 68 degrees 08 minutes 38 seconds East, a distance of 901.90 feet to a nail found at a northwesterly corner of said 77.902 acre tract;

Thence with the westerly line of said 77.902 acre tract, South 20 degrees 54 minutes 54 seconds East, a distance of 3,791.46 feet to the northerly R.O.W. line of aforesaid F.M. 2243, from which a 1/2-inch iron rod found bears North 22 degrees East, a distance of 0.50 feet;

Thence, with said northerly R.O.W. line, the following five (5) courses:

1. South 69 degrees 01 minutes 48 seconds West, a distance of 1,585.42 feet to a concrete monument found;
2. 849.64 feet along the arc of a curve to the right, said curve having a central angle of 17 degrees 14 minutes 00 seconds, a radius of 2,824.79 feet and a chord which bears South 77 degrees 38 minutes 50 seconds West, a distance of 846.44 feet, from which a concrete monument found, bears North 28 degrees East, a distance of 0.50 feet;
3. South 86 degrees 15 minutes 50 seconds West, a distance of 563.49 feet to a 1/2-inch iron rod set;
4. 562.37 feet along the arc of a curve to the left, said curve having a central angle of 16 degrees 31 minutes 30 seconds, a radius of 1,949.86 feet and a chord which bears South 78 degrees 00 minutes 05 seconds West, a distance of 560.42 feet, from which a concrete monument found, bears North 82 degrees East, a distance of 0.90 feet;
5. South 69 degrees 44 minutes 20 seconds West, a distance of 71.58 feet to a 1/2-inch iron rod (1847 cap) found at the southwesterly corner of aforesaid 71.001 acre tract, also being the southeasterly corner of a call 22.60 acre tract of land recorded in the name of Dufner, Elizabeth Anne in W.C.C.F. Number 2014063697.

Thence, leaving said R.O.W. line, with the westerly line of said 71.001 acre tract, North 10 degrees 42 minutes 53 seconds West, a distance of 2,663.31 feet to the northwesterly corner of the said 71.001 acre tract, and the northeasterly corner of said 22.60 Acre tract, from which a cotton spindle found bears North 16 degrees 21 minutes 55 seconds East, a distance of 0.50 feet;

Thence, with said northerly line of a said 22.60 acre tract, and a call 17.60 acre tract of land recorded in the name of Elizabeth Anne Dufner in W.C.C.F. Number 2014063597, and a call 93.60 acre tract of land recorded in the name of Arthur and Gordon Faubion in W.C.C.F. Number 2005043418, styled tract A. the following five (5) courses:

1. South 69 degrees 14 minutes 42 seconds West, a distance of 375.66 feet to a 1/2-inch iron rod set;
2. South 69 degrees 12 minutes 50 seconds West, a distance of 185.31 feet to a 1/2-inch iron rod set;
3. South 69 degrees 16 minutes 08 seconds West, a distance of 386.65 feet to 1/2-inch iron rod found;

4. South 69 degrees 43 minutes 16 seconds West, a distance of 277.23 feet to a 1/2-inch iron rod (1847 cap) found;
5. South 69 degrees 32 minutes 42 seconds West, a distance of 957.57 feet to a 1/2-inch iron rod (1847 cap) found at the northwesterly corner of said 93.60 acre tract;

Thence, with the westerly line of said 93.60 acre tract, the following three (3) courses:

1. South 21 degrees 20 minutes 43 seconds East, a distance of 854.12 feet to a 1/2-inch iron rod (1847 cap) found;
2. South 20 degrees 57 minutes 06 seconds East, a distance of 930.97 feet to a 1/2-inch iron rod found;
3. South 20 degrees 43 minutes 36 seconds East, a distance of 754.25 feet to the aforesaid northerly R.O.W. line of F.M. 2243, from which a 1/2-inch iron rod found, bears South 01 degree East, a distance of 0.39 feet;

Thence, with said northerly R.O.W. line, the following three (3) courses:

1. 63.33 feet along the arc of a curve to the right, said curve having a central angle of 00 degree 38 minutes 16 seconds, a radius of 5,689.53 feet and a chord which bears South 78 degrees 04 minutes 28 seconds West, a distance of 63.33 feet to a 1/2-inch iron rod set;
2. South 79 degrees 37 minutes 29 seconds West, a distance of 2,643.52 feet to a 1/2-inch iron rod found;
3. South 79 degrees 44 minutes 55 seconds West, a distance of 201.05 feet to the **Point of Beginning** and containing 1,156.001 acres of land.

GBI Partners, L.P.  
Ph: 512-296-2675  
December 20, 2018





**EXHIBIT A-2**

County: Williamson  
 Project: Water Oak South  
 Job No.: A180801  
 MBS No.: 15-128

**FIELD NOTES FOR 9.410 ACRES**

Being a 9.410 acre tract of land located in the J. Thompson Survey, Abstract Number 608 and being a portion of a called 77.902 acre tract of land recorded in the name of Edwin H. Hale, Jr. in W.C.C.F. No. 2017014736, said 9.410 acres being more particularly described by metes and bounds descriptions as follows (bearings are referenced to the Texas Coordinate System, NAD 1983, Central Zone);

**Beginning** at a 1/2-inch iron rod found at the most westerly corner of said 9.410 acre tract, the northerly line of aforesaid 195.193 acre tract, and an easterly line of aforesaid 77.399 acre tract;

Thence, with the westerly line of said 9.410 acre tract, 837.65 feet along the arc of a curve to the right, said curve having a central angle of 34 degrees 46 minutes 41 seconds, a radius of 1380.00 feet and a chord which bears North 03 degrees 17 minutes 54 seconds East, a distance of 824.85 feet to a 1/2-inch iron rod found at the southerly corner of aforesaid 203.137 acre tract;

Thence, through and across said 77.902 acre tract, the following two (2) courses:

1. 231.13 feet along the arc of a curve to the left, said curve having a central angle of 12 degrees 16 minutes 44 seconds, a radius of 1078.50 feet and a chord which bears South 70 degrees 45 minutes 37 seconds East, a distance of 230.69 feet to a 1/2-inch iron rod set;
2. 696.23 feet along the arc of a curve to the right, said curve having a central angle of 55 degrees 17 minutes 21 seconds, a radius of 721.50 feet and a chord which bears South 49 degrees 15 minutes 19 seconds East, a distance of 669.53 feet to a 1/2-inch iron rod found on the northerly line of aforesaid 195.193 acre tract;

Thence, with said northerly line, the following two courses:

1. South 68 degrees 09 minutes 20 seconds West, a distance of 590.44 feet to a 1/2-inch iron rod (1847 cap) found;
2. South 67 degrees 58 minutes 56 seconds West, a distance of 242.14 feet to the Point of Beginning and containing 9.410 acres of land.

GBI Partners, L.P.  
 Ph: 512-296-2675  
 October 17, 2018

DESCRIPTION OF 3.080 ACRES OF LAND OUT OF THE J. THOMPSON SURVEY, ABSTRACT NO. 608, SITUATED IN WILLIAMSON COUNTY, TEXAS; BEING A PORTION OF THAT CERTAIN 168.32 ACRE TRACT DESCRIBED IN A DEED TO CHARLIE A. BARTON AND WIFE, OLLIE A. BARTON OF RECORD IN VOLUME 470, PAGE 303 OF THE DEED RECORDS OF WILLIAMSON COUNTY, TEXAS, SAID 3.080 ACRES BEING MORE PARTICULARLY DESCRIBED BY METES AND BOUNDS AS FOLLOWS:

COMMENCING, at a 1/2 inch iron rod with cap found for the northwesterly corner of that certain 195.2 acre tract described as "Tract C", in a deed to Donald C. Faubion, an undivided 47%; Debra Ann Faubion, an undivided 26.5%; and Cynthia Jo Barba, an undivided 26.5% by the Partition and Exchange Deed of record in Document No. 2005043418 of the Official Public Records of Williamson County, Texas, the same being an angle point in the southerly line of said 168.32 acre tract and an angle point in the occupied easterly line of that certain 100 acre tract conveyed to Charles Grady Barton, of record in Volume 899, Page 791 of said Deed Records, also being the Point of Beginning of a Boundary Line Agreement recorded in Document No. 2005007159 of said Official Public Records;

THENCE, along the northerly line of said 195.2 acre tract and southerly line of said 168.32 acre tract, the following three courses and distances:

- 1) N67°39'04"E, a distance of 312.47 feet to a 1/2 inch iron rod with cap found;
- 2) N67°58'56"E, a distance of 378.76 feet to a 1/2 inch iron rod with cap found;
- 3) N68°09'20"E, a distance of 590.44 feet to a 1/2 inch iron rod with cap set for the POINT OF BEGINNING and the southwesterly corner hereof;

THENCE, leaving the northerly line of said 195.2 acre tract, over and across said 168.32 acre tract, along the westerly, northerly and easterly lines hereof, the following four (4) courses and distances:

- 1) Along a curve to the left having a radius of 721.50 feet, a central angle of 55°17'21", an arc length of 696.23 feet, a chord which bears N49°15'19"W, a distance of 669.53 feet to a 1/2 inch iron rod with cap set for a point of reverse curvature to the right;
- 2) Along said reverse curve to the right having a radius of 1078.50 feet, a central angle of 12°16'44", an arc length of 231.13 feet, a chord which bears N70°45'37"W, a distance of 230.69 feet to a 1/2 inch iron rod with cap set for the northwesterly corner hereof;
- 3) N80°42'01"E, a distance of 337.11 feet to a 1/2 inch iron rod with cap set for the point of curvature of a curve to the right;
- 4) Along said curve to the right having a radius of 878.50 feet, a central angle of 49°52'11", an arc length of 764.64 feet, a chord which bears S46°35'20"E, a distance of 740.73 feet to a 1/2 inch iron rod with cap set in the southerly line of said 168.32 acre tract for the southeasterly corner hereof, from which a 60-D nail found in concrete at the base of a metal fence post, for the common northerly corner of said 195.2 acre tract and that certain 51.56 acre tract described in the deed to Charles Grady Barton, of record in Volume 1976, Page 703, of the Official Records of Williamson County, Texas bears N68°08'39"E, a distance of 1031.81 feet;

**THENCE**, along the northerly line of said 195.2 acre tract and the southerly line of said 168.32 acre tract, the following two (2) courses and distances:

- 1) S68°08'39"W, a distance of 129.91 feet to a 2 inch pipe fence post found;
- 2) S68°09'20"W, a distance of 27.09 feet to the **POINT OF BEGINNING**, containing an area of 3.080 acres (134,164 square feet) of land, more or less, within these metes and bounds.

**BEARING BASIS:** THE BASIS OF BEARINGS FOR THIS SURVEY IS THE TEXAS COORDINATE SYSTEM, NAD83(96) CENTRAL ZONE, ESTABLISHED BY NGS OPUS SOLUTION USING CORS STATIONS DF5370, AF9638, DF4062, & DE5999.

**Exhibit B****Permitted Exceptions**

1. Restrictions contained in plat recorded under Document No. 2013033404, Official Public Records of Williamson County, Texas (Tract 2 only).
2. Environmental setback as shown on the plat recorded in Cabinet EE, Slide 310 of the Plat Records of Williamson County, Texas, as shown on the Survey dated December 20, 2018, prepared by Alan Jay Horton, Registered Professional Land Surveyor No. 5768 (the "Survey"). (TRACT 1)
3. Pipeline easement granted to Seminole Pipeline Company, by instrument dated July 1, 1981, recorded in Volume 844, Page 624 of the Deed Records of Williamson County, Texas and as amended in Volume 2171, Page 554 of the Official Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
4. Petroleum pipeline easement granted to Seminole Pipeline Company as recorded in Volume 851, Page 698 of the Official Public Records and as amended in Volume 2244, Page 297 of the Official Public Records and under Document No. 2018066453 of the Official Public Records, all of Williamson County, Texas. (TRACT 1)
5. 15 foot public utility easement dated August 9, 1999, granted by Norma Nell Faubion et al to City of Georgetown, recorded under Document No. 199955406 of the Official Public Records of Williamson County, Texas. (TRACT 1)
6. 15 foot utility easement executed by Anne V. Patience to City of Georgetown, dated April 29, 1999, recorded under Document No. 199968547 of the Real Property Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
7. Notice of Voluntary Inclusion into the Extraterritorial Jurisdiction of the City of Georgetown dated 10/12/1999 and recorded under Document No. 199971384 of the Official Public Records of Williamson County, Texas. (TRACT 1)
8. Water line easement executed by Thomas E. Dreiss, Trustee, to Brushy Creek Municipal Utility District, dated March 1, 2004. recorded under Document No. 2004018609 of the Real Property Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
9. Water line easement dated April 15, 2004, granted by Grady Barton and Carrie Ann Barton-Smith to Brushy Creek Municipal Utility District, recorded under Document No. 2004029224 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
10. Water line easement granted to Brushy Creek Municipal Utility District, by instrument dated June 22, 2004, recorded under Document No. 2004049691 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
11. Water line easement dated June 1, 2005, granted by Debra Ann Faubion et al to Brushy Creek Municipal Utility District, recorded under Document No. 2005040893 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)

12. All interests in water, together with all rights relating thereto, express or implied, reserved in instrument recorded under Document No. 2007014282 of the Official Records of Williamson County, Texas. (TRACT 1)
13. All oil, gas and other minerals, together with all rights relating thereto, express or implied, reserved in instrument recorded under Document No. 2007014282 of the Official Records of Williamson County, Texas. (TRACT 1)
14. Road Easement created in that certain Road and Sewer Line Easement Agreement dated February 22, 2007, recorded under Document No. 2007014284 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
15. Terms, conditions and stipulations of Road Improvements and Sewer Line Development and Conditional Easement Agreement by and between Thomas E. Dreiss, Trustee, and Laredo WO, Ltd., a Texas limited partnership, dated February 22, 2007, and recorded under Document No. 2007014288 of the Official Public Records of Williamson County, Texas, and as further affected by Document No. 2009022806 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
16. Amended and Restated Development Agreement filed of record under Document No. 2012027844, and as further affected under Document Nos. 2016008515, 2012006198 and 2018036246 of the Official Public Records of Williamson County, Texas.
17. Wastewater easement as recorded under Document Number 2007064713 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
18. Utility access easement as recorded under Document No. 2008085853 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
19. The terms, conditions and stipulations of that certain Sanitary Sewer Easement Agreement dated September 27, 2010, recorded under Document No. 2010065269 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
20. The terms, conditions and stipulations of that certain Drainage Easement Agreement dated September 27, 2010, recorded under Document No. 2010065270; and as amended under Document No. 2017104825 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
21. All terms, conditions, and provisions of that certain Agreement regarding Williamson County Municipal Utility District 25 dated January 11, 2012, recorded under Document No. 2012006198 of the Official Public Records of Williamson County, Texas.
22. Williamson County Regional Habitat Conservation Plan Memorandum of Participation Agreement Relative to U.S. Fish and Wildlife Service Permit dated May 15, 2012, recorded under Document No. 2012043627 of the Official Public Records of Williamson County, Texas.
23. Sanitary Sewer Easement Agreement dated August 1, 2013, recorded under Document No. 2013080603 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)

24. Drainage Easement Agreement as recorded under Document No. 2013095986 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
25. Sanitary Sewer Easement Agreement as recorded under Document No. 2013095987 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
26. The terms, conditions and stipulations of that certain Access Easement and Right of Way dated January 31, 2014, recorded under Document No. 2014011208 of the Official Public Records of Williamson County, Texas. (TRACT 1)
27. Drainage Easement Agreement as recorded under Document No. 2014026475 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
28. Sanitary Sewer Easement Agreement as recorded under Document No. 2014026476 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
29. The terms, conditions and stipulations of that certain Drainage Easement Agreement dated August 25, 2014, recorded under Document No. 2014071869 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
30. The terms, conditions and stipulations of that certain Sanitary Sewer Easement Agreement dated August 25, 2014, recorded under Document No. 2014071870 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
31. Waiver of Special Appraisal dated May 31, 2012, as recorded under Document No. 2014076279 of the Official Public Records of Williamson County, Texas.
32. Waiver of Special Appraisal dated August 19, 2014, as recorded under Document No. 2014076284 of the Official Public Records of Williamson County, Texas.
33. The terms, conditions and stipulations of that certain Permanent Easement Agreement dated January 21, 2016, recorded under Document No. 2016010600 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
34. Sanitary sewer easement granted to City of Georgetown, by instrument dated August 18, 2016, recorded under Document No. 2016077685 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
35. The terms, conditions and stipulations of that certain Wastewater Easement dated June 23, 2017, recorded under Document No. 2017098157 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
36. The terms, conditions and stipulations of that certain Utility Access Easement dated June 23, 2017, recorded under Document No. 2017098158 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
37. The terms, conditions and stipulations of that certain Roadway, Utility and Drainage Easement Agreement dated October 20, 2017, recorded under Document No. 2017098160 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)

38. The terms, conditions and stipulations of that certain Access Easement dated October 20, 2017, recorded under Document No. 2017098161 of the Official Public Records of Williamson County, Texas. (TRACT 1)
39. Guying utility easement granted to Pedernales Electric Cooperative, Inc., by instrument dated December 16, 2016, recorded under Document No. 2018062791 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 1)
40. Any and all easements and building setbacks shown on Plat(s) recorded under Document No(s). 2013033404 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 2)
41. Lot 2, Block G, to be reserved for use by the City of Georgetown Fire Department, as stated on the plat recorded under Document No. 2013033404 of the Official Public Records of Williamson County, Texas. (TRACT 2)
42. The terms, conditions and stipulations of that certain Water Line Easement and Right-of-Way dated March 24, 2006, recorded under Document No. 2006027343 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 2)
43. The terms, conditions and stipulations of that certain Memorandum of Development Agreement dated July 9, 2012, recorded under Document No. 2012056684 of the Official Public Records of Williamson County, Texas. (TRACT 2)
44. Wastewater easement granted to City of Georgetown, by instrument dated July 18, 2018, recorded under Document No. 2018075352 of the Official Public Records of Williamson County, Texas, as shown on the Survey. (TRACT 2)
45. Water Line Easements granted to Chisholm Trail Special Utility District as recorded under Document Nos. 2013044607, 2013044608, 2013044609, 2013044610, 2013044611, 2013044612, 2013044613, 2013044616, 2013044617, 2013048344, 2013062167, 2013064547, 2013062168, 2013091201, 2013100385, 2014019467, 2014025124, 2014025144, 2014033910, 2014038543, 2014038544, 2014047251, 2014047260, 2014058853, 2014058854 and 2014058871, all of the Official Public Records of Williamson County, Texas. (TRACT 2)
46. The rights of Williamson County Municipal Utility District No. 25 to levy taxes and issue bonds.

11-GF# 201802592 JPB  
RETURN TO: HERITAGE TITLE  
401 CONGRESS, SUITE 1500  
AUSTIN, TEXAS 78701



**ELECTRONICALLY RECORDED  
OFFICIAL PUBLIC RECORDS**

**2018114043**

Pages: 24 Fee: \$113.00  
12/31/2018 11:38 AM



*Nancy E. Rister*

Nancy E. Rister, County Clerk  
Williamson County, Texas

PRELIMINARY PLAT  
FOR  
PARKSIDE ON THE RIVER  
SECTIONS 8, 9A & 10A

GEORGETOWN, WILLIAMSON COUNTY, TEXAS  
2023-25-PP

OWNER/DEVELOPER: HM PARKSIDE, LP  
1011 NORTH LAMAR BLVD.,  
AUSTIN, TX 78703  
(512) 481-0303  
BLAKE@BLAKEMAGEECO.COM

ENGINEER/SURVEYOR: HR GREEN DEVELOPMENT TX, LLC  
5508 HIGHWAY 290 WEST, SUITE 150  
AUSTIN, TEXAS 78735  
512.872.6696  
SHERVIN.NOOSHIN@HRGREEN.COM

WATERSHED STATUS:

THIS SITE IS LOCATED IN THE SOUTH FORK OF THE SAN GABRIEL WATERSHED. THIS SITE IS LOCATED OVER THE EDWARDS AQUIFER RECHARGE ZONE AND CONTRIBUTING ZONE.

FLOODPLAIN INFORMATION:

PORTIONS OF THIS SUBDIVISION ARE WITHIN SPECIAL FLOOD HAZARD AREAS INUNDATED BY THE 100 YEAR FLOOD AS IDENTIFIED BY THE U.S. FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD INSURANCE RATE MAP NUMBER 48491C0460F, EFFECTIVE DATE DECEMBER 20, 2019.

LEGAL DESCRIPTION:

75.68 ACRES OF LAND IN THE JOSEPH THOMPSON SURVEY, ABSTRACT NO. 608 AND THE W.E. PATE SURVEY, ABSTRACT NO. 836, WILLIAMSON COUNTY, TEXAS; BEING A PORTION OF A CERTAIN CALLED 1,143.511 ACRE TRACT OF LAND, DESIGNATED AS TRACT 1, AND DESCRIBED IN THE SPECIAL WARRANTY DEED TO HM PARKSIDE, LP OF RECORD IN DOCUMENT NO. 2018114043, OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, AND ALSO BEING A PORTION OF A CERTAIN CALLED 314.00 ACRE TRACT OF LAND DESIGNATED AS TRACT 1 AND DESCRIBED IN THE SPECIAL WARRANTY DEED TO HM GP II, LP OF RECORD IN DOCUMENT NO. 2021027159, OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS

BENCHMARK NOTE:

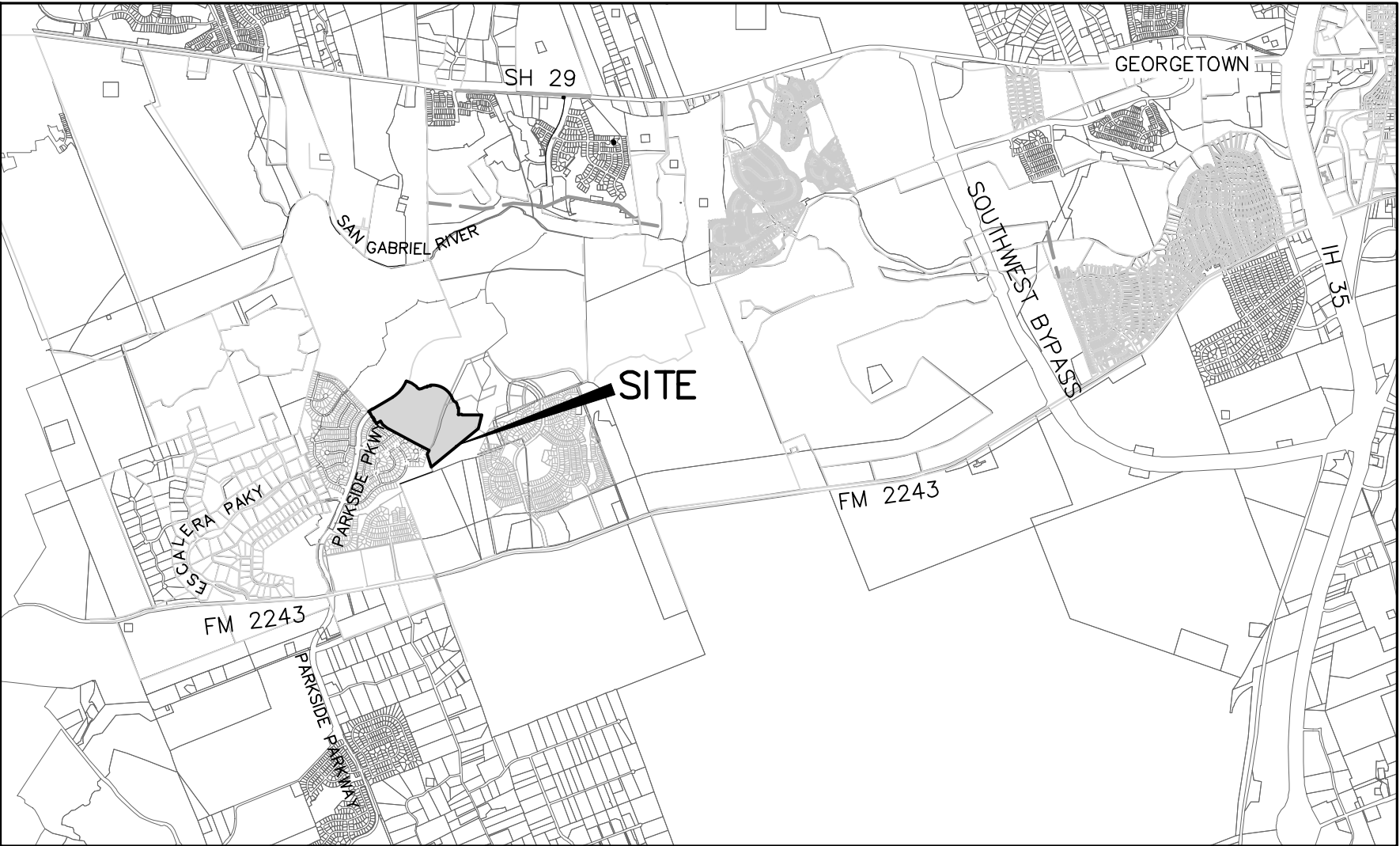
NAVD88 - GEOID12B

BM(1380)-221:  
COTTON GIN SPINDLE FOUND IN THE SOUTH EDGE  
OF A CONCRETE SIDEWALK  
ELEVATION = 962.21 FEET.

BM(1380)-700100:  
MAGNAIL WITH WASHER STAMPED HR GREEN  
SET IN CONCRETE RIM OF WATER MANHOLE  
ELEVATION = 940.16 FEET.

BM(1380)-700200:  
MAGNAIL WITH WASHER STAMPED HR GREEN  
SET IN CONCRETE BASE OF BOLLARD  
ELEVATION = 890.30 FEET.

INITIAL SUBMITTAL DATE: 08/21/2023



VICINITY MAP  
SCALE: 1"=4000'

PROJECT SUMMARY

TOTAL SITE AREA: 75.68 ACRES

RESIDENTIAL LOTS..... - 191 (41.00 ACRES)

OPEN SPACE LOTS..... - 6 (0.87 ACRES)

OPEN SPACE  
/DRAINAGE LOTS..... - 2 (14.60 ACRES)

OPEN SPACE  
/DRAINAGE  
/WATER QUALITY LOTS..... - 1 (4.83 ACRES)

TOTAL LOTS..... - 200 (61.30 ACRES)

NUMBER OF BLOCKS..... - 8

STREETS (ROW AREA): 14.38 ACRES

STREETS

NAME	CLASSIFICATION	ROW WIDTH	MIN. PVMT WIDTH (F-F)	CURB TYPE	DESIGN SPEED	LENGTH (LF)	CUL-DE-SAC	MAINTENANCE AUTHORITY
PARKSIDE PARKWAY	MINOR ARTERIAL	135'	48"	24" CURB & GUTTER	40 MPH	1,340	NONE	PUBLIC
PEACEFUL SERENITY DRIVE	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	308	NONE	PUBLIC
PANSY TRAIL	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	2,011	NONE	PUBLIC
BEAUTIFUL WISDOM COURT	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	143	60' RADIUS	PUBLIC
ANGELS JOY COVE	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	77	60' RADIUS	PUBLIC
TWISTED TARPLEY LANE	LOCAL STREET	VARIES	28"	24" CURB & GUTTER	25 MPH	1,649	NONE	PUBLIC
GLORIOUS GARDEN WAY	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	155	NONE	PUBLIC
WHITE DAISY LANE	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	1,426	NONE	PUBLIC
SCARLET SAGE DRIVE	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	1,143	60' RADIUS	PUBLIC
MIGHTY COUNSELOR LANE	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	1,096	NONE	PUBLIC
GLORIOUS DAY COVE	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	216	60' RADIUS	PUBLIC
GOLDEN RIGHTEOUS COURT	LOCAL STREET	50'	28"	24" CURB & GUTTER	25 MPH	190	60' RADIUS	PUBLIC

SHEET INDEX

SHEET NUMBER	SHEET TITLE
1	COVER SHEET
2	OVERALL PRELIMINARY PLAT
3	PHASING PLAN
4	PRELIMINARY PLAT (1 OF 2)
5	PRELIMINARY PLAT (2 OF 2)
6	CURVE TABLES
7	PRELIMINARY PLAT NOTES



Know what's below.  
Call before you dig.

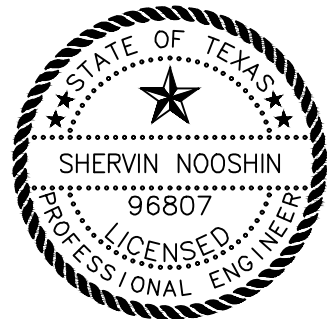
5508 HIGHWAY 290 WEST  
SUITE 150  
AUSTIN, TX 78735  
512.872.6696  
HRGREEN.COM

TPE NO. 10384  
TPELS NO. 10194101



COVER SHEET

PARKSIDE ON THE RIVER  
SECTIONS 8, 9A & 10A  
PRELIMINARY PLAT  
GEORGETOWN, WILLIAMSON, TEXAS



SUBMITTAL DATE : DECEMBER 1, 2023

SUBMITTED BY : *Shervin Nooshin* 12/1/2023

SHERVIN NOOSHIN, P.E. DATE

HR GREEN DEVELOPMENT TX, LLC  
5508 HIGHWAY 290 WEST, SUITE 150  
AUSTIN, TEXAS 78735  
512.872.6696

I, SHERVIN NOOSHIN, P.E., CERTIFY THAT THESE ENGINEERING DOCUMENTS ARE COMPLETE, ACCURATE AND ADEQUATE FOR THE INTENDED PURPOSES, INCLUDING CONSTRUCTION, BUT ARE NOT AUTHORIZED FOR CONSTRUCTION PRIOR TO FORMAL CITY APPROVAL.

DESIGNED BY: CC

DRAWN BY: MM

CHECKED BY: SN

APPROVED BY: \_\_\_\_\_

SHEET 1 OF 7

2023 - 25 - PP





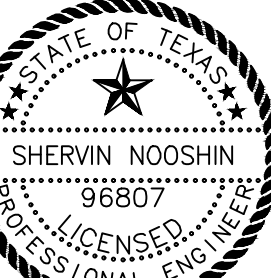
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SIGNED

DRAWN BY:

AWN BT.

HECKED B

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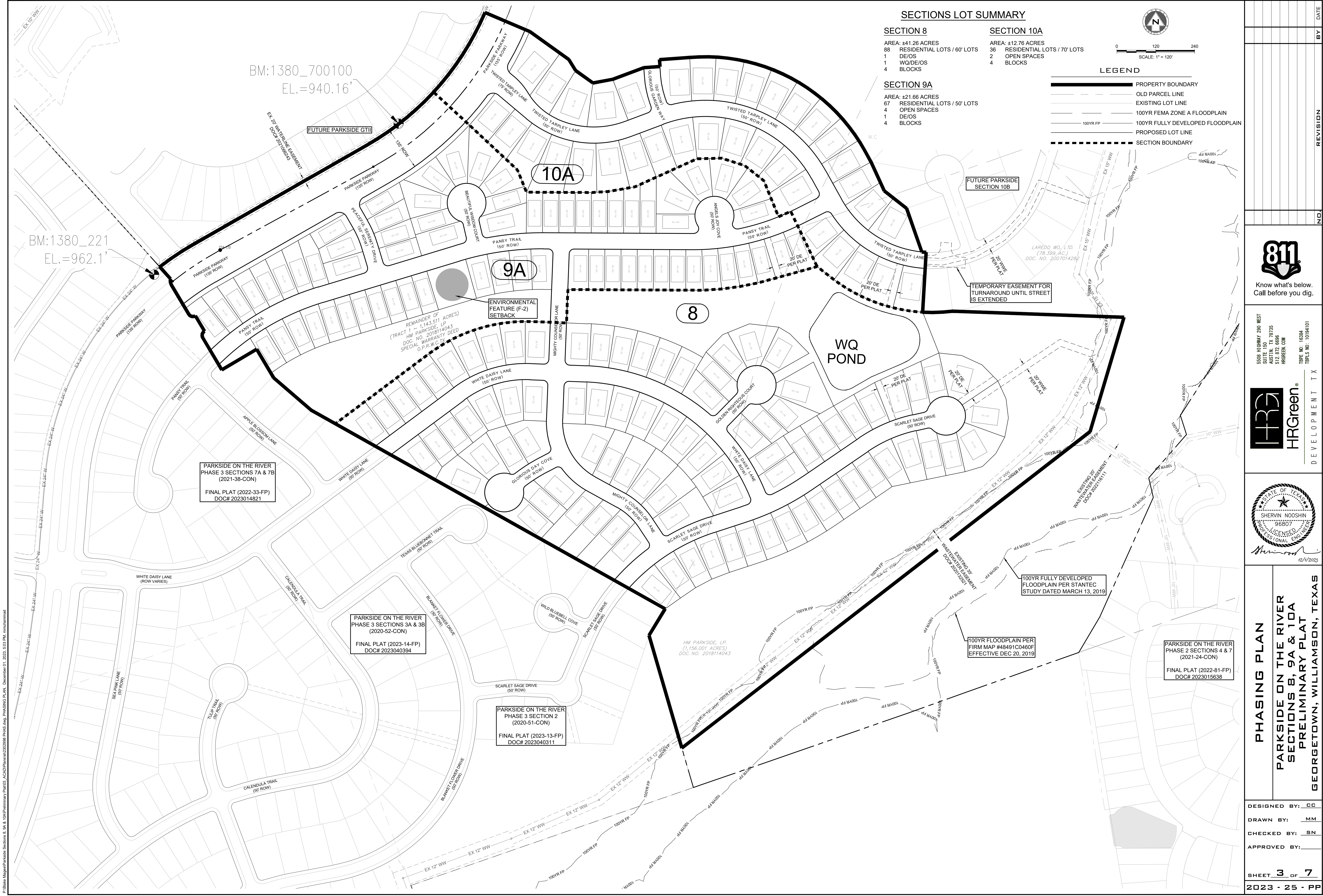
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**FEET 2**

\_\_\_\_\_

23 - 2





SECTIONS LOT SUMMARY

SECTION 8

AREA: ±41.26 ACRES  
88 RESIDENTIAL LOTS / 60' LOTS  
1 DE/OS  
1 WQ/DE/OS  
4 BLOCKS

SECTION 10A

AREA: ±12.78 ACRES  
36 RESIDENTIAL LOTS / 70' LOTS  
2 OPEN SPACES  
4 BLOCKS

SECTION 9A

AREA: ±21.66 ACRES  
67 RESIDENTIAL LOTS / 50' LOTS  
4 OPEN SPACES  
1 DE/OS  
4 BLOCKS

LEGEND

- PROPERTY BOUNDARY
- OLD PARCEL LINE
- EXISTING LOT LINE
- 100YR FEMA ZONE A FLOODPLAIN
- 100YR FULLY DEVELOPED FLOODPLAIN
- PROPOSED LOT LINE
- SECTION BOUNDARY

0 120 240  
SCALE: 1" = 120'

| NO. | REVISION | BY | DATE |
|-----|----------|----|------|
|     |          |    |      |
|     |          |    |      |
|     |          |    |      |
|     |          |    |      |

Know what's below.  
Call before you dig.

5508 HIGHWAY 290 WEST  
SUITE 150  
MCKINNEY, TX 75069  
CITY OF MCKINNEY  
HRGREEN, CON  
TIRE NO. 10384  
TIFLS NO. 10194101  
DEVELOPMENT TX

12/1/2023

PHASING PLAN

PARKSIDE ON THE RIVER  
SECTIONS 8, 9A & 10A  
PRELIMINARY PLAT  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC

DRAWN BY: MM

CHECKED BY: SN

APPROVED BY: \_\_\_\_\_

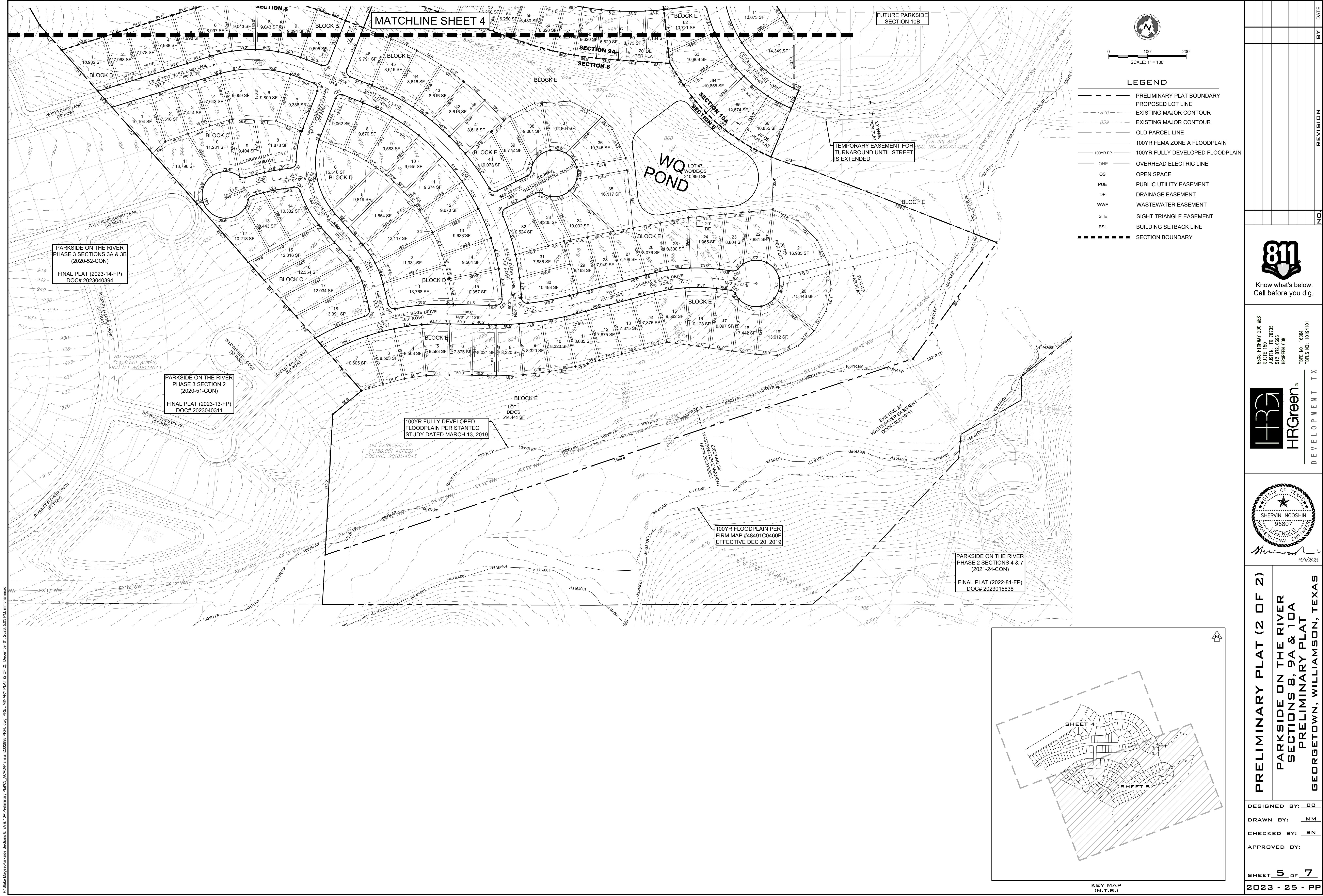
SHEET 3 OF 7

2023 - 25 - PP









|                                                                                                    |  |                                     |
|----------------------------------------------------------------------------------------------------|--|-------------------------------------|
| BY                                                                                                 |  | DATE                                |
| REVISION                                                                                           |  |                                     |
| 811                                                                                                |  |                                     |
| Know what's below.<br>Call before you dig.                                                         |  |                                     |
| 5508 HIGHWAY 290 WEST<br>SUITE 150<br>MCKINNEY, TX 75069<br>HRGREEN.COM                            |  | TYPE NO: 10384<br>TPLS NO: 10194101 |
|                                                                                                    |  | DEVELOPMENT TX                      |
|                                                                                                    |  |                                     |
| PRELIMINARY PLAT (2 OF 2)                                                                          |  |                                     |
| PARKSIDE ON THE RIVER<br>SECTIONS 8, 9A & 10A<br>PRELIMINARY PLAT<br>GEORGETOWN, WILLIAMSON, TEXAS |  |                                     |
| DESIGNED BY: CC                                                                                    |  |                                     |
| DRAWN BY: MM                                                                                       |  |                                     |
| CHECKED BY: SN                                                                                     |  |                                     |
| APPROVED BY:                                                                                       |  |                                     |
| SHEET 5 OF 7                                                                                       |  |                                     |
| 2023 - 25 - PP                                                                                     |  |                                     |







I, SHERVIN NOOSHIN, REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS SUBDIVISION IS IN THE EDWARDS AQUIFER RECHARGE ZONE AND CONTRIBUTING ZONE, THAT PORTIONS OF THIS SUBDIVISION ARE WITHIN ZONE A FLOOD AREA, AS DENOTED HEREIN, AS DEFINED BY FEDERAL EMERGENCY MANAGEMENT ADMINISTRATION FLOOD HAZARD FLOOD INSURANCE RATE MAP, COMMUNITY PANEL NUMBER 48491C0460F, EFFECTIVE DATE DECEMBER 20, 2019, AND THAT EACH LOT CONFORMS TO THE CITY OF GEORGETOWN REGULATIONS AS MODIFIED BY THE DEVELOPMENT AGREEMENT.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT AUSTIN, TRAVIS COUNTY, TEXAS, THIS  
 \_\_\_\_ DAY OF \_\_\_\_\_, 20\_\_\_\_.

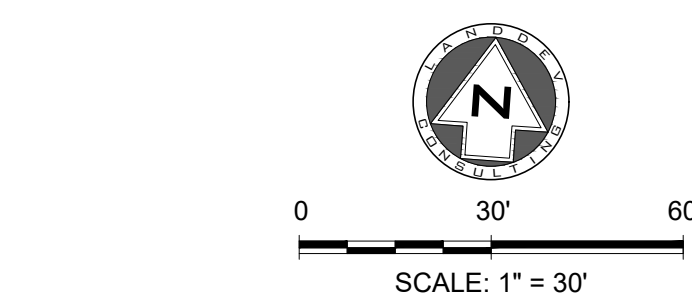
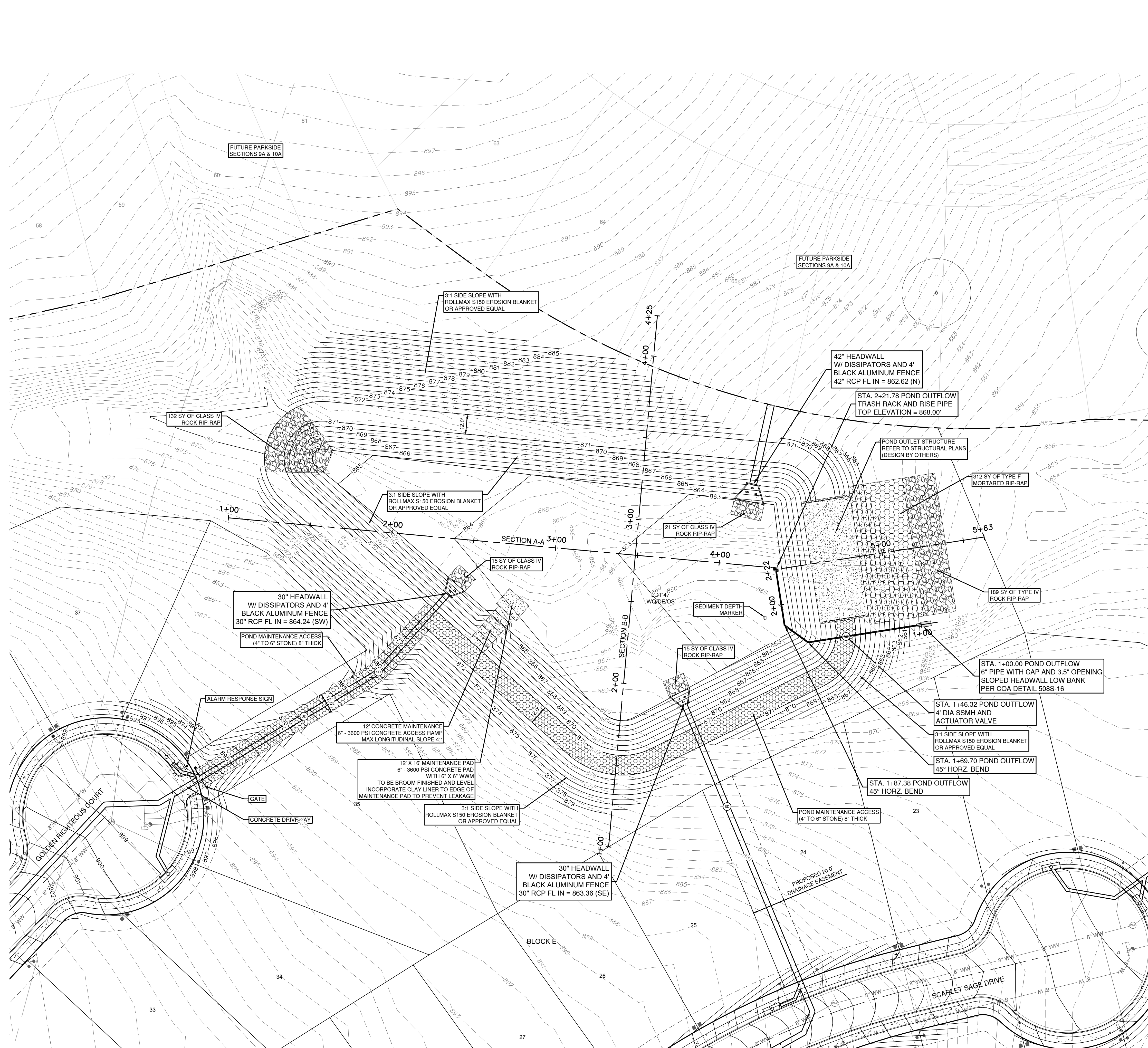
## METES AND BOUNDS

**BEARING BASIS:** TEXAS COORDINATE SYSTEM, CENTRAL ZONE, NAD83, GRID.

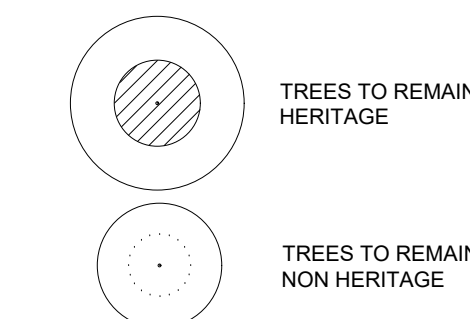
APPROVED BY:\_\_\_\_\_

2023 - 25 - P1

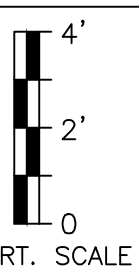




- ### LEGEND
- |     |     |     |                                  |
|-----|-----|-----|----------------------------------|
| --- | 834 | --- | EXISTING MINOR CONTOUR           |
| --- | 835 | --- | EXISTING MAJOR CONTOUR           |
| --- | 834 | --- | PROPOSED MINOR CONTOUR           |
| --- | 835 | --- | PROPOSED MAJOR CONTOUR           |
| --- |     | --- | BOUNDARY                         |
| --- |     | --- | EASEMENT                         |
| --- |     | --- | 100YR FEMA ZONE A FLOODPLAIN     |
| --- |     | --- | 100YR FULLY DEVELOPED FLOODPLAIN |
| --- |     | --- | 100YR FP                         |
| --- | SD  | --- | PROPOSED STORM LINE              |
| --- |     | --- | FIRE HYDRANT                     |
| --- |     | --- | WATER VALVE                      |
| --- | SD  | --- | STORM SEWER MANHOLE              |
| --- | WW  | --- | WASTEWATER MANHOLE               |
| --- |     | --- | CURB INLET                       |



VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'



GENERAL NOTES:

1. CONTRACTOR TO UTILIZE A TEMPORARY CONSTRUCTION PUMP TO DISCHARGE WATER FROM THE POND AFTER A RAINFALL EVENT. DURING CONSTRUCTION, PUMP IS TO DISCHARGE UPSTREAM OF PROPOSED ROCK BERM LOCATED BEFORE THE CREEK BED. AT NO TIME SHALL THE PUMP BE DISCHARGED DIRECTLY INTO STORMSEWER SYSTEM BEFORE CROSSING A ROCK BERM.
2. ALL MUD, DIRT, ROCKS, DEBRIS, ETC., SPILLED, TRACKED OR OTHERWISE DEPOSITED ON EXISTING PAVED STREETS, DRIVES, AND AREAS USED BY THE PUBLIC SHALL BE CLEANED UP IMMEDIATELY. CONTRACTOR WILL CLEAN UP SPOILS THAT MIGRATE ONTO ROADS A MINIMUM OF ONCE DAILY.
3. ALL DISTURBED AREAS TO BE REVEGETATED PRIOR TO ACCEPTANCE.
4. IF DISTURBED AREA IS NOT TO BE WORKED ON FOR MORE THAN 14 DAYS, DISTURBED AREA NEEDS TO BE STABILIZED BY REVEGETATION, MULCH, TARP OR REVEGETATION MATTING.
5. THE STABILIZED CONSTRUCTION ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT INTO PUBLIC ROADWAY.
6. THE ENVIRONMENTAL INSPECTOR HAS THE AUTHORITY TO ADD AND/OR MODIFY EROSION/SEDIMENTATION CONTROLS ON SITE TO KEEP PROJECT IN COMPLIANCE WITH THE CITY OF GEORGETOWN RULES AND REGULATIONS. HOWEVER, MODIFICATIONS TO THE ENGINEERING DESIGN AND FUNCTIONS OF THE EROSION AND SEDIMENTATION CONTROLS SYSTEMS CONTAINED HERE IN IS STRICTLY FORBIDDEN WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE SIGNING PROJECT PROFESSIONAL ENGINEER (TAC22 §137.3 AND §137.37).
7. CONTRACTOR SHALL UTILIZE DUST CONTROL MEASURES DURING SITE CONSTRUCTION SUCH AS IRRIGATION TRUCKS AND MULCHING AS PER 1.4.5(a) OR AS DIRECTED BY THE ENVIRONMENTAL INSPECTOR.
8. ALL POND BOTTOMS, SIDE SLOPES, AND EARTHEN EMBANKMENTS SHALL BE COMPACTED TO 95% MAXIMUM DENSITY, IN ACCORDANCE WITH THE CITY OF GEORGETOWN STANDARD SPECIFICATIONS AND PER GEOTECHNICAL ENGINEER'S RECOMMENDATION. ALLOW ADEQUATE VOLUME FOR TOPSOIL TO SUPPORT VEGETATION.
9. GRADING WITHIN THE 1/2 CRITICAL ROOT ZONE OF PROTECTED TREES, IDENTIFIED BY A HATCH PATTERN ON THESE PLANS, SHALL BE LIMITED TO LESS THAN 12 INCHES OF DISTURBANCE. NO GRADING ACTIVITY WITH DISTURBANCE OF MORE THAN 6 INCHES IS ALLOWED IN THE 1/4 CRITICAL ROOT ZONE.
10. GRADING WORK WITHIN THE 1/2 CRITICAL ROOT ZONE OF ALL PROTECTED TREES SHALL BE DONE BY HAND OR WITH RUBBER Tired EQUIPMENT.
11. ALL RETAINING WALLS GREATER THAN FOUR FEET IN HEIGHT MEASURED FROM THE BOTTOM OF THE FOOTING TO THE TOP OF THE WALL SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER.

DEWATERING PLAN NOTES

1. CONTRACTOR SHALL MAINTAIN THE DEWATERING SYSTEM TO ENSURE PERFORMANCE. IF THE DEWATERING SYSTEM IS NOT PERFORMING, THE CONTRACTOR MUST IMMEDIATELY MAKE THE NECESSARY MODIFICATIONS, FOLLOWING THE ENVIRONMENTAL INSPECTOR'S DIRECTION TO ENSURE ADEQUATE SYSTEM PERFORMANCE. CONTRACTOR SHALL PROVIDE THE DEWATERING PLAN AT THE PRECONSTRUCTION MEETING.
2. THE SKIMMER IS TO BE USED DURING CONSTRUCTION AND SHALL BE REMOVED AFTER COMPLETING CONSTRUCTION OF THE BATCH DETENTION POND.

**WATER QUALITY POND**

DESIGNED BY: CC  
DRAWN BY: TG/MK  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_

SHEET 37 OF 72

2023-XX-CON

5508 HIGHWAY 290 WEST  
SUITE 150  
AUSTIN, TX 78735  
512.872.6696  
HARGREEN.COM

BPPE NO: 16384  
BPLS NO: 10194101



IRGreen®

**WATER QUALITY POND**

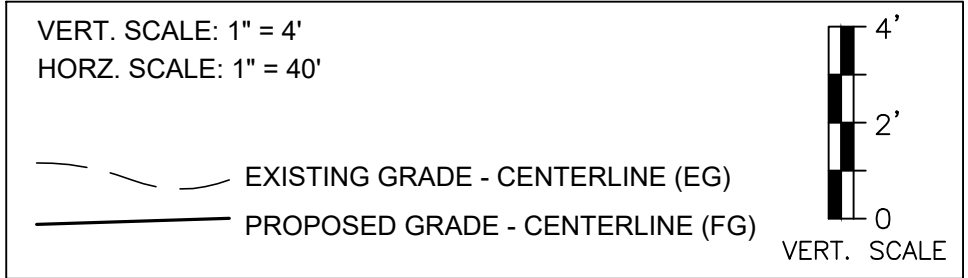
DESIGNED BY: CC  
DRAWN BY: TG/MK  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_

SHEET 37 OF 72

2023-XX-CON



SECTION A-A



Pond Volume

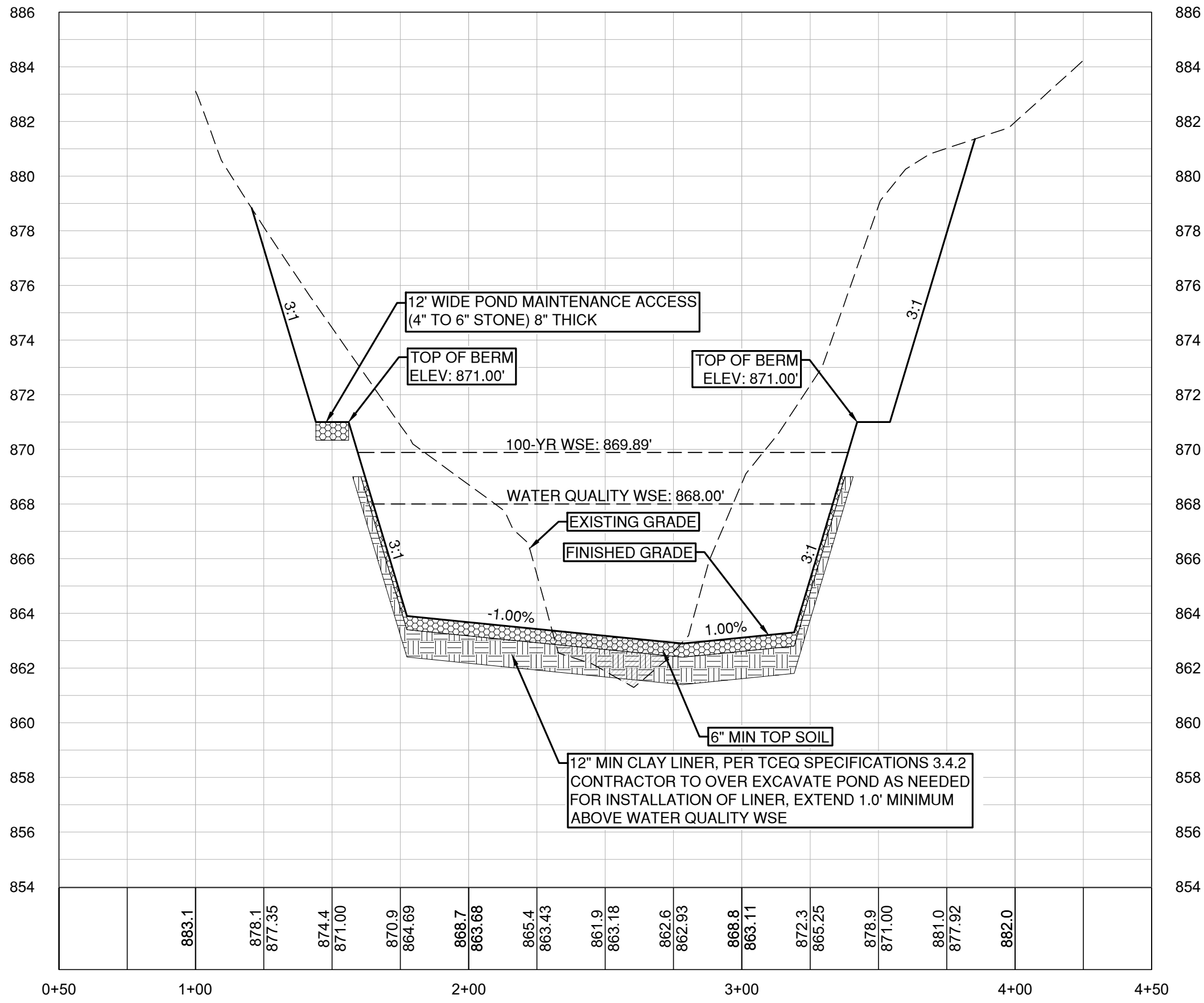
| Elevation | Area   |      | Volume |       | Cumulative Volume |       | Comments             |
|-----------|--------|------|--------|-------|-------------------|-------|----------------------|
|           | SF     | ac   | cf     | ac*ft | cf                | ac*ft |                      |
| 862       | 0      | 0.00 |        |       |                   |       | Water Quality Volume |
| 863       | 7,571  | 0.17 | 3,786  | 0.09  | 3,786             | 0.09  |                      |
| 864       | 21,138 | 0.49 | 14,355 | 0.33  | 18,140            | 0.42  |                      |
| 865       | 27,889 | 0.64 | 24,514 | 0.56  | 42,654            | 0.98  |                      |
| 866       | 30,599 | 0.70 | 29,244 | 0.67  | 71,898            | 1.65  |                      |
| 867       | 32,964 | 0.76 | 31,782 | 0.73  | 103,679           | 2.38  |                      |
| 868       | 35,384 | 0.81 | 34,174 | 0.78  | 137,853           | 3.16  |                      |
| 869       | 37,861 | 0.87 | 36,623 | 0.84  | 174,476           | 4.01  | Routing              |
| 870       | 40,391 | 0.93 | 39,126 | 0.90  | 213,602           | 4.90  |                      |
| 871       | 42,978 | 0.99 | 41,685 | 0.96  | 255,286           | 5.86  | Freeboard            |

| OUTFLOW STRUCTURE |      |
|-------------------|------|
| Elevation         | Flow |
| ft                | cfs  |
| 868.00            | 0    |
| 868.50            | 67   |
| 869.00            | 190  |
| 869.50            | 349  |
| 870.00            | 537  |
| 870.50            | 750  |
| 871.00            | 986  |

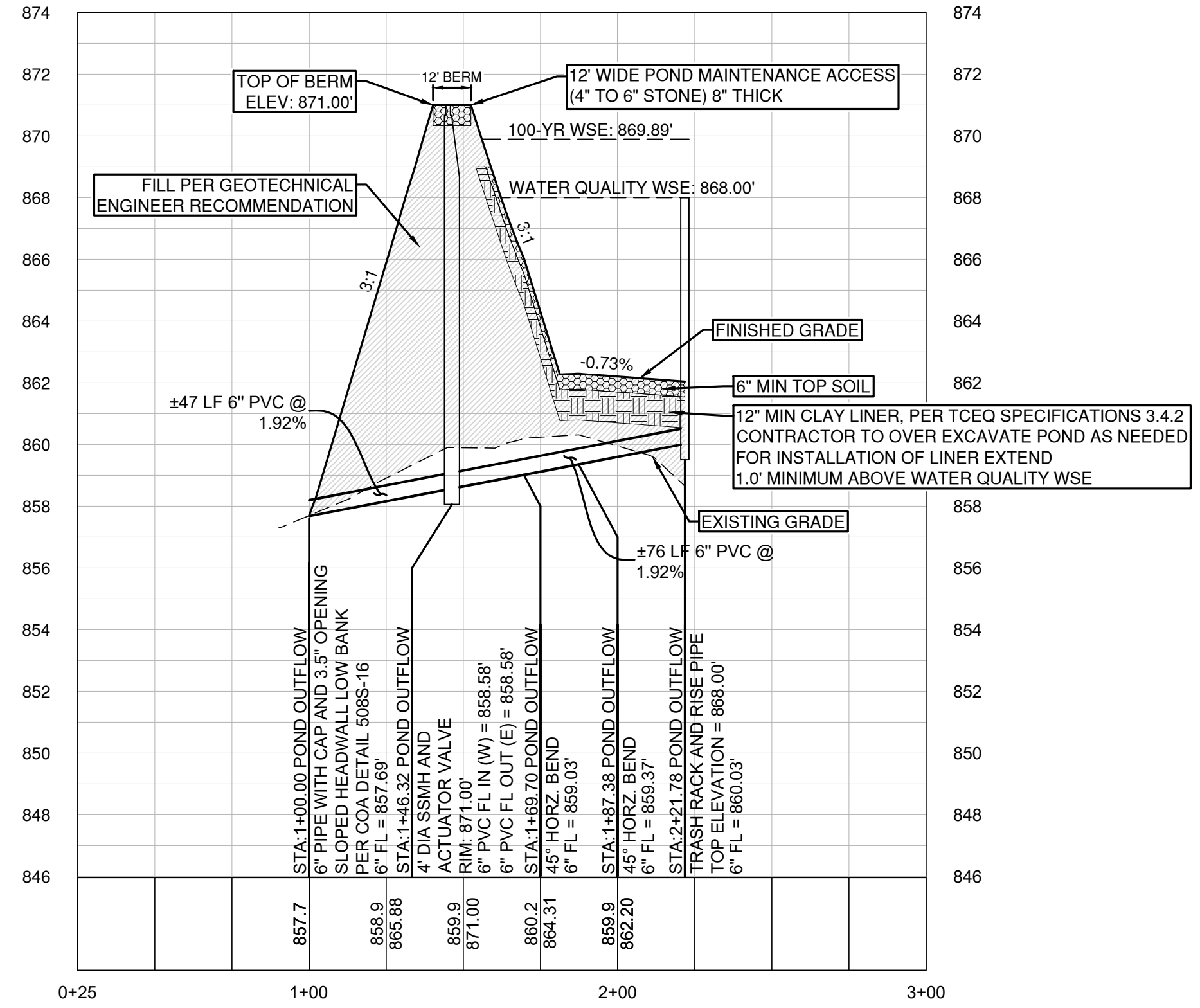
$$Q = C_w L H^{1.5}$$

$Q$  - weir flow rate (cfs)  
 $C_w$  - Weir Coefficient BROAD: 2.60  
 $L$  - horizontal length of weir crest (ft) BROAD: 73 FT  
 $H$  - head above weir crest elevation (ft)

SECTION B-B

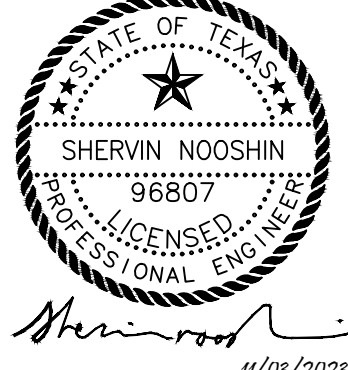


POND OUTFLOW



5508 HIGHWAY 290 WEST  
SUITE 150  
DALLAS, TX 75235  
CHAS. GREEN, INC.  
HARGREEN.COM

TYPE NO.: 10384  
RPLS NO.: 10194101



POND SECTIONS

PARKSIDE SECTION B

CONSTRUCTION PLANS

GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC

DRAWN BY: TB/MKM

CHECKED BY: SN

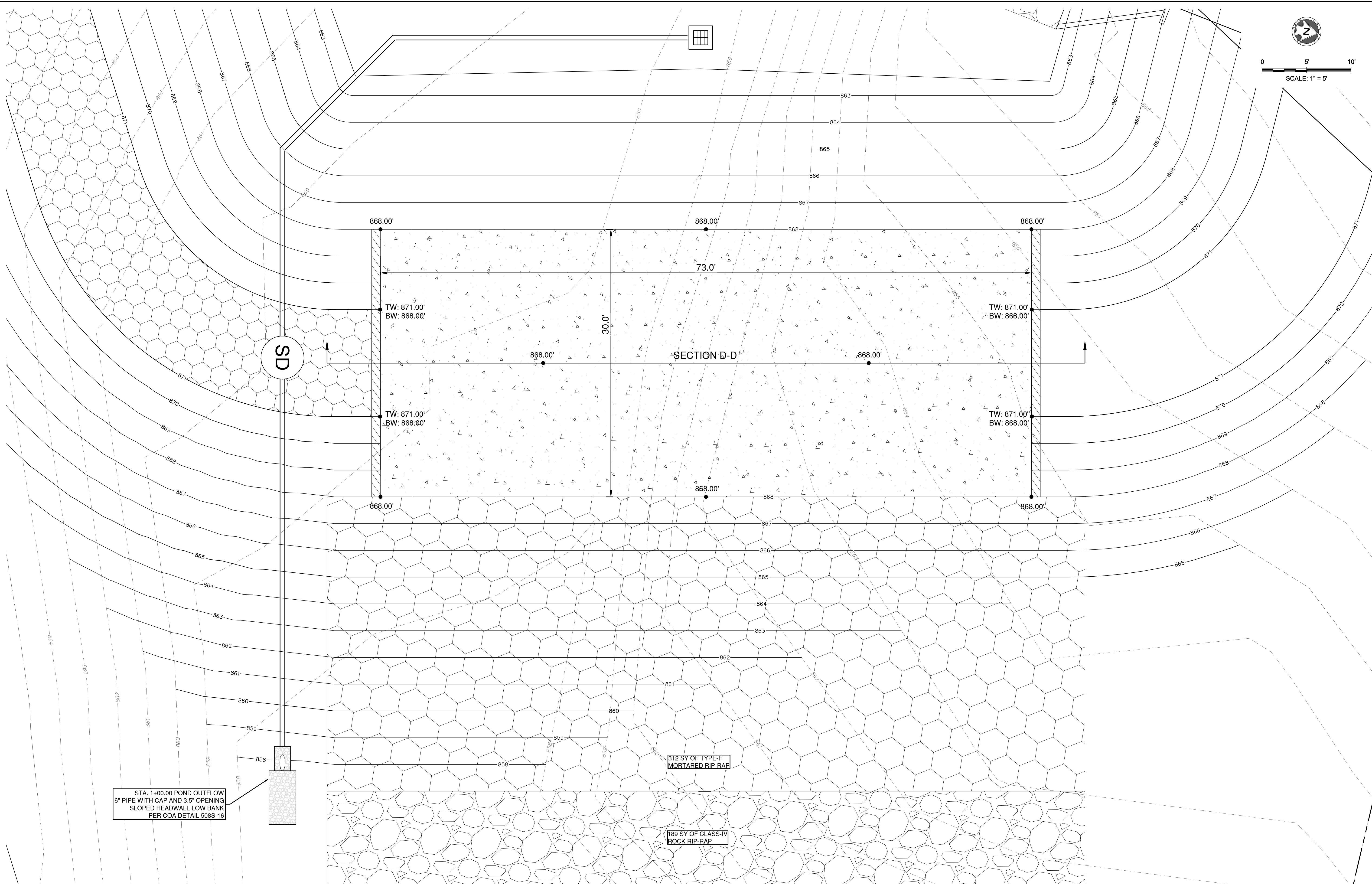
APPROVED BY: \_\_\_\_\_

SHEET 38 OF 72

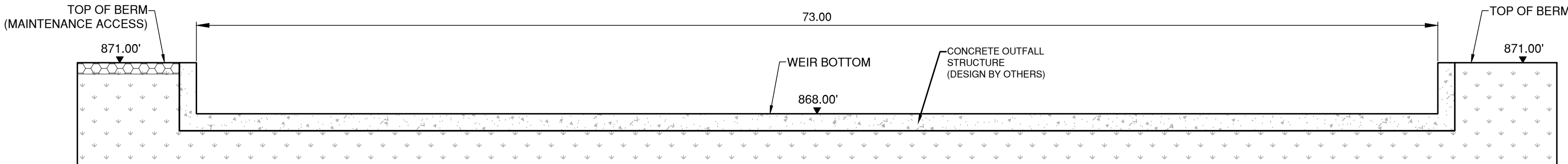
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P:\blake\_mfg\parkside sections 8, 9 & 10\section 803\_ACOFPlan\1203297 POND JWS\_POND OUTLET STRUCTURE DETAILS November 01, 2023 10:33 PM. lgraza



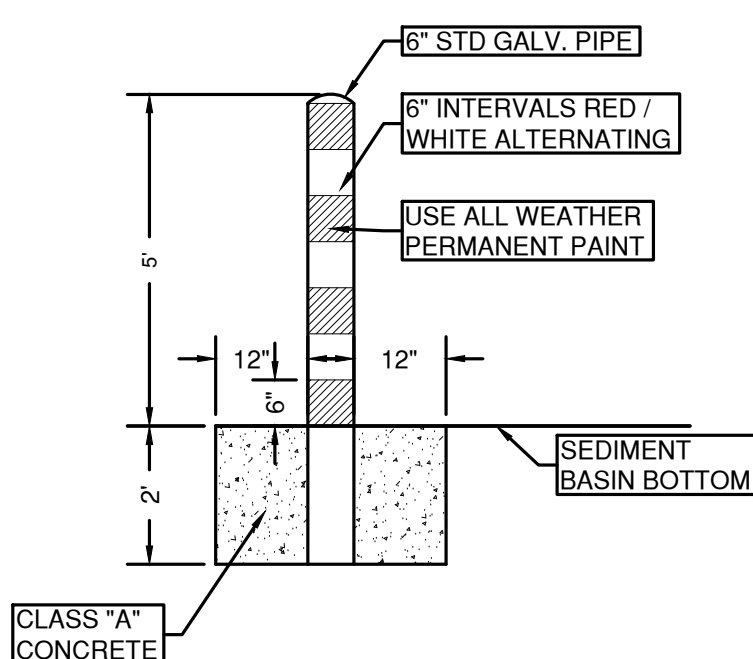
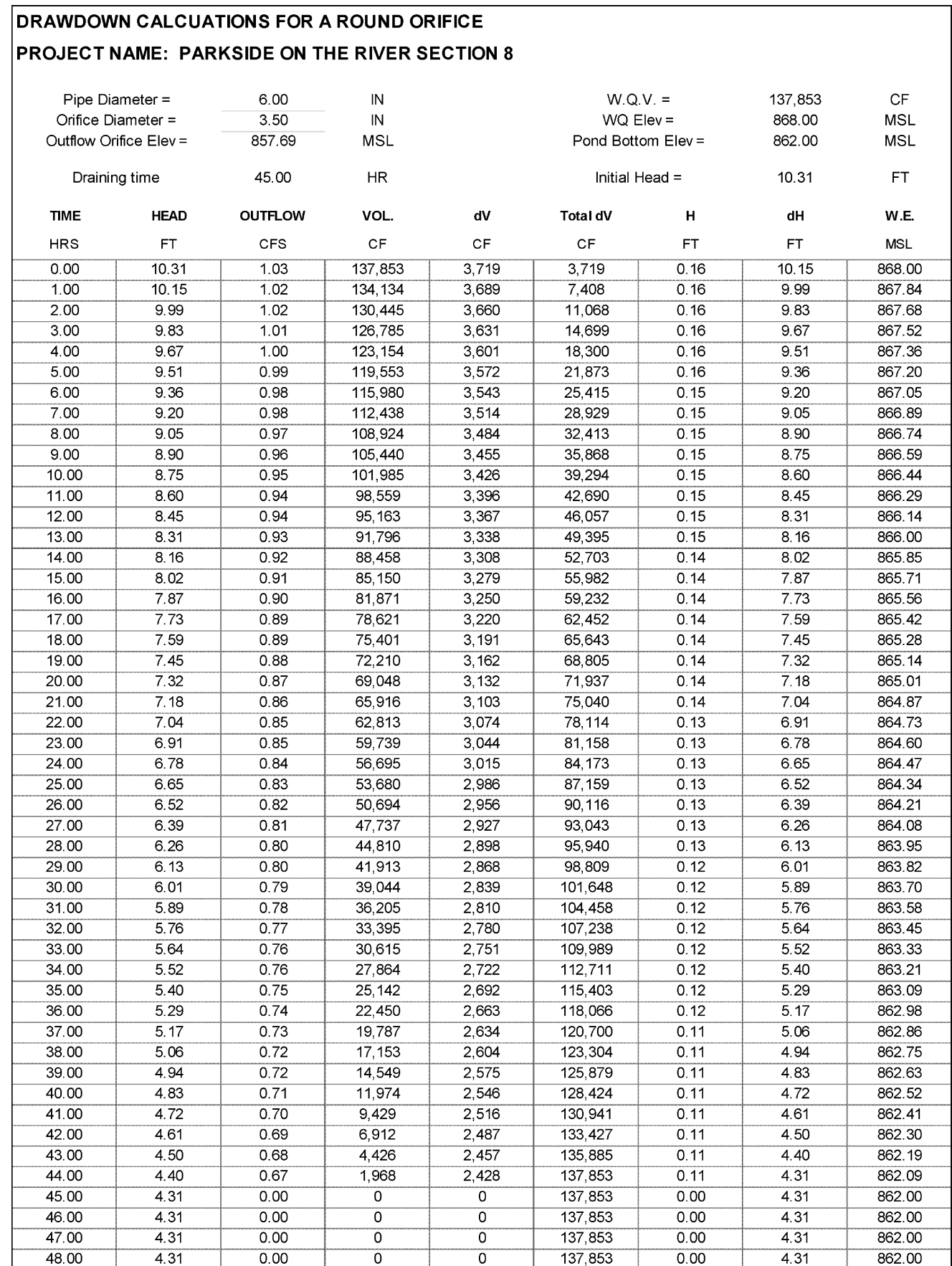
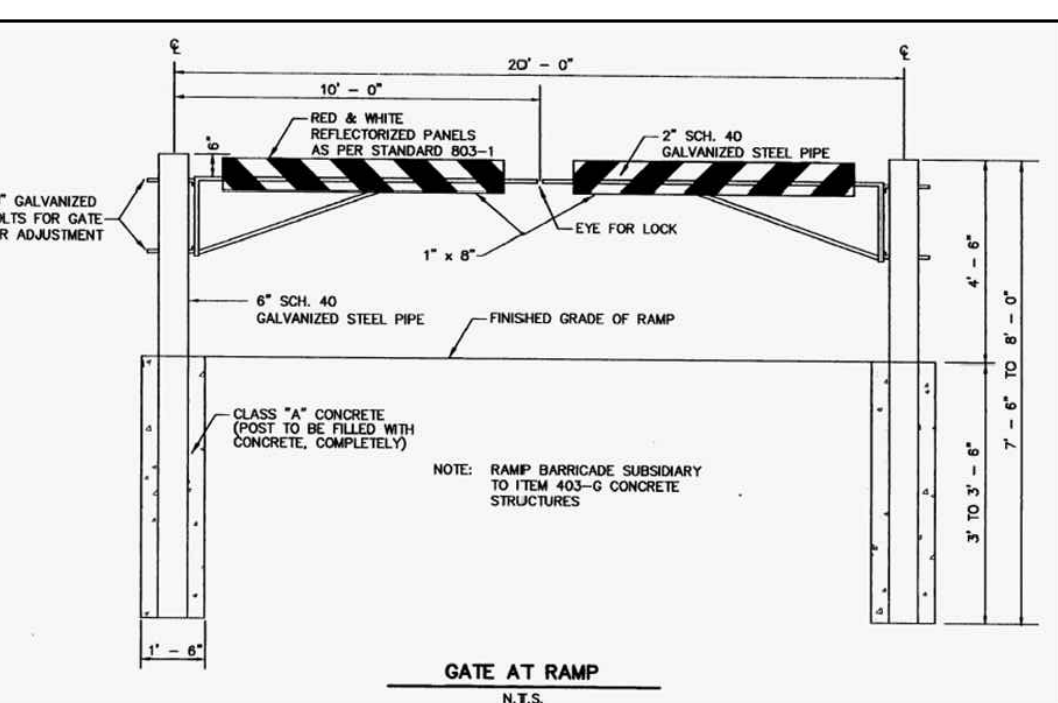
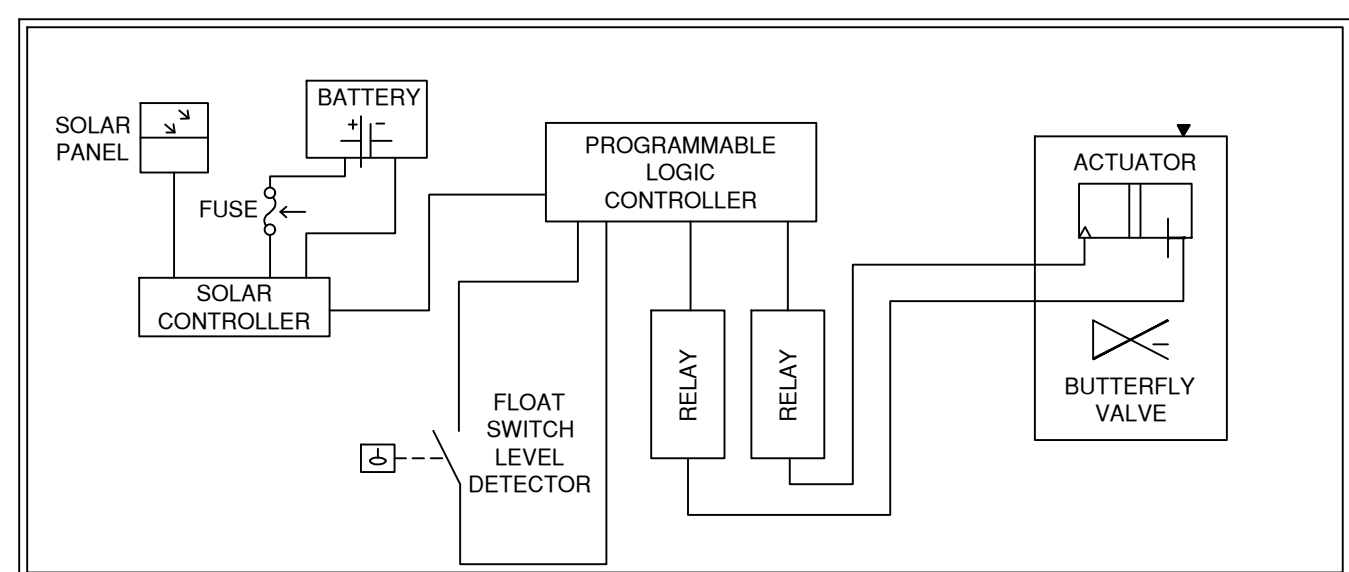
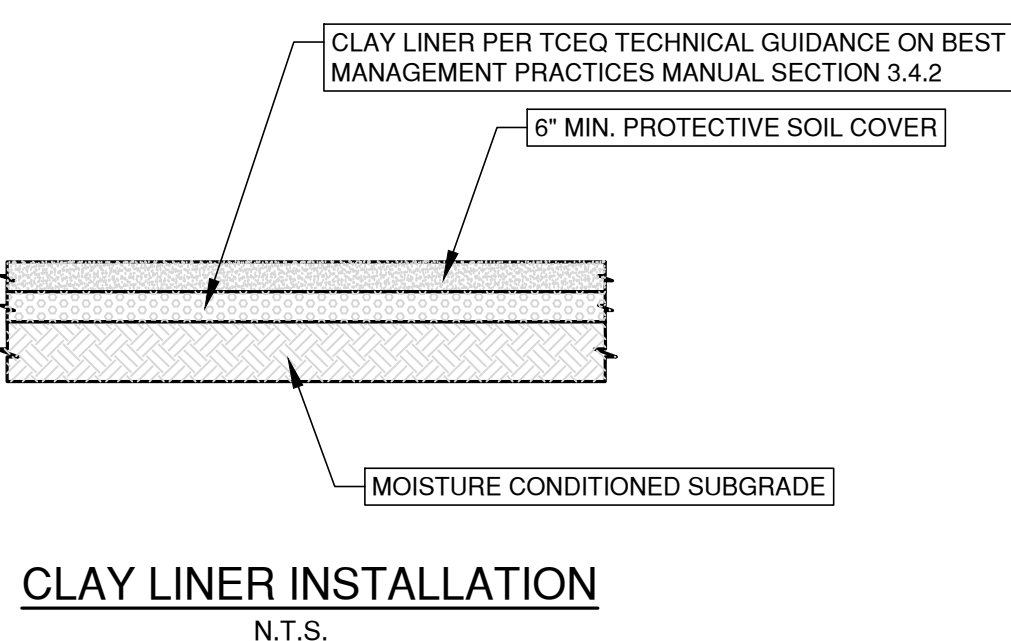
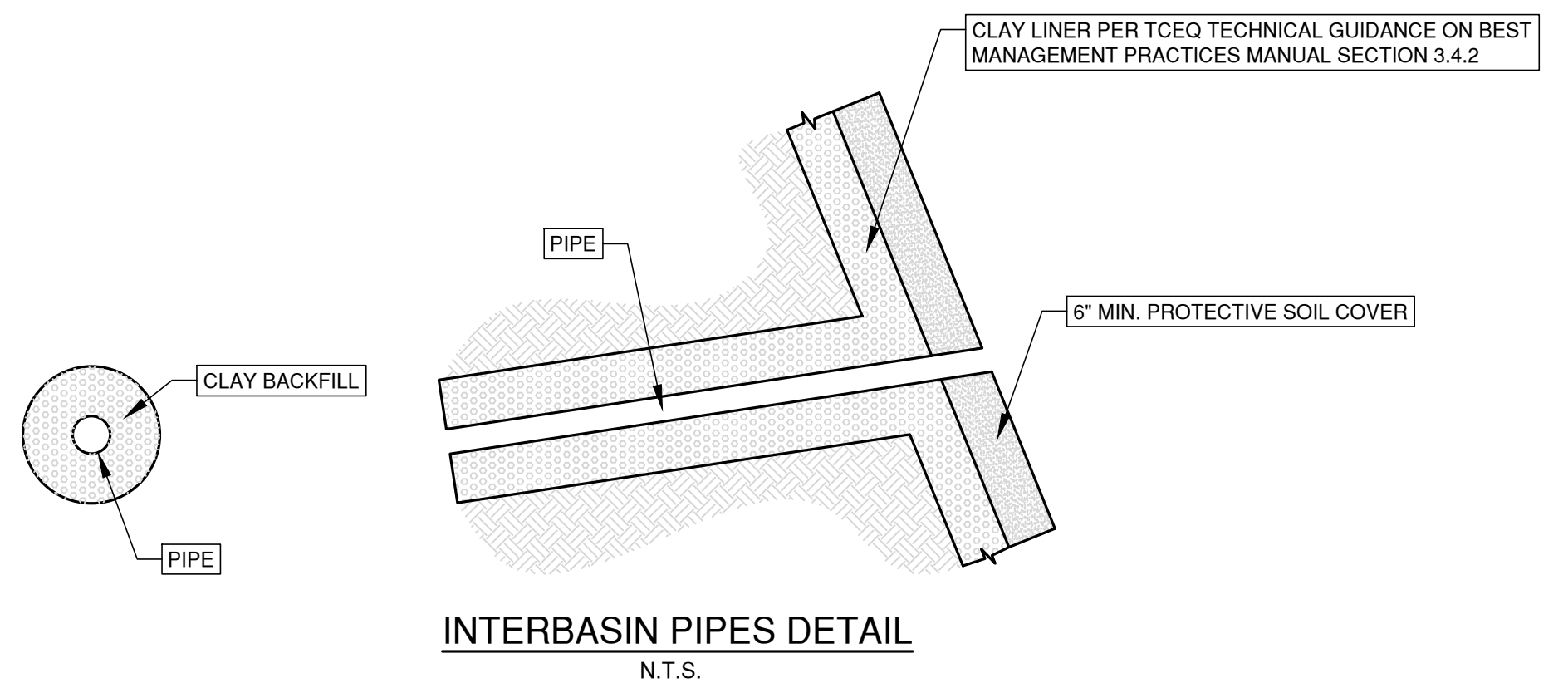
STA. 1+00.00 POND OUTFLOW  
6" PIPE WITH CAP AND 3.5" OPENING  
SLOPED HEADWALL LOW BANK  
PER COA DETAIL 508S-16



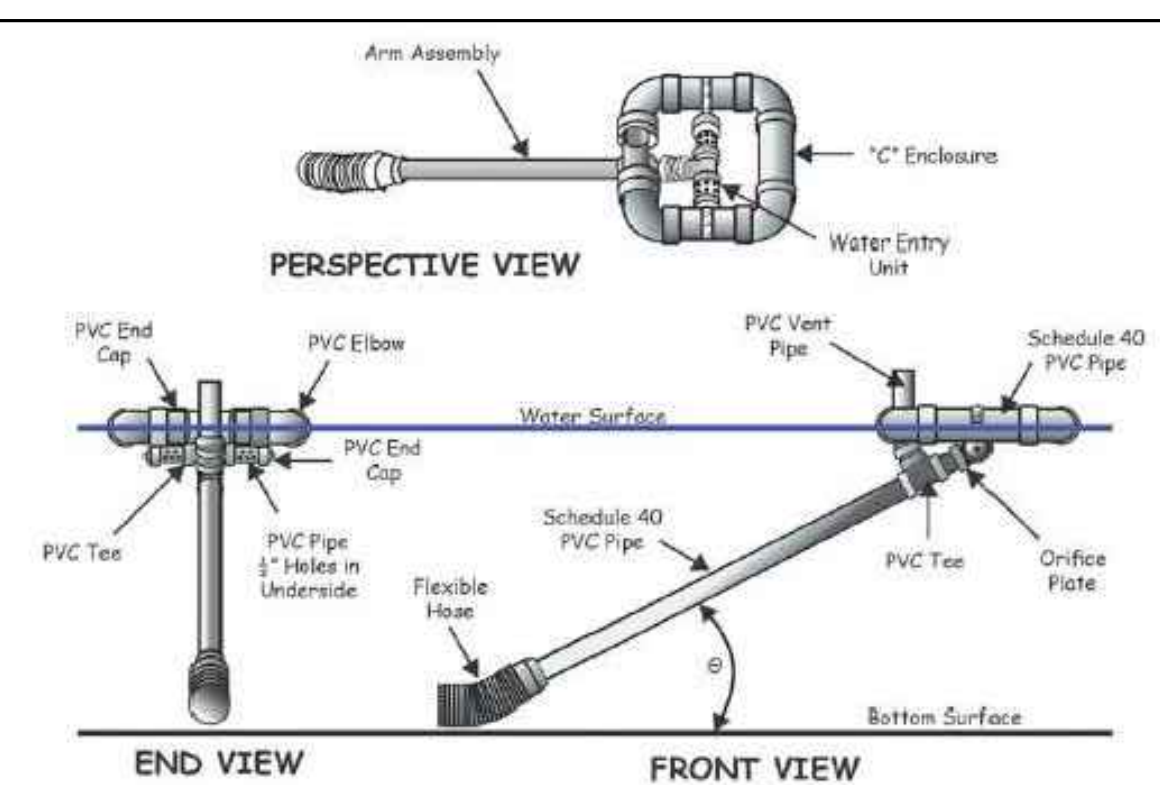
POND OUTFLOW STRUCTURE  
SECTION D-D  
SCALE 1:5

|                                                                                            |                                      |
|--------------------------------------------------------------------------------------------|--------------------------------------|
| 811<br>Know what's below.<br>Call before you dig.                                          |                                      |
| 5508 HIGHWAY 290 WEST<br>SUITE 150<br>MCKINNEY, TX 75065<br>CHAS. GREEN<br>HARGREEN, CON   | TYPE NO: 10384<br>TIFLS NO: 10194101 |
| HARGREEN<br>DEVELOPMENT TX                                                                 |                                      |
| STATE OF TEXAS<br>SHERVIN NOOSHIN<br>96807<br>LICENSED PROFESSIONAL ENGINEER<br>11/03/2023 |                                      |
| POND OUTLET<br>STRUCTURE DETAILS                                                           |                                      |
| PARKSIDE SECTION 8<br>CONSTRUCTION PLANS                                                   |                                      |
| GEORGETOWN, WILLIAMSON, TEXAS                                                              |                                      |
| DESIGNED BY: CC                                                                            |                                      |
| DRAWN BY: TB/MKM                                                                           |                                      |
| CHECKED BY: SN                                                                             |                                      |
| APPROVED BY:                                                                               |                                      |
| SHEET 39 OF 72                                                                             |                                      |
| 2023-XX-CON                                                                                |                                      |



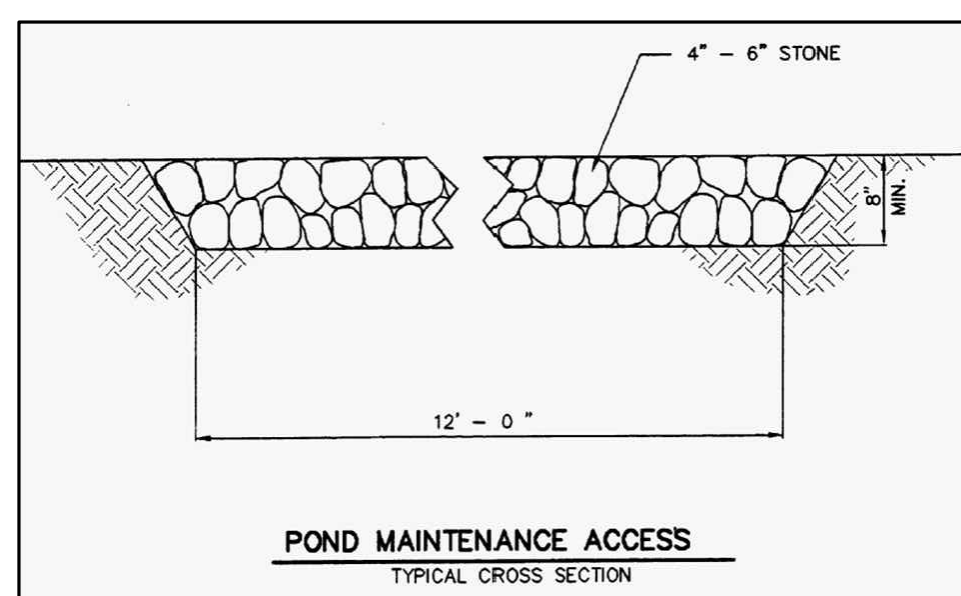
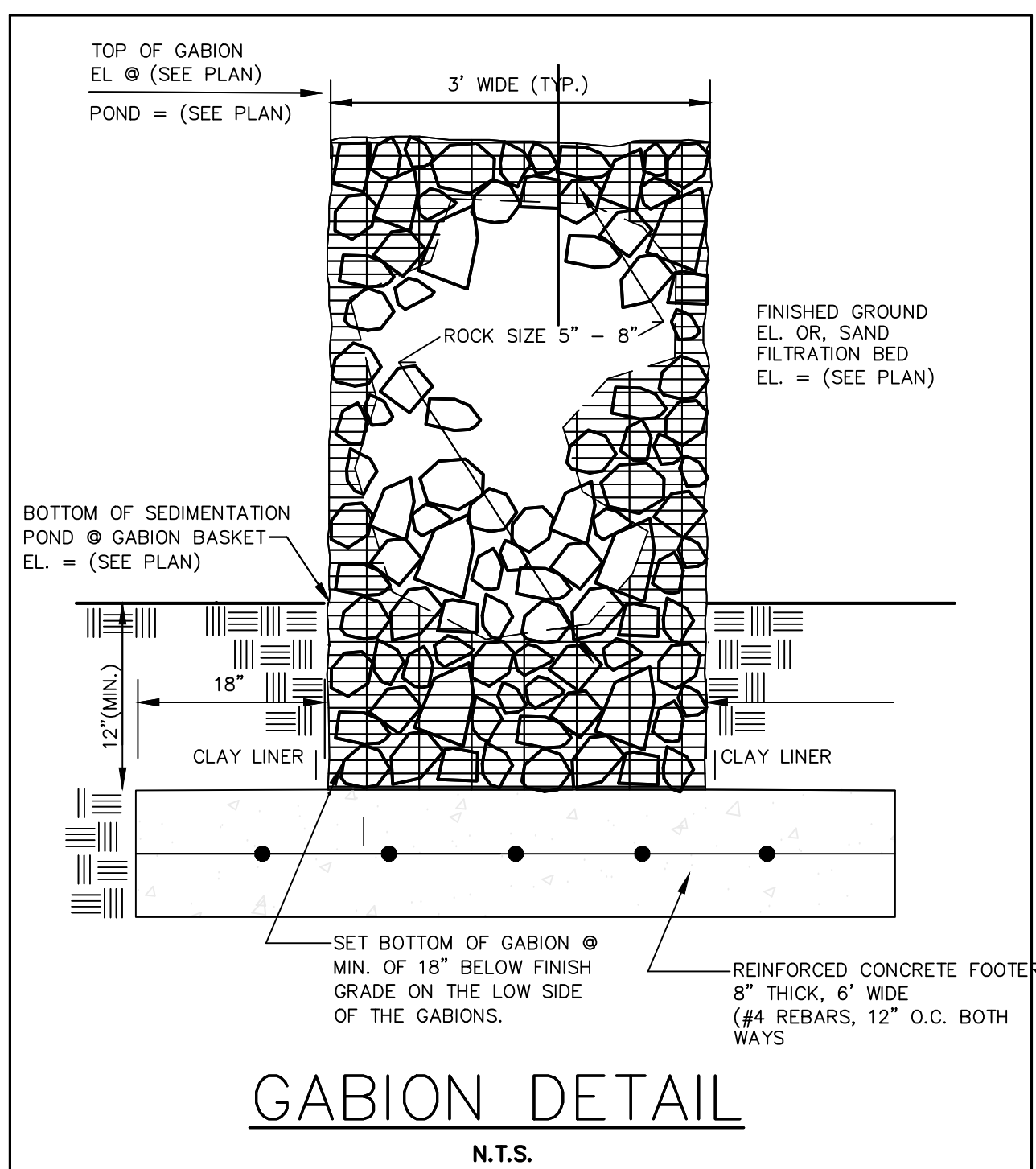


- NOTE:
1. BATCH DETENTION POND SHALL BE IN ACCORDANCE WITH THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ) RG-348 MANUAL (ADDENDUM)
2. THE BATCH DETENTION POND AND RISER PIPE / TRASH RACK WILL FUNCTION AS THE DEWATERING OUTLET AND SHALL BE INSTALLED AND FUNCTIONAL PRIOR TO ANY GENERAL GRADING AND UTILITY WORK.
3. SYSTEM SHALL BE 12 VDC WITH SOLAR CHARGED 12 VDC BATTERY. ALTERNATE ELECTRICAL DESIGN MAY ALSO BE UTILIZED IN LIEU OF SOLAR POWER WITH ELECTRICAL APPROVAL.
4. ACTUATOR SHALL BE ELECTRONIC QUARTER-TURN WITH MANUAL OVERRIDE AND POSITION INDICATOR.
5. ACTUATOR SHALL BE "AVID 12V ACTUATOR, EPI-6" OR EQUIVALENT.
6. ACTUATOR VALVE TO BE SET AT "NORMALLY CLOSED" POSITION.
7. CONTROLLER SHALL BE SET TO OPEN VALVE 12 HOURS AFTER INITIAL RAINFALL DETECTION. VALVE TO REMAIN OPEN UNTIL 2 HRS FOLLOWING BASIN EMPTY SIGNAL.
8. LOGIC CONTROLLER SYSTEM SHALL HAVE TEST SEQUENCE TO DEAL WITH OVERVOLTAGE, OVER CURRENT, OVERTEMPERATURE, OFF/RESET SWITCH AND THE PROGRAMMING SHALL BE FIELD UPLOADABLE.
9. CONTROLLER SHALL BE "MORNINGSTAR SOLAR CONTROLLER, 12V, 20 AMP" OR EQUIVALENT.
10. ALL WIRING SHALL BE INSTALLED IN CONDUIT AND BURIED. CONTACT ENGINEER FOR ADDITIONAL CONTROLLER SCHEMATICS.
11. CONTRACTOR TO INSTALL LIBERTY ALARM MODEL ALM-2W OR EQUIVALENT AT A CONTROLLER PANEL.
12. AT TOTAL ALARM, BELL RING AND CHIME SIGN TO CONTROLLER POLE. REFERENCE ALARM RESPONSE SIGN TO RIGHT.
13. HAZARDOUS MATERIAL THREAT (HMT) OPERATION - THE BASIN'S OUTLET VALVE IS NORMALLY CLOSED AND WILL DETAIN A HAZARDOUS MATERIAL SPILL. HOWEVER, AFTER A SPILL OCCURS, THE MANUAL CONTROLS ON THE CONTROLLER OR THE ACTUATOR/VALVE ARE USED TO PREVENT THE VALVE FROM AUTOMATICALLY OPENING PRIOR TO REMOVAL OF THE HAZARDOUS MATERIAL. ALTHOUGH NOT REQUIRED BY THE EDWARDS RULES, THE HMT OPERATION CALE USED TO COMPLY WITH APPENDIX A OF RG-348, IF A SPILL DOES OCCUR IN THE BASIN, ALL COMPONENTS OF THE CONTROLLER MUST BE INSPECTED AND CHECKED FOR PROPER OPERATION WITHIN 7 DAYS.
14. POWER - THE POND CONTROL SYSTEM CONTROLLER AND ACTUATOR SHALL BE 12 VOLT POWERED OR 120 VOLT SOLAR POWERED WITH BACKUP BATTERY POWER TO RESPOND TO A LOSS OF POWER IN THE MIDDLE OF A CYCLE.
15. PARTS ENCLOSURE & ALARM SYSTEM - THE PARTS ENCLOSURE SHALL BE LOCKABLE. AN ALARM SYSTEM CLEARLY VISIBLE TO INDICATE SYSTEM MALFUNCTION.
16. TEMPERATURE/WEATHER - THE SYSTEM SHALL BE CAPABLE OF OPERATION FROM 0 TO 130 DEGREES FAHRENHEIT AND FROM 10 TO 90% HUMIDITY.
17. RELIABILITY - THE SYSTEM SHALL HAVE A MINIMUM RELIABILITY OF 40,000 HOURS (4.6 YEARS).



**DEWATERING SKIMMER**  
N.T.S.

NOTE: DISCHARGE WATER MUST BE FILTERED USING FILTER BAG OR SOCK. DISCHARGE SHALL ALSO BE DIRECTED TOWARD SILT FENCE FOR ADDITIONAL FILTERING PRIOR TO LEAVING THE SITE.



| Property                 | Test Method        | Unit   | Specification                   |
|--------------------------|--------------------|--------|---------------------------------|
| Permeability             | ASTM D-2434        | cm/sec | $1 \times 10^{-6}$              |
| Plasticity Index of Clay | ASTM D-423 & D-424 | %      | Not less than 15                |
| Liquid Limit of Clay     | ASTM D-2216        | %      | Not less than 30                |
| Clay Particles Passing   | ASTM D-422         | %      | Not less than 30                |
| Clay Compaction          | ASTM D-2216        | %      | 95% of Standard Proctor Density |

### CLAY LINER SPECIFICATIONS PER TCEQ SPECIFICATIONS 3.4.2



CIVIL CONSTRUCTION PLANS  
PARKSIDE ON THE RIVER MUNICIPAL UTILITY DISTRICT NO. 2  
PARKSIDE ON THE RIVER  
SECTIONS 9A & 10A  
GEORGETOWN, WILLIAMSON COUNTY, TEXAS  
2023-xx-CON

OWNER/DEVELOPER: HM PARKSIDE, LP  
1011 NORTH LAMAR BLVD.  
AUSTIN, TX 78703  
(512) 481-0303

ENGINEER/SURVEYOR: HR GREEN DEVELOPMENT TX, LLC  
5508 HIGHWAY 290 WEST, SUITE 150  
AUSTIN, TEXAS 78735  
512.872.6696

WATERSHED STATUS:

THIS SITE IS LOCATED IN THE SOUTH FORK OF THE SAN GABRIEL WATERSHED. THIS SITE IS LOCATED OVER THE EDWARDS AQUIFER RECHARGE ZONE AND CONTRIBUTING ZONE.

FLOODPLAIN INFORMATION:

NO PORTIONS OF THIS SUBDIVISION ARE WITHIN SPECIAL FLOOD HAZARD AREAS INUNDATED BY THE 100 YEAR FLOOD AS IDENTIFIED BY THE U.S. FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD INSURANCE RATE MAP NUMBER 48491C0460F, EFFECTIVE DATE DECEMBER 20, 2019.

LEGAL DESCRIPTION:

34.42 ACRES OF LAND IN THE JOSEPH THOMPSON SURVEY, ABSTRACT NO. 608 AND THE W.E. PATE SURVEY, ABSTRACT NO. 836, WILLIAMSON COUNTY, TEXAS; BEING A PORTION OF A CERTAIN CALLED 1,143.511 ACRE TRACT OF LAND, DESIGNATED AS TRACT 1, AND DESCRIBED IN THE SPECIAL WARRANTY DEED TO HM PARKSIDE, LP OF RECORD IN DOCUMENT NO. 2018114043, OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, AND ALSO BEING A PORTION OF A CERTAIN CALLED 314.00 ACRE TRACT OF LAND DESIGNATED AS TRACT 1 AND DESCRIBED IN THE SPECIAL WARRANTY DEED TO HM GP II, LP OF RECORD IN DOCUMENT NO. 2021027159, OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS

BENCHMARK NOTE:

NAVD 88 (GEOID 12A)

BM(1380)-221:  
COTTON GIN SPINDLE FOUND IN THE SOUTH EDGE OF A CONCRETE SIDEWALK  
ELEVATION = 962.21 FEET.

BM(1380)-700100:  
MAGNAIL WITH WASHER STAMPED HR GREEN SET IN CONCRETE RIM OF WATER MANHOLE  
ELEVATION = 940.16 FEET.

BM(1380)-700200:  
MAGNAIL WITH WASHER STAMPED HR GREENSET IN CONCRETE BASE OF BOLLARD  
ELEVATION = 890.30 FEET.

UTILITY PROVIDERS:

WATER & WASTEWATER: GEORGETOWN UTILITY SYSTEMS  
300-1 INDUSTRIAL AVENUE, GEORGETOWN TX 78626  
(512) 930-3555  
GUS@GEORGETOWN.ORG

ELECTRIC: PEDERNALES ELECTRIC COOPERATIVE  
(877) 372-0391

NO LIABILITY NOTE:

LIMITATION OF LIABILITY – HR GREEN DEVELOPMENT TX, LLC ASSUMES NO LIABILITY FOR ANY DESIGN OR DRAWINGS IN THESE PLANS, THAT ARE NOT SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED WITH THE TEXAS BOARD OF PROFESSIONAL ENGINEERS AS A MEMBER OF THIS FIRM (#F-16384). OTHER CONSULTANTS WORK SHOWN IN THESE PLANS IS THE RESPONSIBILITY OF THE CONSULTANT WHO PREPARED SUCH WORK, AND IS INCLUDED IN THIS PLAN SET FOR REVIEW REQUIREMENTS ONLY.

SITE PLAN COMPONENTS – ALL BUILDING AND STRUCTURAL IMPROVEMENTS SHOWN HEREON ARE SHOWN FOR CONCEPTUAL PURPOSES ONLY. HR GREEN DEVELOPMENT TX, LLC IS NOT RESPONSIBLE OR LIABLE FOR THE DESIGN OF BUILDING OR STRUCTURAL IMPROVEMENTS BY OTHERS.

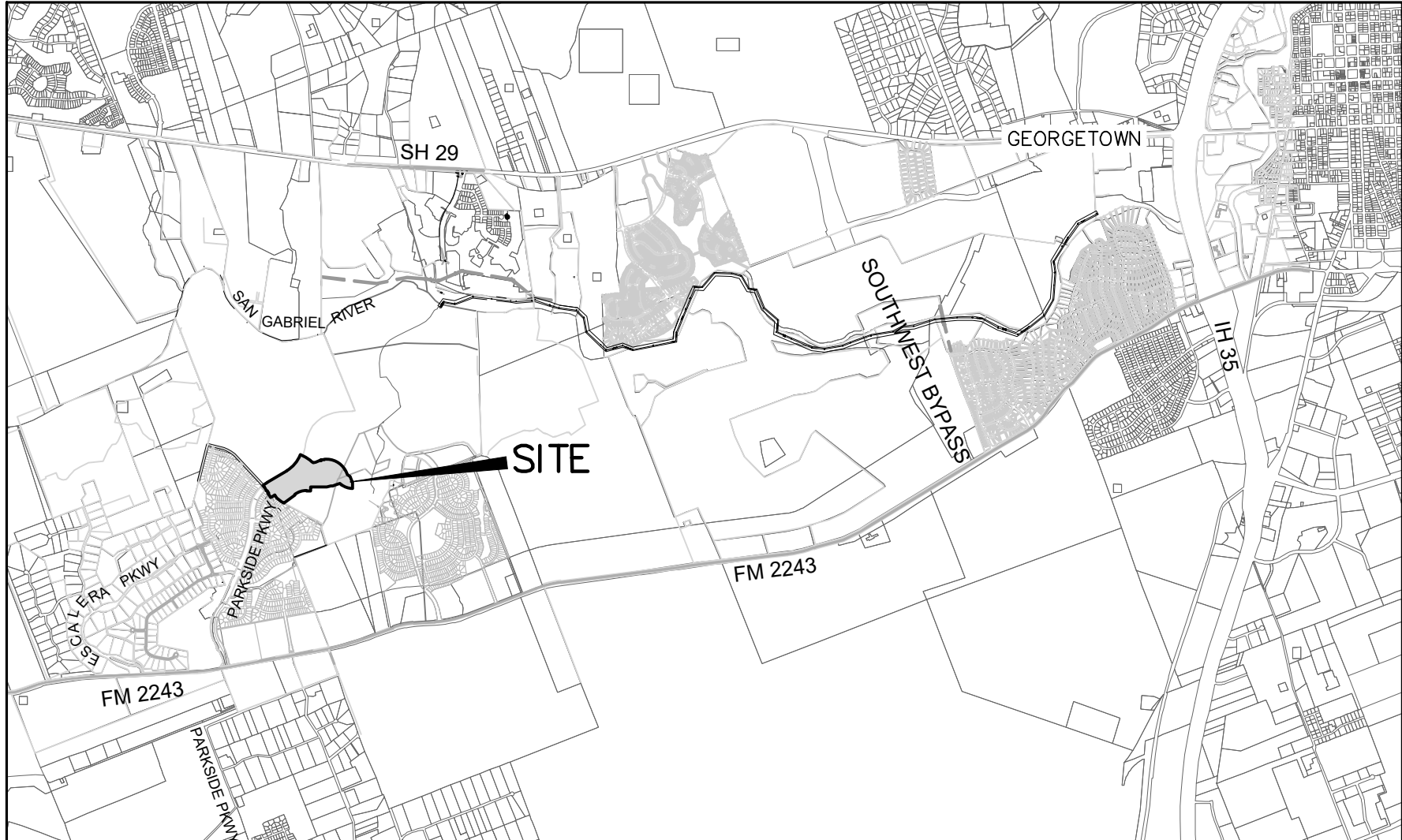
STRUCTURAL COMPONENTS - ALL STRUCTURAL DESIGN IS THE RESPONSIBILITY OF THE OWNER'S STRUCTURAL ENGINEER. STRUCTURAL DESIGN SHOWN HEREON IS THE DESIGN OF THE OWNER'S STRUCTURAL ENGINEER.

PAVEMENT DESIGN – PAVEMENT DESIGN SHOWN HEREON IS THE DESIGN OF THE OWNER'S GEOTECHNICAL CONSULTANT. HR GREEN DEVELOPMENT TX, LLC MAKES NO WARRANTY OR GUARANTEE AS TO ITS SUITABILITY, AND ASSUMES NO LIABILITY THEREFOR.

NOTES:

- THESE PLANS WERE PREPARED, SEALED, SIGNED AND DATED BY A TEXAS LICENSED PROFESSIONAL ENGINEER. THEREFORE BASED ON THE ENGINEER'S CONCURRENCE OF COMPLIANCE, THE PLANS FOR CONSTRUCTION OF THE PROPOSED PROJECT ARE HEREBY APPROVED SUBJECT TO THE STANDARD CONSTRUCTION SPECIFICATIONS AND DETAILS MANUAL AND ALL OTHER APPLICABLE CITY, STATE AND FEDERAL REQUIREMENTS AND CODES.
- THIS PROJECT IS SUBJECT TO ALL CITY STANDARD SPECIFICATIONS AND DETAILS IN EFFECT AT THE TIME OF SUBMITTAL OF THE PROJECT TO THE CITY.
- THE PROPERTY SUBJECT TO THIS APPLICATION IS SUBJECT TO THE WATER QUALITY REGULATIONS OF THE CITY OF GEORGETOWN.
- A GEOLOGIC ASSESSMENT, IN ACCORDANCE WITH THE CITY OF GEORGETOWN WATER QUALITY REGULATIONS, WAS COMPLETED ON OCTOBER 18, 2023). ANY SPRINGS AND STREAMS AS IDENTIFIED IN THE GEOLOGIC ASSESSMENT ARE SHOWN HEREIN.
- THIS PROJECT IS SUBJECT TO THE REQUIREMENTS OF PARKSIDE ON THE RIVER DEVELOPMENT AGREEMENT (ORDINANCE NO. 2019-69).

INITIAL SUBMITTAL DATE: 11/15/2023



VICINITY MAP

SCALE: 1"=4000'

I, SHERVIN NOOSHIN, P.E., CERTIFY THAT THESE ENGINEERING DOCUMENTS ARE COMPLETE, ACCURATE AND ADEQUATE FOR THE INTENDED PURPOSES, INCLUDING CONSTRUCTION, BUT ARE NOT AUTHORIZED FOR CONSTRUCTION PRIOR TO FORMAL CITY APPROVAL.

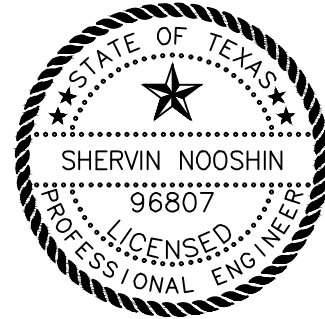
SUBMITTED BY :

*Sherwin*

11/15/2023

SHERVIN NOOSHIN, P.E.  
HR GREEN DEVELOPMENT TX, LLC  
5508 HIGHWAY 290 WEST, SUITE 150  
AUSTIN, TEXAS 78735  
512.872.6696

DATE



REVIEWED FOR COMPLIANCE WITH

PARKSIDE ON THE RIVER M.U.D. NO. 2

DATE

Sheet Index

| Sheet Number | Sheet Title                                          |
|--------------|------------------------------------------------------|
| 1            | COVER SHEET                                          |
| 2            | GENERAL NOTES                                        |
| 3            | TCEQ NOTES                                           |
| 4            | PRELIMINARY PLAT (1 OF 2)                            |
| 5            | PRELIMINARY PLAT (2 OF 2)                            |
| 6            | EXISTING CONDITIONS PLAN                             |
| 7            | PROPOSED CONDITIONS PLAN                             |
| 8            | TREE LIST                                            |
| 9            | EROSION & SEDIMENTATION CONTROL PLAN A               |
| 10           | EROSION & SEDIMENTATION CONTROL PLAN B               |
| 11           | EROSION & SEDIMENTATION CONTROL PLAN C               |
| 12           | EROSION & SEDIMENTATION CONTROL PLAN D               |
| 13           | EROSION & SEDIMENTATION CONTROL DETAILS              |
| 14           | SIGNAGE STRIPING & LIGHTING PLAN A                   |
| 15           | SIGNAGE STRIPING & LIGHTING PLAN B                   |
| 16           | SIGNAGE STRIPING & LIGHTING PLAN C                   |
| 17           | PARKSIDE PARKWAY INBOUND PLAN & PROFILE 1+00 - 8+00  |
| 18           | PARKSIDE PARKWAY INBOUND PLAN & PROFILE 8+00 - END   |
| 19           | PARKSIDE PARKWAY OUTBOUND PLAN & PROFILE 1+00 - 8+25 |
| 20           | PARKSIDE PARKWAY OUTBOUND PLAN & PROFILE 8+25 - END  |
| 21           | PEACEFUL SERENITY DRIVE PLAN & PROFILE 1+00 - END    |
| 22           | PANSY TRAIL PLAN & PROFILE 1+00 - 8+00               |
| 23           | PANSY TRAIL PLAN & PROFILE 8+00 - 15+50              |
| 24           | PANSY TRAIL PLAN & PROFILE 15+50 - END               |
| 25           | BEAUTIFUL WISDOM COURT PLAN & PROFILE 1+00 - END     |
| 26           | ANGELS JOY COVE PLAN & PROFILE 1+00 - END            |
| 27           | TWISTED TARPLEY LANE PLAN & PROFILE 1+00 - 10+00     |
| 28           | TWISTED TARPLEY LANE PLAN & PROFILE 10+00 - END      |
| 29           | GLORIOUS GARDEN WAY PLAN & PROFILE 1+00 - END        |
| 30           | MIGHTY COUNSELOR LANE PLAN & PROFILE 9+43.83 - END   |
| 31           | PAVING & GRADING PLAN A                              |
| 32           | PAVING & GRADING PLAN B                              |
| 33           | PAVING & GRADING PLAN C                              |
| 34           | INTERSECTION DETAILS SHEET 1                         |
| 35           | INTERSECTION DETAILS SHEET 2                         |
| 36           | PAVING DETAILS SHEET 1 OF 2                          |
| 37           | PAVING DETAILS SHEET 2 OF 2                          |
| 38           | EXISTING DRAINAGE AREA MAP                           |
| 39           | PROPOSED DRAINAGE AREA MAP                           |
| 40           | INLET DRAINAGE AREA MAP                              |
| 41           | INLET DRAINAGE CALCULATIONS                          |
| 42           | WATER QUALITY DRAINAGE AREA MAP (PROPOSED)           |
| 43           | WATER QUALITY CALCULATIONS (PROPOSED)                |
| 44           | OVERALL STORM SEWER PLAN A                           |
| 45           | OVERALL STORM SEWER PLAN B                           |
| 46           | OVERALL STORM SEWER PLAN C                           |
| 47           | STORM A-1 PLAN & PROFILE 1+00 - 10+25                |
| 48           | STORM A-1 PLAN & PROFILE 10+25 - END                 |
| 49           | STORM A-1 LATERALS SHEET 1                           |
| 50           | STORM A-1 LATERALS SHEET 2                           |
| 51           | STORM A-2 PLAN & PROFILE 1+00 - END                  |
| 52           | STORM A-3 PLAN & PROFILE 1+00 - END                  |
| 53           | STORM A-4 PLAN & PROFILE 1+00 - END                  |
| 54           | STORM A-5 PLAN & PROFILE 1+00 - END                  |
| 55           | STORM B-1 PLAN & PROFILE 1+00 - 10+50                |
| 56           | STORM B-1 PLAN & PROFILE 10+50 - END                 |
| 57           | STORM B-1 LATERALS 1                                 |
| 58           | STORM B-1 LATERALS 2                                 |
| 59           | STORM B-2 PLAN & PROFILE 1+00 - END                  |
| 60           | STORM C-1 & D-1 PLAN & PROFILE                       |
| 61           | DRAINAGE DETAILS                                     |
| 62           | OVERALL WASTEWATER PLAN A                            |
| 63           | OVERALL WASTEWATER PLAN B                            |
| 64           | OVERALL WASTEWATER PLAN C                            |
| 65           | OVERALL WASTEWATER PLAN D                            |
| 66           | WWL A PLAN & PROFILE 1+00 - 8+50                     |
| 67           | WWL A PLAN & PROFILE 8+50 - 16+00                    |
| 68           | WWL A PLAN & PROFILE 16+00 - 24+00                   |
| 69           | WWL A PLAN & PROFILE 24+00 - END                     |
| 70           | WWL B PLAN & PROFILE 1+00 - 8+00                     |
| 71           | WWL B PLAN & PROFILE 8+00 - END                      |
| 72           | WWL C PLAN & PROFILE 1+00 - END                      |
| 73           | WWL D PLAN & PROFILE 1+00 - END                      |
| 74           | WWL E PLAN & PROFILE 1+00 - END                      |
| 75           | WWL F PLAN & PROFILE 1+00 - END                      |
| 76           | WASTEWATER DETAILS SHT 1 OF 2                        |
| 77           | WASTEWATER DETAILS SHT 2 OF 2                        |
| 78           | OVERALL WATER PLAN A                                 |
| 79           | OVERALL WATER PLAN B                                 |
| 80           | OVERALL WATER PLAN C                                 |
| 81           | WL F PLAN & PROFILE 1+00 - 10+00                     |
| 82           | WL F PLAN & PROFILE 10+00 - END                      |
| 83           | WATER DETAILS SHT 1 OF 2                             |
| 84           | WATER DETAILS SHT 2 OF 2                             |

COVER SHEET

PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC

DRAWN BY: MM/MKM

CHECKED BY: SN

APPROVED BY:

SHEET 1 OF 84

2023-XX-CON



1. ALL RESPONSIBILITY FOR THE ADEQUACY OF THESE PLANS REMAINS WITH THE ENGINEER WHO PREPARED THEM. IN REVIEWING THESE PLANS, THE CITY OF GEORGETOWN MUST RELY ON THE ADEQUACY OF THE WORK OF THE DESIGN ENGINEER.
2. CONTRACTOR SHALL NOTIFY GEORGETOWN UTILITIES AT 512-930-3555 AT LEAST 24 HOURS PRIOR TO THE INSTALLATION OF ANY DRAINAGE FACILITY WITHIN A DRAINAGE EASEMENT OR STREET R.O.W. THE METHOD OF PLACEMENT AND COMPACTION OF BACKFILL IN THE CITY'S R.O.W. MUST BE APPROVED PRIOR TO THE START OF BACKFILL OPERATIONS.
3. FOR SLOPES OR TRENCHES GREATER THAN FIVE (5) FEET IN DEPTH, A NOTE MUST BE ADDED STATING THAT CONSTRUCTION OPERATIONS SHALL BE ACCOMPLISHED IN ACCORDANCE WITH APPLICABLE REGULATIONS OF THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION. COPIES OF OSHA STANDARDS MAY BE PURCHASED FROM THE U.S. DEPARTMENT OF LABOR, PRINTING OFFICE, INFORMATION AND RELATED REFERENCE MATERIALS MAY BE PURCHASED FROM OSHA, 611 E. 6TH STREET, AUSTIN, TEXAS.
4. ALL SITE WORK MUST ALSO COMPLY WITH ENVIRONMENTAL REQUIREMENTS.
5. CONTRACTOR INFORMATION

HM PARKSIDE DEVELOPMENT INC. PHONE# 512-481-0303

6. TOPOGRAPHIC DATA SHOWN HEREON BASED ON GROUND TOPO SURVEY BY HR GREEN ON OCTOBER 2023.
7. IF CONTRACTOR FINDS A DISCREPANCY WITH THE TOPOGRAPHIC INFORMATION ON THESE PLANS, HE/SHE SHOULD CONTACT THE ENGINEER/SURVEYOR IMMEDIATELY.
8. ALL AREAS DISTURBED BY CONSTRUCTION SHALL BE RESTORED AND GRADED TO DRAIN.
9. ANY TEMPORARY SPOILS STOCKPILE MUST BE LOCATED OUTSIDE OF ANY TREE DRIPLINES AND IN THE TEMPORARY SPOILS AREA DESIGNATED ON THE APPROVED PLANS. ALL SURPLUS MATERIAL WILL BE DISPOSED OF OFFSITE.
10. ALL DEBRIS AND EXCESS MATERIAL SHALL BE REMOVED FROM THE SITE IN A MANNER NOT TO DAMAGE THE OWNER'S PROPERTY PRIOR TO ACCEPTANCE OF THE PROJECT.
11. IF CONTRACTOR ENCOUNTERS A VOID ON THE PROJECT, CONTRACTOR IS TO CONTACT ENGINEER AT (512) 872-6896 OR CRAIG CRAWFORD AT CAMBRIAN ENVIRONMENTAL AT (512) 705-5541 FOR EVALUATION OF THE FEATURE. ONCE CAMBRIAN ENVIRONMENTAL HAS VERIFIED THAT THE FEATURE IS NOT AN ENDANGERED SPECIES HABITAT, CONTRACTOR MAY PROCEED AS DIRECTED BY THE DETAILS ON THESE PLANS.
12. ALL WATER CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE CITY OF GEORGETOWN CONSTRUCTION SPECIFICATION (MOST CURRENT EDITION).

1. IN ACCORDANCE WITH THIRAWS OF THE STATE OF TEXAS AND THE U. S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, ALL TRENCHES OVER 5 FEET IN DEPTH IN EITHER HARD AND COMPACT OR SOFT AND UNCONSOLIDATED GROUND SHALL BE PROTECTED BY SHIELDING OR SHIELDING. ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT WILL BE PROVIDED BY THE CONTRACTOR.
2. IN ACCORDANCE WITH THE U. S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4-FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED SO AS TO REQUIRE NO MORE THAN 25 FEET OF LATERNAL TRAVEL.
3. ALL TRENCH SHIELDING SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE SPECIFICATIONS OF A PROFESSIONAL ENGINEER, ARE RETAINED AND COPIES SUBMITTED TO THE CITY OF GEORGETOWN.

1. INSTALL TREE PROTECTION AND INITIATE TREE MITIGATION MEASURES.
2. CONSTRUCT EROSION CONTROL MEASURES AS INDICATED ON APPROVED PLANS.
3. CONTACT CITY OF GEORGETOWN AND WILMINGTON COUNTY TO SCHEDULE PRE-CONSTRUCTION COORDINATION MEETING
4. CONSTRUCT TEMPORARY EROSION CONTROL INSTALLATION. REVIEW CONSTRUCTION SCHEDULE WITH THE EROSION CONTROL PLAN.
5. BEGIN SITE CLEARING AND GRADING, INSPECT AND MAINTAIN ALL CONTROLS AS PER GENERAL NOTES.
6. CONSTRUCT UTILITY LINES (E. WATER, WASTEWATER, STORM DRAINAGE & PONDS.
7. CONSTRUCT SIDEWALK RAMPS.
8. CONSTRUCT PAVING/STREETS.
9. REVEGETATE OR COMPLETE A DEVELOPERS CONTRACT FOR THE REVEGETATION ALONG WITH THE ENGINEERS CONCURRENCE LETTER.
10. PROJECT ENGINEER INSPECTS JOB AND WRITES CONCURRENCE LETTER TO THE CITY. FINAL INSPECTION IS SCHEDULED UPON RECEIPT OF LETTER.
11. REMOVE TEMPORARY EROSION/SEDIMENTATION CONTROLS AT GRASS GROWTH.

1. THESE CONSTRUCTION PLANS WERE PREPARED, SEALED, SIGNED, AND DATED BY A TEXAS LICENSED PROFESSIONAL ENGINEER. THEREFORE BASED ON THE ENGINEER'S CONCURRENCE OF COMPLIANCE, THE CONSTRUCTION PLANS FOR CONSTRUCTION OF THE PROPOSED PROJECT ARE HEREBY SUBMITTED SUBJECT TO THE STANDARD CONSTRUCTION SPECIFICATIONS AND DETAILS MANUAL, AND ALL OTHER APPLICABLE CITY, STATE, AND FEDERAL REQUIREMENTS AND CODES.
2. THIS PROJECT IS SUBJECT TO ALL CITY STANDARD SPECIFICATIONS AND DETAILS IN EFFECT AT THE TIME OF SUBMITTAL OF THE PROJECT TO THE CITY.
3. THE SITE CONSTRUCTION PLANS SHALL MEET ALL REQUIREMENTS OF THE APPROVED SITE PLAN.
4. WASTEWATER MAINS AND SERVICE LINES SHALL BE SDR 26 PVC.
5. WASTEWATER MAINS SHALL BE INSTALLED WITHOUT HORIZONTAL OR VERTICAL BENDS.
6. MAXIMUM DISTANCE BETWEEN WASTEWATER MANHOLES IS 500 FEET.
7. WASTEWATER MAINS SHALL BE LOW PRESSURE AIR TESTED AND MANDREL TESTED BY THE CONTRACTOR ACCORDING TO CITY OF GEORGETOWN AND TCEQ REQUIREMENTS.
8. WASTEWATER MANHOLES SHALL BE VACUUM TESTED AND COATED BY THE CONTRACTOR ACCORDING TO CITY OF GEORGETOWN AND TCEQ REQUIREMENTS.
9. WASTEWATER MAINS SHALL BE CAMERA TESTED BY THE CONTRACTOR AND SUBMITTED TO THE CITY ON DVD FORMAT PRIOR TO PAVING THE STREETS.
10. PRIVATE WATER SYSTEM FIRE LINES SHALL BE TESTED BY THE CONTRACTOR TO 200 PSI FOR 2 HOURS.
11. PRIVATE WATER SYSTEM FIRE LINES SHALL BE DUCTILE IRON PIPING FROM THE WATER MAIN TO THE BUILDING SPRINKLER SYSTEM, AND 200 PSI C900 FOR ALL OTHERS.
12. PUBLIC WATER SYSTEM MAINS SHALL BE 150 PSI C900 PVC AND TESTED BY THE CONTRACTOR AT 150 PSI FOR 4 HOURS.
13. ALL BEND AND CHANGES IN DIRECTION ON WATER MAINS SHALL BE RESTRAINED AND THRUST BLOCKED.
14. LONG FIRE HYDRANT LEADS SHALL BE RESTRAINED.
15. ALL WATER LINES ARE TO BE BACTERIA TESTED BY THE CONTRACTOR ACCORDING TO THE CITY STANDARDS AND SPECIFICATIONS.
16. WATER AND SEWER MAIN CROSSINGS SHALL MEET ALL REQUIREMENTS OF THE TCEQ AND THE CITY.
17. FLEXIBLE BASE MATERIAL FOR PUBLIC STREETS SHALL BE TYPOT TYPE A GRADE 1.
18. HOT MIX ASPHALT CONCRETE PAVEMENT SHALL BE TYPE D UNLESS OTHERWISE SPECIFIED AND SHALL BE A MINIMUM OF 2 INCHES THICK ON PUBLIC STREETS AND ROADWAYS.
19. ALL SIDEWALK RAMPS ARE TO BE INSTALLED WITH THE PUBLIC INFRASTRUCTURE.
20. A MAINTENANCE BOND IS REQUIRED TO BE SUBMITTED TO THE CITY PRIOR TO ACCEPTANCE OF HTE PUBLIC IMPROVEMENTS. THIS BOND SHALL BE ESTABLISHED FOR 2 YEAR IN THE AMOUNT OF 10% OF THE COST OF THE PUBLIC IMPROVEMENTS AND SHALL FOLLOW THE CITY FORMAT.
21. RECORD DRAWINGS OF THE PUBLIC IMPROVEMENTS SHALL BE SUBMITTED TO THE CITY BY THE DESIGN ENGINEER PRIOR TO THE PROJECT BEING COMPLETED. THESE DRAWINGS SHALL BE SUBMITTED AS A PDF TO THE CITY ASH DOWNS SOURCE.

1. PIPE MATERIAL FOR WATER MAINS SHALL BE PVC (AWWA C-900, MIN. CLASS 200), OR DUCTILE IRON (C-115, MIN. CLASS 200) UNLESS SPECIFIED OTHERWISE.
2. PIPE MATERIAL FOR GRAVITY WASTEWATER MAINS SHALL BE PVC (ASTM D3034, SDR-26) UNLESS SPECIFIED OTHERWISE.
3. THE CONTRACTOR SHALL CONTACT THE CITY INSPECTOR TO COORDINATE UTILITY TIE-INS AND NOTIFY HIM AT LEAST 48 HOURS PRIOR TO CONNECTING TO EXISTING LINES.
4. ALL MANHOLES SHALL HAVE ECCENTRIC CONES AND SHALL BE CONCRETE WITH CAST IRON RING AND COVER. ALL MANHOLES LOCATED OUTSIDE OF THE PAVEMENT SHALL HAVE BOLTED COVERS. TAPPING OF FIBERGLASS MANHOLES SHALL NOT BE ALLOWED.
5. THE CONTRACTOR MUST OBTAIN A BULK WATER PERMIT OR PURCHASE AND INSTALL A WATER METER FOR ALL WATER USED DURING CONSTRUCTION. A COPY OF THIS PERMIT MUST BE CARRIED AT ALL TIMES BY ALL WHO USE WATER. CONTRACTOR TO INSTALL ABOVE GROUND WATER TANK WITH SUPPLY LINE AS INDICATED ON PLANS.
6. LINE FLUSHING OR ANY ACTIVITY USING A LARGE QUANTITY OF WATER MUST BE SCHEDULED WITH THE CITY INSPECTOR.
7. THE CONTRACTOR, AT HIS EXPENSE, SHALL PERFORM QUALITY TESTING FOR ALL WASTEWATER PIPE INSTALLED AND SHALL PROVIDE ALL EQUIPMENT (INCLUDING PUMPS AND GAUGES), SUPPLIES AND LABOR NECESSARY TO PERFORM THE TESTS. QUALITY AND PRESSURE TESTING SHALL BE MONITORED BY CITY OF GEORGETOWN PERSONNEL.
8. THE CONTRACTOR SHALL COORDINATE TESTING WITH THE CITY OF INSPECTOR AND PROVIDE NO LESS THAN 24 HOURS NOTICE PRIOR TO PERFORMING STERILIZATION, QUALITY TESTING OR PRESSURE TESTING.
9. THE CONTRACTOR SHALL NOT OPEN OR CLOSE ANY VALVES UNLESS AUTHORIZED BY THE CITY OF GEORGETOWN.
10. ALL VALVE BOXES AND COVERS SHALL BE CAST IRON.
11. TOOLS FOR MARKING THE CURB SHALL BE PROVIDED BY THE CONTRACTOR. OTHER APPROPRIATE MEANS OF MARKING SERVICE AND UTILITY LOCATIONS SHALL BE PROVIDED IN AREAS WITHOUT CURBS. SUCH MEANS OF MARKING SHALL BE AS SPECIFIED BY THE ENGINEER AND ACCEPTED BY THE CITY OF GEORGETOWN.
12. CONTACT CITY OF GEORGETOWN INSPECTION DEPARTMENT FOR ASSISTANCE IN OBTAINING EXISTING WATER AND WASTEWATER LOCATIONS.
13. SAND, AS DESCRIBED IN SPECIFICATION ITEM 510 PIPE, SHALL NOT BE USED AS BEDDING FOR WATER AND WASTEWATER LINES. ACCEPTABLE BEDDING MATERIALS ARE PIPE BEDDING STONE, PEA GRAVEL AND IN LIEU OF SAND NATURALLY OCCURRING OR MANUFACTURED STONE MATERIAL CONFORMING TO ASTM C33 FOR STONE QUALITY AND MEETING THE FOLLOWING GRADATION SPECIFICATION:

| <u>SIEVE SIZE</u> | <u>PERCENT RETAINED BY WEIGHT</u> |
|-------------------|-----------------------------------|
| 1/2"              | 0                                 |
| 3/8"              | 0-2                               |
| #4                | 40-85                             |
| #10               | 95-100                            |

15. THE CONTRACTOR IS HEREBY NOTIFIED THAT CONNECTING TO, SHUTTING DOWN, OR TERMINATING EXISTING UTILITY LINES MAY HAVE TO OCCUR AT OFF-PEAK HOURS. SUCH HOURS ARE USUALLY OUTSIDE NORMAL WORKING HOURS AND POSSIBLY BETWEEN 12 A.M. AND 6 A.M.
16. ALL WASTEWATER CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ) REGULATIONS, 30 TAC CHAPTER 313 AND 317, AS APPLICABLE. WHENEVER TCEQ AND CITY OF GEORGETOWN SPECIFICATIONS CONFLICT, THE MORE STRINGENT SHALL APPLY.
17. THE CONTRACTOR SHALL EXCAVATE THE "DIG TEST" SYSTEM AT 1-800-344-8377 FOR EXISTING UTILITY LOCATIONS PRIOR TO ANY CONSTRUCTION OR RELOCATION. THE CONTRACTOR SHALL VERIFY THE LOCATIONS OF ALL UTILITIES TO BE EXTENDED, TIED TO, OR ALTERED, OR SUBJECT TO DAMAGE/INCONVENIENCE BY THE CONSTRUCTION OPERATIONS. THE CITY OF GEORGETOWN WATER AND WASTEWATER MAINTENANCE RESPONSIBILITY ENDS AT R.O.W./EASEMENT LINES.
18. ALL MANHOLES IN UNPAVED AREAS PROVIDING DIRECT ACCESS TO A WASTEWATER LINE SHALL BE WATER TIGHT AND BEAR THE WORDING AND INSIGNIA FOR THE CITY OF GEORGETOWN.
19. THE OWNER IS RESPONSIBLE FOR ALL COST OF RELOCATION OR DAMAGE TO UTILITIES.
20. THE CONTRACTOR IS RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH OCCUR DUE TO HIS/HER FAILURE TO LOCATE AND PRESERVE ANY AND ALL UTILITIES.
21. THE ENGINEER, IN PREPARING THESE PLANS HAS ATTEMPTED TO LOCATE ALL EXISTING UTILITIES IN THE AREAS OF EXISTING OR NEW CONSTRUCTION. HOWEVER, THERE MAY BE UTILITIES THAT COULD NOT BE OR WERE NOT LOCATED. UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE SHOWN IN APPROXIMATE LOCATIONS ONLY. CONTRACTOR SHALL DETERMINE THE EXACT LOCATIONS AND ELEVATIONS OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. THE CONTRACTOR SHALL CALL APPROPRIATE UTILITY COMPANIES FOR LOCATIONS OF THEIR UTILITIES AT LEAST 48 HOURS BEFORE COMMENCING EXCAVATION. IN THE EVENT THAT A UTILITY IS SITUATED SUCH THAT CONSTRUCTION CANNOT PROCEED AS SHOWN ON THE PLANS, THE CONSTRUCTION MANAGER/SUPERVISOR SHALL BE NOTIFIED IMMEDIATELY.
22. CONTRACTOR TO COORDINATE WITH APPROPRIATE UTILITY COMPANIES PRIOR TO CONSTRUCTION, ADJUSTMENT, OR RELOCATION OF EXISTING UTILITIES AS DESIGNATED ON PLANS.
23. THE MINIMUM HORIZONTAL SEPARATION BETWEEN WATER AND ASSOCIATED VALVING AND SEWER LINES AND ASSOCIATED MANHOLES, IS NINE (9) FEET OUTSIDE DIAMETER TO OUTSIDE DIAMETER. THE MINIMUM VERTICAL SEPARATION BETWEEN WATER AND SEWER LINES IS EIGHTEEN (18) INCHES.
24. THE TOP ELEVATION OF MANHOLES IN PAVED AREAS SHALL MATCH FINISH GRADE. THE TOP ELEVATION OF MANHOLES IN UNPAVED AREAS SHALL BE 3" (MIN.) ABOVE FINISH GRADE, UNLESS OTHERWISE NOTED ON PLANS.
25. CONTRACTOR SHALL COORDINATE INSPECTION OF UTILITY LINES WITH APPROPRIATE AUTHORITIES PRIOR TO BACKFILLING TRENCHES.
26. ALL WATER AND WASTEWATER LINES IN CITY R.O.W. AND EASEMENTS WILL MEET THE CITY OF GEORGETOWN WATER AND WASTEWATER DEPARTMENT DESIGN CRITERIA, AT A MINIMUM.
27. CITY MAINTENANCE OF UTILITIES ENDS AT THE PROPERTY LINE UNLESS IN AN EASEMENT.
28. EXTEND ALL EXISTING UTILITY MANHOLES, BOXES, COVERS, ETC. TO PROPOSED FINISH GRADE, UNLESS APPROVED OTHERWISE.
29. ALL UNDERGROUND UTILITY CONSTRUCTION WITHIN CITY R.O.W. OR PUBLIC EASEMENTS MUST BE ACCOMPLISHED IN ACCORDANCE WITH THE CITY OF GEORGETOWN STANDARD SPECIFICATIONS.
30. AN 80 MIL COAT OF RAYEN LINING SYSTEMS, RAYEN 405 ULTRA HIGH BUILD EPOXY COATING, OR APPROVED EQUAL, TO BE APPLIED TO ENTIRE INTERIOR OF EACH WASTEWATER MANHOLE AND UNDERSIDE OF FLAT TOPS.
31. ALL WATER SERVICE, WASTEWATER SERVICE AND VALVE LOCATIONS SHALL BE APPROPRIATELY MARKED AS FOLLOWS:

|                    |                      |
|--------------------|----------------------|
| WATER SERVICE      | "W" ON TOP OF CURB   |
| WASTEWATER SERVICE | "S" ON TOP OF CURB   |
| VALVE              | "V" ON FACE OF CURB  |
| DRY UTILITIES      | "DU" ON FACE OF CURB |
32. CENTER ONE 20-FOOT 150 PSI PRESSURE RATED WASTEWATER PIPE SECTION AT ALL WATERLINE CROSSINGS.
33. WHERE WATER LINES AND NEW SEWER LINE ARE INSTALLED WITH A SEPARATION DISTANCE CLOSER THAN NINE FEET (I.E., WATER LINES CROSSING WASTEWATER LINES, WATER LINES PARALLELING WASTEWATER LINES, OR WATER LINES NEXT TO MANHOLES) THE INSTALLATION MUST MEET THE REQUIREMENTS OF 30 TAC CHAPTER 217 (DESIGN CRITERIA FOR DOMESTIC WASTEWATER SYSTEMS) OR 30 TAC CHAPTER 290 (PUBLIC DRINKING WATER).

1. THE CONTRACTOR SHALL INSTALL EROSION/SEDIMENTATION CONTROLS AND TREE/NATURAL AREA PROTECTIVE FENCING PRIOR TO ANY SITE PREPARATION WORK (CLEARING, CRUISING OR EXCAVATION).
2. THE PLACEMENT OF EROSION/SEDIMENTATION CONTROLS SHALL BE IN ACCORDANCE WITH THE APPROVED EROSION AND SEDIMENTATION CONTROL PLAN.
3. THE PLACEMENT OF TREE/NATURAL AREA PROTECTIVE FENCING SHALL BE IN ACCORDANCE WITH THE CITY OF GEORGETOWN STANDARD NOTES FOR TREE AND NATURAL AREA PROTECTION AND THE APPROVED GRADING/TREE AND NATURAL AREA PLAN.
4. A PRE-CONSTRUCTION CONFERENCE SHALL BE HELD WITH THE CONTRACTOR, DESIGN ENGINEER, PERMIT APPLICANT AND CITY INSPECTOR PRIOR TO INSTALLATION OF THE EROSION/SEDIMENTATION CONTROLS AND TREE/NATURAL AREA PROTECTION MEASURES AND PRIOR TO BEGINNING ANY SITE PREPARATION WORK. THE CONTRACTOR SHALL NOTIFY THE CITY OF GEORGETOWN, AT LEAST THREE DAYS PRIOR TO THE MEETING DATE.
5. THE CONTRACTOR IS REQUIRED TO INSPECT THE CONTROLS AND FENCES AT WEEKLY INTERVALS AND AFTER SIGNIFICANT RAINFALL EVENTS TO INSURE THAT THEY ARE FUNCTIONING PROPERLY. THE PERSON(S) RESPONSIBLE FOR MAINTENANCE OF CONTROLS AND FENCES SHALL IMMEDIATELY MAKE ANY NECESSARY REPAIRS TO DAMAGED AREAS. SILT ACCUMULATION AT CONTROLS MUST BE REMOVED WHEN THE DEPTH REACHES SIX (6) INCHES.
6. PRIOR TO FINAL ACCEPTANCE BY THE CITY, ALL ROADS AND WATERWAY CROSSINGS CONSTRUCTED FOR TEMPORARY CONTRACTOR ACCESS MUST BE REMOVED, ACCUMULATED SEDIMENT REMOVED FROM THE WATERWAY AND THE AREA RESTORED TO THE ORIGINAL GRADE AND VEGETATED. ALL LAND CLEARING DEBRIS SHALL BE DISPOSED OF IN APPROVED SPOIL DISPOSAL SITES.

1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE CITY OF GEORGETOWN STANDARD CONSTRUCTION SPECIFICATIONS AS ADOPTED AND AMENDED UNLESS OTHERWISE SPECIFIED.
2. ANY EXISTING UTILITIES, PAVEMENT, CURBS, SIDEWALKS, STRUCTURES, TREES, ETC., THAT ARE DAMAGED OR REMOVED SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR AT NO COST TO THE OWNER.
3. THE CONTRACTOR SHALL VERIFY ALL DEPTHS AND LOCATIONS OF EXISTING UTILITIES PRIOR TO ANY CONSTRUCTION. ANY DISCREPANCIES WITH THE CONSTRUCTION PLANS FOUND IN THE FIELD SHALL BE BROUGHT IMMEDIATELY TO THE ATTENTION OF THE ENGINEER.
4. THE CONTRACTOR SHALL GIVE THE CITY OF GEORGETOWN 48 HOURS NOTICE BEFORE BEGINNING EACH PHASE OF CONSTRUCTION.
5. ALL AREAS DISTURBED OR EXPOSED DURING CONSTRUCTION SHALL BE REVEGETATED IN ACCORDANCE WITH THE PLANS AND CITY OF GEORGETOWN STANDARD SPECIFICATIONS. REVEGETATION OF ALL DISTURBED OR EXPOSED AREAS SHALL CONSIST OF SODDING OR SEEDING AT THE CONTRACTOR'S OPTION. HOWEVER, THE TYPE OF REVEGETATION MUST EQUAL OR EXCEED THE TYPE OF VEGETATION PRESENT BEFORE CONSTRUCTION UNLESS OTHERWISE REQUESTED BY THE OWNER.
6. PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR SHALL CONVENE A PRECONSTRUCTION CONFERENCE BETWEEN THE CITY OF GEORGETOWN, THE CITY ENGINEER, THE OWNER, THE ENVIRONMENTAL ENGINEER, GEOTECHNICAL ENGINEER, UTILITY COMPANIES, ANY AFFECTED PARTIES AND ANY OTHER ENTITY THE COUNTY OR ENGINEER MAY REQUIRE.
7. WHEN CONSTRUCTION IS BEING CARRIED OUT WITHIN EASEMENTS, THE CONTRACTOR SHALL CONFINE HIS WORK TO WITHIN THE PERMANENT AND ANY TEMPORARY EASEMENTS. PRIOR TO FINAL ACCEPTANCE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING ALL TRASH AND DEBRIS WITHIN THE PERMANENT AND TEMPORARY EASEMENTS. CLEANUP SHALL BE TO THE SATISFACTION OF THE ENGINEER.
8. PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR SHALL APPLY FOR AND SECURE ALL PROPER PERMITS FROM THE APPROPRIATE AUTHORITIES.

9. AVAILABLE BENCHMARK(S) THAT MAY BE UTILIZED FOR THE CONSTRUCTION OF THIS PROJECT ARE DESCRIBED AS FOLLOWS:

NAVD 88 (GENCH 12A)

BM(1380)-221:  
COTTON GIN SPINDLE FOUND IN THE SOUTH EDGE OF A CONCRETE SIDEWALK  
ELEVATION = 962.21 FEET.

BM(1380)-700100:  
MAGNAIL WITH WASHER STAMPED HR GREEN SET IN CONCRETE RIM OF WATER MANHOLE  
ELEVATION = 940.16 FEET.

BM(1380)-700200:  
MAGNAIL WITH WASHER STAMPED HR GREENSET IN CONCRETE BASE OF BOLLARD  
ELEVATION = 890.30 FEET.

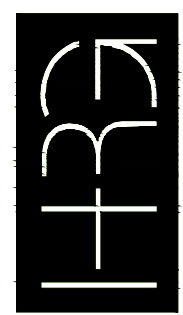
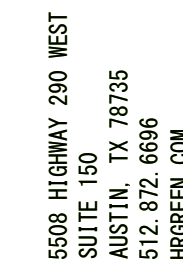
10. SIDEWALK RAMPS AND SIDEWALKS LOCATED IN FRONT OF COMMON AREAS TO BE INSTALLED WITH INFRASTRUCTURE CONSTRUCTION
11. CONTRACTOR IS RESPONSIBLE FOR DAMAGE TO ANY EXISTING UTILITY OR IMPROVEMENTS.
12. CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT TITLED "GEOTECHNICAL INVESTIGATION PAVEMENT THICKNESS RECOMMENDATIONS – PARKSIDE ON THE RIVER SECTIONS 8, 9, & 10 GEORGETOWN, TEXAS," DATED OCTOBER 23<sup>RD</sup> BY MLA GEOTECHNICAL ENGINEER'S JOB# 23101123.001 FOR PAVEMENT DESIGN RECOMMENDATIONS. ANY CONFLICT BETWEEN THESE CONSTRUCTION PLANS AND THE GEOTECHNICAL REPORT SHALL BE RESOLVED IN FAVOR OF THE GEOTECHNICAL REPORT.
13. THE DISTRICT ENGINEER, JONES-HEROY & ASSOCIATES, INC. (KEN HEROY, PH: 512-989-2200) SHALL BE CONTACTED 48 HOURS PRIOR TO THE FOLLOWING:
  - 1) PRE-CONSTRUCTION MEETINGS
  - 2) BEGINNING EACH PHASE OF CONSTRUCTION
  - 3) TESTING OF WATER AND/OR WASTEWATER LINES
  - 4) FINAL WALK-THROUGH OF FACILITIES
14. WHEN REQUIRED, CONTRACTOR SHALL REMOVE PAVEMENT IN ACCORDANCE WITH THE TEXAS DEPARTMENT OF HIGHWAY AND PUBLIC TRANSPORTATION STANDARD SPECIFICATIONS, LATEST EDITION.
15. ALL PAVEMENT REMOVED SHALL BE DONE SUCH THAT THE REMAINING PAVEMENT IS LEFT WITH A CLEAN STRAIGHT EDGE.
16. WHEN REQUIRED, CONTRACTOR SHALL REMOVE EXISTING PAVEMENT STRIPING BY SAND BLASTING FROM EXISTING PAVEMENT IN ACCORDANCE WITH ITEM 678 OF THE TxDOT LATEST EDITION.
17. ALL WORK IN STATE R.O.W. AND EASEMENTS SHALL BE IN ACCORDANCE WITH THE TxDOT LATEST EDITION.
18. EARTHWORK FOR ALL BUILDING FOUNDATIONS AND SLABS SHALL BE IN ACCORDANCE WITH ARCHITECTURAL BUILDING PLANS AND SPECIFICATIONS AND THE GEOTECHNICAL STUDY.
19. IF THE CONTRACTOR FINDS A DISCREPANCY WITH THE TOPOGRAPHIC INFORMATION ON THESE PLANS HE/SHE SHOULD CONTACT THE ENGINEER OR OWNER IMMEDIATELY.
20. CONTRACTOR SHALL PROTECT ALL BENCHMARKS AND PROPERTY MONUMENTATION DISTURBED DURING CONSTRUCTION.
21. DESIGN OF MAJOR DRAINAGE WAYS THROUGH A SUBDIVISION AND MAJOR STRUCTURES SUCH AS BOX CULVERTS OR BRIDGES ACROSS A MAJOR DRAINAGE CHANNEL SHALL BE COORDINATED WITH THE REQUIREMENTS OF THE WILLIAMSON COUNTY HEALTH DISTRICT WHEN ANY PORTION OF THE SUBDIVISION LIES OUTSIDE THE CITY LIMITS, AND WHEN APPLICABLE, A LETTER REQUESTING A LOCAL FLOOD PLAIN MAP ADJUSTMENT FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) SHALL BE PROVIDED PRIOR TO FINAL CONSTRUCTION PLAN APPROVAL.

1. ANY METHODS, STREET MARKINGS AND SIGNAGE NECESSARY FOR WARNING MOTORISTS, WARNING PEDESTRIANS OR DIVERTING TRAFFIC DURING CONSTRUCTION SHALL CONFORM TO THE TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS, LATEST EDITION.
2. ALL PAVEMENT MARKINGS, MARKERS, PAINT, TRAFFIC BUTTONS, TRAFFIC CONTROLS AND SIGNS SHALL BE INSTALLED IN ACCORDANCE WITH THE TEXAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR CONSTRUCTION OF HIGHWAYS, STREETS AND BRIDGES AND, THE TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS, LATEST EDITION.

1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MOWING AND THE REMOVAL OF ALL LITTER WITHIN THE PROJECT LIMITS SO AS TO KEEP THE SITE OF THE WORK IN A NEAT AND PRESENTABLE CONDITION AT ALL TIMES. THIS WORK WILL BE CONSIDERED SUBSIDIARY TO THE VARIOUS BID ITEMS.
2. THE CONTRACTOR SHALL PROTECT ALL AREAS WHICH ARE NOT INCLUDED IN THE ACTUAL LIMITS OF THE PROPOSED CONSTRUCTION AREAS FROM DESTRUCTION. CARE SHALL BE EXERCISED TO PREVENT DAMAGE TO TREES, VEGETATION, FENCES, POWER POLES, AND OTHER NATURAL SURROUNDINGS. THE AREAS NOT TO BE DISTURBED INCLUDE ALL GOLF COURSE AREAS, UNLESS SPECIFIED OTHERWISE. THE CONTRACTOR SHALL, AT HIS EXPENSE, RESTORE ANY AREA DISTURBED AS A RESULT OF HIS OPERATIONS TO A CONDITION AS GOOD AS, OR BETTER THAN, THAT PRESENT PRIOR TO CONSTRUCTION.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MARKING EVERY 100 FOOT ROAD STATION, AND SHALL MAINTAIN THE MARKINGS FOR THE DURATION OF THE PROJECT. THIS WORK SHALL BE CONSIDERED SUBSIDIARY TO THE ITEMIZED CONSTRUCTION CONTRACT.
4. THE SUPERINTENDENT SHALL BE AVAILABLE ON THE PROJECT AT ALL TIMES WHEN WORK IS BEING PERFORMED.
5. NO BLASTING IS ALLOWED ON THIS PROJECT.
6. NO STORAGE OF HYDROCARBON OR HAZARDOUS MATERIAL IS ALLOWED ON SITE.

1. THE DISTRICT ENGINEER, JONES-HEROY & ASSOCIATES, INC. (KEN HEROY, PH: 512-989-2200) SHALL BE CONTACTED 48 HOURS PRIOR TO:
  - i) PRE-CONSTRUCTION MEETINGS;
  - ii) BEGINNING EACH PHASE OF CONSTRUCTION
  - iii) TESTING OF WATER AND/OR WASTEWATER LINES; AND,
  - iv) FINAL WALK-THROUGH OF FACILITIES
2. REVIEW OF THE PLANS BY THE DISTRICT IS LIMITED TO WATER, WASTEWATER, AND DRAINAGE, AND DOES NOT INDICATE A REVIEW OF THE ADEQUACY OF THE DESIGN FOR THE FACILITIES. IN APPROVING THESE PLANS, THE DISTRICT MUST RELY ON THE ADEQUACY OF THE WORK OF THE DESIGN ENGINEER.

1. 1,500 GPM FIRE FLOW SHALL BE PROVIDED FOR THIS PROJECT.
2. AT THE CONCLUSION OF CONSTRUCTION AND AS PART OF THE PROCESS FOR THE CITY TO ACCEPT THIS PHASE:
  - \* THE FIRE HYDRANTS SHALL BE FLOWED AND TESTED
  - \* A COPY OF THE REPORT SHALL BE EMAILED INTO THE FIRE DEPARTMENT
  - \* THE HYDRANTS SHALL BE PAINTED AND COLOR CODED.
- \*\*\* CAUTION :IF PRESSURE REDUCING VALVES WERE INSTALLED IN THIS PHASING THEY MUST BE SET PRIOR TO FIRE HYDRANT FLOW TESTING.
3. PER CITY ORDINANCE SEC. 13.15.120, HYDRANT FLOW CODING STANDARDS, PUBLIC HYDRANTS WILL HAVE THE BARRELS PAINTED SILVER, THE HYDRANTS WILL BE FLOW TESTED, AND THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN PARAGRAPH C.
  - \* GREATER THAN 1500 GPM BLUE
  - \* 1000 TO 1500 GPM GREEN
  - \* 500 – 999 GPM ORANGE
  - \* LESS THAN 500 GPM RED
  - \* NOT WORKING BLACK OR BAGGED



**GENERAL NOTES**

**PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS**

GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
  
SHEET 2 OF 84  
  
**2023-XX-CON**



P:\baker\magnagrainside sections 8, 9a & 10a03\ACAD\Plan\20200505\0505-01NTS.dwg TCEQ NOTES December 19, 2020 8:37 AM Campbell

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY  
ORGANIZED SEWAGE COLLECTION SYSTEM (SCS)  
GENERAL CONSTRUCTION NOTES

1. THIS ORGANIZED SEWAGE COLLECTION SYSTEM MUST BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY'S (TCEQ) EDWARDS AQUIFER RULES 30 TEXAS ADMINISTRATIVE CODE (TAC) §213.5(C) AND 217.51 – 217.70 AND 30 TAC CHAPTER 217, SUBCHAPTER D, AND THE CITY OF ROUND ROCK STANDARD SPECIFICATIONS.
2. ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROPOSED REGULATED PROJECT MUST BE PROVIDED WITH COPIES OF THE SEWAGE COLLECTION SYSTEM PLAN AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS MUST BE REQUIRED TO KEEP ON-SITE COPIES OF THE PLAN AND THE APPROVAL LETTER.
3. NO LATER THAN 48 HOURS PRIOR TO COMMENCING ANY REGULATED ACTIVITY, THE APPLICANT OR HIS AGENT MUST NOTIFY THE TCEQ AUSTIN REGIONAL OFFICE, IN WRITING, OF THE DATE ON WHICH THE REGULATED ACTIVITY WILL BEGIN.
4. ANY MODIFICATION TO THE ACTIVITIES DESCRIBED IN THE REFERENCED SCS APPLICATION FOLLOWING THE DATE OF APPROVAL MAY REQUIRE THE SUBMITTAL OF AN SCS APPLICATION TO MODIFY THIS APPROVAL, INCLUDING THE PAYMENT OF APPROPRIATE FEES AND ALL INFORMATION NECESSARY FOR ITS REVIEW AND APPROVAL.
5. ALL TEMPORARY EROSION AND SEDIMENTATION CONTROLS MUST BE INSTALLED PRIOR TO CONSTRUCTION, MUST BE MAINTAINED DURING CONSTRUCTION, AND MUST BE REMOVED WHEN SUFFICIENT VEGETATION IS ESTABLISHED TO CONTROL THE EROSION AND SEDIMENTATION AND THE CONSTRUCTION AREA IS STABILIZED.
6. THE SEWER LINE TRENCH DETAILS SHOWING THE CROSS SECTION WITH THE DIMENSIONS, PIPE PLACEMENT, AND BACKFILL INSTRUCTIONS ARE INCLUDED ON PLAN SHEET 80 OF 124 OF THESE PLANS. ALL SEWER PIPES JOINTS MUST MEET THE REQUIREMENTS IN 30 TAC §217.53(C) AN 217.65.

GRAVITY LINES MUST HAVE A SDR 35 OR LESS. PRESSURIZED SEWER SYSTEMS MUST HAVE PIPE WITH A MINIMUM WORKING PRESSURE RATING OF 150 PSI.

THE ASTM, ANSI, OR AWWA SPECIFICATION NUMBERS FOR THE PIPE(S) AND JOINTS ARE ASTM–D3034.

THE PIPE MATERIAL, THE PRESSURE CLASSES, AND THE SDR AND/OR DR DESIGNATIONS ARE SDR–26.

7. IF ANY SENSITIVE FEATURES ARE DISCOVERED DURING THE WASTEWATER LINE TRENCHING ACTIVITIES, ALL REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE SUSPENDED IMMEDIATELY. THE APPLICANT MUST IMMEDIATELY NOTIFY THE APPROPRIATE REGIONAL OFFICE OF THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY OF THE FEATURE DISCOVERED. A GEOLOGIST'S ASSESSMENT OF THE LOCATION AND EXTENT OF THE FEATURE DISCOVERED MUST BE REPORTED TO THAT REGIONAL OFFICE IN WRITING WITHIN TWO WORKING DAYS. THE APPLICANT MUST SUBMIT A PLAN FOR ENSURING THE STRUCTURAL INTEGRITY OF THE SEWER LINE OR FOR MODIFYING THE PROPOSED COLLECTION SYSTEM ALIGNMENT AROUND THE FEATURE. THE REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MAY NOT PROCEED UNTIL THE EXECUTIVE DIRECTOR HAS REVIEWED AND APPROVED THE METHODS PROPOSED TO PROTECT THE SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM ANY POTENTIALLY ADVERSE IMPACTS TO WATER QUALITY WHILE MAINTAINING THE STRUCTURAL INTEGRITY OF THE LINE.
8. SEWER LINES LOCATED WITHIN OR CROSSING THE 5-YEAR FLOODPLAIN OF A DRAINAGE WAY WILL BE PROTECTED FROM INUNDATION AND STREAM WIDENING WHICH COULD CAUSE EROSION AND SCOURING OF BACKFILL. THE TRENCH MUST BE CAPPED WITH CONCRETE TO PREVENT SCOURING OF BACKFILL, OR THE SEWER LINES MUST BE ENCASED IN CONCRETE. ALL CONCRETE SHALL HAVE A MINIMUM THICKNESS OF SIX (6) INCHES.
9. BLASTING PROCEDURES FOR PROTECTION OF EXISTING SEWER LINES AND OTHER UTILITIES WILL BE IN ACCORDANCE WITH THE NATIONAL FIRE PROTECTION ASSOCIATION CRITERIA. SAND IS NOT ALLOWED AS BEDDING OR BACKFILL IN TRENCHES THAT HAVE BEEN BLASTED. IF ANY EXISTING SEWER LINES ARE DAMAGED, THE LINES MUST BE REPAIRED AND RETESTED.
10. ALL MANHOLES CONSTRUCTED OR REHABILITATED ON THIS PROJECT MUST HAVE WATERTIGHT SIZE ON SIZE RESILIENT CONNECTORS ALLOWING FOR DIFFERENTIAL SETTLEMENT. IF MANHOLES ARE CONSTRUCTED WITHIN THE 100-YEAR FLOODPLAIN, THE COVER MUST HAVE A GASKET AND BE BOLTED TO THE RING. WHERE GASKETED MANHOLE COVERS ARE REQUIRED FOR MORE THAN THREE MANHOLES IN SEQUENCE OR FOR MORE THAN 1500 FEET, ALTERNATE MEANS OF VENTING WILL BE PROVIDED. BRICKS ARE NOT AN ACCEPTABLE CONSTRUCTION MATERIAL FOR ANY PORTION OF THE MANHOLE.

THE DIAMETER OF THE MANHOLES MUST BE A MINIMUM OF FOUR FEET AND THE MANHOLE FOR ENTRY MUST HAVE A MINIMUM CLEAR OPENING DIAMETER OF 30 INCHES. THESE DIMENSIONS AND OTHER DETAILS SHOWING COMPLIANCE WITH THE COMMISSION'S RULES CONCERNING MANHOLES AND SEWER LINE/MANHOLE INVERTS DESCRIBED IN 30 TAC §217.55 ARE INCLUDED ON PLAN SHEET 75 & 77 OF 124.

IT IS SUGGESTED THAT ENTRANCE INTO MANHOLES IN EXCESS OF FOUR FEET DEEP BE ACCOMPLISHED BY MEANS OF A PORTABLE LADDER. THE INCLUSION OF STEPS IN A MANHOLE IS PROHIBITED.

11. WHERE WATER LINES AND NEW SEWER LINE ARE INSTALLED WITH A SEPARATION DISTANCE CLOSER THAN NINE FEET (I.E., WATER LINES CROSSING WASTEWATER LINES, WATER LINES PARALLELING WASTEWATER LINES, OR WATER LINES NEXT TO MANHOLES) THE INSTALLATION MUST MEET THE REQUIREMENTS OF 30 TAC §217.53(D) (PIPE DESIGN) AND 30 TAC §290.44(E) (WATER DISTRIBUTION).

12. WHERE SEWERS LINES DEViate FROM STRAIGHT ALIGNMENT AND UNIFORM GRADE ALL CURVATURE OF SEWER PIPE MUST BE ACHIEVED BY THE FOLLOWING PROCEDURE WHICH IS RECOMMENDED BY THE PIPE MANUFACTURER: N/A.

IF PIPE FLEXURE IS PROPOSED, THE FOLLOWING METHOD OF PREVENTING DEFLECTION OF THE JOINT MUST BE USED: N/A.

SPECIFIC CARE MUST BE TAKEN TO ENSURE THAT THE JOINT IS PLACED IN THE CENTER OF THE TRENCH AND PROPERLY BEDDED IN ACCORDANCE WITH 30 TAC §217.54.

13. NEW SEWAGE COLLECTION SYSTEM LINES MUST BE CONSTRUCTED WITH STUB OUTS FOR THE CONNECTION OF ANTICIPATED EXTENSIONS. THE LOCATION OF SUCH STUB OUTS MUST BE MARKED ON THE GROUND SUCH THAT THEIR LOCATION CAN BE EASILY DETERMINED AT THE TIME OF CONNECTION OF THE EXTENSIONS. SUCH STUB OUTS MUST BE MANUFACTURED WYES OR TEES THAT ARE COMPATIBLE IN SIZE AND MATERIAL WITH BOTH THE SEWER LINE AND THE EXTENSION. AT THE TIME OF ORIGINAL CONSTRUCTION, NEW STUB-OUTS MUST BE CONSTRUCTED SUFFICIENTLY TO EXTEND BEYOND THE END OF THE STREET PAVEMENT. ALL STUB-OUTS MUST BE SEALED WITH A MANUFACTURED CAP TO PREVENT LEAKAGE. EXTENSIONS THAT WERE NOT ANTICIPATED AT THE TIME OF ORIGINAL CONSTRUCTION OR THAT ARE TO BE CONNECTED TO AN EXISTING SEWER LINE NOT FURNISHED WITH STUB OUTS MUST BE CONNECTED USING A MANUFACTURED SADDLE AND IN ACCORDANCE WITH ACCEPTED PLUMBING TECHNIQUES.

14. TRENCHING, BEDDING AND BACKFILL MUST CONFORM WITH 30 TAC §217.54. THE BEDDING AND BACKFILL FOR FLEXIBLE PIPE MUST COMPLY WITH THE STANDARDS OF ASTM D–2321, CLASSES IA, IB, II OR III. RIGID PIPE BEDDING MUST COMPLY WITH THE REQUIREMENTS OF ASTM C 12 (ANSI A 106.2) CLASSES A, B OR C.

15. SEWER LINES MUST BE TESTED FROM MANHOLE TO MANHOLE. WHEN A NEW SEWER LINE IS CONNECTED TO AN EXISTING STUB OR CLEAN-OUT, IT MUST BE TESTED FROM EXISTING MANHOLE TO NEW MANHOLE. IF A STUB OR CLEAN-OUT IS USED AT THE END OF THE PROPOSED SEWER LINE, NO PRIVATE SERVICE ATTACHMENTS MAY BE CONNECTED BETWEEN THE LAST MANHOLE AND THE CLEANOUT UNLESS IT CAN BE CERTIFIED AS CONFORMING WITH THE PROVISIONS OF 30 TAC §213.5(C)(3)(C).

16. ALL SEWER LINES MUST BE TESTED IN ACCORDANCE WITH 30 TAC §217.57. THE ENGINEER MUST RETAIN COPIES OF ALL TEST RESULTS WHICH MUST BE MADE AVAILABLE TO THE EXECUTIVE DIRECTOR UPON REQUEST. THE ENGINEER MUST CERTIFY IN WRITING THAT ALL WASTEWATER LINES HAVE PASSED ALL REQUIRED TESTING TO THE APPROPRIATE REGIONAL OFFICE WITHIN 30 DAYS OF TEST COMPLETION AND PRIOR TO USE OF THE NEW COLLECTION SYSTEM. THE TESTING METHOD WILL BE
- (A) OR A COLLECTION SYSTEM PIPE THAT WILL TRANSPORT WASTEWATER BY GRAVITY FLOW, THE DESIGN MUST SPECIFY AN INFILTRATION AND EXFILTRATION TEST OR A LOW-PRESSURE AIR TEST. A TEST MUST CONFORM TO THE FOLLOWING REQUIREMENTS:

- (1) LOW PRESSURE AIR TEST
- (A) A LOW PRESSURE AIR TEST MUST FOLLOW THE PROCEDURES DESCRIBED IN AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) C-828, ASTM C-924, OR ASTM F-1417 OR OTHER PROCEDURE APPROVED BY THE EXECUTIVE DIRECTOR, EXCEPT AS TO TESTING TIMES AS REQUIRED IN TABLE C.3 IN SUBPARAGRAPH (C) OF THIS PARAGRAPH OR EQUIVALENT C.3 IN SUBPARAGRAPH (B)(II) OF THIS PARAGRAPH.
- (B) FOR SECTIONS OF COLLECTION SYSTEM PIPE LESS THAN 36 INCH AVERAGE INSIDE DIAMETER, THE FOLLOWING PROCEDURE MUST APPLY, UNLESS A PIPE IS TO BE TESTED AS REQUIRED BY PARAGRAPH (2) OF THIS SUBSECTION.
- (I) A PIPE MUST BE PRESSURIZED TO 3.5 POUNDS PER SQUARE INCH (PSI) GREATER THAN THE PRESSURE EXERTED BY GROUNDWATER ABOVE THE PIPE.
- (II) ONCE THE PRESSURE IS STABILIZED, THE MINIMUM TIME ALLOWABLE FOR THE PRESSURE TO DROP FROM 3.5 PSI GAUGE TO 2.5 PSI GAUGE IS COMPUTED FROM THE FOLLOWING EQUATION:

$$\text{EQUATION C.3 } T = \frac{0.085 \times K \times D \times L}{Q}$$

WHERE:

- T = TIME FOR PRESSURE TO DROP 1.0 POUND PER SQUARE INCH GAUGE IN SECONDS
- K = 0.000419 X D X L, BUT NOT LESS THAN 1.0
- D = AVERAGE INSIDE PIPE DIAMETER IN INCHES
- L = LENGTH OF LINE OF SAME SIZE BEING TESTED, IN FEET
- Q = RATE OF LOSS, 0.0015 CUBIC FEET PER MINUTE PER SQUARE FOOT INTERNAL SURFACE
- (C) SINCE A K VALUE OF LESS THAN 1.0 MAY NOT BE USED, THE MINIMUM TESTING TIME FOR EACH PIPE DIAMETER IS SHOWN IN THE FOLLOWING

TABLE C.3:

| PIPE DIAMETER (INCHES) | MINIMUM TIME (SECONDS) | MAXIMUM LENGTH FOR MINIMUM TIME (FEET) | TIME FOR LONGER LENGTH (SECONDS/FOOT) |
|------------------------|------------------------|----------------------------------------|---------------------------------------|
| 6                      | 340                    | 398                                    | 0.855                                 |
| 8                      | 454                    | 298                                    | 1.520                                 |
| 10                     | 567                    | 239                                    | 2.374                                 |
| 12                     | 680                    | 199                                    | 3.419                                 |
| 15                     | 850                    | 159                                    | 5.342                                 |
| 18                     | 1020                   | 133                                    | 7.693                                 |
| 21                     | 1190                   | 114                                    | 10.471                                |
| 24                     | 1360                   | 100                                    | 13.676                                |
| 27                     | 1530                   | 88                                     | 17.309                                |
| 30                     | 1700                   | 80                                     | 21.369                                |
| 33                     | 1870                   | 72                                     | 25.856                                |

- (D) AN OWNER MAY STOP A TEST IF NO PRESSURE LOSS HAS OCCURRED DURING THE FIRST 25% OF THE CALCULATED TESTING TIME.
- (E) IF ANY PRESSURE LOSS OR LEAKAGE HAS OCCURRED DURING THE FIRST 25% OF A TESTING PERIOD, THEN THE TEST MUST CONTINUE FOR THE ENTIRE TEST DURATION AS OUTLINED ABOVE OR UNTIL FAILURE.
- (F) WASTEWATER COLLECTION SYSTEM PIPES WITH A 27 INCH OR LARGER AVERAGE INSIDE DIAMETER MAY BE AIR TESTED AT EACH JOINT INSTEAD OF FOLLOWING THE PROCEDURE OUTLINED IN THIS SECTION.
- (G) A TESTING PROCEDURE FOR PIPE WITH AN INSIDE DIAMETER GREATER THAN 33 INCHES MUST BE APPROVED BY THE EXECUTIVE DIRECTOR.
- (2) INFILTRATION/EXFILTRATION TEST.

- (A) THE TOTAL EXFILTRATION, AS DETERMINED BY A HYDROSTATIC HEAD TEST, MUST NOT EXCEED 50 GALLONS PER INCH OF DIAMETER PER MILE OF PIPE PER 24 HOURS AT A MINIMUM TEST HEAD OF 2.0 FEET ABOVE THE CROWN OF A PIPE AT AN UPSTREAM MANHOLE.
- (B) AN OWNER SHALL USE AN INFILTRATION TEST IN LIEU OF AN EXFILTRATION TEST WHEN PIPES ARE INSTALLED BELOW THE GROUNDWATER LEVEL.
- (C) THE TOTAL EXFILTRATION, AS DETERMINED BY A HYDROSTATIC HEAD TEST, MUST NOT EXCEED 50 GALLONS PER INCH DIAMETER PER MILE OF PIPE PER 24 HOURS AT A MINIMUM TEST HEAD OF TWO FEET ABOVE THE CROWN OF A PIPE AT AN UPSTREAM MANHOLE OR AT LEAST TWO FEET ABOVE EXISTING GROUNDWATER LEVEL, WHICHEVER IS GREATER.
- (D) FOR CONSTRUCTION WITHIN A 25-YEAR FLOOD PLAIN, THE INFILTRATION OR EXFILTRATION MUST NOT EXCEED 10 GALLONS PER INCH DIAMETER PER MILE OF PIPE PER 24 HOURS AT THE SAME MINIMUM TEST HEAD AS IN SUBPARAGRAPH (C) OF THIS PARAGRAPH.
- (E) IF THE QUANTITY OF INFILTRATION OR EXFILTRATION EXCEEDS THE MAXIMUM QUANTITY SPECIFIED, AN OWNER SHALL UNDERTAKE REMEDIAL ACTION IN ORDER TO REDUCE THE INFILTRATION OR EXFILTRATION TO AN AMOUNT WITHIN THE LIMITS SPECIFIED. AN OWNER SHALL RETEST A PIPE FOLLOWING A REMEDIATION ACTION.
- (F) IF A GRAVITY COLLECTION PIPE IS COMPOSED OF FLEXIBLE PIPE, DEFLECTION TESTING IS ALSO REQUIRED. THE FOLLOWING PROCEDURES MUST BE FOLLOWED:
- (1) FOR A COLLECTION PIPE WITH INSIDE DIAMETER LESS THAN 27 INCHES, DEFLECTION MEASUREMENT REQUIRES A RIGID MANDREL.
- (A) MANDREL SIZING.
- (I) A RIGID MANDREL MUST HAVE AN OUTSIDE DIAMETER (OD) NOT LESS THAN 95% OF THE BASE INSIDE DIAMETER (ID) OR AVERAGE ID OF A PIPE, AS SPECIFIED IN THE APPROPRIATE STANDARD BY THE ASTM'S, AMERICAN WATER WORKS ASSOCIATION, UN-BELL, OR AMERICAN NATIONAL STANDARDS INSTITUTE, OR ANY RELATED APPENDIX.
- (II) IF A MANDREL SIZING DIAMETER IS NOT SPECIFIED IN THE APPROPRIATE STANDARD, THE MANDREL MUST HAVE AN OD EQUAL TO 95% OF THE ID OF A PIPE. IN THIS CASE, THE ID OF THE PIPE, FOR THE PURPOSE OF DETERMINING THE OD OF THE MANDREL, MUST EQUAL BE THE AVERAGE OUTSIDE DIAMETER MINUS TWO MINIMUM WALL THICKNESSES FOR OD CONTROLLED PIPE AND THE AVERAGE INSIDE DIAMETER FOR ID CONTROLLED PIPE.
- (III) ALL DIMENSIONS MUST MEET THE APPROPRIATE STANDARD.
- (B) MANDREL DESIGN.
- (I) A RIGID MANDREL MUST BE CONSTRUCTED OF A METAL OR A RIGID PLASTIC MATERIAL THAT CAN WITHSTAND 200 PSI WITHOUT BEING DEFORMED.
- (II) A MANDREL MUST HAVE NINE OR MORE ODD NUMBER OF RUNNERS OR LEGS.
- (III) A BARREL SECTION LENGTH MUST EQUAL AT LEAST 75% OF THE INSIDE DIAMETER OF A PIPE.
- (IV) EACH SIZE MANDREL MUST USE A SEPARATE PROVING RING.
- (C) METHOD OPTIONS.
- (I) AN ADJUSTABLE OR FLEXIBLE MANDREL IS PROHIBITED.
- (II) A TEST MAY NOT USE TELEVISION INSPECTION AS A SUBSTITUTE FOR A DEFLECTION TEST.
- (III) IF REQUESTED, THE EXECUTIVE DIRECTOR MAY APPROVE THE USE OF A DEFLECTOMETER OR A MANDREL WITH REMOVABLE LEGS OR RUNNERS ON A CASE-BY-CASE BASIS.
- (2) FOR A GRAVITY COLLECTION SYSTEM PIPE WITH AN INSIDE DIAMETER 27 INCHES AND GREATER, OTHER TEST METHODS MAY BE USED TO DETERMINE VERTICAL DEFLECTION.
- (3) A DEFLECTION TEST METHOD MUST BE ACCURATE TO WITHIN PLUS OR MINUS 0.2% DEFLECTION.
- (4) AN OWNER SHALL NOT CONDUCT A DEFLECTION TEST UNTIL AT LEAST 30 DAYS AFTER THE FINAL BACKFILL.
- (5) GRAVITY COLLECTION SYSTEM PIPE DEFLECTION MUST NOT EXCEED FIVE PERCENT (5%).
- (6) IF A PIPE SECTION FAILS A DEFLECTION TEST, AN OWNER SHALL CORRECT THE PROBLEM AND CONDUCT A SECOND TEST AFTER THE FINAL BACKFILL HAS BEEN IN PLACE AT LEAST 30 DAYS.
17. ALL MANHOLES MUST BE TESTED TO MEET OR EXCEED THE REQUIREMENTS OF 30 TAC §217.58.

18. ALL PRIVATE SERVICE LATERALS MUST BE INSPECTED AND CERTIFIED IN ACCORDANCE WITH 30 TAC §213.5(C)(3)(I). AFTER INSTALLATION OF AND, PRIOR TO COVERING AND CONNECTING A PRIVATE SERVICE LATERAL TO AN EXISTING ORGANIZED SEWAGE COLLECTION SYSTEM, A TEXAS LICENSED PROFESSIONAL ENGINEER, TEXAS REGISTERED SANITARIAN, OR APPROPRIATE CITY INSPECTOR MUST VISUALLY INSPECT THE PRIVATE SERVICE LATERAL AND THE CONNECTION TO THE SEWAGE COLLECTION SYSTEM, AND CERTIFY THAT IT IS CONSTRUCTED IN CONFORMITY WITH THE APPLICABLE PROVISIONS OF THIS SECTION. THE OWNER OF THE COLLECTION SYSTEM MUST MAINTAIN SUCH CERTIFICATIONS FOR FIVE YEARS AND FORWARD COPIES TO THE APPROPRIATE REGIONAL OFFICE UPON REQUEST. CONNECTIONS MAY ONLY BE MADE TO AN APPROVED SEWAGE COLLECTION SYSTEM.

SUPPLEMENTAL TCEQ NOTES:

1. WATERTIGHT, SIZE ON SIZE RESILIENT CONNECTORS CONFORMING TO ASTM C–923 ARE REQUIRED FOR CONNECTING PIPE TO MANHOLES.
2. IF FAULTS, CAVERNS, OR SUBSIDENCE ARE DISCOVERED DURING CONSTRUCTION, CONSTRUCTION SHOULD BE HALTED TO ALLOW THE FEATURES TO BE INSPECTED BY THE DESIGN ENGINEER OR GEOLOGICAL OR GEOTECHNICAL PROFESSIONAL.
3. TRENCH WALLS MUST BE VERTICAL TO AT LEAST ONE FOOT ABOVE THE PIPE. TRENCH BACKFILL MUST BE FREE OF STONES GREATER THAN 6-INCHES AND FREE OF ORGANIC OR ANY OTHER UNSTABLE MATERIAL.
4. ALL WASTEWATER PIPE MATERIAL PVC SDR26–ASTM–3034 USED MUST HAVE A MINIMUM ALLOWABLE TENSILE.

TCEQ WATER DISTRIBUTION SYSTEM  
GENERAL CONSTRUCTION NOTES

1. This water distribution system must be constructed in accordance with the current Texas Commission on Environmental Quality (TCEQ) Rules and Regulations for Public Water Systems 30 Texas Administrative Code (TAC) Chapter 290 Subchapter D. When conflicts are noted with local standards, the more stringent requirement shall be applied. At a minimum, construction for public water systems must always meet TCEQ's "Rules and Regulations for Public Water Systems."
2. All newly installed pipes and related products must conform to American National Standards Institute (ANSI)/NSF International Standard 61 and must be certified by an organization accredited by ANSI [§290.44(a)(1)].
3. Plastic pipe for use in public water systems must bear the NSF International Seal of Approval (NSF–pw) and have an ASTM design pressure rating of at least 150 psi or a standard dimension ratio of 26 or less [§290.44(a)(2)].
4. No pipe which has been used for any purpose other than the conveyance of drinking water shall be accepted or relocated for use in any public drinking water supply [§290.44(a)(3)].
5. All water line crossings of wastewater mains shall be perpendicular [§290.44(e)(4)(B)].
6. Water transmission and distribution lines shall be installed in accordance with the manufacturer's instructions. However, the top of the water line must be located below the frost line and in no case shall the top of the water line be less than 24 inches below ground surface [§290.44(a)(4)].
7. The maximum allowable lead content of pipes, pipe fittings, plumbing fittings, and fixtures is 0.25 percent [§290.44(b)].
8. The contractor shall install appropriate air release devices with vent openings to the atmosphere covered with 16-mesh or finer, corrosion resistant screening material or an acceptable equivalent [§290.44(d)(1)].
9. The contractor shall not place the pipe in water or where it can be flooded with water or sewage during its storage or installation [§290.44(f)(1)].
10. When waterlines are laid under any flowing or intermittent stream or semi-permanent body of water the waterline shall be installed in a separate watertight pipe encasement. Valves must be provided on each side of the crossing with facilities to allow the underwater portion of the system to be isolated and tested [§290.44(d)(2)].

11. Pursuant to 30 TAC §290.44(a)(5), the hydrostatic leakage rate shall not exceed the amount allowed or recommended by the most current AWWA formulas for PVC pipe, cast iron and ductile iron pipe. Include the formulas in the notes on the plans.

- o The hydrostatic leakage rate for polyvinyl chloride (PVC) pipe and appurtenances shall not exceed the amount allowed or recommended by formulas in America Water Works Association (AWWA) C–600 as required in 30 TAC §290.44(a)(5). Please ensure that the formula for this calculation is correct and most current formula is in use;

$$Q = \frac{LD\sqrt{P}}{148,000}$$

Where:

- Q = the quantity of makeup water in gallons per hour,
  - L = the length of the pipe section being tested, in feet,
  - D = the nominal diameter of the pipe in inches, and
  - P = the average test pressure during the hydrostatic test in pounds per square inch (psi).
- o The hydrostatic leakage rate for ductile iron (DI) pipe and appurtenances shall not exceed the amount allowed or recommended by formulas in America Water Works Association (AWWA) C–600 as required in 30 TAC §290.44(a)(5). Please ensure that the formula for this calculation is correct and most current formula is in use;

$$L = \frac{SD\sqrt{P}}{148,000}$$

Where:

- L = the quantity of makeup water in gallons per hour,
- S = the length of the pipe section being tested, in feet,
- D = the nominal diameter of the pipe in inches, and
- P = the average test pressure during the hydrostatic test in pounds per square inch (psi).

12. The contractor shall maintain a minimum separation distance in all directions of nine feet between the proposed waterline and wastewater collection facilities including manholes. If this distance cannot be maintained, the contractor must immediately notify the project engineer for further direction. Separation distances, installation methods, and materials utilized must meet §290.44(e)(1)–(4).

13. The separation distance from a potable waterline to a wastewater main or lateral manhole or cleanout shall be a minimum of nine feet. Where the nine-foot separation distance cannot be achieved, the potable waterline shall be encased in a joint of at least 150 psi pressure class pipe at least 18 feet long and two nominal sizes larger than the new conveyance. The space around the carrier pipe shall be supported at five-foot intervals with spacers or be filled to the springline with washed sand. The encasement pipe shall be centered on the crossing and both ends sealed with cement grout or manufactured sealant [§290.44(e)(5)].

14. Fire hydrants shall not be installed within nine feet vertically or horizontally of any wastewater line, wastewater lateral, or wastewater service line regardless of construction [§290.44(e)(6)].

15. Suction mains to pumping equipment shall not cross wastewater mains, wastewater laterals, or wastewater service lines. Raw water supply lines shall not be installed within five feet of any tile or concrete wastewater main, wastewater lateral, or wastewater service line [§290.44(e)(7)].

16. Waterlines shall not be installed closer than ten feet to septic tank drainfields [§290.44(e)(8)].

17. The contractor shall disinfect the new waterlines in accordance with AWWA Standard C–651–14 or most recent, then flush and sample the lines before being placed into service. Samples shall be collected for microbiological analysis to check the effectiveness of the disinfection procedure which shall be repeated if contamination persists. A minimum of one sample for each 1,000 feet of completed waterline will be required or at the next available sampling point beyond 1,000 feet as designated by the design engineer [§290.44(f)(3)].

18. Dechlorination of disinfecting water shall be in strict accordance with current AWWA Standard C655–09 or most recent.

Texas Commission on Environmental Quality  
Water Pollution Abatement Plan  
General Construction Notes

Edwards Aquifer Protection Program Construction Notes – Legal Disclaimer

The following listed "construction notes" are intended to be advisory in nature only and do not constitute an approval or conditional approval by the Executive Director (ED). nor do they constitute a comprehensive listing of rules or conditions to be followed during construction. Further actions may be required to achieve compliance with TCEQ regulations found in Title 30, Texas Administrative Code (TAC), Chapters 213 and 217, as well as local ordinances and regulations providing for the protection of water quality. Additionally, nothing contained in the following listed "construction notes" restricts the powers of the ED, the commission or any other governmental entity to prevent, correct, or curtail activities that result or may result in pollution of the Edwards Aquifer or hydrologically connected surface waters. The holder of any Edwards Aquifer Protection Plan containing "construction notes" is still responsible for compliance with Title 30, TAC, Chapters 213 or any other applicable TCEQ regulation, as well as all conditions of an Edwards Aquifer Protection Plan through all phases of plan implementation. Failure to comply with any condition of the ED's approval, whether or not in contradiction of any "construction notes," is a violation of TCEQ regulations and any violation is subject to administrative rules, orders, and penalties as provided under Title 30, TAC § 213.10 (relating to Enforcement). Such violations may also be subject to civil penalties and injunction. The following listed "construction notes" in no way represent an approved exception by the ED to any part of Title 30 TAC, Chapters 213 and 217, or any other TCEQ applicable regulation

1. A written notice of construction must be submitted to the TCEQ regional office at least 48 hours prior to the start of any regulated activities. This notice must include:
- the name of the approved project;
  - the activity start date; and
  - the contact information of the prime contractor.
2. All contractors conducting regulated activities associated with this project must be provided with complete copies of the approved Water Pollution Abatement Plan (WPAP) and the TCEQ letter indicating the specific conditions of its approval. During the course of these regulated activities, the contractors are required to keep on-site copies of the approved plan and approval letter.
3. If any sensitive feature(s) (caves, solution cavity, sink hole, etc.) is discovered during construction, all regulated activities near the sensitive feature must be suspended immediately. The appropriate TCEQ regional office must be immediately notified of any sensitive features encountered during construction. Construction activities may not be resumed until the TCEQ has reviewed and approved the appropriate protective measures in order to protect any sensitive feature and the Edwards Aquifer from potentially adverse impacts to water quality.
4. No temporary or permanent hazardous substance storage tank shall be installed within 150 feet of a water supply source, distribution system, well, or sensitive feature.
5. Prior to beginning any construction activity, all temporary erosion and sedimentation (E&S) control measures must be properly installed and maintained in accordance with the approved plans and manufacturers specifications. If inspections indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations. These controls must remain in place until the disturbed areas have been permanently stabilized.
6. Any sediment that escapes the construction site must be collected and properly disposed of before the next rain event to ensure it is not washed into surface streams, sensitive features, etc.
7. Sediment must be removed from the sediment traps or sedimentation basins not later than when it occupies 50% of the basin's design capacity.
8. Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from being discharged offsite.
9. All spoils (excavated material) generated from the project site must be stored on-site with proper E&S controls. For storage or disposal of spoils at another site on the Edwards Aquifer Recharge Zone, the owner of the site must receive approval of a water pollution abatement plan for the placement of fill material or mass grading prior to the placement of spoils at the other site.
10. If portions of the site will have a temporary or permanent cease in construction activity lasting longer than 14 days, soil stabilization in those areas shall be initiated as soon as possible prior to the 14<sup>th</sup> day of inactivity. If activity will resume prior to the 21<sup>st</sup> day, stabilization measures are not required. If drought conditions or inclement weather prevent action by the 14<sup>th</sup> day, stabilization measures shall be initiated as soon as possible.
11. The following records shall be maintained and made available to the TCEQ upon request:
- the dates when major grading activities occur;
  - the dates when construction activities temporarily or permanently cease on a portion of the site; and
  - the dates when stabilization measures are initiated.
12. The holder of any approved Edward Aquifer protection plan must notify the appropriate regional office in writing and obtain approval from the executive director prior to initiating any of the following:

- A. any physical or operational modification of any water pollution abatement structure(s), including but not limited to ponds, dams, berms, sewage treatment plants, and diversionary structures;
- B. any change in the nature or character of the regulated activity from that which was originally approved or a change which would significantly impact the ability of the plan to prevent pollution of the Edwards Aquifer;
- C. any development of land previously identified as undeveloped in the original water pollution abatement plan.

|                                                                                                                                      |                                                                                                                                 |
|--------------------------------------------------------------------------------------------------------------------------------------|---------------------------------------------------------------------------------------------------------------------------------|
| Austin Regional Office<br>12100 Park 35 Circle, Building A<br>Austin, Texas 78753-1808<br>Phone (512) 339-2929<br>Fax (512) 339-3795 | San Antonio Regional Office<br>14250 Judson Road<br>San Antonio, Texas 78233-4480<br>Phone (210) 490-3096<br>Fax (210) 545-4329 |
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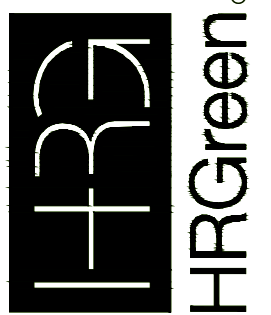
THESE GENERAL CONSTRUCTION NOTES MUST BE INCLUDED ON THE CONSTRUCTION PLANS PROVIDED TO THE CONTRACTOR AND ALL SUBCONTRACTORS.



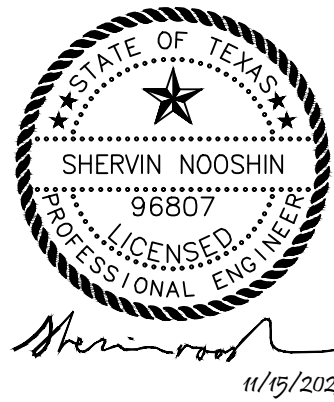
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DEVELOPMENT TX



TCEQ NOTES  
PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC

DRAWN BY: MM/MKM

CHECKED BY: SN

APPROVED BY: \_\_\_\_\_

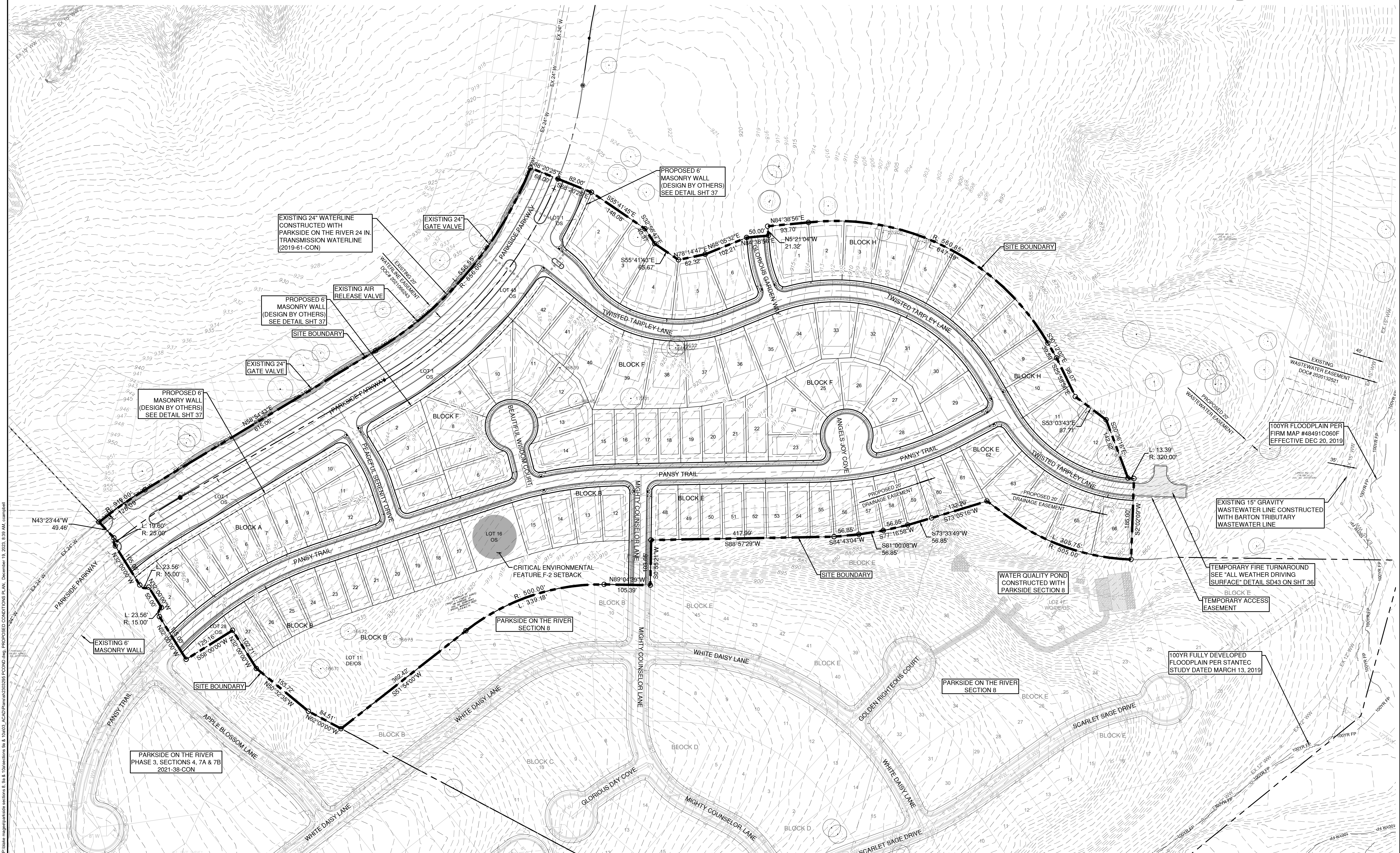
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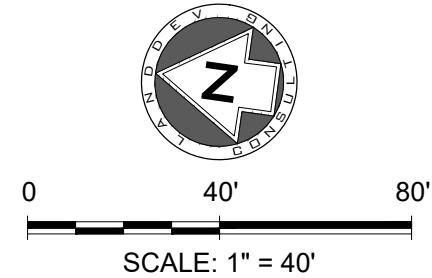
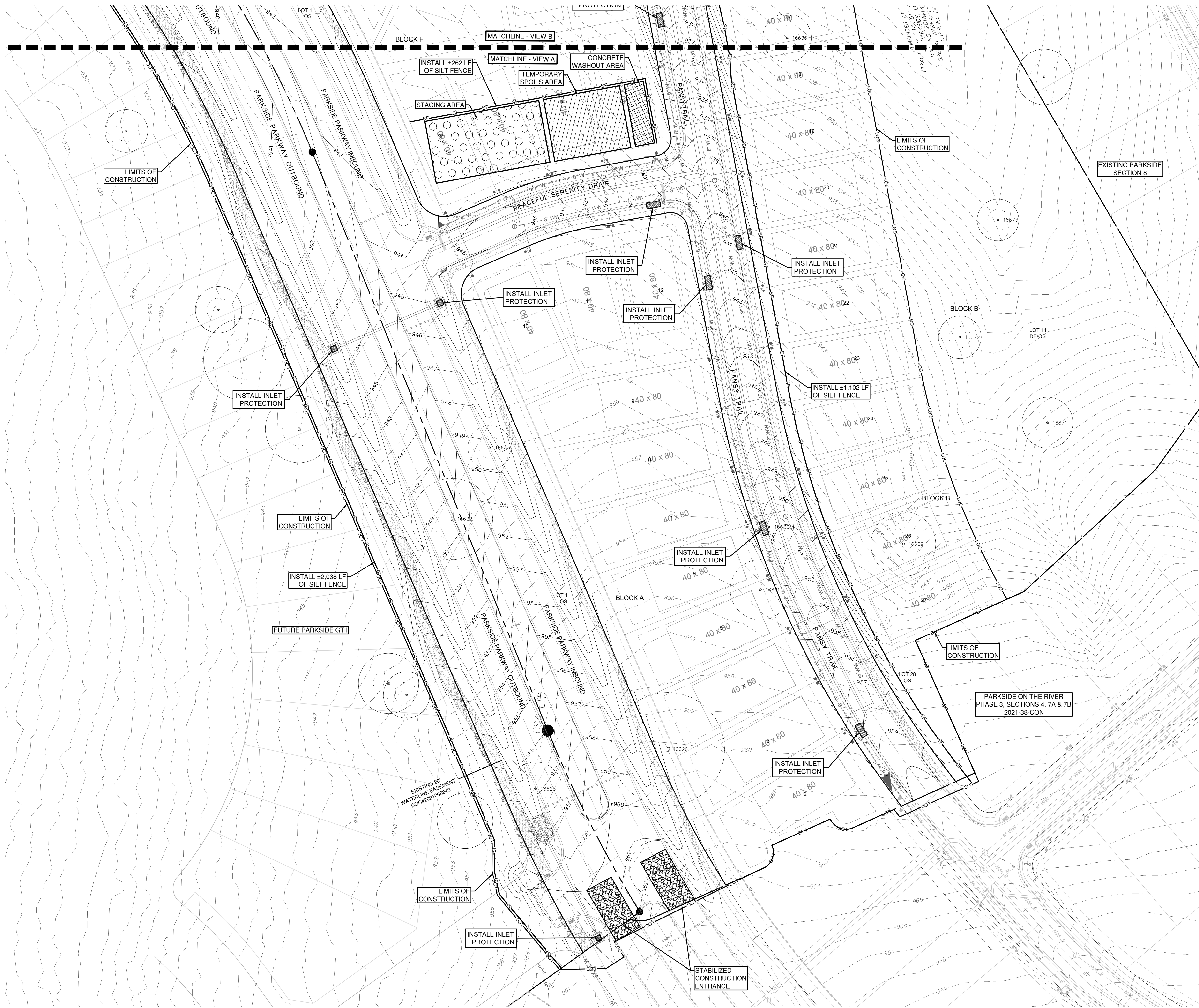








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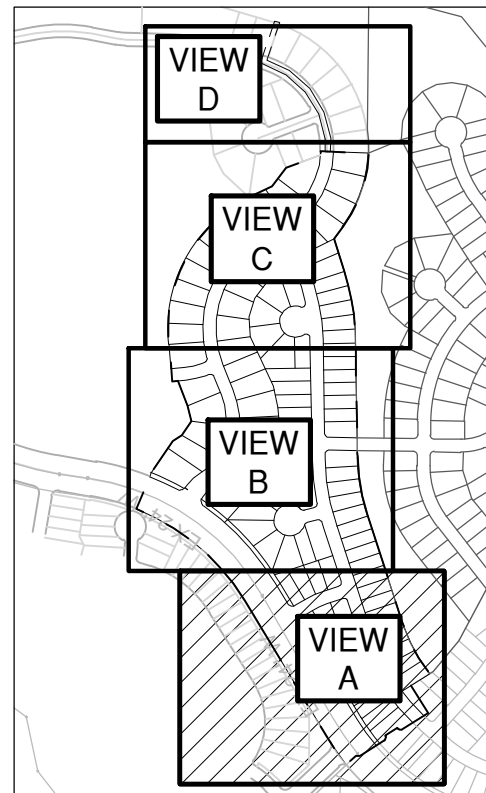


- LEGEND**
- PROPERTY BOUNDARY
  - EASEMENT
  - EXISTING MAJOR CONTOUR
  - EXISTING MINOR CONTOUR
  - PROPOSED MAJOR CONTOUR
  - PROPOSED MINOR CONTOUR
  - LIMITS OF CONSTRUCTION
  - SILT FENCE
  - TREE PROTECTION FENCE
  - 100YR FEMA ZONE A FLOODPLAIN
  - 100YR FULLY DEVELOPED FLOODPLAIN
  - MASONRY WALL

- TREES TO REMAIN - HERITAGE
- TREES TO BE REMOVED - HERITAGE
- TREES TO REMAIN - NON HERITAGE
- TREES TO BE REMOVED - NON HERITAGE
- STABILIZED CONSTRUCTION ENTRANCE
- INLET PROTECTION
- TEMPORARY SPOILS AREA
- CONCRETE WASHOUT AREA
- STOCKPILE
- ROCK BERM

**SEQUENCE OF MAJOR ACTIVITIES:**

- TEMPORARY EROSION AND SEDIMENTATION CONTROL MEASURES TO INCLUDE SILT FENCE, ROCK BERMS, AND STABILIZED CONSTRUCTION ENTRANCES WILL BE INSTALLED ACCORDING TO CONSTRUCTION PLANS AND IN ACCORDANCE WITH THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP).
- TREE PROTECTION MEASURES WILL ALSO BE INSTALLED FOR ALL TREES WITH CONSTRUCTION ACTIVITIES WITHIN CRITICAL ROOT ZONE.
- TEMPORARY SPOILS, CONSTRUCTION STAGING AND CONCRETE WASHOUT AREA WILL BE CONSTRUCTED.
- THE SEDIMENTATION TRAP LOCATED AT THE SOUTHEAST CORNER OF THE SITE WILL BE EXCAVATED AND THE OUTFALL STRUCTURE CONSTRUCTED.
- FILTER FABRIC WILL BE USED TO COVER THE OVERFLOW WEIR TO PREVENT UNFILTERED RUNOFF FROM ENTERING THE LAND DOWNSTREAM.
- AFTER THE SITE IS MASS GRADED, THE ONSITE DRAINAGE, UTILITIES, AND PAVING WILL BE INSTALLED.
- INLET PROTECTION BARRIERS WILL BE INSTALLED AS CONSTRUCTION OF STORM SEWER TAKES PLACE IN ORDER TO PREVENT SEDIMENTS FROM ENTERING THE SYSTEM.
- TEMPORARY SEDIMENTATION BASINS WILL BE CONVERTED TO PERMANENT WATER QUALITY PONDS.
- SEDIMENTATION TRAPS SHALL BE CLEANED OUT AND FILTER MEDIUM INSTALLED CONCURRENT WITH RE-VEGETATION.
- THE DISTURBED AREA TO REMAIN PERVIOUS WILL BE VEGETATED USING THE PROCEDURES DETAILED IN THE CONSTRUCTION PLANS AND ALL TEMPORARY EROSION CONTROL MEASURES WILL BE REMOVED UPON RE-VEGETATION.

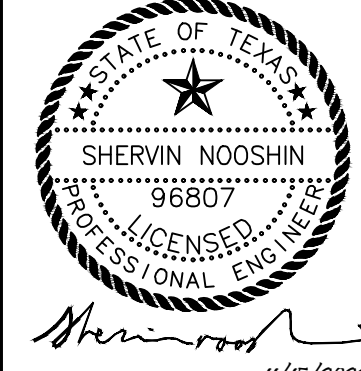


**EROSION & SEDIMENTATION  
CONTROL PLAN A**  
**PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS**  
**GEORGETOWN, WILLIAMSON, TEXAS**

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
SHEET 9 OF 84  
2023-XX-CON



5508 HIGHWAY 290 WEST  
SUITE 150  
MCKINNEY, TX 75069  
817.484.1111  
HRGREEN.COM



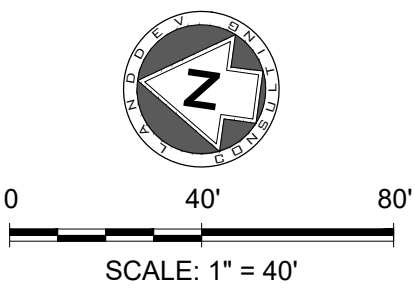
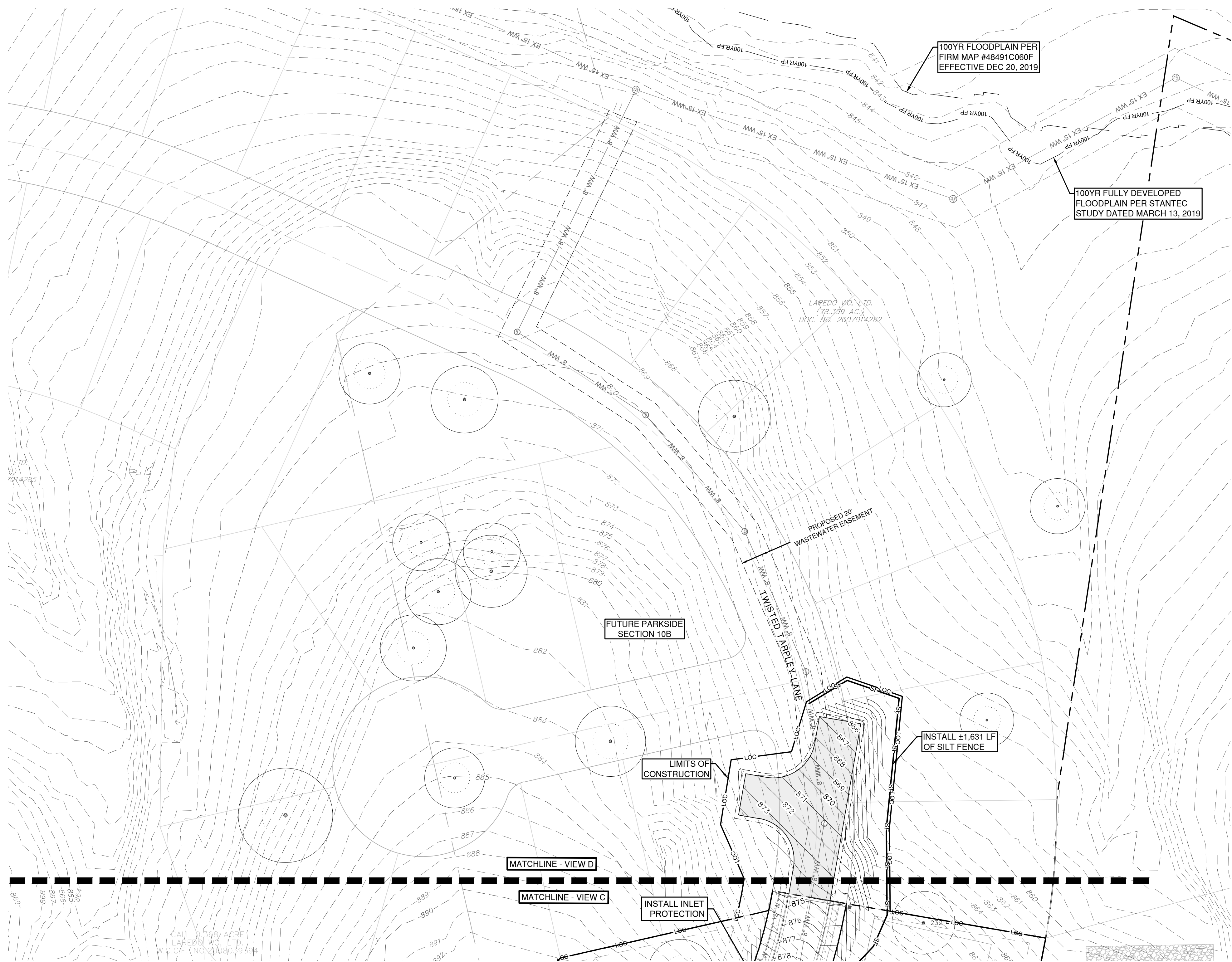






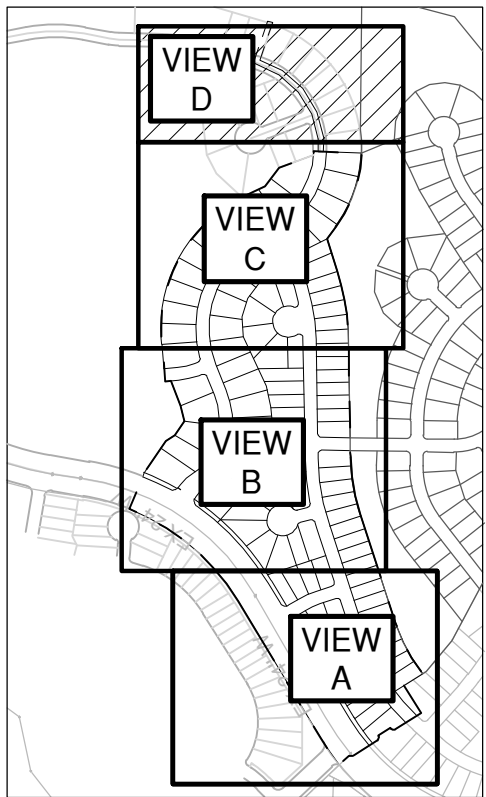






- LEGEND**
- PROPERTY BOUNDARY
  - EASEMENT
  - EXISTING MAJOR CONTOUR
  - EXISTING MINOR CONTOUR
  - PROPOSED MAJOR CONTOUR
  - PROPOSED MINOR CONTOUR
  - LOC
  - SILT FENCE
  - TREE PROTECTION FENCE
  - 100YR FEMA ZONE A FLOODPLAIN
  - 100YR FULLY DEVELOPED FLOODPLAIN
  - MASONRY WALL
  - TREES TO REMAIN - HERITAGE
  - TREES TO BE REMOVED - HERITAGE
  - TREES TO REMAIN - NON HERITAGE
  - TREES TO BE REMOVED - NON HERITAGE
  - STABILIZED CONSTRUCTION ENTRANCE
  - INLET PROTECTION
  - TEMPORARY SPOILS AREA
  - CONCRETE WASHOUT AREA
  - STOCKPILE
  - ROCK BERM

- SEQUENCE OF MAJOR ACTIVITIES:**
- TEMPORARY EROSION AND SEDIMENTATION CONTROL MEASURES TO INCLUDE SILT FENCE, ROCK BERMS, AND STABILIZED CONSTRUCTION ENTRANCES WILL BE INSTALLED ACCORDING TO CONSTRUCTION PLANS AND IN ACCORDANCE WITH THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP).
  - TREE PROTECTION MEASURES WILL ALSO BE INSTALLED FOR ALL TREES WITH CONSTRUCTION ACTIVITIES WITHIN CRITICAL ROOT ZONE.
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  - THE DISTURBED AREA TO REMAIN PERVIOUS WILL BE VEGETATED USING THE PROCEDURES DETAILED IN THE CONSTRUCTION PLANS AND ALL TEMPORARY EROSION CONTROL MEASURES WILL BE REMOVED UPON RE-VEGETATION.



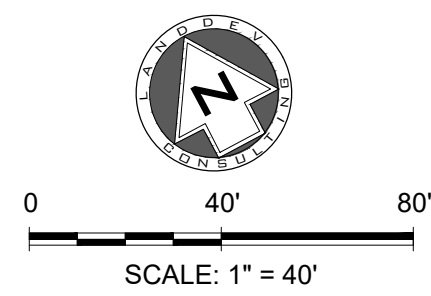
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|--------------------------------------------------------------------------------------------------------|----------------------------------------|
| 811<br>Know what's below.<br>Call before you dig.                                                      |                                        |
| 5508 HIGHWAY 290 WEST<br>SUITE 150<br>DUSTY, TX 78735<br>CALL 811 OR VISIT<br>811TX.GOV<br>HRGREEN.COM | TYPE NO.: 10384<br>TPPLS NO.: 10194101 |
| <b>HRGreen</b><br>DEVELOPMENT TX                                                                       |                                        |
|                                                                                                        |                                        |
| <b>EROSION &amp; SEDIMENTATION<br/>CONTROL PLAN D</b>                                                  |                                        |
| <b>PARKSIDE SECTION 9A &amp; 10A<br/>CONSTRUCTION PLANS</b>                                            |                                        |
| <b>GEORGETOWN, WILLIAMSON, TEXAS</b>                                                                   |                                        |
| DESIGNED BY: CC                                                                                        |                                        |
| DRAWN BY: MM/MKM                                                                                       |                                        |
| CHECKED BY: SN                                                                                         |                                        |
| APPROVED BY:                                                                                           |                                        |
| SHEET <b>12</b> OF <b>84</b>                                                                           |                                        |
| 2023-XX-CON                                                                                            |                                        |



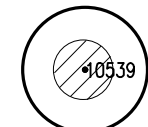




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- LEGEND**
- PROPERTY BOUNDARY
  - EASEMENT
  - EXISTING MAJOR CONTOUR
  - EXISTING MINOR CONTOUR
  - PROPOSED MAJOR CONTOUR
  - PROPOSED MINOR CONTOUR
  - 100YR FEMA ZONE A FLOODPLAIN
  - 100YR FP
  - 100YR FULLY DEVELOPED FLOODPLAIN
  - MASONRY WALL (BY OTHERS)



TREES TO REMAIN - HERITAGE



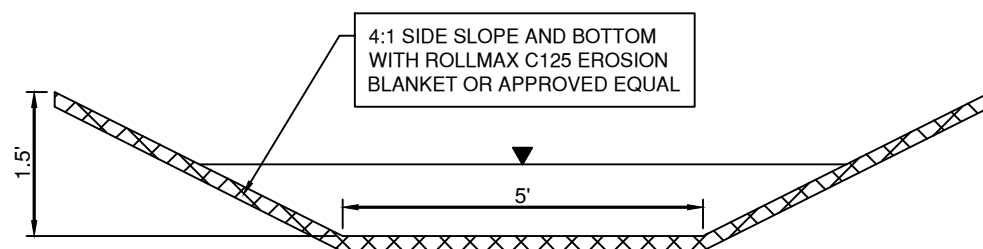
TREES TO REMAIN - NON-HERITAGE

ASPHALT PAVING  
(REFER TO SHEET 37 FOR  
PAVEMENT DESIGN SECTIONS)

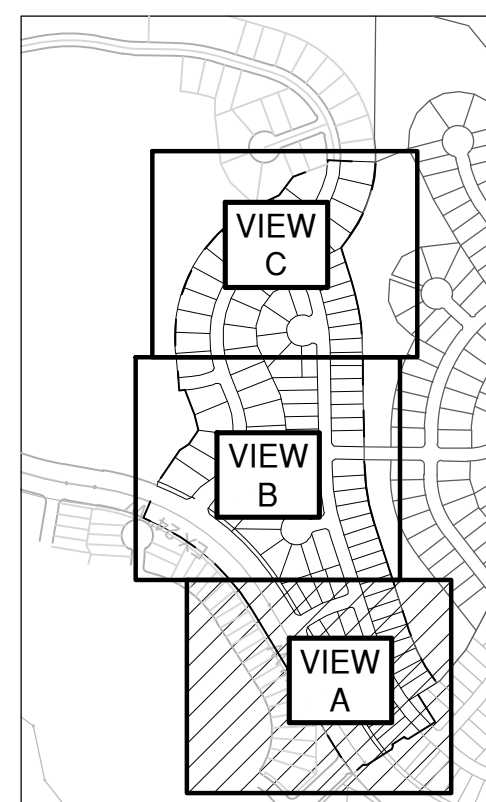
VEGETATIVE FILTER STRIP

**NOTES:**

- FILL SHOULD BE COMPACTED TO AT LEAST 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY TxDOT - 114 - E, AND PER GEOTECHNICAL ENGINEERS RECOMMENDATION.
- FILL WITHIN THE RESIDENTIAL LOT AREAS SHALL BE PLACED ACCORDING TO THE HUD 79G REQUIREMENTS SPECIFICATIONS.
- VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.



**SECTION A-A**  
**ROAD SIDE DRAINAGE SWALE**  
NOT TO SCALE  
VERTICALLY EXAGGERATED



**PAVING & GRADING  
PLAN A**

**PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS**  
GEORGETOWN, WILLIAMSON, TEXAS

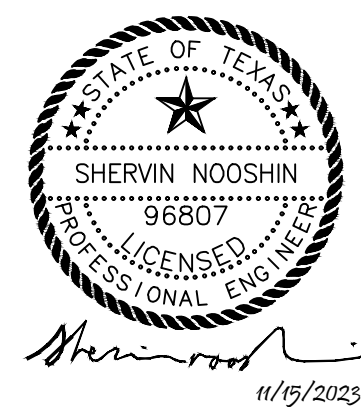
DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY:

SHEET 31 OF 84

2023-XX-CON

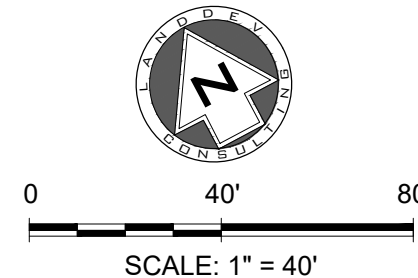
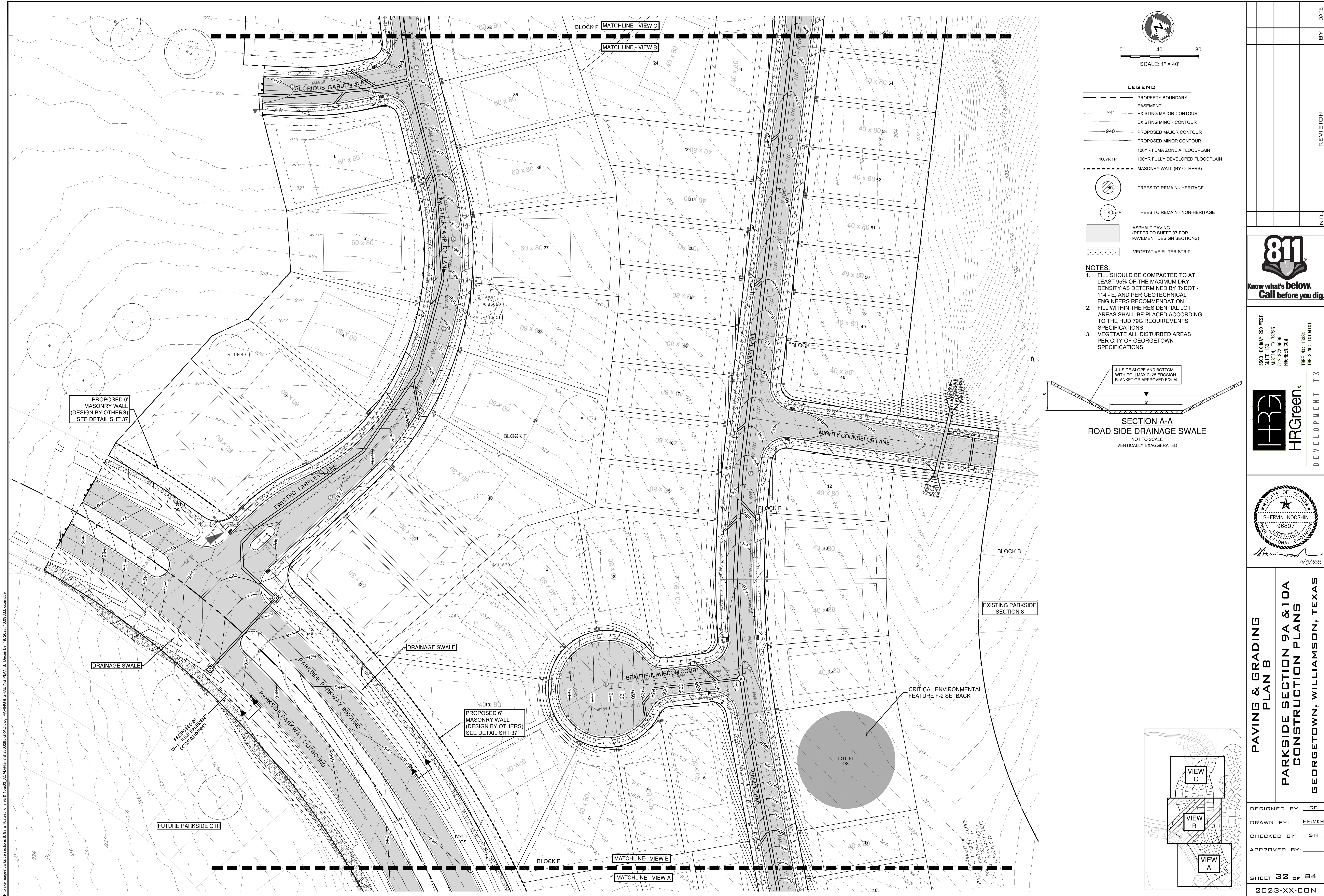


5508 HIGHWAY 290 WEST  
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MUSTIN, TX 78735  
512.466.8888  
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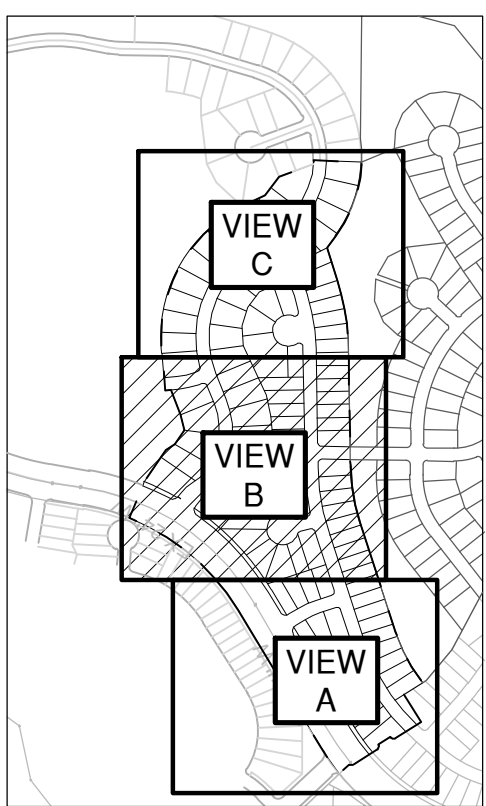
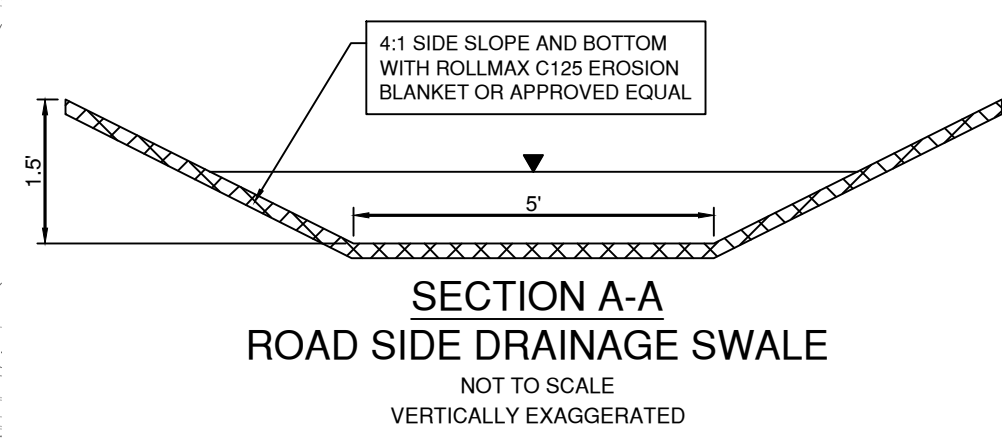


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- LEGEND**
- PROPERTY BOUNDARY
  - EASEMENT
  - EXISTING MAJOR CONTOUR
  - EXISTING MINOR CONTOUR
  - PROPOSED MAJOR CONTOUR
  - PROPOSED MINOR CONTOUR
  - 100YR FEMA ZONE A FLOODPLAIN
  - 100YR FP
  - 100YR FULLY DEVELOPED FLOODPLAIN
  - MASONRY WALL (BY OTHERS)
  - TREES TO REMAIN - HERITAGE
  - TREES TO REMAIN - NON-HERITAGE
  - ASPHALT PAVING  
(REFER TO SHEET 37 FOR  
PAVEMENT DESIGN SECTIONS)
  - VEGETATIVE FILTER STRIP

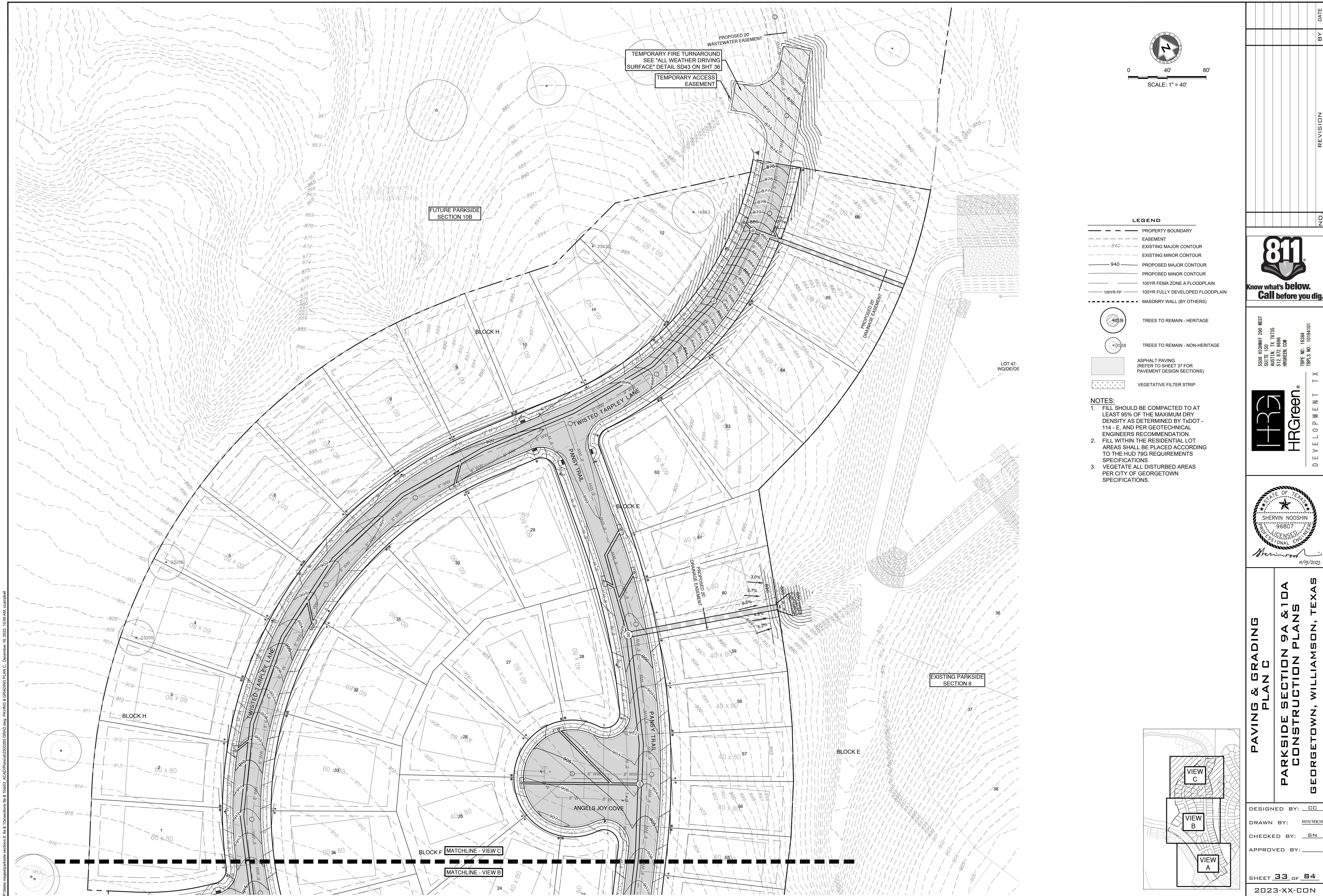
- NOTES:**
- FILL SHOULD BE COMPACTED TO AT LEAST 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY TxDOT - 114 - E, AND PER GEOTECHNICAL ENGINEERS RECOMMENDATION.
  - FILL WITHIN THE RESIDENTIAL LOT AREAS SHALL BE PLACED ACCORDING TO THE HUD 793 REQUIREMENTS SPECIFICATIONS.
  - VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.



|                                                                                           |  |      |
|-------------------------------------------------------------------------------------------|--|------|
| BY                                                                                        |  | DATE |
| REVISION                                                                                  |  | NO.  |
|                                                                                           |  |      |
| 5508 HIGHWAY 290 WEST<br>SUITE 150<br>MCKINNEY, TX 75069<br>817.484.1000<br>WWW.811TX.COM |  |      |
| TYPE NO.: 10384<br>TPLS NO.: 10194101                                                     |  |      |
|                                                                                           |  |      |
|                                                                                           |  |      |
| <b>PAVING &amp; GRADING<br/>PLAN B</b>                                                    |  |      |
| <b>PARKSIDE SECTION 9A &amp; 10A<br/>CONSTRUCTION PLANS</b>                               |  |      |
| <b>GEORGETOWN, WILLIAMSON, TEXAS</b>                                                      |  |      |
| DESIGNED BY: CC                                                                           |  |      |
| DRAWN BY: MM/MKM                                                                          |  |      |
| CHECKED BY: SN                                                                            |  |      |
| APPROVED BY:                                                                              |  |      |
| SHEET <b>32</b> OF <b>84</b>                                                              |  |      |
| 2023-XX-CON                                                                               |  |      |



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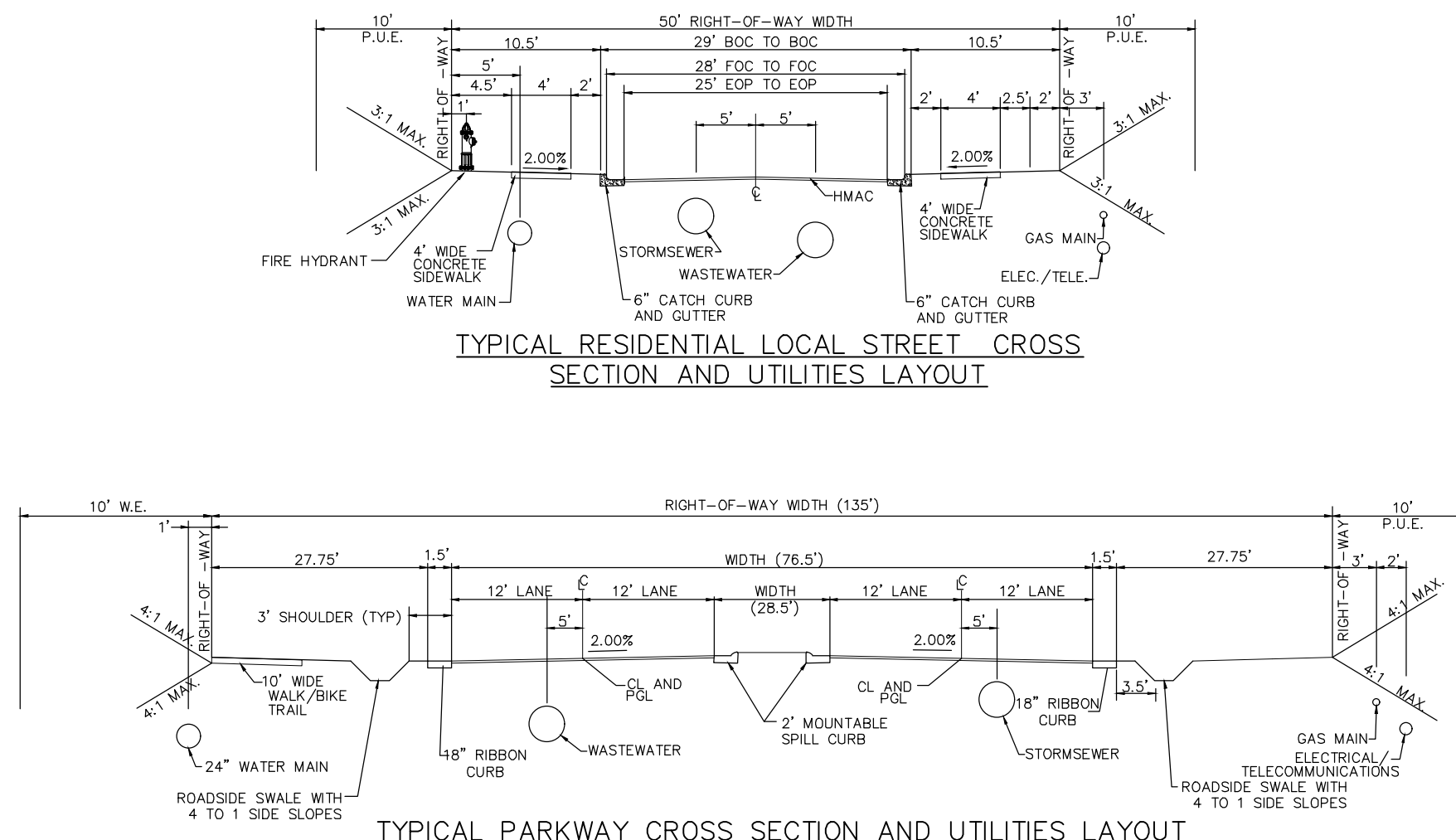
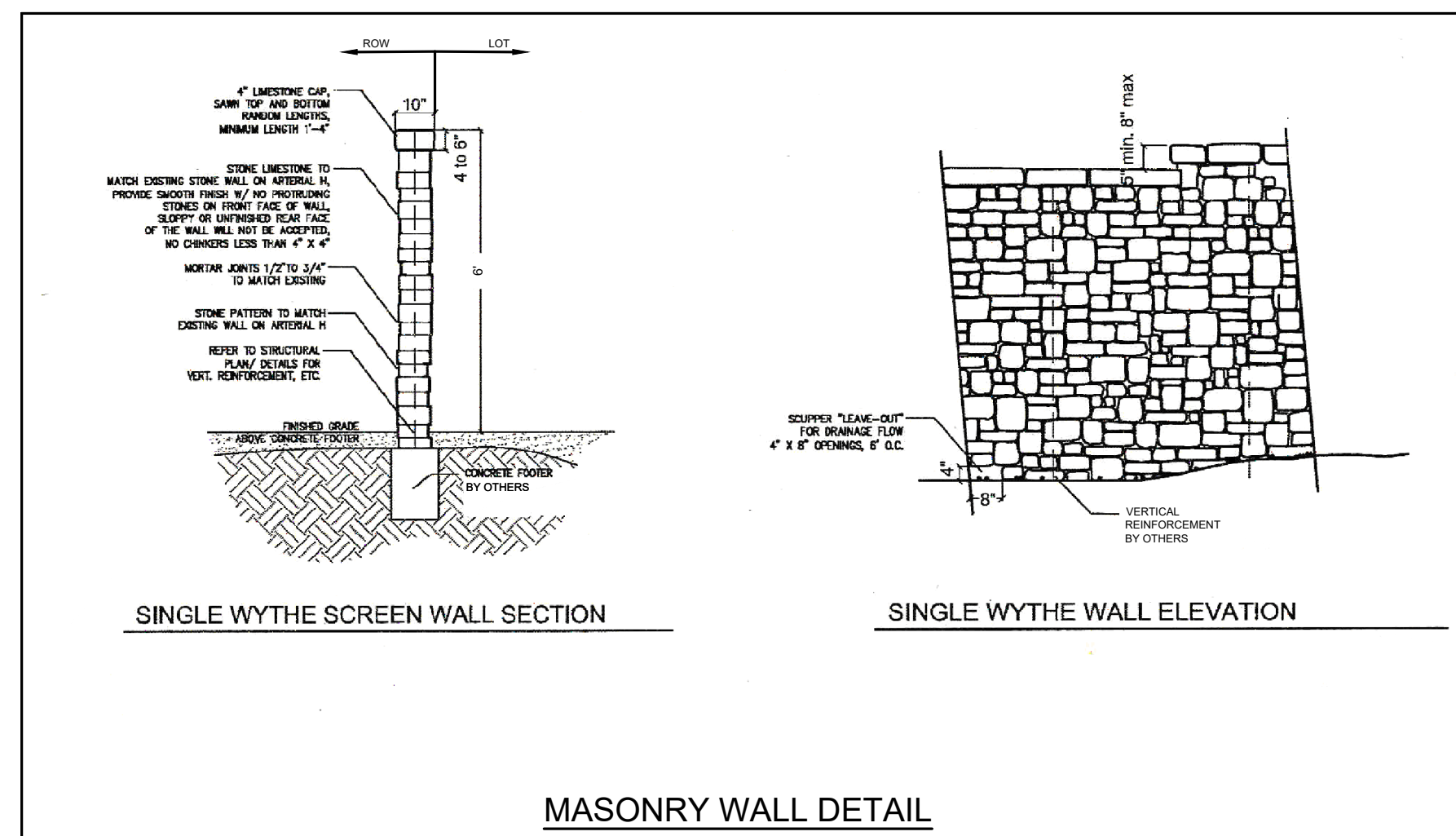
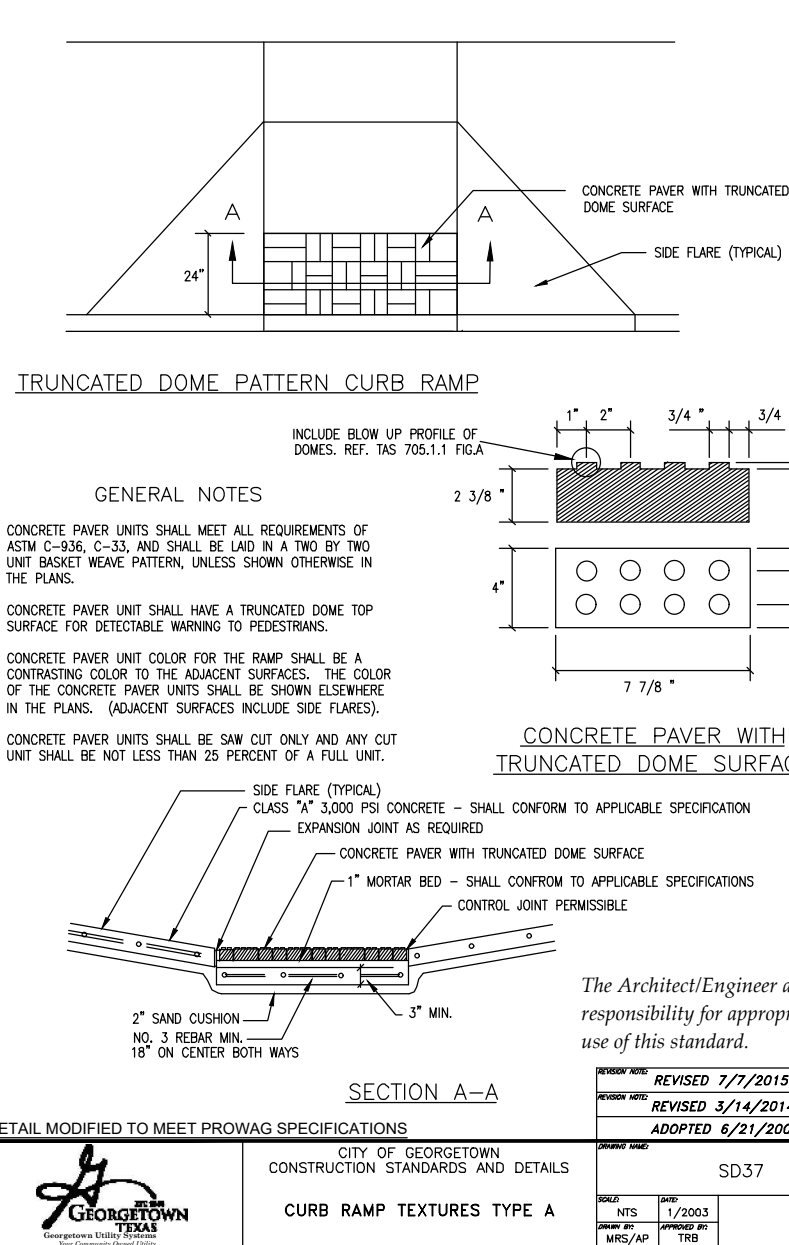
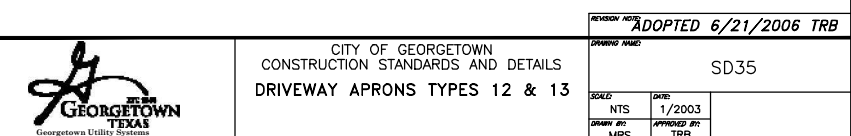
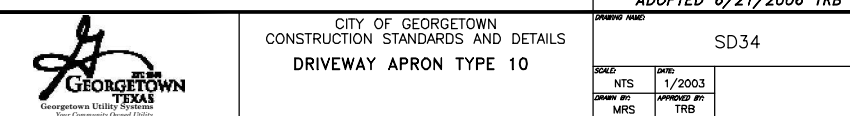
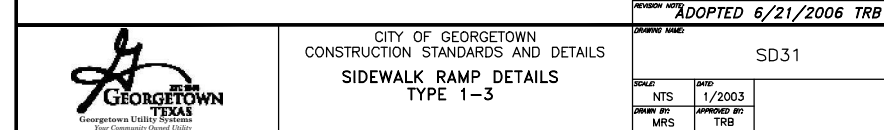
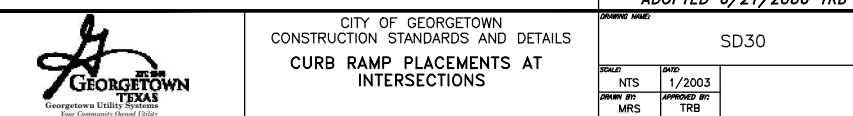
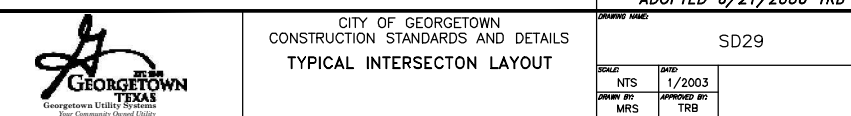






2. COMMERCIAL SIDEWALKS WIDTHS - 6' MINIMUM
3. SIDEWALKS SHALL BE CONCRETE OR EQUIVALENT. FLATTER SLOPES THAT WILL STILL DRAIN PROPERLY ARE REQUIRED.
4. SIDEWALKS SHALL BE FINISHED TO PROVIDE A LIGHT BROWN FINISH NOT OTHERWISE IN THE PLANS.
5. FOR PURPOSES OF WARNING, THE CURB RAMPS SHALL HAVE A LIGHT BROWN FINISH. THERE SHALL BE A SIGNIFICANT CONTRAST WITH THE ADJACENT PEDESTRIAN SURFACES.
6. TEXTURES MAY CONSIST OF PAVERS WITH TRUNCATED DOWD SURFACES. SURFACES ARE REQUIRED TO BE FINISHED TO PROVIDE A CONTRAST WITH SURFACES THAT WOULD ALLOW WATER TO ACCUMULATE ARE PROHIBITED.
7. COLOR CONTRAST: FOR EXAMPLE, MAY BE ACCOMPLISHED WITH COLORED POLYMER FINISHES OR COLORED CONCRETE. FINISHES SHALL PROVIDE A CONTRAST WITH TYPICALLY LIGHT COLORED CONCRETE.
8. ADDITIONAL INFORMATION ON CURB RAMP LOCATION, DRAIN VELOCITY, SLOPE, AND FINISH TO BE FURNISHED BY THE DESIGNER.
9. ACCESSIBILITY STANDARDS (RAMP) REQUIRED AND ADMINISTERED BY THE TEXAS DEPARTMENT OF TRANSPORTATION.
10. RAISED MEDIANS SEPARATE OPPOSING DIRECTIONS OF TRAFFIC AND PROVIDE A REFUGE AREA FOR PEDESTRIANS IF THEY ARE UNABLE TO CROSS THE STREET. THE MINIMUM WIDTH OF A RAISED MEDIAN SHALL BE 10 FEET. THE MINIMUM WIDTH OF A RAISED MEDIAN SHALL BE A MINIMUM OF 5' WIDE. MEDIANS SHOULD BE DESIGNED TO ACCOMMODATE PEDESTRIANS AND BICYCLISTS.
11. ALL SIDEWALK PLANS AND DETAILS SHALL BE SUBMITTED AND APPROVED BY "REGISTERED ACCESSIBILITY SPECIALIST" (RAS).
12. THE TOP OF THE ACCESSIBLE RAMP WITH A SLOPE GREATER THAN 1:20 SHALL BE FINISHED TO CONSIDER THE FOLLOWING:
  - a. SIDEWALKS WITH A SLOPE GREATER THAN 1:20 SHALL BE FINISHED TO BE 6 INCHES OR A HORIZONTAL PROJECTION GREATER THAN 72 INCHES ON THE DOWN SLOPE.
  - b. SIDEWALKS WITH A SLOPE GREATER THAN 1:20 SHALL BE FINISHED TO BE 6 INCHES OR A HORIZONTAL PROJECTION GREATER THAN 72 INCHES ON THE UP SLOPE.
  - c. EXCEPTION IS THAT CURB RAMPS, HANDRAILS ARE NOT REQUIRED ON CURB RAMPS SHALL BE PROVIDED WHERE AN ACCESSIBLE ROUTE CROSS THE STREET.
13. TRAFFIC SIGNAL OR LUMINATION POLES, GROUND BONES, CONTROLLER BOXES, SIGNS, DRAINAGE FACILITIES AND OTHER ITEMS SHALL BE PLACED SO AS NOT TO OBSTRUCT THE ACCESSIBILITY OF THE SIDEWALK OR CROSSWALK.
14. ALL SIDEWALKS SHALL BE DOWNGRDED INTO EXISTING SIDEWALKS, DRIVEWAYS, DRIVEWAYS, NILET BONES, RETAINING WALLS, ETC.
15. SIDEWALKS SHALL BE FINISHED TO PROVIDE RETAINING WALLS, ETC. A VARIATION IS PROVIDED TO TYPICAL.

|                                                                                   |                                                                                            |                                                         |  |
|-----------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------|---------------------------------------------------------|--|
| (PENETRATES) A CURB.                                                              |                                                                                            | PROJECT NO. <b>ADOPTED 6/21/2006 TWS</b><br>DRAWING NO. |  |
|  | CITY OF GEORGETOWN<br>CONSTRUCTION STANDARDS AND DETAILS<br>PEDESTRIAN RAMPS GENERAL NOTES |                                                         |  |
|                                                                                   | SHEET NO. <b>SD28</b>                                                                      |                                                         |  |



- ### RECOMMENDATIONS - PAVEMENT THICKNESS SECTIONS

| Street Classification  | Subgrade Material                      | Hot Mix Asphaltic Concrete, in | Crushed Limestone Base, in | Low Plasticity Sub-Base, in | Time Stabilized Subgrade, in |
|------------------------|----------------------------------------|--------------------------------|----------------------------|-----------------------------|------------------------------|
| Local Streets          | Subgrade PI greater than 20 – Option 1 | 2.0                            | 14                         | -                           | -                            |
|                        | Subgrade PI greater than 20 – Option 2 | 2.0                            | 8                          | 18**                        | -                            |
|                        | Subgrade PI greater than 20 – Option 3 | 2.0                            | 8                          | -                           | 8                            |
|                        | Subgrade PI less than 20               | 2.0                            | 8                          | -                           | -                            |
| Residential Collectors | Subgrade PI greater than 20 – Option 1 | 2.0                            | 15                         | -                           | -                            |
|                        | Subgrade PI greater than 20 – Option 2 | 2.0                            | 10                         | 18**                        | -                            |
|                        | Subgrade PI greater than 20 – Option 3 | 2.0                            | 10                         | -                           | 8                            |
|                        | Subgrade PI less than 20               | 2.0                            | 10                         | -                           | -                            |
| Neighborhood Collector | Subgrade PI greater than 20 – Option 1 | 2.0                            | 17                         | -                           | -                            |
|                        | Subgrade PI greater than 20 – Option 2 | 2.0                            | 11                         | 18**                        | -                            |
|                        | Subgrade PI greater than 20 – Option 3 | 2.0                            | 11                         | -                           | 8                            |
|                        | Subgrade PI less than 20               | 2.0                            | 11                         | -                           | -                            |
| Minor Arterial         | Subgrade PI greater than 20 – Option 1 | 4.0                            | 21                         | -                           | -                            |
|                        | Subgrade PI greater than 20 – Option 2 | 4.0                            | 14                         | 18**                        | -                            |
|                        | Subgrade PI greater than 20 – Option 3 | 4.0                            | 14                         | -                           | 8                            |
|                        | Subgrade PI less than 20               | 4.0                            | 14                         | -                           | -                            |
| Major Arterial         | Subgrade PI greater than 20 – Option 1 | 5.5                            | 24                         | -                           | -                            |
|                        | Subgrade PI greater than 20 – Option 2 | 5.5                            | 16                         | 18**                        | -                            |
|                        | Subgrade PI greater than 20 – Option 3 | 5.5                            | 16                         | -                           | 8                            |
|                        | Subgrade PI less than 20               | 5.5                            | 16                         | -                           | -                            |

1. \*\* - Or the remaining thickness of surface clay. Natural weathered or intact limestone should not be removed to place low plasticity subbase. The low plasticity subbase should consist of on-site soils only.
2. Any expansive fill (PI > 20) placed in the subgrade after boring completion shall be considered expansive subgrade.
3. **Delineation between these different pavement thickness sections should be completed in the field by observation of open utility trenches and the pavement subgrade by the Geotechnical Engineer or his designee.** Given the known variability of surface soils at this site, the Geotechnical Engineer must verify the subgrade before installation of the pavement system can proceed. Multiple site visits may be required depending upon the construction schedule. Finalized distinction between pavement thickness section options can be provided as addendum to this report as these observations are completed. Please contact the Geotechnical Engineer when the utility trenches are open.
4. The subgrade improvement (be it a 50% increase in base course or low plasticity sub-base) should be extended 18 inches beyond the back of the curb line for subgrade PI greater than 20 options.
5. These pavement thickness designs are intended to transfer the load from the anticipated traffic conditions.
6. The responsibility of assigning street classification to the streets in this project is left to the civil engineer.
7. If pavement designs other than those listed above are desired, please contact MLA Geotechnical.

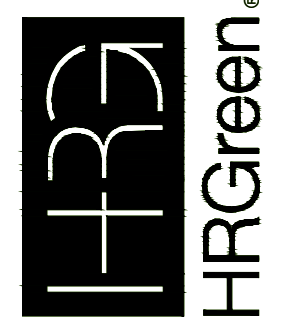
-6

1. CONTRACTOR SHALL REFERENCE "GEOTECHNICAL INVESTIGATION PAVEMENT THICKNESS RECOMMENDATIONS FOR PARKSIDE ON THE RIVER SECTION 8, 9, & 10" CREATED OCTOBER 2023 BY MLA GEOTECHNICAL. ENGINEER'S JOB # 23101123.001 CONTRACTOR IS RESPONSIBLE FOR VERIFYING WITH MLA GEOTECHNICAL.
2. CONTRACTOR TO USE "VIRGIN MIX" TXDOT HMAC WEARING SURFACE 340 TYPE D, WITHOUT RAP OR RAZ. IN THE EVENT THAT THE CITY SPECIFICATIONS OR GEOTECH REPORT CONFLICT WITH THIS THEN THIS NOTE SHALL CONTROL.
3. THE BASE OVERBUILD SHOULD BE EXTENDED A MINIMUM OF 18" BEYOND THE BACK OF CURB. IF LIME TREATMENT IS BEING USED, THE BASE OVERBUILD SHOULD BE EXTENDED 3 FEET BEYOND THE BACK OF THE CURB.
4. AVOID INSTALLATION OF IRRIGATION, PLANTINGS, SILT FENCE, ETC. IN THE BASE OVERBUILD.
5. ALL MATERIAL IMPORTED AND USED FOR ROADWAY FILL AND SUBGRADE SHALL BE LOW P.I MATERIAL UNLESS CONTRACTOR OBTAINS APPROVAL FROM DEVELOPER . IF CONTRACTOR CHOOSES TO BRING IN HIGH PI MATERIAL FOR SUBGRADE, CONTRACTOR IS RESPONSIBLE FOR ALTERNATE PAVEMENT DESIGN PER THE GEOTECH REPORT AND ASSOCIATED COSTS.

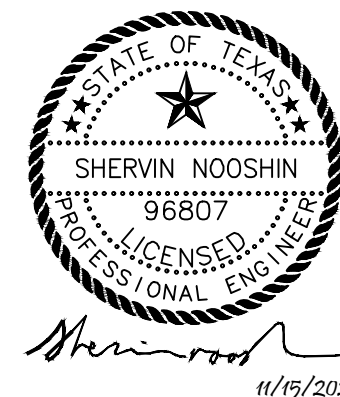
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5508 HIGHWAY 290 WEST  
SUITE 150  
AUSTIN, TX 78735  
512.872.6696  
HGREEN.COM

TBPE NO: 16384



DEVELOPMENT TX



PAVING DETAILS  
SHEET 2 OF 2

PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS

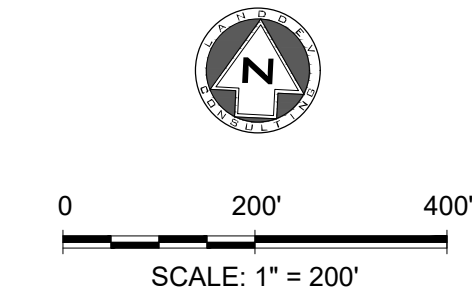
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
  
SHEET 37 OF 84  
  
2023-XX-CON



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| Existing Drainage Conditions |           |               |                 |                       |                  |                      |           |                         |                        |          |                         | Time of Concentration Calculations |            |               |                       |                    |                                     |               |                      |                                   |               |                    |                     |               |                            |
|------------------------------|-----------|---------------|-----------------|-----------------------|------------------|----------------------|-----------|-------------------------|------------------------|----------|-------------------------|------------------------------------|------------|---------------|-----------------------|--------------------|-------------------------------------|---------------|----------------------|-----------------------------------|---------------|--------------------|---------------------|---------------|----------------------------|
| User Inputs                  |           |               |                 |                       | Auto-Calculation |                      | TOC Calcs | Routing Analysis Inputs |                        |          |                         | Contributing Area                  | Sheet Flow |               |                       |                    | Shallow Concentrated Flow (Unpaved) |               |                      | Shallow Concentrated Flow (Paved) |               |                    | Pipe/Channel Flow 1 |               |                            |
| Contributing Area            | Area (sf) | CN (Pervious) | CN (Impervious) | Impervious Cover (sf) | Area (ac)        | Impervious Cover (%) | TOC (min) | Area (sq. mi.)          | Composite Curve Number | Lag Time | Reach Lag (if required) |                                    | Length     | Slope (ft/ft) | Roughness Coefficient | T <sub>sheet</sub> | Length (ft)                         | Slope (ft/ft) | T <sub>unpaved</sub> | Length (ft)                       | Slope (ft/ft) | T <sub>paved</sub> | Length (ft)         | Velocity (ft) | T <sub>channel</sub> (min) |
| E-01                         | 483,952   | 77            | 98              | 0                     | 11.11            | 0.0%                 | 10.16     | 0.01736                 | 77.0                   | 6.10     |                         | E-01                               | 100        | 0.046         | 0.150                 | 6.77               | 733                                 | 0.050         | 3.39                 |                                   |               | 0.00               |                     |               | 0.00                       |
| E-02                         | 49,658    | 77            | 98              | 0                     | 1.14             | 0.0%                 | 7.92      | 0.00178                 | 77.0                   | 4.75     |                         | E-02                               | 100        | 0.044         | 0.150                 | 6.89               | 280                                 | 0.079         | 1.03                 |                                   |               | 0.00               |                     |               | 0.00                       |
| E-03                         | 1,789,009 | 77            | 98              | 61,768                | 41.07            | 3.5%                 | 14.37     | 0.06417                 | 77.7                   | 8.62     |                         | E-03                               | 100        | 0.034         | 0.150                 | 7.64               | 601                                 | 0.062         | 2.49                 |                                   |               | 0.00               | 1523                | 6             | 4.23                       |
| E-04                         | 302,306   | 77            | 98              | 0                     | 6.94             | 0.0%                 | 7.71      | 0.01084                 | 77.0                   | 4.63     |                         | E-04                               | 100        | 0.068         | 0.150                 | 5.79               | 496                                 | 0.071         | 1.92                 |                                   |               | 0.00               |                     |               | 0.00                       |
| E-05                         | 1,572,080 | 77            | 98              | 0                     | 36.09            | 0.0%                 | 18.77     | 0.05639                 | 77.0                   | 11.26    |                         | E-05                               | 100        | 0.013         | 0.150                 | 11.23              | 1728                                | 0.056         | 7.54                 |                                   |               | 0.00               |                     |               | 0.00                       |
| E-06                         | 209,524   | 77            | 98              | 7,967                 | 4.81             | 3.8%                 | 8.10      | 0.00752                 | 77.8                   | 4.86     |                         | E-06                               | 100        | 0.037         | 0.150                 | 7.39               | 151                                 | 0.048         | 0.71                 |                                   |               | 0.00               |                     |               | 0.00                       |



- LEGEND**
- 8.34 - - - EXISTING MINOR CONTOUR
  - 8.35 - - - EXISTING MAJOR CONTOUR
  - 8.34 - - - PROPOSED MINOR CONTOUR
  - 8.35 - - - PROPOSED MAJOR CONTOUR
  - - - BOUNDARY
  - - - EASEMENT
  - 100YR - - - 100 YR PROPOSED CONDITION FLOODPLAIN
  - 100YR - - - 100 YR FEMA ZONE A FLOODPLAIN
  - - - CREEK CENTERLINE
  - - - PROPOSED STORM LINE
  - FD - - - FIRE HYDRANT
  - SV - - - WATER VALVE
  - SM - - - STORM SEWER MAHNOLE
  - WM - - - WASTEWATER MAHNOLE
  - CI - - - CURB INLET
  - (hatched) - TREES TO REMAIN HERITAGE
  - (dotted) - TREES TO REMAIN NON HERITAGE
  - (thick) - DRAINAGE AREA
  - Tc - - - TIME OF CONCENTRATION

| Existing Conditions - Flows & Volumes - Atlas 14 |                  |        |        |        |                 |       |       |        |  |
|--------------------------------------------------|------------------|--------|--------|--------|-----------------|-------|-------|--------|--|
| ID                                               | Peak Flows (cfs) |        |        |        | Volumes (ac-ft) |       |       |        |  |
|                                                  | 2-yr             | 10-yr  | 25-yr  | 100-yr | 2-yr            | 10-yr | 25-yr | 100-yr |  |
| E-01                                             | 19.29            | 41.09  | 57.82  | 88.44  | 1.63            | 3.46  | 4.92  | 7.66   |  |
| E-02                                             | 2.07             | 4.40   | 6.19   | 9.48   | 0.17            | 0.36  | 0.50  | 0.79   |  |
| E-03                                             | 67.28            | 141.94 | 199.29 | 304.14 | 6.22            | 13.05 | 18.46 | 28.63  |  |
| E-04                                             | 12.66            | 26.89  | 37.87  | 57.96  | 1.02            | 2.16  | 3.07  | 4.78   |  |
| E-05                                             | 52.41            | 112.38 | 158.56 | 243.22 | 5.31            | 11.26 | 15.97 | 24.88  |  |
| E-06                                             | 9.03             | 18.89  | 26.46  | 40.31  | 0.73            | 1.53  | 2.17  | 3.36   |  |
| POI-1                                            | 19.29            | 41.09  | 57.82  | 88.44  | 1.63            | 3.46  | 4.92  | 7.66   |  |
| POI-2                                            | 2.07             | 4.40   | 6.19   | 9.48   | 0.17            | 0.36  | 0.50  | 0.79   |  |
| POI-3                                            | 67.28            | 141.94 | 199.29 | 304.14 | 6.22            | 13.05 | 18.46 | 28.63  |  |
| POI-4                                            | 12.66            | 26.89  | 37.87  | 57.96  | 1.02            | 2.16  | 3.07  | 4.78   |  |
| POI-5                                            | 52.41            | 112.38 | 158.56 | 243.22 | 5.31            | 11.26 | 15.97 | 24.88  |  |
| POI-6                                            | 9.03             | 18.89  | 26.46  | 40.31  | 0.73            | 1.53  | 2.17  | 3.36   |  |

| Proposed (Interim) Conditions - Flows & Volumes - Atlas 14 |                  |        |        |        |                 |       |       |        |  |
|------------------------------------------------------------|------------------|--------|--------|--------|-----------------|-------|-------|--------|--|
| ID                                                         | Peak Flows (cfs) |        |        |        | Volumes (ac-ft) |       |       |        |  |
|                                                            | 2-yr             | 10-yr  | 25-yr  | 100-yr | 2-yr            | 10-yr | 25-yr | 100-yr |  |
| P-01                                                       | 10.85            | 19.65  | 26.11  | 37.70  | 0.86            | 1.61  | 2.17  | 3.20   |  |
| P-02                                                       | 3.29             | 5.87   | 7.75   | 11.13  | 0.26            | 0.48  | 0.65  | 0.95   |  |
| P-03                                                       | 134.53           | 246.79 | 329.26 | 477.52 | 12.14           | 22.75 | 30.81 | 45.66  |  |
| P-04                                                       | 11.78            | 24.34  | 33.92  | 51.37  | 0.95            | 1.96  | 2.76  | 4.26   |  |
| P-05                                                       | 47.66            | 100.75 | 141.40 | 215.73 | 4.82            | 10.10 | 14.27 | 22.12  |  |
| P-06                                                       | 1.96             | 3.40   | 4.45   | 6.34   | 0.16            | 0.28  | 0.37  | 0.54   |  |
| POI-1                                                      | 10.85            | 19.65  | 26.11  | 37.70  | 0.86            | 1.61  | 2.17  | 3.20   |  |
| POI-2                                                      | 3.29             | 5.87   | 7.75   | 11.13  | 0.26            | 0.48  | 0.65  | 0.95   |  |
| POI-3                                                      | 129.40           | 239.35 | 321.01 | 467.96 | 12.14           | 22.75 | 30.81 | 45.66  |  |
| POI-4                                                      | 11.78            | 24.34  | 33.92  | 51.37  | 0.95            | 1.96  | 2.76  | 4.26   |  |
| POI-5                                                      | 47.66            | 100.75 | 141.40 | 215.73 | 4.82            | 10.10 | 14.27 | 22.12  |  |
| POI-6                                                      | 1.96             | 3.40   | 4.45   | 6.34   | 0.16            | 0.28  | 0.37  | 0.54   |  |

| Flow & Volume Comparison (Interim - Existing) - Atlas 14 |                  |        |        |        |                 |       |       |        |  |
|----------------------------------------------------------|------------------|--------|--------|--------|-----------------|-------|-------|--------|--|
| ID                                                       | Peak Flows (cfs) |        |        |        | Volumes (ac-ft) |       |       |        |  |
|                                                          | 2-yr             | 10-yr  | 25-yr  | 100-yr | 2-yr            | 10-yr | 25-yr | 100-yr |  |
| POI-1                                                    | -8.44            | -21.44 | -31.71 | -50.74 | -0.77           | -1.85 | -2.75 | -4.46  |  |
| POI-2                                                    | 1.22             | 1.47   | 1.56   | 1.65   | 0.09            | 0.12  | 0.15  | 0.16   |  |
| POI-3                                                    | 62.12            | 97.41  | 121.72 | 163.82 | 5.92            | 9.70  | 12.35 | 17.03  |  |
| POI-4                                                    | -0.88            | -2.55  | -3.95  | -6.59  | -0.07           | -0.20 | -0.31 | -0.52  |  |
| POI-5                                                    | -4.75            | -11.63 | -17.16 | -27.49 | -0.49           | -1.16 | -1.70 | -2.76  |  |
| POI-6                                                    | -7.07            | -15.49 | -22.01 | -33.97 | -0.57           | -1.25 | -1.80 | -2.82  |  |

NOTES:

- PLEASE REFER TO THE DETENTION WAIVER ANALYSIS ADDENDUM 1, SEALED JULY 28, 2023, SUBMITTED WITH THE 2023-22-PP.

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11/19/2023

EXISTING DRAINAGE  
AREA MAP

PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS

GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC

DRAWN BY: MM/MKM

CHECKED BY: SN

APPROVED BY:

SHEET 38 OF 84

2023-XX-CON

REVISION

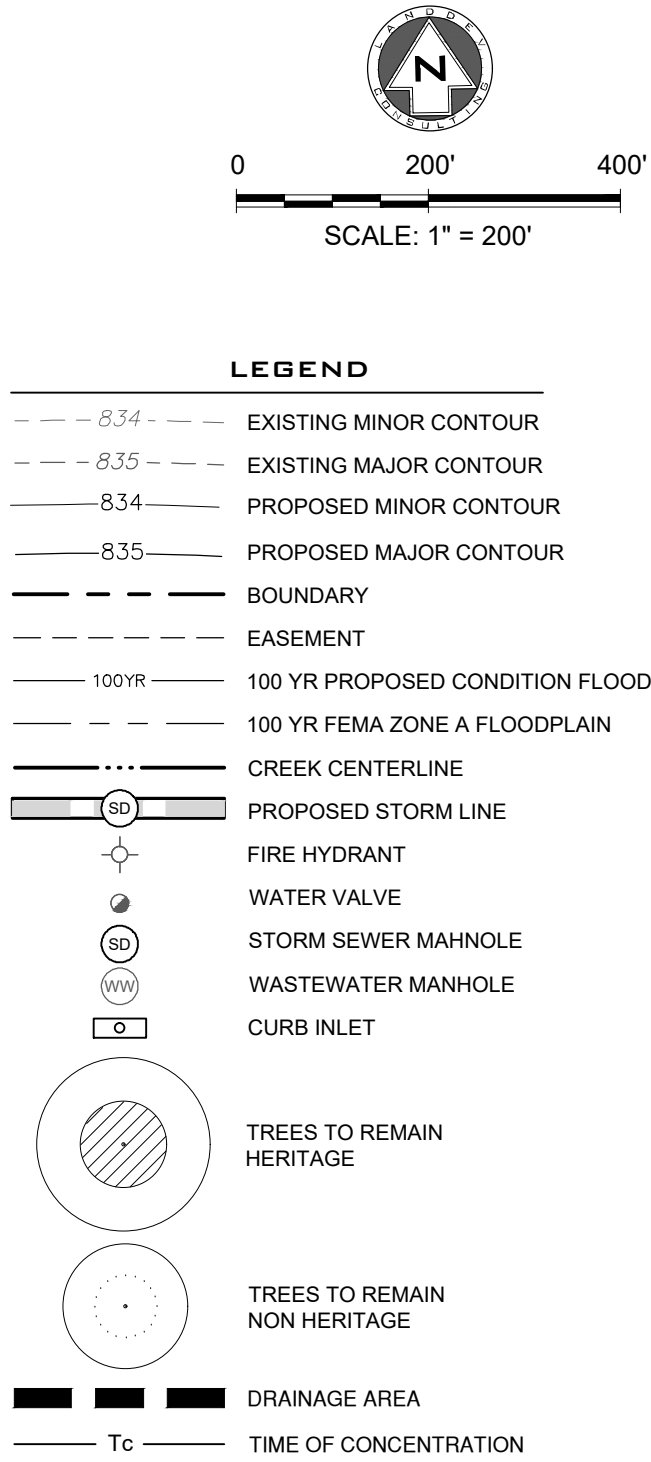
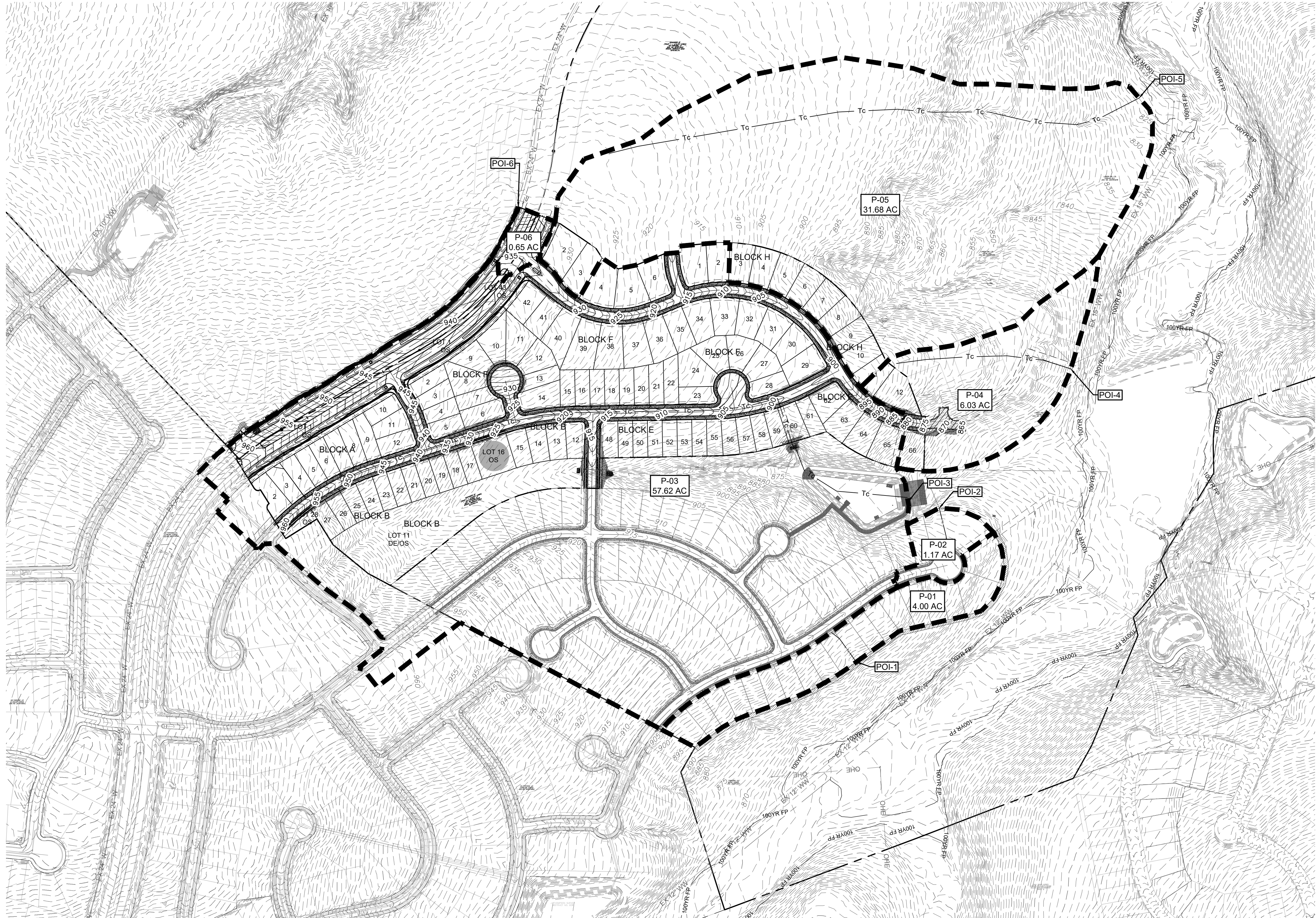
BY

DATE



P:\blake\_magnatoparkside\sections 8 & 10\0303\_ACD\Drawings\2023\2023-PP-DRAINAGE AREA MAP.dwg, December 19, 2023, 10:13 AM, campbell

| Proposed (Interim) Drainage Conditions |           |               |                 |                       |                  |                      |           |                         |                        |          |                         | Time of Concentration Calculations |            |               |                       |                    |                                     |               |                      |                                   |               |                    |                     |               |                            |
|----------------------------------------|-----------|---------------|-----------------|-----------------------|------------------|----------------------|-----------|-------------------------|------------------------|----------|-------------------------|------------------------------------|------------|---------------|-----------------------|--------------------|-------------------------------------|---------------|----------------------|-----------------------------------|---------------|--------------------|---------------------|---------------|----------------------------|
| User Inputs                            |           |               |                 |                       | Auto-Calculation |                      | TOC Calcs | Routing Analysis Inputs |                        |          |                         | Contributing Area                  | Sheet Flow |               |                       |                    | Shallow Concentrated Flow (Unpaved) |               |                      | Shallow Concentrated Flow (Paved) |               |                    | Pipe/Channel Flow 1 |               |                            |
| Contributing Area                      | Area (sf) | CN (Pervious) | CN (Impervious) | Impervious Cover (sf) | Area (ac)        | Impervious Cover (%) | TOC (min) | Area (sq. mi.)          | Composite Curve Number | Lag Time | Reach Lag (if required) |                                    | Length     | Slope (ft/ft) | Roughness Coefficient | T <sub>sheet</sub> | Length (ft)                         | Slope (ft/ft) | T <sub>unpaved</sub> | Length (ft)                       | Slope (ft/ft) | T <sub>paved</sub> | Length (ft)         | Velocity (ft) | T <sub>channel</sub> (min) |
| P-01                                   | 174,240   | 77            | 98              | 83,600                | 4.00             | 48.0%                | 6.64      | 0.00625                 | 87.1                   | 3.99     |                         | P-01                               | 35         | 0.020         | 0.240                 | 5.94               | 96                                  | 0.020         | 0.70                 |                                   |               | 0.00               |                     |               | 0.00                       |
| P-02                                   | 50,965    | 77            | 98              | 27,399                | 1.17             | 53.8%                | 6.66      | 0.00183                 | 88.3                   | 4.00     |                         | P-02                               | 35         | 0.020         | 0.240                 | 5.94               | 98                                  | 0.020         | 0.72                 |                                   |               | 0.00               |                     |               | 0.00                       |
| P-03                                   | 2,509,927 | 77            | 98              | 1,127,550             | 57.62            | 44.9%                | 13.20     | 0.09003                 | 86.4                   | 7.92     |                         | P-03                               | 35         | 0.020         | 0.240                 | 5.94               | 124                                 | 0.020         | 0.91                 |                                   |               | 0.00               | 2287                | 6             | 6.35                       |
| P-04                                   | 262,667   | 77            | 98              | 20,420                | 6.03             | 7.8%                 | 7.71      | 0.00942                 | 78.6                   | 4.63     |                         | P-04                               | 100        | 0.068         | 0.150                 | 5.79               | 496                                 | 0.071         | 1.92                 |                                   |               | 0.00               |                     |               | 0.00                       |
| P-05                                   | 1,379,981 | 77            | 98              | 54,600                | 31.68            | 4.0%                 | 18.77     | 0.04950                 | 77.8                   | 11.26    |                         | P-05                               | 100        | 0.013         | 0.150                 | 11.23              | 1728                                | 0.056         | 7.54                 |                                   |               | 0.00               |                     |               | 0.00                       |
| P-06                                   | 28,314    | 77            | 98              | 17,784                | 0.65             | 62.8%                | 6.00      | 0.00102                 | 90.2                   | 3.60     |                         | P-06                               |            |               |                       | 0.00               |                                     |               | 0.00                 |                                   |               | 0.00               |                     |               | 0.00                       |



| Existing Conditions - Flows & Volumes - Atlas 14 |                  |        |        |        |                 |       |       |        |
|--------------------------------------------------|------------------|--------|--------|--------|-----------------|-------|-------|--------|
| ID                                               | Peak Flows (cfs) |        |        |        | Volumes (ac-ft) |       |       |        |
|                                                  | 2-yr             | 10-yr  | 25-yr  | 100-yr | 2-yr            | 10-yr | 25-yr | 100-yr |
| E-01                                             | 19.29            | 41.09  | 57.82  | 88.44  | 1.63            | 3.46  | 4.92  | 7.66   |
| E-02                                             | 2.07             | 4.40   | 6.19   | 9.48   | 0.17            | 0.36  | 0.50  | 0.79   |
| E-03                                             | 67.28            | 141.94 | 199.29 | 304.14 | 6.22            | 13.05 | 18.46 | 28.63  |
| E-04                                             | 12.66            | 26.89  | 37.87  | 57.96  | 1.02            | 2.16  | 3.07  | 4.78   |
| E-05                                             | 52.41            | 112.38 | 158.56 | 243.22 | 5.31            | 11.26 | 15.97 | 24.88  |
| E-06                                             | 9.03             | 18.89  | 26.46  | 40.31  | 0.73            | 1.53  | 2.17  | 3.36   |
| POI1                                             | 19.29            | 41.09  | 57.82  | 88.44  | 1.63            | 3.46  | 4.92  | 7.66   |
| POI2                                             | 2.07             | 4.40   | 6.19   | 9.48   | 0.17            | 0.36  | 0.50  | 0.79   |
| POI3                                             | 67.28            | 141.94 | 199.29 | 304.14 | 6.22            | 13.05 | 18.46 | 28.63  |
| POI4                                             | 12.66            | 26.89  | 37.87  | 57.96  | 1.02            | 2.16  | 3.07  | 4.78   |
| POI5                                             | 52.41            | 112.38 | 158.56 | 243.22 | 5.31            | 11.26 | 15.97 | 24.88  |
| POI6                                             | 9.03             | 18.89  | 26.46  | 40.31  | 0.73            | 1.53  | 2.17  | 3.36   |

| Proposed (Interim) Conditions - Flows & Volumes - Atlas 14 |                  |        |        |        |                 |       |       |        |
|------------------------------------------------------------|------------------|--------|--------|--------|-----------------|-------|-------|--------|
| ID                                                         | Peak Flows (cfs) |        |        |        | Volumes (ac-ft) |       |       |        |
|                                                            | 2-yr             | 10-yr  | 25-yr  | 100-yr | 2-yr            | 10-yr | 25-yr | 100-yr |
| P-01                                                       | 10.85            | 19.65  | 26.11  | 37.70  | 0.86            | 1.61  | 2.17  | 3.20   |
| P-02                                                       | 3.29             | 5.87   | 7.75   | 11.13  | 0.26            | 0.48  | 0.65  | 0.95   |
| P-03                                                       | 134.53           | 246.79 | 329.26 | 477.52 | 12.14           | 22.75 | 30.81 | 45.66  |
| P-04                                                       | 11.78            | 24.34  | 33.92  | 51.37  | 0.95            | 1.96  | 2.76  | 4.26   |
| P-05                                                       | 47.66            | 100.75 | 141.40 | 215.73 | 4.82            | 10.10 | 14.27 | 22.12  |
| P-06                                                       | 1.96             | 3.40   | 4.45   | 6.34   | 0.16            | 0.28  | 0.37  | 0.54   |
| POI1                                                       | 10.85            | 19.65  | 26.11  | 37.70  | 0.86            | 1.61  | 2.17  | 3.20   |
| POI2                                                       | 3.29             | 5.87   | 7.75   | 11.13  | 0.26            | 0.48  | 0.65  | 0.95   |
| POI3                                                       | 129.40           | 239.35 | 321.01 | 467.96 | 12.14           | 22.75 | 30.81 | 45.66  |
| POI4                                                       | 11.78            | 24.34  | 33.92  | 51.37  | 0.95            | 1.96  | 2.76  | 4.26   |
| POI5                                                       | 47.66            | 100.75 | 141.40 | 215.73 | 4.82            | 10.10 | 14.27 | 22.12  |
| POI6                                                       | 1.96             | 3.40   | 4.45   | 6.34   | 0.16            | 0.28  | 0.37  | 0.54   |

| Flow & Volume Comparison (Interim - Existing) - Atlas 14 |                  |        |        |        |                 |       |       |        |  |  |
|----------------------------------------------------------|------------------|--------|--------|--------|-----------------|-------|-------|--------|--|--|
| ID                                                       | Peak Flows (cfs) |        |        |        | Volumes (ac-ft) |       |       |        |  |  |
|                                                          | 2-yr             | 10-yr  | 25-yr  | 100-yr | 2-yr            | 10-yr | 25-yr | 100-yr |  |  |
| POI1                                                     | -8.44            | -21.44 | -31.71 | -50.74 | -0.77           | -1.85 | -2.75 | -4.46  |  |  |
| POI2                                                     | 1.22             | 1.47   | 1.56   | 1.65   | 0.09            | 0.12  | 0.15  | 0.16   |  |  |
| POI3                                                     | 62.12            | 97.41  | 121.72 | 163.82 | 5.92            | 9.70  | 12.35 | 17.03  |  |  |
| POI4                                                     | -0.88            | -2.55  | -3.95  | -6.59  | -0.07           | -0.20 | -0.31 | -0.52  |  |  |
| POI5                                                     | -4.75            | -11.63 | -17.16 | -27.49 | -0.49           | -1.16 | -1.70 | -2.76  |  |  |
| POI6                                                     | -7.07            | -15.49 | -22.01 | -33.97 | -0.57           | -1.25 | -1.80 | -2.82  |  |  |

NOTES:

- PLEASE REFER TO THE DETENTION WAIVER ANALYSIS ADDENDUM 1, SEALED JULY 28, 2023, SUBMITTED WITH THE 2023-22-PP.

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PROPOSED DRAINAGE  
AREA MAP  
PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_

SHEET 39 OF 84  
2023-XX-CON

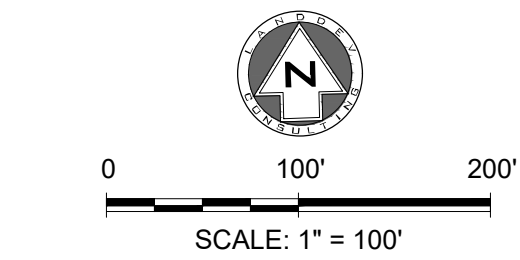
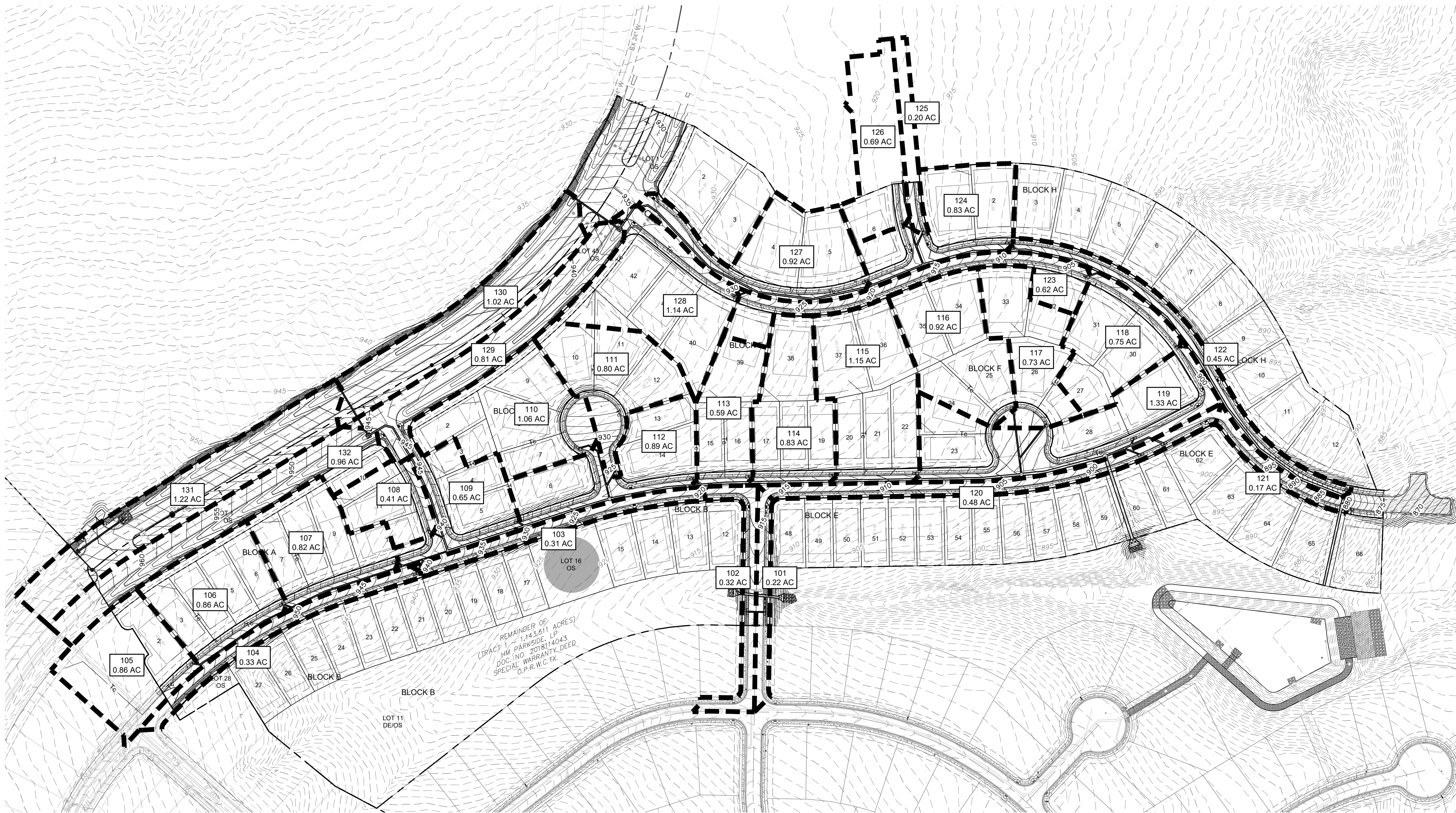
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P:\blake\_magnatoparkside sections 8, 9a & 10a\03\_ACO\Drawings\2302295 phd\DM.dwg, INLET DRAINAGE AREA MAP December 19, 2023 10:14 AM, compall



- LEGEND**
- 8.34 - - - EXISTING MINOR CONTOUR
  - 8.35 - - - EXISTING MAJOR CONTOUR
  - 8.34 - - - PROPOSED MINOR CONTOUR
  - 8.35 - - - PROPOSED MAJOR CONTOUR
  - - - BOUNDARY
  - - - EASEMENT
  - - - 100 YR PROPOSED CONDITION FLOODPLAIN
  - - - 100 YR FEMA ZONE A FLOODPLAIN
  - - - CREEK CENTERLINE
  - - - PROPOSED STORM LINE
  - ⊕ FIRE HYDRANT
  - ⊙ WATER VALVE
  - ⊙ STORM SEWER MAHNOLE
  - ⊙ WASTEWATER MAHNOLE
  - ⊙ CURB INLET
  - ⊙ TREES TO REMAIN HERITAGE
  - ⊙ TREES TO REMAIN NON HERITAGE
  - - - DRAINAGE AREA
  - - - Tc TIME OF CONCENTRATION



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**INLET DRAINAGE AREA MAP**  
**PARKSIDE SECTION 9A & 10A**  
**CONSTRUCTION PLANS**  
**GEORGETOWN, WILLIAMSON, TEXAS**

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_

SHEET **40** OF **84**  
**2023-XX-CON**



P:\baker\magnagrass\side sections 8, 9a & 10a\03\_ACOIDParam\25020505\_pfile\DWG.dwg INLET DRAINAGE CALCULATIONS December 19, 2023 10:14 AM c:\campbell

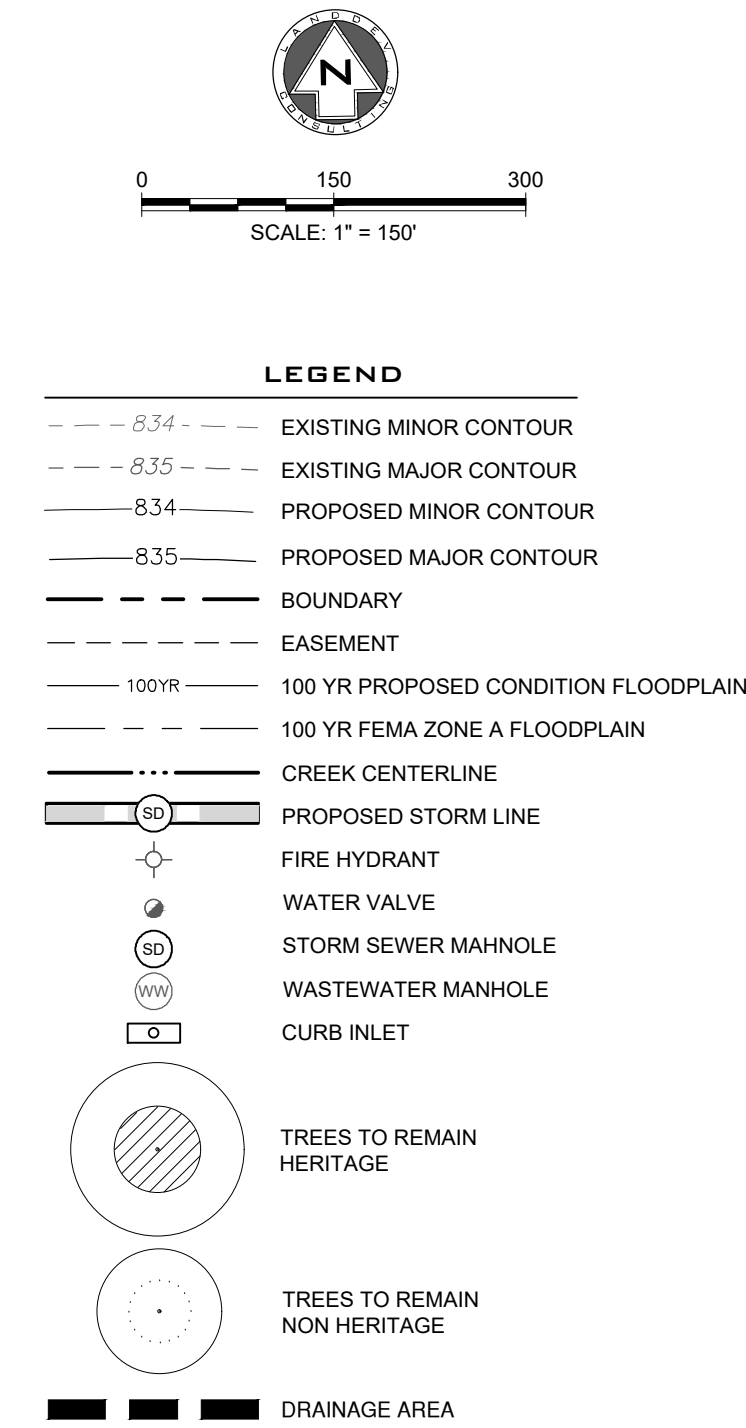
| COG C-Values |      |      |      |      |
|--------------|------|------|------|------|
|              | 2    | 10   | 25   | 100  |
| Impervious   | 0.95 | 0.95 | 0.95 | 0.95 |
| Pervious     | 0.24 | 0.28 | 0.31 | 0.36 |

| COG IDF Curve Values |        |       |        |
|----------------------|--------|-------|--------|
| Year                 | a      | b     | c      |
| 2                    | 106.29 | 16.81 | 0.9076 |
| 10                   | 96.84  | 15.88 | 0.7952 |
| 25                   | 111.07 | 17.23 | 0.7815 |
| 100                  | 129.03 | 17.83 | 0.7625 |

| PARKSIDE ON THE RIVER SECTIONS 9A & 10A<br>RATIONAL METHOD FLOW CALCULATIONS FOR STORM INLETS |             |             |              |           |                        |                         |              |            |          |      |      |      |       |      |      |       | PARKSIDE ON THE RIVER SECTIONS 9A & 10A<br>TIME OF CONCENTRATION CALCULATIONS |      |        |       |       |                   |             |               |                       |                    |                                     |               |                       |                      |             |                 |        |
|-----------------------------------------------------------------------------------------------|-------------|-------------|--------------|-----------|------------------------|-------------------------|--------------|------------|----------|------|------|------|-------|------|------|-------|-------------------------------------------------------------------------------|------|--------|-------|-------|-------------------|-------------|---------------|-----------------------|--------------------|-------------------------------------|---------------|-----------------------|----------------------|-------------|-----------------|--------|
| BASIN LABEL                                                                                   | INLET LABEL | INLET TYPE* | AREA (SQ FT) | AREA (AC) | IMPERVIOUS (LOTS) (SF) | IMPERVIOUS (ROADS) (SF) | IMPERVIOUS % | PERVIOUS % | TC (MIN) | 2-YR |      |      | 10-YR |      |      | 25-YR |                                                                               |      | 100-YR |       |       | Contributing Area | Sheet Flow  |               |                       |                    | Shallow Concentrated Flow (Unpaved) |               |                       |                      | Gutter Flow |                 |        |
|                                                                                               |             |             |              |           |                        |                         |              |            |          | C    | I    | Q    | C     | I    | Q    | C     | I                                                                             | Q    | C      | I     | Q     |                   | Length (ft) | Slope (ft/ft) | Roughness Coefficient | T <sub>sheet</sub> | Length (ft)                         | Slope (ft/ft) | Roughness Coefficient | T <sub>unpaved</sub> | Length (ft) | Velocity (ft/s) | Traped |
| 101                                                                                           | D5          | CSAG        | 9,614        | 0.22      | 0                      | 7,281                   | 76%          | 24%        | 5.0      | 0.78 | 6.48 | 1.11 | 0.79  | 8.64 | 1.50 | 0.79  | 9.84                                                                          | 1.73 | 0.81   | 11.88 | 2.12  | 101               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 102                                                                                           | D6          | CSAG        | 14,019       | 0.32      | 0                      | 10,777                  | 77%          | 23%        | 5.0      | 0.79 | 6.48 | 1.64 | 0.80  | 9.84 | 2.21 | 0.80  | 11.88                                                                         | 2.54 | 0.81   | 11.88 | 3.11  | 102               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 103                                                                                           | A28         | CGRD        | 13,314       | 0.31      | 0                      | 9,866                   | 74%          | 26%        | 5.0      | 0.77 | 6.48 | 1.52 | 0.78  | 8.64 | 2.05 | 0.78  | 9.84                                                                          | 2.36 | 0.80   | 11.88 | 2.89  | 103               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 104                                                                                           | A31         | CGRD        | 14,294       | 0.33      | 0                      | 10,665                  | 75%          | 25%        | 5.0      | 0.77 | 6.48 | 1.64 | 0.78  | 8.64 | 2.21 | 0.79  | 9.84                                                                          | 2.54 | 0.80   | 11.88 | 3.12  | 104               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 105                                                                                           | A24         | CGRD        | 37,429       | 0.86      | 10,800                 | 3,447                   | 38%          | 62%        | 5.8      | 0.51 | 6.27 | 2.75 | 0.54  | 8.38 | 3.85 | 0.55  | 9.57                                                                          | 4.55 | 0.58   | 11.57 | 5.81  | 105               | 35          | 0.02          | 0.24                  | 1.41               | 143                                 | 0.02          | 0.24                  | 4.04                 | 129         | 6               | 0.36   |
| 106                                                                                           | A33         | CGRD        | 37,475       | 0.86      | 14,400                 | 3,554                   | 48%          | 52%        | 5.3      | 0.58 | 6.40 | 3.19 | 0.60  | 8.54 | 4.42 | 0.62  | 9.74                                                                          | 5.16 | 0.64   | 11.76 | 6.50  | 106               | 35          | 0.02          | 0.24                  | 1.41               | 124                                 | 0.02          | 0.24                  | 3.51                 | 137         | 6               | 0.38   |
| 107                                                                                           | A32         | CGRD        | 35,715       | 0.82      | 12,400                 | 3,962                   | 54%          | 46%        | 5.8      | 0.57 | 6.28 | 2.91 | 0.59  | 8.40 | 4.04 | 0.60  | 9.58                                                                          | 4.74 | 0.63   | 11.59 | 5.99  | 107               | 35          | 0.02          | 0.24                  | 1.41               | 138                                 | 0.02          | 0.24                  | 3.90                 | 161         | 6               | 0.45   |
| 108                                                                                           | A55         | CGRD        | 17,722       | 0.41      | 7,400                  | 2,585                   | 50%          | 50%        | 5.4      | 0.54 | 6.38 | 2.06 | 0.57  | 8.52 | 2.87 | 0.58  | 9.71                                                                          | 3.37 | 0.61   | 11.73 | 4.27  | 108               | 35          | 0.02          | 0.24                  | 1.41               | 136                                 | 0.02          | 0.24                  | 3.85                 | 43          | 6               | 0.12   |
| 109                                                                                           | A30         | CGRD        | 28,216       | 0.65      | 9,200                  | 8,085                   | 61%          | 39%        | 7.9      | 0.67 | 5.78 | 2.52 | 0.69  | 7.78 | 3.48 | 0.70  | 8.93                                                                          | 4.06 | 0.72   | 10.83 | 5.06  | 109               | 35          | 0.02          | 0.24                  | 1.41               | 231                                 | 0.02          | 0.24                  | 6.53                 | 0           | 6               | 0.00   |
| 110                                                                                           | A46         | CGRD        | 46,131       | 1.06      | 16,000                 | 4,303                   | 44%          | 56%        | 7.9      | 0.55 | 5.78 | 3.38 | 0.57  | 7.78 | 4.74 | 0.59  | 8.93                                                                          | 5.60 | 0.62   | 10.83 | 7.11  | 110               | 35          | 0.02          | 0.24                  | 1.41               | 227                                 | 0.02          | 0.24                  | 6.42                 | 37          | 6               | 0.10   |
| 111                                                                                           | A47         | CGRD        | 34,993       | 0.80      | 10,800                 | 5,647                   | 49%          | 51%        | 5.0      | 0.57 | 6.48 | 2.99 | 0.59  | 8.64 | 4.13 | 0.61  | 9.84                                                                          | 4.83 | 0.64   | 11.88 | 6.08  | 111               | 35          | 0.02          | 0.24                  | 1.41               | 101                                 | 0.02          | 0.24                  | 2.86                 | 172         | 6               | 0.48   |
| 112                                                                                           | A29         | CGRD        | 38,624       | 0.89      | 10,800                 | 8,298                   | 47%          | 53%        | 5.0      | 0.59 | 6.48 | 3.40 | 0.61  | 8.64 | 4.68 | 0.63  | 9.84                                                                          | 5.47 | 0.65   | 11.88 | 6.87  | 112               | 35          | 0.02          | 0.24                  | 1.41               | 119                                 | 0.02          | 0.24                  | 3.37                 | 0           | 6               | 0.00   |
| 113                                                                                           | A27         | CGRD        | 25,916       | 0.59      | 9,200                  | 1,862                   | 43%          | 57%        | 5.4      | 0.54 | 6.38 | 2.06 | 0.57  | 8.52 | 2.87 | 0.58  | 9.71                                                                          | 3.37 | 0.61   | 11.73 | 4.27  | 113               | 35          | 0.02          | 0.24                  | 1.41               | 136                                 | 0.02          | 0.24                  | 3.85                 | 43          | 6               | 0.12   |
| 114                                                                                           | A26         | CGRD        | 36,011       | 0.83      | 15,200                 | 2,775                   | 50%          | 50%        | 5.5      | 0.59 | 6.34 | 3.12 | 0.61  | 8.47 | 4.30 | 0.63  | 9.66                                                                          | 5.03 | 0.65   | 11.68 | 6.32  | 114               | 35          | 0.02          | 0.24                  | 1.41               | 136                                 | 0.02          | 0.24                  | 3.85                 | 93          | 6               | 0.26   |
| 115                                                                                           | A25         | CGRD        | 50,257       | 1.15      | 19,600                 | 2,775                   | 45%          | 55%        | 5.9      | 0.56 | 6.26 | 4.01 | 0.58  | 8.37 | 5.98 | 0.59  | 9.55                                                                          | 6.56 | 0.62   | 11.55 | 8.30  | 115               | 35          | 0.02          | 0.24                  | 1.41               | 148                                 | 0.02          | 0.24                  | 4.19                 | 93          | 6               | 0.26   |
| 116                                                                                           | A41         | CGRD        | 39,970       | 0.92      | 14,200                 | 590                     | 37%          | 63%        | 5.6      | 0.50 | 6.32 | 2.91 | 0.53  | 8.45 | 4.09 | 0.55  | 9.63                                                                          | 4.83 | 0.58   | 11.64 | 6.18  | 116               | 35          | 0.02          | 0.24                  | 1.41               | 145                                 | 0.02          | 0.24                  | 4.10                 | 35          | 6               | 0.10   |
| 117                                                                                           | A42         | CGRD        | 31,777       | 0.73      | 12,000                 | 2,572                   | 46%          | 54%        | 7.9      | 0.57 | 5.79 | 2.39 | 0.59  | 7.80 | 3.34 | 0.60  | 8.95                                                                          | 3.94 | 0.63   | 10.86 | 4.99  | 117               | 35          | 0.02          | 0.24                  | 1.41               | 225                                 | 0.02          | 0.24                  | 6.36                 | 30          | 6               | 0.08   |
| 118                                                                                           | B31         | CGRD        | 32,720       | 0.75      | 10,400                 | 3,879                   | 44%          | 56%        | 5.5      | 0.55 | 6.34 | 2.62 | 0.57  | 8.47 | 3.64 | 0.59  | 9.66                                                                          | 4.28 | 0.62   | 11.68 | 5.42  | 118               | 35          | 0.02          | 0.24                  | 1.41               | 136                                 | 0.02          | 0.24                  | 3.85                 | 95          | 6               | 0.26   |
| 119                                                                                           | A39         | CSAG        | 57,752       | 1.33      | 11,600                 | 20,014                  | 55%          | 45%        | 5.2      | 0.63 | 6.44 | 5.37 | 0.65  | 8.59 | 7.37 | 0.66  | 9.79                                                                          | 8.57 | 0.68   | 11.82 | 10.70 | 119               | 35          | 0.02          | 0.24                  | 1.41               | 102                                 | 0.02          | 0.24                  | 2.88                 | 309         | 6               | 0.86   |
| 120                                                                                           | A38         | CSAG        | 21,094       | 0.48      | 0                      | 15,743                  | 75%          | 25%        | 5.0      | 0.77 | 6.48 | 2.42 | 0.78  | 8.64 | 3.26 | 0.79  | 9.84                                                                          | 3.75 | 0.80   | 11.88 | 4.60  | 120               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 121                                                                                           | B30         | CGRD        | 7,272        | 0.17      | 0                      | 5,654                   | 78%          | 22%        | 5.0      | 0.79 | 6.48 | 0.86 | 0.80  | 8.64 | 1.16 | 0.81  | 9.84                                                                          | 1.33 | 0.82   | 11.88 | 1.62  | 121               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 122                                                                                           | B29         | CGRD        | 19,669       | 0.45      | 0                      | 14,785                  | 75%          | 25%        | 5.0      | 0.77 | 6.48 | 2.26 | 0.78  | 8.64 | 3.06 | 0.79  | 9.84                                                                          | 3.51 | 0.80   | 11.88 | 4.31  | 122               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 123                                                                                           | B32         | CGRD        | 26,891       | 0.62      | 4,800                  | 12,099                  | 63%          | 37%        | 5.0      | 0.69 | 6.48 | 2.74 | 0.70  | 8.64 | 3.74 | 0.71  | 9.84                                                                          | 4.33 | 0.73   | 11.88 | 5.36  | 123               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 124                                                                                           | B33         | CGRD        | 36,204       | 0.83      | 11,200                 | 7,131                   | 51%          | 49%        | 5.2      | 0.60 | 6.43 | 3.20 | 0.62  | 8.58 | 4.41 | 0.63  | 9.77                                                                          | 5.15 | 0.66   | 11.80 | 6.46  | 124               | 35          | 0.02          | 0.24                  | 1.41               | 128                                 | 0.02          | 0.24                  | 3.62                 | 59          | 6               | 0.16   |
| 125                                                                                           | B41         | CGRD        | 8,824        | 0.20      | 0                      | 6,517                   | 74%          | 26%        | 5.0      | 0.76 | 6.48 | 1.00 | 0.77  | 8.64 | 1.36 | 0.78  | 9.84                                                                          | 1.56 | 0.80   | 11.88 | 1.92  | 125               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 126                                                                                           | B42         | CGRD        | 30,002       | 0.69      | 7,600                  | 8,922                   | 55%          | 45%        | 5.0      | 0.63 | 6.48 | 2.82 | 0.65  | 8.64 | 3.86 | 0.66  | 9.84                                                                          | 4.49 | 0.68   | 11.88 | 5.60  | 126               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 127                                                                                           | B34         | CGRD        | 40,275       | 0.92      | 8,800                  | 9,934                   | 47%          | 53%        | 5.6      | 0.57 | 6.32 | 3.33 | 0.59  | 8.44 | 4.62 | 0.61  | 9.63                                                                          | 5.41 | 0.63   | 11.64 | 6.83  | 127               | 35          | 0.02          | 0.24                  | 1.41               | 136                                 | 0.02          | 0.24                  | 3.85                 | 128         | 6               | 0.36   |
| 128                                                                                           | B35         | CGRD        | 49,565       | 1.14      | 13,200                 | 6,390                   | 40%          | 60%        | 5.9      | 0.52 | 6.23 | 3.69 | 0.54  | 8.34 | 5.17 | 0.56  | 9.52                                                                          | 6.10 | 0.59   | 11.52 | 7.77  | 128               | 35          | 0.02          | 0.24                  | 1.41               | 140                                 | 0.02          | 0.24                  | 3.96                 | 207         | 6               | 0.58   |
| 129                                                                                           | B26         | ASAG        | 35,273       | 0.81      | 0                      | 21,441                  | 61%          | 39%        | 5.0      | 0.67 | 6.48 | 3.52 | 0.69  | 8.64 | 4.81 | 0.70  | 9.84                                                                          | 5.57 | 0.72   | 11.88 | 6.91  | 129               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 130                                                                                           | B27         | ASAG        | 44,282       | 1.02      | 0                      | 20,430                  | 54%          | 46%        | 5.0      | 0.57 | 6.48 | 3.74 | 0.58  | 8.64 | 5.18 | 0.61  | 9.84                                                                          | 6.05 | 0.63   | 11.88 | 7.84  | 130               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 131                                                                                           | A54         | ASAG        | 53,289       | 1.22      | 0                      | 28,584                  | 54%          | 46%        | 5.0      | 0.62 | 6.48 | 4.92 | 0.64  | 8.64 | 6.76 | 0.65  | 9.84                                                                          | 7.86 | 0.68   | 11.88 | 9.83  | 131               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |
| 132                                                                                           | A53         | ASAG        | 41,852       | 0.96      | 1,800                  | 20,688                  | 54%          | 46%        | 5.0      | 0.62 | 6.48 | 3.87 | 0.64  | 8.64 | 5.31 | 0.65  | 9.84                                                                          | 6.18 | 0.68   | 11.88 | 7.73  | 132               |             |               |                       | 0.00               |                                     |               |                       | 0.00                 |             |                 | 0.00   |

| Curb Inlets On Grade Calculation Summary: 25 year |           |                       |                         |                          |           |       |       |                   |                   |                          |      |      |      |         |        |        |                     |         |         |                |             |      |                         |            |                 |
|---------------------------------------------------|-----------|-----------------------|-------------------------|--------------------------|-----------|-------|-------|-------------------|-------------------|--------------------------|------|------|------|---------|--------|--------|---------------------|---------|---------|----------------|-------------|------|-------------------------|------------|-----------------|
| Drainage Area No.                                 | Inlet No. | Q <sub>25</sub> (cfs) | Q <sub>pass</sub> (cfs) | Q <sub>total</sub> (cfs) | Slope (%) | n     | Ku    | Street Width (ft) | Crown Height (ft) | Inlet Depression, a (ft) | K0   | K1   | K2   | y0 (ft) | a      | b      | Flow Spread, T (ft) | H1 (ft) | H2 (ft) | Qa/La (cfs/ft) | Length (ft) | Qa   | Q <sub>pass</sub> (cfs) | % Captured | Bypass to Inlet |
| 103                                               | A28       | 2.36                  | 0.00                    | 2.36                     | 3.40%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.27    | 0.0714 | 0.0026 | 4.42                | 0.68    | 0.42    | 0.72           | 10.00       | 7.18 |                         | 100%       | 102             |
| 104                                               | A31       | 2.54                  | 0.00                    | 2.54                     | 4.00%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.27    | 0.0714 | 0.0026 | 4.41                | 0.68    | 0.42    | 0.72           | 10.00       | 7.18 |                         | 100%       | 103             |
| 105                                               | A24       | 4.55                  | 0.00                    | 4.55                     | 3.90%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.32    | 0.0714 | 0.0026 | 5.67                | 0.74    | 0.42    | 0.78           | 10.00       | 7.77 |                         | 100%       | 106             |
| 106                                               | A33       | 5.16                  | 0.00                    | 5.16                     | 3.90%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.34    | 0.0714 | 0.0026 | 6.00                | 0.75    | 0.42    | 0.79           | 10.00       | 7.91 |                         | 100%       | 107             |
| 107                                               | A32       | 4.74                  | 0.00                    | 4.74                     | 4.00%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.33    | 0.0714 | 0.0026 | 5.74                | 0.74    | 0.42    | 0.78           | 10.00       | 7.80 |                         | 100%       | 109             |
| 108                                               | A55       | 2.68                  | 0.00                    | 2.68                     | 3.50%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.28    | 0.0714 | 0.0026 | 4.63                | 0.69    | 0.42    | 0.73           | 10.00       | 7.29 |                         | 100%       | 109             |
| 109                                               | A30       | 4.06                  | 0.00                    | 4.06                     | 6.00%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.29    | 0.0714 | 0.0026 | 4.92                | 0.71    | 0.42    | 0.74           | 10.00       | 7.42 |                         | 100%       | 112             |
| 110                                               | A46       | 5.60                  | 0.00                    | 5.60                     | 6.50%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.32    | 0.0714 | 0.0026 | 5.55                | 0.73    | 0.42    | 0.77           | 10.00       | 7.71 |                         | 100%       | 112             |
| 111                                               | A47       | 4.83                  | 0.00                    | 4.83                     | 6.40%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.30    | 0.0714 | 0.0026 | 5.22                | 0.72    | 0.42    | 0.76           | 10.00       | 7.57 |                         | 100%       | 112             |
| 112                                               | A29       | 5.47                  | 0.00                    | 5.47                     | 3.30%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.35    | 0.0714 | 0.0026 | 6.40                | 0.77    | 0.42    | 0.81           | 10.00       | 8.08 |                         | 100%       | 113             |
| 113                                               | A27       | 3.37                  | 0.00                    | 3.37                     | 3.30%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.30    | 0.0714 | 0.0026 | 5.16                | 0.72    | 0.42    | 0.75           | 10.00       | 7.54 |                         | 100%       | 114             |
| 114                                               | A26       | 5.03                  | 0.00                    | 5.03                     | 3.00%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.35    | 0.0714 | 0.0026 | 6.29                | 0.77    | 0.42    | 0.80           | 10.00       | 8.04 |                         | 100%       | 115             |
| 115                                               | A25       | 6.56                  | 0.00                    | 6.56                     | 2.00%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.41    | 0.0714 | 0.0026 | 7.95                | 0.82    | 0.42    | 0.87           | 10.00       | 8.66 |                         | 100%       | 119             |
| 116                                               | A41       | 4.83                  | 0.00                    | 4.83                     | 3.00%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.34    | 0.0714 | 0.0026 | 6.18                | 0.76    | 0.42    | 0.80           | 10.00       | 7.99 |                         | 100%       | 117             |
| 117                                               | A42       | 3.94                  | 0.00                    | 3.94                     | 3.00%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.32    | 0.0714 | 0.0026 | 5.64                | 0.74    | 0.42    | 0.78           | 10.00       | 7.75 |                         | 100%       | 119             |
| 118                                               | B31       | 4.28                  | 0.00                    | 4.28                     | 0.50%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.44    | 0.0714 | 0.0026 | 9.32                | 0.86    | 0.42    | 0.91           | 10.00       | 9.07 |                         | 100%       | 119             |
| 121                                               | B30       | 1.33                  | 0.00                    | 1.33                     | 8.90%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.19    | 0.0714 | 0.0026 | 2.93                | 0.60    | 0.42    | 0.64           | 10.00       | 6.41 |                         | 100%       | O5              |
| 122                                               | B29       | 3.51                  | 0.00                    | 3.51                     | 10.10%    | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.25    | 0.0714 | 0.0026 | 4.17                | 0.67    | 0.42    | 0.71           | 10.00       | 7.06 |                         | 100%       | O5              |
| 123                                               | B32       | 4.33                  | 0.00                    | 4.33                     | 3.40%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.32    | 0.0714 | 0.0026 | 5.71                | 0.74    | 0.42    | 0.78           | 10.00       | 7.79 |                         | 100%       | 118             |
| 124                                               | B33       | 5.15                  | 0.00                    | 5.15                     | 3.90%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.34    | 0.0714 | 0.0026 | 5.99                | 0.75    | 0.42    | 0.79           | 10.00       | 7.91 |                         | 100%       | 122             |
| 125                                               | B41       | 1.56                  | 0.00                    | 1.56                     | 1.60%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.26    | 0.0714 | 0.0026 | 4.36                | 0.68    | 0.42    | 0.72           | 10.00       | 7.15 |                         | 100%       | 124             |
| 126                                               | B42       | 4.49                  | 0.00                    | 4.49                     | 0.50%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.45    | 0.0714 | 0.0026 | 9.63                | 0.87    | 0.42    | 0.91           | 10.00       | 9.15 |                         | 100%       | 124             |
| 127                                               | B34       | 5.41                  | 0.00                    | 5.41                     | 4.10%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.34    | 0.0714 | 0.0026 | 6.06                | 0.76    | 0.42    | 0.79           | 10.00       | 7.94 |                         | 100%       | 124             |
| 128                                               | B35       | 6.10                  | 0.00                    | 6.10                     | 3.80%     | 0.015 | 0.560 | 28.00             | 0.500             | 0.42                     | 2.85 | 0.50 | 3.03 | 0.36    | 0.0714 | 0.0026 | 6.52                | 0.77    | 0.42    | 0.81           | 10.00       | 8.13 |                         | 100%       | 123             |





1 - FOR THE GEORGETOWN TSS REMOVAL REQUIREMENT, WE CONSIDER 85% OF TSS REMOVAL FOR THE DRAINAGE AREA THAT DRAINS TOWARD THE BATCH DETENTION PONDS



## VEGETATIVE FILTER STRIP - VFS-01

[illegible]

|                                                                                                                                                                                                                                 |                                                                                                                                                                                                                                                                   |                                                            |       |
|---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------------------------------------------------------|-------|
| Texas Commission on Environmental Quality                                                                                                                                                                                       |                                                                                                                                                                                                                                                                   | Parks on the River                                         |       |
| TSS Removal Calculations 04-20-2009                                                                                                                                                                                             |                                                                                                                                                                                                                                                                   | Project Name: Section 9A & 10A<br>Date Prepared: 11/7/2023 |       |
| Additional information is provided for cells with a red triangle in the upper right corner. Place the cursor over the cell.<br>Text shown in blue indicates location of instructions in the Technical Guidance Manual - RG-348. |                                                                                                                                                                                                                                                                   |                                                            |       |
| <b>Characters shown in red are data entry fields.</b>                                                                                                                                                                           |                                                                                                                                                                                                                                                                   |                                                            |       |
| <b>Characters shown in black (Bold)</b> are calculated fields. Changes to these fields will remove the equations used in the spreadsheet.                                                                                       |                                                                                                                                                                                                                                                                   |                                                            |       |
| <b>1. The Required Load Reduction for the total project:</b>                                                                                                                                                                    | Calculations from RG-348                                                                                                                                                                                                                                          | Pages 3-27 to 3-30                                         |       |
| Page 3-29 Equation 3.3: $L_{\text{TOTAL PROJECT}} = L_{\text{TSS}} \times 27(A_p + P)$                                                                                                                                          |                                                                                                                                                                                                                                                                   |                                                            |       |
| where:                                                                                                                                                                                                                          | $L_{\text{TOTAL PROJECT}}$ = Required TSS removal resulting from the proposed development = 80% of increased load<br>$A_p$ = Net increase in impervious area for the project<br>$P$ = Average annual precipitation, inches                                        |                                                            |       |
| Site Data: Determine Required Load Removal Based on the Entire Project                                                                                                                                                          |                                                                                                                                                                                                                                                                   |                                                            |       |
| Total project area included in plan                                                                                                                                                                                             | County = <b>Williamson*</b>                                                                                                                                                                                                                                       | acres                                                      |       |
| Predevelopment impervious area within the limits of the plan                                                                                                                                                                    | <b>0.00</b>                                                                                                                                                                                                                                                       | acres                                                      |       |
| Total post-development impervious area within the limits of the plan                                                                                                                                                            | <b>18.30</b>                                                                                                                                                                                                                                                      | acres                                                      |       |
| Total post-development impervious cover fraction                                                                                                                                                                                | <b>0.48</b>                                                                                                                                                                                                                                                       |                                                            |       |
|                                                                                                                                                                                                                                 | <b>32</b>                                                                                                                                                                                                                                                         | inches                                                     |       |
|                                                                                                                                                                                                                                 | $L_{\text{TOTAL PROJECT}} =$                                                                                                                                                                                                                                      | <b>14266</b>                                               | lbs.  |
| * The values entered in these fields should be for the total project area.                                                                                                                                                      |                                                                                                                                                                                                                                                                   |                                                            |       |
| Number of drainage basins / outfalls areas leaving the plan area =                                                                                                                                                              | <b>5</b>                                                                                                                                                                                                                                                          |                                                            |       |
| <b>2. Drainage Basin Parameters (This information should be provided for each basin):</b>                                                                                                                                       |                                                                                                                                                                                                                                                                   |                                                            |       |
| Drainage Basin/Outfall Area No. =                                                                                                                                                                                               | <b>VF01-01</b>                                                                                                                                                                                                                                                    |                                                            |       |
| Total drainage basin/outfall area =                                                                                                                                                                                             | <b>4.00</b>                                                                                                                                                                                                                                                       | acres                                                      |       |
| Predevelopment impervious area within drainage basin/outfall area =                                                                                                                                                             | <b>0.00</b>                                                                                                                                                                                                                                                       | acres                                                      |       |
| Post-development impervious area within drainage basin/outfall area =                                                                                                                                                           | <b>1.92</b>                                                                                                                                                                                                                                                       | acres                                                      |       |
| Post-development impervious fraction within drainage basin/outfall area =                                                                                                                                                       | <b>0.48</b>                                                                                                                                                                                                                                                       |                                                            |       |
|                                                                                                                                                                                                                                 | $L_{\text{BASIN THIS BASIN}} =$                                                                                                                                                                                                                                   | <b>1671</b>                                                | lbs.  |
| <b>3. Indicate the proposed BMP Code for this basin.</b>                                                                                                                                                                        |                                                                                                                                                                                                                                                                   |                                                            |       |
| Proposed BMP = <b>Vegetated Filter Strips</b>                                                                                                                                                                                   |                                                                                                                                                                                                                                                                   |                                                            |       |
| Removal efficiency =                                                                                                                                                                                                            | <b>85</b>                                                                                                                                                                                                                                                         | percent                                                    |       |
| <b>4. Calculate Maximum TSS Load Removed (<math>R_L</math>) for this Drainage Basin by the selected BMP Type.</b>                                                                                                               |                                                                                                                                                                                                                                                                   |                                                            |       |
| RG-348 Page 3-33 Equation 3.7: $L_R = (\text{BMP Efficiency}) \times P \times (A_i \times 34.6 + A_p \times 0.54)$                                                                                                              |                                                                                                                                                                                                                                                                   |                                                            |       |
| where:                                                                                                                                                                                                                          | $A_i$ = Total On-Site drainage area in the BMP catchment area<br>$A_p$ = Impervious area proposed in the BMP catchment area<br>$A_r$ = PerVIOUS area remaining in the BMP catchment area<br>$L_R$ = TSS Load removed from this catchment area by the proposed BMP |                                                            |       |
|                                                                                                                                                                                                                                 | $A_i =$                                                                                                                                                                                                                                                           | <b>4.00</b>                                                | acres |
|                                                                                                                                                                                                                                 | $A_p =$                                                                                                                                                                                                                                                           | <b>1.92</b>                                                | acres |
|                                                                                                                                                                                                                                 | $A_r =$                                                                                                                                                                                                                                                           | <b>2.08</b>                                                | acres |
|                                                                                                                                                                                                                                 | $L_R =$                                                                                                                                                                                                                                                           | <b>1838</b>                                                | lbs   |
| <b>5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area</b>                                                                                                                                        |                                                                                                                                                                                                                                                                   |                                                            |       |
| Desired $f_{\text{THIS BASIN}}$                                                                                                                                                                                                 | <b>1838</b>                                                                                                                                                                                                                                                       | lbs.                                                       |       |
| F =                                                                                                                                                                                                                             | <b>1.00</b>                                                                                                                                                                                                                                                       |                                                            |       |

| Texas Commission on Environmental Quality                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          |                     | Parkspide on the River         |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|---------------------|--------------------------------|--------------------|-----------------------------------|---------------------|--|---------------------------------------|-------|-------|---------------------------------------------------------------------|------|-------|------------------------------------------------------------------------|-------|-------|---------------------------------------------------------------------------|------|--|------------------------|------|--------|--------------------------|-------|------|
| TSS Removal Calculations 04-20-2009                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                |                     | Project Name: Section 9A & 10A |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
|                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                    |                     | Date Prepared: 11/7/2023       |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| <p>Additional Information is provided for cells with a red triangle in the upper right corner. Place the cursor over the cell. Text shown in blue indicates location of instructions in the Technical Guidance Manual - RG-348.</p> <p>Characters shown in red are data entry fields.</p> <p>Characters shown in black (Bold) are calculated fields. Changes to these fields will remove the equations used in the spreadsheet.</p>                                                                                                                                                                                                                                                                                                                                                                                                                                |                     |                                |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| 1. The Required Load Reduction for the total project:                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              |                     | Calculations from RG-348       | Pages 5-27 to 3-30 |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| <p>Page 3-29 Equation 3.3: <math>L_{40} = 27(A_{40} \times P)</math></p> <p>where: <math>L_{40}</math> TOTAL PROJECT = Required TSS removal resulting from the proposed development = 80% of increased load</p> <p><math>A_{40}</math> = Net increase in impervious area for the project</p> <p>P = Average annual precipitation, inches</p>                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                       |                     |                                |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| <p>Site Data: Determine Required Load Removal Based on the Entire Project</p> <table border="1"> <thead> <tr> <th></th> <th>County = Williamson</th> <th></th> </tr> </thead> <tbody> <tr> <td>Total project area included in plan =</td> <td>34.42</td> <td>acres</td> </tr> <tr> <td>Predevelopment impervious area within the limits of the plan =</td> <td>0.00</td> <td>acres</td> </tr> <tr> <td>Total post-development impervious area within the limits of the plan =</td> <td>10.39</td> <td>acres</td> </tr> <tr> <td>Total post-development impervious cover fraction =</td> <td>0.48</td> <td></td> </tr> <tr> <td>P =</td> <td>32</td> <td>inches</td> </tr> <tr> <td><math>L_{40}</math> TOTAL PROJECT =</td> <td>14266</td> <td>lbs.</td> </tr> </tbody> </table> <p>* The values entered in these fields should be for the total project area.</p> |                     |                                |                    |                                   | County = Williamson |  | Total project area included in plan = | 34.42 | acres | Predevelopment impervious area within the limits of the plan =      | 0.00 | acres | Total post-development impervious area within the limits of the plan = | 10.39 | acres | Total post-development impervious cover fraction =                        | 0.48 |  | P =                    | 32   | inches | $L_{40}$ TOTAL PROJECT = | 14266 | lbs. |
|                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                    | County = Williamson |                                |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| Total project area included in plan =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              | 34.42               | acres                          |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| Predevelopment impervious area within the limits of the plan =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                     | 0.00                | acres                          |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| Total post-development impervious area within the limits of the plan =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                             | 10.39               | acres                          |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| Total post-development impervious cover fraction =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 | 0.48                |                                |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| P =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                | 32                  | inches                         |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| $L_{40}$ TOTAL PROJECT =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                           | 14266               | lbs.                           |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| <p>Number of drainage basins / outfalls areas leaving the plan area = 5</p>                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                        |                     |                                |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| 2. Drainage Basin Parameters (This information should be provided for each basin):                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |                     |                                |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| <table border="1"> <thead> <tr> <th>Drainage Basin/Outfall Area No. =</th> <th>BP-02</th> <th></th> </tr> </thead> <tbody> <tr> <td>Total drainage basin/outfall area =</td> <td>2.90</td> <td>acres</td> </tr> <tr> <td>Predevelopment impervious area within drainage basin/outfall area =</td> <td>0.00</td> <td>acres</td> </tr> <tr> <td>Post-development impervious area within drainage basin/outfall area =</td> <td>1.48</td> <td>acres</td> </tr> <tr> <td>Post-development impervious fraction within drainage basin/outfall area =</td> <td>0.51</td> <td></td> </tr> <tr> <td><math>L_{40}</math> BASIN BASIN =</td> <td>1288</td> <td>lbs.</td> </tr> </tbody> </table>                                                                                                                                                                              |                     |                                |                    | Drainage Basin/Outfall Area No. = | BP-02               |  | Total drainage basin/outfall area =   | 2.90  | acres | Predevelopment impervious area within drainage basin/outfall area = | 0.00 | acres | Post-development impervious area within drainage basin/outfall area =  | 1.48  | acres | Post-development impervious fraction within drainage basin/outfall area = | 0.51 |  | $L_{40}$ BASIN BASIN = | 1288 | lbs.   |                          |       |      |
| Drainage Basin/Outfall Area No. =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                  | BP-02               |                                |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| Total drainage basin/outfall area =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                | 2.90                | acres                          |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| Predevelopment impervious area within drainage basin/outfall area =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                | 0.00                | acres                          |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| Post-development impervious area within drainage basin/outfall area =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              | 1.48                | acres                          |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| Post-development impervious fraction within drainage basin/outfall area =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          | 0.51                |                                |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |
| $L_{40}$ BASIN BASIN =                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                             | 1288                | lbs.                           |                    |                                   |                     |  |                                       |       |       |                                                                     |      |       |                                                                        |       |       |                                                                           |      |  |                        |      |        |                          |       |      |



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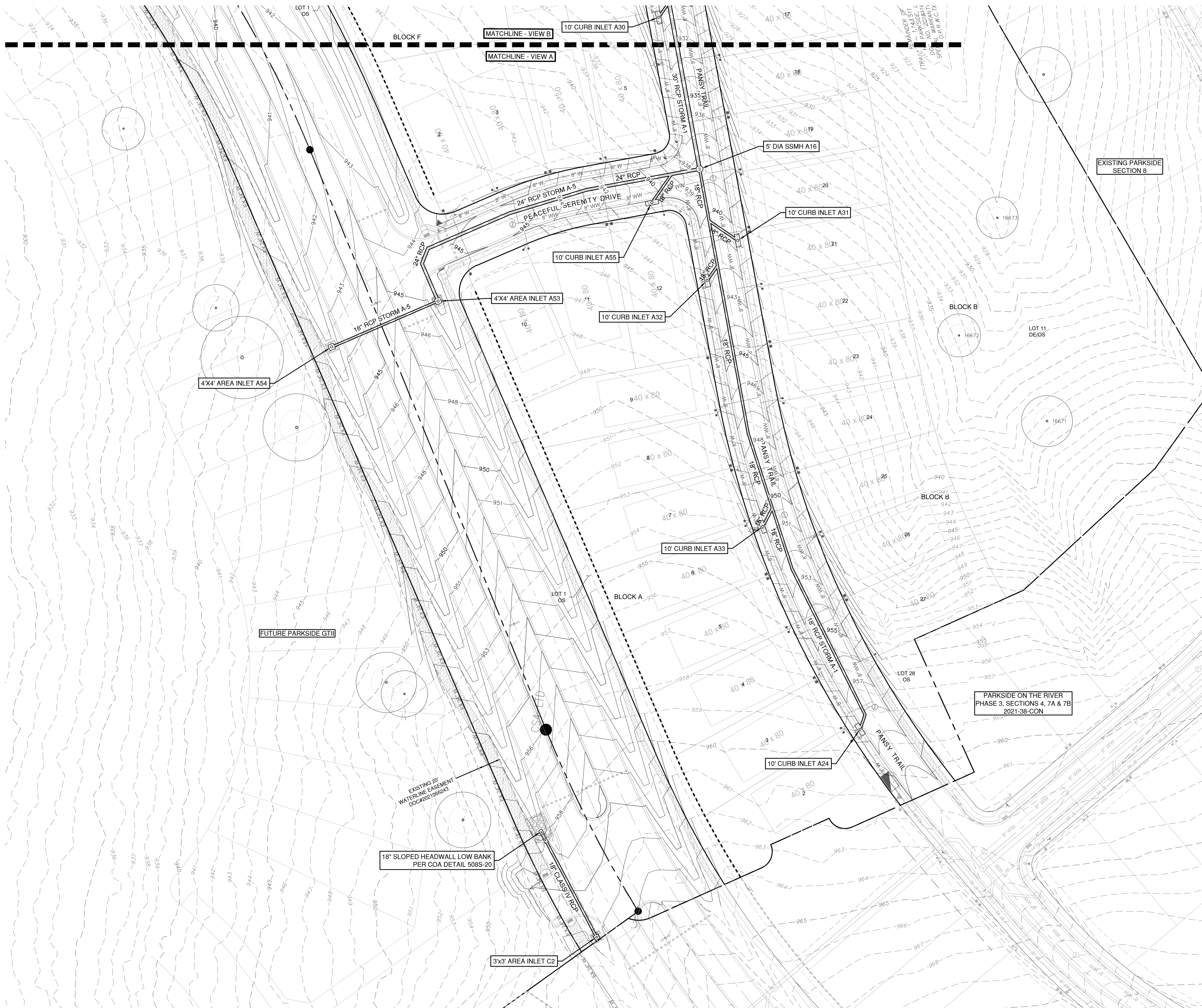
*Shervin Nooshin*

11/6/2013

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
  
SHEET 43 OF 84  
  
2023-XX-CON

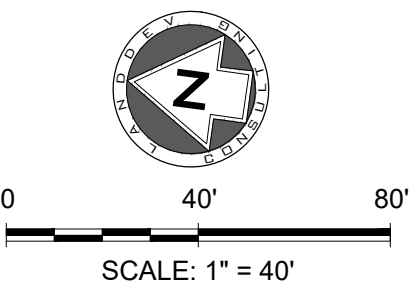
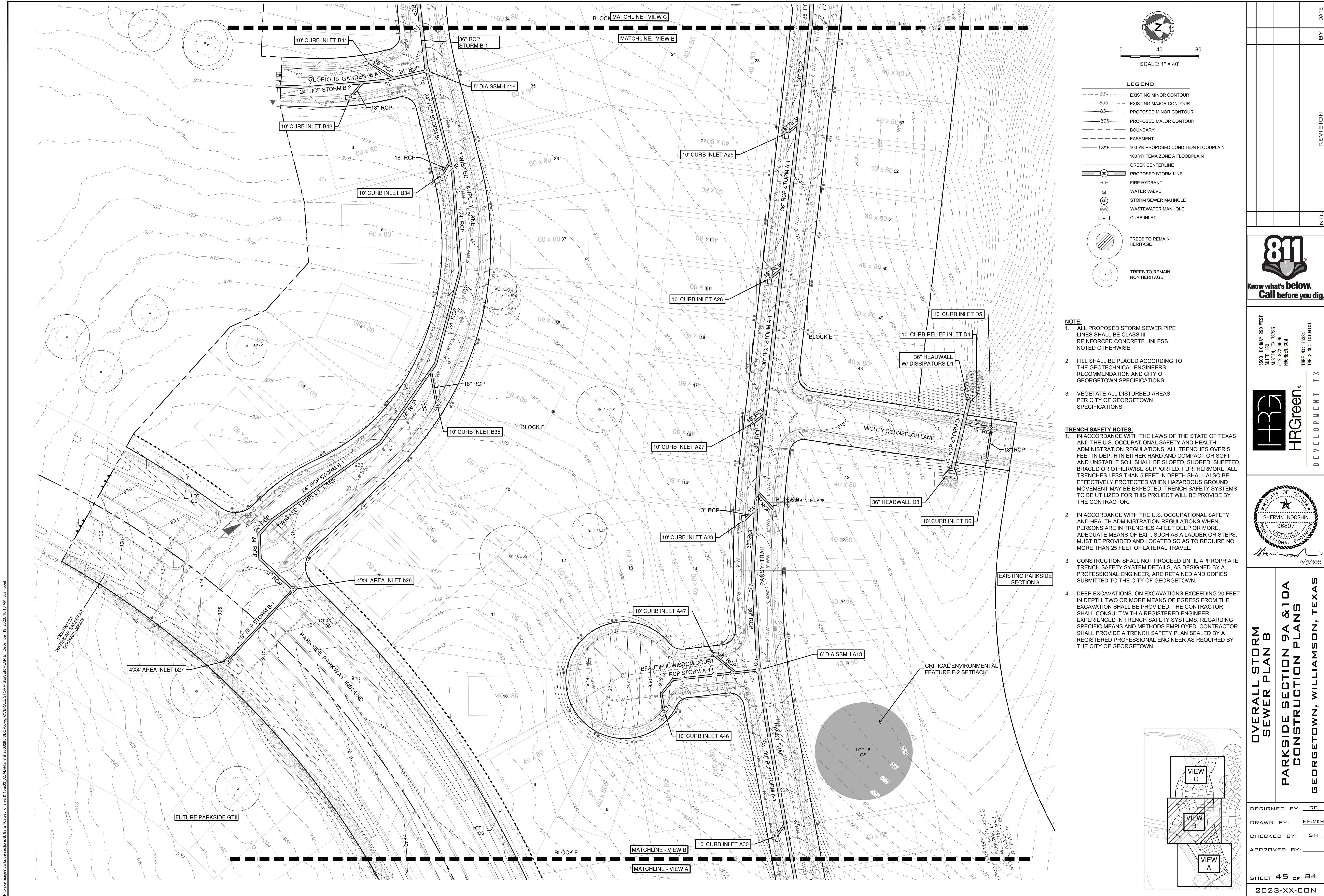


P:\bldg\magnaparkside sections 8, 9a & 10a\03\_ACO\Drawings\20200905 SDOV.dwg, OVERALL STORM SEWER PLAN A, December 19, 2023, 10:16 AM, campbell



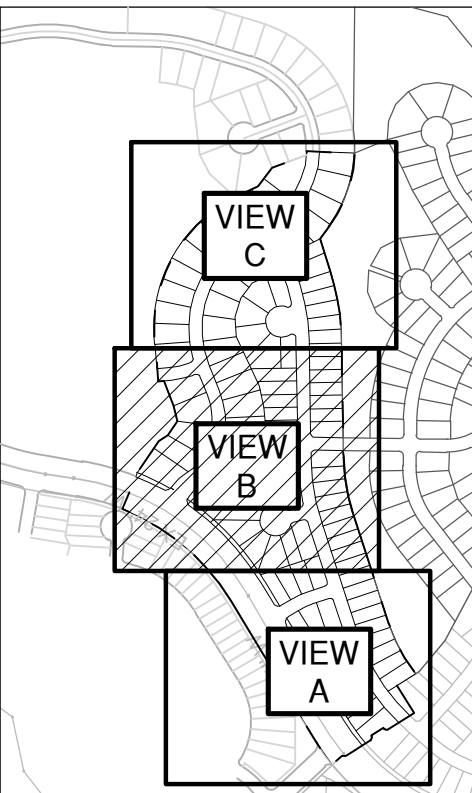


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- LEGEND**
- 834 --- EXISTING MINOR CONTOUR
  - 835 --- EXISTING MAJOR CONTOUR
  - 834 --- PROPOSED MINOR CONTOUR
  - 835 --- PROPOSED MAJOR CONTOUR
  - --- BOUNDARY
  - --- EASEMENT
  - 100 YR --- 100 YR PROPOSED CONDITION FLOODPLAIN
  - 100 YR FEMA --- 100 YR FEMA ZONE A FLOODPLAIN
  - --- CREEK CENTERLINE
  - --- PROPOSED STORM LINE
  - --- FIRE HYDRANT
  - --- WATER VALVE
  - --- STORM SEWER MANHOLE
  - --- WASTEWATER MANHOLE
  - --- CURB INLET
  - --- TREES TO REMAIN HERITAGE
  - --- TREES TO REMAIN NON HERITAGE

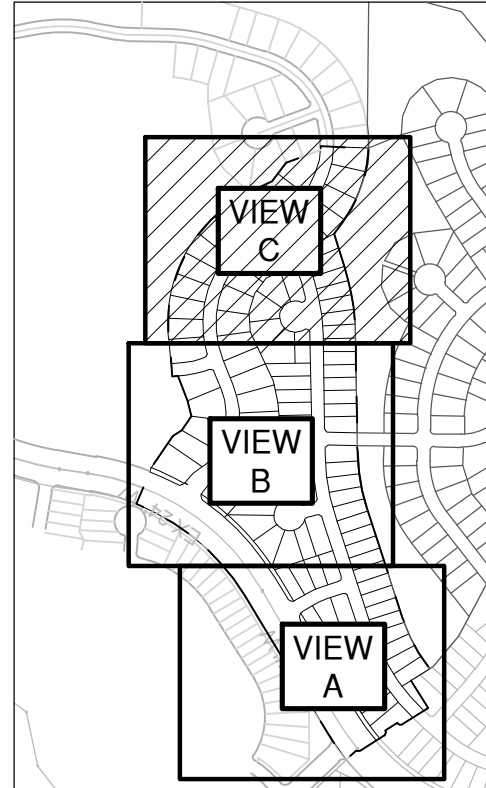
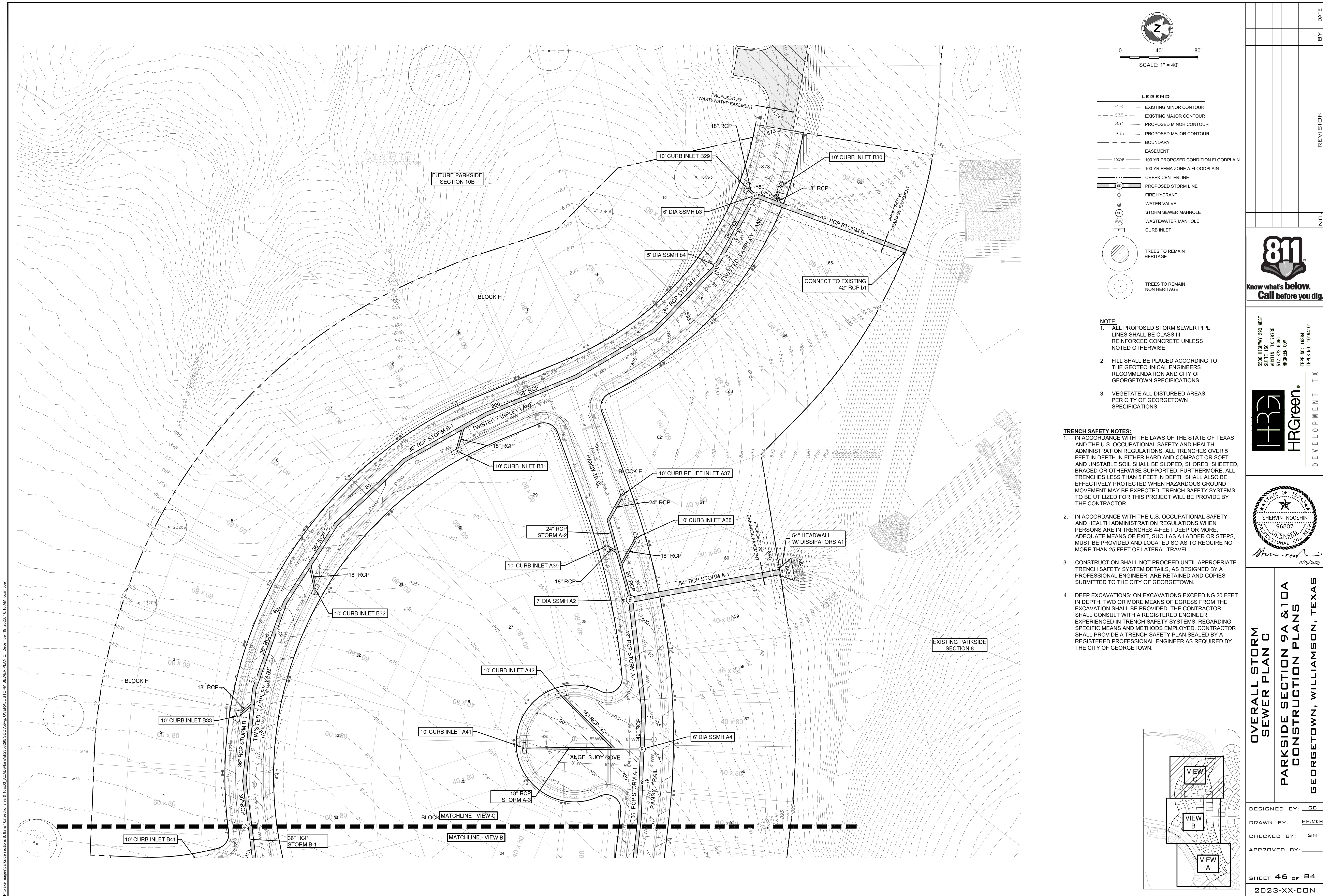
- NOTE:**
- ALL PROPOSED STORM SEWER PIPE LINES SHALL BE CLASS III REINFORCED CONCRETE UNLESS NOTED OTHERWISE.
  - FILL SHALL BE PLACED ACCORDING TO THE GEOTECHNICAL ENGINEERS RECOMMENDATION AND CITY OF GEORGETOWN SPECIFICATIONS.
  - VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.
- TRENCH SAFETY NOTES:**
- IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, ALL TRENCHES OVER 5 FEET IN DEPTH IN EITHER HARD AND COMPACT OR SOFT AND UNSTABLE SOIL SHALL BE SLOPED, SHORED, SHEETED, BRACED OR OTHERWISE SUPPORTED. FURTHERMORE, ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT WILL BE PROVIDED BY THE CONTRACTOR.
  - IN ACCORDANCE WITH THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4-FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED SO AS TO REQUIRE NO MORE THAN 25 FEET OF LATERAL TRAVEL.
  - CONSTRUCTION SHALL NOT PROCEED UNTIL APPROPRIATE TRENCH SAFETY SYSTEM DETAILS, AS DESIGNED BY A PROFESSIONAL ENGINEER, ARE RETAINED AND COPIES SUBMITTED TO THE CITY OF GEORGETOWN.
  - DEEP EXCAVATIONS: ON EXCAVATIONS EXCEEDING 20 FEET IN DEPTH, TWO OR MORE MEANS OF EGRESS FROM THE EXCAVATION SHALL BE PROVIDED. THE CONTRACTOR SHALL CONSULT WITH A REGISTERED ENGINEER, EXPERIENCED IN TRENCH SAFETY SYSTEMS, REGARDING SPECIFIC MEANS AND METHODS EMPLOYED. CONTRACTOR SHALL PROVIDE A TRENCH SAFETY PLAN SEALED BY A REGISTERED PROFESSIONAL ENGINEER AS REQUIRED BY THE CITY OF GEORGETOWN.



|                                                                                                                                                |                                     |
|------------------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------|
| 811<br>Know what's below.<br>Call before you dig.                                                                                              |                                     |
| 5508 HIGHWAY 290 WEST<br>SUITE 150<br>AUSTIN, TX 78735<br>CITY OF GEORGETOWN<br>HARGREEN, CON                                                  | TYPE NO. 10384<br>DPLS NO. 10194101 |
| <b>HRGreen</b><br>DEVELOPMENT TX                                                                                                               |                                     |
|                                                                                                                                                |                                     |
| <b>OVERALL STORM SEWER PLAN B</b><br><b>PARKSIDE SECTION 9A &amp; 10A</b><br><b>CONSTRUCTION PLANS</b><br><b>GEORGETOWN, WILLIAMSON, TEXAS</b> |                                     |
| DESIGNED BY: CC                                                                                                                                | DRAWN BY: MM/MKM                    |
| CHECKED BY: SN                                                                                                                                 | APPROVED BY: _____                  |
| SHEET <b>45</b> OF <b>84</b>                                                                                                                   |                                     |
| 2023-XX-CON                                                                                                                                    |                                     |



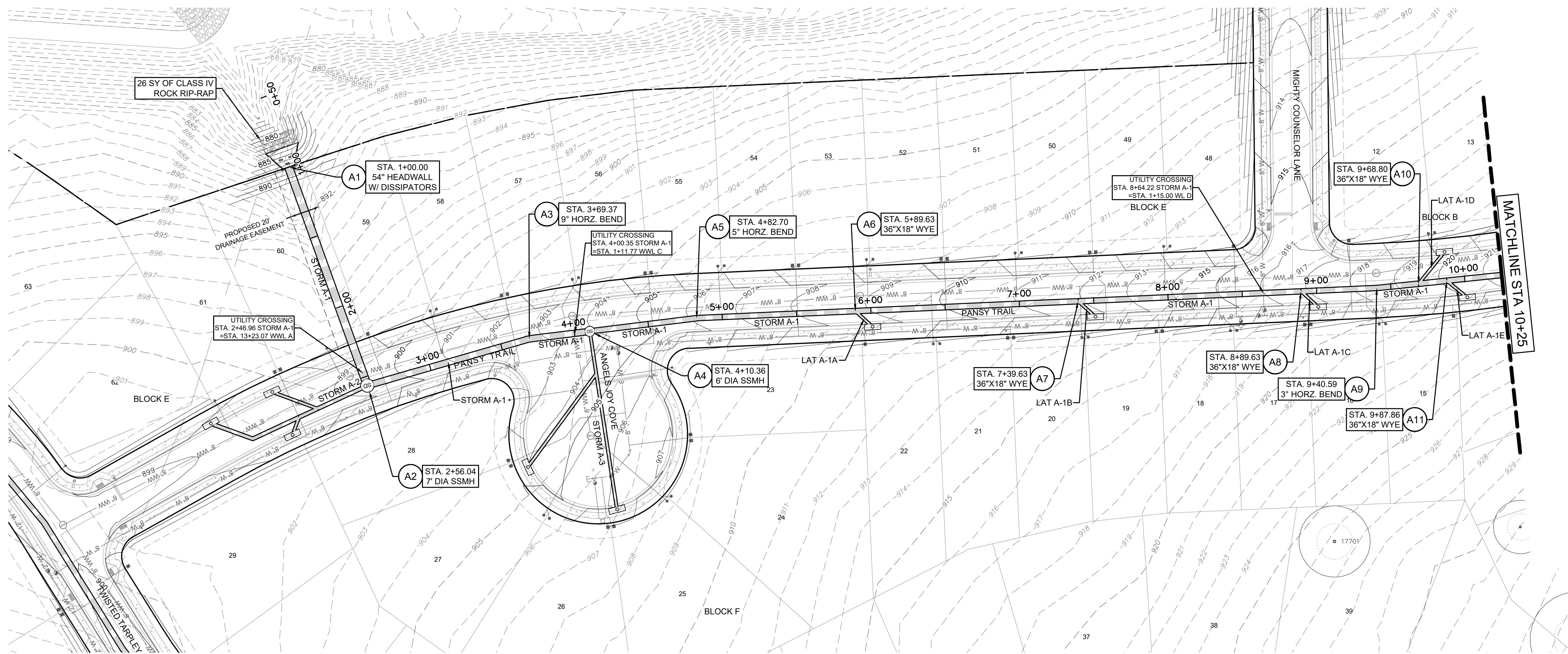
P:\bldg\magnaparkside\sections 8, 9a & 10a\03\_ACO\Drawings\SDOV.dwg, OVERALL STORM SEWER PLAN C, December 19, 2023, 10:18 AM, campbell



|                                                                                                                                                |                                        |
|------------------------------------------------------------------------------------------------------------------------------------------------|----------------------------------------|
| 811<br>Know what's below.<br>Call before you dig.                                                                                              |                                        |
| 5508 HIGHWAY 290 WEST<br>SUITE 150<br>AUSTIN, TX 78735<br>512.453.1234<br>HARGREEN.COM                                                         | TITLE NO.: 10384<br>TPLS NO.: 10194101 |
| <b>HARGREEN</b><br>DEVELOPMENT TX                                                                                                              |                                        |
|                                                                                                                                                |                                        |
| <b>OVERALL STORM SEWER PLAN C</b><br><b>PARKSIDE SECTION 9A &amp; 10A</b><br><b>CONSTRUCTION PLANS</b><br><b>GEORGETOWN, WILLIAMSON, TEXAS</b> |                                        |
| DESIGNED BY: CC                                                                                                                                |                                        |
| DRAWN BY: MM/MKM                                                                                                                               |                                        |
| CHECKED BY: SN                                                                                                                                 |                                        |
| APPROVED BY:                                                                                                                                   |                                        |
| SHEET <b>46</b> OF <b>84</b>                                                                                                                   |                                        |
| 2023-XX-CON                                                                                                                                    |                                        |



P:\bids\magnificent\sections\_9a & 10a\303\_ACAD\Plan\2020\SDP\STORM A-1.dwg, STORM A-1 PLAN & PROFILE 1+00 - 10+25, December 19, 2023, 10:17 AM, c:\campbell

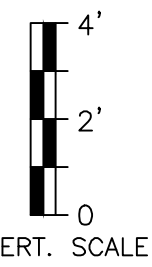


0 40' 80'  
SCALE: 1" = 40'

#### LEGEND

- EXISTING MINOR CONTOUR
- EXISTING MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- BOUNDARY
- EASEMENT
- 100 YR 100 YR PROPOSED CONDITION FLOODPLAIN
- 100 YR FEMA ZONE A FLOODPLAIN
- CREEK CENTERLINE
- PROPOSED STORM LINE
- FIRE HYDRANT
- WATER VALVE
- STORM SEWER MAHOLE
- WASTEWATER MAHOLE
- CURB INLET
- TREES TO REMAIN HERITAGE
- TREES TO REMAIN NON HERITAGE

VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'

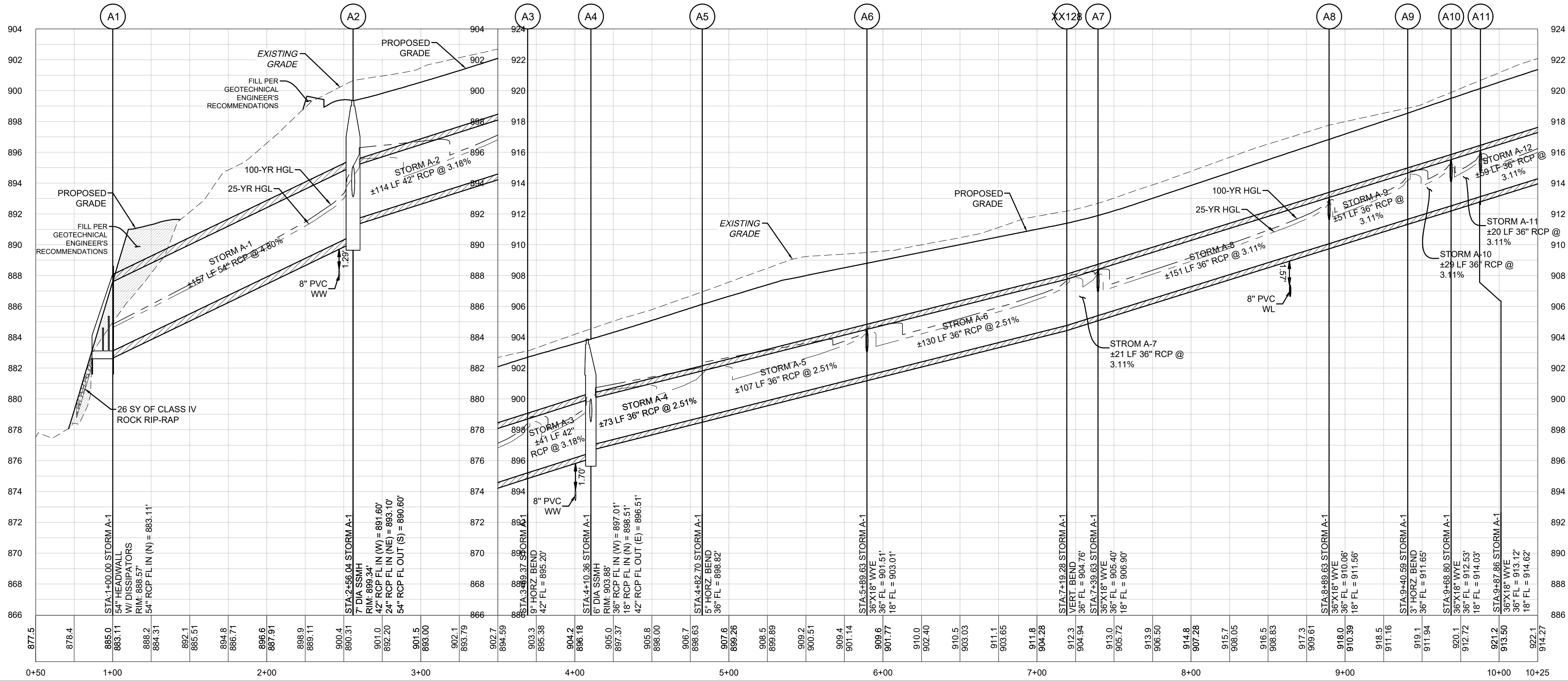


- NOTE:**
- ALL PROPOSED STORM SEWER PIPE LINES SHALL BE CLASS III REINFORCED CONCRETE UNLESS NOTED OTHERWISE.
  - FILL SHALL BE PLACED ACCORDING TO THE GEOTECHNICAL ENGINEERS RECOMMENDATION AND CITY OF GEORGETOWN SPECIFICATIONS.
  - VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.

- TRENCH SAFETY NOTES:**
- IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, ALL TRENCHES OVER 5 FEET IN DEPTH IN EITHER HARD AND COMPACT OR SOFT AND UNSTABLE SOIL SHALL BE SLOPED, SHORED, SHEETED, BRACED OR OTHERWISE SUPPORTED. FURTHERMORE, ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT WILL BE PROVIDED BY THE CONTRACTOR.
  - IN ACCORDANCE WITH THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4- FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED SO AS TO REQUIRE NO MORE THAN 25 FEET OF LATERAL TRAVEL.
  - CONSTRUCTION SHALL NOT PROCEED UNTIL APPROPRIATE TRENCH SAFETY SYSTEM DETAILS, AS DESIGNED BY A PROFESSIONAL ENGINEER, ARE RETAINED AND COPIES SUBMITTED TO THE CITY OF GEORGETOWN.
  - DEEP EXCAVATIONS: ON EXCAVATIONS EXCEEDING 20 FEET IN DEPTH, TWO OR MORE MEANS OF EGRESS FROM THE EXCAVATION SHALL BE PROVIDED. THE CONTRACTOR SHALL CONSULT WITH A REGISTERED ENGINEER, EXPERIENCED IN TRENCH SAFETY SYSTEMS, REGARDING SPECIFIC MEANS AND METHODS EMPLOYED. CONTRACTOR SHALL PROVIDE A TRENCH SAFETY PLAN SEALED BY A REGISTERED PROFESSIONAL ENGINEER AS REQUIRED BY THE CITY OF GEORGETOWN.

| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM A-1  | 4.80%     | 92.09     | 14.33      | 2.82     | 115.70     | 15.21       | 3.17      |
| STORM A-2  | 3.18%     | 79.77     | 9.00       | 3.96     | 100.40     | 10.83       | 4.69      |
| STORM A-3  | 3.18%     | 79.77     | 9.04       | 3.38     | 100.40     | 10.83       | 3.63      |
| STORM A-4  | 2.51%     | 71.00     | 10.36      | 3.47     | 89.23      | 12.62       | 3.72      |
| STORM A-5  | 2.51%     | 71.00     | 10.36      | 3.14     | 89.23      | 12.76       | 3.57      |
| STORM A-6  | 2.51%     | 64.44     | 9.63       | 2.86     | 80.93      | 11.65       | 3.16      |
| STORM A-7  | 3.11%     | 64.44     | 9.55       | 3.06     | 80.93      | 11.65       | 3.25      |
| STORM A-8  | 3.11%     | 59.41     | 9.16       | 2.71     | 74.61      | 10.83       | 3.01      |
| STORM A-9  | 3.11%     | 56.04     | 8.91       | 2.58     | 70.34      | 10.36       | 2.87      |
| STORM A-10 | 3.11%     | 56.04     | 8.56       | 2.93     | 70.34      | 10.28       | 3.13      |
| STORM A-11 | 3.11%     | 53.68     | 8.75       | 2.49     | 67.45      | 10.10       | 2.77      |
| STORM A-12 | 3.11%     | 48.21     | 8.10       | 2.46     | 60.58      | 9.21        | 2.80      |

### STORM A-1



811  
Know what's below.  
Call before you dig.

5508 HERMAN 2ND WEST  
SUITE 100  
AUSTIN, TX 78735  
512.872.6996  
HRGREEN.COM  
TYPE NO: 14384  
TPLS NO: 10184101

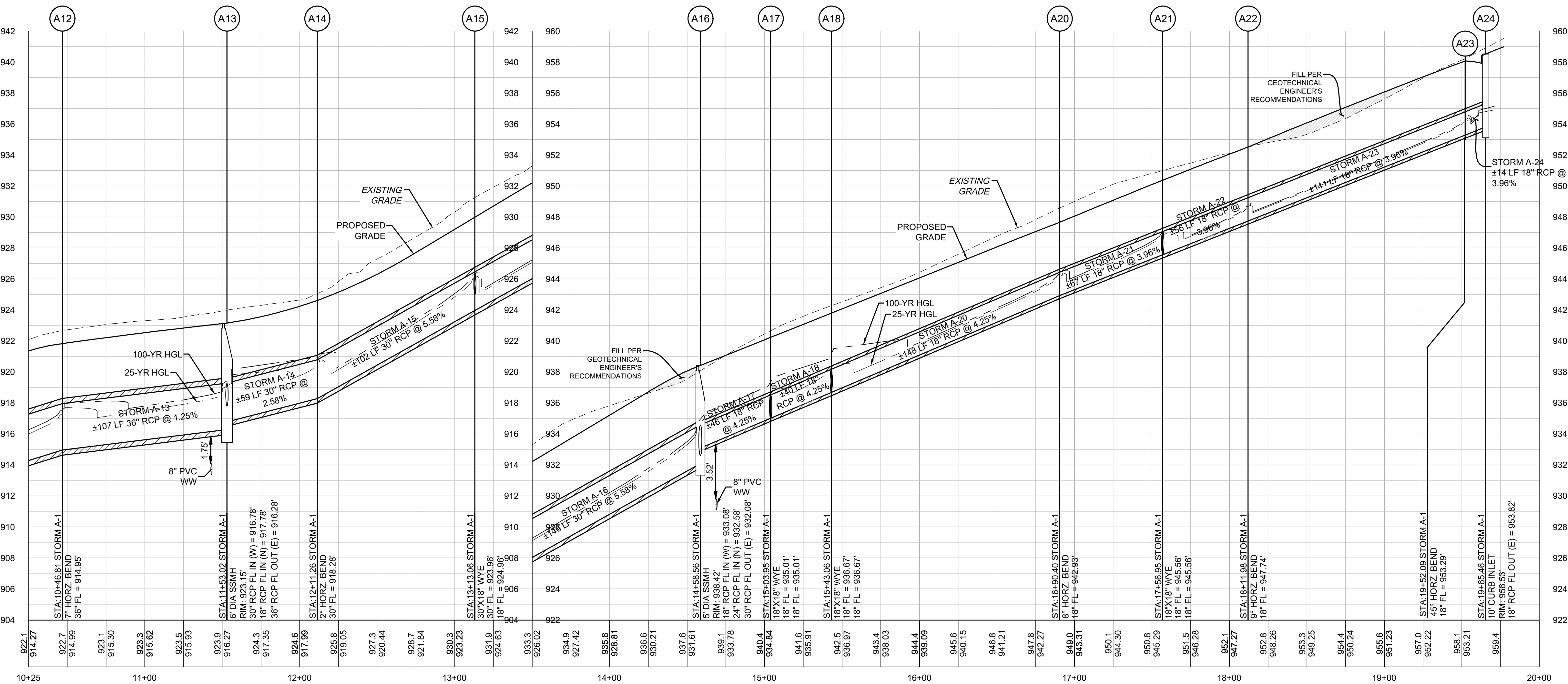
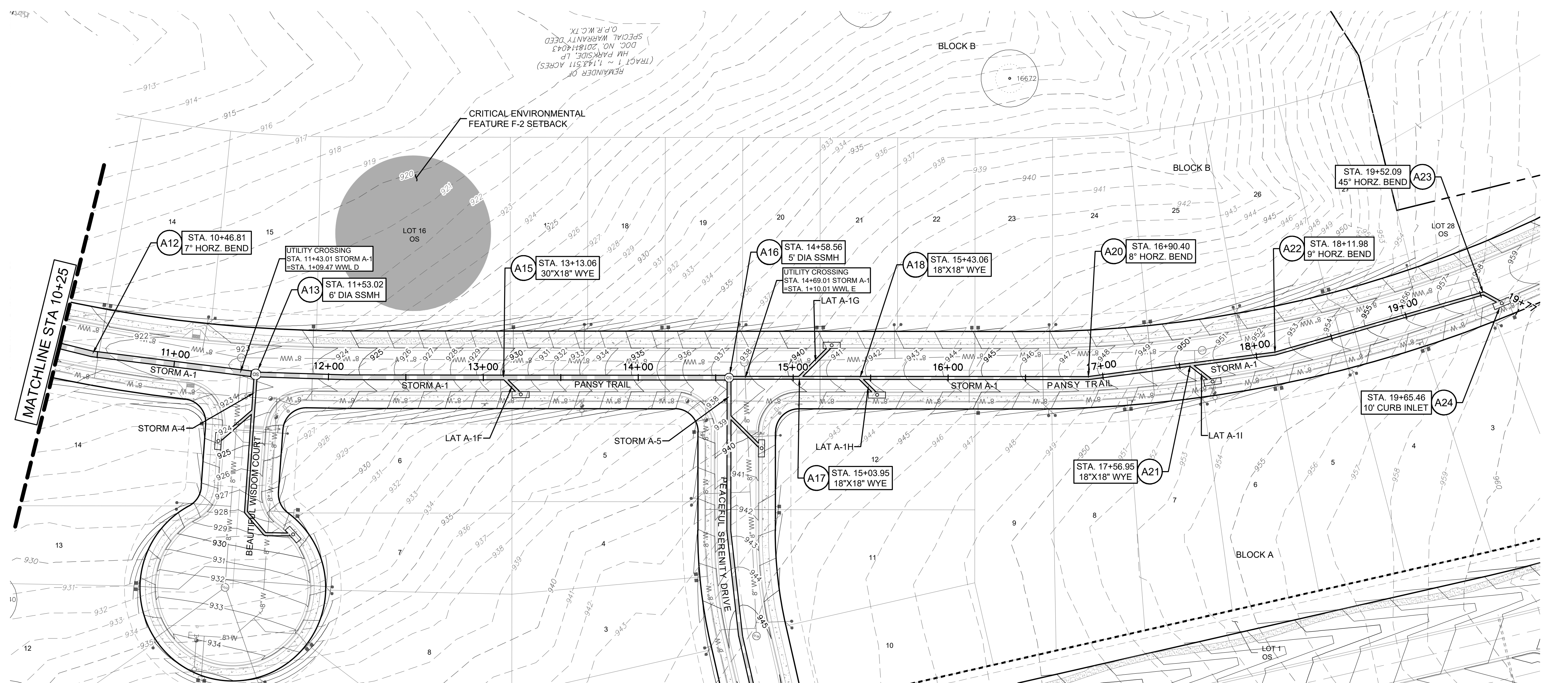
HRGreen  
DEVELOPMENT TX

SHERWIN NOOSHIN  
96807  
REGISTERED PROFESSIONAL ENGINEER  
11/15/2023

STORM A-1 PLAN &  
PROFILE 1+00 - 10+25  
PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
SHEET 47 OF 84  
2023-XX-CON





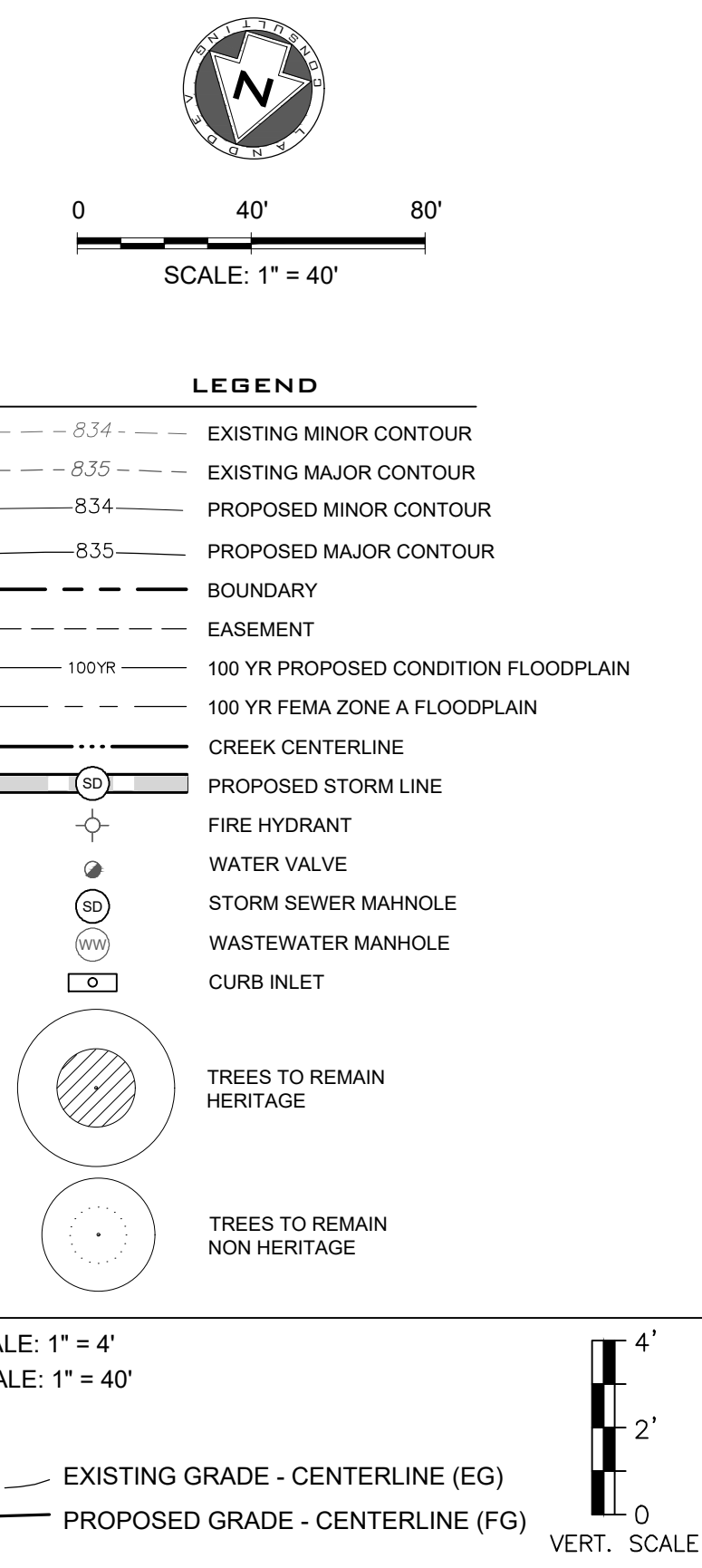
| Pipe Label | Slope<br>(%) | Q25<br>(cfs) | Q25<br>(ft/s) | D25<br>(ft) | Q100<br>(cfs) | V100<br>(ft/s) | D100<br>(ft) |
|------------|--------------|--------------|---------------|-------------|---------------|----------------|--------------|
| STORM A-13 | 1.25%        | 48.21        | 7.77          | 2.75        | 60.58         | 9.08           | 3.01         |
| STORM A-14 | 2.58%        | 37.78        | 8.18          | 3.04        | 47.39         | 9.65           | 3.38         |
| STORM A-15 | 5.58%        | 37.78        | 8.18          | 2.50        | 47.39         | 9.90           | 2.88         |
| STORM A-16 | 5.58%        | 33.72        | 7.73          | 2.21        | 42.33         | 8.98           | 2.49         |
| STORM A-17 | 4.25%        | 16.99        | 9.68          | 2.06        | 21.42         | 12.12          | 2.13         |
| STORM A-18 | 4.25%        | 14.45        | 8.32          | 1.79        | 18.30         | 10.36          | 2.64         |
| STORM A-20 | 4.25%        | 9.71         | 5.94          | 1.80        | 12.31         | 7.21           | 2.83         |
| STORM A-21 | 3.96%        | 9.71         | 5.97          | 1.45        | 12.31         | 7.21           | 1.56         |
| STORM A-22 | 3.96%        | 4.55         | 3.66          | 1.36        | 5.81          | 4.17           | 1.61         |
| STORM A-23 | 3.96%        | 4.55         | 4.16          | 0.99        | 5.81          | 4.57           | 1.12         |
| STORM A-24 | 3.96%        | 4.55         | 3.94          | 1.11        | 5.81          | 4.35           | 1.26         |

**NOTE:**

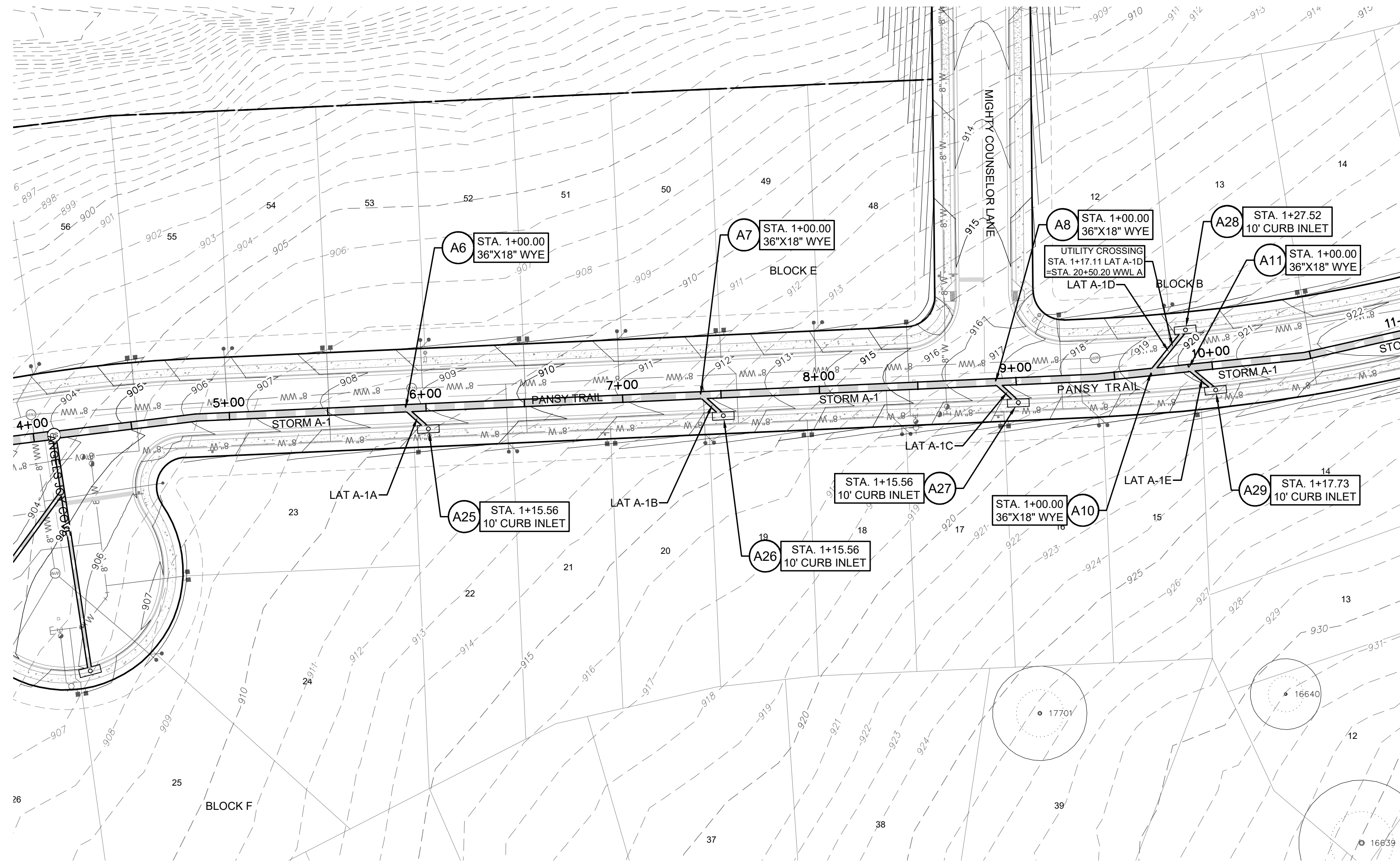
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3. VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.

**TRENCH SAFETY NOTES:**

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[illegible]



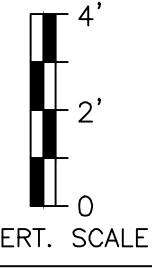


0 40' 80'  
SCALE: 1" = 40'

LEGEND

- 8.34 --- EXISTING MINOR CONTOUR
- 8.35 --- EXISTING MAJOR CONTOUR
- 8.34 --- PROPOSED MINOR CONTOUR
- 8.35 --- PROPOSED MAJOR CONTOUR
- --- BOUNDARY
- --- EASEMENT
- 100YR --- 100 YR PROPOSED CONDITION FLOODPLAIN
- 100YR --- 100 YR FEMA ZONE A FLOODPLAIN
- --- CREEK CENTERLINE
- --- PROPOSED STORM LINE
- --- FIRE HYDRANT
- --- WATER VALVE
- --- STORM SEWER MANHOLE
- --- WASTEWATER MANHOLE
- --- CURB INLET
- --- TREES TO REMAIN HERITAGE
- --- TREES TO REMAIN NON HERITAGE

VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'



NOTE:

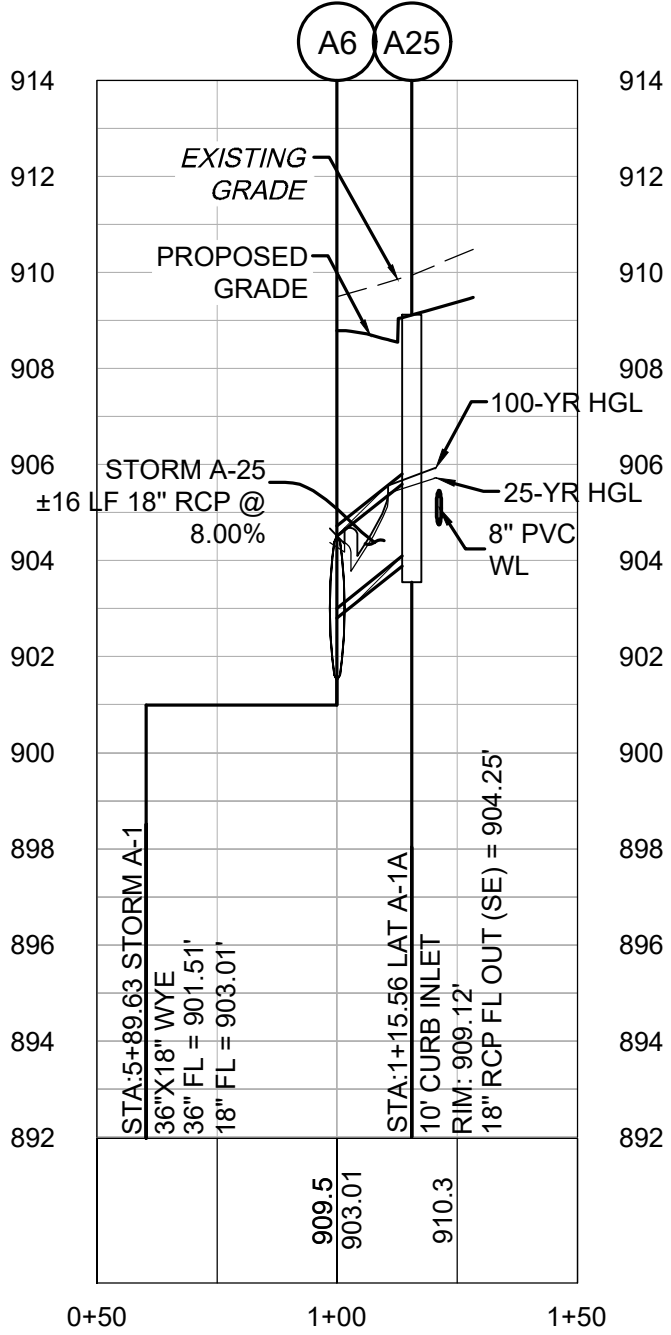
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TRENCH SAFETY NOTES:

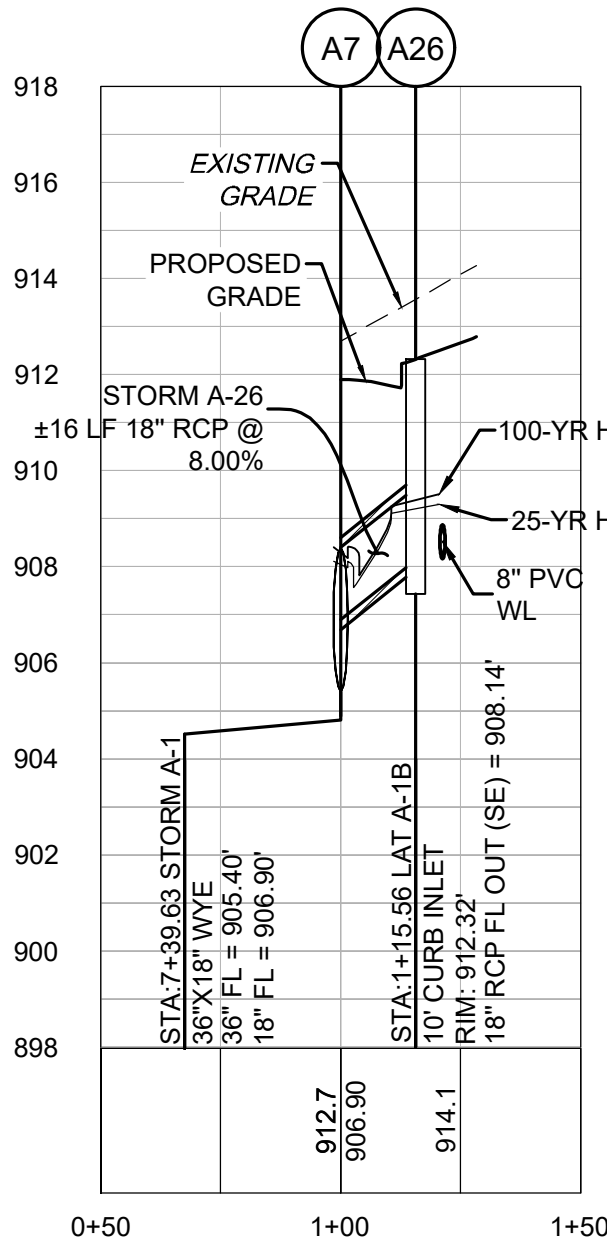
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| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM A-25 | 8.00%     | 6.56      | 4.60       | 1.36     | 8.30       | 5.30        | 1.66      |
| STORM A-26 | 8.00%     | 5.03      | 4.04       | 1.21     | 6.32       | 4.40        | 1.51      |
| STORM A-27 | 8.00%     | 3.37      | 3.32       | 1.08     | 4.27       | 3.52        | 1.37      |
| STORM A-28 | 7.50%     | 2.36      | 2.83       | 0.98     | 2.89       | 2.90        | 1.26      |
| STORM A-29 | 8.00%     | 5.47      | 4.75       | 0.96     | 6.87       | 4.82        | 1.30      |

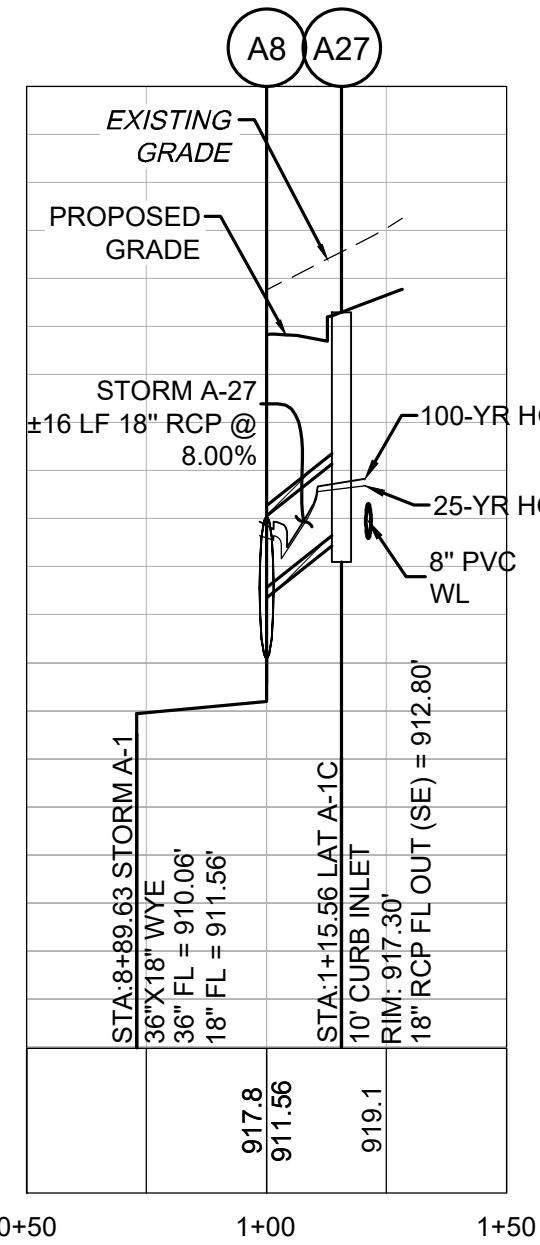
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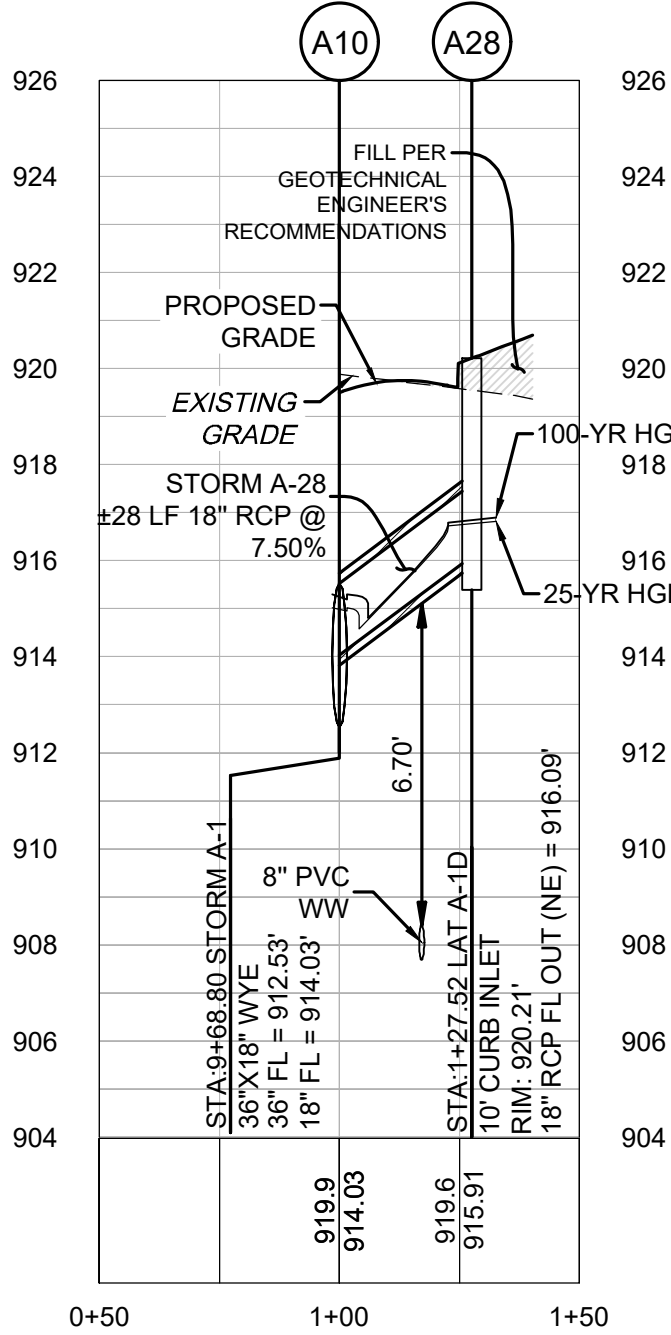
LAT A-1B



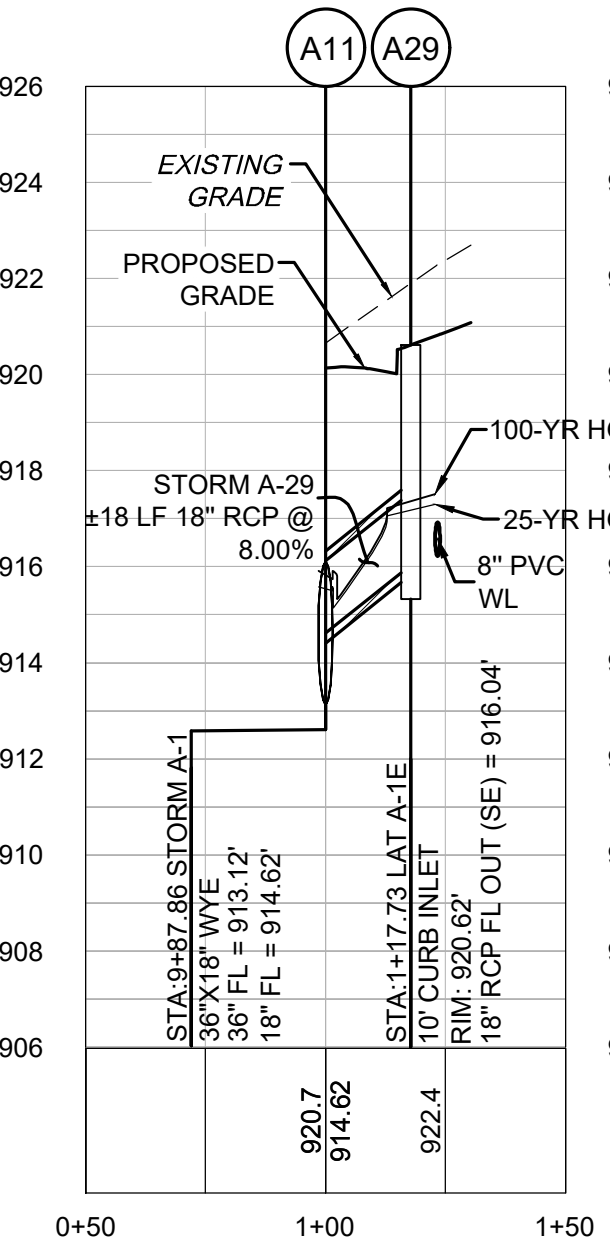
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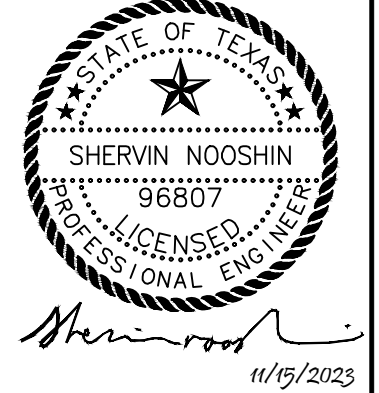
LAT A-1D



LAT A-1E



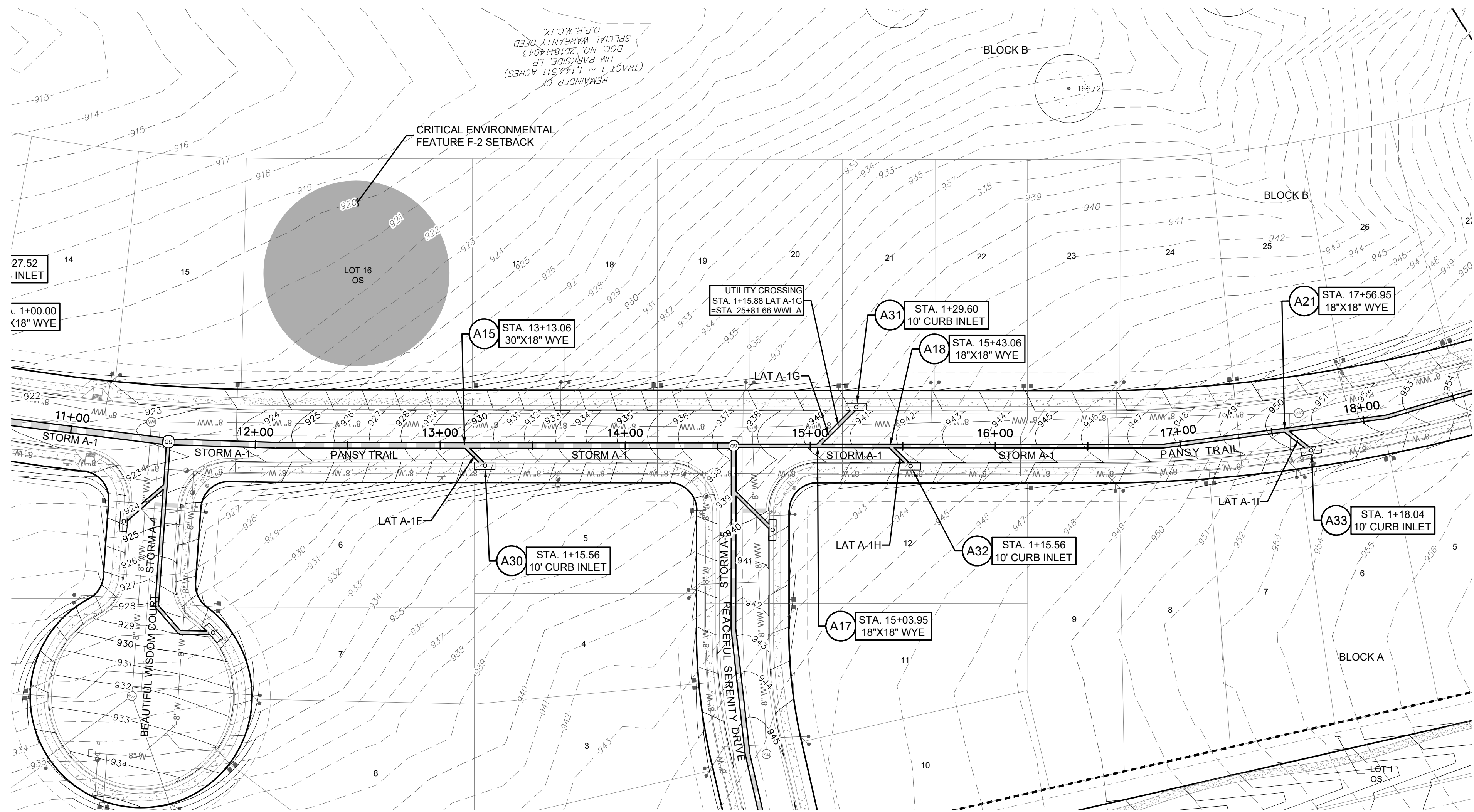
5508 HIGHWAY 290 WEST  
SUITE 100  
AUSTIN, TX 78735  
512.872.6996  
HGREEN.COM



STORM A-1 LATERALS  
SHEET 1  
PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
SHEET 49 OF 84  
2023-XX-CON



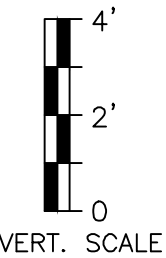


0 40' 80'  
SCALE: 1" = 40'

LEGEND

- EXISTING MINOR CONTOUR
- EXISTING MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- BOUNDARY
- EASEMENT
- 100 YR PROPOSED CONDITION FLOODPLAIN
- 100 YR FEMA ZONE A FLOODPLAIN
- CREEK CENTERLINE
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- WASTEWATER MAHOLE
- CURB INLET
- TREES TO REMAIN HERITAGE
- TREES TO REMAIN NON HERITAGE

VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'



NOTE:

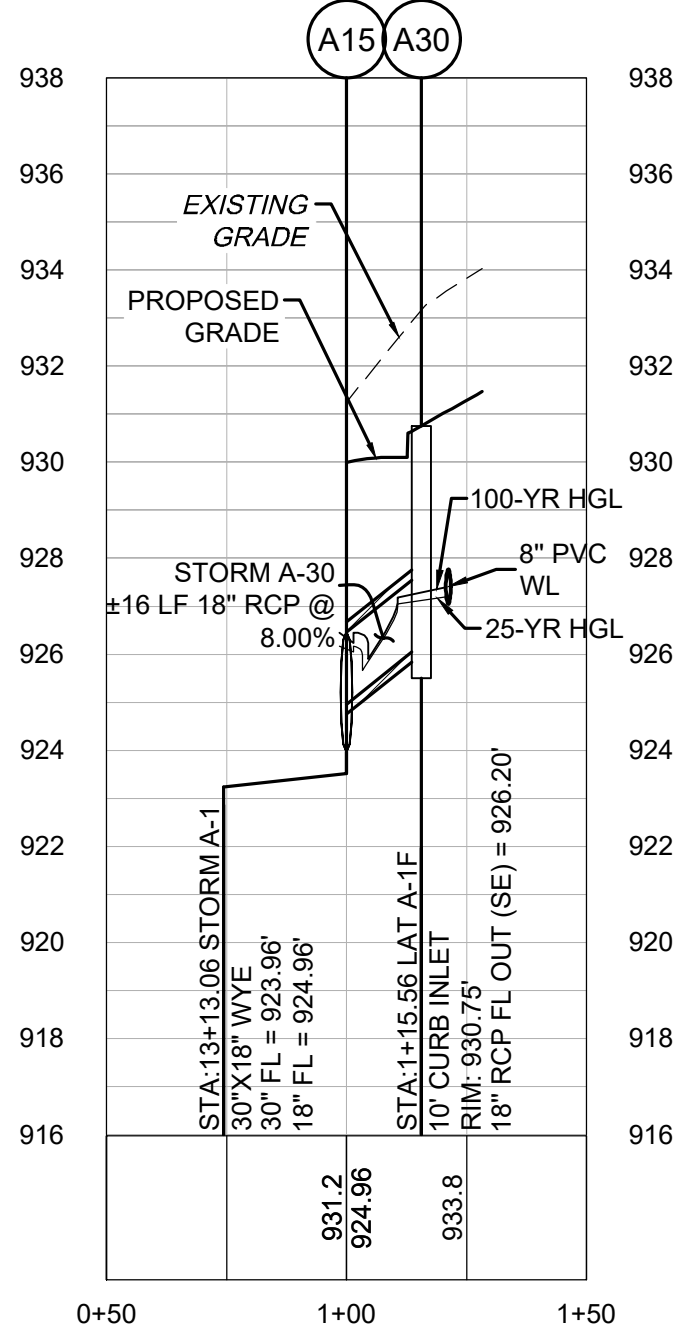
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TRENCH SAFETY NOTES:

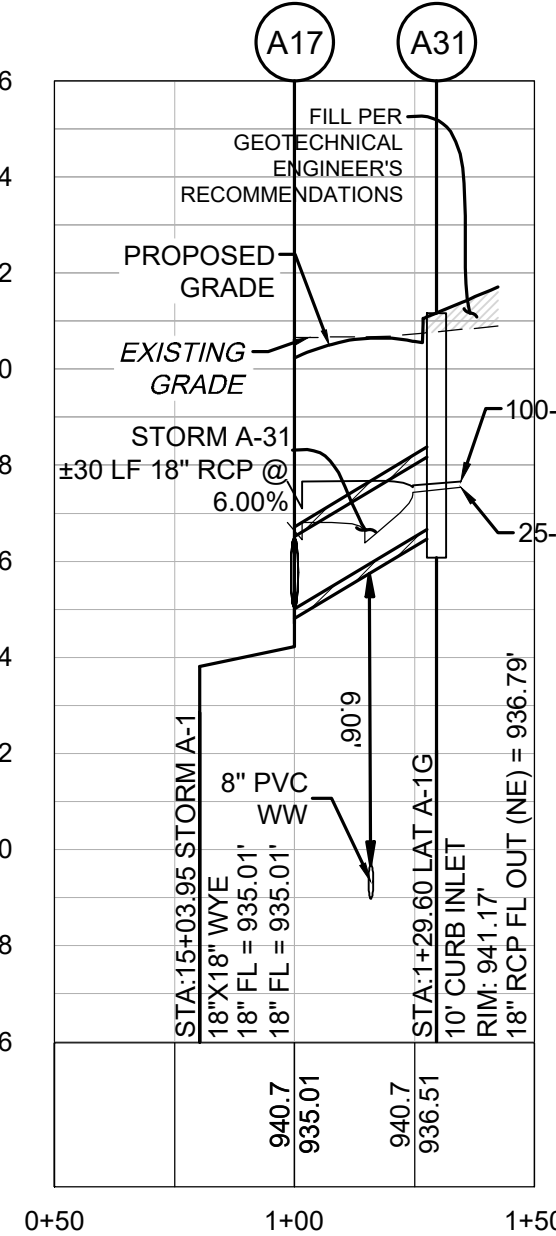
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| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM A-30 | 8.00%     | 4.06      | 3.55       | 1.21     | 5.06       | 3.83        | 1.49      |
| STORM A-31 | 6.00%     | 2.54      | 2.63       | 1.79     | 3.12       | 2.64        | 2.64      |
| STORM A-32 | 10.00%    | 4.74      | 3.68       | 1.81     | 5.99       | 3.55        | 2.83      |
| STORM A-33 | 6.50%     | 5.16      | 3.94       | 1.36     | 6.50       | 4.48        | 1.61      |

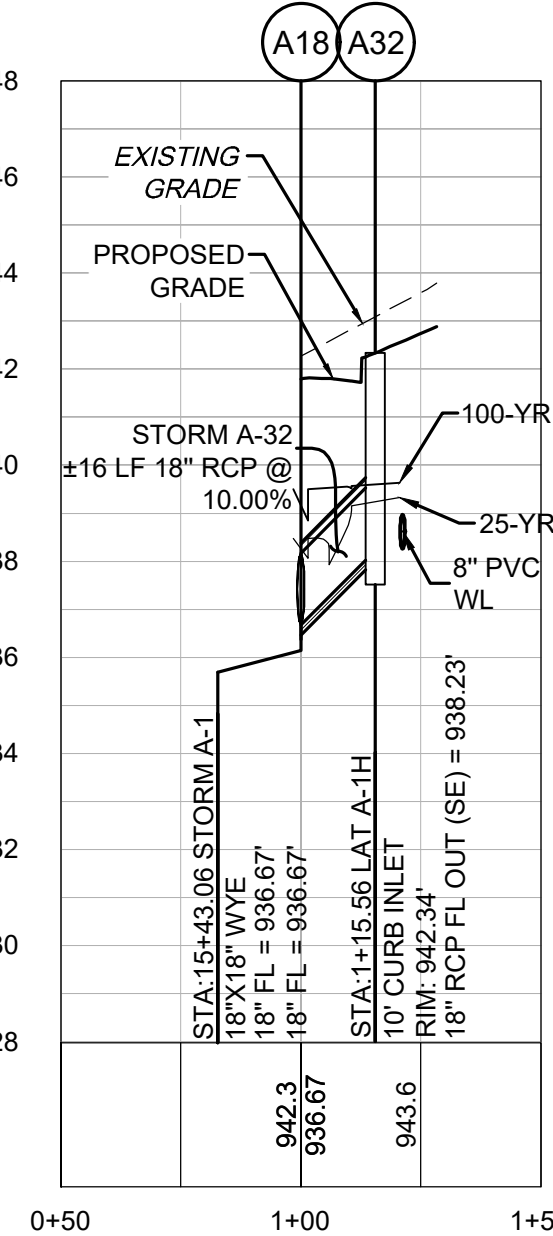
LAT A-1F



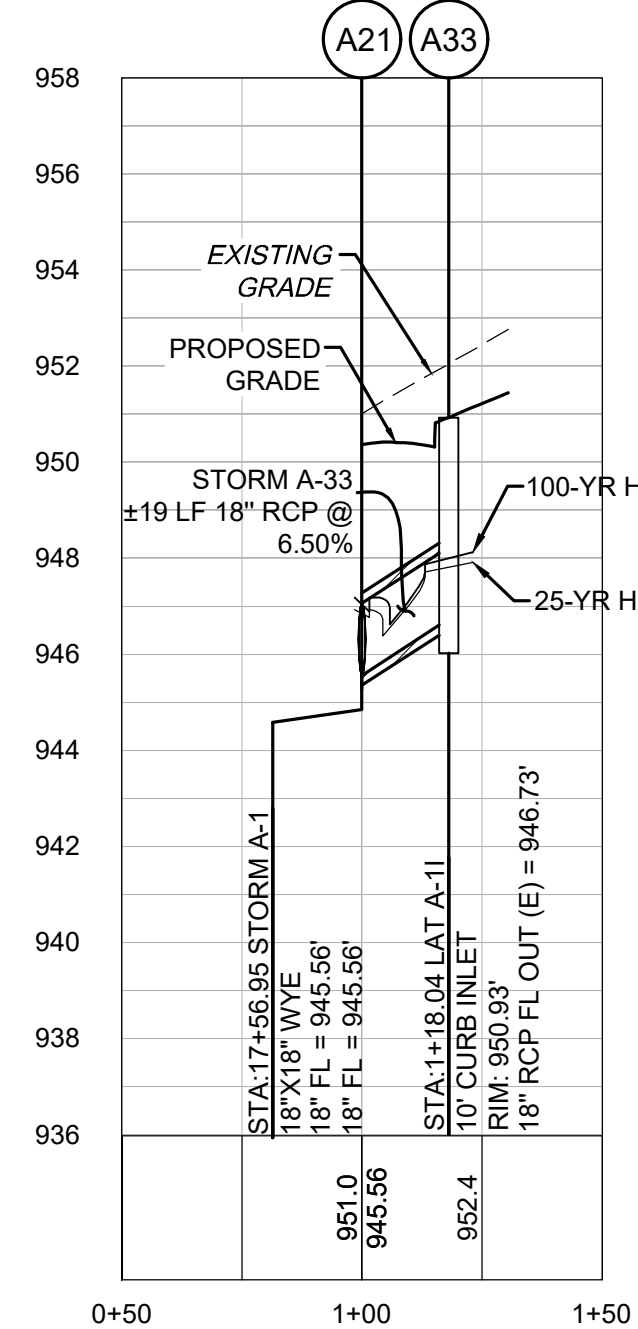
LAT A-1G



LAT A-1H



LAT A-1I



DESIGNED BY: CC

DRAWN BY: MM/MKM

CHECKED BY: SN

APPROVED BY:

811

Know what's below.  
Call before you dig.

5508 HIGHWAY 290 WEST  
SUITE 100  
AUSTIN, TX 78735  
512.872.6996  
HGREEN.COM

TYPE NO: 16384  
TPLS NO: 10184101

HRGreen

DEVELOPMENT TX

STATE OF TEXAS  
SHERVIN NOOSHIN  
96807  
LICENSED PROFESSIONAL ENGINEER

11/15/2023

STORM A-1 LATERALS  
SHEET 2

PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS

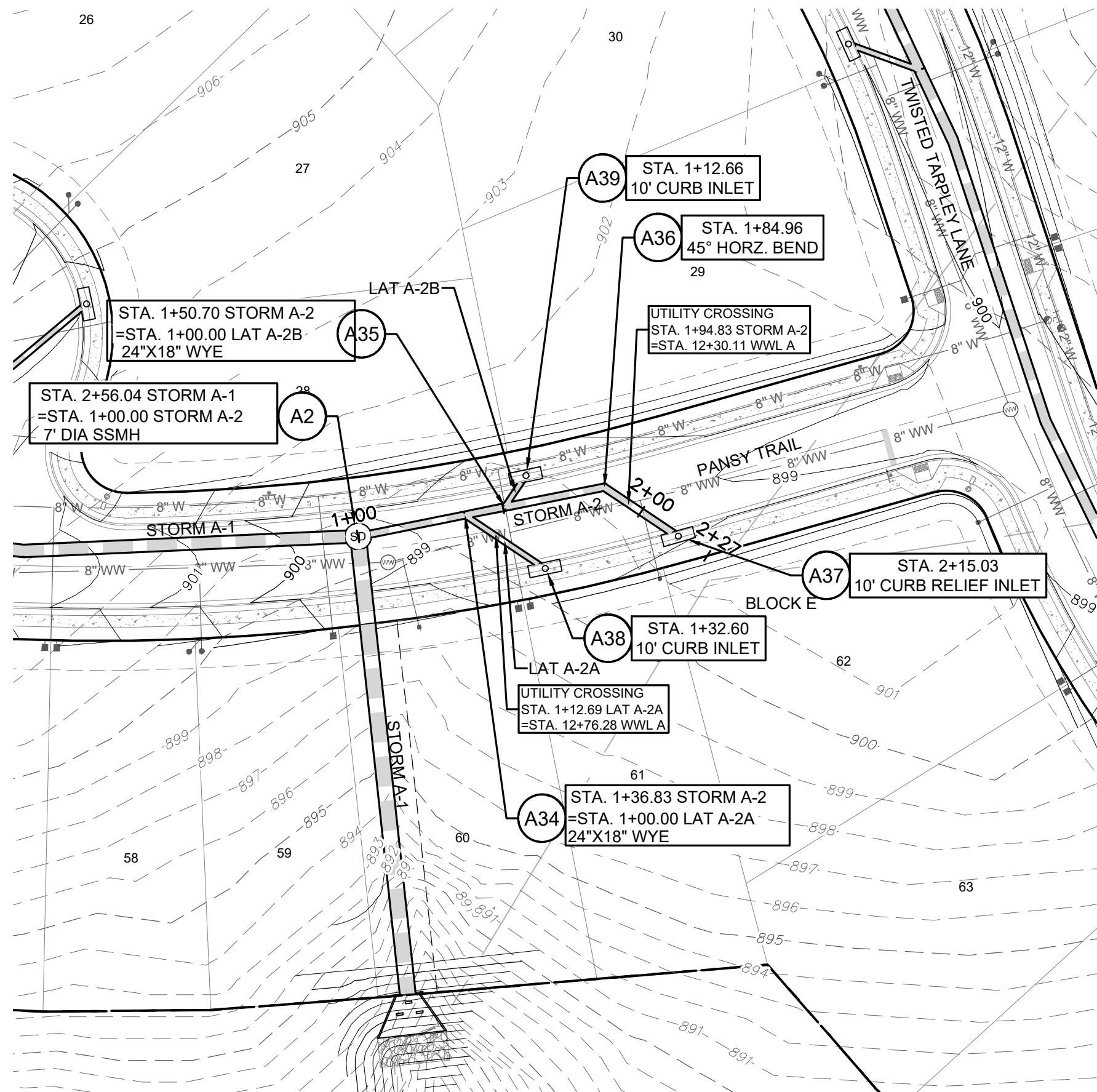
GEORGETOWN, WILLIAMSON, TEXAS

SHEET 50 OF 84

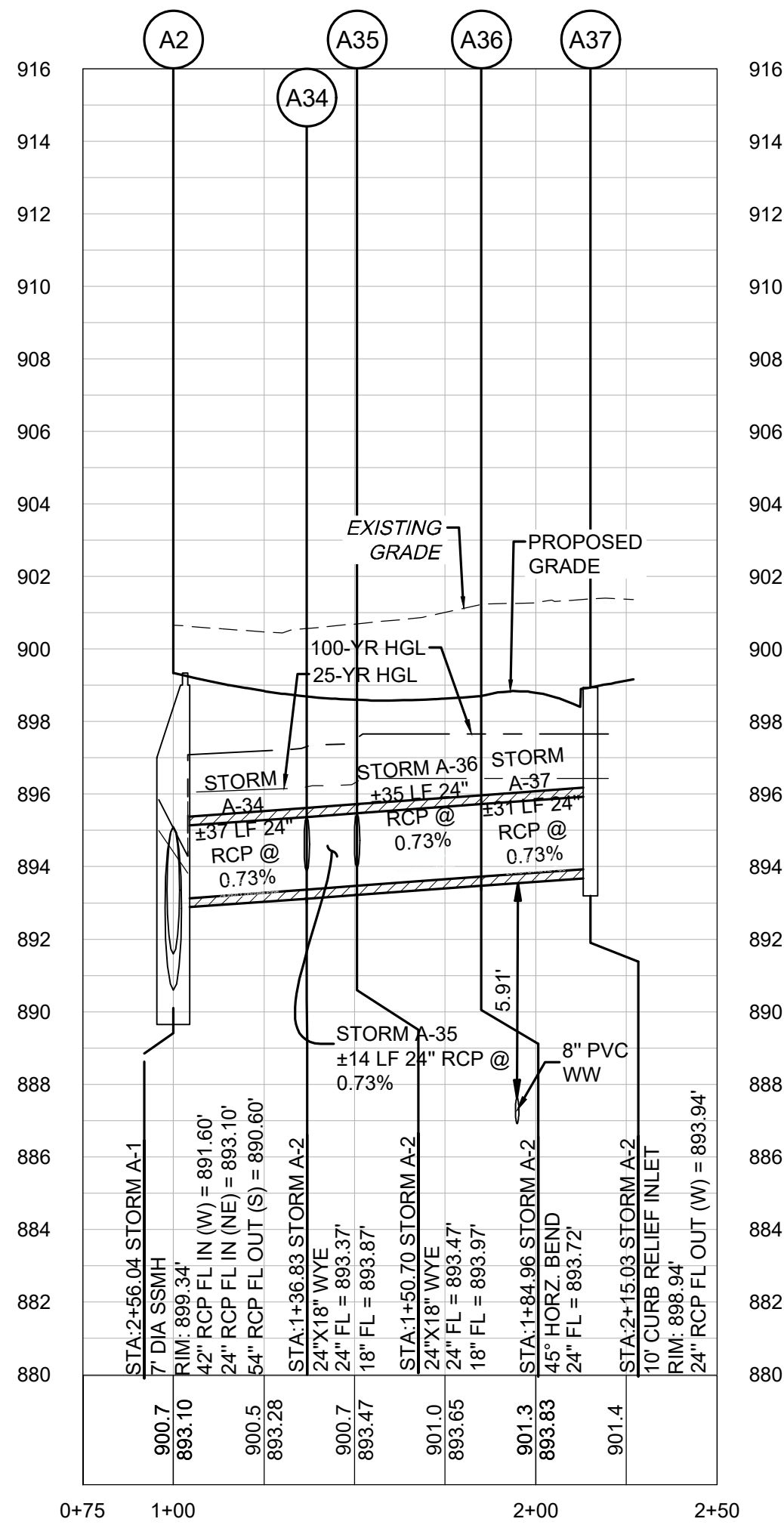
2023-XX-CON



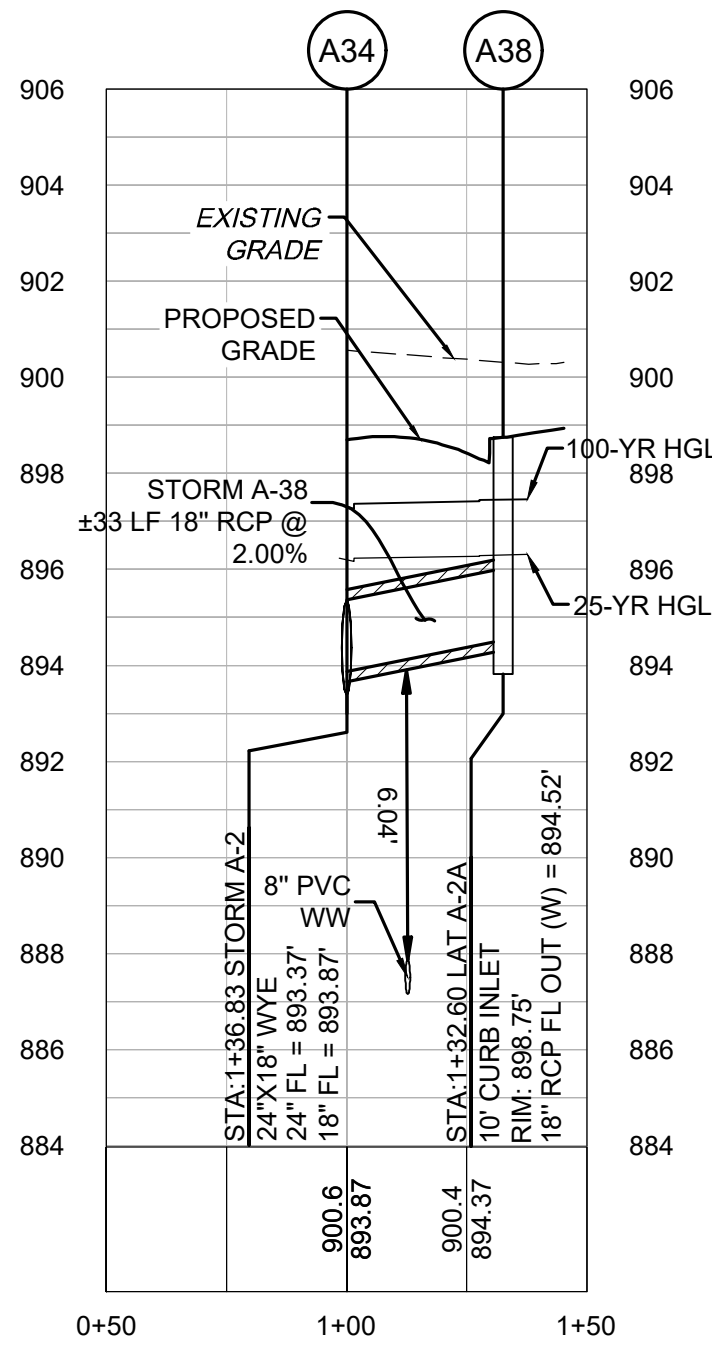
P:\blake\_magnolia\parkside sections 8, 9a & 10\03\_03\_ACAD\Plan\sh-203206 SGRP STORM A-2.dwg STORM A-2 PLAN & PROFILE 1+00 - END December 19, 2023, 10:20 AM rcampbell



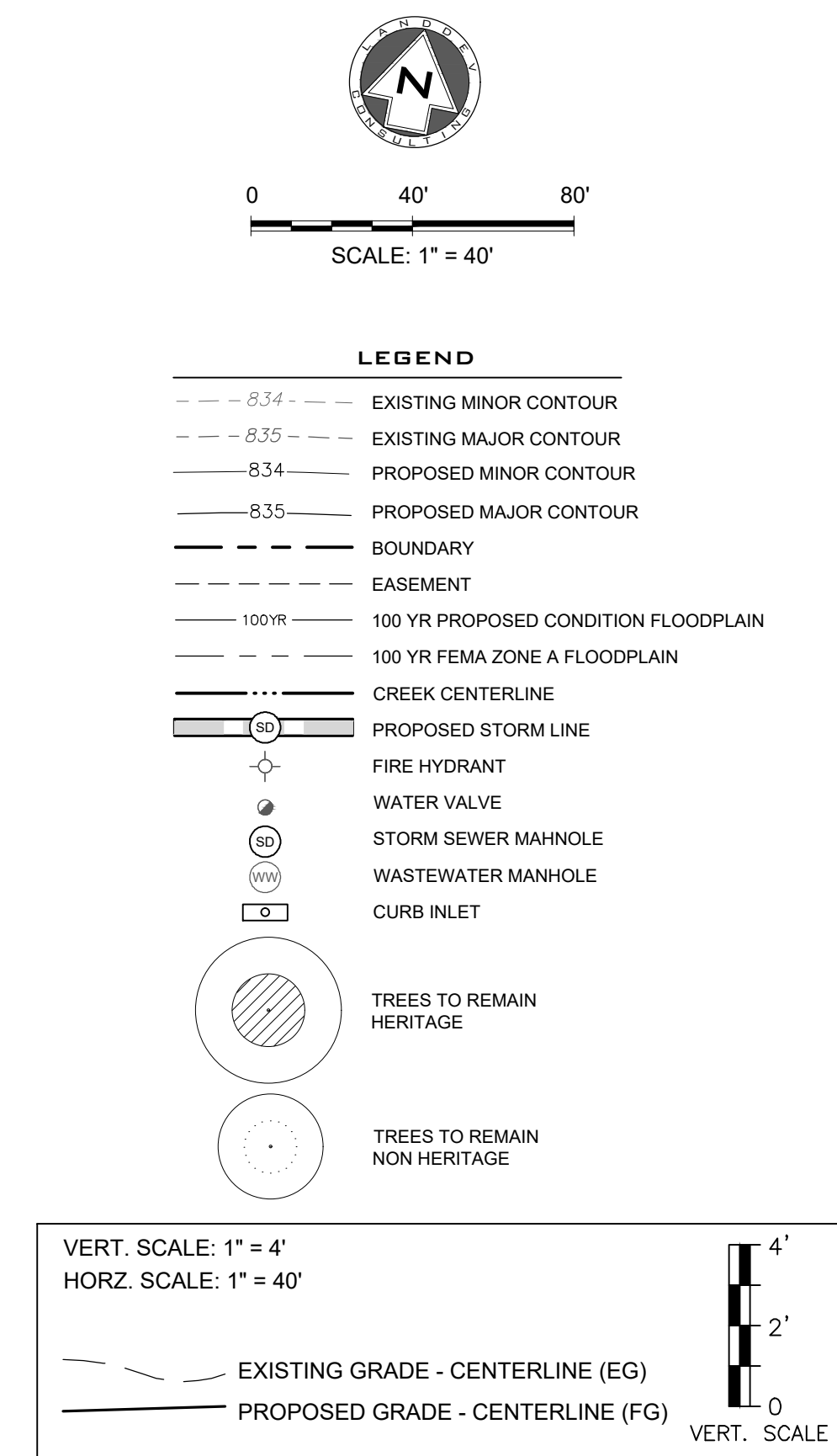
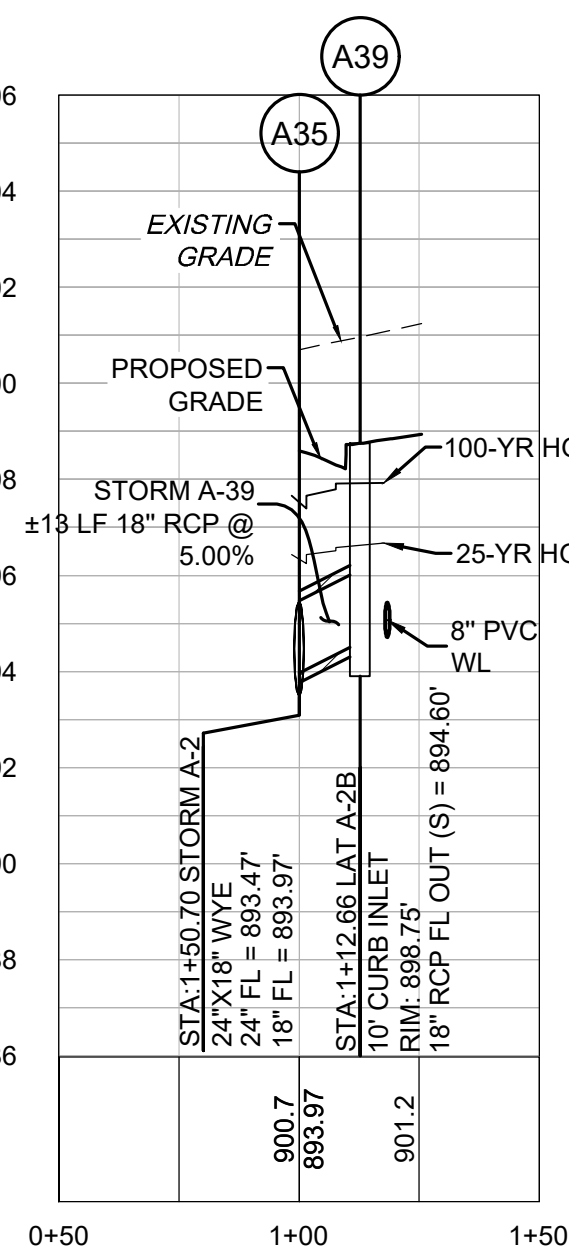
STORM A-2



LAT A-2A



LAT A-2B



NOTE:

- ALL PROPOSED STORM SEWER PIPE LINES SHALL BE CLASS III REINFORCED CONCRETE UNLESS NOTED OTHERWISE.
- FILL SHALL BE PLACED ACCORDING TO THE GEOTECHNICAL ENGINEERS RECOMMENDATION AND CITY OF GEORGETOWN SPECIFICATIONS.
- VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.

TRENCH SAFETY NOTES:

- IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, ALL TRENCHES OVER 5 FEET IN DEPTH IN EITHER HARD AND COMPACT OR SOFT AND UNSTABLE SOIL SHALL BE SLOPED, SHORED, SHEETED, BRACED OR OTHERWISE SUPPORTED. FURTHERMORE, ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT WILL BE PROVIDE BY THE CONTRACTOR.
- IN ACCORDANCE WITH THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4- FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED SO AS TO REQUIRE NO MORE THAN 25 FEET OF LATERAL TRAVEL.
- CONSTRUCTION SHALL NOT PROCEED UNTIL APPROPRIATE TRENCH SAFETY SYSTEM DETAILS, AS DESIGNED BY A PROFESSIONAL ENGINEER, ARE RETAINED AND COPIES SUBMITTED TO THE CITY OF GEORGETOWN.
- DEEP EXCAVATIONS: ON EXCAVATIONS EXCEEDING 20 FEET IN DEPTH, TWO OR MORE MEANS OF EGRESS FROM THE EXCAVATION SHALL BE PROVIDED. THE CONTRACTOR SHALL CONSULT WITH A REGISTERED ENGINEER, EXPERIENCED IN TRENCH SAFETY SYSTEMS, REGARDING SPECIFIC MEANS AND METHODS EMPLOYED. CONTRACTOR SHALL PROVIDE A TRENCH SAFETY PLAN SEALED BY A REGISTERED PROFESSIONAL ENGINEER AS REQUIRED BY THE CITY OF GEORGETOWN.

| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM A-34 | 0.73%     | 12.32     | 3.92       | 2.95     | 15.30      | 4.87        | 3.98      |
| STORM A-35 | 0.73%     | 8.57      | 2.73       | 2.86     | 10.70      | 3.41        | 3.99      |
| STORM A-36 | 0.73%     | 0.00      | 0.00       | 2.95     | 0.00       | 0.00        | 4.19      |
| STORM A-37 | 0.73%     | 0.00      | 0.00       | 2.70     | 0.00       | 0.00        | 3.94      |
| STORM A-38 | 2.00%     | 3.75      | 2.12       | 2.36     | 4.60       | 2.60        | 3.49      |
| STORM A-39 | 5.00%     | 8.57      | 4.85       | 2.46     | 10.70      | 6.05        | 3.69      |

DESIGNED BY: CC

DRAWN BY: MM/MKM

CHECKED BY: SN

APPROVED BY:

811

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TBRLS NO: 10184101

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96807  
LICENSED PROFESSIONAL ENGINEER

11/15/2023

STORM A-2 PLAN &  
PROFILE 1+00 - END

PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS

GEORGETOWN, WILLIAMSON, TEXAS

SHEET 51 OF 84

2023-XX-CON

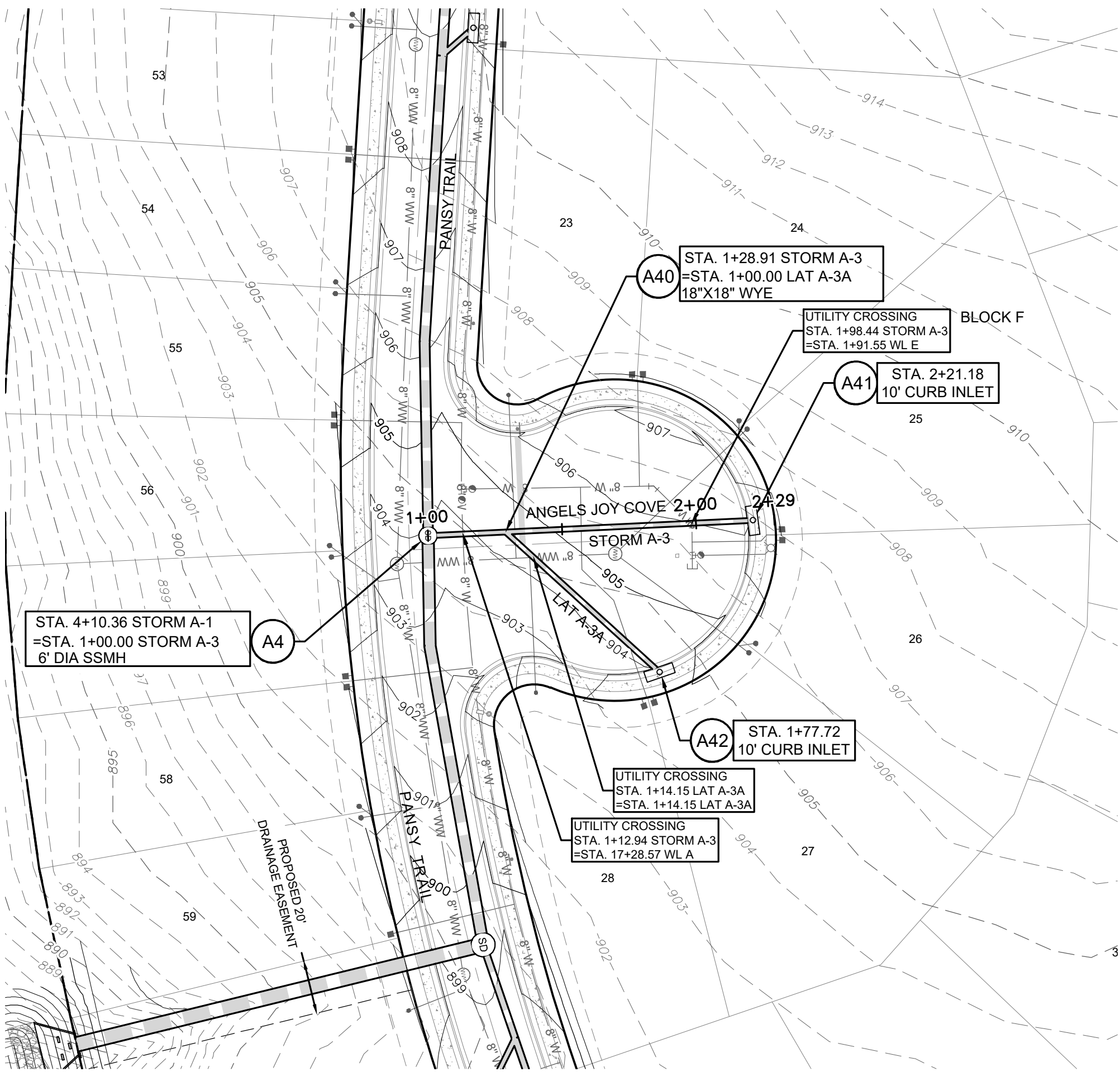
REVISION

BY

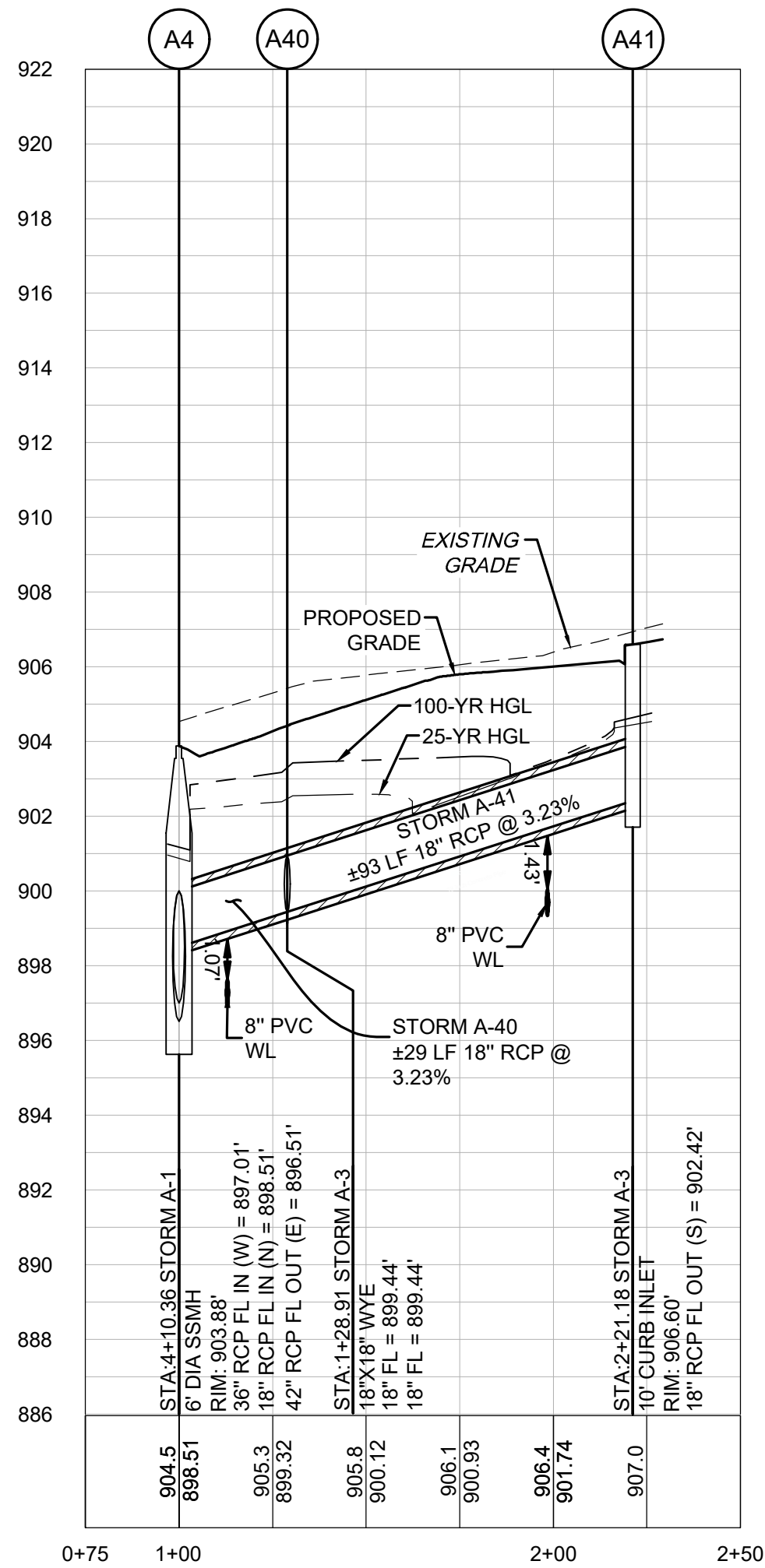
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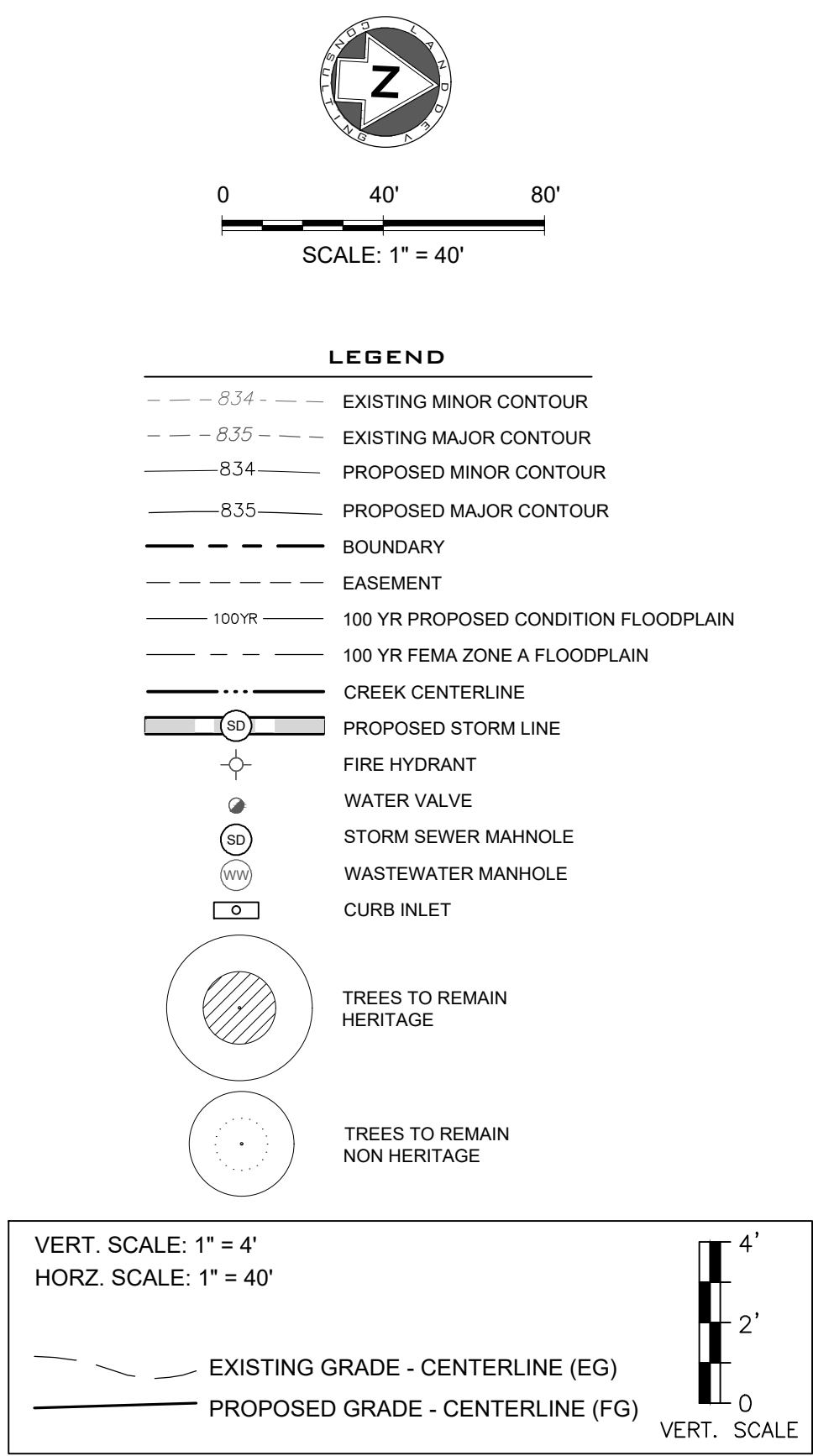
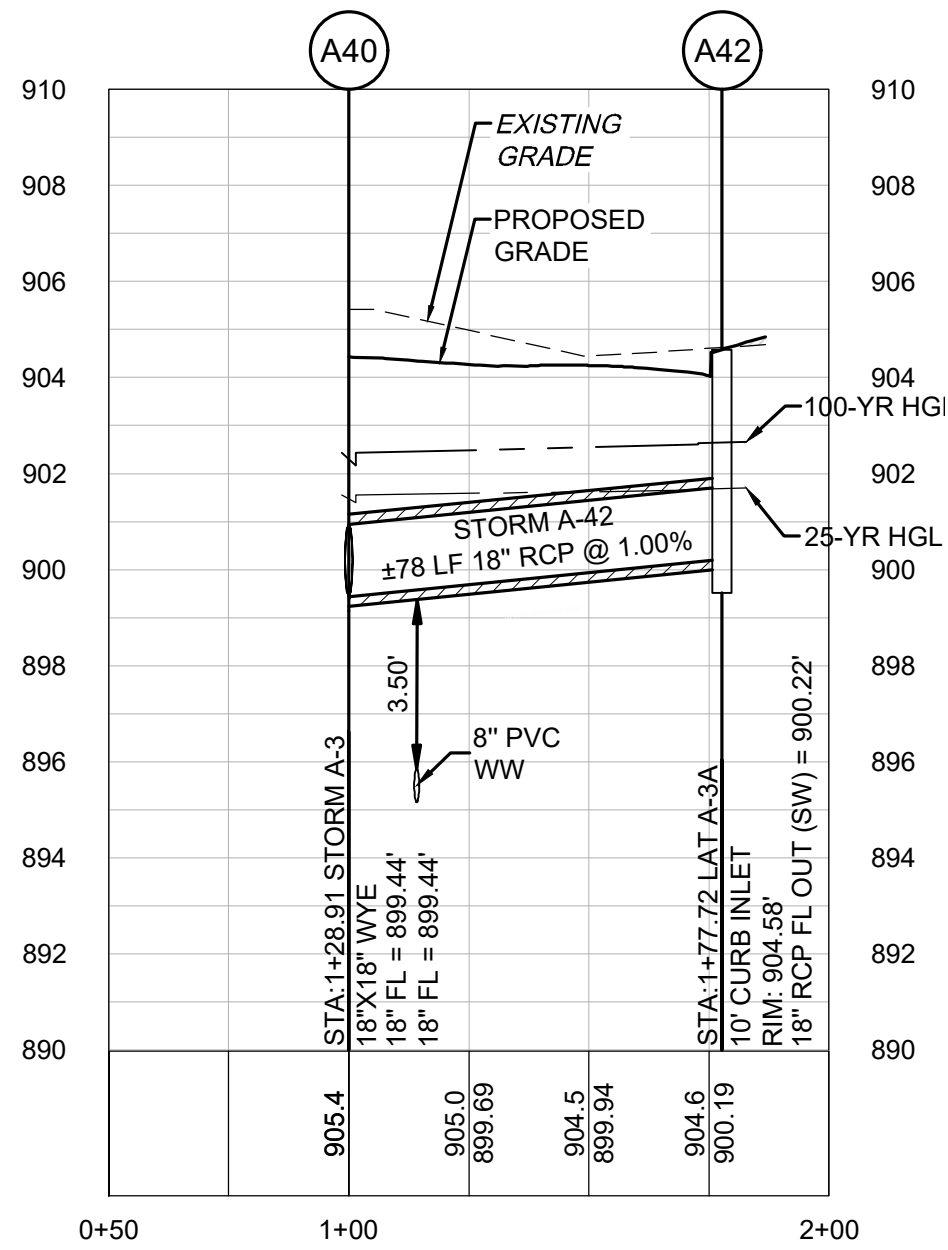
P:\blake\_magnel\parkside sections 8, 9a & 10a\03\_ACAD\Plan\sh-203206\_SGRP\_STORM A-3 PLAN & PROFILE 1+00 - END December 19, 2023, 10:27 AM campbell



STORM A-3



LAT A-3A



NOTE:

1. ALL PROPOSED STORM SEWER PIPE LINES SHALL BE CLASS III REINFORCED CONCRETE UNLESS NOTED OTHERWISE.
2. FILL SHALL BE PLACED ACCORDING TO THE GEOTECHNICAL ENGINEERS RECOMMENDATION AND CITY OF GEORGETOWN SPECIFICATIONS.
3. VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.

TRENCH SAFETY NOTES:

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| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM A-40 | 3.23%     | 8.77      | 4.96       | 2.68     | 11.17      | 6.32        | 3.33      |
| STORM A-41 | 3.23%     | 4.83      | 3.72       | 2.10     | 6.18       | 4.34        | 2.98      |
| STORM A-42 | 1.00%     | 3.94      | 2.25       | 2.11     | 4.99       | 2.82        | 2.99      |

DESIGNED BY: CC

DRAWN BY: MM/MKM

CHECKED BY: SN

APPROVED BY:

811

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11/15/2023

STORM A-3 PLAN &  
PROFILE 1+00 - END

PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS

GEORGETOWN, WILLIAMSON, TEXAS

SHEET 52 OF 84

2023-XX-CON

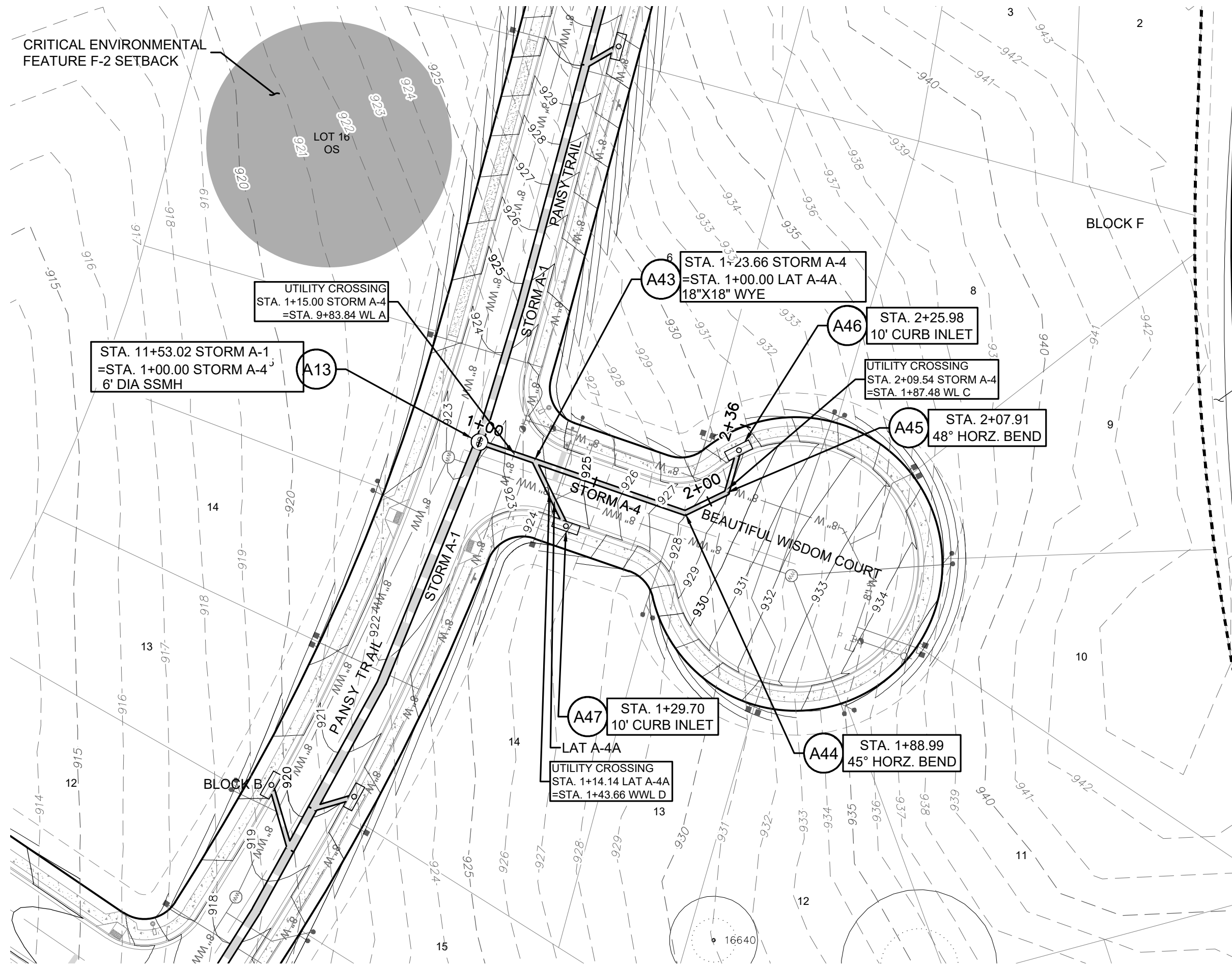
REVISION

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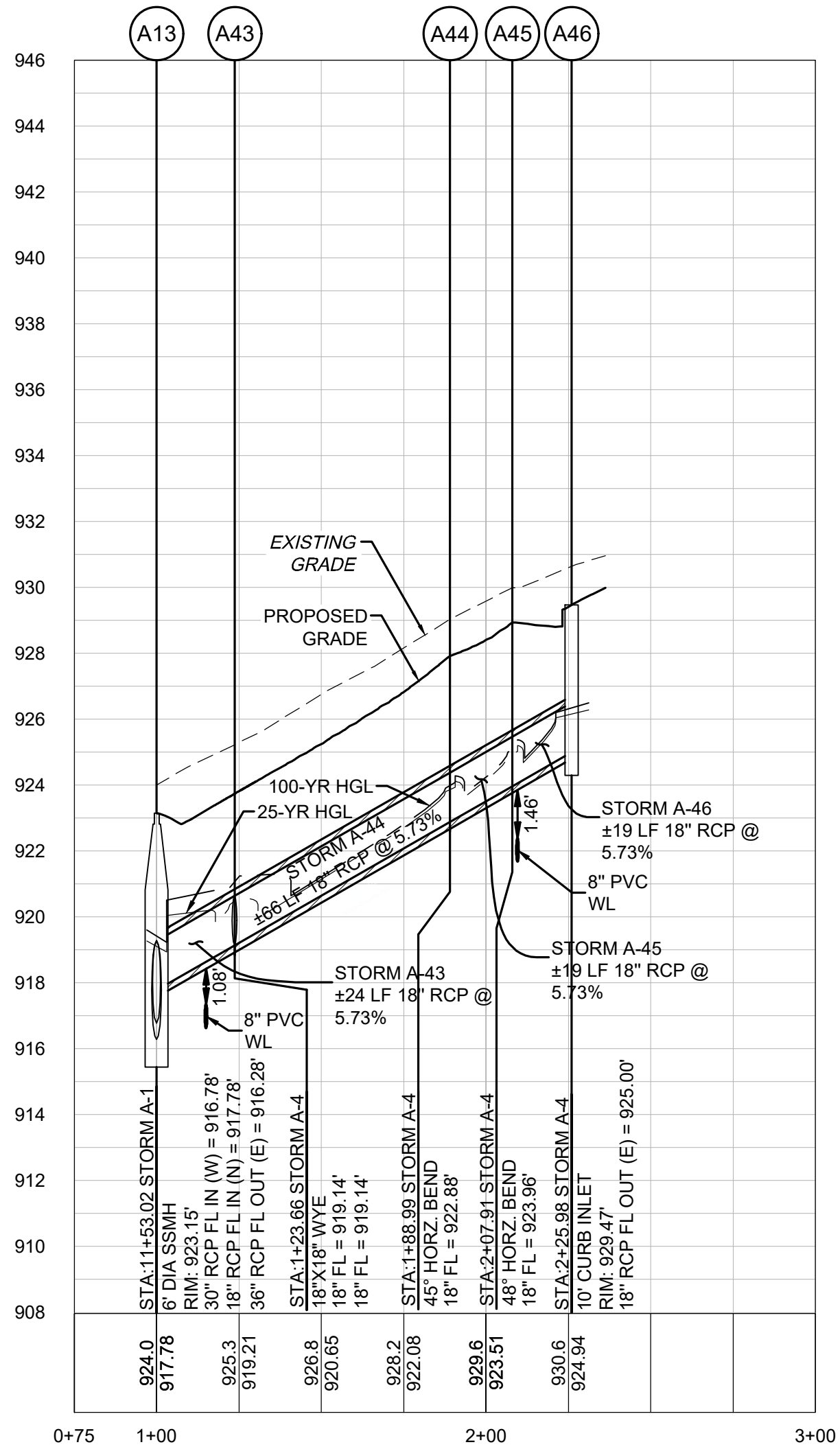
DATE



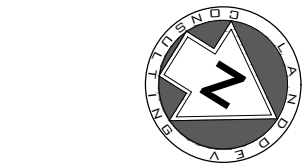
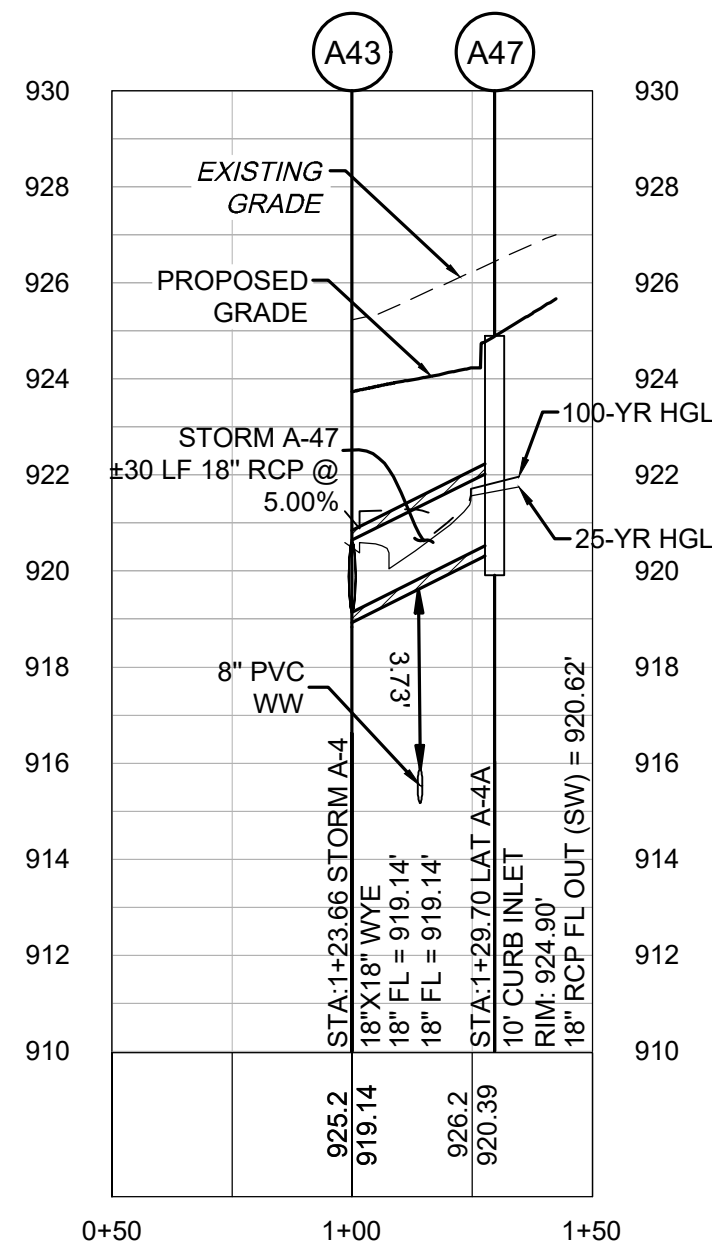
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STORM A-4



LAT A-4A

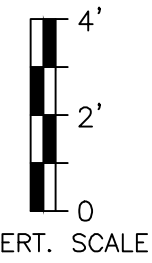


0 40' 80'  
SCALE: 1" = 40'

LEGEND

- 8.34 --- EXISTING MINOR CONTOUR
- 8.35 --- EXISTING MAJOR CONTOUR
- 8.34 --- PROPOSED MINOR CONTOUR
- 8.35 --- PROPOSED MAJOR CONTOUR
- --- BOUNDARY
- --- EASEMENT
- 100YR --- 100 YR PROPOSED CONDITION FLOODPLAIN
- 100YR --- 100 YR FEMA ZONE A FLOODPLAIN
- --- CREEK CENTERLINE
- --- PROPOSED STORM LINE
- --- FIRE HYDRANT
- --- WATER VALVE
- --- STORM SEWER MAHOLE
- --- WASTEWATER MAHOLE
- --- CURB INLET
- --- TREES TO REMAIN HERITAGE
- --- TREES TO REMAIN NON HERITAGE

VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'



NOTE:

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- VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.

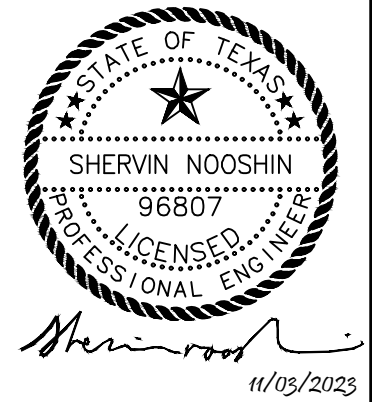
TRENCH SAFETY NOTES:

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| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM A-43 | 5.73%     | 10.43     | 6.29       | 2.26     | 13.19      | 7.46        | 2.73      |
| STORM A-44 | 5.73%     | 5.60      | 4.09       | 1.45     | 7.11       | 4.75        | 2.11      |
| STORM A-45 | 5.73%     | 5.60      | 4.28       | 1.24     | 7.11       | 4.81        | 1.41      |
| STORM A-46 | 5.73%     | 5.60      | 4.28       | 1.24     | 7.11       | 4.81        | 1.41      |
| STORM A-47 | 5.00%     | 4.83      | 3.74       | 1.45     | 6.08       | 4.29        | 2.10      |



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SUITE 100  
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512.872.6996  
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TPLS NO: 10104101

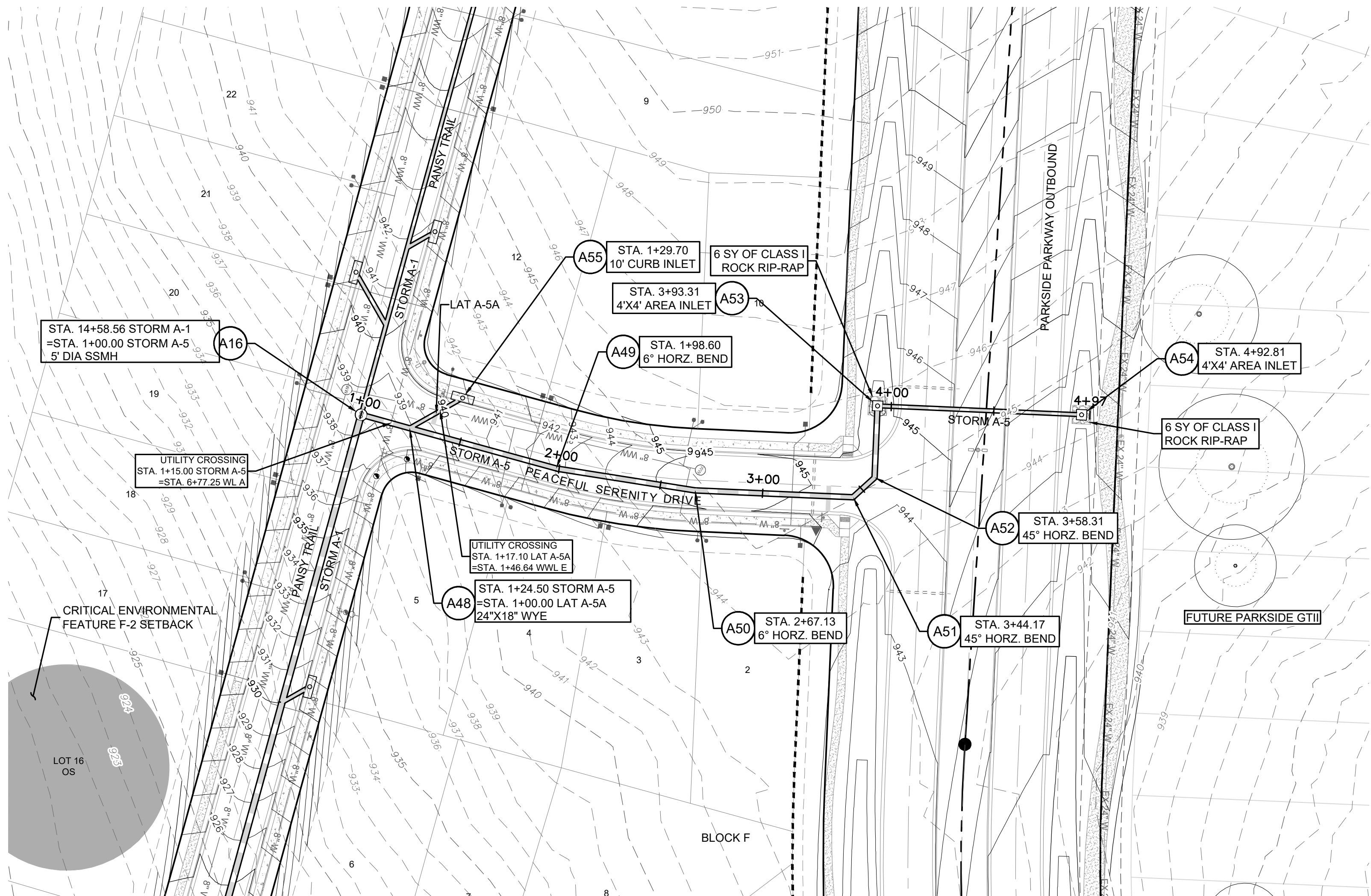


STORM A-4 PLAN &  
PROFILE 1+00 - END  
PARKSIDE SECTION 8  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

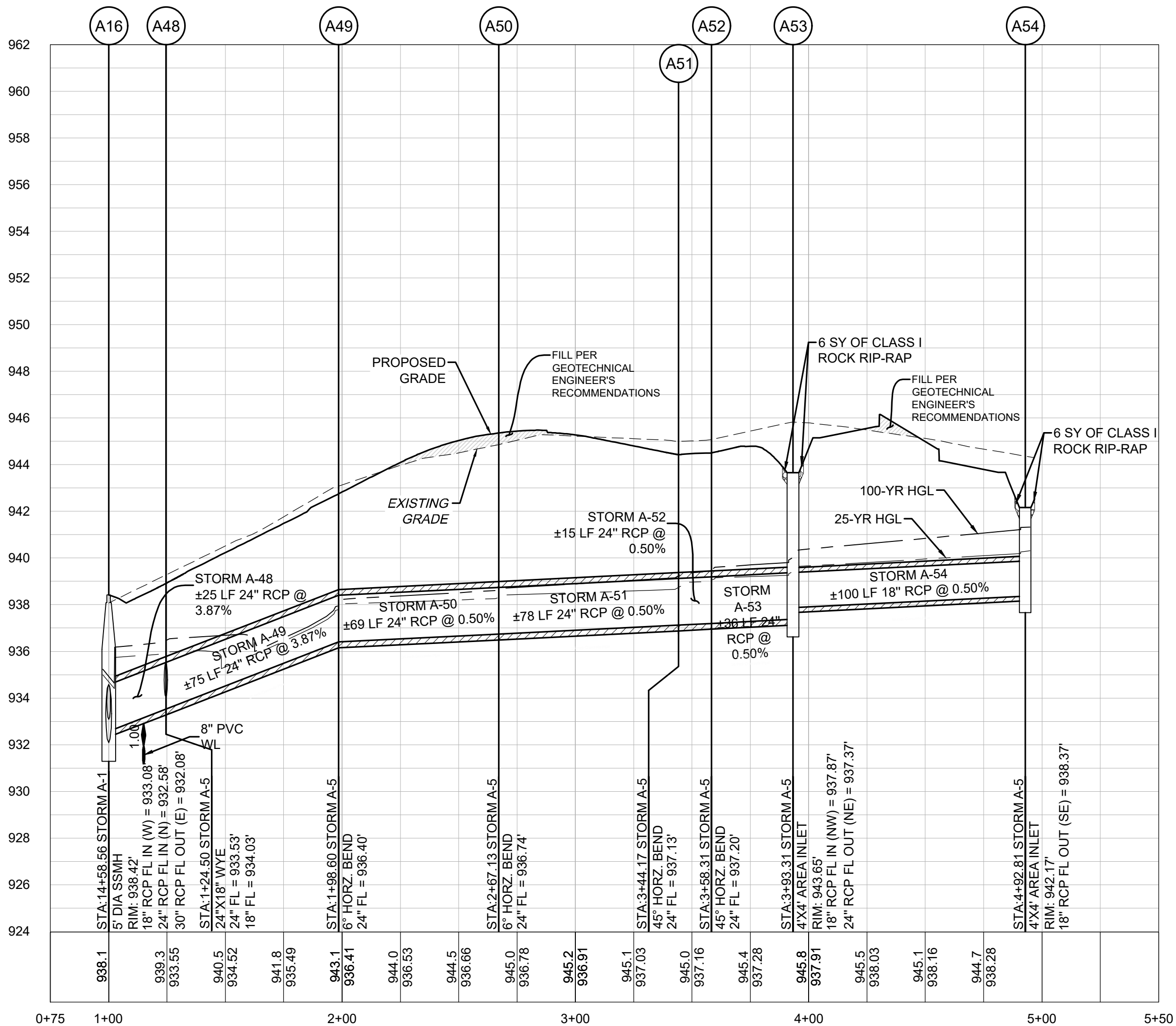
DESIGNED BY: CC  
DRAWN BY: TG/MKM  
CHECKED BY: SN  
APPROVED BY:



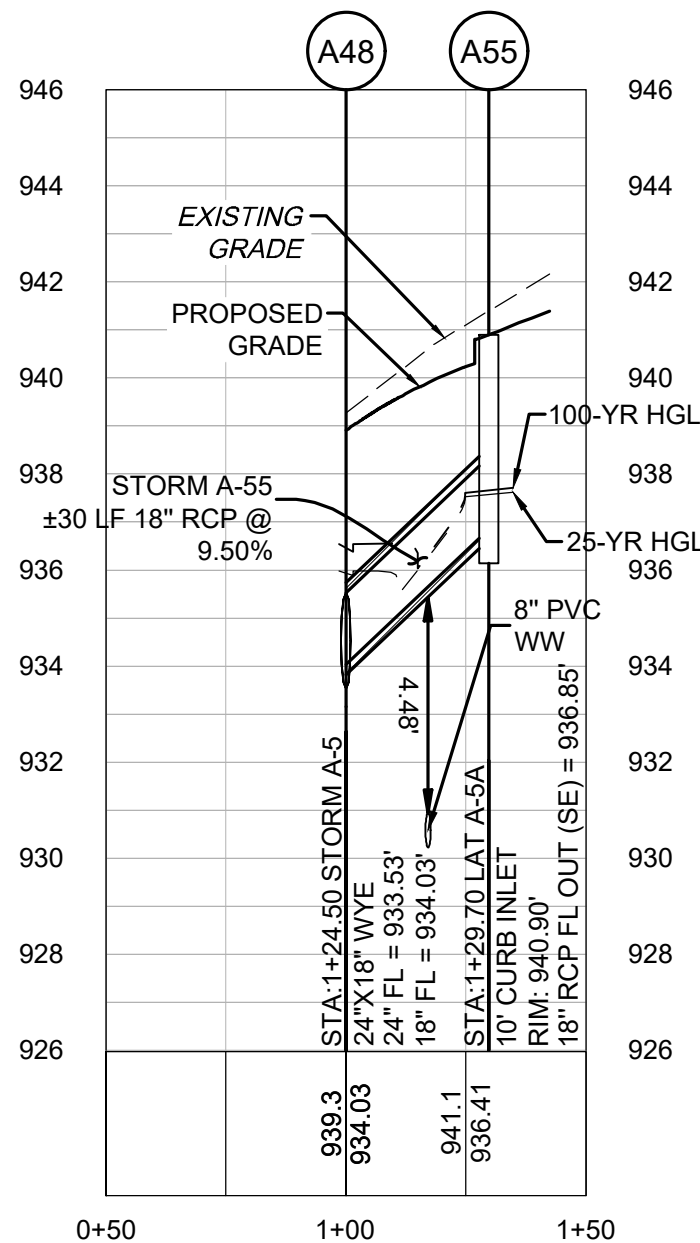
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STORM A-5

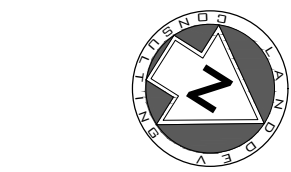


LAT A-5A



| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM A-48 | 3.87%     | 16.73     | 5.33       | 3.16     | 20.91      | 6.66        | 3.59      |
| STORM A-49 | 3.87%     | 14.05     | 5.35       | 2.45     | 17.56      | 6.25        | 3.01      |
| STORM A-50 | 0.50%     | 14.05     | 5.30       | 1.64     | 17.56      | 5.80        | 1.86      |
| STORM A-51 | 0.50%     | 14.05     | 5.25       | 1.67     | 17.56      | 5.60        | 2.02      |
| STORM A-52 | 0.50%     | 14.05     | 4.71       | 1.82     | 17.56      | 5.59        | 2.22      |
| STORM A-53 | 0.50%     | 14.05     | 4.54       | 1.95     | 17.56      | 5.59        | 2.44      |
| STORM A-54 | 0.50%     | 7.86      | 4.45       | 1.82     | 9.83       | 5.56        | 2.84      |
| STORM A-55 | 9.50%     | 2.68      | 2.70       | 1.95     | 3.35       | 3.03        | 2.51      |

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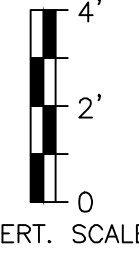


0 40' 80'  
SCALE: 1" = 40'

LEGEND

- EXISTING MINOR CONTOUR
- EXISTING MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- BOUNDARY
- EASEMENT
- 100 YR PROPOSED CONDITION FLOODPLAIN
- 100 YR FEMA ZONE A FLOODPLAIN
- CREEK CENTERLINE
- PROPOSED STORM LINE
- FIRE HYDRANT
- WATER VALVE
- STORM SEWER MAHOLE
- WASTEWATER MAHOLE
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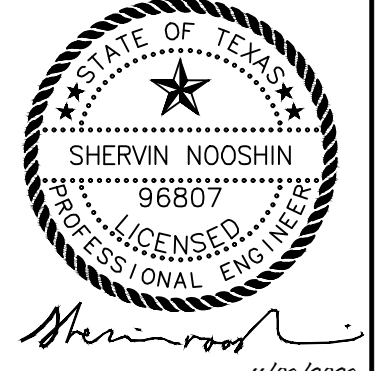
VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'



- EXISTING GRADE - CENTERLINE (EG)
- PROPOSED GRADE - CENTERLINE (FG)



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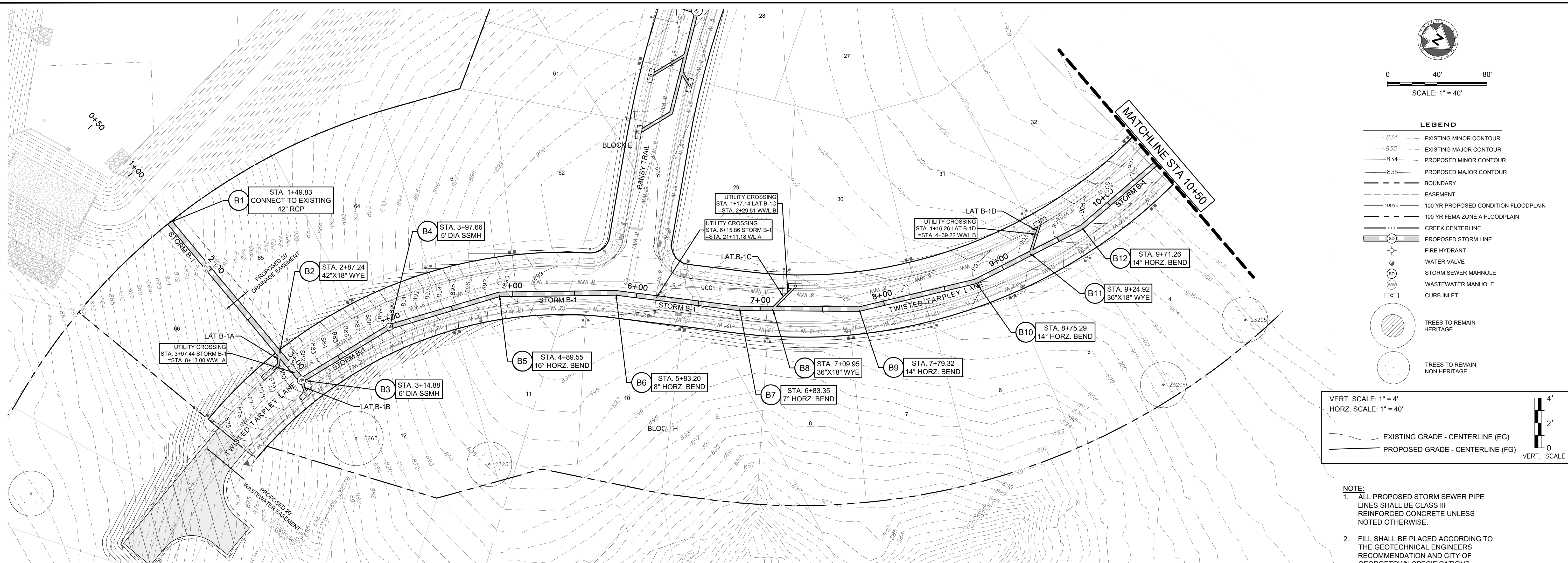


STORM A-5 PLAN &  
PROFILE 1+00 - END  
PARKSIDE SECTION 8  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: TGM/KM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
SHEET 54 OF 72  
2023-XX-CON



P:\bldg\magnificent\sections\_8a & 8b\0303\_ACAD\Plan\sh-203206\_SDP\STORM B-1 PLAN & PROFILE 1+00 - 10+50.dwg, December 19, 2023, 10:25 AM, ccmagbll

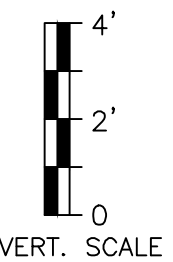


0 40' 80'  
SCALE: 1" = 40'

#### LEGEND

- EXISTING MINOR CONTOUR
- EXISTING MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- BOUNDARY
- EASEMENT
- 100 YR PROPOSED CONDITION FLOODPLAIN
- 100 YR FEMA ZONE A FLOODPLAIN
- CREEK CENTERLINE
- PROPOSED STORM LINE
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- TREES TO REMAIN HERITAGE
- TREES TO REMAIN NON HERITAGE

VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'



#### NOTE:

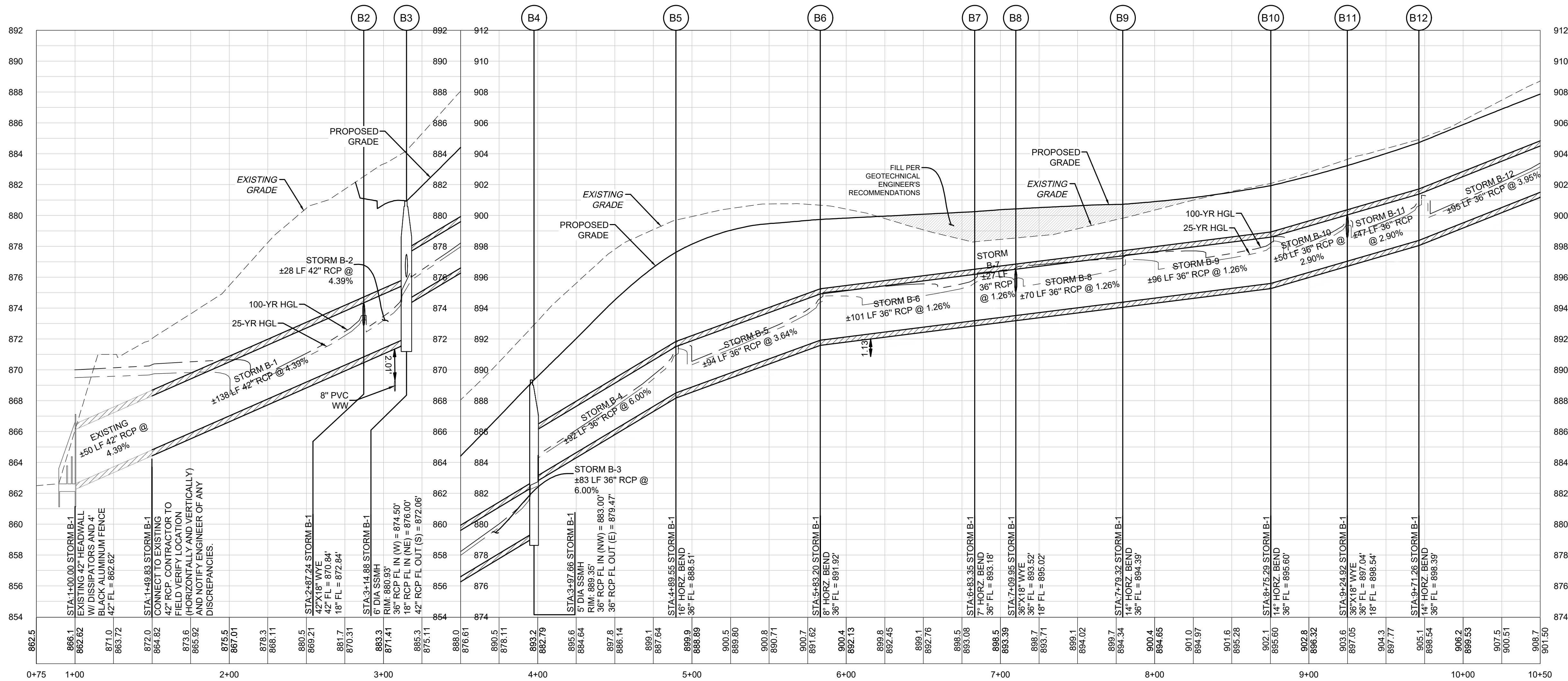
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| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM B-1  | 4.39%     | 57.77     | 7.15       | 4.94     | 72.55      | 8.38        | 5.58      |
| STORM B-2  | 4.39%     | 56.44     | 8.14       | 2.39     | 70.93      | 9.02        | 2.69      |
| STORM B-3  | 6.00%     | 52.93     | 13.40      | 2.36     | 66.62      | 14.48       | 2.61      |
| STORM B-4  | 6.00%     | 52.93     | 13.57      | 2.36     | 66.62      | 14.65       | 2.61      |
| STORM B-5  | 3.64%     | 52.93     | 8.24       | 2.87     | 66.62      | 9.81        | 3.09      |
| STORM B-6  | 1.26%     | 52.93     | 8.24       | 2.87     | 66.62      | 9.81        | 3.09      |
| STORM B-7  | 1.26%     | 52.93     | 8.24       | 2.87     | 66.62      | 9.42        | 3.09      |
| STORM B-8  | 1.26%     | 48.65     | 8.19       | 2.44     | 61.20      | 8.78        | 3.18      |
| STORM B-9  | 1.26%     | 48.65     | 7.81       | 2.76     | 61.20      | 9.16        | 3.06      |
| STORM B-10 | 2.90%     | 48.65     | 7.81       | 2.76     | 61.20      | 9.16        | 3.02      |
| STORM B-11 | 2.90%     | 44.32     | 7.82       | 2.32     | 55.84      | 8.80        | 2.65      |
| STORM B-12 | 3.95%     | 44.32     | 7.42       | 2.63     | 55.84      | 8.54        | 2.93      |

### STORM B-1



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H3GREEN.COM

TYPE NO: 14384  
TPLS NO: 10184101

H3Green  
DEVELOPMENT TX

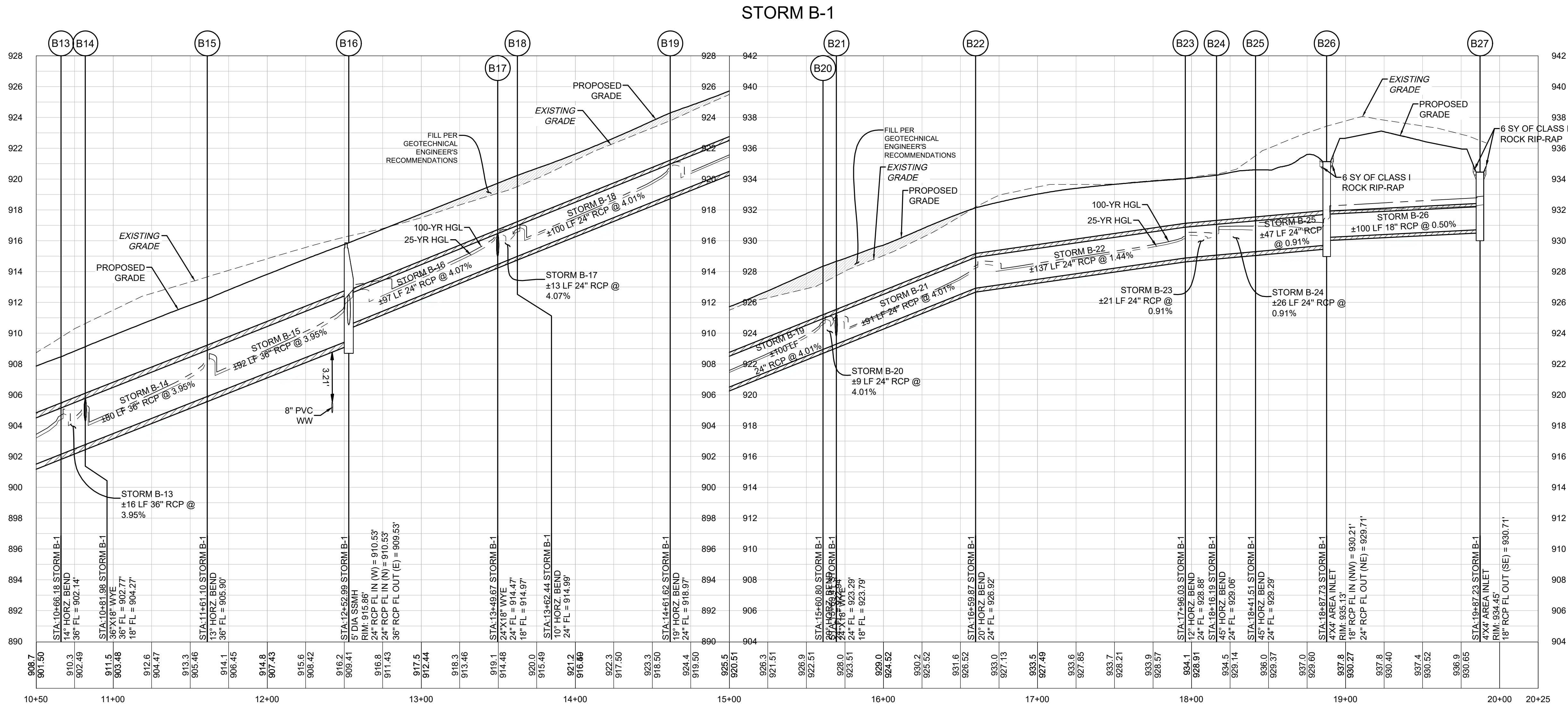
STORM B-1 PLAN &  
PROFILE 1+00 - 10+50  
PARKSIDE SECTION B  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: TGM/KM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_

SHEET 55 OF 72  
2023-XX-CON



P:\parkside\parkside sections 8, 9 & 10\03\_03\_ACAD\Plan\1030206 SGP\STORM B-1.dwg, STORM B-1 PLAN & PROFILE, 10-50 - END, December 19, 2023, 10:26 AM, c:\campbell



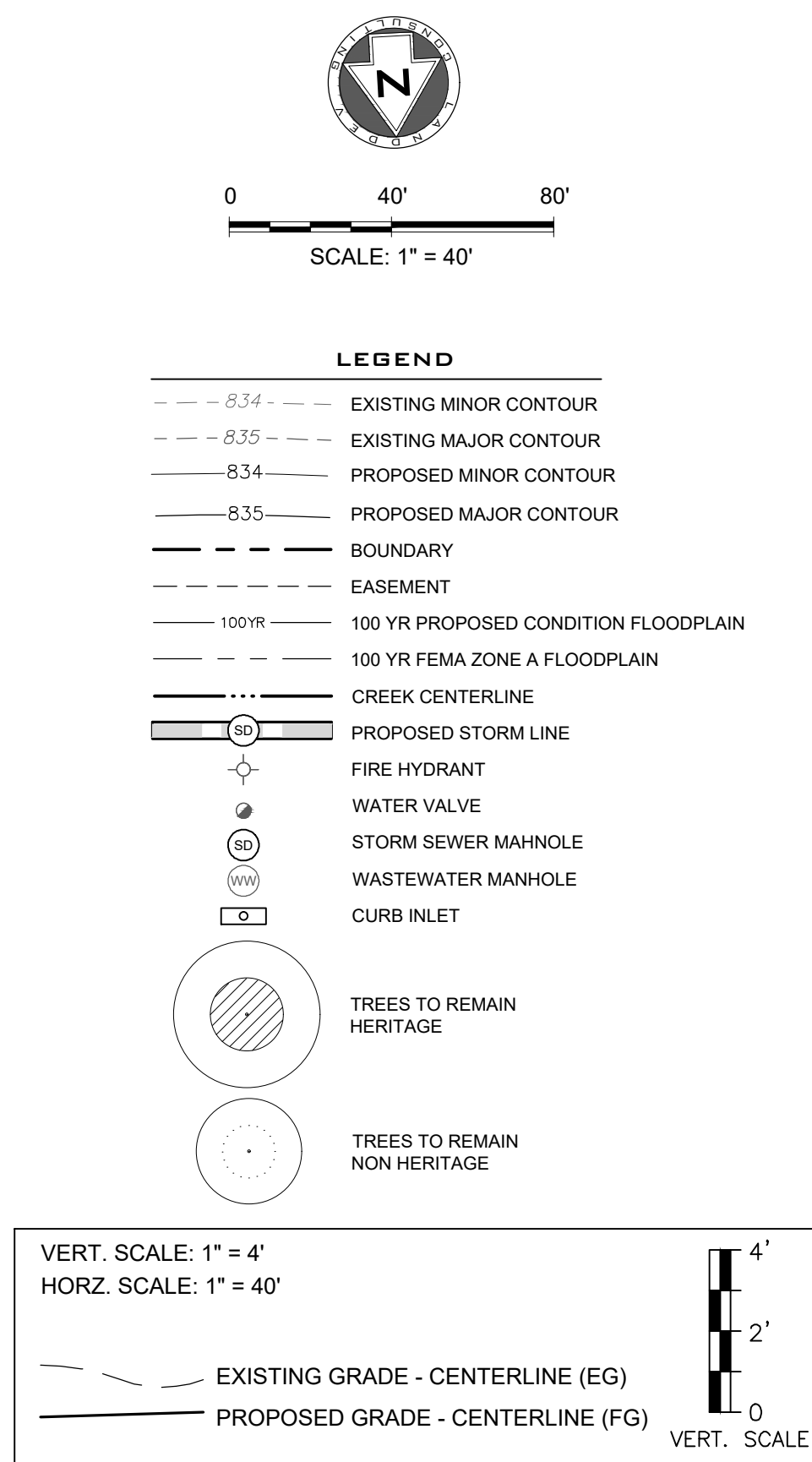
| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft/s) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|------------|------------|-------------|-----------|
| STORM B-13 | 3.95%     | 44.32     | 7.42       | 2.63       | 55.84      | 8.54        | 2.93      |
| STORM B-14 | 3.95%     | 39.17     | 7.39       | 2.18       | 49.38      | 8.17        | 2.52      |
| STORM B-15 | 3.95%     | 39.17     | 6.98       | 2.47       | 49.38      | 7.89        | 2.78      |
| STORM B-16 | 4.07%     | 23.13     | 7.73       | 2.23       | 29.15      | 9.45        | 2.59      |
| STORM B-17 | 4.07%     | 17.72     | 6.32       | 1.93       | 22.32      | 7.50        | 2.22      |
| STORM B-18 | 4.01%     | 17.72     | 6.40       | 1.84       | 22.32      | 7.50        | 2.02      |
| STORM B-19 | 4.01%     | 17.72     | 6.31       | 1.95       | 22.32      | 7.50        | 2.11      |
| STORM B-20 | 4.01%     | 17.72     | 6.31       | 1.95       | 22.32      | 7.50        | 2.11      |
| STORM B-21 | 4.01%     | 11.62     | 5.06       | 1.59       | 14.55      | 5.54        | 1.88      |
| STORM B-22 | 1.44%     | 11.62     | 5.09       | 1.57       | 14.55      | 5.64        | 1.77      |
| STORM B-23 | 0.91%     | 11.62     | 5.22       | 1.48       | 14.55      | 5.67        | 1.67      |
| STORM B-24 | 0.91%     | 11.62     | 4.57       | 1.66       | 14.55      | 5.02        | 1.86      |
| STORM B-25 | 0.91%     | 11.62     | 4.99       | 1.64       | 14.55      | 5.36        | 1.89      |
| STORM B-26 | 0.50%     | 6.05      | 3.42       | 1.70       | 7.64       | 4.32        | 2.08      |

- TRENCH SAFETY NOTES:**
- IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, ALL TRENCHES OVER 5 FEET IN DEPTH IN EITHER HARD AND COMPACT OR SOFT AND UNSTABLE SOIL SHALL BE SLOPED, SHORED, SHEETED, BRACED OR OTHERWISE SUPPORTED. FURTHERMORE, ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT WILL BE PROVIDED BY THE CONTRACTOR.
  - IN ACCORDANCE WITH THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4- FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED SO AS TO REQUIRE NO MORE THAN 25 FEET OF LATERAL TRAVEL.
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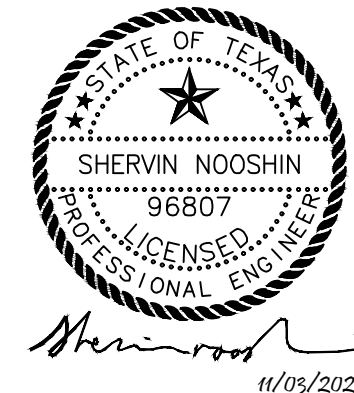
STORM B-1 PLAN &  
PROFILE 10+50 - END  
PARKSIDE SECTION B  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: TGM/KMM  
CHECKED BY: SN  
APPROVED BY:

SHEET 56 OF 72  
2023-XX-CON

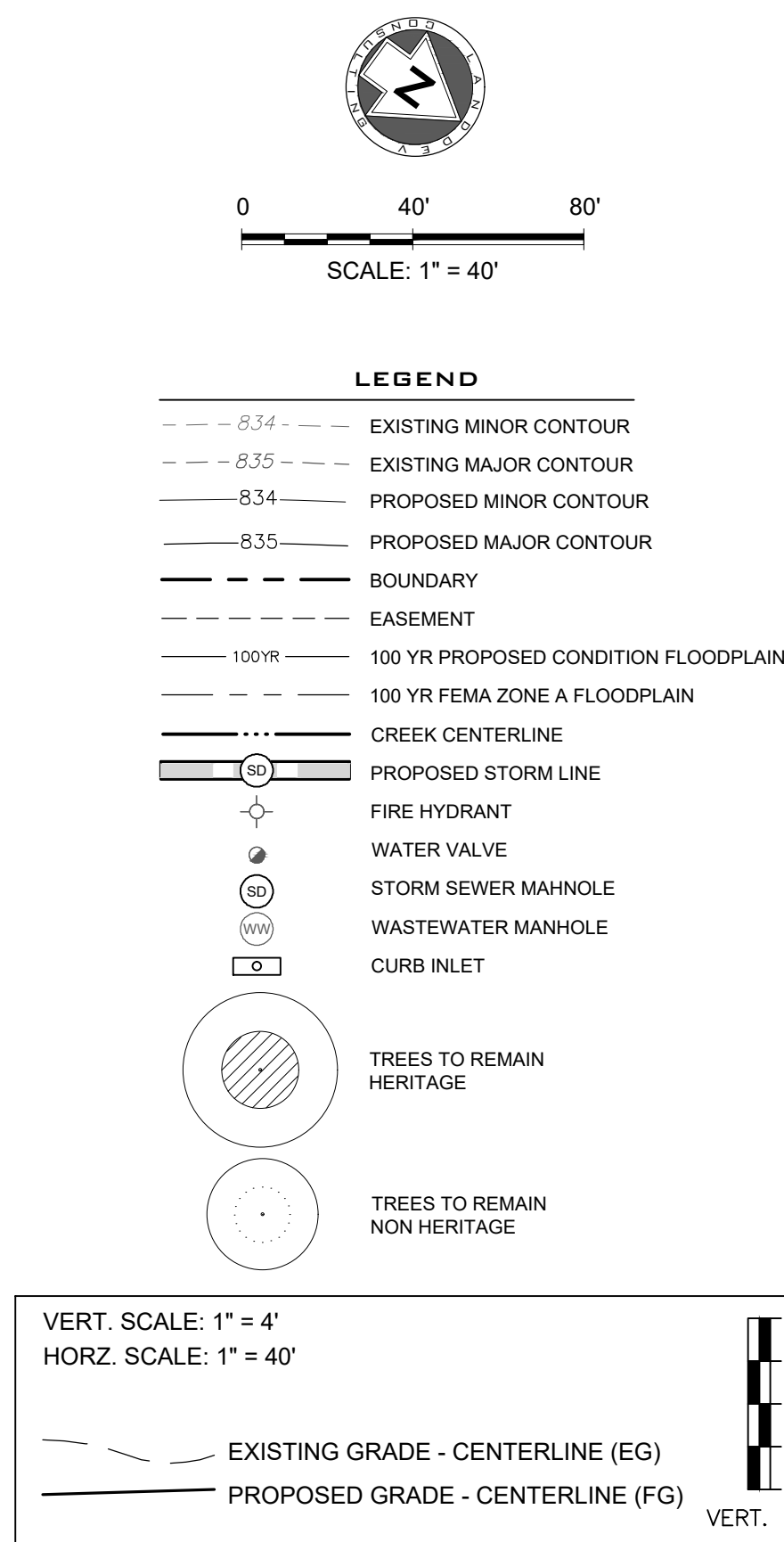


- NOTE:**
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  - FILL SHALL BE PLACED ACCORDING TO THE GEOTECHNICAL ENGINEERS RECOMMENDATION AND CITY OF GEORGETOWN SPECIFICATIONS.
  - VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.



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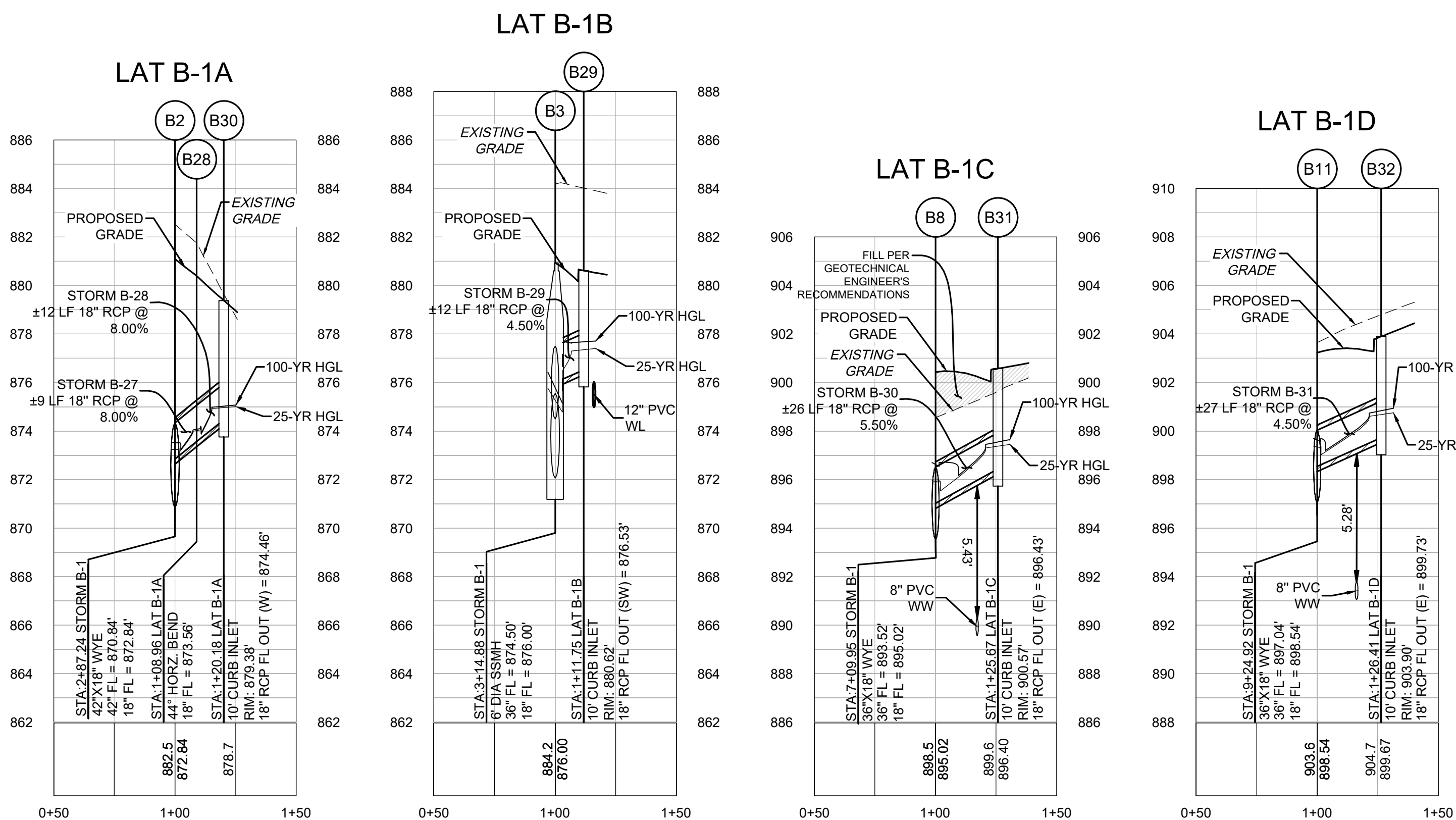




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3. VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.

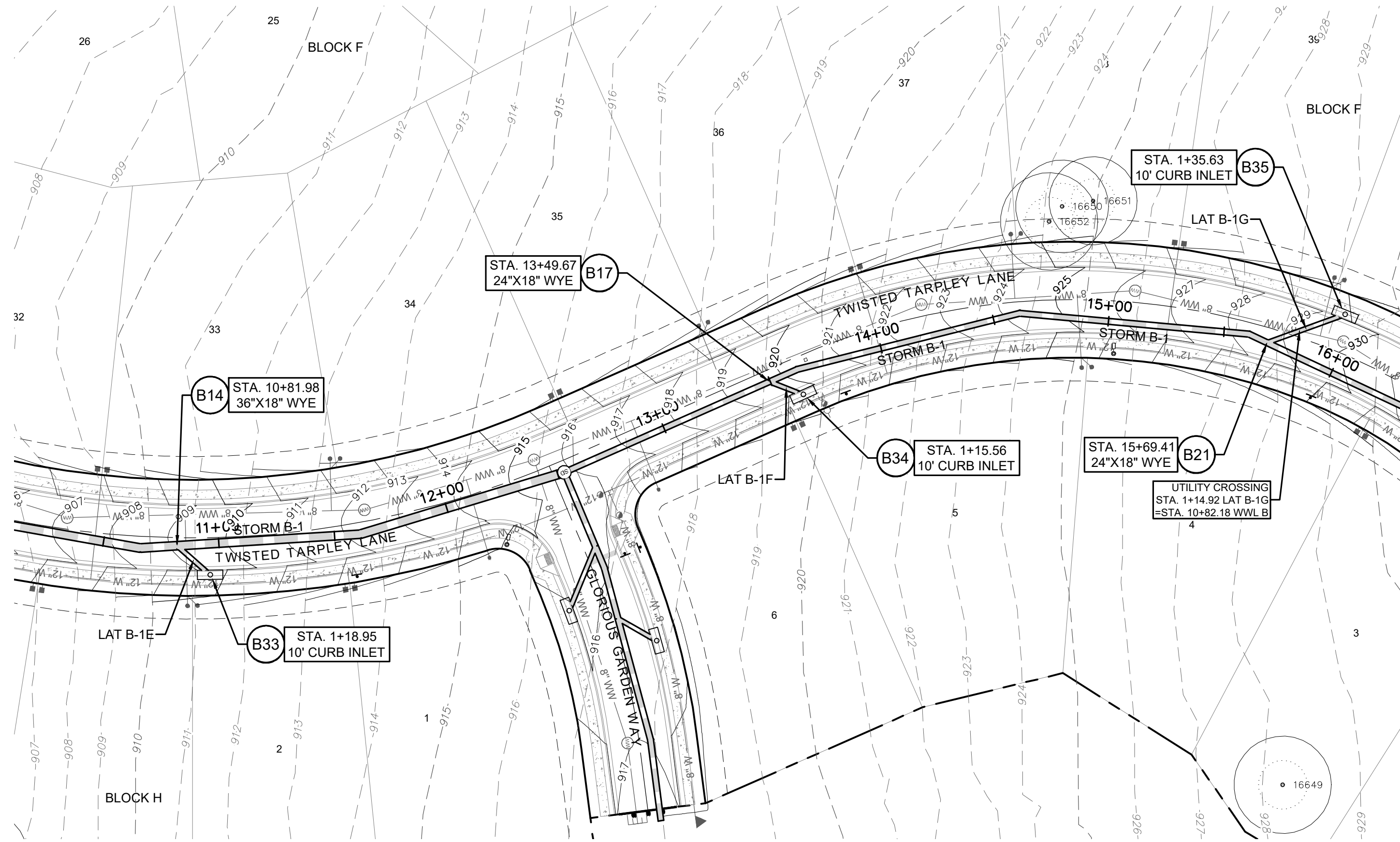
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| Pipe Label | Slope | Q25   | V25    | D25  | Q100  | V100   | D100 |
|------------|-------|-------|--------|------|-------|--------|------|
|            | (%)   | (cfs) | (ft/s) | (ft) | (cfs) | (ft/s) | (ft) |
| STORM B-27 | 8.00% | 1.33  | 5.14   | 0.43 | 1.62  | 2.69   | 0.69 |
| STORM B-28 | 8.00% | 1.33  | 2.66   | 0.57 | 1.62  | 2.81   | 0.63 |
| STORM B-29 | 4.50% | 3.51  | 5.82   | 0.71 | 4.31  | 2.79   | 1.66 |
| STORM B-30 | 5.50% | 4.28  | 4.10   | 0.94 | 5.42  | 3.99   | 1.68 |
| STORM B-31 | 4.50% | 4.33  | 4.43   | 0.83 | 5.36  | 4.29   | 1.15 |





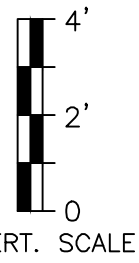
P:\blake\_magnel\parkside sections 8, 9a & 10a\03\_ACAD\Plan\sh-20206 SGRP STORM B-1 LATS.dwg, STORM B-1 LATERALS 2, December 19, 2023, 10:27 AM, campbell



0 40' 80'  
SCALE: 1" = 40'

- LEGEND**
- 8.34 --- EXISTING MINOR CONTOUR
  - 8.35 --- EXISTING MAJOR CONTOUR
  - 8.34 --- PROPOSED MINOR CONTOUR
  - 8.35 --- PROPOSED MAJOR CONTOUR
  - --- BOUNDARY
  - --- EASEMENT
  - 100YR --- 100 YR PROPOSED CONDITION FLOODPLAIN
  - 100 YR FEMA ZONE A FLOODPLAIN
  - --- CREEK CENTERLINE
  - --- PROPOSED STORM LINE
  - --- FIRE HYDRANT
  - --- WATER VALVE
  - --- STORM SEWER MAHNOLE
  - --- WASTEWATER MAHNOLE
  - --- CURB INLET
  - --- TREES TO REMAIN HERITAGE
  - --- TREES TO REMAIN NON HERITAGE

VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'



**NOTE:**

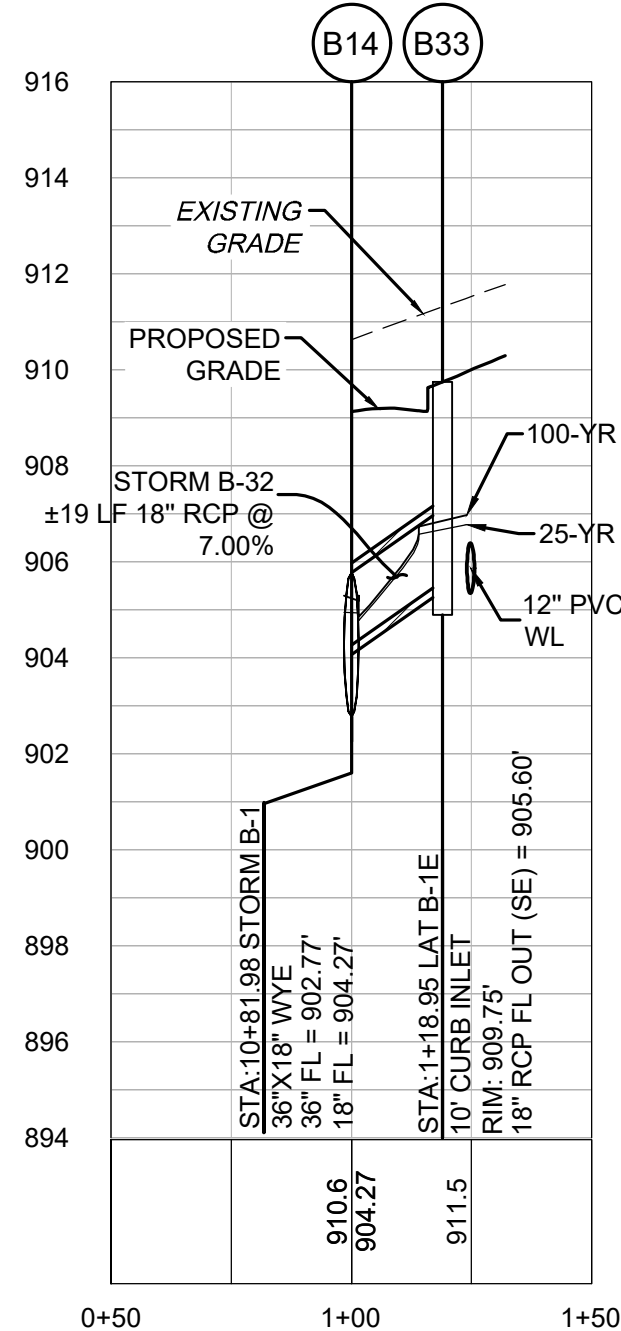
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- VEGETATE ALL DISTURBED AREAS PER CITY OF GEORGETOWN SPECIFICATIONS.

**TRENCH SAFETY NOTES:**

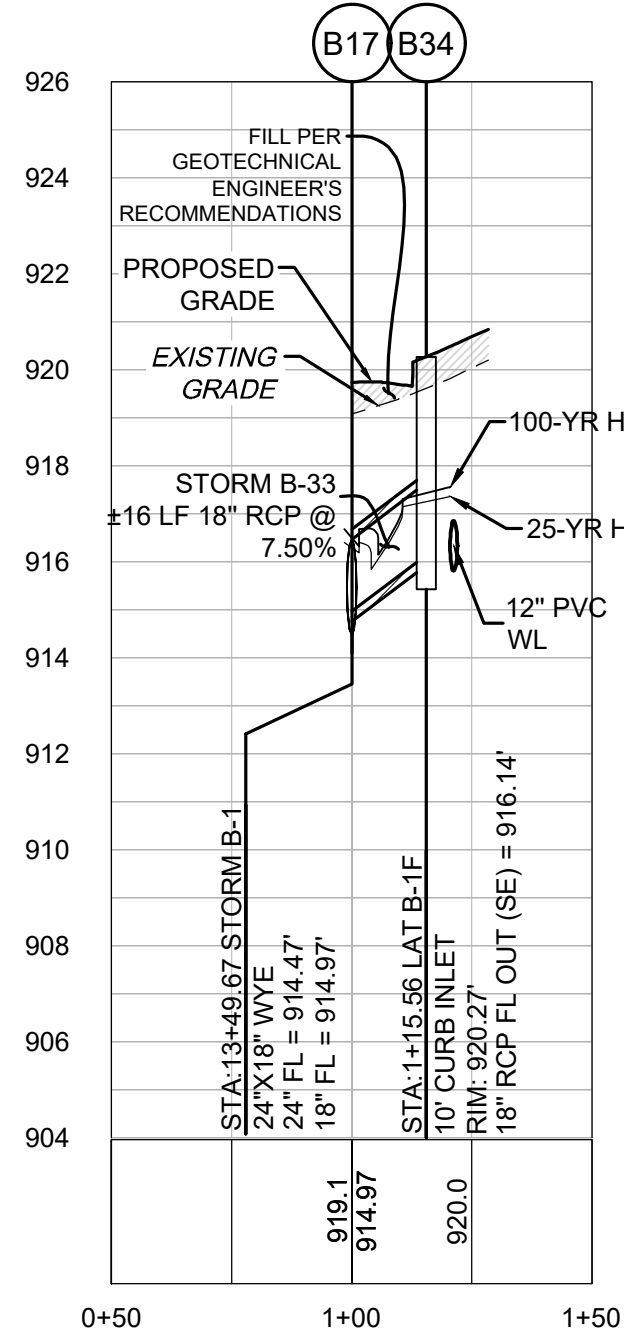
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| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM B-32 | 7.00%     | 5.15      | 7.41       | 0.87     | 6.46       | 5.16        | 1.02      |
| STORM B-33 | 7.50%     | 5.41      | 4.01       | 1.43     | 6.83       | 4.63        | 1.71      |
| STORM B-34 | 5.50%     | 6.10      | 4.79       | 1.09     | 7.77       | 5.14        | 1.38      |

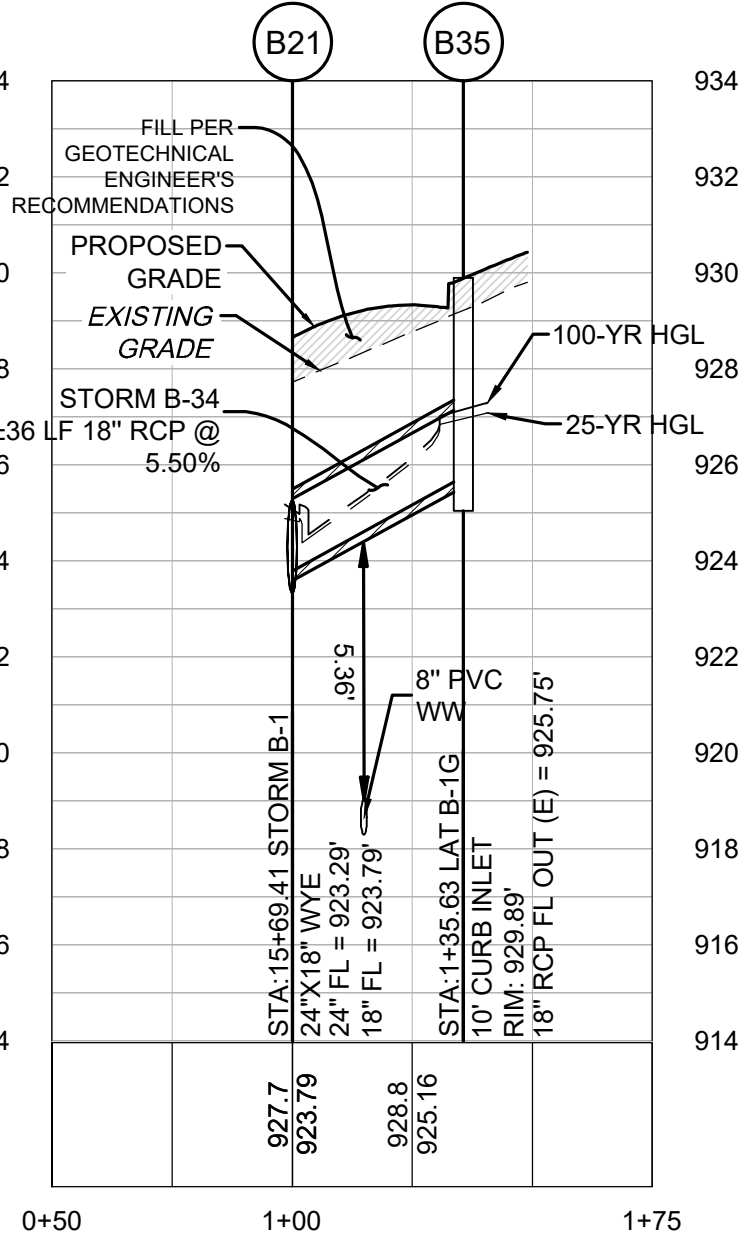
**LAT B-1E**



**LAT B-1F**



**LAT B-1G**



**STORM B-1 LATERALS 2**

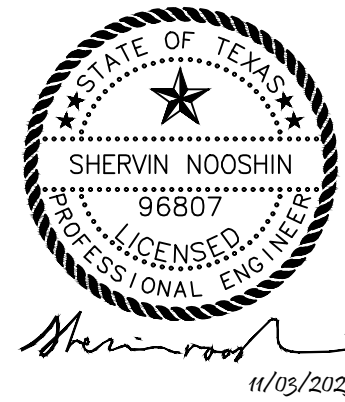
**PARKSIDE SECTION 8  
CONSTRUCTION PLANS**  
**GEORGETOWN, WILLIAMSON, TEXAS**

DESIGNED BY: CC  
DRAWN BY: TG/MKM  
CHECKED BY: SN  
APPROVED BY:

SHEET **58** OF **72**  
**2023-XX-CON**

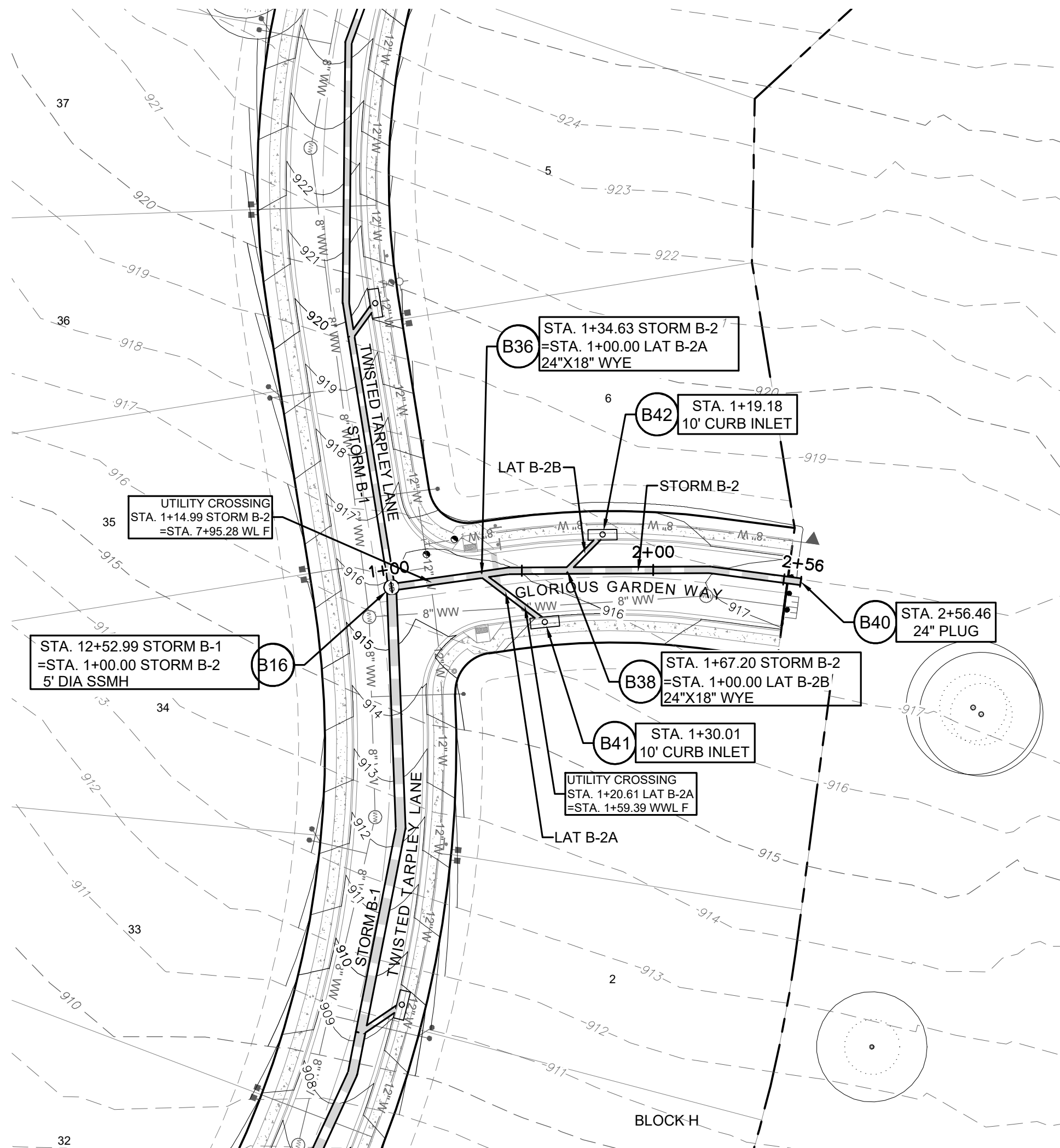
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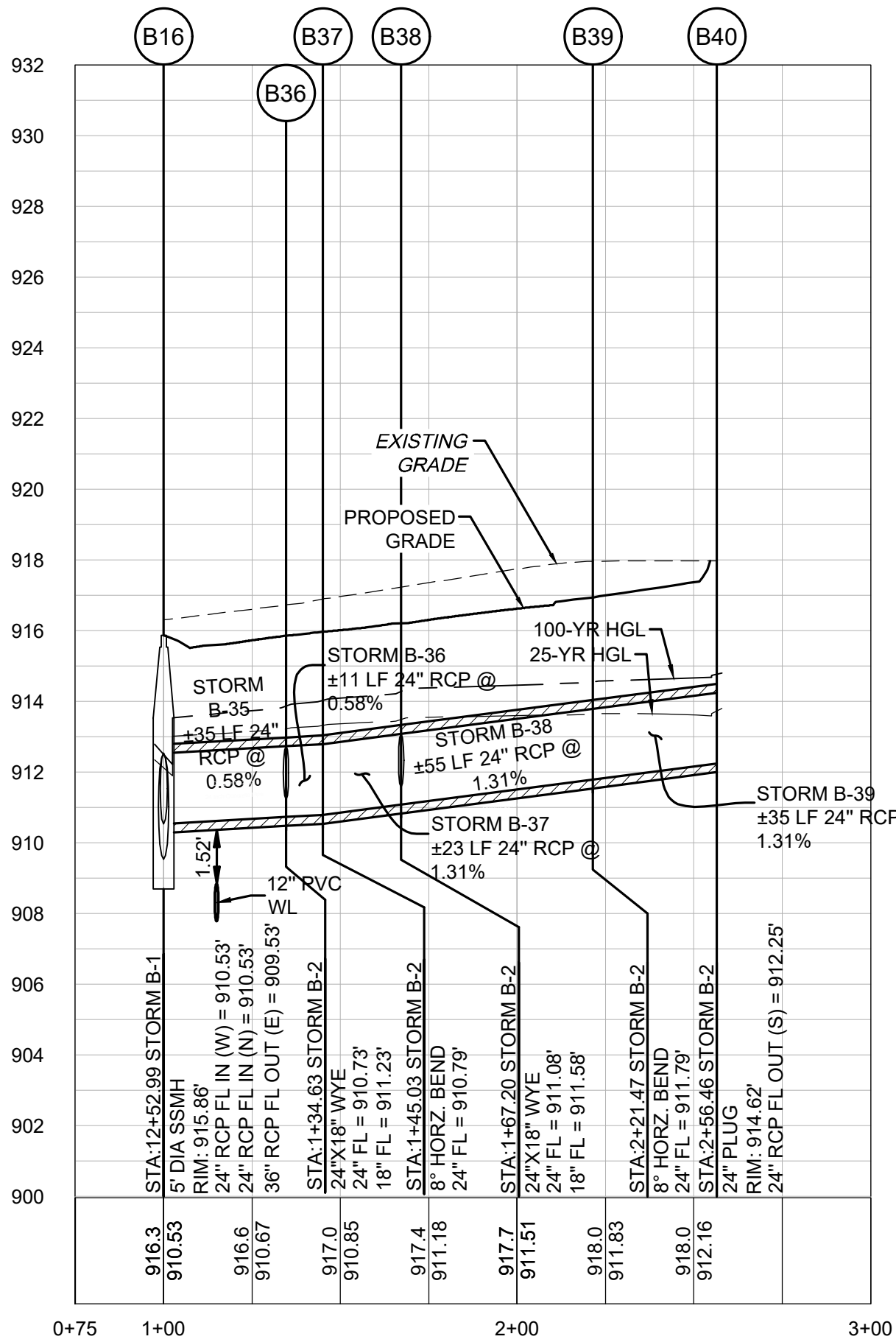




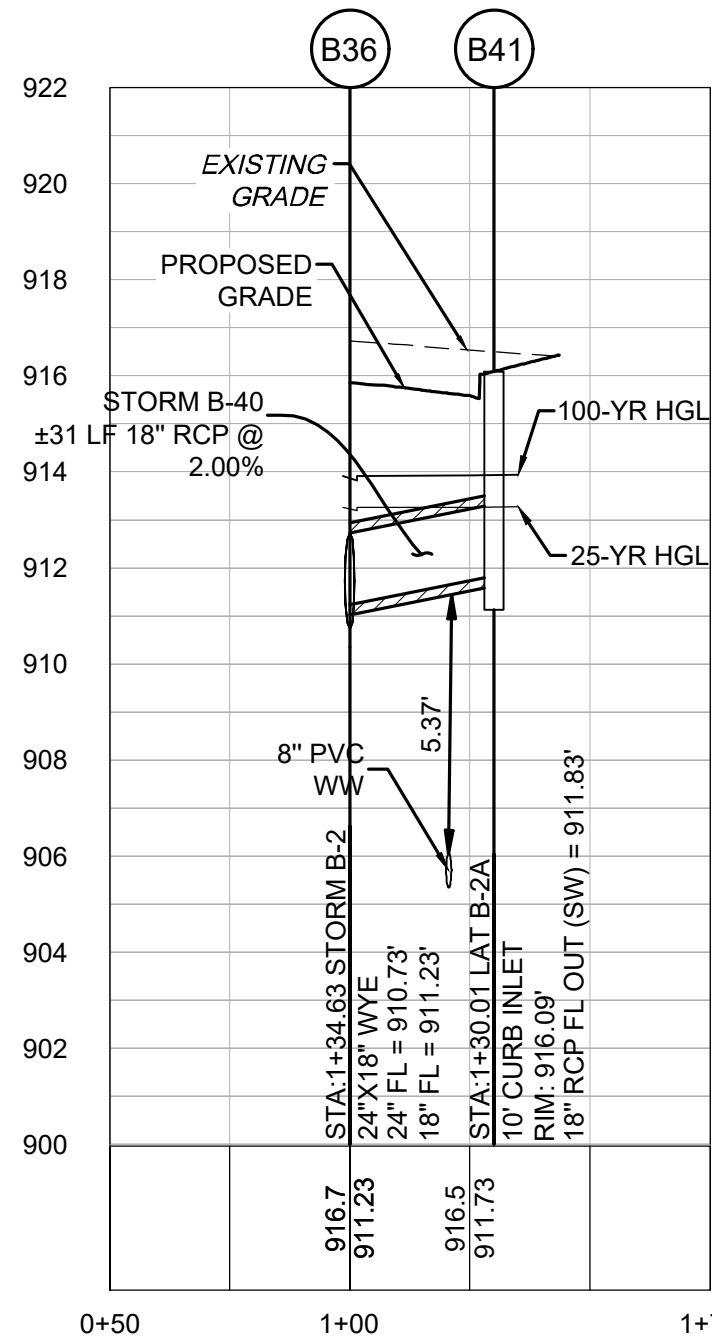
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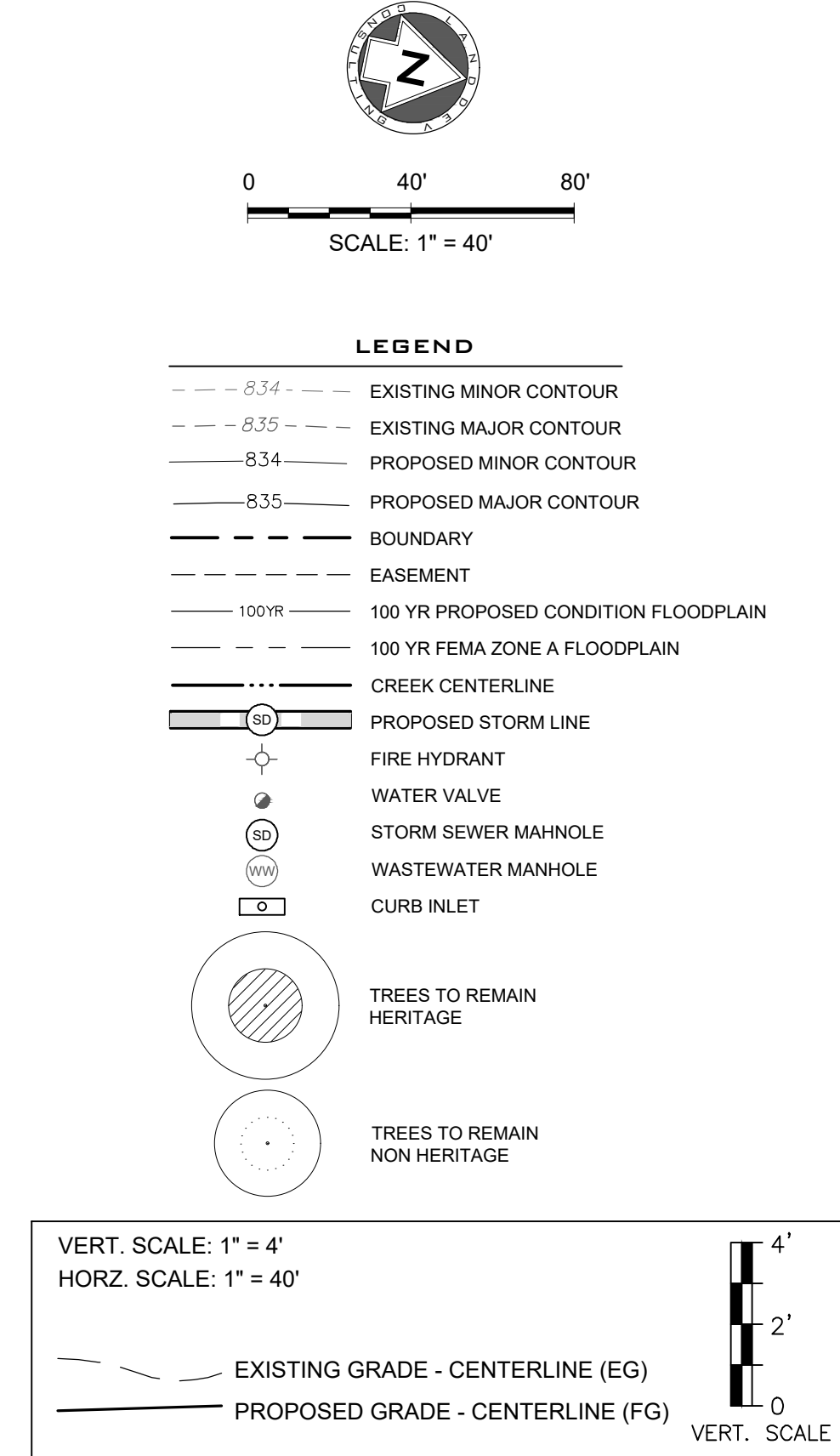
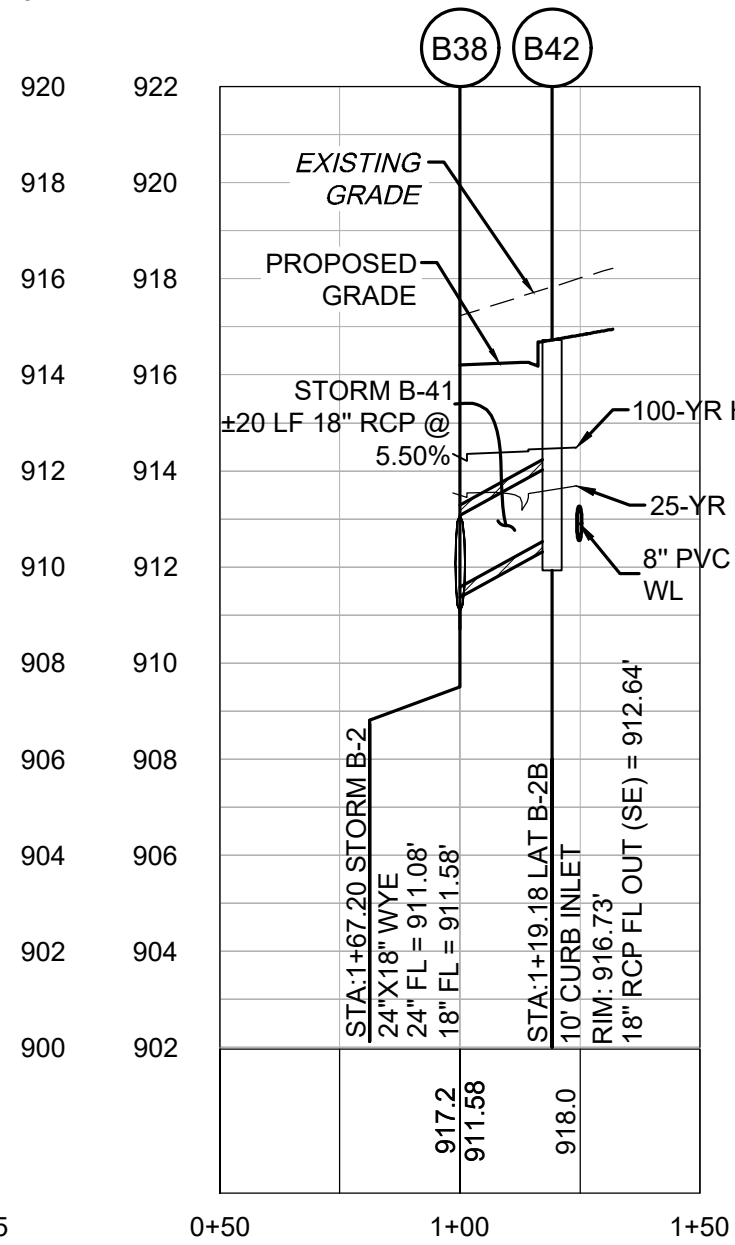
STORM B-2



LAT B-2A



LAT B-2B



NOTE:

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| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM B-35 | 0.58%     | 16.04     | 5.11       | 2.49     | 20.23      | 6.44        | 3.08      |
| STORM B-36 | 0.58%     | 14.48     | 4.61       | 2.53     | 18.31      | 5.83        | 3.19      |
| STORM B-37 | 1.31%     | 14.48     | 4.61       | 2.56     | 18.31      | 5.83        | 3.27      |
| STORM B-38 | 1.31%     | 9.99      | 3.25       | 2.45     | 12.71      | 4.05        | 3.28      |
| STORM B-39 | 1.31%     | 9.99      | 3.87       | 1.87     | 12.71      | 4.05        | 2.78      |
| STORM B-40 | 2.00%     | 1.56      | 0.89       | 2.03     | 1.92       | 1.09        | 2.68      |
| STORM B-41 | 5.50%     | 4.49      | 3.57       | 1.95     | 5.60       | 3.17        | 2.78      |

DESIGNED BY: CC

DRAWN BY: TG/MKM

CHECKED BY: SN

APPROVED BY:

811

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96807  
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11/05/2023

STORM B-2 PLAN &  
PROFILE 1+00 - END

PARKSIDE SECTION B  
CONSTRUCTION PLANS

GEORGETOWN, WILLIAMSON, TEXAS

REVISION

BY

DATE

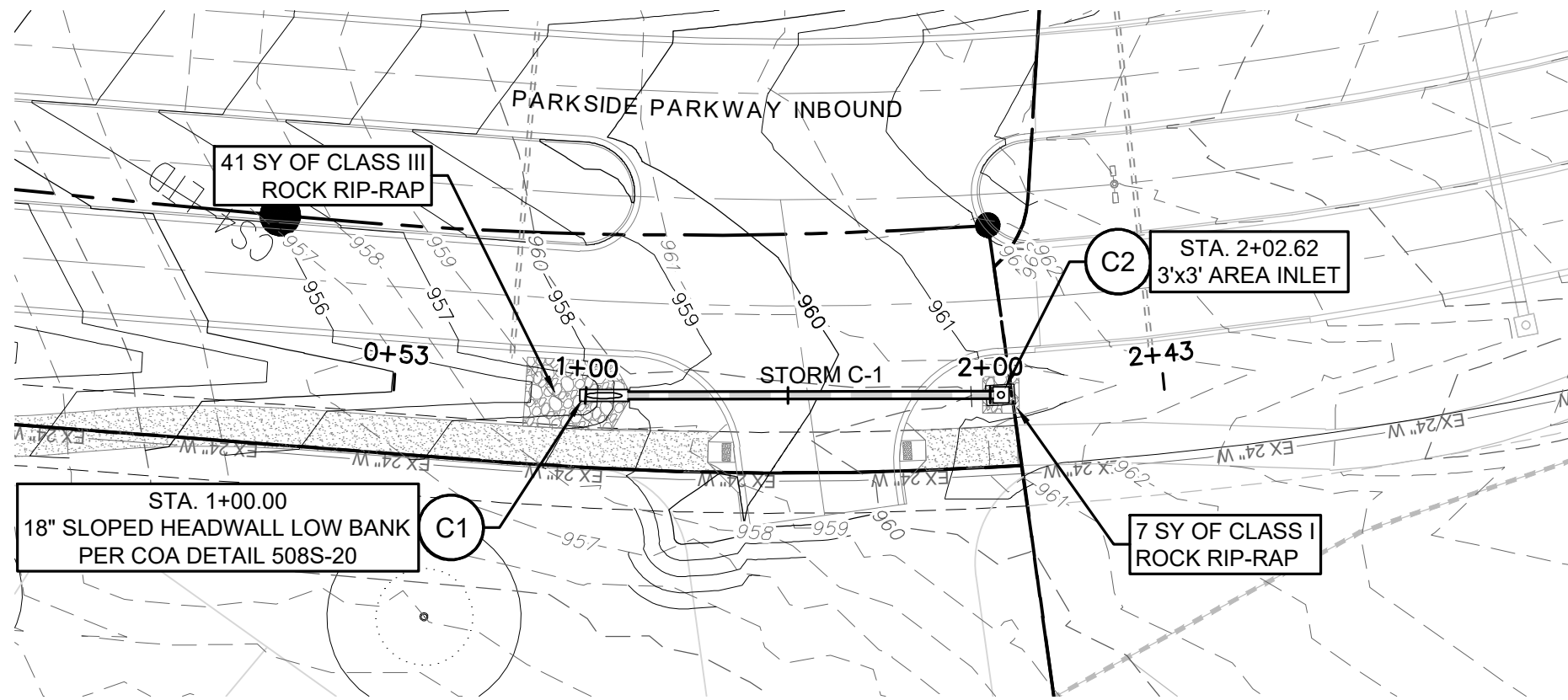
NO.

SHEET 59 OF 72

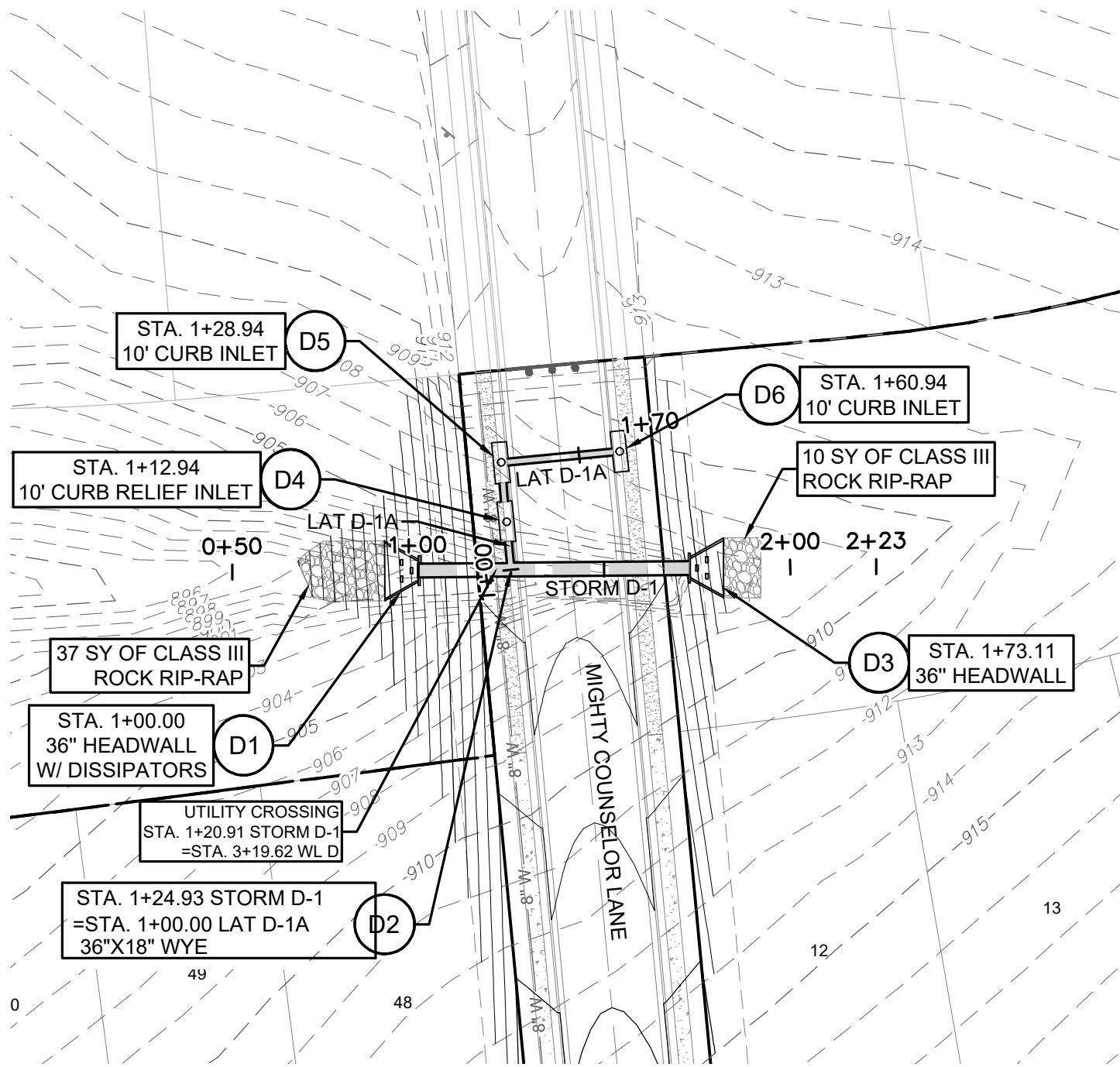
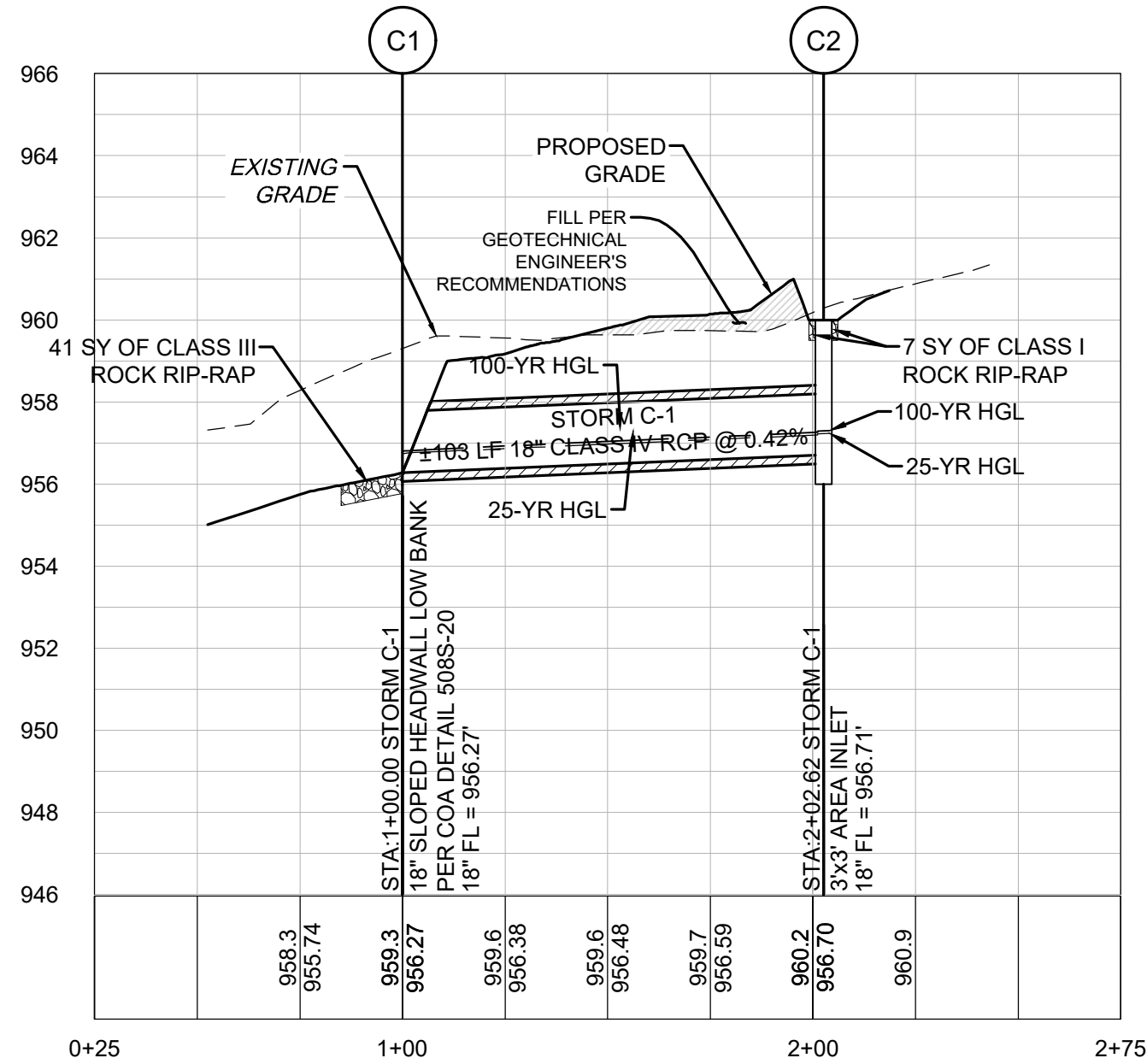
2023-XX-CON



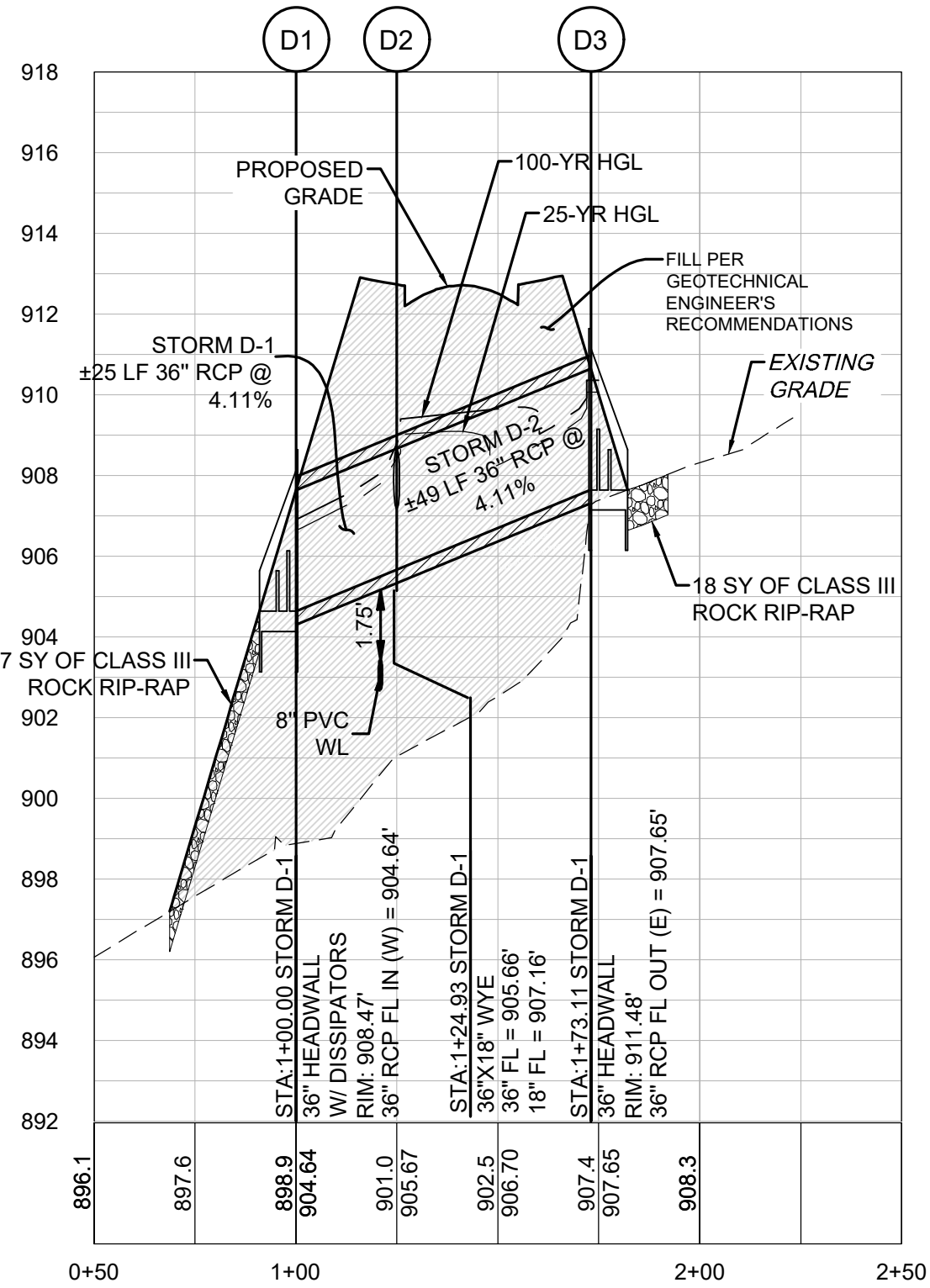
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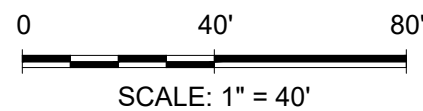
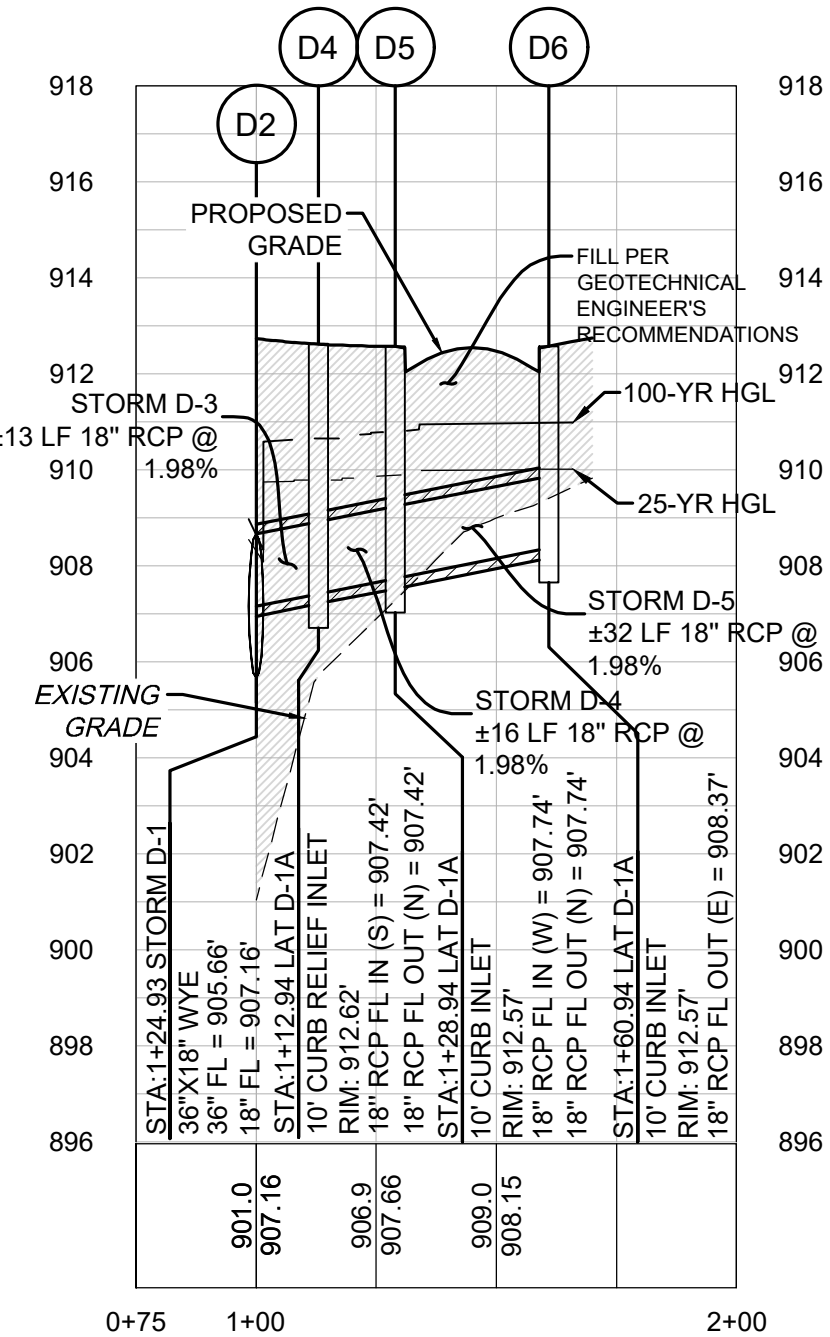
STORM C-1



STORM D-1



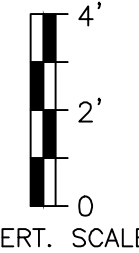
LAT D-1A



LEGEND

- 8.34 --- EXISTING MINOR CONTOUR
- 8.35 --- EXISTING MAJOR CONTOUR
- 8.34 --- PROPOSED MINOR CONTOUR
- 8.35 --- PROPOSED MAJOR CONTOUR
- --- BOUNDARY
- --- EASEMENT
- 100YR --- 100 YR PROPOSED CONDITION FLOODPLAIN
- 100YR --- 100 YR FEMA ZONE A FLOODPLAIN
- --- CREEK CENTERLINE
- --- PROPOSED STORM LINE
- --- FIRE HYDRANT
- --- WATER VALVE
- --- STORM SEWER MANHOLE
- --- WASTEWATER MANHOLE
- --- CURB INLET
- --- TREES TO REMAIN HERITAGE
- --- TREES TO REMAIN NON HERITAGE

VERT. SCALE: 1" = 4'  
HORZ. SCALE: 1" = 40'



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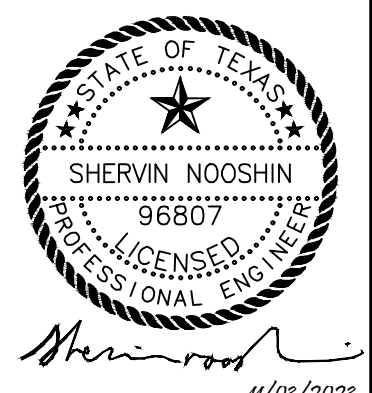
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| Pipe Label | Slope (%) | Q25 (cfs) | V25 (ft/s) | D25 (ft) | Q100 (cfs) | V100 (ft/s) | D100 (ft) |
|------------|-----------|-----------|------------|----------|------------|-------------|-----------|
| STORM C-1  | 0.42%     | 1.56      | 3.23       | 0.49     | 1.96       | 3.44        | 0.55      |
| STORM D-1  | 3.40%     | 55.37     | 10.80      | 2.41     | 70.44      | 12.07       | 2.66      |
| STORM D-2  | 3.40%     | 51.10     | 7.96       | 3.36     | 65.21      | 9.65        | 3.74      |
| STORM D-3  | 1.98%     | 4.27      | 2.42       | 2.58     | 5.23       | 2.96        | 3.43      |
| STORM D-4  | 1.98%     | 4.27      | 2.42       | 2.40     | 5.23       | 2.96        | 3.29      |
| STORM D-5  | 1.98%     | 2.54      | 1.44       | 2.25     | 3.11       | 1.76        | 3.21      |



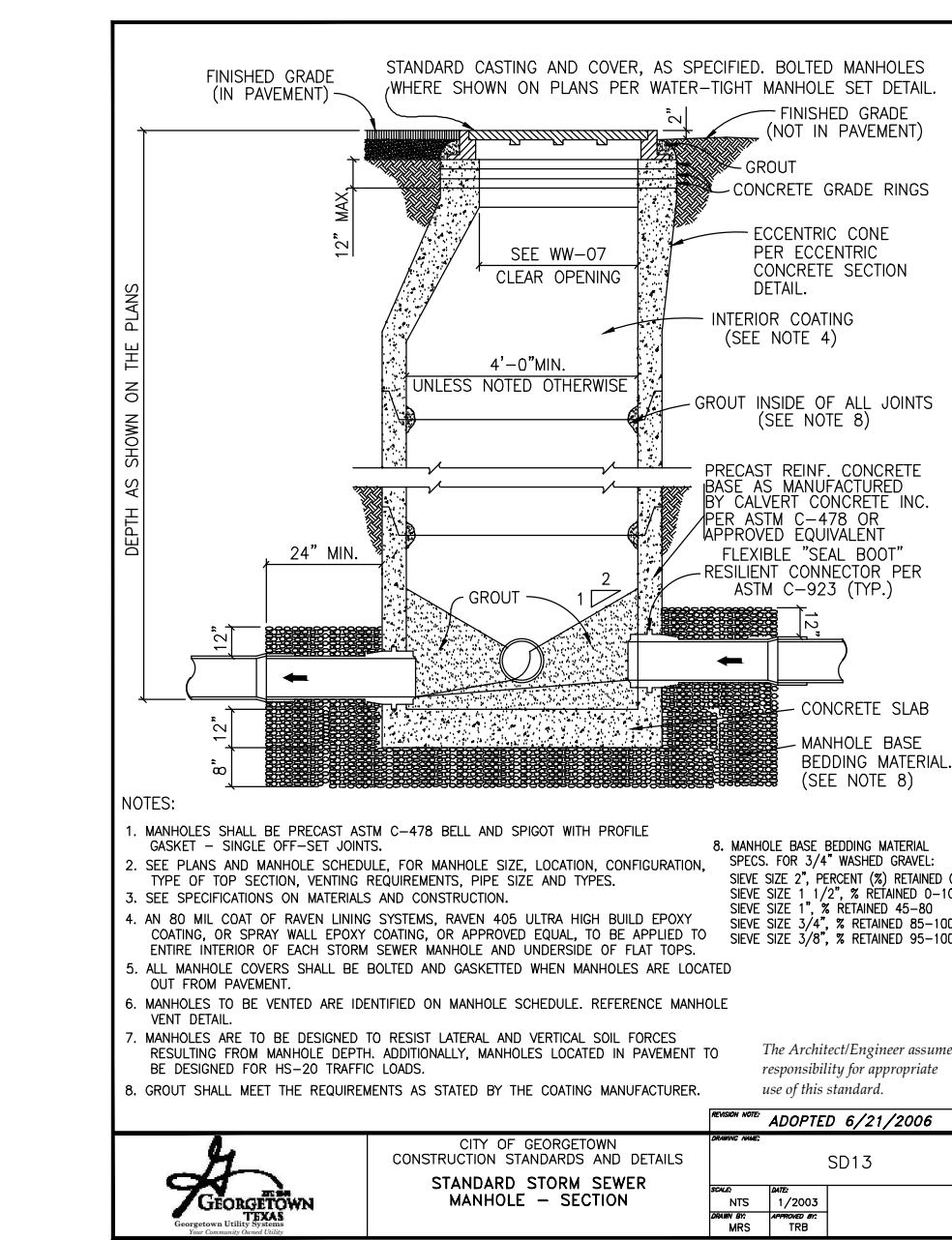
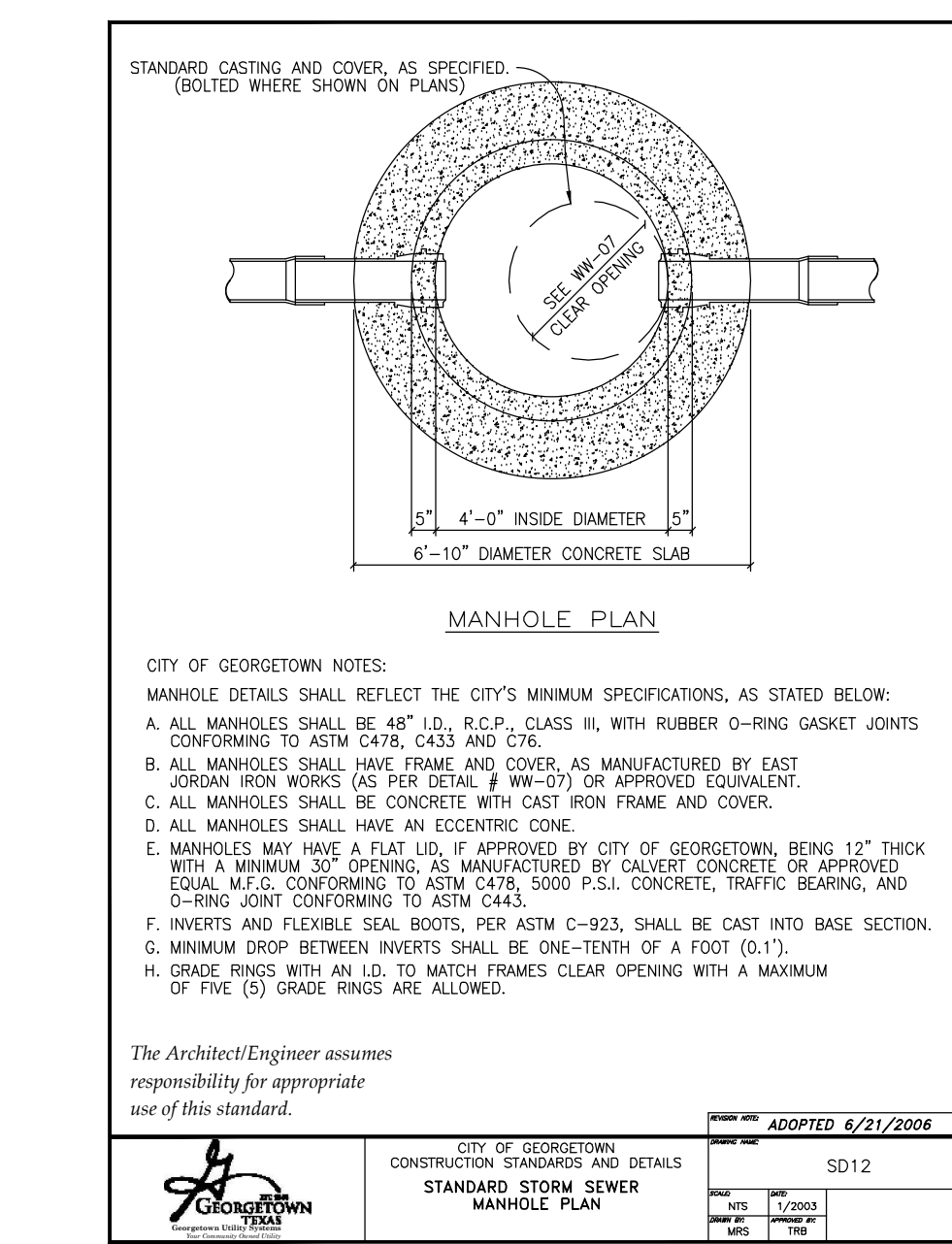
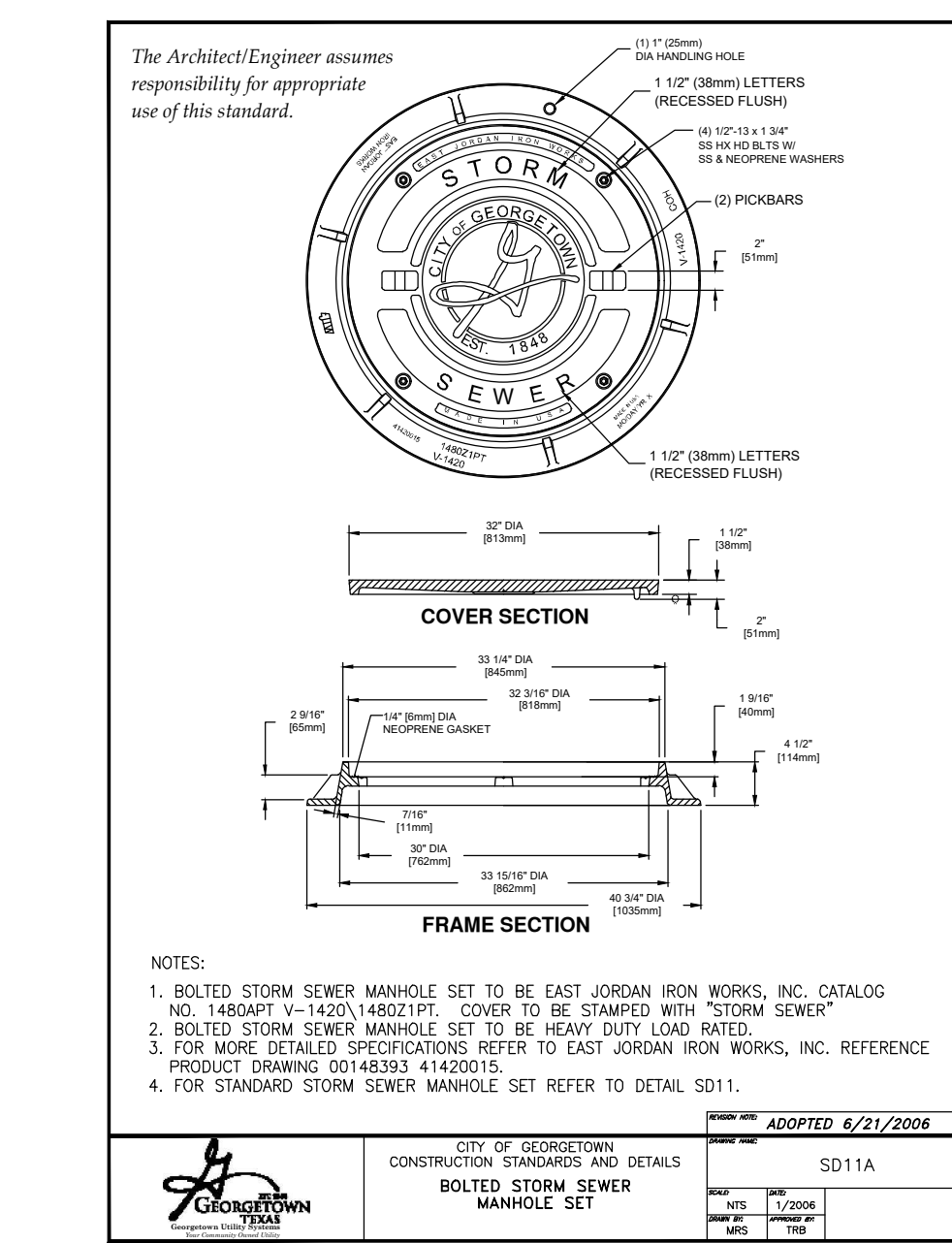
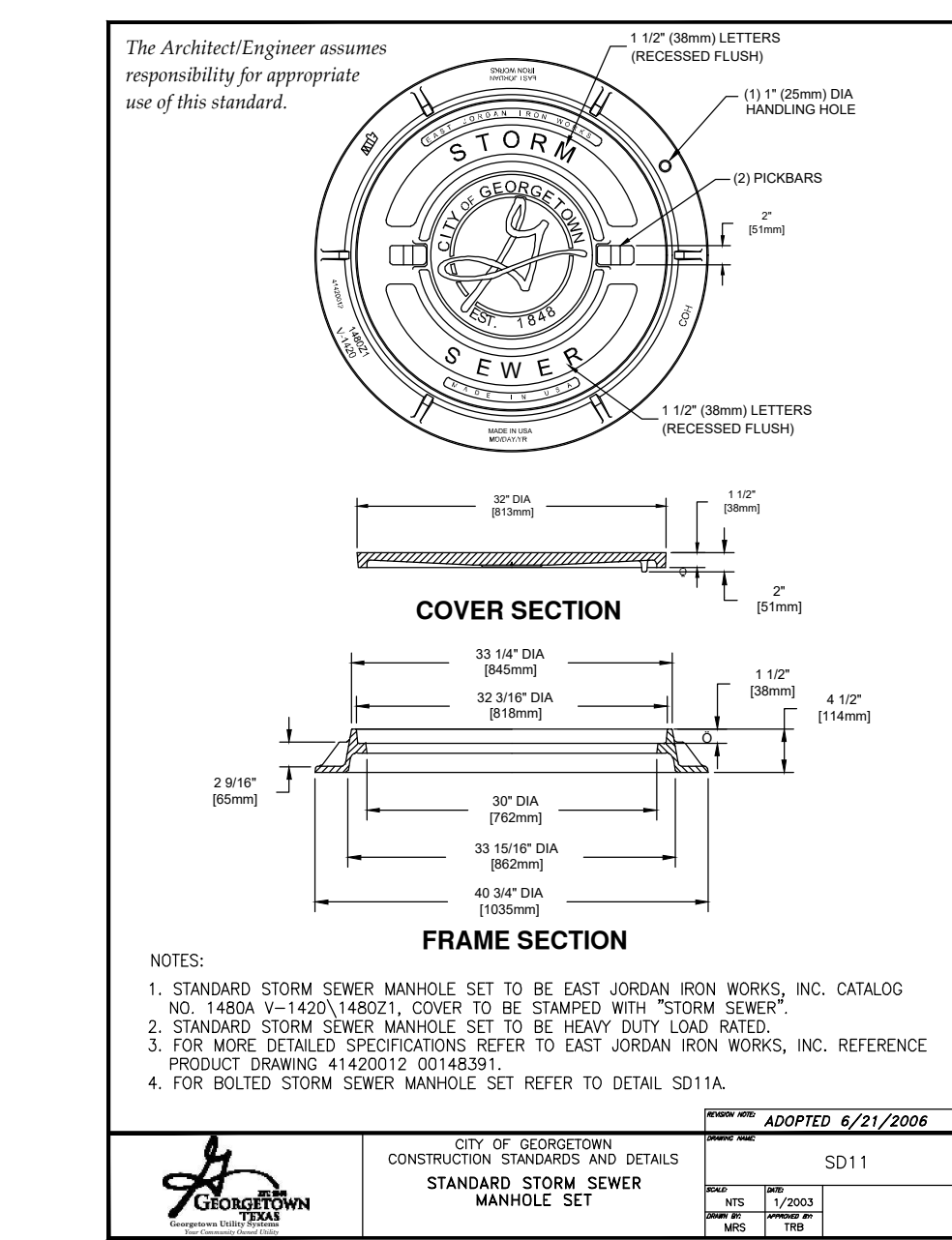
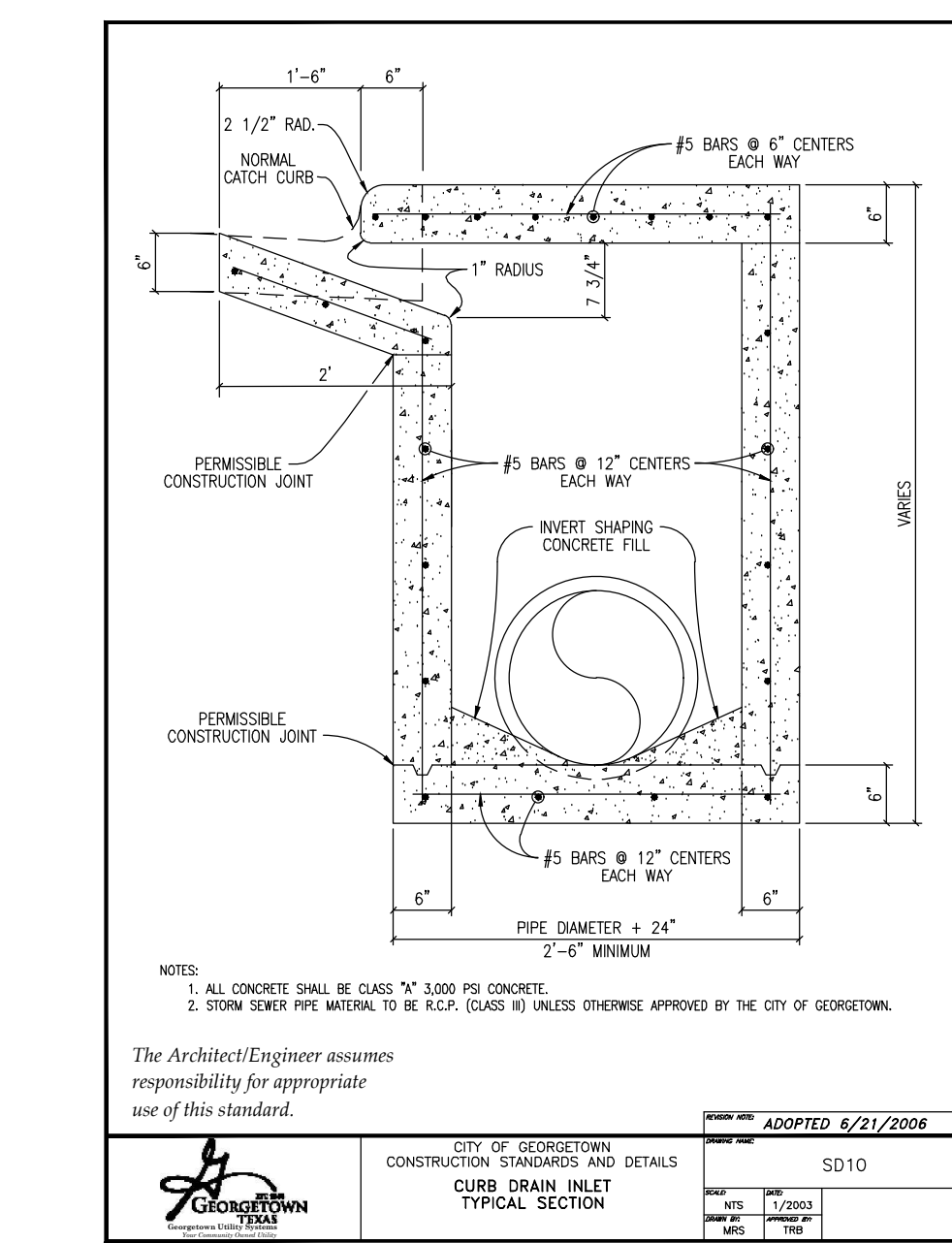
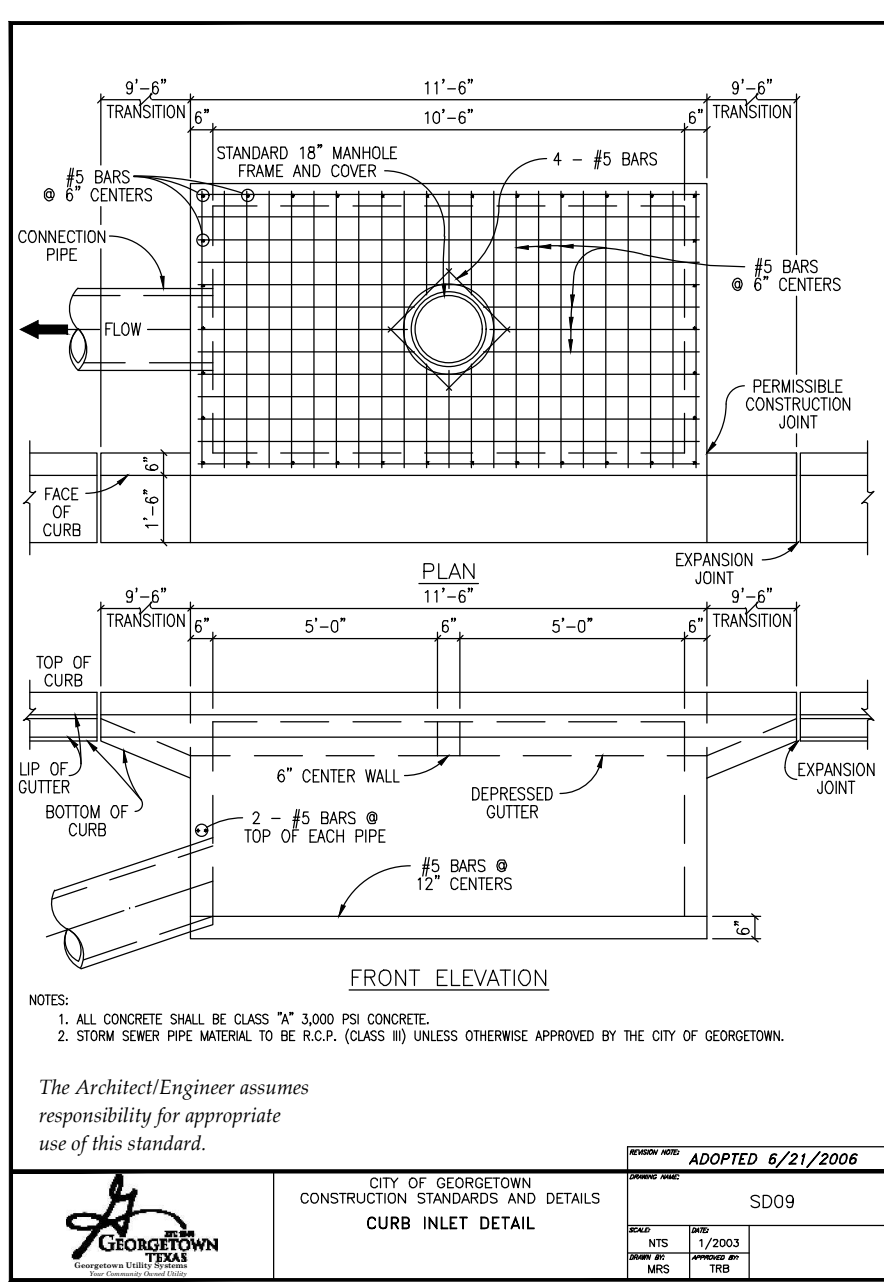
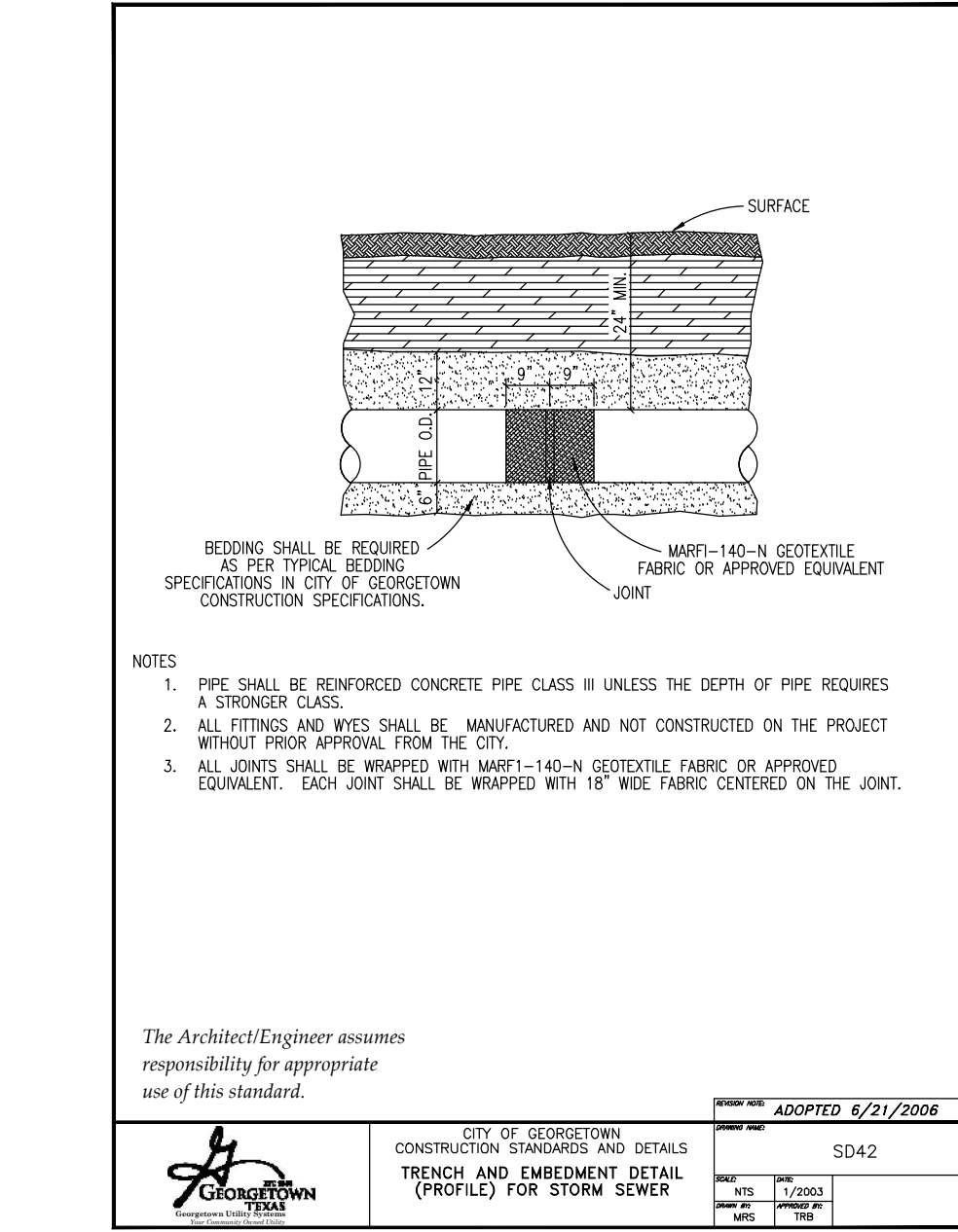
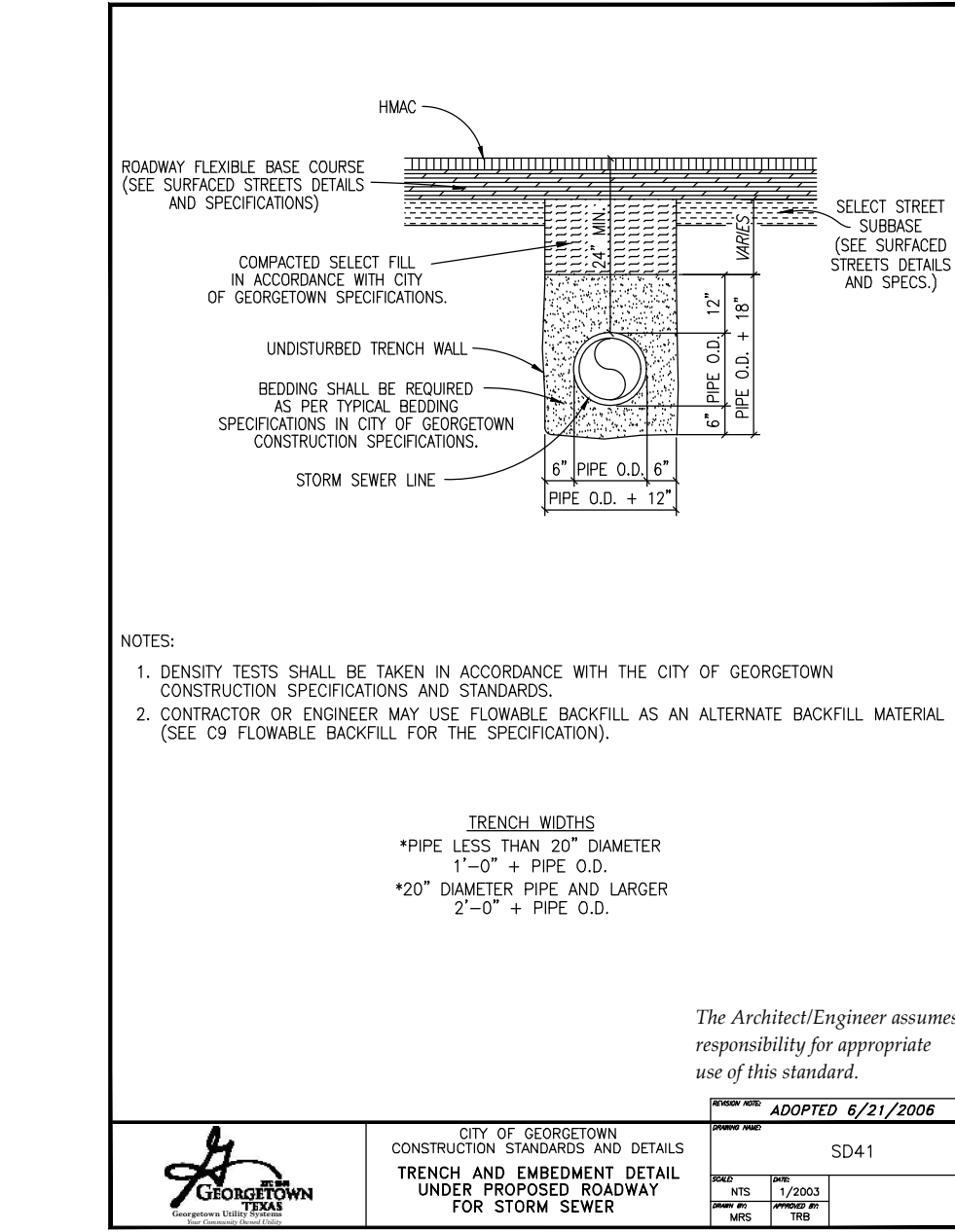
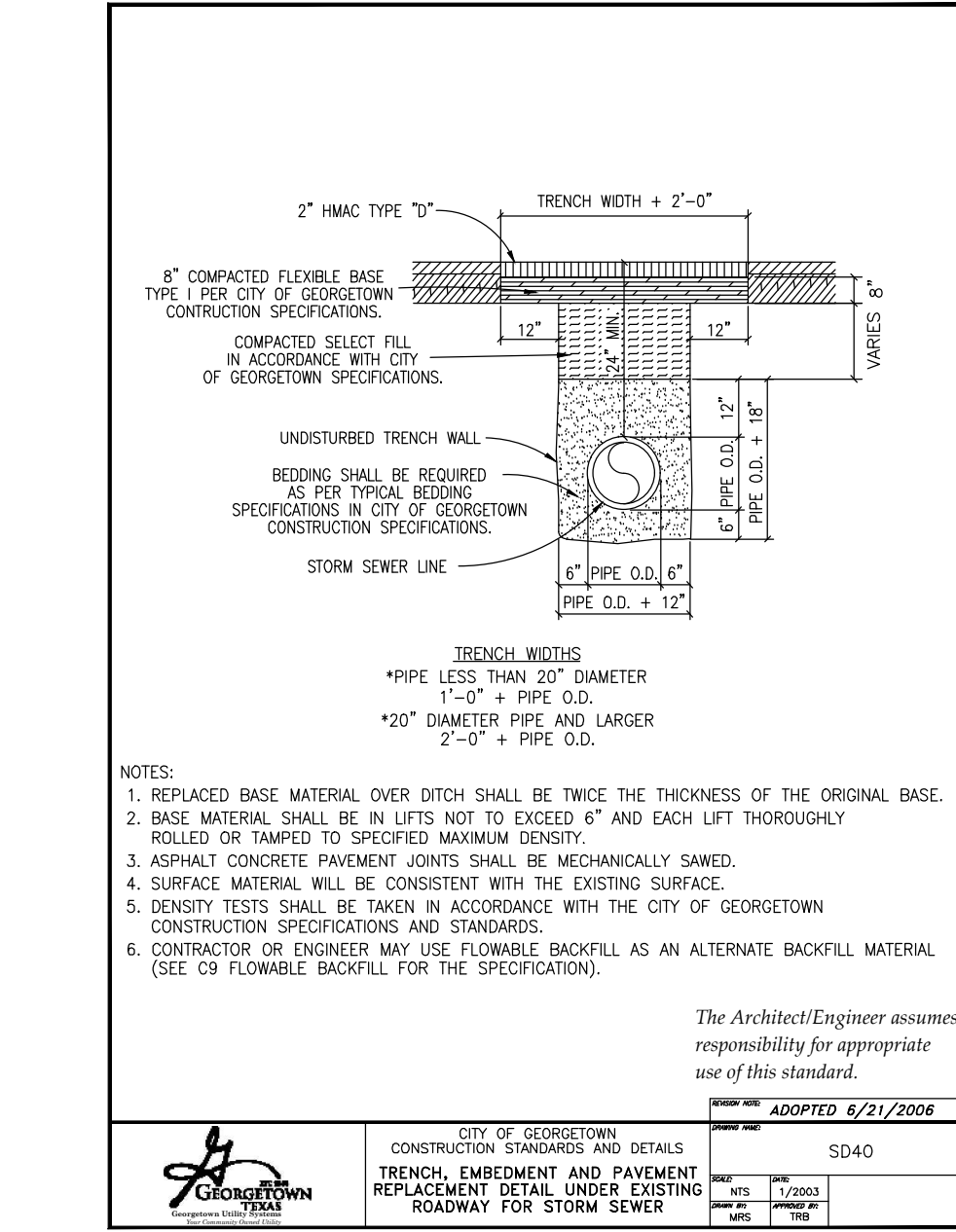
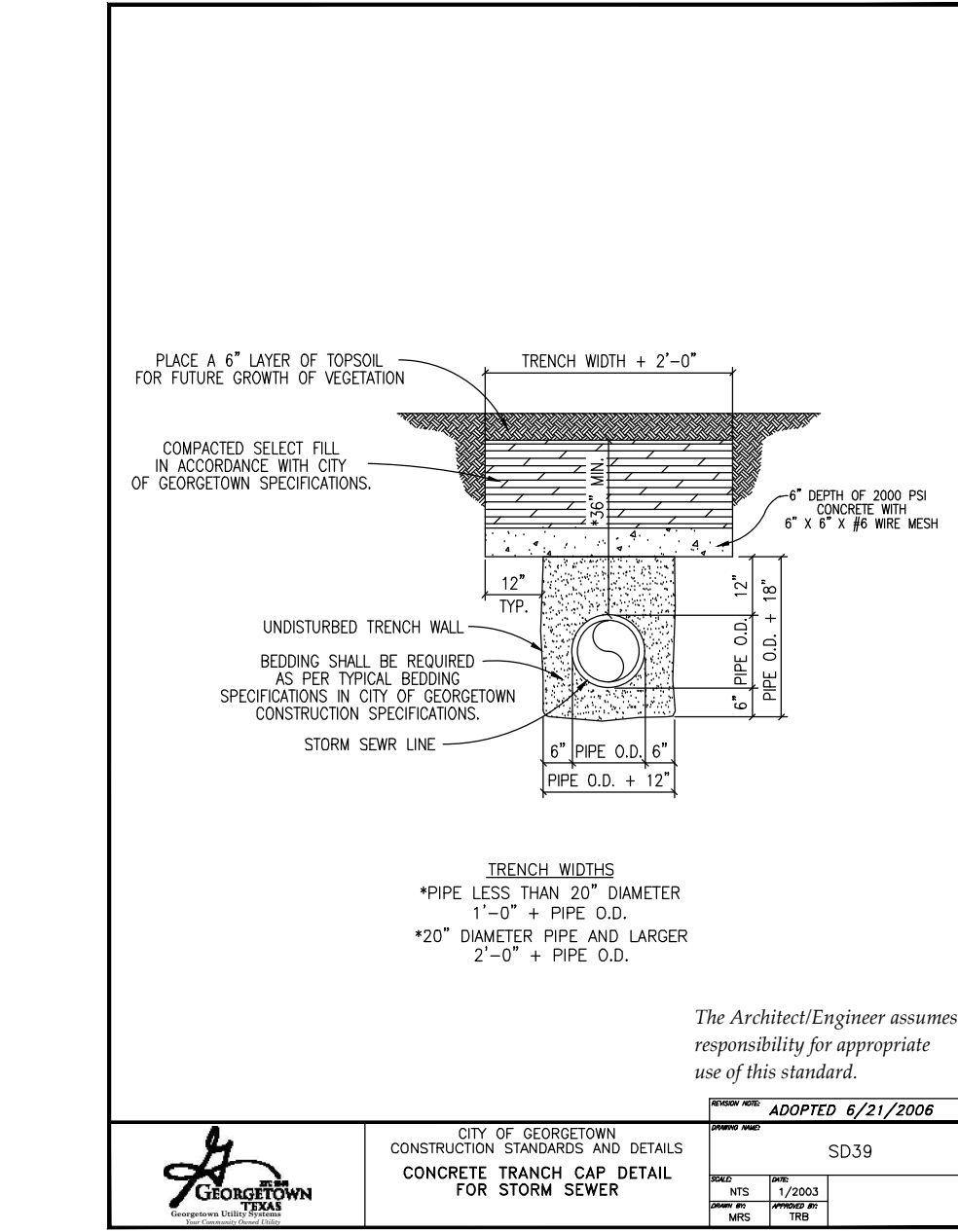
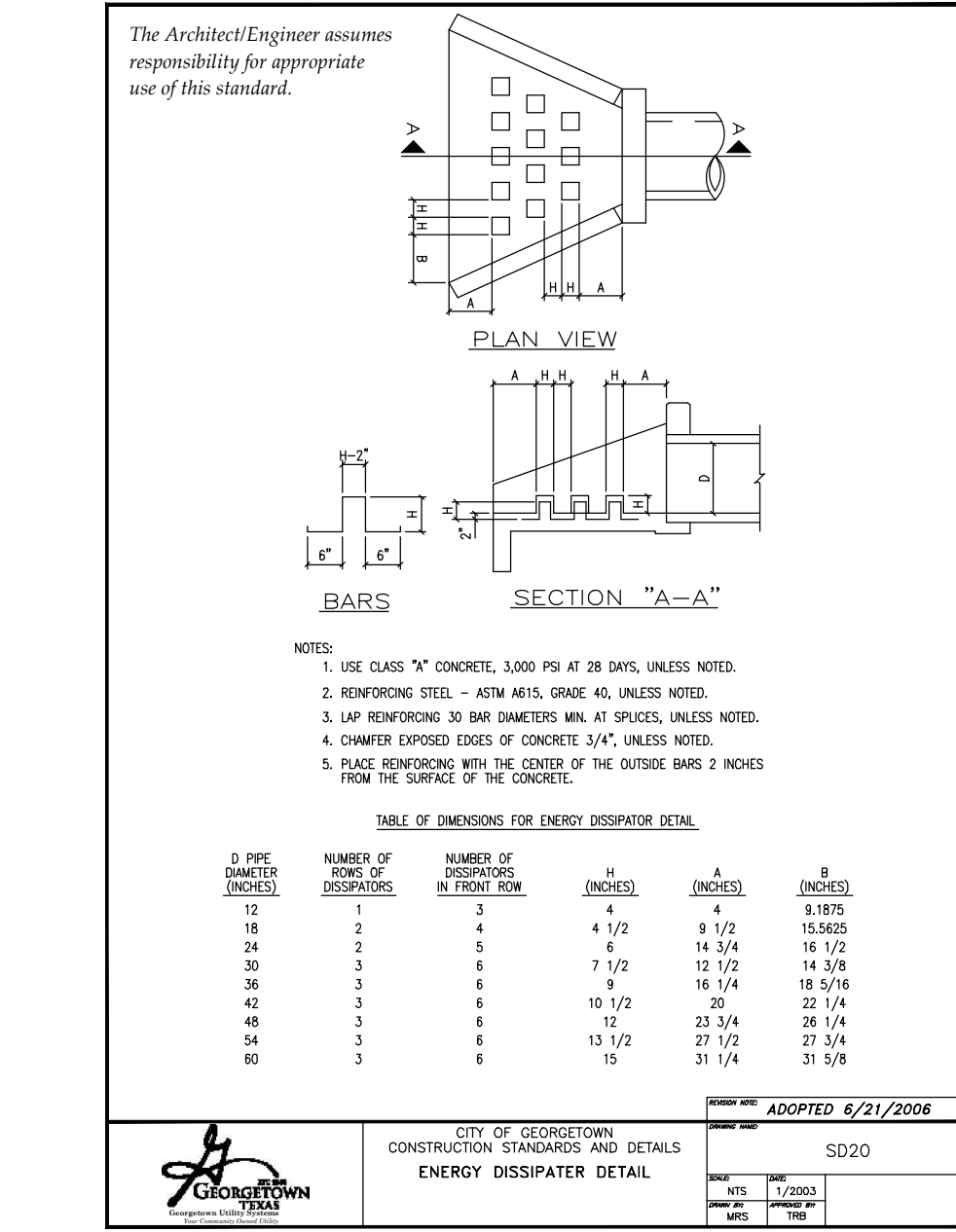
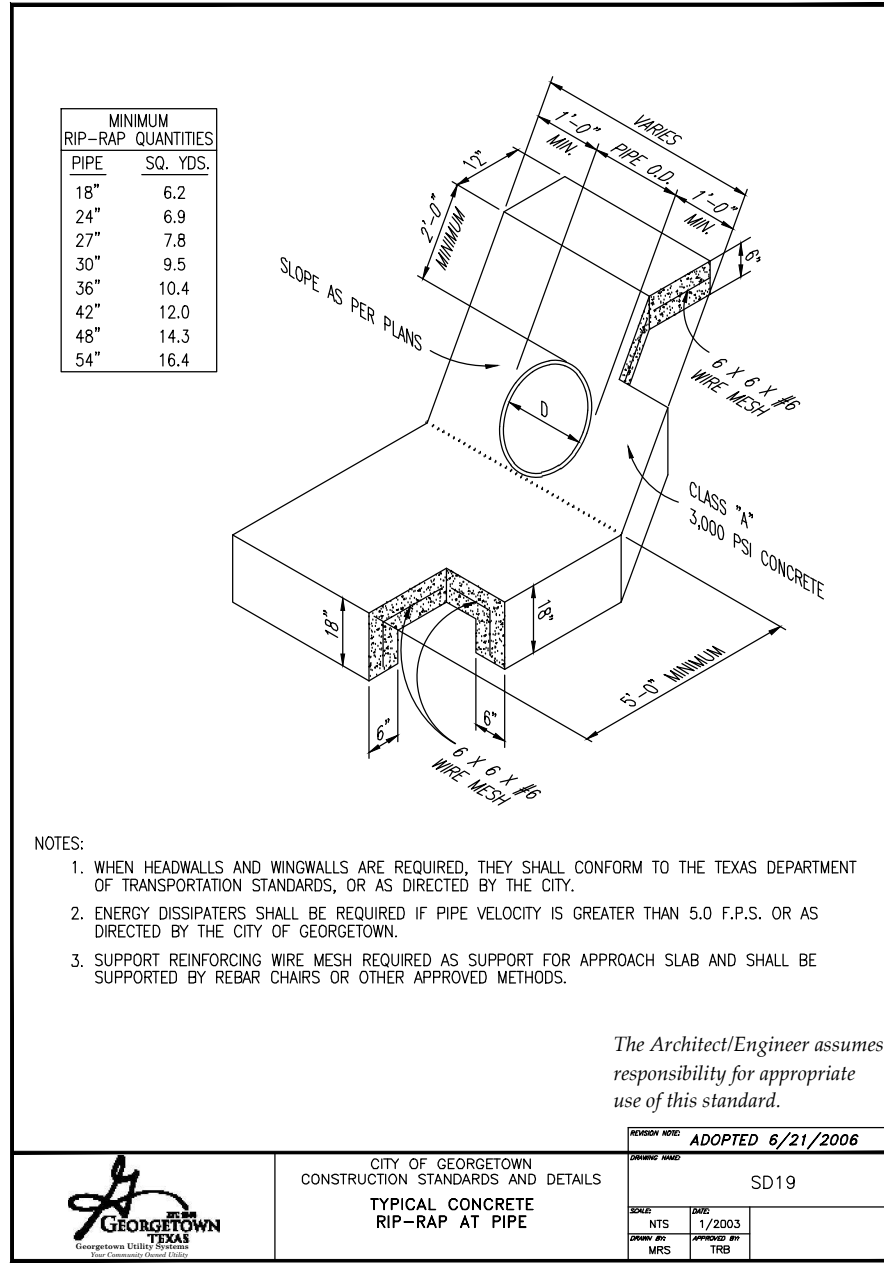
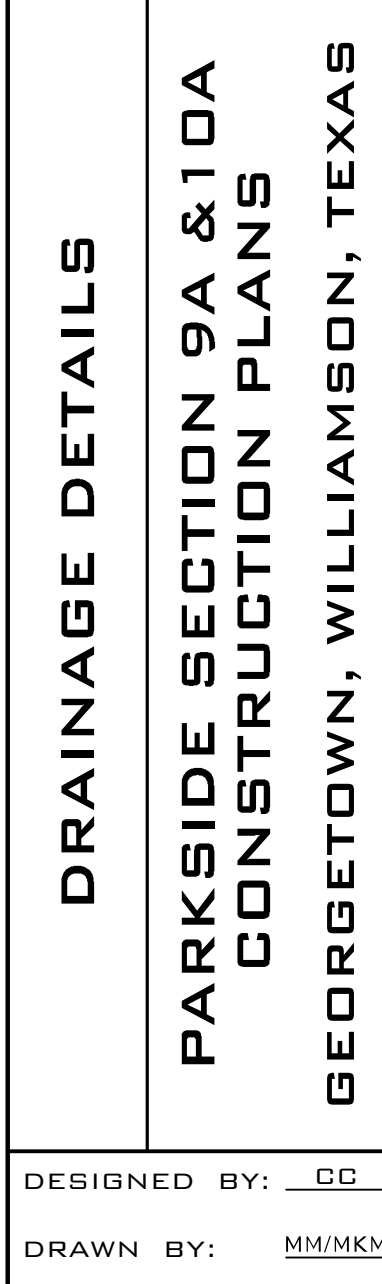
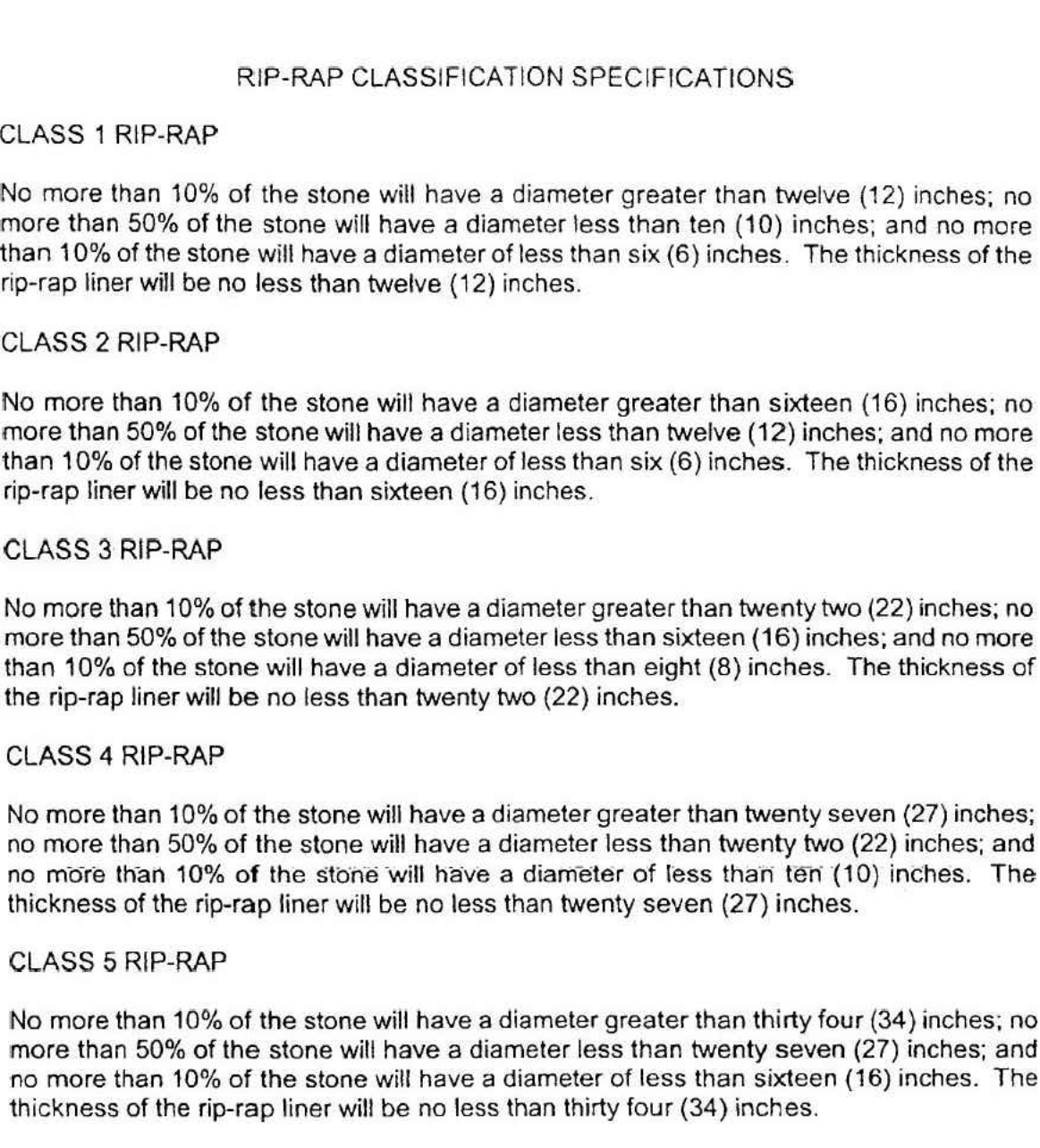
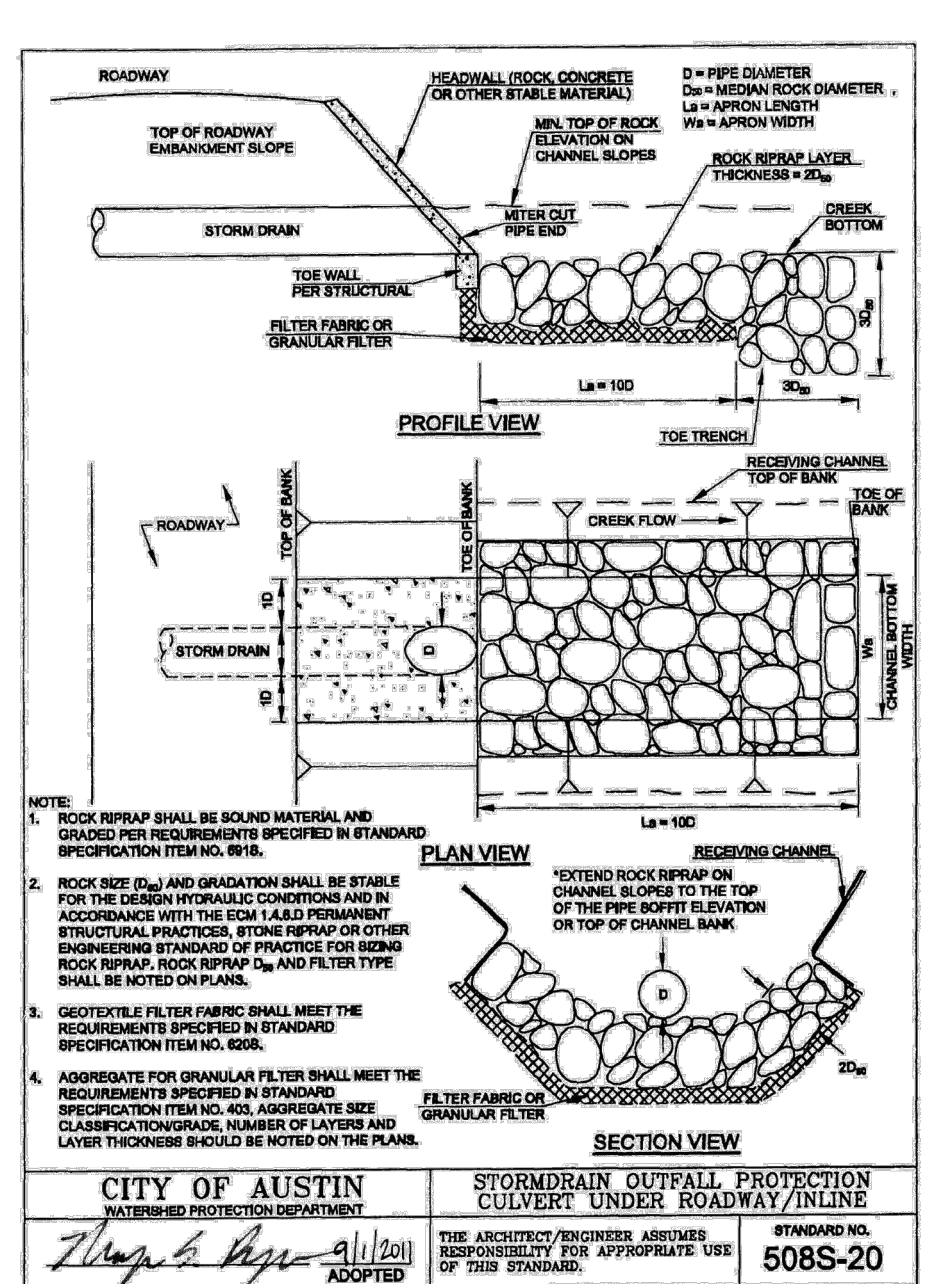
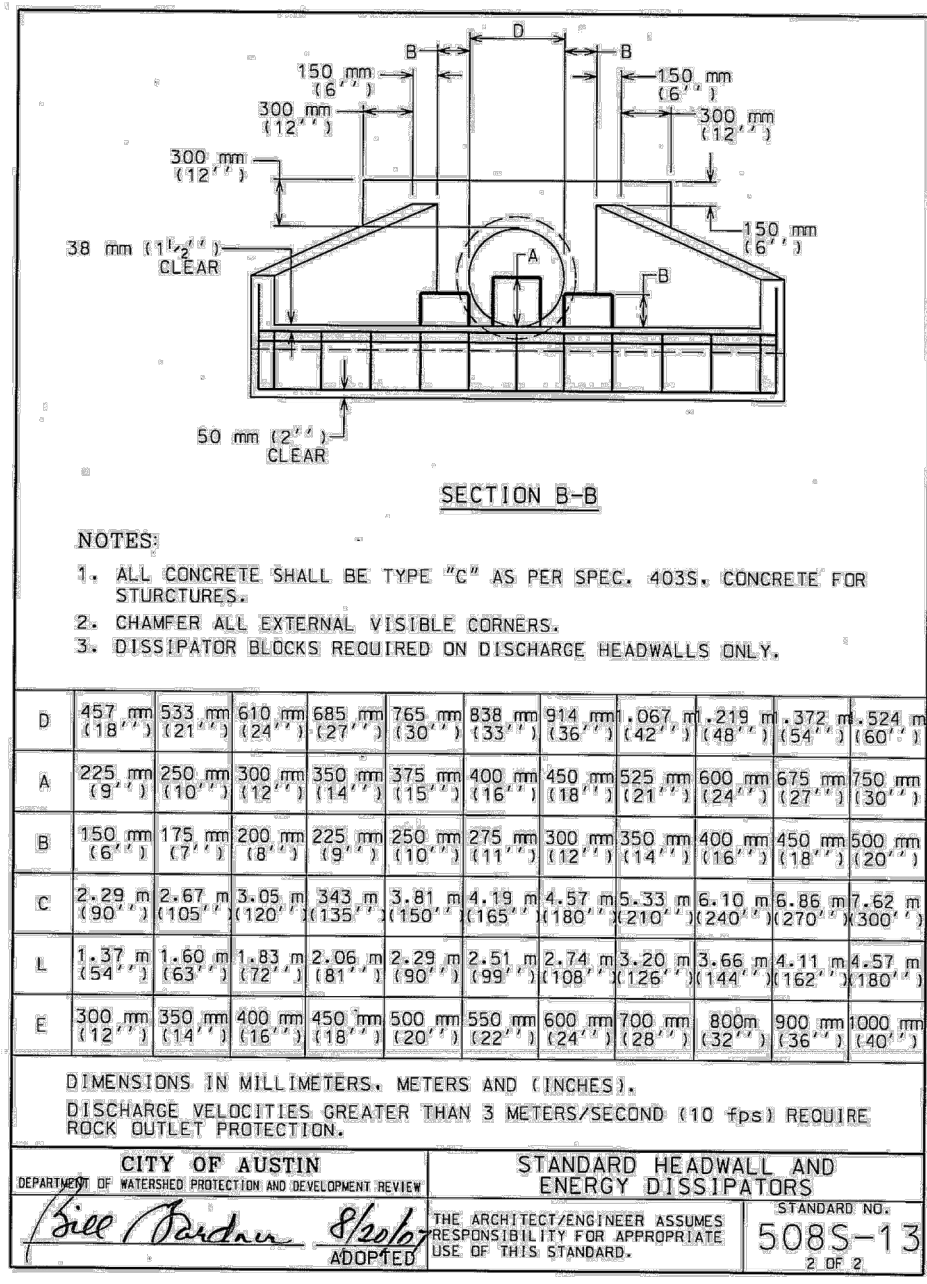
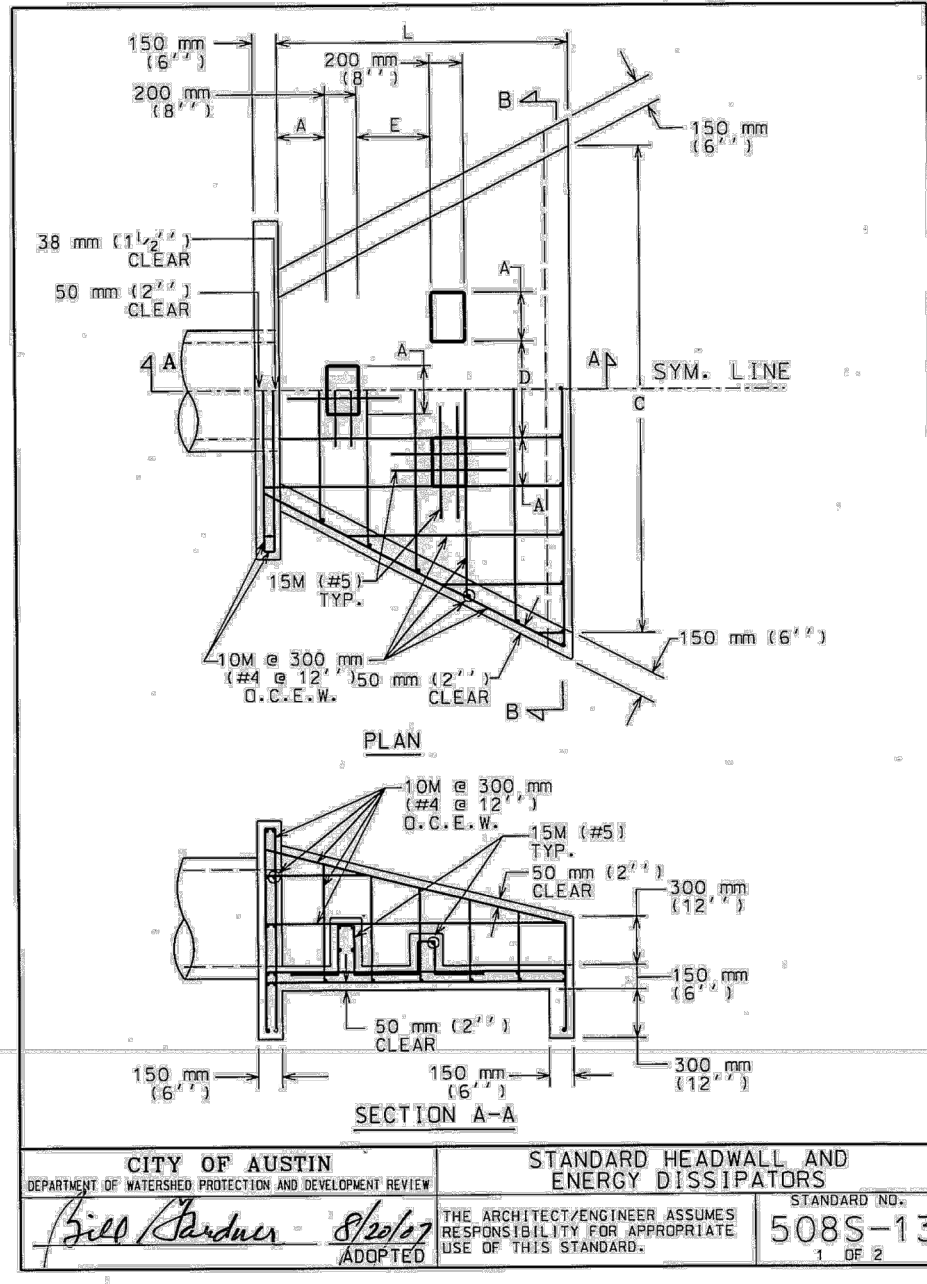
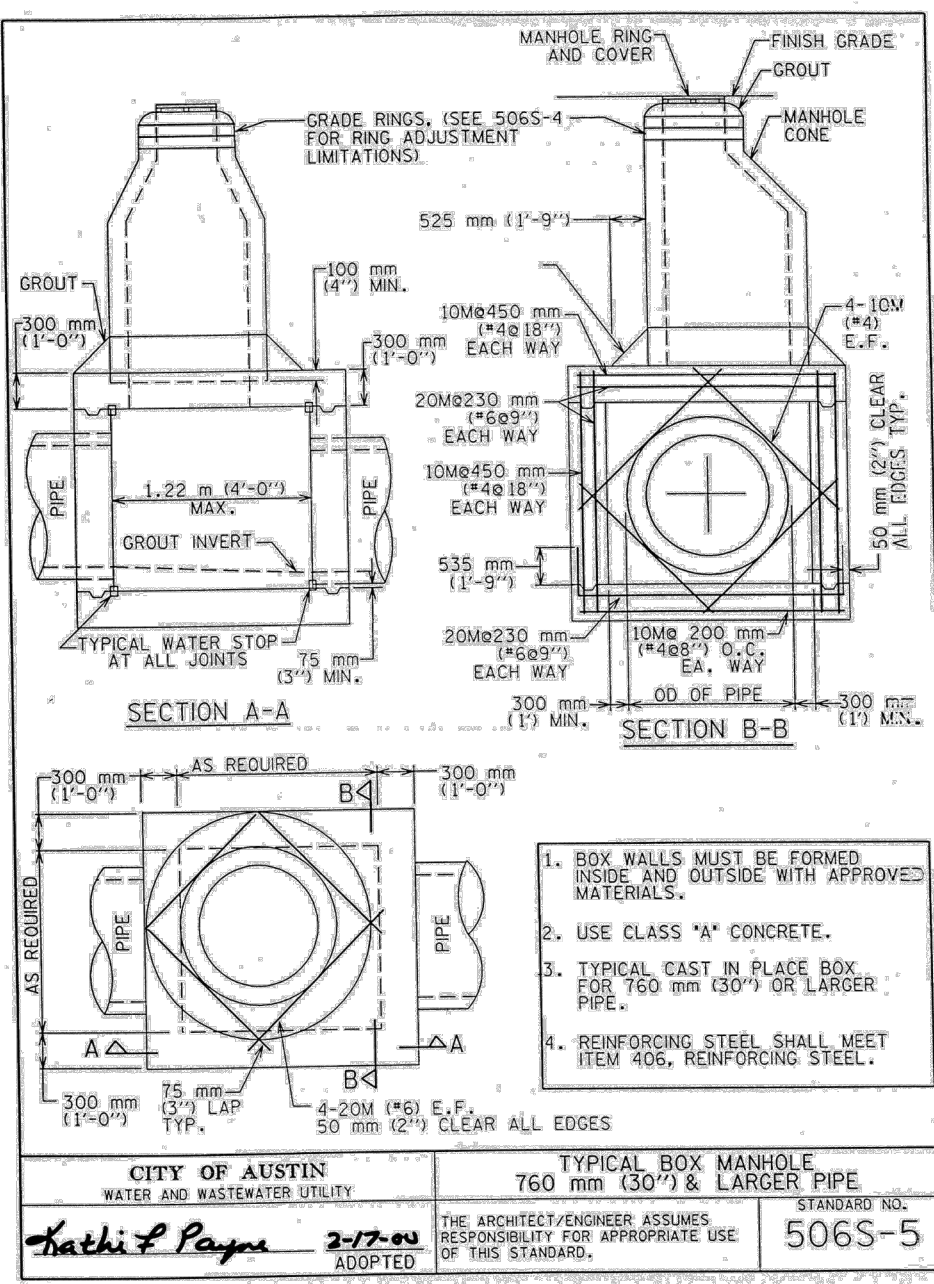
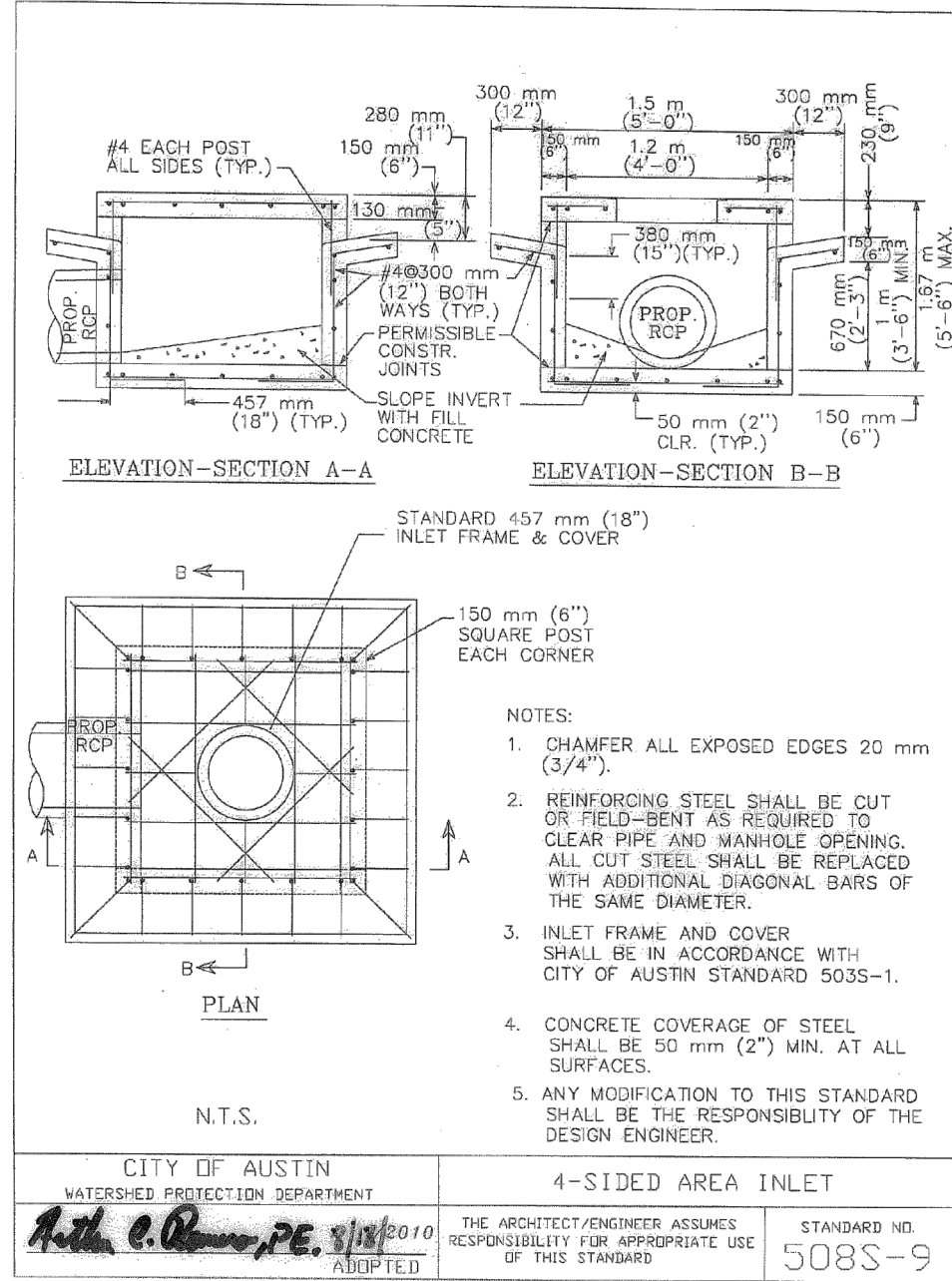
5508 HIGHWAY 290 WEST  
SUITE 100  
AUSTIN, TX 78735  
512.872.6996  
HGREEN.COM



STORM C-1 & D-1 PLAN  
& PROFILE  
PARKSIDE SECTION 8  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS

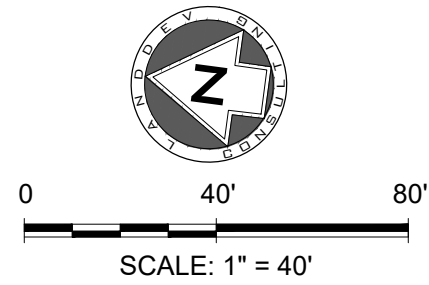
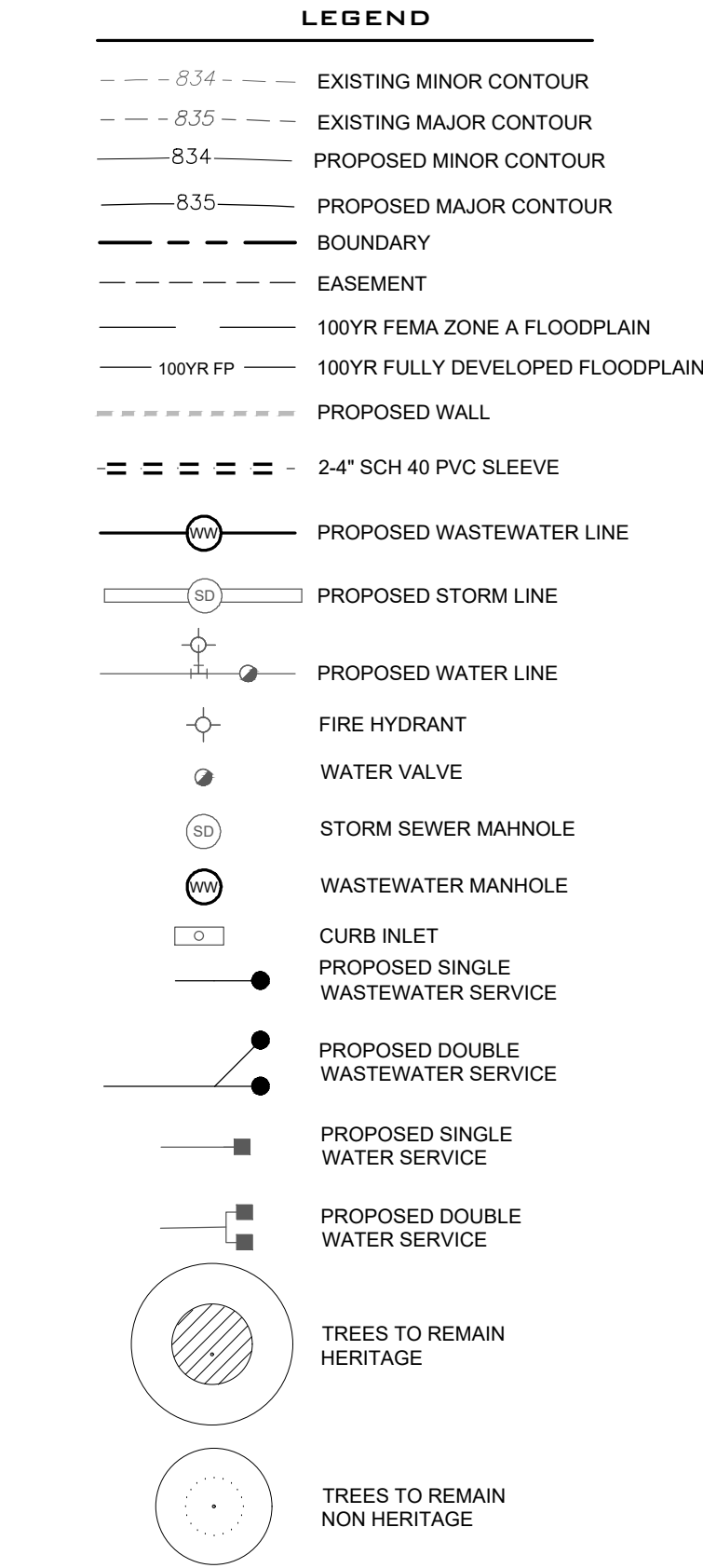
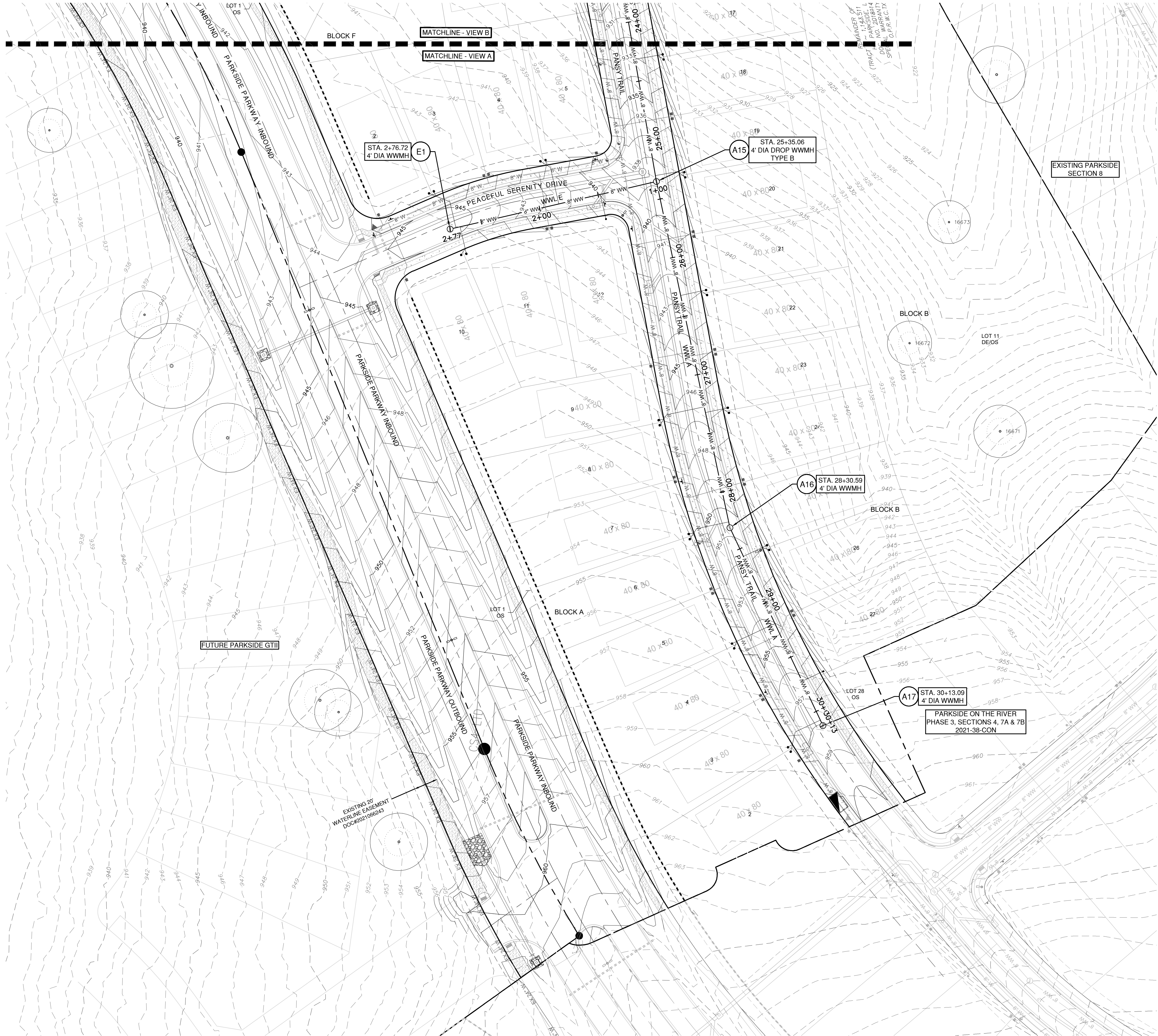
DESIGNED BY: CC  
DRAWN BY: TG/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
SHEET 60 OF 72  
2023-XX-CON





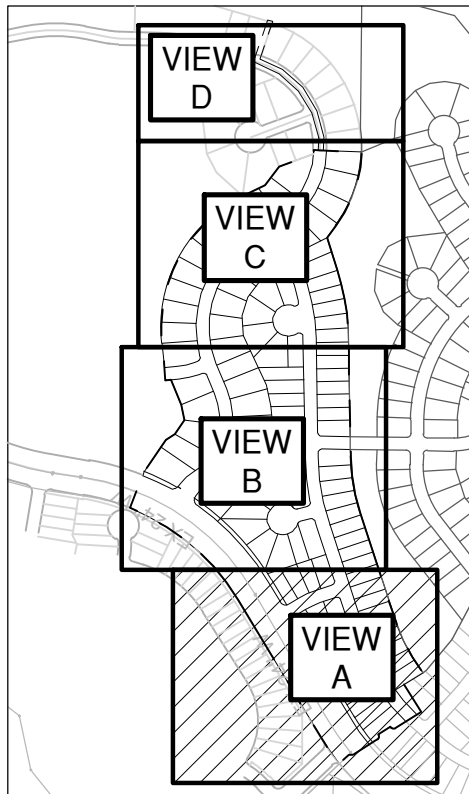


P:\bldg\magnaparkside sections 8, 9a & 10a\03\_ACO\Plan\magnaparkside sections 8, 9a & 10a.dwg, OVERALL WASTEWATER PLAN A, December 19, 2023, 10:30 AM, ccmptell



- NOTES:**
1. REFER TO THE WATER AND WASTEWATER DETAIL SHEET(S) FOR TYPE S AND TYPE D SERVICE CONNECTIONS.
  2. UTILITIES ON THIS SITE TO BE BUILT PER THE APPROVED UTILITY ASSIGNMENT SHOWN ON THE WATER AND WASTEWATER DETAIL SHEET.
  3. NO PORTION OF THIS SITE LIES WITHIN THE FEMA 100-YR FLOODPLAIN.
  4. ALL PROPOSED GRAVITY WASTEWATER PIPES TO BE SDR-26 UNLESS OTHERWISE SPECIFIED ON THE PLANS.
  5. UNLESS OTHERWISE NOTED, CONTRACTOR TO ENSURE WW SERVICES ARE SET TO MAXIMUM DEPTH (PER DETAIL WW13) FOR ALL LOTS IN ORDER TO PROVIDE SUFFICIENT DEPTH TO CONNECT SERVICE.
  6. ALL MANHOLES TO BE COATED PER CITY OF GEORGETOWN SPECIFICATIONS. EXISTING MANHOLES WITH PROPOSED CONNECTIONS TO BE COATED AS NEEDED.
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**OVERALL WASTEWATER  
PLAN A**

**PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS**

**GEORGETOWN, WILLIAMSON, TEXAS**

DESIGNED BY: CC

DRAWN BY: MM/MKM

CHECKED BY: SN

APPROVED BY: \_\_\_\_\_

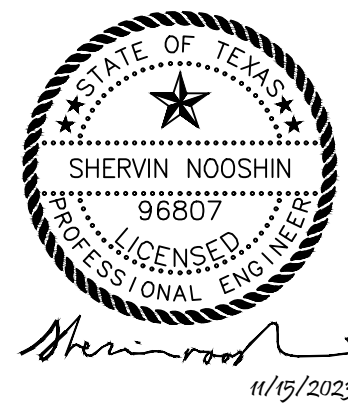
SHEET **62** OF **84**

**2023-XX-CON**



5508 HIGHWAY 290 WEST  
SUITE 150  
MUSTIN, TX 78735  
CITY OF GEORGETOWN  
HARGREEN, CON

TYPE NO: 10384  
TPLS NO: 10194101



11/19/2023

REVISION

NO

BY DATE

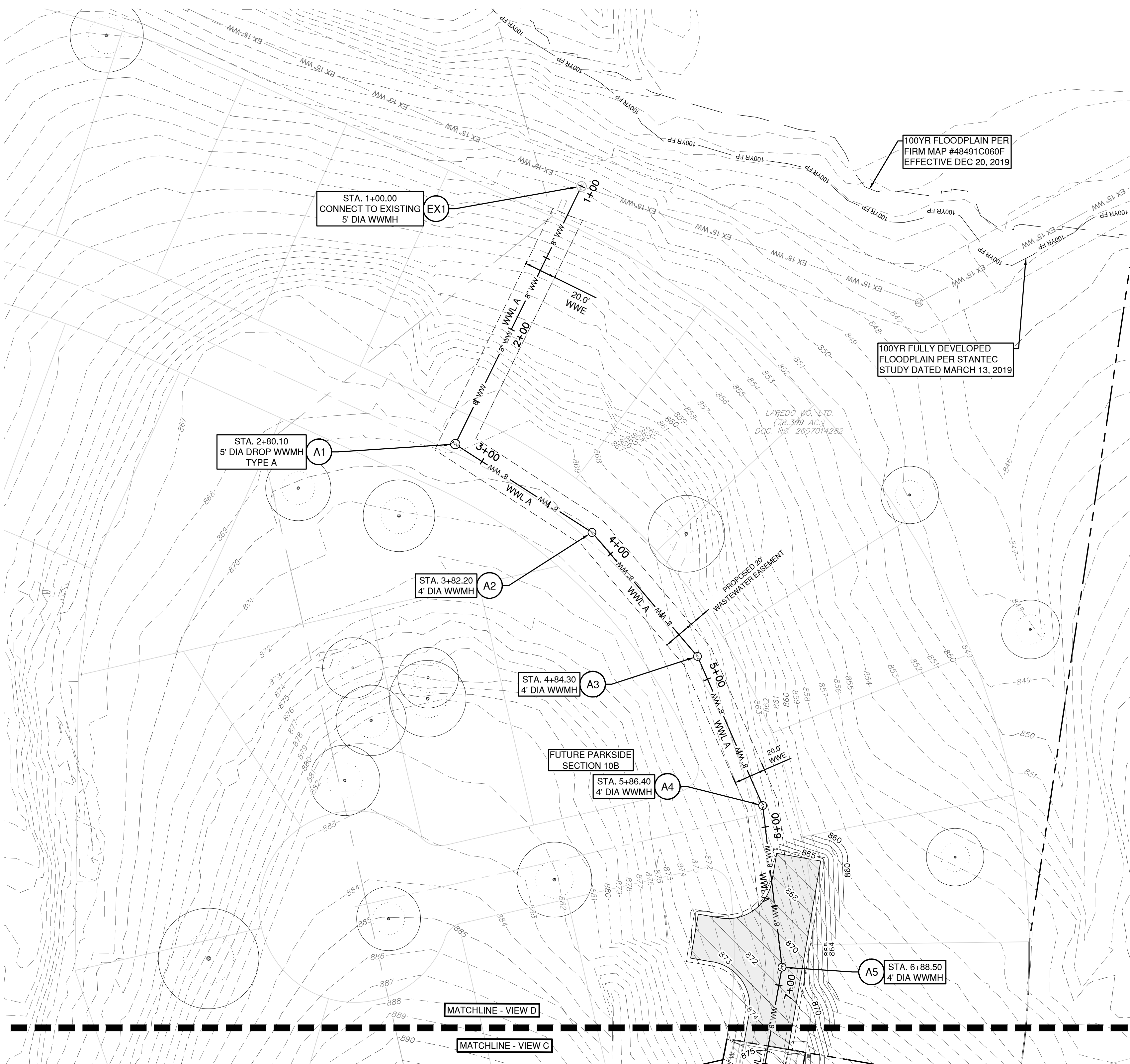






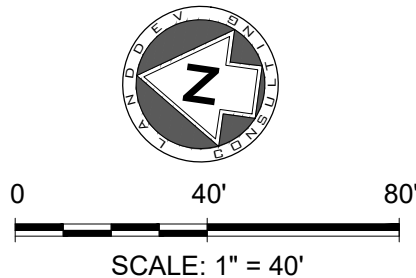






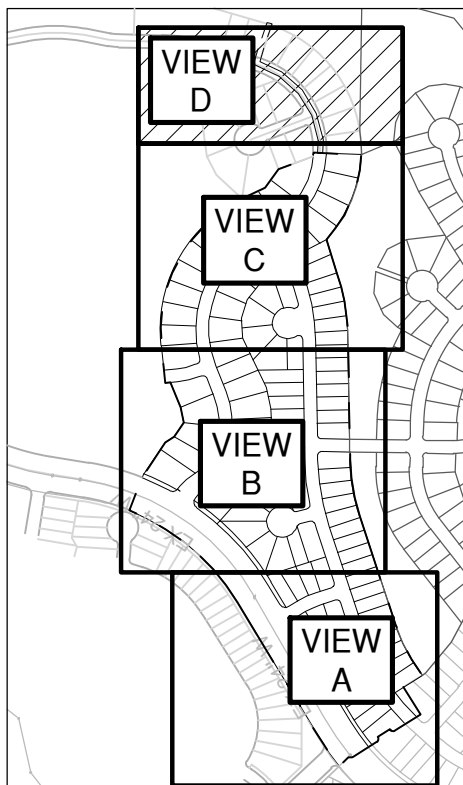
**LEGEND**

- 834 --- EXISTING MINOR CONTOUR
- 835 --- EXISTING MAJOR CONTOUR
- 834 --- PROPOSED MINOR CONTOUR
- 835 --- PROPOSED MAJOR CONTOUR
- --- BOUNDARY
- --- EASEMENT
- 100YR FEMA ZONE A FLOODPLAIN
- 100YR FP 100YR FULLY DEVELOPED FLOODPLAIN
- --- PROPOSED WALL
- --- 2-4" SCH 40 PVC SLEEVE
- --- PROPOSED WASTEWATER LINE
- --- PROPOSED STORM LINE
- --- PROPOSED WATER LINE
- --- FIRE HYDRANT
- --- WATER VALVE
- --- STORM SEWER MAHNOLE
- --- WASTEWATER MAHNOLE
- --- CURB INLET
- --- PROPOSED SINGLE WASTEWATER SERVICE
- --- PROPOSED DOUBLE WASTEWATER SERVICE
- --- PROPOSED SINGLE WATER SERVICE
- --- PROPOSED DOUBLE WATER SERVICE
- --- TREES TO REMAIN HERITAGE
- --- TREES TO REMAIN NON HERITAGE



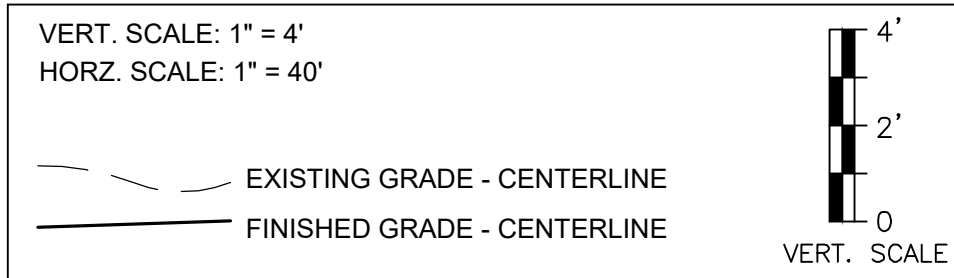
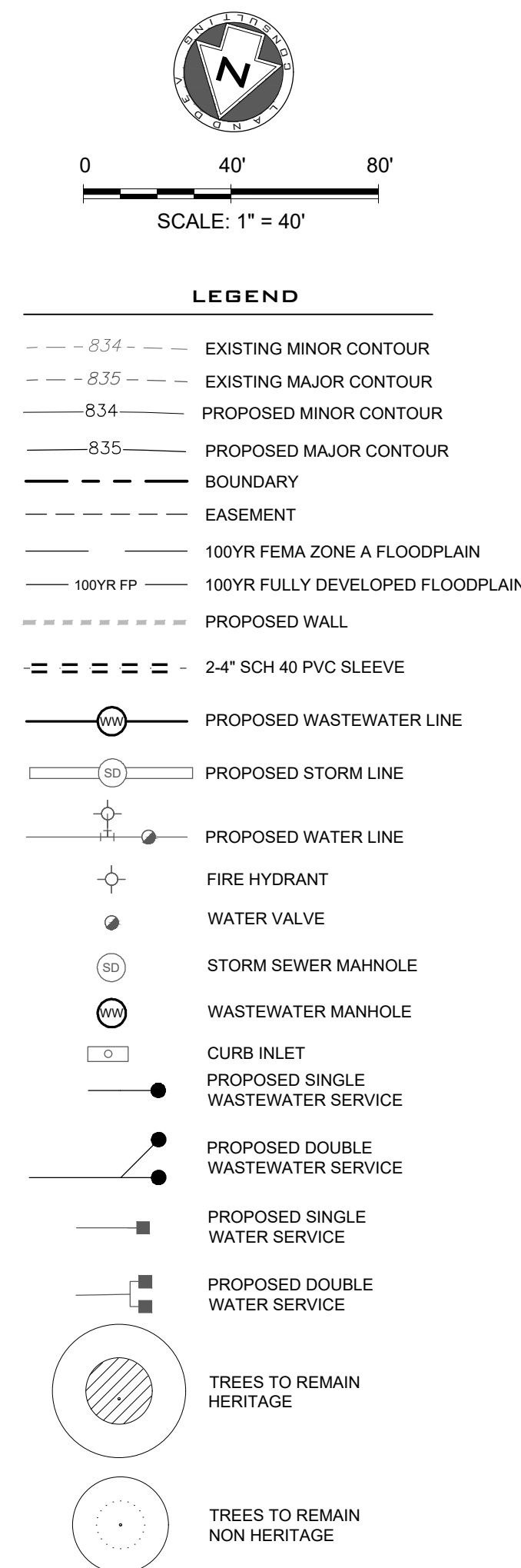
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|                                                                                        |  |      |
|----------------------------------------------------------------------------------------|--|------|
| BY                                                                                     |  | DATE |
| REVISION                                                                               |  |      |
| NO.                                                                                    |  |      |
|                                                                                        |  |      |
| 5508 HIGHWAY 290 WEST<br>SUITE 150<br>MUSTIN, TX 78735<br>817.460.1000<br>HARGREEN.COM |  |      |
| TYPE NO.: 10384<br>DPLS NO.: 10194101                                                  |  |      |
| <b>HARGREEN</b><br>DEVELOPMENT TX                                                      |  |      |
|                                                                                        |  |      |
| <b>OVERALL WASTEWATER<br/>PLAN D</b>                                                   |  |      |
| <b>PARKSIDE SECTION 9A &amp; 10A<br/>CONSTRUCTION PLANS</b>                            |  |      |
| <b>GEORGETOWN, WILLIAMSON, TEXAS</b>                                                   |  |      |
| DESIGNED BY: CC                                                                        |  |      |
| DRAWN BY: MM/MKM                                                                       |  |      |
| CHECKED BY: SN                                                                         |  |      |
| APPROVED BY:                                                                           |  |      |
| SHEET <b>65</b> OF <b>84</b>                                                           |  |      |
| 2023-XX-CON                                                                            |  |      |



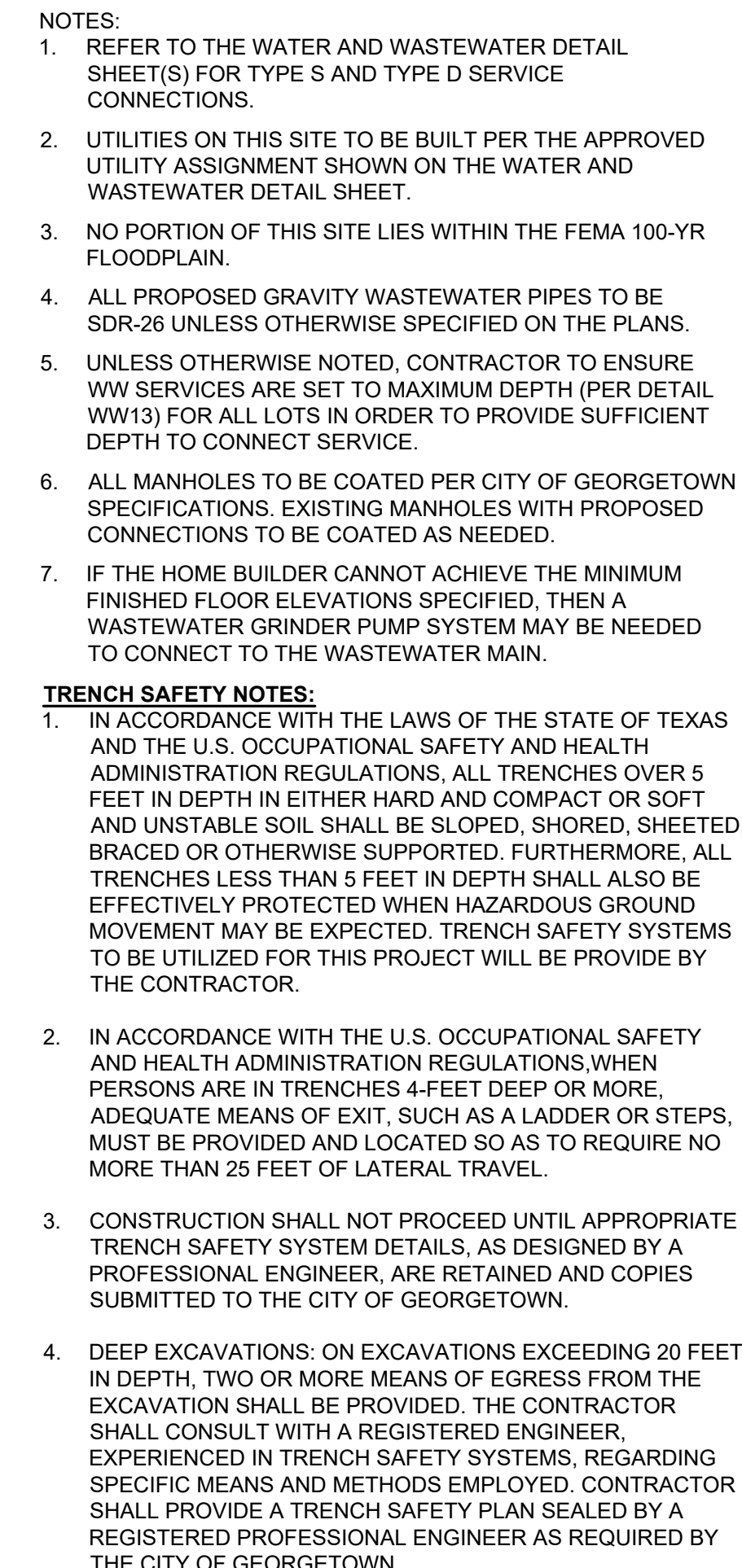
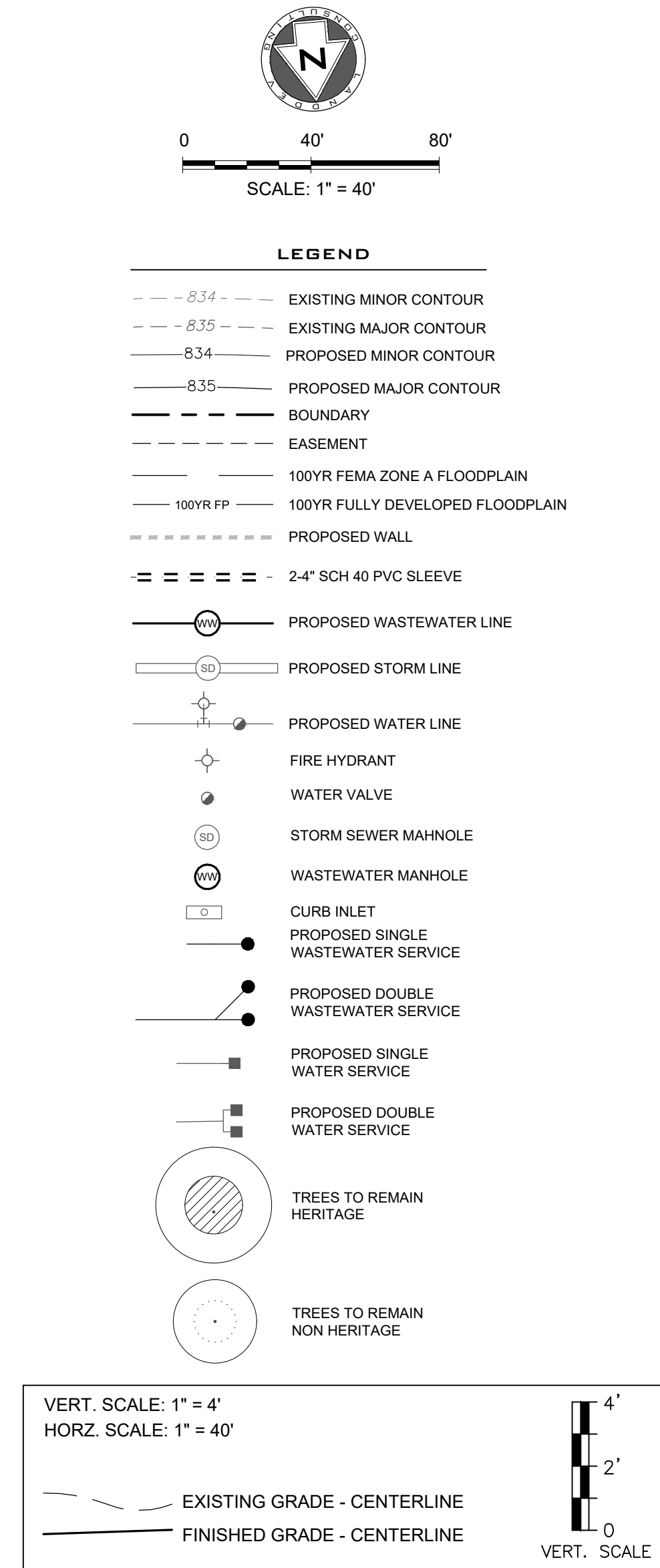


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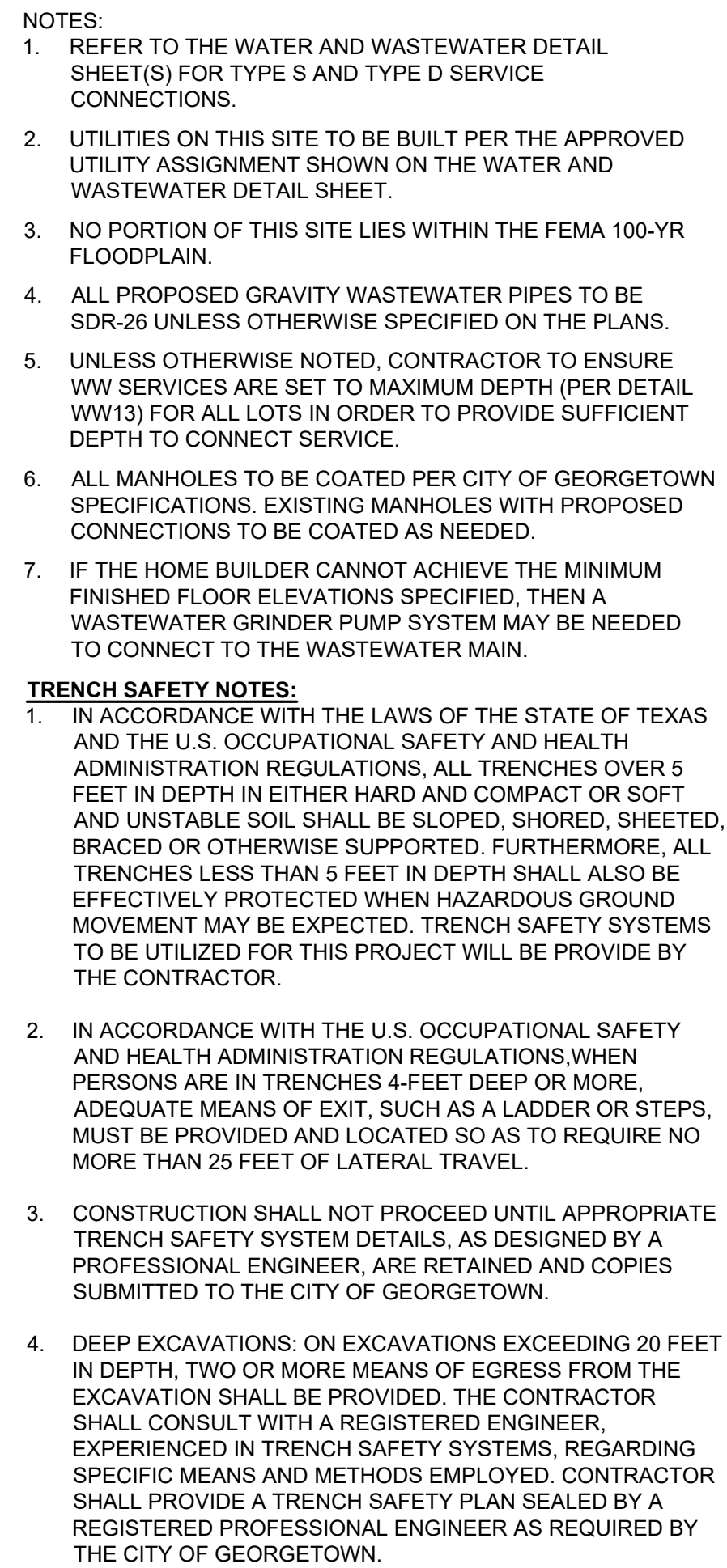
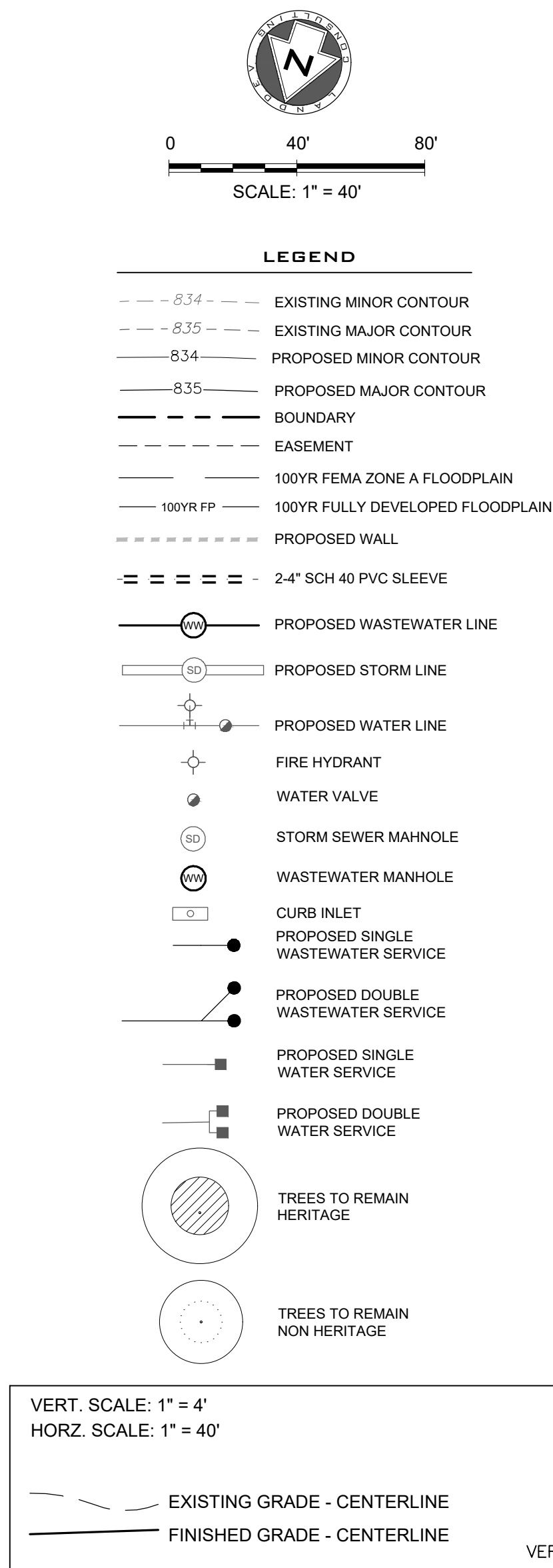






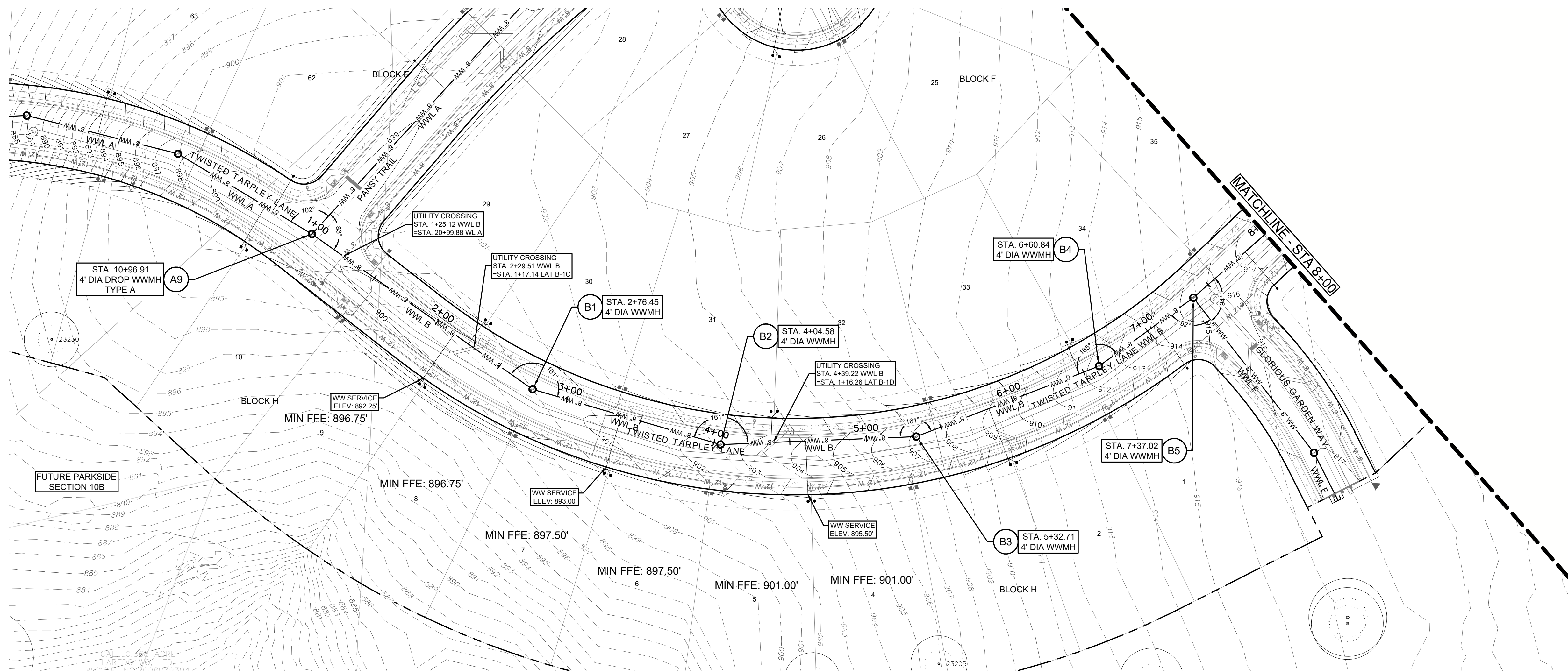




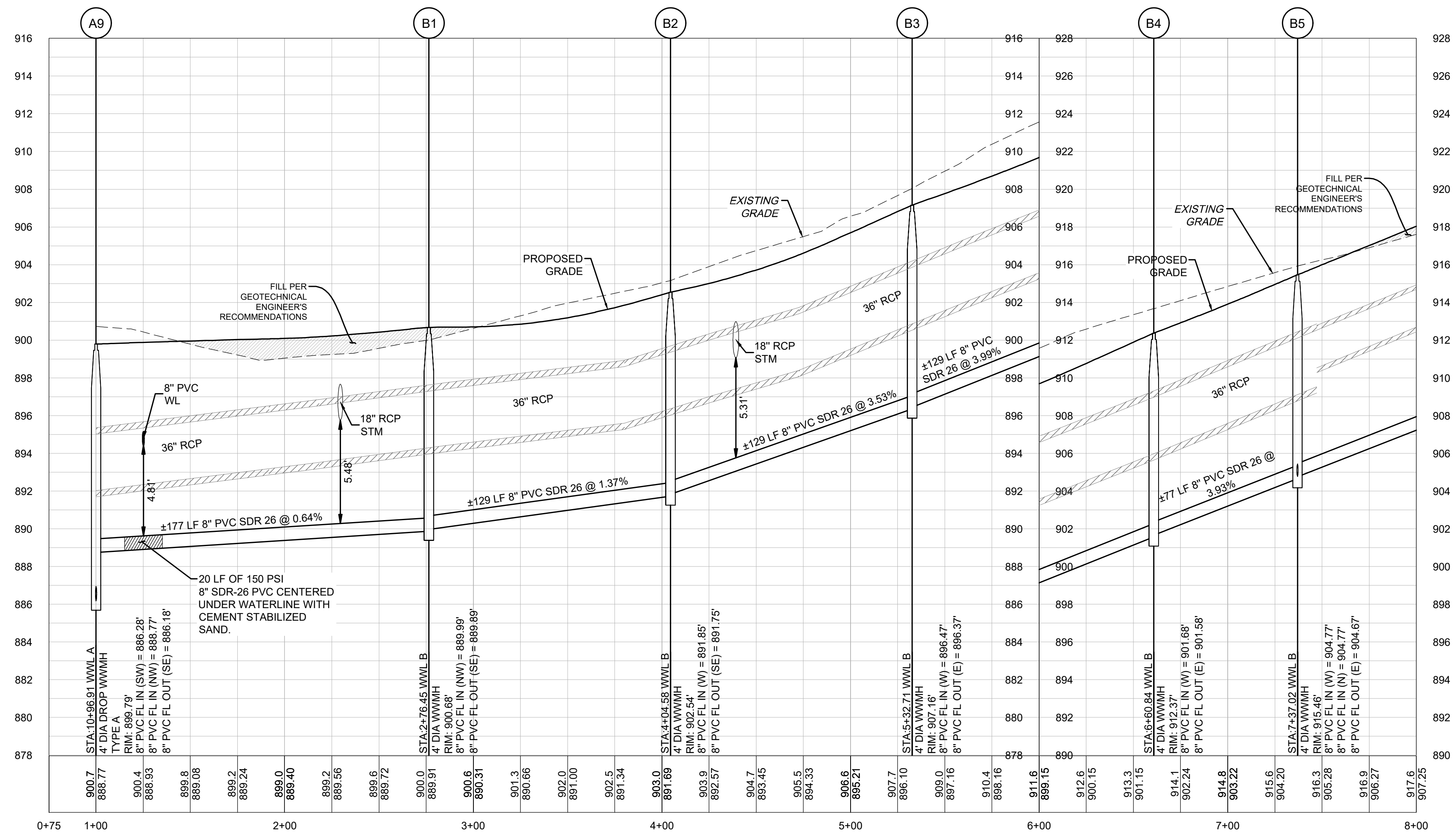




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WWL B



- NOTES:
1. REFER TO THE WATER AND WASTEWATER DETAIL SHEET(S) FOR TYPE S AND TYPE D SERVICE CONNECTIONS.
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WWL B PLAN & PROFILE  
1+00 - 8+00

PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS

GEORGETOWN, WILLIAMSON, TEXAS

DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
SHEET 70 OF 84  
2023-XX-CON

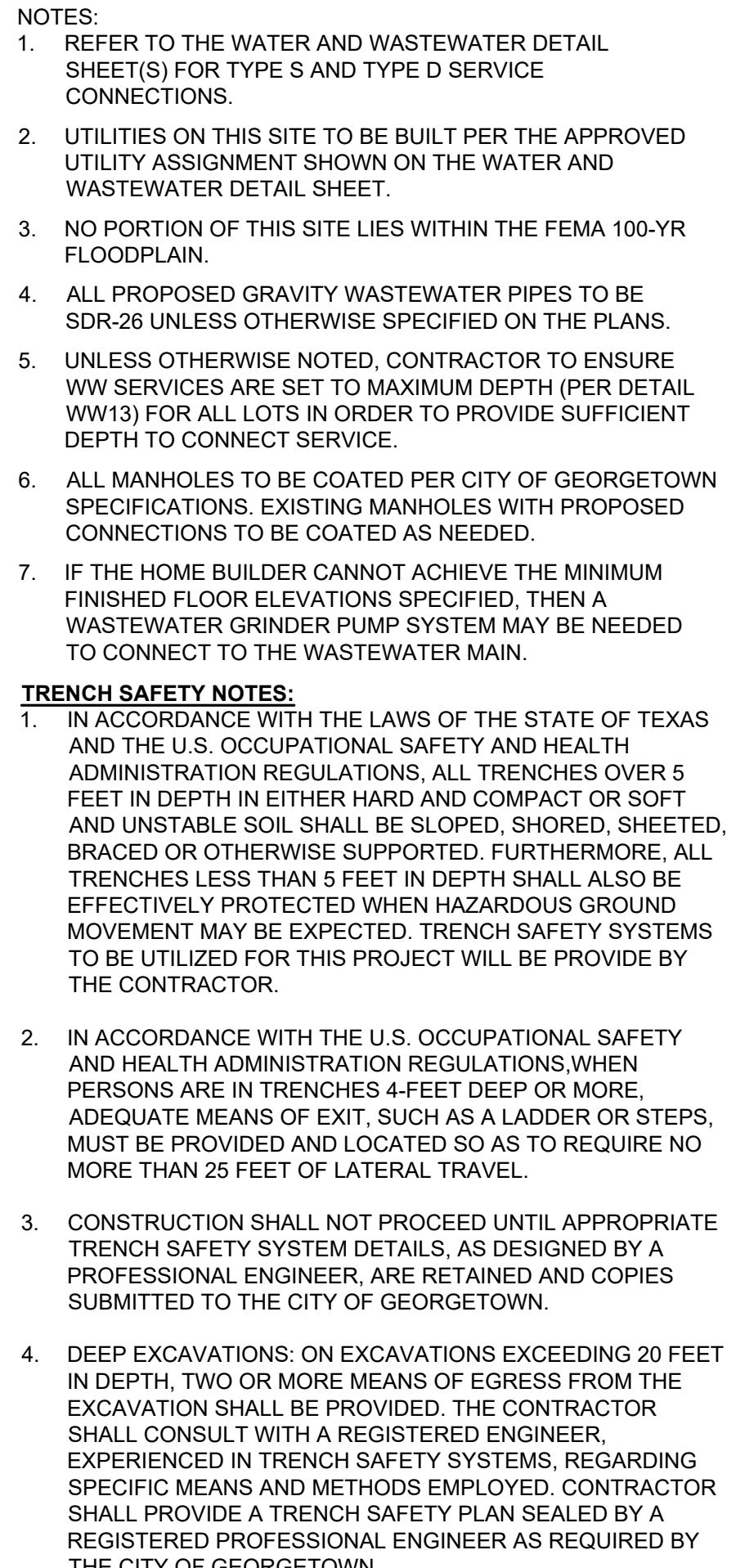
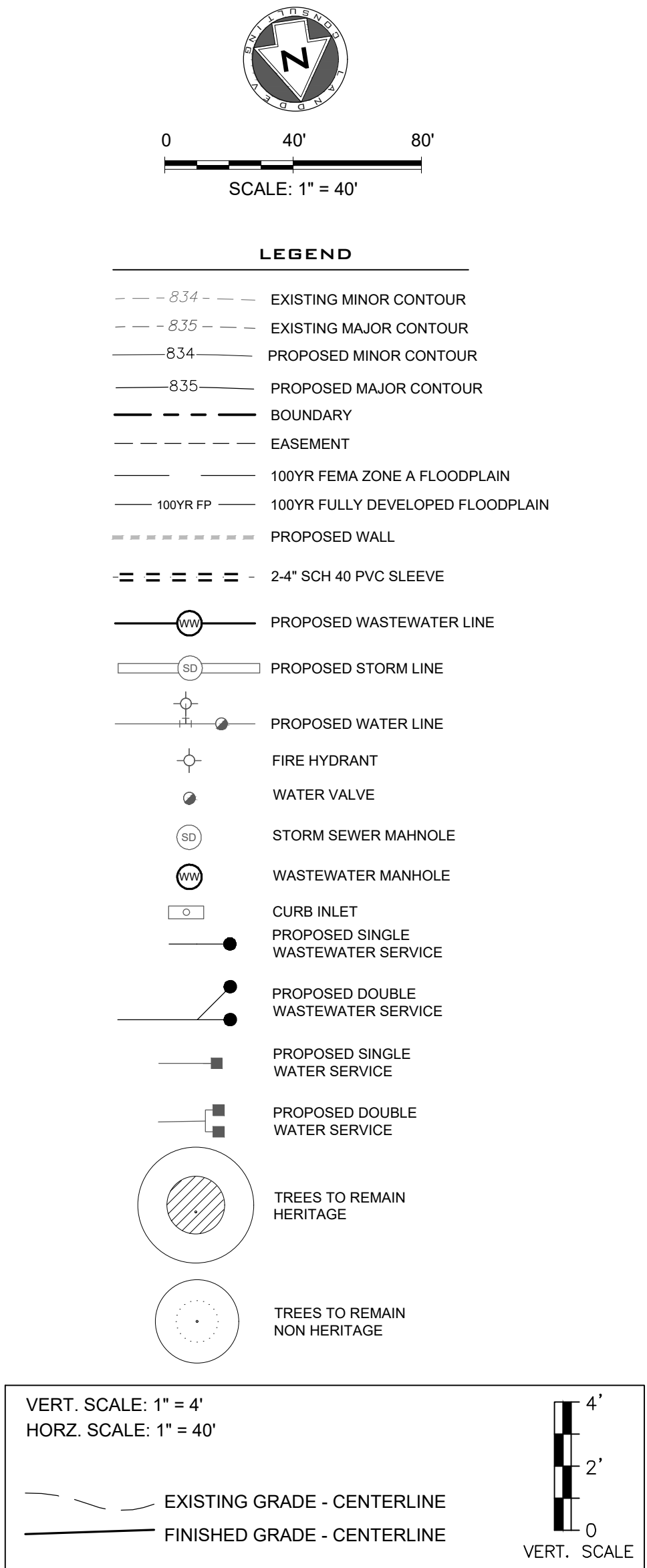
811  
Know what's below.  
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5508 HERMAN 2ND WEST  
SUITE 100  
AUSTIN, TX 78735  
512.872.6996  
HGREEN.COM

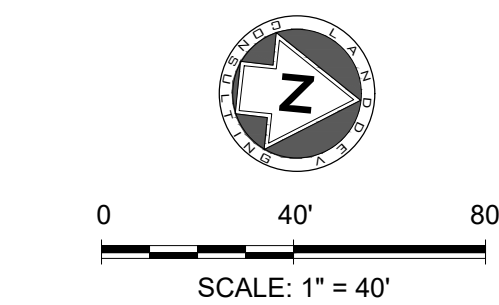
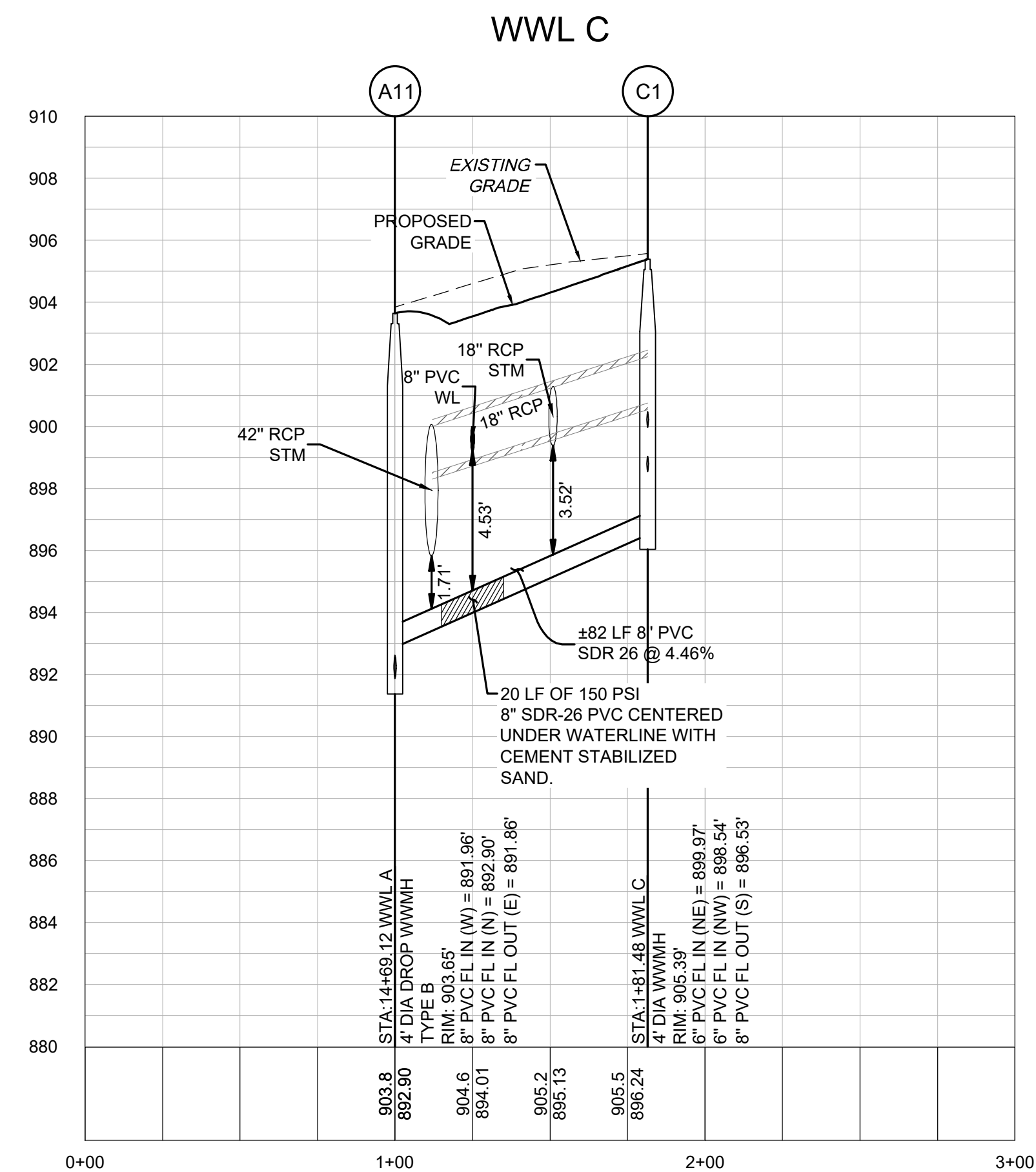
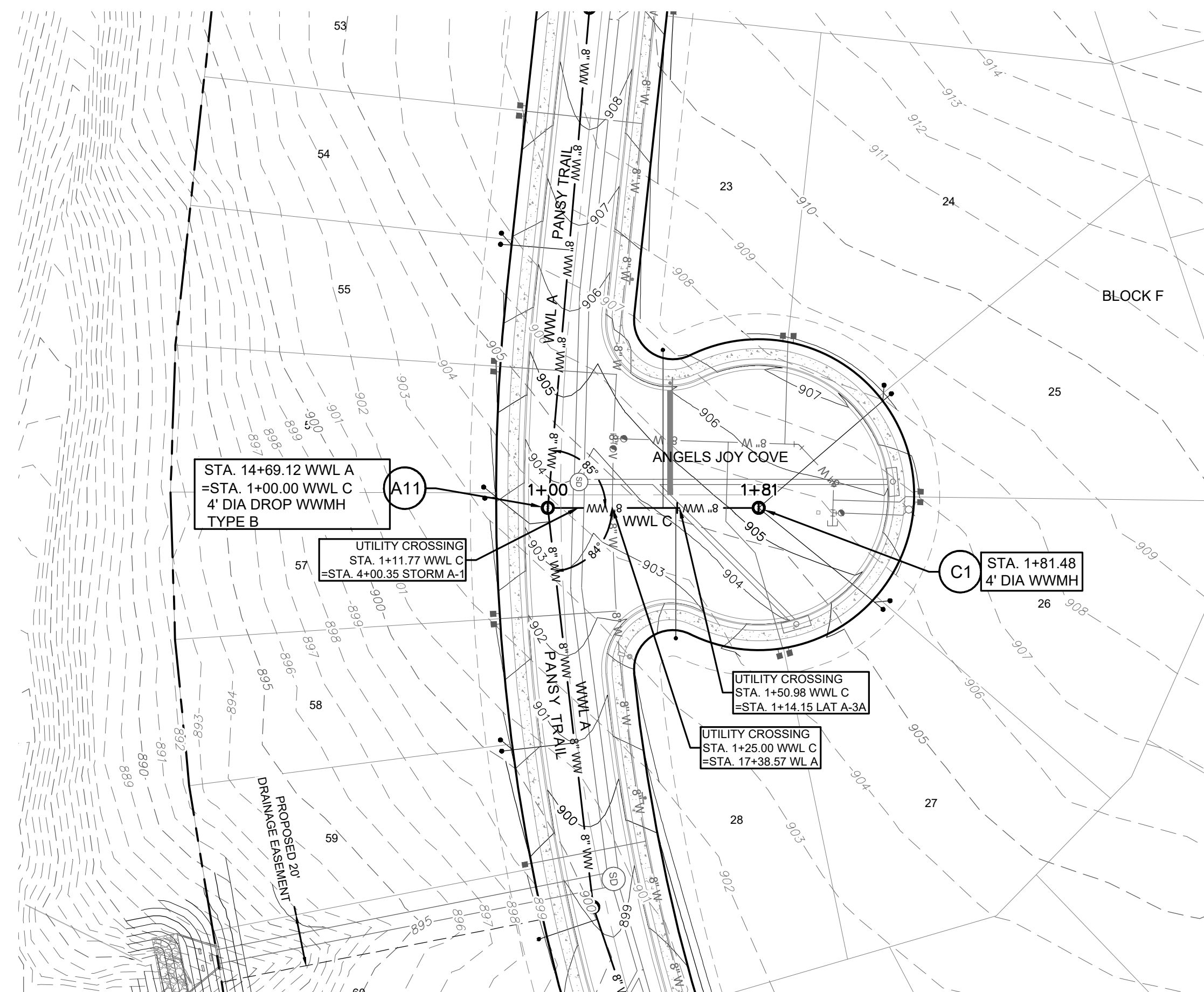
HRGreen  
DEVELOPMENT TX

STATE OF TEXAS  
SHERVIN NOOSHIN  
96807  
LICENSED PROFESSIONAL ENGINEER  
11/15/2023

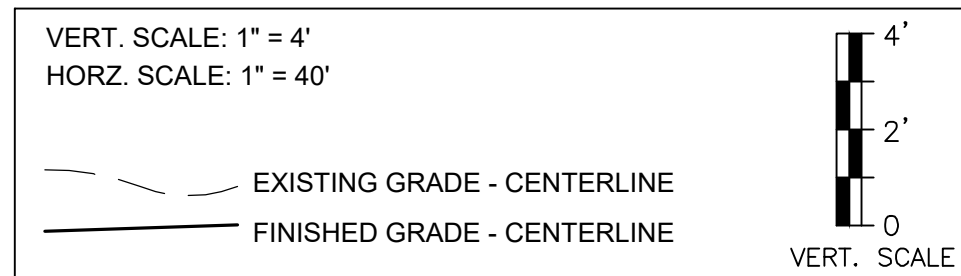








| LEGEND |          |                                    |
|--------|----------|------------------------------------|
| ---    | 834      | EXISTING MINOR CONTOUR             |
| ---    | 835      | EXISTING MAJOR CONTOUR             |
| ---    | 834      | PROPOSED MINOR CONTOUR             |
| ---    | 835      | PROPOSED MAJOR CONTOUR             |
| ---    |          | BOUNDARY                           |
| ---    |          | EASEMENT                           |
| ---    |          | 100YR FEMA ZONE A FLOODPLAIN       |
| ---    | 100YR FP | 100YR FULLY DEVELOPED FLOODPLAIN   |
| ---    |          | PROPOSED WALL                      |
| ---    |          | 2-4" SCH 40 PVC SLEEVE             |
| ---    | WW       | PROPOSED WASTEWATER LINE           |
| ---    | SW       | PROPOSED STORM LINE                |
| ---    |          | PROPOSED WATER LINE                |
| ---    |          | FIRE HYDRANT                       |
| ---    |          | WATER VALVE                        |
| ---    | SD       | STORM SEWER MAHNOLE                |
| ---    | WW       | WASTEWATER MANHOLE                 |
| ---    | C        | CURB INLET                         |
| ---    |          | PROPOSED SINGLE WASTEWATER SERVICE |
| ---    |          | PROPOSED DOUBLE WASTEWATER SERVICE |
| ---    |          | PROPOSED SINGLE WATER SERVICE      |
| ---    |          | PROPOSED DOUBLE WATER SERVICE      |
| ---    |          | TREES TO REMAIN HERITAGE           |
| ---    |          | TREES TO REMAIN NON HERITAGE       |



## NOTES

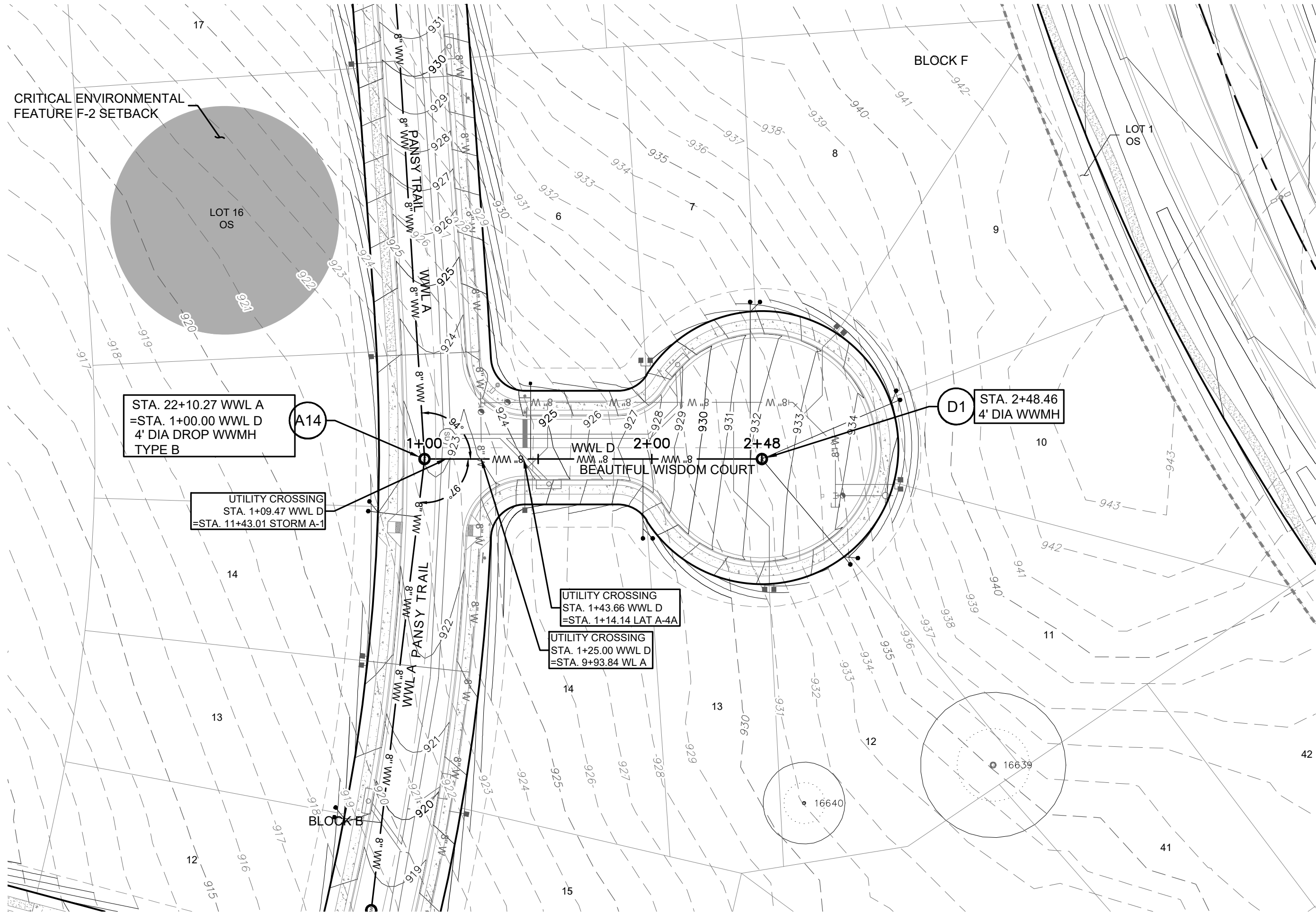
1. REFER TO THE WATER AND WASTEWATER DETAIL SHEET(S) FOR TYPE A AND TYPE D SERVICE CONNECTIONS.
2. UTILITIES ON THIS SITE TO BE BUILT PER THE APPROVED UTILITY ASSIGNMENT SHOWN ON THE WATER AND WASTEWATER DETAIL SHEET.
3. NO PORTION OF THIS SITE LIES WITHIN THE FEMA 100-YR FLOODPLAIN.
4. ALL PROPOSED GRAVITY WASTEWATER PIPES TO BE SDR-26 UNLESS OTHERWISE SPECIFIED ON THE PLANS.
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6. ALL MANHOLES TO BE COATED PER CITY OF GEORGETOWN SPECIFICATIONS. EXISTING MANHOLES WITH PROPOSED CONNECTIONS TO BE COATED AS NEEDED.
7. IF THE HOME OWNER CANNOT ACHIEVE THE MINIMUM FINISHED FLOOR ELEVATIONS SPECIFIED, THEN A WASTEWATER GRINDER PUMP SYSTEM MAY BE NEEDED TO CONNECT TO THE WASTEWATER MAIN.

**TRENCH SAFETY NOTES:**

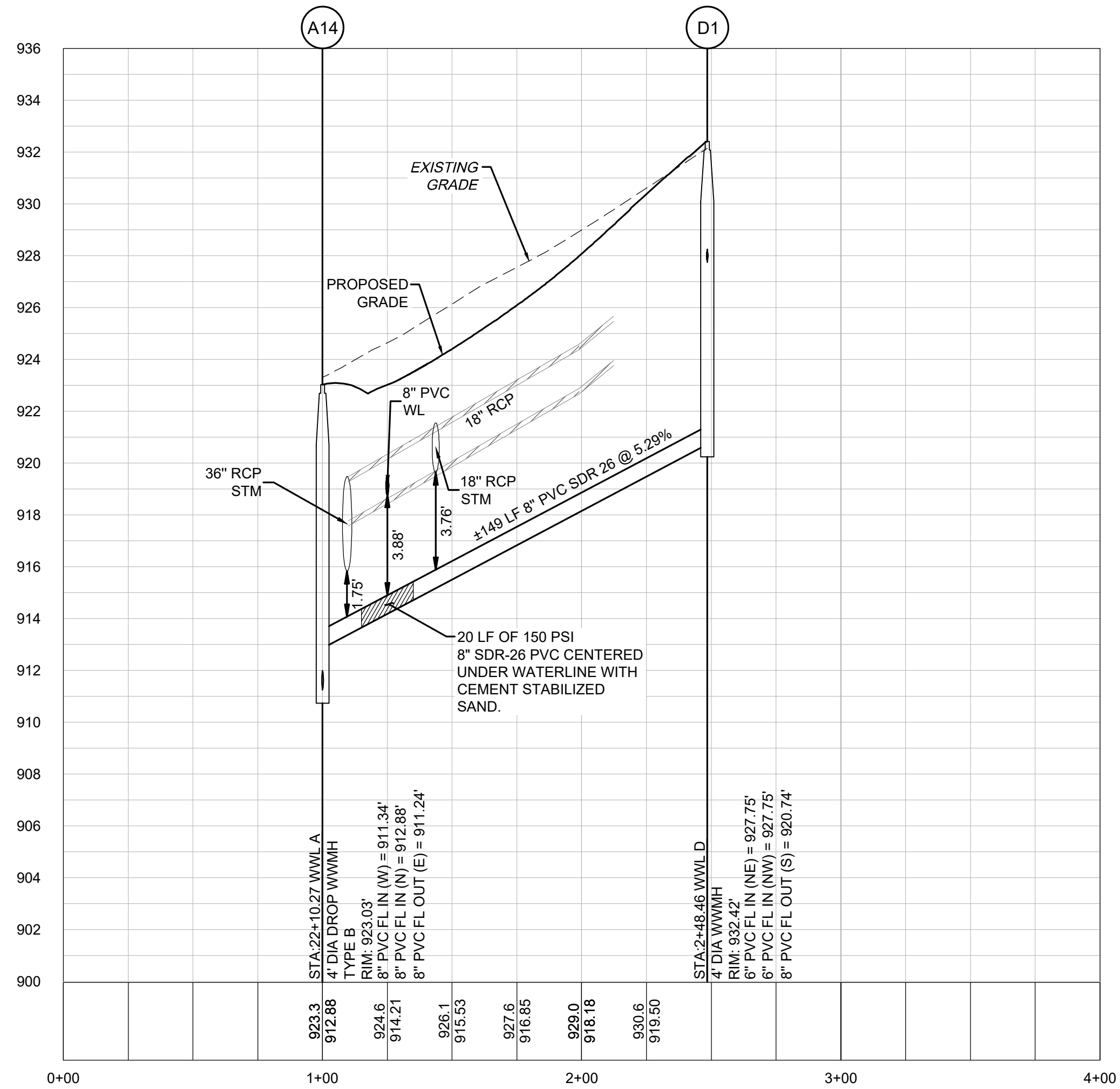
1. IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, ALL TRENCHES OVER 5 FEET IN DEPTH IN EITHER HARD AND COMPACT OR SOFT AND UNSTABLE SOIL SHALL BE SLOPED, SHORED, SHEETED, BRACED OR OTHERWISE SUPPORTED. FURTHERMORE, ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT WILL BE PROVIDED BY THE CONTRACTOR.
2. IN ACCORDANCE WITH THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4-FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED SO AS TO REQUIRE NO MORE THAN 25 FEET OF LATERAL TRAVEL.
3. CONSTRUCTION SHALL NOT PROCEED UNTIL APPROPRIATE TRENCH SAFETY SYSTEM DETAILS, AS DESIGNED BY A PROFESSIONAL ENGINEER, ARE RETAINED AND COPIES SUBMITTED TO THE CITY OF GEORGETOWN.
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P:\blake.magnifico\parkside sections 8, 9a & 10a\03\_ACAD\plans\sh-203296\_WWPP\_WWL D.dwg, WWL D PLAN & PROFILE 1+00 - END, December 19, 2023, 10:37 AM, ccampbell



WWL D

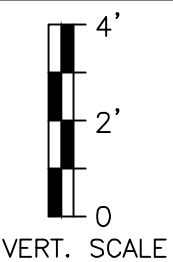


0 40' 80'  
SCALE: 1" = 40'

LEGEND

- 834 --- EXISTING MINOR CONTOUR
- 835 --- EXISTING MAJOR CONTOUR
- 834 --- PROPOSED MINOR CONTOUR
- 835 --- PROPOSED MAJOR CONTOUR
- BOUNDARY
- EASEMENT
- 100YR FEMA ZONE A FLOODPLAIN
- 100YR FP 100YR FULLY DEVELOPED FLOODPLAIN
- PROPOSED WALL
- 24" SCH 40 PVC SLEEVE
- PROPOSED WASTEWATER LINE
- PROPOSED STORM LINE
- PROPOSED WATER LINE
- FIRE HYDRANT
- WATER VALVE
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- WASTEWATER MANHOLE
- CURB INLET
- PROPOSED SINGLE WASTEWATER SERVICE
- PROPOSED DOUBLE WASTEWATER SERVICE
- PROPOSED SINGLE WATER SERVICE
- PROPOSED DOUBLE WATER SERVICE
- TREES TO REMAIN HERITAGE
- TREES TO REMAIN NON HERITAGE

VERT. SCALE: 1" = 4'  
HORIZ. SCALE: 1" = 40'



NOTES:

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- ALL MANHOLES TO BE COATED PER CITY OF GEORGETOWN SPECIFICATIONS. EXISTING MANHOLES WITH PROPOSED CONNECTIONS TO BE COATED AS NEEDED.
- IF THE HOME BUILDER CANNOT ACHIEVE THE MINIMUM FINISHED FLOOR ELEVATIONS SPECIFIED, THEN A WASTEWATER GRINDER PUMP SYSTEM MAY BE NEEDED TO CONNECT TO THE WASTEWATER MAIN.

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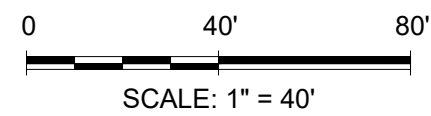
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SUITE 100  
AUSTIN, TX 78735  
512.872.6996  
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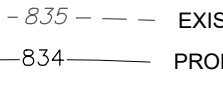










WWL D PLAN &  
PROFILE 1+00 - END  
PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS  
GEORGETOWN, WILLIAMSON, TEXAS










DESIGNED BY: CC  
DRAWN BY: MM/MKM  
CHECKED BY: SN  
APPROVED BY: \_\_\_\_\_  
SHEET 73 OF 84  
2023-XX-CON






## LEGEND


- 
-  EXISTING MINOR CONTOUR
  -  EXISTING MAJOR CONTOUR
  -  PROPOSED MINOR CONTOUR
  -  PROPOSED MAJOR CONTOUR
  -  BOUNDARY
  -  EASEMENT
  -  100YR FEMA ZONE A FLOODPLAIN
  -  100YR FULLY DEVELOPED FLOODPLAIN
  -  PROPOSED WALL
  -  2-4" SCH 40 PVC SLEEVE

-  PROPOSED WASTEWATER LINE  
 PROPOSED STORM LINE  
 PROPOSED WATER LINE  
 FIRE HYDRANT  
 WATER VALVE  
 STORM SEWER MAHNOLE  
 WASTEWATER MAHNOLE  
 CURB INLET  
 PROPOSED SINGLE WASTEWATER SERVICE

- 
- PROPOSED DOUBLE  
WASTEWATER SERVICE

- 
- PROPOSED SINGLE WATER SERVICE

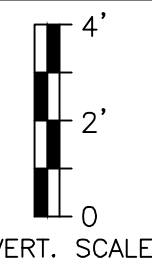
- 
- PROPOSED DOUBLE  
WATER SERVICE

- 
- TREES TO  
HERITAGE**

- 

- 
- TREES TO REMAIN  
NON HERITAGE

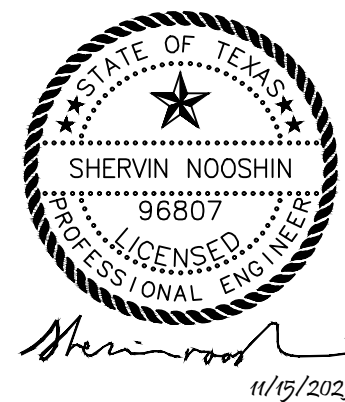
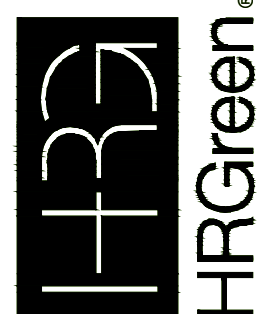
VERT. SCALE: 1" = 4'  
HORIZ. SCALE: 1" = 40'



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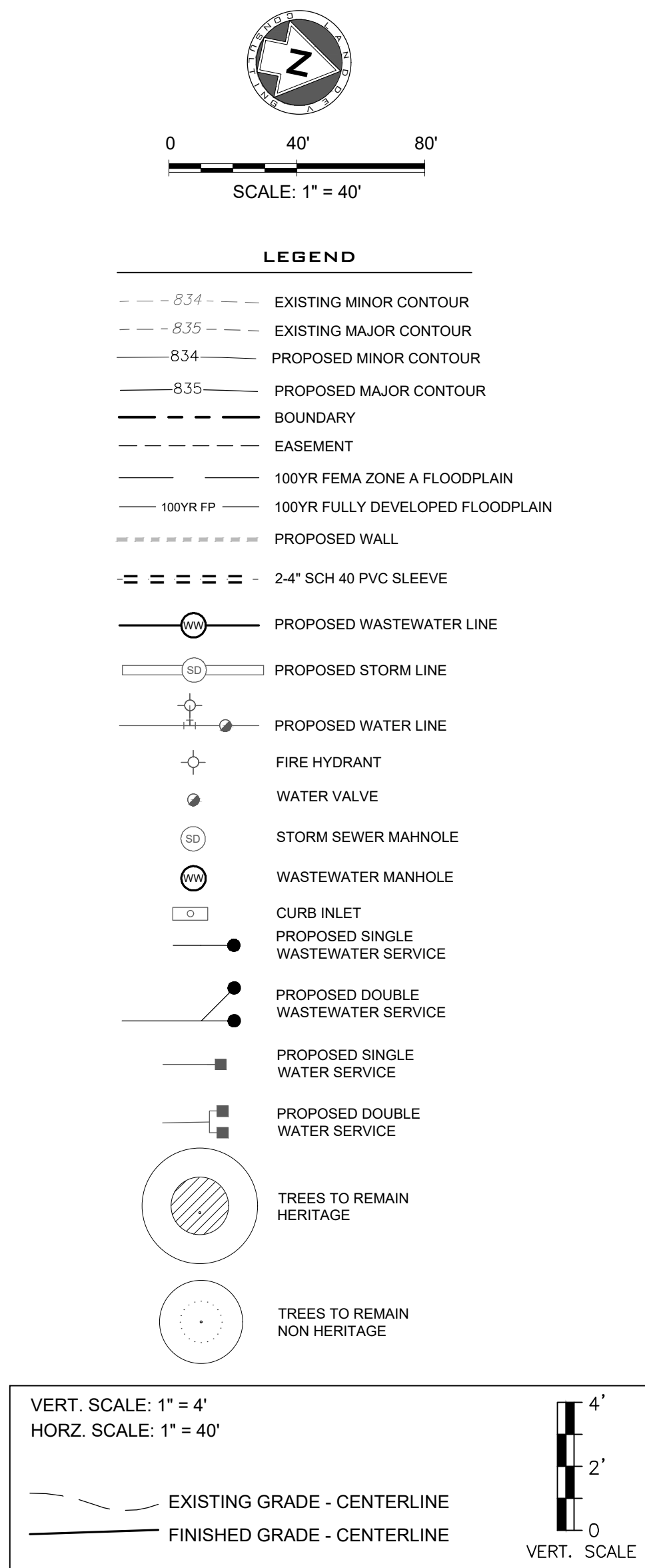
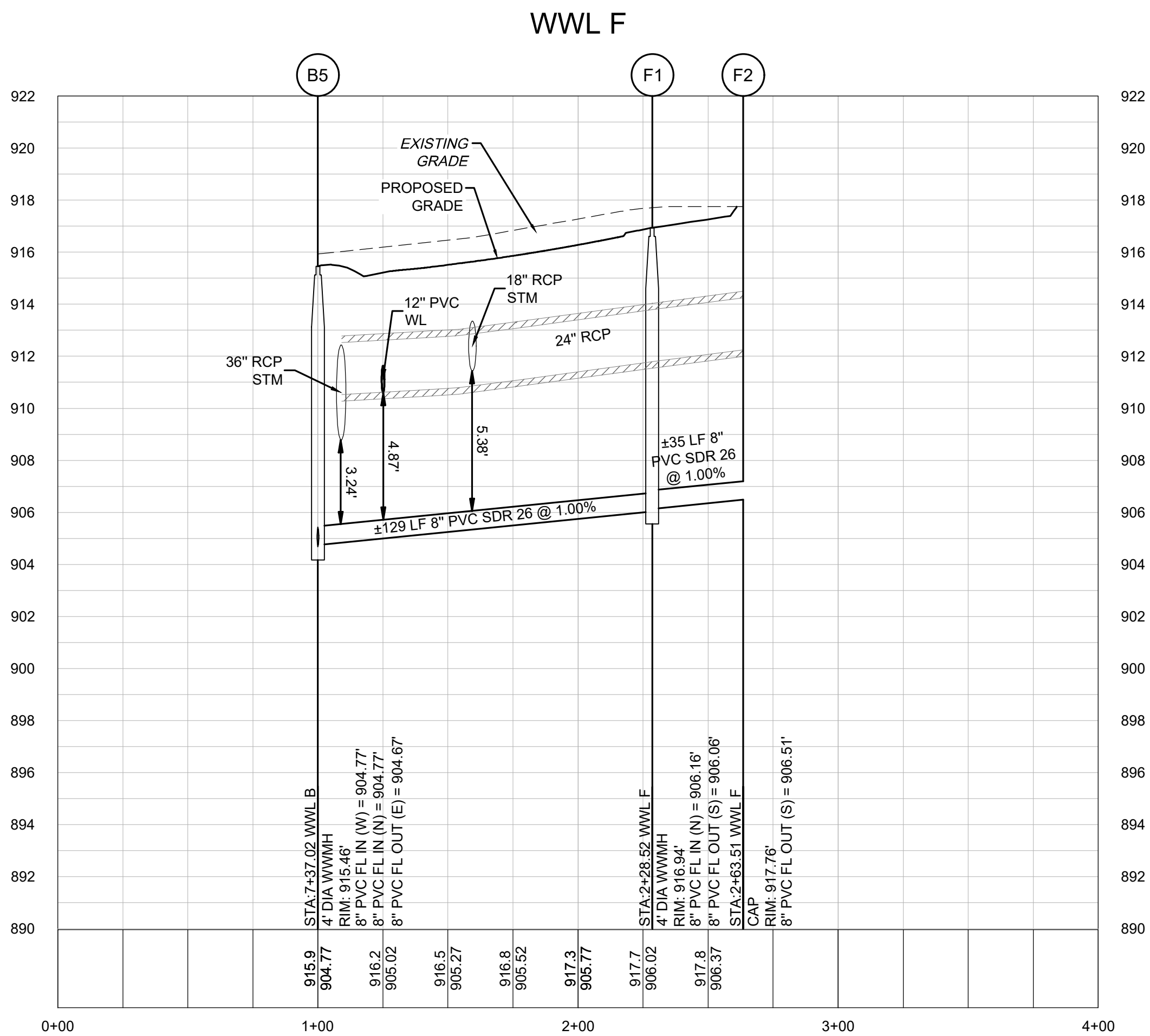
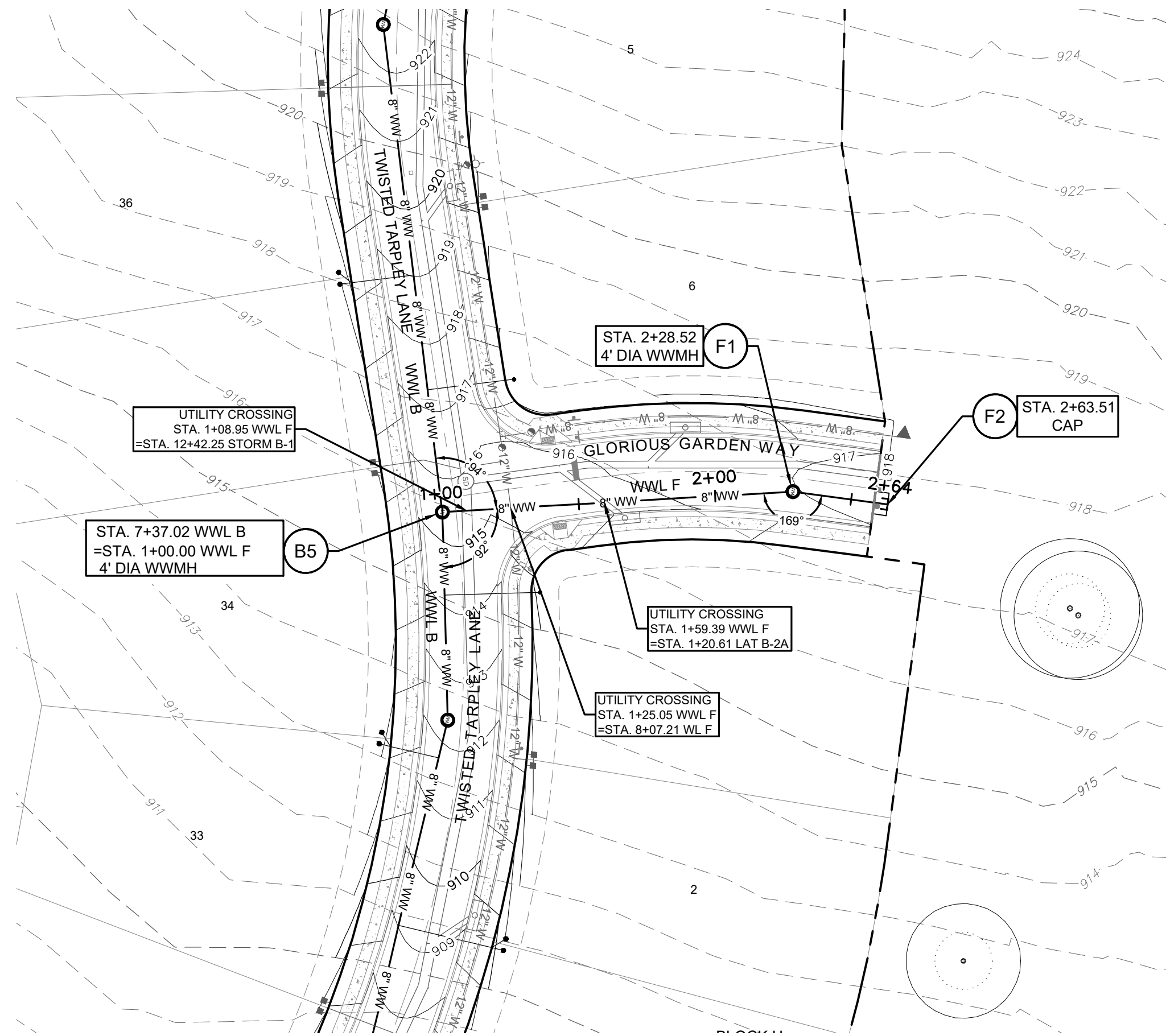
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|                  |    |      |
|------------------|----|------|
| DESIGNED BY: CC  | BY | DATE |
| DRAWN BY: MM/MKM |    |      |
| CHECKED BY: SN   |    |      |
| APPROVED BY:     |    |      |
| SHEET 75 OF 84   |    |      |
| 2023-XX-CON      |    |      |

811

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DEVELOPMENT TX

STATE OF TEXAS  
SHERVIN NOOSHIN  
9680  
LICENSED PROFESSIONAL ENGINEER

11/15/2023

WWL F PLAN & PROFILE  
1+00 - END

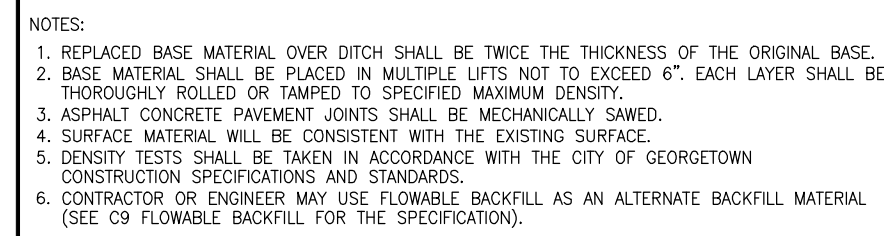
PARKSIDE SECTION 9A & 10A  
CONSTRUCTION PLANS

GEORGETOWN, WILLIAMSON, TEXAS

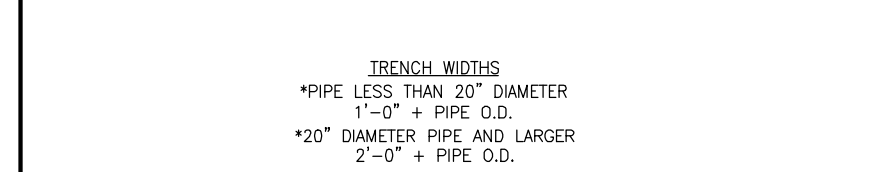
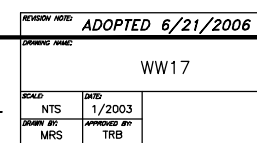








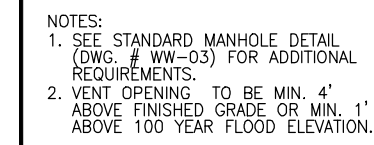
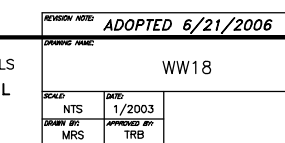
The Architect/Engineer assumes responsibility for appropriate use of this standard.



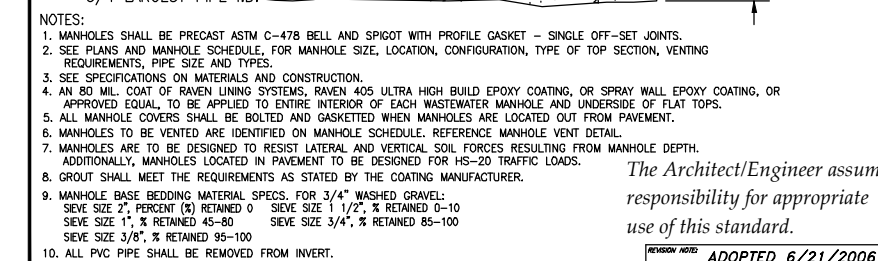
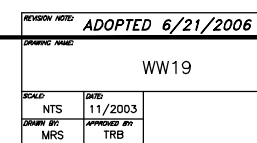
NOTES:

1. DENSITY TESTS SHALL BE TAKEN IN ACCORDANCE WITH THE CITY OF GEORGETOWN CONSTRUCTION SPECIFICATIONS AND STANDARDS.
2. CONTRACTOR OR ENGINEER MAY USE FLOWABLE BACKFILL AS AN ALTERNATE BACKFILL MATERIAL (SEE C9 FLOWABLE BACKFILL FOR THE SPECIFICATION).

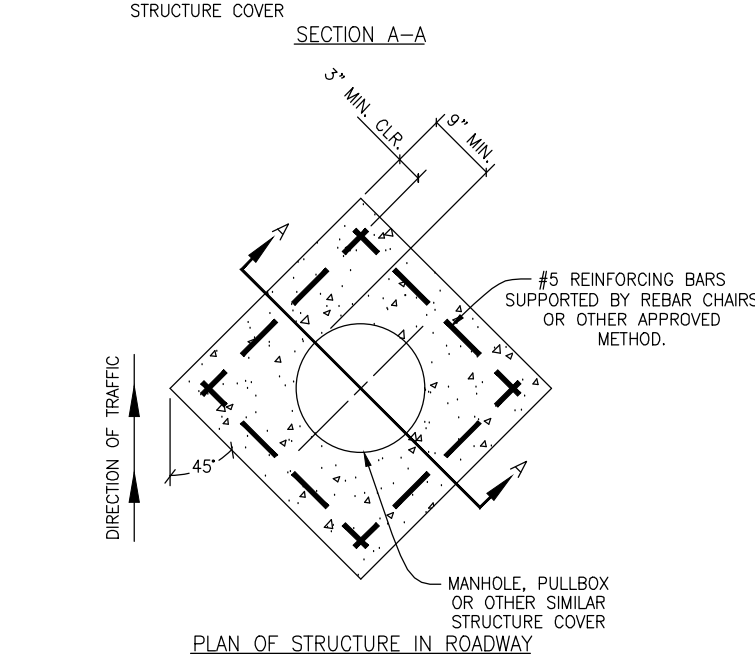
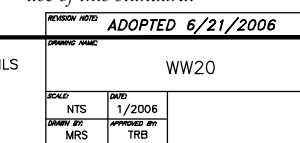
*The Architect/Engineer assumes responsibility for appropriate use of this standard.*



The Architect/Engineer assumes responsibility for appropriate use of this standard.

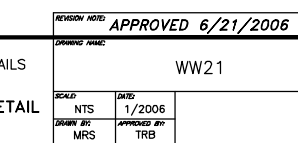


*The Architect/Engineer assumes responsibility for appropriate use of this standard.*



NOTE:  
INSTALLATION OF THE CONCRETE CASTING IS REQUIRED  
FOR AND APPLIES TO ALL TYPES OF  
MANHOLES TO BE LOCATED IN THE ROADWAY.

The Architect/Engineer assumes responsibility for appropriate use of this standard.

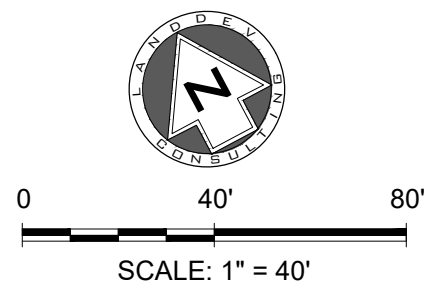
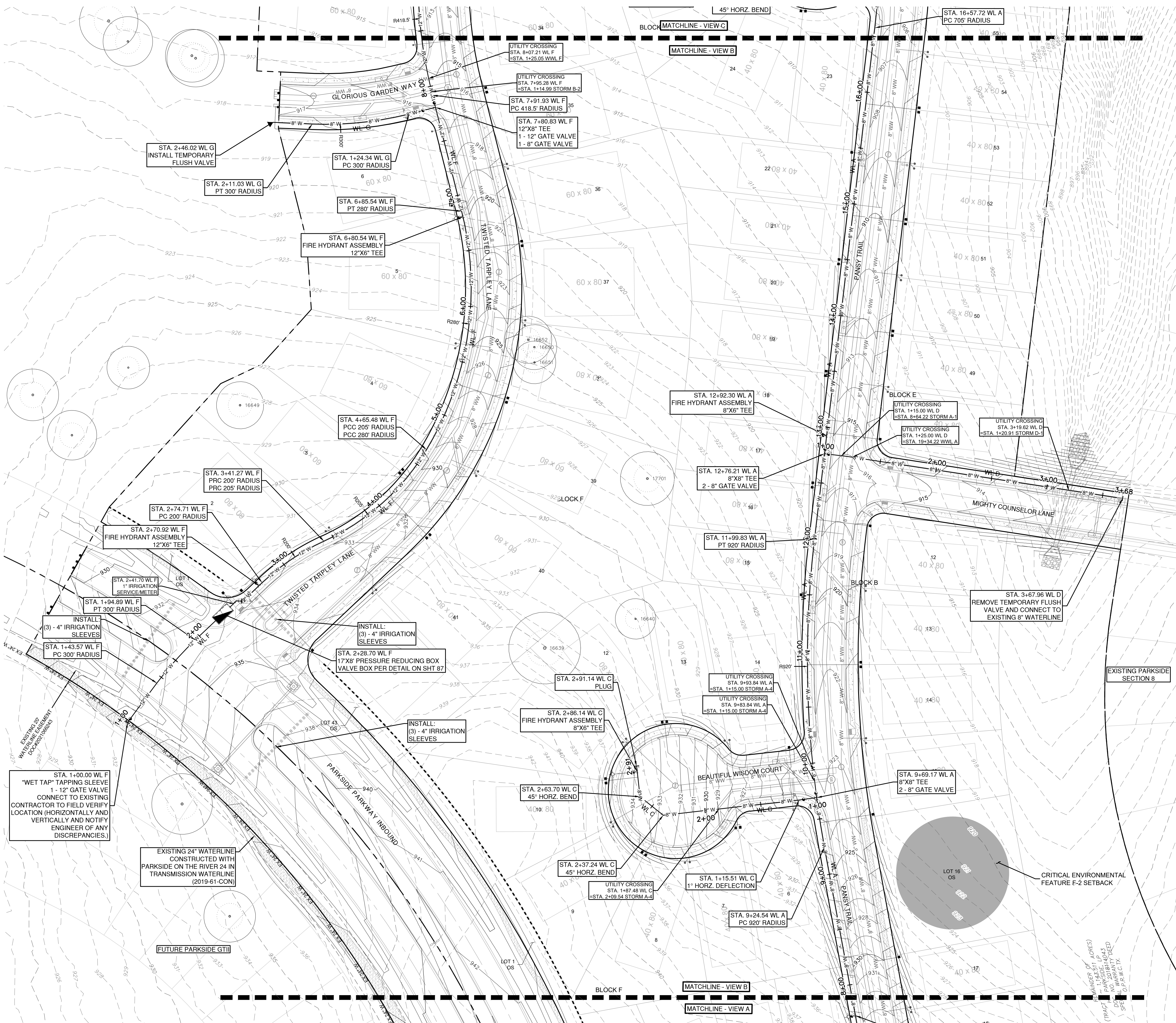








P:\Public\magnaparkside sections 8, 9 & 10.dwg, ACAD\Plan\magnaparkside sections 8, 9 & 10.dwg, December 19, 2023, 10:41 AM campbell



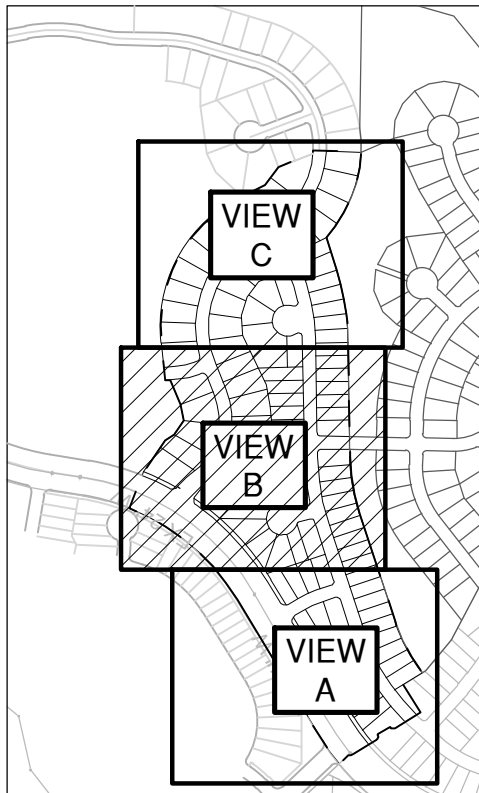
- LEGEND**
- 8.34 --- EXISTING MINOR CONTOUR
  - 8.35 --- EXISTING MAJOR CONTOUR
  - 8.34 --- PROPOSED MINOR CONTOUR
  - 8.35 --- PROPOSED MAJOR CONTOUR
  - --- BOUNDARY
  - --- EASEMENT
  - 8" WW --- PROPOSED WASTEWATER LINE
  - --- PROPOSED STORM LINE
  - 8" W --- PROPOSED WATER LINE
  - TREES TO REMAIN - HERITAGE
  - TREES TO REMAIN - NON-HERITAGE
  - FIRE HYDRANT
  - WATER VALVE
  - TEMP FLUSH VALVE
  - AIR RELEASE VALVE
  - STORM SEWER MAHNOLE
  - WASTEWATER MANHOLE
  - CURB INLET
  - PROPOSED SINGLE WASTEWATER SERVICE
  - PROPOSED DOUBLE WASTEWATER SERVICE
  - PROPOSED SINGLE WATER SERVICE
  - PROPOSED DOUBLE WATER SERVICE

- NOTES:**
- ALL PROPOSED WATERLINES EXCEPT FIRE LEADS SHALL BE PVC C900 DR 18 PER CITY OF GEORGETOWN SPECIFICATIONS.
  - THRUST BLOCKING SHALL BE PROVIDED AT BENDS, PTS, TEES & FITTINGS ACCORDING TO CITY OF GEORGETOWN.
  - ALL JOINT RESTRAINT SHALL BE IN ACCORDANCE TO CITY OF GEORGETOWN SPECIFICATION. REFER TO THE JOINT RESTRAINT TABLE ON SHEET 84.
  - ANY JOINT DEFLECTION VERTICAL OR HORIZONTAL SHALL BE WITHIN THE MANUFACTURES MAXIMUM ALLOWABLE JOINT DEFLECTION.
  - NO PORTION OF THIS PROPOSED DEVELOPMENT LIES WITHIN THE FEMA 100-YR FLOODPLAIN.
  - AT THE CONCLUSION OF CONSTRUCTION AND AS PART OF THE PROCESS FOR THE CITY TO ACCEPT THIS PHASE, THE FIRE HYDRANTS SHALL BE FLOWED AND TESTED. A COPY OF THE REPORT SHALL BE EMAILED INTO THE FIRE DEPARTMENT AND THE HYDRANTS SHALL BE PAINTED AND COLOR CODED.
  - CAUTION: IF PRESSURE REDUCING VALVES WERE INSTALLED IN THIS PHASING, THEY MUST BE SET PRIOR TO FIRE HYDRANT FLOW TESTING.
  - CONTRACTOR AND HOME BUILDER TO ENSURE GATE VALVES ARE ADJUSTED FOR FINISHED GRADES.

**GEORGETOWN FIRE DEPARTMENT NOTES**

- 1,500 GPM FIRE FLOW SHALL BE PROVIDED FOR THIS PROJECT.
- AT THE CONCLUSION OF CONSTRUCTION AND AS PART OF THE PROCESS FOR THE CITY TO ACCEPT THIS PHASE:
  - THE FIRE HYDRANTS SHALL BE FLOWED AND TESTED
  - A COPY OF THE REPORT SHALL BE EMAILED INTO THE FIRE DEPARTMENT
  - THE HYDRANTS SHALL BE PAINTED AND COLOR CODED.
- PER CITY ORDINANCE SEC. 13.15.120, HYDRANT FLOW CODING STANDARDS. PUBLIC HYDRANTS WILL HAVE THE BARRELS PAINTED SILVER, THE HYDRANTS WILL BE FLOW TESTED, AND THE BONNET PAINTED USING THE HYDRANT FLOW STANDARD IN PARAGRAPH C. FLOW COLOR:
  - GREATER THAN 1500 GPM BLUE
  - 1000 TO 1500 GPM GREEN
  - 500 - 999 GPM ORANGE
  - LESS THAN 500 GPM RED
  - NOT WORKING BLACK OR BAGGED

\*\*\* CAUTION: IF PRESSURE REDUCING VALVES WERE INSTALLED IN THIS PHASING THEY MUST BE SET PRIOR TO FIRE HYDRANT FLOW TESTING.



|                                                                                                                                                                   |  |
|-------------------------------------------------------------------------------------------------------------------------------------------------------------------|--|
| DATE                                                                                                                                                              |  |
| BY                                                                                                                                                                |  |
| REVISION                                                                                                                                                          |  |
| NO.                                                                                                                                                               |  |
| <br>Know what's below.<br>Call before you dig.                                 |  |
| 5508 HIGHWAY 290 WEST<br>SUITE 150<br>DUSTIN<br>CITY OF GEORGETOWN, TEXAS<br>TDCS NO. 10194101                                                                    |  |
| TDCS NO. 10384<br>TDCS NO. 10194101                                                                                                                               |  |
| <br>HRGreen®<br>DEVELOPMENT TX                                                |  |
| <br>SHERWIN NOOSHIN<br>96807<br>LICENSED PROFESSIONAL ENGINEER<br>11/19/2023 |  |
| <b>OVERALL WATER PLAN B</b>                                                                                                                                       |  |
| <b>PARKSIDE SECTION 9A &amp; 10A</b>                                                                                                                              |  |
| <b>CONSTRUCTION PLANS</b>                                                                                                                                         |  |
| <b>GEORGETOWN, WILLIAMSON, TEXAS</b>                                                                                                                              |  |
| DESIGNED BY: CC                                                                                                                                                   |  |
| DRAWN BY: MM/MKM                                                                                                                                                  |  |
| CHECKED BY: SN                                                                                                                                                    |  |
| APPROVED BY:                                                                                                                                                      |  |
| SHEET <b>79</b> OF <b>84</b>                                                                                                                                      |  |
| 2023-XX-CON                                                                                                                                                       |  |



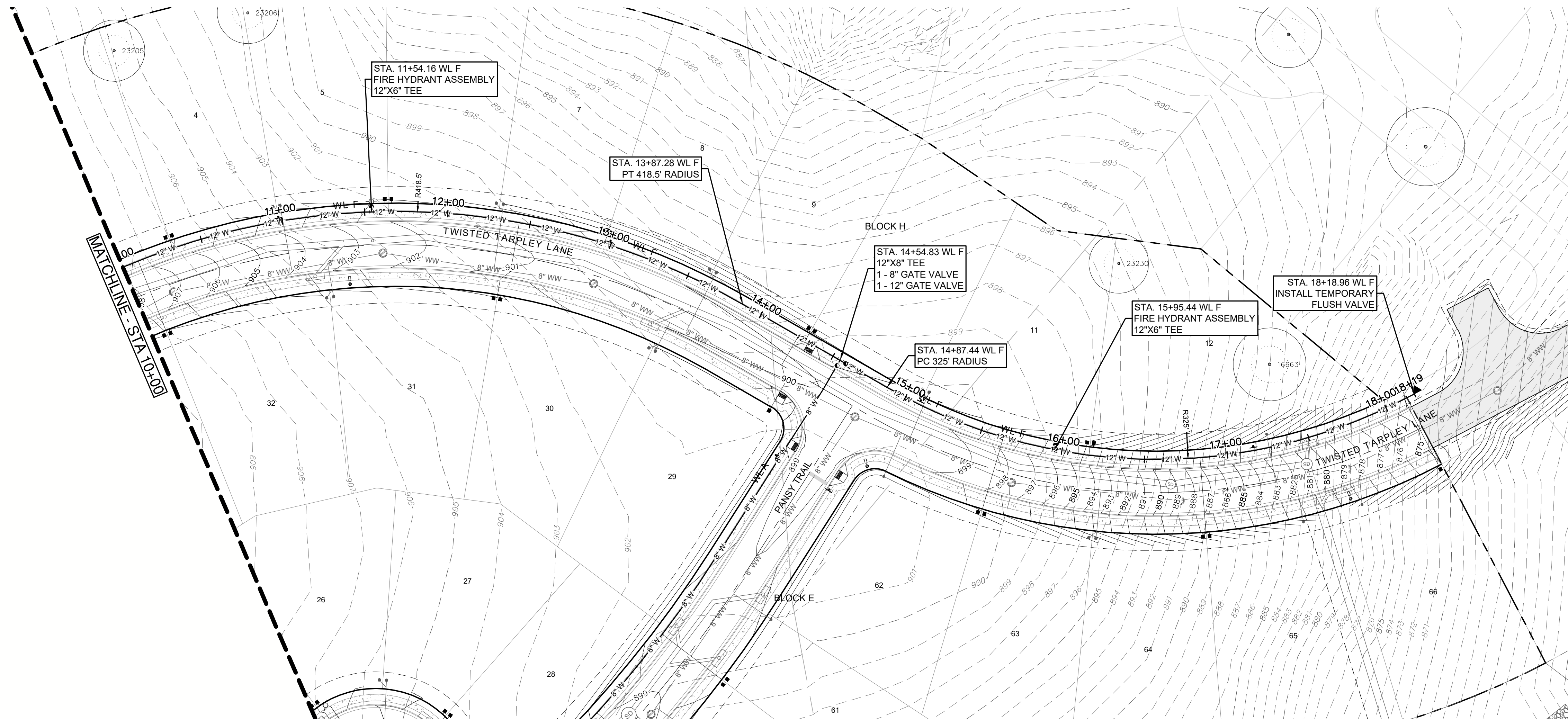




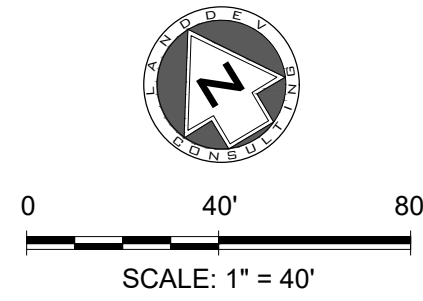
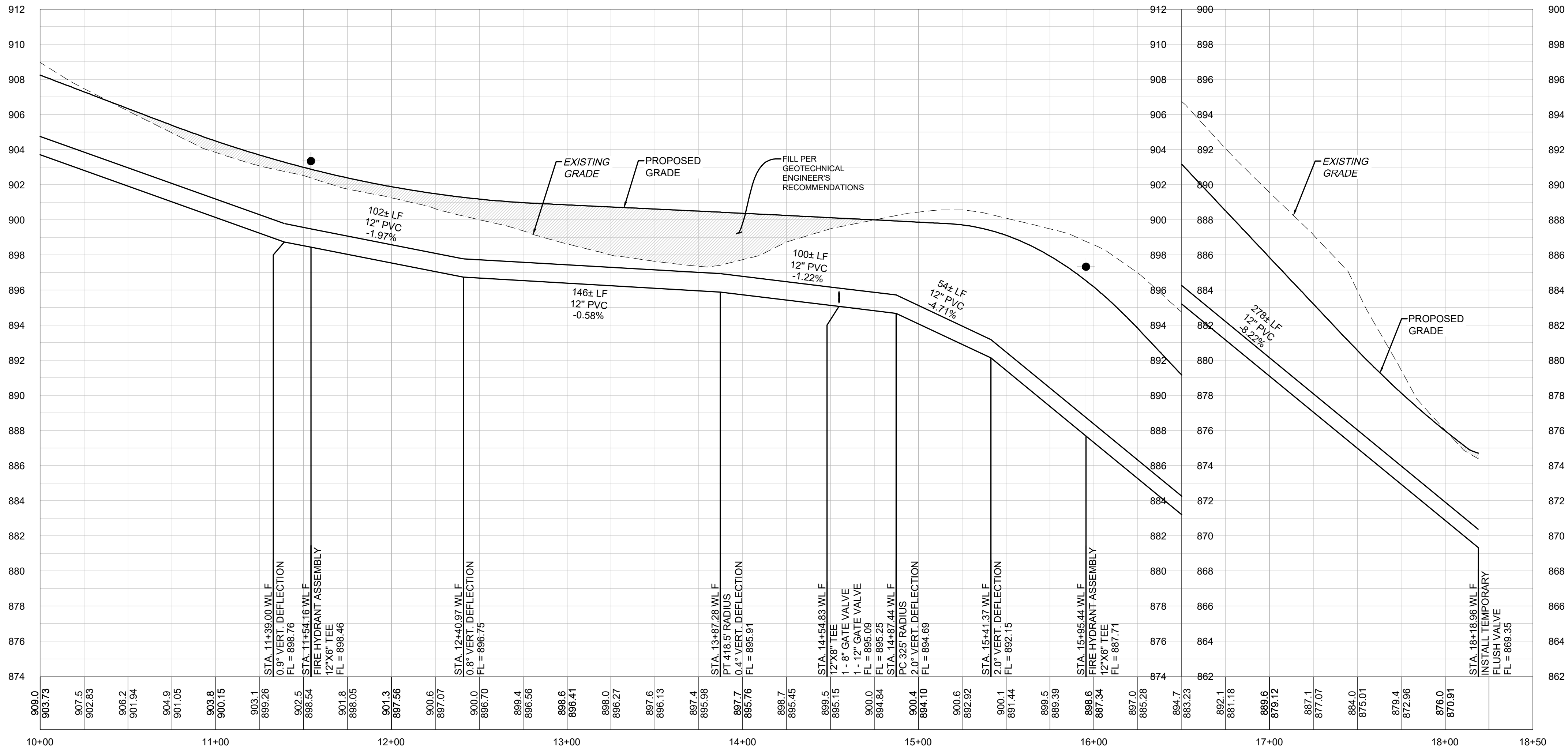




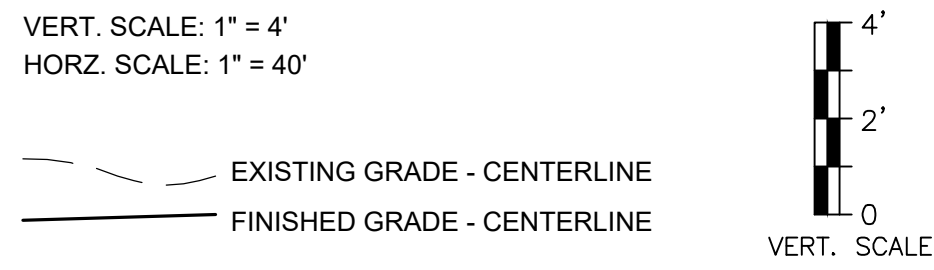
P:\State\magnificent\sections 8, 9a & 10a\03\_ACAD\Plan\sh-203295 WL F PLAN & PROFILE (10+0 - END). December 19, 2023, 10:42 AM, ccampbell



WL F



- LEGEND**
- 834 --- EXISTING MINOR CONTOUR
  - 835 --- EXISTING MAJOR CONTOUR
  - 834 --- PROPOSED MINOR CONTOUR
  - 835 --- PROPOSED MAJOR CONTOUR
  - BOUNDARY
  - EASEMENT
  - 8" WW --- PROPOSED WASTEWATER LINE
  - PROPOSED STORM LINE
  - 8" W --- PROPOSED WATER LINE
  - TREES TO REMAIN - HERITAGE
  - TREES TO REMAIN - NON-HERITAGE
  - FIRE HYDRANT
  - WATER VALVE
  - TEMP FLUSH VALVE
  - AIR RELEASE VALVE
  - STORM SEWER MAINHOLE
  - WASTEWATER MANHOLE
  - CURB INLET
  - PROPOSED SINGLE WASTEWATER SERVICE
  - PROPOSED DOUBLE WASTEWATER SERVICE
  - PROPOSED SINGLE WATER SERVICE
  - PROPOSED DOUBLE WATER SERVICE



- NOTES:**
1. ALL PROPOSED WATERLINES EXCEPT FIRE LEADS SHALL BE PVC C900 DR 18 PER CITY OF GEORGETOWN SPECIFICATIONS.
  2. THRUST BLOCKING SHALL BE PROVIDED AT BENDS, PIS, TEES & FITTINGS ACCORDING TO CITY OF GEORGETOWN.
  3. ALL JOINT RESTRAINT SHALL BE IN ACCORDANCE TO CITY OF GEORGETOWN SPECIFICATION. REFER TO THE JOINT RESTRAINT TABLE ON SHEET 84.
  4. ANY JOINT DEFLECTION VERTICAL OR HORIZONTAL SHALL BE WITHIN THE MANUFACTURES MAXIMUM ALLOWABLE JOINT DEFLECTION.
  5. NO PORTION OF THIS PROPOSED DEVELOPMENT LIES WITHIN THE FEMA 100-YR FLOODPLAIN.
  6. AT THE CONCLUSION OF CONSTRUCTION AND AS PART OF THE PROCESS FOR THE CITY TO ACCEPT THIS PHASE, THE FIRE HYDRANTS SHALL BE FLOWED AND TESTED. A COPY OF THE REPORT SHALL BE EMAILED INTO THE FIRE DEPARTMENT AND THE HYDRANTS SHALL BE PAINTED AND COLOR CODED.
  7. CAUTION: IF PRESSURE REDUCING VALVES WERE INSTALLED IN THIS PHASING, THEY MUST BE SET PRIOR TO FIRE HYDRANT FLOW TESTING.
  8. CONTRACTOR AND HOME BUILDER TO ENSURE GATE VALVES ARE ADJUSTED FOR FINISHED GRADES.

|                                                                                                                                                                   |    |          |     |
|-------------------------------------------------------------------------------------------------------------------------------------------------------------------|----|----------|-----|
| DATE                                                                                                                                                              | BY | REVISION | NO. |
|                                                                                                                                                                   |    |          |     |
| <br>Know what's below.<br>Call before you dig.                                 |    |          |     |
| 5508 HERMAN 2ND WEST<br>SUITE 100<br>AUSTIN, TX 78735<br>512.872.6996<br>HGREEN.COM                                                                               |    |          |     |
| TYPE NO: 14384<br>TBL: NO: 10184101                                                                                                                               |    |          |     |
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| WL F PLAN & PROFILE<br>10+00 - END<br>PARKSIDE SECTION 9A & 10A<br>CONSTRUCTION PLANS<br>GEORGETOWN, WILLIAMSON, TEXAS                                            |    |          |     |
| DESIGNED BY: CC                                                                                                                                                   |    |          |     |
| DRAWN BY: MM/MKM                                                                                                                                                  |    |          |     |
| CHECKED BY: SN                                                                                                                                                    |    |          |     |
| APPROVED BY:                                                                                                                                                      |    |          |     |
| SHEET 82 OF 84                                                                                                                                                    |    |          |     |
| 2023-XX-CON                                                                                                                                                       |    |          |     |







