CONTRIBUTING ZONE PLAN

for

SADDLEBACK AT SANTA RITA RANCH PHASE 1 SECTION 3 WILLIAMSON COUNTY, TEXAS

Prepared For:

Mr. Stephen Ashlock Pulte Homes of Texas, LP 9401 Amberglen Blvd., Bldg I, Suite 150 Austin, TX 78729 (512) 532-3355

Prepared By:

Steven P. Cates, P.E. CARLSON, BRIGANCE & DOERING, INC.

5501 West William Cannon Drive Austin, Texas 78749 (512) 280-5160 Firm #F3791



CBD No. 5413 December 2023



CARLSON, BRIGANCE & DOERING, INC.
ID# F3791

12-1-2023

TABLE OF CONTENTS

- I. Edwards Aquifer Application Cover Page (TCEQ-20705)
- II. Contributing Zone Plan Application (TCEQ-10257)
 - Attachment A Road Map
 - > Attachment B USGS Quadrangle Map
 - Attachment C Project Narrative
 - ➤ Attachment D Factors Affecting Surface Water Quality
 - Attachment E Volume and Character of Stormwater
 - ➤ Attachment J BMPs for Upgradient Stormwater
 - ➤ Attachment K BMPs for On-site Stormwater
 - ➤ Attachment L BMPs for Surface Streams Contamination
 - Attachment M Construction Plans
 - ➤ Attachment N Inspection, Maintenance, Repair, and Retrofit Plan
 - ➤ Attachment P Measures for Minimizing Surface Stream Contamination

III. Temporary Stormwater Section (TCEQ-0602)

- ➤ Attachment A Spill Response Actions
- ➤ Attachment B Potential Sources of Contamination
- ➤ Attachment C Sequence of Major Activities
- Attachment D Temporary Best Management Practices and Measures
- ➤ Attachment F Structural Practices
- Attachment G Drainage Area Map
- ➤ Attachment I Inspection and Maintenance for BMPs
- Attachment J Schedule of Interim and Permanent Soil Stabilization Practices

IV. Appendices

- Appendix A BMP TSS Removal Worksheets
- ➤ Appendix B CZP Approval Letter
- Appendix C Water Quality Calculation Spreadsheet
- V. Agent Authorization Form (TCEQ-0599), if application submitted by agent
- VI. Application Fee Form (TCEQ-0574)
- VII. Check Payable to the "Texas Commission on Environmental Quality"
- VIII. Core Data Form (TCEQ-10400)

Texas Commission on Environmental Quality

Edwards Aquifer Application Cover Page

Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with 30 TAC 213.

Administrative Review

- Edwards Aquifer applications must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.
 - To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: http://www.tceq.texas.gov/field/eapp.
- 2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
- 3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
- 4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.
 - An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.
- 5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
- 6. If the geologic assessment was completed before October 1, 2004 and the site contains "possibly sensitive" features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

Technical Review

- When an application is deemed administratively complete, the technical review period begins. The regional
 office will distribute copies of the application to the identified affected city, county, and groundwater
 conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days
 to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
- 2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

- clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.
- 3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited**.
- 4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

Mid-Review Modifications

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a "Mid-Review Modification". Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ's Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ's San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: Saddleback at Santa Rita Ranch Phase 1 Section 3				2. Regulated Entity No.:					
3. Customer Name: Pulte Homes of Texas, LP			4. Customer No.: 603410499						
5. Project Type: (Please circle/check one)	New		Modif	icatior	1	Exte	nsion	Exception	
6. Plan Type: (Please circle/check one)	WPAP	CZP	scs	UST	AST	EX P	EXT	Technical Clarification	Optional Enhanced Measures
7. Land Use: (Please circle/check one)	Residen	tial	Non-r	esiden	tial		8. Sit	e (acres):	37.02
9. Application Fee:	\$4,000.	00	10. P	10. Permanent BM			s):	N/A	
11. SCS (Linear Ft.):	N/A		12. AST/UST (No. 7			o. Tai	ıks):	N/A	
13. County:	Williams	son	14. W	14. Watershed:				Sowes Branch / North Fork San Gabriel River	

Application Distribution

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the "Texas Groundwater Conservation Districts within the EAPP Boundaries" map found at:

http://www.tceq.texas.gov/assets/public/compliance/field_ops/eapp/EAPP%20GWCD%20map.pdf

For more detailed boundaries, please contact the conservation district directly.

Austin Region					
County:	Hays	Travis	Williamson		
Original (1 req.)	_	_	_X_		
Region (1 req.)	_	_	_X_		
County(ies)	_	_	_X_		
Groundwater Conservation District(s)	Edwards Aquifer AuthorityBarton Springs/ Edwards AquiferHays TrinityPlum Creek	Barton Springs/ Edwards Aquifer	NA		
City(ies) Jurisdiction	AustinBudaDripping SpringsKyleMountain CitySan MarcosWimberleyWoodcreek	AustinBee CavePflugervilleRollingwoodRound RockSunset ValleyWest Lake Hills	AustinCedar ParkFlorence _x_GeorgetownJerrellLeander _x_Liberty HillPflugervilleRound Rock		

San Antonio Region					
County:	Bexar	Comal	Kinney	Medina	Uvalde
Original (1 req.)	_	_		_	
Region (1 req.)	_	_	_		
County(ies)	_	_			
Groundwater Conservation District(s)	Edwards Aquifer Authority Trinity-Glen Rose	Edwards Aquifer Authority	Kinney	EAA Medina	EAA Uvalde
City(ies) Jurisdiction	Castle HillsFair Oaks RanchHelotesHill Country VillageHollywood ParkSan Antonio (SAWS)Shavano Park	Bulverde Fair Oaks Ranch Garden Ridge New Braunfels Schertz	NA	San Antonio ETJ (SAWS)	NA

I certify that to the best of my knowledge, that the application is complete and accurate. This application is hereby submitted to TCEQ for administrative review and technical review.				
SRFV Development, LLC / Carlson, Brigance, & Doering, Inc.				
Print Name of Customer/Authorized Agent				
Sturk Cate	12-1-2023			
Signature of Customer/Authorized Agent	Date			

FOR TCEQ INTERNAL USE ONLY				
Date(s)Reviewed:	D	Date Administratively Complete:		
Received From:	C	orrect N	Tumber of Copies:	•
Received By:	D	istribut	ion Date:	
EAPP File Number:	C	omplex	:	
Admin. Review(s) (No.):	N	No. AR Rounds:		
Delinquent Fees (Y/N):	R	Review Time Spent:		
Lat./Long. Verified:	S	SOS Customer Verification:		
Agent Authorization Complete/Notarized (Y/N):	F	ee	Payable to TCEQ (Y	/N):
Core Data Form Complete (Y/N):	-	Check: Signed (Y/N):		
Core Data Form Incomplete Nos.:			Less than 90 days o	ld (Y/N):

Texas Commission on Environmental Quality

for Regulated Activities on the Contributing Zone to the Edwards Aquifer and Relating to 30 TAC §213.24(1), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Contributing Zone Plan Application** is hereby submitted for TCEQ review and Executive Director approval. The application was prepared by:

Print Name of Customer/Agent: Steven P. Cates, P.E.

Date: <u>12/01/2023</u>

Signature of Customer/Agent:

Regulated Entity Name: Saddleback at Santa Rita Ranch Phase 1 Section 3

Project Information

1. County: Williamson

2. Stream Basin: Sowes Branch / North Fork San Gabriel River

3. Groundwater Conservation District (if applicable): N/A

4. Customer (Applicant):

Contact Person: <u>Stephen Ashlock</u> Entity: <u>Pulte Homes of Texas, LP</u>

Mailing Address: 9401 Amberglen Blvd., Bldg. I, Suite 150
City, State: Austin, TX
Telephone: (512) 532-3355
Fax: _____

Email Address: Stephen.Ashlock@PulteGroup.com

5.	Agent/Representative (If any):
	Contact Person: Steven P. Cates, P.E. Entity: Carlson, Brigance & Doering, Inc. Mailing Address: 5501 West William Cannon City, State: Austin, TX Telephone: (512) 280-5160 Email Address: steve@cbdeng.com
6.	Project Location:
	 ☐ The project site is located inside the city limits of ☐ The project site is located outside the city limits but inside the ETJ (extra-territorial jurisdiction) of the City of Liberty Hill. ☐ The project site is not located within any city's limits or ETJ.
7.	The location of the project site is described below. Sufficient detail and clarity has been provided so that the TCEQ's Regional staff can easily locate the project and site boundaries for a field investigation.
	South of Donovan Drive, West of Groveland Drive.
8.	Attachment A - Road Map. A road map showing directions to and the location of the project site is attached. The map clearly shows the boundary of the project site.
9.	Attachment B - USGS Quadrangle Map. A copy of the official 7 ½ minute USGS Quadrangle Map (Scale: 1" = 2000') is attached. The map(s) clearly show:
	☑ Project site boundaries.☑ USGS Quadrangle Name(s).
10	Attachment C - Project Narrative. A detailed narrative description of the proposed project is attached. The project description is consistent throughout the application and contains, at a minimum, the following details:
	 Area of the site ○ Offsite areas ○ Impervious cover ○ Permanent BMP(s) ○ Proposed site use ○ Site history ○ Previous development ○ Area(s) to be demolished
11.	Existing project site conditions are noted below:
	Existing commercial site Existing industrial site Existing residential site

15. The amount and tune of importance cover avacated after a

15. The amount and type of impervious cover expected after construction is complete is shown below:

Table 1 - Impervious Cover

14. Estimated projected population: 462

Impervious Cover of Proposed Project	Sq. Ft.	Sq. Ft./Acre	Acres
Structures/Rooftops	544,936	÷ 43,560 =	12.51
Parking	0	÷ 43,560 =	0
Other paved surfaces	264,845	÷ 43,560 =	6.08
Total Impervious Cover	809,780	÷ 43,560 =	18.59

Total Impervious Cover $18.59 \div$ Total Acreage $37.02 \times 100 = 50.22 \%$ Impervious Cover

- 16. Attachment D Factors Affecting Surface Water Quality. A detailed description of all factors that could affect surface water quality is attached. If applicable, this includes the location and description of any discharge associated with industrial activity other than construction.
- 17. Only inert materials as defined by 30 TAC 330.2 will be used as fill material.

For Road Projects Only

Complete questions 18 - 23 if this application is exclusively for a road project.

\	/ .
\times I	NI/A
/ N	1 1 / / 1

18. I	ype of project:
	 TXDOT road project. County road or roads built to county specifications. City thoroughfare or roads to be dedicated to a municipality. Street or road providing access to private driveways.
19. T	ype of pavement or road surface to be used:
	Concrete Asphaltic concrete pavement Other:
20. R	tight of Way (R.O.W.):
V	ength of R.O.W.: feet. Vidth of R.O.W.: feet. x W = Ft ² ÷ 43,560 Ft ² /Acre = acres.
21. P	Pavement Area:
۷ L	ength of pavement area: feet. Vidth of pavement area: feet. x W = Ft² ÷ 43,560 Ft²/Acre = acres. Pavement area acres ÷ R.O.W. area acres x 100 =% impervious cover.
22.	A rest stop will be included in this project.
	A rest stop will not be included in this project.
23.	Maintenance and repair of existing roadways that do not require approval from the TCEQ Executive Director. Modifications to existing roadways such as widening roads/adding shoulders totaling more than one-half (1/2) the width of one (1) existing lane require prior approval from the TCEQ.
Sto	ormwater to be generated by the Proposed Project
24.	Attachment E - Volume and Character of Stormwater. A detailed description of the volume (quantity) and character (quality) of the stormwater runoff which is expected to occur from the proposed project is attached. The estimates of stormwater runoff quality and quantity are based on area and type of impervious cover. Include the runof coefficient of the site for both pre-construction and post-construction conditions.
Wa	stewater to be generated by the Proposed Project
25 . [Wastewater is to be discharged in the contributing zone. Requirements under 30 TAC §213.6(c) relating to Wastewater Treatment and Disposal Systems have been satisfied. N/A

•	stem, the containm umulative storage ca		ed to capture one an ns.	d one-half (1 1/2)
for providin		nment are propose	ent Methods. Alterr d. Specifications sho	
29. Inside dimensio	ons and capacity of o	containment struct	ure(s):	
	ary Containment			
Length (L)(Ft.)	Width(W)(Ft.)	Height (H)(Ft.)	L x W x H = (Ft3)	Gallons
				otal: Gallons
Some of the structure. The piping v The piping v The piping v The contain substance(s	piping to dispensed will be aboveground will be underground ment area must be) being stored. The	rs or equipment wi constructed of and proposed containr	side the containmen Il extend outside the I in a material imperv ment structure will b	containment vious to the e constructed of:
containmen Interior Internal Tanks cle	t structure is attach dimensions (length,	ned that shows the width, depth and	rings. A scaled drawi following: wall and floor thickn e collection of any sp	ess).
storage tanl			for collection and rec controlled drainage a	
	vent of a spill, any sp 4 hours of the spill a	_	oved from the contai operly.	nment structure

26. Wastewater will be	disposed of by:		
On-Site Sewage	Facility (OSSF/Septic Ta	nk):	
will be used licensing authe land is sthe requirer relating to C Each lot in to size. The sy	to treat and dispose of thority's (authorized age uitable for the use of pri	the wastewater from thent) written approval is a vate sewage facilities are facilities as specified until the facilities as specified until th	attached. It states that and will meet or exceed ander 30 TAC Chapter 285 (43,560 square feet) in engineer or registered
The sewage collecti	on System (Sewer Lines) on system will convey th he treatment facility is:		berty Hill (name)
∑ Existing. ☐ Proposed.			
□ N/A			
Gallons	oveground Stol 7 - 33 if this project inclu to 500 gallons.		-
⊠N/A			
27. Tanks and substanc	e stored:		
Table 2 - Tanks and	Substance Storage		
AST Number	Size (Gallons)	Substance to be Stored	Tank Material
1			
2			
3			
4			
5			
		To	otal x 1.5 = Gallons
	placed within a containr times the storage capac		•

5 of 11

In the event of a spill, any spillage will be drained from the containment structure through a drain and valve within 24 hours of the spill and disposed of properly. The drain and valve system are shown in detail on the scaled drawing.
Site Plan Requirements
Items 34 - 46 must be included on the Site Plan.
34. \boxtimes The Site Plan must have a minimum scale of 1" = 400'.
Site Plan Scale: 1" = <u>40</u> '.
35. 100-year floodplain boundaries:
 Some part(s) of the project site is located within the 100-year floodplain. The floodplain is shown and labeled. No part of the project site is located within the 100-year floodplain. The 100-year floodplain boundaries are based on the following specific (including date of material) sources(s): FEMA- FIRM Panel #48491C0275E; Effective Date: September 26, 2008.
36. The layout of the development is shown with existing and finished contours at appropriate, but not greater than ten-foot contour intervals. Lots, recreation centers, buildings, roads, etc. are shown on the site plan.
The layout of the development is shown with existing contours at appropriate, but not greater than ten-foot contour intervals. Finished topographic contours will not differ from the existing topographic configuration and are not shown. Lots, recreation centers, buildings, roads, etc. are shown on the site plan.
37. A drainage plan showing all paths of drainage from the site to surface streams.
38. \boxtimes The drainage patterns and approximate slopes anticipated after major grading activities.
39. Areas of soil disturbance and areas which will not be disturbed.
40. 🔀 Locations of major structural and nonstructural controls. These are the temporary and permanent best management practices.
41. \(\sum \) Locations where soil stabilization practices are expected to occur.
42. Surface waters (including wetlands). N/A
43. Locations where stormwater discharges to surface water.
There will be no discharges to surface water.
44. Temporary aboveground storage tank facilities.
☐ Temporary aboveground storage tank facilities will not be located on this site.

45. Permanent aboveground storage tank facilities.	
igwedge Permanent aboveground storage tank facilities will not be located on this site.	
46. 🔀 Legal boundaries of the site are shown.	
Permanent Best Management Practices (BMPs)	
Practices and measures that will be used during and after construction is completed.	
47. Permanent BMPs and measures must be implemented to control the discharge of pollution from regulated activities after the completion of construction.	
∐ N/A	
48. These practices and measures have been designed, and will be constructed, operate and maintained to insure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity removed. These quantities have been calculated in accordance with technical guida prepared or accepted by the executive director.	/ is
 The TCEQ Technical Guidance Manual (TGM) was used to design permanent BM and measures for this site. A technical guidance other than the TCEQ TGM was used to design permanent B and measures for this site. The complete citation for the technical guidance that was used is: 	MPs
□ N/A	
49. Owners must insure that permanent BMPs and measures are constructed and funct as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the appropriate regional office within 30 days of site completion.	er
∐ N/A	
50. Where a site is used for low density single-family residential development and has 20 % less impervious cover, other permanent BMPs are not required. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.	the
 □ The site will be used for low density single-family residential development and had 20% or less impervious cover. □ The site will be used for low density single-family residential development but had more than 20% impervious cover. □ The site will not be used for low density single-family residential development. 	

fai im re ind th an	mily residential developments, schools, or small business sites where 20% or less spervious cover is used at the site. This exemption from permanent BMPs must be corded in the county deed records, with a notice that if the percent impervious cover creases above 20% or land use changes, the exemption for the whole site as described in e property boundaries required by 30 TAC §213.4(g) (relating to Application Processing ad Approval), may no longer apply and the property owner must notify the appropriate gional office of these changes.
	 Attachment I - 20% or Less Impervious Cover Waiver. The site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is attached. □ The site will be used for multi-family residential developments, schools, or small business sites but has more than 20% impervious cover. □ The site will not be used for multi-family residential developments, schools, or small business sites.
52. 🔀	Attachment J - BMPs for Upgradient Stormwater.
	 A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is attached. No surface water, groundwater or stormwater originates upgradient from the site and flows across the site, and an explanation is attached. Permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site, and an explanation is attached.
53. 🗵	Attachment K - BMPs for On-site Stormwater.
	 ☑ A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is attached. ☑ Permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, and an explanation is attached.
54. 🔀	Attachment L - BMPs for Surface Streams. A description of the BMPs and measures that prevent pollutants from entering surface streams is attached.
] N/A
55. 🔀	Attachment M - Construction Plans. Construction plans and design calculations for the proposed permanent BMPs and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer, and are signed, sealed, and dated. Construction plans for the proposed permanent BMPs and measures are

	attached and include: Design calculations, TCEQ Construction Notes, all proposed structural plans and specifications, and appropriate details.
	N/A
56. 🔀	Attachment N - Inspection, Maintenance, Repair and Retrofit Plan . A site and BMP specific plan for the inspection, maintenance, repair, and, if necessary, retrofit of the permanent BMPs and measures is attached. The plan fulfills all of the following:
	Prepared and certified by the engineer designing the permanent BMPs and measures
	 Signed by the owner or responsible party ○ Outlines specific procedures for documenting inspections, maintenance, repairs, and, if necessary, retrofit. ○ Contains a discussion of record keeping procedures
	N/A
57.	Attachment O - Pilot-Scale Field Testing Plan . Pilot studies for BMPs that are not recognized by the Executive Director require prior approval from the TCEQ. A plan for pilot-scale field testing is attached.
	N/A
58.	Attachment P - Measures for Minimizing Surface Stream Contamination. A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity, which increase erosion that result in water quality degradation.
	N/A
-	consibility for Maintenance of Permanent BMPs and sures after Construction is Complete.
59.	The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
60. 🔀	A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development,

or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.

Administrative Information

- 61. Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions.
- 62. Any modification of this Contributing Zone Plan may require TCEQ review and Executive Director approval prior to construction, and may require submission of a revised application, with appropriate fees.
- 63. The site description, controls, maintenance, and inspection requirements for the storm water pollution prevention plan (SWPPP) developed under the EPA NPDES general permits for stormwater discharges have been submitted to fulfill paragraphs 30 TAC §213.24(1-5) of the technical report. All requirements of 30 TAC §213.24(1-5) have been met by the SWPPP document.
 - The Temporary Stormwater Section (TCEQ-0602) is included with the application.

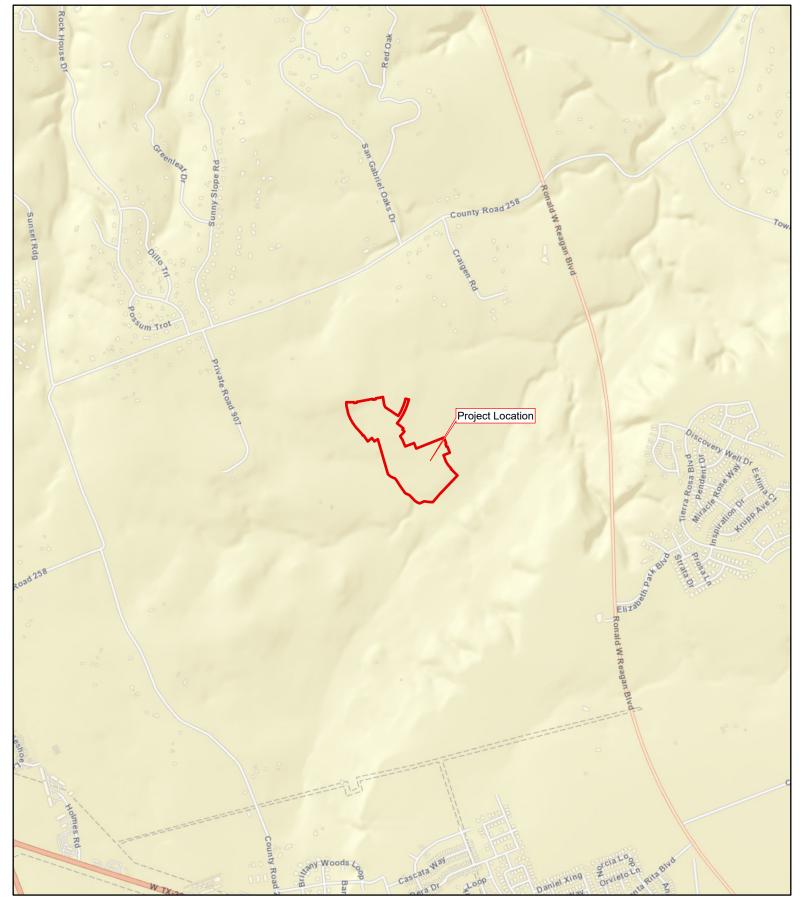
ATTACHMENT A

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

ROAD MAP



Santa Rita Ranch

Water Pollution Abatement Plan Map Leander NE Quadrant



1,000 2,000

0

4,000 Feet



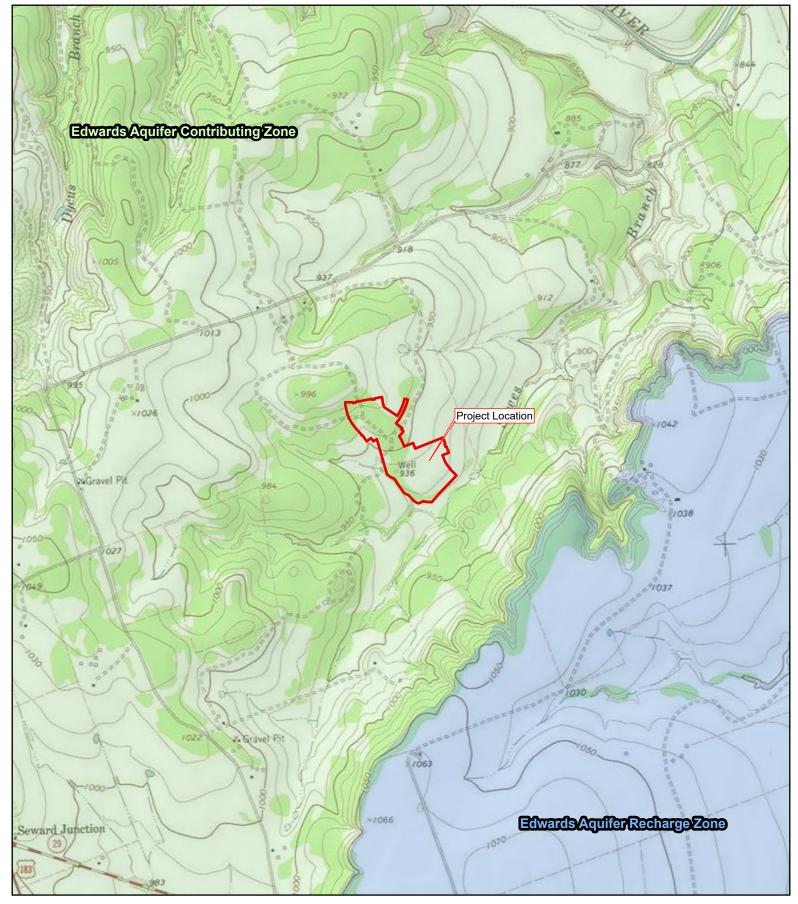
ATTACHMENT B

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

USGS QUADRANGLE MAP



Santa Rita Ranch

Water Pollution Abatement Plan Map Leander NE Quadrant





0 1,000 2,000

4,000 Feet

ATTACHMENT C

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Project Narrative:

Saddleback at Santa Rita Ranch Phase 1 Section 3 is a 37.02-acre residential development that is composed of 154 single-family lots. The project is located on the west side of Ronald Reagan Boulevard, approximately 3 miles north of State Highway 29. The project is located within the City of Liberty Hill ETJ, in Williamson County, Texas. This project includes 6,301 linear feet of roadway, 5,964 linear feet of water main line, and 5,526 linear feet of 8" SDR 26 PVC ASTM D3034 wastewater main line.

The proposed wastewater line will flow into an existing SCS gravity system to the approved Lift Station 2A and then the Liberty Hill Wastewater Treatment Plant.

The site may have soil imported. The fill material shall consist of crushed limestone, select fill, and topsoil. The fill material will be used to facilitate drainage, roadway construction, revegetation of the property, and to elevate the building foundations.

This project is located within the Edwards Aquifer Contributing Zone. Flows were calculated using the National Resource Conservation hydrologic method. Water quality will be provided by one proposed and one existing batch detention ponds.

Within the 37.02-acre improvement area, approximately 18.59 acres of impervious cover will be installed (50.22% of total project site). Batch detention ponds have been designed in accordance with the January 20, 2017 Addendum Sheet to RG-348 which establishes Batch Detention Basins in Section 3.2.17. They have been sized to treat and detain for this and future sections.

ATTACHMENT D

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Factors Affecting Surface Water Quality:

During Construction

The following non-stormwater discharges may occur from the site during the construction period:

- Utility water line flushing during the initial line testing must use uncontaminated water that is not hyperchlorinated.
- Pavement wash waters (where no spills or leaks of toxic or hazardous materials have occurred)
- Groundwater (from dewatering of excavation) must be uncontaminated.
- Water used to wash vehicles or control dust must be accomplished using potable water without detergents.

All non-stormwater discharge will be directed to the temporary Erosion and Sedimentation Controls (Best Management Practices) to remove any suspended solids contained therein. Stormwater during construction will remove loose material and transport it downstream.

Post Construction

The following non-stormwater discharges may occur from the site after construction has been completed:

- Fertilizers and pesticides
- Household chemicals
- Pet Waste
- Used oil
- Car washing
- Mulching
- Sediment

Post-construction stormwater discharges typically transport sediment in the form of dirt and dust accumulated on the streets and other impervious flatwork, rooftops, and sediment from erosion of grassy areas. That material will be transported through the storm sewer system to the wet basins, where most of the pollutants will be removed.

ATTACHMENT E

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Volume and Character of Stormwater:

Existing and developed hydrology models were created in HEC-HMS, v.4.8. A 24-hour frequency storm was applied to the meteorological models for the 2, 10, 25, and 100-year storm events. The model was run over a 24-hour period with a computational time interval of 1 minute. Subbasins utilized an SCS Curve Number Loss Method and SCS Unit Hydrograph Transform Method. Reaches utilized either a Muskingum-Cunge or Lag Routing Method. See below for specific model input data.

Meteorological Model

Frequency storms with the following parameters were used to model storm events:

HEC-HMS Meteorological Model Parameters

120-1 mo meteorological model i arameters		
Probability	2-yr = 50%, 10-yr = 10%, 25-100-yr = Other	
Input Type	Partial Duration	
Output Type	Annual Duration (only applicable for 2-10-yr)	
Intensity Duration	5 Minutes	
Storm Duration	1 Day	
Intensity Position	50%	
Storm Area	(Blank if less than 10 mi ²)	
Curve	Uniform for all subbasins	

Partial-duration precipitation depths are per the Depth-Duration-Frequency Estimates for the San Gabriel River Zone in Williamson County, Texas, according to NOAA Atlas 14, Volume 11, Version 2. See the table below:

Precipitation Depths (in) per Recurrence Interval

recipitation Depths (iii) per recontence interval				
Duration	2-YR	10-YR	25-YR	100-YR
5-min	0.51	0.757	0.921	1.19
15-min	1.02	1.51	1.84	2.37
60-min	1.88	2.79	3.4	4.39
2-hr	2.3	3.55	4.43	5.98
3-hr	2.55	4.02	5.09	7.06
6-hr	2.98	4.81	6.18	8.75
12-hr	3.44	5.54	7.12	10.1
24-hr	3.94	6.3	8.04	11.2

Land Use & Curve Numbers

In existing conditions, the soils are primarily hydrologic soil group D, as per the USDA Web Soil Survey. The soils map and data have been included in Appendix B. The curve numbers were selected from Urban

Hydrology for Small Watersheds¹ based on hydrologic soil groups and aerial maps. See table below. Curve numbers were assessed independently from impervious cover.

Runoff Curve Numbers

Cover Type	Hydrologic Condition	Hydrologic Soil Group	Curve Number
Pasture	Good	D	80
Woods-grass combination	Good	D	79
Meadow	Good	D	78

Existing impervious cover was determined from aerial imagery. Proposed impervious cover was estimated from the proposed and anticipated future layout using TCEQ assumptions for residential tracts. Impervious cover was calculated as a percent of the total drainage basin. Curve number and impervious cover percents were loss inputs for subbasins in the model.

Time of Concentration

All time of concentration calculations were generated using SCS methodology provided in Urban Hydrology for Small Watersheds² for sheet, shallow concentrated, and channel flow. A maximum of 100 feet was used for sheet flow calculations. Lag times were calculated as 60 percent of the time of concentration. Lag times were transform inputs for subbasins and reaches in the model. Times of concentration for future developed drainage areas were approximated based on assumed basin size.

Reaches

Reaches representing the Middle Fork San Gabriel River were modeled using the Muskingum-Cunge routing method with 8-point cross-sections. In developed conditions, reaches contributing to the Middle Fork were modeled with the Lag method.

Reservoirs

All reservoirs were modeled using outflow structures with an elevation-storage method. Initial conditions were elevations set to the bottom of pond elevation for batch detention facilities. The model assumed no tailwater condition. Future batch detention ponds were modeled with a generic stage-storage and outflow spillways assigned to the assumed water quality volume elevation.

¹ Natural Resources Conservation Service, Conservation Engineering Division. 1986. Urban Hydrology for Small Watersheds. Technical Release 55. U.S. Department of Agriculture. https://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/stelprdb1044171.pdf
2 Ibid.

ATTACHMENT J

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Best Management Practices for Upgradient Stormwater:

Upgradient stormwater will travel overland to the curb and gutter and captured by the curb inlets to be conveyed to existing Batch Detention Pond S3 and proposed Batch Detention Pond S4.

ATTACHMENT K

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Best Management Practices for On-site Stormwater:

Stormwater runoff from this section will sheet flow across lawns, be captured in gutters and curb inlets, and piped into existing Batch Detention Pond S3 and proposed Batch Detention Pond S4. The water quality volume provided in these ponds is sufficient to accommodate TSS removal for this and future sections.

TCEQ project and drainage area maps are provided in the included construction plans. TCEQ TSS removal calculations are provided in Appendix A of this application.

ATTACHMENT L

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Best Management Practices for Surface Streams Contamination:

Stormwater runoff in this section will sheet flow across lawns, be captured in gutters and curb inlets, and piped into proposed batch detention ponds as shown on the Overall Drainage Area Plan. Drainage areas G and H will flow to Pond S3. Drainage areas A through F will flow to Pond S4.

The existing and proposed batch detention ponds discharge through rock rip-rap and rock berms which deters heavy floods from entering streams and aids in sediment collection. The remaining site stormwater runoff will sheet flow across the lots and discharge directly into Sowes Branch which feeds the North Fork of the San Gabriel River.

No stormwater from the improved area will drain to sensitive geological features.

ATTACHMENT M

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Construction Plans:

Construction Plans for the erosion and sedimentation control measures proposed with this development are included at the end of this report.

ATTACHMENT N

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Inspection, Maintenance, Repair and Retrofit Plan:

Maintenance Guidelines for Batch Detention Basins

Batch detention basins may have somewhat higher maintenance requirements than an extended detention basin since they are active stormwater controls. The maintenance activities are identical to those of extended detention basins with the addition of maintenance and inspections of the automatic controller and the valve at the outlet.

- Inspections. Inspections should take place a minimum of twice a year. One inspection should take place during wet weather to determine if the basin is meeting the target detention time of 12 hours and a drawdown time of no more than 48 hours. The remaining inspections should occur between storm events so that manual operation of the valve and controller can be verified. The level sensor in the basin should be inspected and any debris or sediment in the area should be removed. The outlet structure and the trash screen should be inspected for signs of clogging. Debris and sediment should be removed from the orifice and outlet(s) as described in previous sections. Debris obstructing the valve should be removed. During each inspection, erosion areas inside and downstream of this BMP should be identified and repaired/revegetated immediately.
- Mowing. The basin, basin side-slopes, and embankment of the basin must be mowed to prevent
 woody growth and control weeds. A mulching mower should be used, or the grass clippings
 should be caught and removed. Mowing should take place at least twice a year, or more
 frequently if vegetation exceeds 18 inches in height. More frequent mowing to maintain aesthetic
 appeal may be necessary in landscaped areas.
- Litter and Debris Removal. Litter and debris removal should take place at least twice a year, as
 part of the periodic mowing operations and inspections. Debris and litter should be removed from
 the surface of the basin. Particular attention should be paid to floatable debris around the outlet
 structure. The outlet should be checked for possible clogging or obstructions and any debris
 removed.
- Erosion control. The basin side slopes and embankment all may periodically suffer from slumping and erosion. To correct these problems, corrective action, such as regrading and revegetation, may be necessary. Correction of erosion control should take place whenever required based on the periodic inspections.
- Nuisance Control. Standing water or soggy conditions may occur in the basin. Some standing
 water may occur after a storm event since the valve may close with 2 to 3 inches of water in the
 basin. Some flow into the basin may also occur between storms due to spring flow and residential
 water use that enters the storm sewer system. Twice a year, the facility should be evaluated in
 terms of nuisance control (insects, weeds, odors, algae, etc.).

- Structural Repairs and Replacement. With each inspection, any damage to structural elements of the basin (pipes, concrete drainage structures, retaining walls, etc.) should be identified and repaired immediately. An example of this type of repair can include patching of cracked concrete, sealing of voids, removal of vegetation from cracks and joints. The various inlet/outlet structures in a basin will eventually deteriorate and must be replaced.
- Sediment Removal. A properly designed batch detention basin will accumulate quantities of sediment over time. The accumulated sediment can detract from the appearance of the facility and reduce the pollutant removal performance of the facility. The sediment also tends to accumulate near the outlet structure and can interfere with the level sensor operation. Sediment shall be removed from the basin at least every 5 years, when sediment depth exceeds 6 inches, when the sediment interferes with the level sensor or when the basin does not drain within 48 hours. Care should be taken not to compromise the basin lining during maintenance.
- Logic Controller. The Logic Controller should be inspected as part of the twice yearly investigations. Verify that the external indicators (active, cycle in progress) are operating properly by turning the controller off and on, and by initiating a cycle by triggering the level sensor in the basin. The valve should be manually opened and closed using the open/close switch to verify valve operation and to assist in inspecting the valve for debris. The solar panel should be inspected and any dust or debris on the panel should be carefully removed. The controller and all other circuitry and wiring should be inspected for signs of corrosion, damage from insects, water leaks, or other damage. At the end of the inspection, the controller should be reset.

Ultimately, these facilities will be owned, operated and maintained by the proposed Williamson County MUD No. 19C. Until the ownership of facilities is transferred to the MUD, SRFV Development, LLC. will be responsible for maintenance of these facilities in accordance with the above stated requirements.

Acknowledged by:

Stephen Ashlock

Pulte Homes of Texas, LP.

BMP DESIGN FIRM INFORMATION

Carlson, Brigance & Doering, Inc. Mr. Steven P. Cates, P.E. Phone: (512) 280-5160 5501 West William Cannon Austin, TX 78749

The above Inspection, Maintenance, Repair, and Retrofit Plan has been prepared by the undersigned Engineer, and I hereby certify that the above Plan conforms with the minimum requirements of the TCEQ

Technical Guidance on Best Management Practices, RG-348.

Steven P. Cates, P.E.

12-1-2023

Date

STEVEN P. CATE

ATTACHMENT P

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Measures for Minimizing Surface Stream Contamination:

The development minimizes surface stream contamination by maintaining the naturally occurring sheet flow across the lots. Drainage from this development will be directed towards proposed batch ponds which will treat the stormwater and reduce the developed flow rate to pre-developed conditions. There are no surface streams within this site. Sowes Branch abuts the southern project boundary.

Temporary Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: <u>Steven P. Cates, P.E.</u>

Date: <u>12/01/2023</u>

Signature of Customer/Agent:

Regulated Entity Name: Saddleback at Santa Rita Ranch Phase 1 Section 3

Project Information

Potential Sources of Contamination

Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.

1.	Fuels for construction equipment and hazardous substances which will be used during construction:
	The following fuels and/or hazardous substances will be stored on the site:
	These fuels and/or hazardous substances will be stored in:
	Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

	 Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year. Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
	$igthered{igwedge}$ Fuels and hazardous substances will not be stored on the site.
2.	Attachment A - Spill Response Actions. A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
3.	Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
4.	Attachment B - Potential Sources of Contamination. A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.
S	equence of Construction
5.	Attachment C - Sequence of Major Activities. A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
	 For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given. For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
6.	Name the receiving water(s) at or near the site which will be disturbed or which will

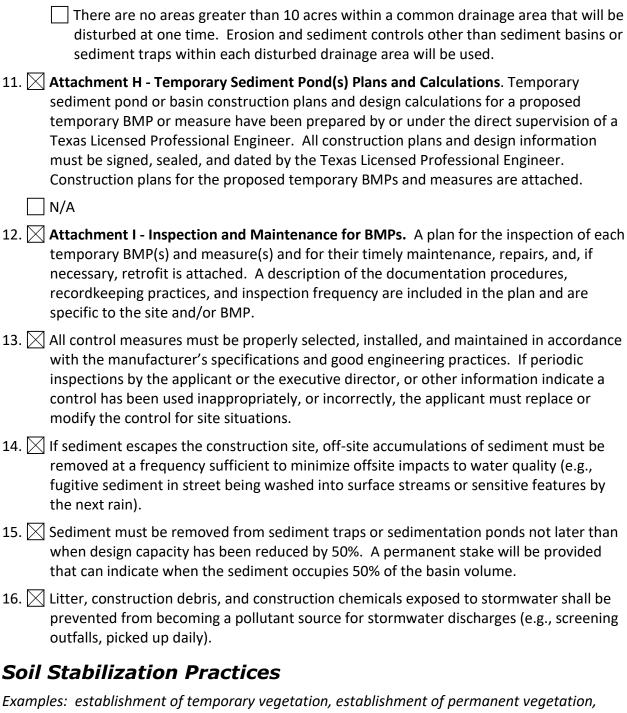
Temporary Best Management Practices (TBMPs)

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

7. Attachment D – Temporary Best Management Practices and Measures. TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

the San Gabriel River

	groundwater or stormwater the across the site. A description of how BMPs and groundwater that originates on contaminated stormwater runce. A description of how BMPs and surface streams, sensitive feature. A description of how, to the man maintain flow to naturally-occur	measures will prevent pollutants from entering
8.		y-occurring sensitive feature which accepts recharge rary pollution abatement measure during active
	seal a feature is attached. The and practicable alternative exis	nporarily Seal a Feature. A request to temporarily request includes justification as to why no reasonable ts for each feature. Iing of naturally-occurring sensitive features on the
9.	used to divert flows away from exp	s. A description of the structural practices that will be losed soils, to store flows, or to otherwise limit runoffed areas of the site is attached. Placement of last been avoided.
10.	10. Attachment G - Drainage Area Ma requirements is attached:	 A drainage area map supporting the following
	disturbed at one time, a sedime For areas that will have more the disturbed at one time, a smalle used. For areas that will have more the disturbed at one time, a sedime attainable, but other TBMPs and down slope and side slope bou There are no areas greater than disturbed at one time. A smalle	nan 10 acres within a common drainage area ent basin will be provided. In an 10 acres within a common drainage area or sediment basin and/or sediment trap(s) will be an 10 acres within a common drainage area ent basin or other equivalent controls are not domeasures will be used in combination to protect and aries of the construction area. In 10 acres within a common drainage area that will be ar sediment basin and/or sediment trap(s) will be a rerosion and sediment controls within each disturbed



mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

17. Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices. A schedule of the interim and permanent soil stabilization practices for the site is attached.

- 18. Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 19. Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

Administrative Information

- 20. All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
- 21. If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
- 22. Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

ATTACHMENT A

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Spill Response Actions:

- 1. Contain the spill.
- 2. Immediately stake off area.
- 3. Notify Hazardous Material team (if necessary); notify TCEQ: (512) 339-2929 or Emergency # 1-800-832-8224
- 4. Take necessary steps to clean up, i.e. notify remediation contractor if large spill, or small spills will be cleaned by the construction contractor.

All site personnel will be made aware of the manufactures' recommended methods for spill cleanup, and the location of the information and cleanup supplies.

Spills will be reported according to the Reportable Quantity, attached on the following page.

Materials and equipment necessary for spill cleanup will be kept onsite in an accessible location known to site personnel.

All Spills will be cleaned up immediately upon discovery. Any spill of hydrocarbons or hazardous substances greater than 25 gallons will require notification to the fire Department Hazardous Materials Team and TCEQ. As with all spills, an effort shall be made to prevent material from entering surface streams and storm drains by using rock or earth berms to contain the material.

Kind of spill	Where discharged	Reportable quantity	Rule, statute, or responsible agency
Hazardous substance	onto land	"Final RQ" in Table 302.4 in 40 CFR 302.4 (PDF)	30 TAC 327 €
	into water	"Final RQ" or 100 lbs, whichever is less	
Any oil	coastal waters	as required by the Texas General Land Office	Texas General Land Office
Crude oil, oil that is neither a petroleum product nor used oil	onto land directly into water	210 gallons (five barrels) enough to create a sheen	30 TAC 327 [™]
Petroleum product, used oil	onto land, from an exempt PST facility onto land, or onto land from	210 gallons (five barrels) 25 gallons	30 TAC 327 €
	a non-exempt PST facility directly into water	enough to create a	
Associated with the exploration, development and production of oil, gas, or geothermal resources	under the jurisdiction of the Railroad Commission of Texas	as required by the Railroad Commission of Texas	Railroad Commission of Texas
Industrial solid waste or other substances	into water	100 lbs	30 TAC 327 ₺
From petroleum storage tanks, underground or aboveground	into water	enough to create a sheen on water	30 TAC 334.75-81 ☑
From petroleum storage tanks, underground or aboveground	onto land	25 gallons or equal to the RQ under 40 CFR	30 TAC 327 [™]
Other substances that may be useful or valuable and are not ordinarily considered to be waste, but will cause pollution if discharged into water in the state	into water	100 lbs	<u>30 TAC 327</u> ፟፟

1.4.16 Spill Prevention and Control

The objective of this section is to describe measures to prevent or reduce the discharge of pollutants to drainage systems or watercourses from leaks and spills by reducing the chance for spills, stopping the source of spills, containing and cleaning up spills, properly disposing of spill materials, and training employees.

The following steps will help reduce the stormwater impacts of leaks and spills:

Education

- (1) Be aware that different materials pollute in different amounts. Make sure that each employee knows what a "significant spill" is for each material they use, and what is the appropriate response for "significant" and "insignificant" spills. Employees should also be aware of when spill must be reported to the TCEQ. Information available in 30 TAC 327.4 and 40 CFR 302.4.
- (2) Educate employees and subcontractors on potential dangers to humans and the environment from spills and leaks.
- (3) Hold regular meetings to discuss and reinforce appropriate disposal procedures (incorporate into regular safety meetings).
- (4) Establish a continuing education program to indoctrinate new employees.
- (5) Have contractor's superintendent or representative oversee and enforce proper spill prevention and control measures.

General Measures

- (1) To the extent that the work can be accomplished safely, spills of oil, petroleum products, substances listed under 40 CFR parts 110,117, and 302, and sanitary and septic wastes should be contained and cleaned up immediately.
- (2) Store hazardous materials and wastes in covered containers and protect from vandalism.
- (3) Place a stockpile of spill cleanup materials where it will be readily accessible.
- (4) Train employees in spill prevention and cleanup.
- (5) Designate responsible individuals to oversee and enforce control measures.
- (6) Spills should be covered and protected from stormwater runon during rainfall to the extent that it doesn't compromise clean up activities.
- (7) Do not bury or wash spills with water.
- (8) Store and dispose of used clean up materials, contaminated materials, and recovered spill material that is no longer suitable for the intended purpose in conformance with the provisions in applicable BMPs.
- (9) Do not allow water used for cleaning and decontamination to enter storm drains or watercourses. Collect and dispose of contaminated water in accordance with applicable regulations.
- (10)Contain water overflow or minor water spillage and do not allow it to discharge into drainage facilities or watercourses.
- (11)Place Material Safety Data Sheets (MSDS), as well as proper storage, cleanup, and spill reporting instructions for hazardous materials stored or used on the project site in an open, conspicuous, and accessible location.
- (12)Keep waste storage areas clean, well organized, and equipped with ample cleanup supplies as appropriate for the materials being stored. Perimeter controls, containment structures, covers, and liners should be repaired or replaced as needed to maintain proper function.

Cleanup

- (1) Clean up leaks and spills immediately.
- (2) Use a rag for small spills on paved surfaces, a damp mop for general cleanup, and absorbent material for larger spills. If the spilled material is hazardous, then the used cleanup materials are also hazardous and must be disposed of as hazardous waste.
- (3) Never hose down or bury dry material spills. Clean up as much of the material as possible and dispose of properly. See the waste management BMPs in this section for specific information.

Minor Spills

- (1) Minor spills typically involve small quantities of oil, gasoline, paint, etc. which can be controlled by the first responder at the discovery of the spill.
- (2) Use absorbent materials on small spills rather than hosing down or burying the spill.
- (3) Absorbent materials should be promptly removed and disposed of properly.
- (4) Follow the practice below for a minor spill:
- (5) Contain the spread of the spill.
- (6) Recover spilled materials.
- (7) Clean the contaminated area and properly dispose of contaminated materials.

Semi-Significant Spills

Semi-significant spills still can be controlled by the first responder along with the aid of other personnel such as laborers and the foreman, etc. This response may require the cessation of all other activities.

Spills should be cleaned up immediately:

- (1) Contain spread of the spill.
- (2) Notify the project foreman immediately
- (3) If the spill occurs on paved or impermeable surfaces, clean up using "dry" methods (absorbent materials, cat litter and/or rags). Contain the spill by encircling with absorbent materials and do not let the spill spread widely.
- (4) If the spill occurs in dirt areas, immediately contain the spill by constructing an earthen dike. Dig up and properly dispose of contaminated soil.
- (5) If the spill occurs during rain, cover spill with tarps or other material to prevent contaminating runoff.

Significant/Hazardous Spills

For significant or hazardous spills that are in reportable quantities:

- (1) Notify the TCEQ by telephone as soon as possible and within 24 hours at 512- 339-2929 (Austin) or 210-490-3096 (San Antonio) between 8 AM and 5 PM. After hours, contact the Environmental Release Hotline at 1-800-832-8224. It is the contractor's responsibility to have all emergency phone numbers at the construction site.
- (2) For spills of federal reportable quantities, in conformance with the requirements in 40 CFR parts 110,119, and 302, the contractor should notify the National Response Center at (800) 424-8802.
- (3) Notification should first be made by telephone and followed up with a written report.
- (4) The services of a spills contractor or a Haz-Mat team should be obtained immediately. Construction personnel should not attempt to clean up until the appropriate and qualified staffs have arrived at the job site.
- (5) Other agencies which may need to be consulted include, but are not limited to, the City Police Department, County Sheriff Office, Fire Departments, etc.

More information on spill rules and appropriate responses is available on the TCEQ website at: https://www.tceq.texas.gov/response/spills

Vehicle and Equipment Maintenance

- (1) If maintenance must occur onsite, use a designated area and a secondary containment, located away from drainage courses, to prevent the run on of stormwater and the runoff of spills.
- (2) Regularly inspect onsite vehicles and equipment for leaks and repair immediately
- (3) Check incoming vehicles and equipment (including delivery trucks, and employee and subcontractor vehicles) for leaking oil and fluids. Do not allow leaking vehicles or equipment onsite.
- (4) Always use secondary containment, such as a drain pan or drop cloth, to catch spills or leaks when removing or changing fluids.
- (5) Place drip pans or absorbent materials under paving equipment when not in use.
- (6) Use absorbent materials on small spills rather than hosing down or burying the spill. Remove the absorbent materials promptly and dispose of properly.
- (7) Promptly transfer used fluids to the proper waste or recycling drums. Don't leave full drip pans or other open containers lying around.
- (8) Oil filters disposed of in trashcans or dumpsters can leak oil and pollute stormwater. Place the oil filter in a funnel over a waste oil-recycling drum to drain excess oil before disposal. Oil filters can also be recycled. Ask the oil supplier or recycler about recycling oil filters.
- (9) Store cracked batteries in a non-leaking secondary container. Do this with all cracked batteries even if you think all the acid has drained out. If you drop a battery, treat it as if it is cracked. Put it into the containment area until you are sure it is not leaking.

Vehicle and Equipment Fueling

- (1) If fueling must occur on site, use designated areas, located away from drainage courses, to prevent the runon of stormwater and the runoff of spills.
- (2) Discourage "topping off" of fuel tanks.
- (3) Always use secondary containment, such as a drain pan, when fueling to catch spills/ leaks.

ATTACHMENT B

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Potential Sources of Contamination:

Gasoline, Diesel, and Hydraulic Fluid from construction equipment, Asphalt products,
Construction Materials,
Trash and Debris,
Paint,
Concrete,
Gypsum from sheet rock,
Sediment.

All materials shall be hauled in a manner consistent with the manufacturer's recommendations. Disposal of waste material shall be in conformance with all state and local laws

ATTACHMENT C

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Sequence of Major Activities:

- 1. Install and maintain Erosion Control and Tree Protection per the Approved Plans and specifications prior to any clearing and grubbing, grading, excavating, etc... Notify Construction Inspection Division when installed.
- 2. Prior to beginning construction, the owner or his representative shall hold a Pre-Construction Conference between TCEQ, Williamson County, Contractor, and any other affected parties. Notify TCEQ at least 48 hours prior to the time of the conference and 48 hours prior to beginning construction. Prior to Pre-Construction Conference.
- 3. Hold Pre-Construction Conference with contractor, TCEQ, EV Inspector, Engineer, and owner or his representative.
- Rough grade roadway.
 (Estimate of disturbed area = 4.92 ac)
- 5. Begin installation of storm sewer. Upon completion, restore as much disturbed areas as possible, particularly channels and large open areas. (Estimate of disturbed area = 0.00 ac)
- 6. Regrade streets to subgrade (Estimate of disturbed area = 4.92 ac)
- 7. Ensure that all underground utility crossings are completed. Lay first course base material on all streets. (4.92 ac)
- 8. Install curb and gutter. (Estimate of disturbed area = 0.58 ac)
- 9. Place concrete for common area 4' sidewalk. (Estimate of disturbed area = 1.16 ac)
- 10. Lay final base course on all streets. (4.92 ac)

- 11. Lot grading. (Estimate of disturbed area = 26.83 ac)
- 12. Lay asphalt. (4.34 ac)
- 13. Clean site and revegetate all disturbed area according to the plans and specifications. Stabilization measures should include seeding and/or mulching.
- 14. Complete permanent erosion control and restoration of site vegetation.
- 15. Project Engineer to provide a written concurrence letter, and scheduling final inspection with EV Inspector, prior to the removal of erosion controls.
- 16. Remove and dispose of temporary erosion/sedimentation control measures.
- 17. Complete any necessary final dress up of areas disturbed by Item 16.
- 18. Conduct a final inspection and complete all punch list items.

Clearing and grubbing under a development permit, solely for the purpose of surveying and soil exploration, shall be a hand-cutting or blade-up operation.

ATTACHMENT D

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Temporary Best Management Practices and Measures:

Install temporary erosion control measures, stabilized construction entrance, concrete washout area, inlet protection, and tree protection according to the plans and specifications prior to any clearing and grubbing, grading, excavating, etc. Upgradient stormwaters during construction crossing disturbed areas will be filtered utilizing standard Best Management Practices, such as erosion logs and silt fences, prior to leaving the site. The silt fences will be placed along down gradient areas of the site to prevent any sediment from entering storm sewers or surface streams.

Geological features on this site are located outside of the Limits of Construction and no stormwater from the disturbed areas will drain to the geological features.

ATTACHMENT F

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Structural Practices:

There are areas 10 acres or greater being disturbed at one time; therefore, a temporary sediment basin will be provided. Additional temporary erosion and sedimentation control will be done by silt fence and rock berms.

ATTACHMENT G

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Drainage Area Map:

An overall drainage area map is included within the plan set submitted with this application.

ATTACHMENT I

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Inspection and Maintenance for Best Management Practices:

The Best Management Practices installed during construction will be maintained in accordance with the requirements of the EPA's NPDES/TPDES storm water pollution prevention program (SWPPP). The following maintenance procedures shall be followed until permanent stabilization is complete.

Silt Fence

- Inspect weekly or after each rainfall event and repair or replacement shall be made promptly as needed.
- b. Silt Fence shall be removed when the site is completely stabilized so as to not block or impede storm flow or drainage.
- c. Accumulated silt shall be removed when it reaches a depth of 6 inches. The Silt shall be disposed of on an approved site and in such a manner that will not contribute to additional siltation.

Fiber Rolls

- a. Inspect prior to forecast rain, daily during extended rain events, after rain events, and weekly.
- b. Repair of replace split, torn, unraveling, or slumping fiber rolls.
- c. If the fiber roll is used as a sediment capture device, or as an erosion control device to maintain sheet flows, sediment that accumulates behind the role shall be periodically removed tin order to maintain its effectiveness. Sediment shall be removed when the accumulation reaches one-half the designated sediment storage depth, usually one-half the distance between the top of the fiber roll and the adjacent ground surface. Sediment removed during maintenance may be incorporated into earthwork on the site or disposed of at an appropriate location.

Stabilized Construction Entrance

- a. The entrance shall be maintained in a condition that will prevent tracking or flowing of sediment onto a public roadway. This may require periodic top dressing with additional stone as conditions demand, as well as repair and clean out of any devices used to trap sediment.
- b. Entrance must be properly graded to incorporate a drain swale or similar measure to prevent runoff from leaving the construction site.

Inlet Protection

- a. Inspection shall be made weekly or after each rainfall event and replacement or repair shall be made promptly as needed.
- b. Accumulated silt shall be removed when it reaches a depth of 6 inches. The Silt shall be disposed of on an approved site and in such a manner that will not contribute to additional siltation
- c. The dyke shall be removed when the site is completely stabilized so as to not block or impede storm flow or drainage.

Temporary Sediment Basins

- a. Inspection shall be made weekly or after each rainfall event. Check the embankment spillways, and outlet for erosion damage, and inspect the embankment for piping and settlement. Repair should be made promptly as needed.
- Trash and other debris should be removed after each rainfall event to prevent clogging of the outlet structure.
- c. Accumulated silt should be removed and the basin should be regraded to its original dimensions at such point that the capacity of the impoundment has been reduced to 75% of its storage capacity.
- The removed sediment should be stockpiled or redistributed in areas that are protected from erosion.

Concrete Washout

- a. Inspection shall be made daily or after each rainfall event to check for leaks, identify any plastic linings and sidewalls which have been damaged by construction activities.
- b. When the washout container is filled over 75 % of its capacity, the washwater should be vacuumed off or allowed to evaporate to avoid overflows. When the remaining cementitious solids have hardened, they should be removed and recycled.
- c. Damages to the container should be repaired promptly and as needed.
- d. Before heavy rains, the washout containers liquid level should be lowered or the container should be covered to avoid an overflow during the rain event.

The owner shall hire an E&S compliance company to inspect E&S measures and keep reports of onsite inspections with deficiencies and solutions.

ATTACHMENT J

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Schedule of Interim and Permanent Soil Stabilization Practices:

Soil Stabilization for all disturbed areas shall be accomplished by hydraulic planting. Following is an outline to accomplish the required stabilization.

- 1. Preparing Seed Bed. After the designated areas have been rough graded to the lines, grades and typical sections indicated in the Drawings or as provided for in other items of this contract and for any other soil area disturbed by the construction, a suitable seedbed shall be prepared. The seedbed shall consist of a minimum of either 4 inches (100 millimeters) of approved topsoil or 4 inches (100 millimeters) of approved salvaged topsoil, cultivated and rolled sufficiently to enhance the soil to a state of good health, when the soil particles on the surface are small enough and lie closely enough together to prevent the seed from being covered too deeply for optimum germination. The optimum depth for seeding shall be 114 inch (6 millimeters). Water shall be gently applied as required to prepare the seedbed prior to the planting operation either by broadcast seeding or hydraulic planting. Bare soils should be seeded or otherwise stabilized within 14 calendar days after final grading or where construction activity has temporarily ceased for more than 21 days. Seeding shall be performed in accordance with the requirements hereinafter described.
- 2. Watering. All watering shall comply with Chisholm Trail Subdivision Rules and Regulations. Broadcast seeded areas shall immediately be watered with a minimum of 5 gallons of water per square yard (22.5 liters of water per square meter) or as needed and in the marmer and quantity as directed by the Engineer or designated representative. Hydraulic seeded areas and native grass seeded areas shall be watered commencing after the tackifier has dried with a minimum of 5 gallons of water per square yard (22.5 liters of water per square meter) or as needed to keep the seedbed in a wet condition favorable for the growth of grass. Watering applications shall constantly maintain the seedbed in a wet condition favorable for the growth of grass. Watering shall continue until the grass is uniformly 1 1/2 inches (40 mm) in height and accepted by the Engineer or designated representative. Watering can be postponed immediately after a 1/2 inch (12.5 mm) or greater rainfall on the site but shall be resumed before the soil dries out.
- 3. Hydraulic Planting. The seedbed shall be prepared as specified above and hydraulic planting equipment, which is capable of placing all materials in a single operation, shall be used.

March 1 to September 15

Hydraulic planting mixture and minimum rate of application pounds per 1000 square feet (kilograms per 100 square meters):

Planting Mixture							
Hulled Bermuda Seed	Fiber Mulch	Fiber Mulch					
(PLS=0.83)	Cellulose	Wood	Tackifier				
	45.9 Lbs/1000 ft2		1.4 lbs/1000 ft2				
1 lbs/1000 ft2	(22.5 kgs/100m2)		(0.7 kgs/100 m2)				
(0.5 kgs/100 m2)		57.4 lbs/1000 ft2	1.5 lbs/1000 ft2				
,		(28.01 kgs/100 m2)	(0.75 kgs/100 m2)				

September 15 to March 1

Add 1.5 pounds per 1000 square feet (0.75 kgs/ 100 m@) of cool season cover crop (see table 1) to above mixture. The fertilizer shall conform to City of Austin Standard Specification Item No. 606S, "Fertilizer".

Table 1 : Cool Season Cover Crop								
Common Name	Botanical Name	Applicati	Application rates					
Common Name	Dotanical Name	Lbs/1000 feet ²	Kg/ 100 meter ²					
Wheat	Triticum aestivum	0.5	0.25					
Oats	Avena sativa	0.5	0.25					
Cereal Rye Grain	Secale cereal	0.5	0.25					
Total Cool Season Cover		1.5	0.75					
Crop Seeding Rate		1.5	0.70					
Total Cool Season Seeding			2.25					
Rate (Grass Wildflowers, &		4.5	2.20					
Cover Crop)			ļ					

Appendix A

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

BMP TSS Removal Worksheet

Saddleback Phase 1 Section 3

TSS Removal Calculations 04-20-2009

Project Name: SADDLEBACK AT SANTA RITA PHASE 1, SECTION 3

Date Prepared: 12/01/2023

Additional information is provided for cells with a red triangle in the upper right corner. Place the cursor over the cell.

Text shown in blue indicate location of instructions in the Technical Guidance Manual - RG-348.

Characters shown in red are data entry fields.

Characters shown in black (Bold) are calculated fields. Changes to these fields will remove the equations used in the spreadsheet.

1. The Required Load Reduction for the total project:

Calculations from RG-348 Pages 3-27 to 3-30

Page 3-29 Equation 3.3: $L_{M} = 27.2(A_{N} \times P)$

where:

 $L_{M \, TOTAL \, PROJECT}$ = Required TSS removal resulting from the proposed development = 80% of increased load A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson

* The values entered in these fields should be for the total project area.

SADDLEBACK ATSANTA RITA RANCH PHASE 1, SECTION 3

Total project area included in plan * =	37.02	acres
Predevelopment impervious area within the limits of the plan * =	0.00	acres
Total post-development impervious area within the limits of the plan* =	18.59	acres
Total post-development impervious cover fraction * =	0.50	1
P =	32	inches
L _{M TOTAL PROJECT} =	16181	lbs.

EXISTING BATCH DETENTION POND S3

acres	25.97	Total project area included in plan *=
acres	0.00	Predevelopment impervious area within the limits of the plan * =
acres	10.29	Total post-development impervious area within the limits of the plan* =
	0.40	Total post-development impervious cover fraction * =
inches	32	P =
lbs.	8956	L _{M TOTAL PROJECT} =

SADDLEBACK AT SANTA RITA RANCH PHASE 1, SECTION 2

47.12 acres	Total project area included in plan *=
0.00 acres	Predevelopment impervious area within the limits of the plan * =
23.85 acres	Total post-development impervious area within the limits of the plan* =
0.51	Total post-development impervious cover fraction * =
32 inche	P =
20759 lbs	LM TOTAL BROJECT =

SADDLEBACK AT SANTA RITA RANCH PHASE 1, SECTION 3

acres	5.93	Total project area included in plan *=
acres	0.00	Predevelopment impervious area within the limits of the plan * =
acres	4.11	Total post-development impervious area within the limits of the plan* =
	0.69	Total post-development impervious cover fraction * =
inches	32	P =
lbs	3577	L _{M TOTAL PROJECT} =

SADDLEBACK (FUTURE)

GADDEEDAGK (1 0 1 GKL)	L		
Total project area included in plan * =		8.85	acres
Predevelopment impervious area within the limits of the plan * =	:	0.00	acres
Total post-development impervious area within the limits of the plan* =	:	2.96	acres
Total post-development impervious cover fraction * =	:	0.33	
P =		32	inches
L _{M TOTAL PROJECT} =	: [2576	lbs.
L _{M TOTAL} =	:	35869	lbs.

PROPOSED BATCH DETENTION POND S4

SADDLEBACK AT SANTA RITA RANCH PHASE 1, SECTION 2

Total project area included in plan * =	0.67	acres
Predevelopment impervious area within the limits of the plan * =	0.00	acres
Total post-development impervious area within the limits of the plan* =	0.33	acres

Total post-development impervious cover fraction * =	0.49]
P =	32	inches
L _{M TOTAL PROJECT} =	287	lbs.
SADDLEBACK AT SANTA RITA RANCH PHASE 1, SECTION 3		
Total project area included in plan * =	29.96	acres
Predevelopment impervious area within the limits of the plan * =	0.00	acres
Total post-development impervious area within the limits of the plan* =	14.74	acres
Total post-development impervious cover fraction * =	0.49	
P =	32	inches
L _{M TOTAL PROJECT} =	12830	lbs.
SADDLEBACK AT SANTA RITA RANCH PHASE 2, SECTION 1		_
Total project area included in plan * =	6.08	acres
Predevelopment impervious area within the limits of the plan * =	0.00	acres
Total post-development impervious area within the limits of the plan* =	3.51	acres
Total post-development impervious cover fraction * =	0.58	
P =	32	inches
L _{M TOTAL PROJECT} =	3055	lbs.
CARRIERACIO AT CANTA RITA RANCH RUAGE A OFOTIONIA		
SADDLEBACK AT SANTA RITA RANCH PHASE 2, SECTION 3	1.67	acres
Total project area included in plan * = Predevelopment impervious area within the limits of the plan * =	0.00	acres
Total post-development impervious area within the limits of the plan* =	0.78	acres
Total post-development impervious cover fraction * =	0.78	acies
P =	32	inches
	679	lbs.
L _M TOTAL PROJECT =	019	ine.
L _{M TOTAL} =	16851	lbs.



CARLSON, BRIGANCE & DOERING, INC. D# F3791

12-1-2023

Appendix A

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

BMP TSS Removal Worksheet

Drainage Basins S3 & S4

(OVERALL)

TSS Removal Calculations 04-20-2009

Project Name: BATCH DETENTION POND S3

Date Prepared: 12/01/2023

Additional information is provided for cells with a red triangle in the upper right corner. Place the cursor over the cell. Text shown in blue indicate location of instructions in the Technical Guidance Manual - RG-348.

Characters shown in red are data entry fields.

Characters shown in black (Bold) are calculated fields. Changes to these fields will remove the equations used in the spreadsheet.

1. The Required Load Reduction for the total project:

where

Calculations from RG-348

Pages 3-27 to 3-30

Page 3-29 Equation 3.3: L_M = 27.2(A_N x P)

 $L_{\text{M TOTAL PROJECT}}$ = Required TSS removal resulting from the proposed development = 80% of increased load A_{N} = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson
Total project area included in plan * = Predevelopment impervious area within the limits of the plan * = 0.00 acres
Total post-development impervious cover fraction * = 0.47
Total post-development impervious area within the limits of the plan * = 0.00
Total post-development impervious area within the limits of the plan * = 0.00
Total post-development impervious area within the limits of the plan * = 0.00
Total post-development impervious area within the limits of the plan * = 0.00
Total post-development impervious area within the limits of the plan * = 0.00
Total post-development impervious area within the limits of the plan * = 0.47
Total post-development impervious area within the limits of the plan * = 0.47
Total post-development impervious cover fraction * = 0.47
Total post-development impervi

L_{M TOTAL PROJECT} = 35869 lbs

Number of drainage basins / outfalls areas leaving the plan area =

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. =

Total drainage basin/outfall area = 83.96 acres
Predevelopment impervious area within drainage basin/outfall area = 0.00 acres
Post-development impervious area within drainage basin/outfall area = 40.87 acres
Post-development impervious fraction within drainage basin/outfall area = 0.49

LM THIS RASIN = 35573 | bs.

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Batch Detention

Removal efficiency = 91 percent

SOURCE:

"COMPLYING WITH THE EDWARDS AQUIFER RULES TECHNICAL
GUIDANCE ON BEST MANAGEMENT PRACTICES RG-348 (REVISED JULY
2005)", ADDENDUM DATED JANUARY 20, 2017, SECTION 3.2.17 'BATCH

Aqualogic Cartridge Filter Batch Detention Bioretention Contech StormFilter Constructed Wetland Extended Detention Grassy Swale Retention / Irrigation Sand Filter Stormceptor Vegetated Filter Strips Vortechs Wet Basin Wet Vault

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

RG-348 Page 3-33 Equation 3.7: L_R = (BMP efficiency) x P x (A_I x 34.6 + A_P x 0.54)

where

 A_C = Total On-Site drainage area in the BMP catchment area A_I = Impervious area proposed in the BMP catchment area A_P = Pervious area remaining in the BMP catchment area

 $L_{\rm R}$ = TSS Load removed from this catchment area by the proposed BMP

 $A_C =$ 83.96 acres $A_I =$ 40.87 acres $A_P =$ 43.09 acres $A_R =$ 41856 lbs

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired L_{M THIS BASIN} = 35869 lbs

F = **0.86**

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

r this drainage basin / outfall area. Calculations from RG-348

Rainfall Depth = 1.38 inches
Post Development Runoff Coefficient = 0.35
On-site Water Quality Volume = 147346 cubic feet

Calculations from RG-348 Pages 3-36 to 3-37

Off-site area draining to BMP = 0.00 acres
Off-site Impervious cover draining to BMP = 0.00 acres
Impervious fraction of off-site area = 0
Off-site Runoff Coefficient = 0.00
Off-site Water Quality Volume = 0 cubic feet

Storage for Sediment = 29469

Total Capture Volume (required water quality volume(s) x 1.20) = 176815 cubic feet

STEVEN P. CATES

93648

93648

OCCENSED

ONAL EXILABITE

PAGE 3-34 to 3-36

CARLSON, BRIGANCE & DOERING, INC.

ID# F3791

12-1-2023

^{*} The values entered in these fields should be for the total project area.

TSS Removal Calculations 04-20-2009

Project Name: BATCH DETENTION POND S3

Date Prepared: 12/01/2023

Additional information is provided for cells with a red triangle in the upper right corner. Place the cursor over the cell. Text shown in blue indicate location of instructions in the Technical Guidance Manual - RG-348.

Characters shown in red are data entry fields.

Characters shown in black (Bold) are calculated fields. Changes to these fields will remove the equations used in the spreadsheet.

1. The Required Load Reduction for the total project:

Calculations from RG-348

Pages 3-27 to 3-30

Page 3-29 Equation 3.3: L_M = 27.2(A_N x P)

where: $L_{M \, TOTAL \, PROJECT}$ = Required TSS removal resulting from the proposed development = 80% of increased load A_N = Net increase in impervious area for the project

P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson

Total project area included in plan * = 38.38 acres

Predevelopment impervious area within the limits of the plan * = 0.00 acres

Total post-development impervious cover fraction * = 19.36 acres

Total post-development impervious cover fraction * = 0.50

P = 32 inches

L_{M TOTAL PROJECT} = 16851 lbs.

Number of drainage basins / outfalls areas leaving the plan area =

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. =

Total drainage basin/outfall area = 33.30 acres
Predevelopment impervious area within drainage basin/outfall area = 0.00 acres
Post-development impervious area within drainage basin/outfall area = 19.12 acres
Post-development impervious fraction within drainage basin/outfall area = 0.57

LM THIS RARNIN = 16642 lbs.

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = Batch Detention
Removal efficiency = 91 percent

SOURCE:

"COMPLYING WITH THE EDWARDS AQUIFER RULES TECHNICAL
GUIDANCE ON BEST MANAGEMENT PRACTICES RG-348 (REVISED JULY
2005)", ADDENDUM DATED JANUARY 20, 2017, SECTION 3.2.17 'BATCH

Aqualogic Cartridge Filter Batch Detention Bioretention Contech StormFilter Constructed Wetland Extended Detention Grassy Swale Retention / Irrigation Sand Filter Stormceptor Vegetated Filter Strips Vortechs Wet Basin Wet Vault

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

RG-348 Page 3-33 Equation 3.7: L_R = (BMP efficiency) x P x (A_I x 34.6 + A_P x 0.54)

where

 A_C = Total On-Site drainage area in the BMP catchment area A_I = Impervious area proposed in the BMP catchment area A_P = Pervious area remaining in the BMP catchment area

 L_{R} = TSS Load removed from this catchment area by the proposed BMP

Calculations from RG-348

 $\begin{array}{llll} A_C = & {\bf 33.30} & \text{acres} \\ A_I = & {\bf 19.12} & \text{acres} \\ A_P = & {\bf 14.18} & \text{acres} \\ L_R = & {\bf 19487} & \text{lbs} \end{array}$

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired L_{M THIS BASIN} = 16851 lbs

F = **0.86**

6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.

Rainfall Depth = 1.38 inches

Post Development Runoff Coefficient = 0.40

On-site Water Quality Volume = 67118 cubic feet

Calculations from RG-348 Pages 3-36 to 3-37

Off-site area draining to BMP = 0.00 acres
Off-site Impervious cover draining to BMP = 0.00 acres
Impervious fraction of off-site area = 0
Off-site Runoff Coefficient = 0.00
Off-site Water Quality Volume = 0 cubic feet

Storage for Sediment = 13424

Total Capture Volume (required water quality volume(s) x 1.20) = 80541 cubic feet

Pages 3-34 to 3-36



CARLSON, BRIGANCE & DOERING, INC.

ID# F3791

12-1-2023

^{*} The values entered in these fields should be for the total project area.

Appendix B

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

Water Quality Calculation
Spreadsheet

SADDLEBACK AT SANTA RITA RANCH PHASE 1 SECTION 3

Table 1 - Impervious Cover per Section

			TCEQ Proje	ect Area Per Se	ction		Onsite Drainage Basin to BMP Per Section					TSS	
Contributing Sections	Project Area			Impervi	ious Areas (ac)		Drainage			Imperviou	Impervious Areas (ac)		Removal Required
	(ac)	# Lots	Lots	ROW	Misc.	Total	Basin (ac)	# Lots	Lots	ROW	Misc.	Total	(lbs)
EXISTING BATCH DETENTION	N POND S3						•		ı	•	•	•	
SADDLEBACK 1-1	25.97	91	7.31	2.98	0.00	10.29	23.44	91	7.31	2.98	0.00	10.29	8,956
SADDLEBACK 1-2	47.12	191	15.60	8.25	0.00	23.85	45.74	191	15.60	8.25	0.00	23.85	20,759
SADDLEBACK 1-3	5.93	38	3.06	1.04	0.00	4.10	5.93	34	2.73	1.04	0.00	3.77	3,577
FUTURE SADDLEBACK	8.85	13	1.15	0.31	1.50	2.96	8.85	13	1.15	0.31	1.50	2.96	2,576
EXISTING BATCH DETENTION	N POND S4												
SADDLEBACK 1-2	0.67	4	0.33	0.00	0.00	0.33	0.67	4	0.33	0.00	0.00	0.33	287
SADDLEBACK 1-3	29.96	115	9.38	5.36	0.00	14.74	24.88	112	9.14	5.36	0.00	14.50	12,830
SADDLEBACK 2-1	6.08	28	2.25	1.26	0.00	3.51	6.08	28	2.25	1.26	0.00	3.51	3,055
FUTURE SADDLEBACK	1.67	7	0.57	0.21	0.00	0.78	1.67	7	0.57	0.21	0.00	0.78	679
SADDLEBACK AT SANTA RIT	A RANCH PHAS	E 1 SECTION	3										
SADDLEBACK 1-3	37.02	154	12.51	6.08	0	18.59							16,181

Table 2 - BMP Treatment Requirements

Table 2 - Bivii Tiea	шені кец	un ements)							
Duning			Drainage Basin							
Project	Area		Onsite Offsite				To	tal	Capacity a	t WQV (cf)
Total (ac)	Impv Area (ac)	Required TSS Removal (lbs)	Total (ac)	Impv Area (ac)	Total (ac) Impv Area (ac)		Total (ac) Impv Area		Required	Provided
EXISTING BATCH DETENTION	N POND S3									
87.87	41.20	35,869	83.96	40.87	3.91	0.33	87.87	41.20	176,815	179,000
EXISTING BATCH DETENTION POND S4										
38.38	19.36	16,851	33.30	19.12	5.08	0.24	38.38	19.36	80,541	80,800

Pond S3 Stage-Storage

	Cumulative	Cumulative	Incremental	Area (ac)	Area (sf)	Stage
	Storage (ac-ft)	Storage (cf)	Storage (cf)	Alea (SI) Alea (ac)		Stage
Pond Bottom	0.00	0	0	0.00	15	904.75
	0.01	493	493	0.09	3,927	905.00
	0.43	18,633	18,141	0.74	32,354	906.00
	1.69	73,799	55,166	1.79	77,977	907.00
	4.00	174,205	100,406	2.82	122,835	908.00
WQV Provided		179,000				908.03
	7.25	315,732	141,527	3.68	160,219	909.00
	11.35	494,278	178,546	4.52	196,873	910.00
	16.10	701,274	206,996	4.98	217,119	911.00
Top of Berm	21.17	922,373	221,100	5.17	225,080	912.00

^{*}Pond S3 Stage-Storage per Saddleback at Santa Rita Ranch Phase 1, Section 1 Construction Plans

Pond S4 Stage-Storage

	A (-6)	A ()	Incremental	Cumulative	Cumulative	
Stage	Area (sf)	Area (ac)	Storage (cf)	Storage (cf)	Storage (ac-ft)	
918.60	36	0.00	0	0	0.00	Pond Bottom
919.00	14,881	0.34	2,983	2,983	0.07	
920.00	33,816	0.78	24,348	27,332	0.63	
921.00	40,105	0.92	36,961	64,292	1.48	
921.39				80,800		WQV Provided
922.00	43,735	1.00	41,920	106,212	2.44	
923.00	47,361	1.09	45,548	151,760	3.48	
924.00	51,052	1.17	49,206	200,967	4.61	
925.00	54,807	1.26	52,930	253,896	5.83	
926.00	58,627	1.35	56,717	310,613	7.13	Top of Berm

^{*}Pond S4 Stage-Storage per Saddleback at Santa Rita Ranch Phase 1, Section 3 Construction Plans



CARLSON, BRIGANCE & DOERING, INC.

ID# F3791

12-1-2023

Appendix C

TCEQ CZP APPLICATION

Saddleback at Santa Rita Ranch Phase 1 Section 3

Williamson County, Texas

CZP Approval Letter

Jon Niermann, *Chairman*Emily Lindley, *Commissioner*Bobby Janecka, *Commissioner*Toby Baker, *Executive Director*



TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

Protecting Texas by Reducing and Preventing Pollution

August 13, 2021

Mr. James Edward Horne Santa Rita KC, LLC 1700 Cross Creek Lane, Suite 100 Liberty Hill, Texas 78642

Re: Edwards Aquifer, Williamson County

NAME OF PROJECT: Saddleback at Santa Rita Ranch Phase 1 Section 1; Located Southwest of Ronald Reagan Blvd. and Tower Rd.; Liberty Hill (ETJ), Texas

TYPE OF PLAN: Request for Approval of a Contributing Zone Plan (CZP); 30 Texas Administrative Code (TAC) Chapter 213 Edwards Aquifer

Edwards Aquifer Protection Program ID No. 11002565; Regulated Entity No. RN111283701

Dear Mr. Horne:

The Texas Commission on Environmental Quality (TCEQ) has completed its review of the CZP application for the above-referenced project submitted to the Austin Regional Office by Carlson, Brigance & Doering, Inc. on behalf of Santa Rita KC, LLC on June 18, 2021. Final review was completed after additional material was received on August 12, 2021. As presented to the TCEO, the Temporary and Permanent Best Management Practices (BMPs) were selected and construction plans were prepared by a Texas Licensed Professional Engineer to be in general compliance with the requirements of 30 TAC Chapter 213. These planning materials were sealed, signed and dated by a Texas Licensed Professional Engineer. Therefore, based on the engineer's concurrence of compliance, the planning materials for construction of the proposed project and pollution abatement measures are hereby approved subject to applicable state rules and the conditions in this letter. The applicant or a person affected may file with the chief clerk a motion for reconsideration of the executive director's final action on this Edwards Aguifer Protection Plan. A motion for reconsideration must be filed no later than 23 days after the date of this approval letter. This approval expires two (2) years from the date of this letter unless, prior to the expiration date, more than 10 percent of the construction has commenced on the project or an extension of time has been requested.

PROJECT DESCRIPTION

The proposed single-family residential project will have an area of approximately 79.21 acres. It will include the development of 222 single-family residential lots, roads and drives, utilities, and three water quality basins (Ponds S1, S2, and S3). The impervious cover will be 30.63 acres (38.7 percent). The wastewater generated by this project will be conveyed to the existing Liberty Hill Wastewater Treatment Plant.

PERMANENT POLLUTION ABATEMENT MEASURES

To prevent the pollution of stormwater runoff originating on-site or upgradient of the site and potentially flowing across and off the site after construction, three batch detentions (Ponds S1, S2, and S3), designed using the TCEQ technical guidance document, Complying with the Edwards Aquifer Rules: Technical Guidance on Best Management Practices (2005), will be constructed to treat stormwater runoff. The required total suspended solids (TSS) treatment for this project is 26,664 pounds of TSS generated from the 30.63 acres of impervious cover. The approved measures meet the required 80 percent removal of the increased load in TSS caused by the project.

The proposed water quality basins are sized for future development. Pond S1 is designed to have a capacity of 62,314 cubic feet (cf), to remove 9,732 pounds of TSS to treat stormwater runoff from a maximum of 11.18 acres of impervious cover. Pond S2 is designed to have a capacity of 208,303 cf, to remove 28,915 pounds of TSS to treat stormwater runoff from a maximum of 33.22 acres of impervious cover. Pond S3 is designed to have a capacity of 174,205 cf, to remove 26,181 pounds of TSS to treat stormwater runoff from a maximum of 30.08 acres of impervious cover.

SPECIAL CONDITIONS

- I. All permanent pollution abatement measures shall be operational prior to occupancy of the houses.
- II. All sediment and/or media removed from the water quality basins during maintenance activities shall be properly disposed of according to 30 TAC 330 or 30 TAC 335, as applicable.

STANDARD CONDITIONS

- 1. Pursuant to Chapter 7 Subchapter C of the Texas Water Code, any violations of the requirements in 30 TAC Chapter 213 may result in administrative penalties.
- 2. The holder of the approved Edwards Aquifer protection plan must comply with all provisions of 30 TAC Chapter 213 and all best management practices and measures contained in the approved plan. Additional and separate approvals, permits, registrations and/or authorizations from other TCEQ Programs (i.e., Stormwater, Water Rights, UIC) can be required depending on the specifics of the plan.
- 3. In addition to the rules of the Commission, the applicant may also be required to comply with state and local ordinances and regulations providing for the protection of water quality.

Prior to Commencement of Construction:

- 4. All contractors conducting regulated activities at the referenced project location shall be provided a copy of this notice of approval. At least one complete copy of the approved CZP and this notice of approval shall be maintained at the project location until all regulated activities are completed.
- 5. Any modification to the activities described in the referenced CZP application following the date of approval may require the submittal of a plan to modify this approval, including the payment of appropriate fees and all information necessary for its review and approval prior to initiating construction of the modifications.
- 6. The applicant must provide written notification of intent to commence construction, replacement, or rehabilitation of the referenced project. Notification must be submitted to the Austin Regional Office no later than 48 hours prior to commencement of the regulated activity. Written notification must include the date on which the regulated activity will

Mr. James Edward Horne Page 3 August 13, 2021

- commence, the name of the approved plan and program ID number for the regulated activity, and the name of the prime contractor with the name and telephone number of the contact person.
- 7. Temporary erosion and sedimentation (E&S) controls, i.e., silt fences, rock berms, stabilized construction entrances, or other controls described in the approved Storm Water Pollution Prevention Plan (SWPPP), must be installed prior to construction and maintained during construction. Temporary E&S controls may be removed when vegetation is established and the construction area is stabilized. If a water quality pond is proposed, it shall be used as a sedimentation basin during construction. The TCEQ may monitor stormwater discharges from the site to evaluate the adequacy of temporary E&S control measures. Additional controls may be necessary if excessive solids are being discharged from the site.

During Construction:

- 8. During the course of regulated activities related to this project, the applicant or agent shall comply with all applicable provisions of 30 TAC Chapter 213, Edwards Aquifer. The applicant shall remain responsible for the provisions and conditions of this approval until such responsibility is legally transferred to another person or entity.
- 9. If sediment escapes the construction site, the sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain). Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been significantly reduced. Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source for stormwater discharges (e.g., screening outfalls, picked up daily).
- 10. Intentional discharges of sediment laden water are not allowed. If dewatering becomes necessary, the discharge will be filtered through appropriately selected best management practices. These may include vegetated filter strips, sediment traps, rock berms, silt fence rings, etc.
- 11. The following records shall be maintained and made available to the executive director upon request: the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 12. Stabilization measures shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, and construction activities will not resume within 21 days. When the initiation of stabilization measures by the 14th day is precluded by weather conditions, stabilization measures shall be initiated as soon as practicable.
- 13. This approval does not authorize the installation of temporary aboveground storage tanks on this project. If the contractor desires to install a temporary aboveground storage tank for use during construction, an application to modify this approval must be submitted and approved prior to installation. The application must include information related to tank location and spill containment. Refer to Standard Condition No. 5, above.

After Completion of Construction:

- 14. Owners of permanent BMPs and measures must insure that the BMPs and measures are constructed and function as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the Austin Regional Office within 30 days of site completion.
- 15. The applicant shall be responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of

Mr. James Edward Horne Page 4 August 13, 2021

the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred. A copy of the transfer of responsibility must be filed with the executive director through the Austin Regional Office within 30 days of the transfer. A copy of the transfer form (TCEQ-10263) is enclosed.

- 16. Upon legal transfer of this property, the new owner(s) is required to comply with all terms of the approved Contributing Zone Plan. If the new owner intends to commence any new regulated activity on the site, a new Contributing Zone Plan that specifically addresses the new activity must be submitted to the executive director. Approval of the plan for the new regulated activity by the executive director is required prior to commencement of the new regulated activity.
- 17. A Contributing Zone Plan approval or extension will expire and no extension will be granted if more than 50 percent of the total construction has not been completed within ten years from the initial approval of a plan. A new Contributing Zone Plan must be submitted to the Austin Regional Office with the appropriate fees for review and approval by the executive director prior to commencing any additional regulated activities.
- 18. At project locations where construction is initiated and abandoned, or not completed, the site shall be returned to a condition such that the aquifer is protected from potential contamination.

This action is taken under authority delegated by the Executive Director of the Texas Commission on Environmental Quality. If you have any questions or require additional information, please contact Ms. Mihaela (Miki) Chilarescu, P.E. of the Edwards Aquifer Protection Program of the Austin Regional Office at (512) 339-2929.

Sincerely, Littian Butter

Lillian Butler, Section Manager

Edwards Aquifer Protection Program

Texas Commission on Environmental Quality

LIB/mec

Enclosure: Change in Responsibility for Maintenance of Permanent BMPs, Form TCEQ-10263

Agent Authorization Form

For Required Signature
Edwards Aquifer Protection Program
Relating to 30 TAC Chapter 213
Effective June 1, 1999

Stephen Ashlock							
	Print Name						
	Vice President of Land Development						
	Title - Owner/President/Other						
of	Pulte Homes of Texas, LP						
	Corporation/Partnership/Entity Name						
have authorized	Steven P. Cates, P.E.						
	Print Name of Agent/Engineer						
of	Carlson, Brigance & Doering, Inc.						
	Print Name of Firm						

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

- 1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
- For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
- 3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
- 4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
- 5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:

Applicant's Signature

11/28/2023

THE STATE OF EXAM

County of 24/15 §

BEFORE ME, the undersigned authority, on this day personally appeared Tepuca Known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this 28th day of November 12023.

SUSAN O MARTIN Notary Public, State of Texas My Commission Expires November 07, 2027 NOTARY ID 10425934

NOTARY PUBLIC

Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 11 07/2

Application Fee Form

Texas Commission on Environmental Quality

TCX43 COMMINISSION ON ENVIRONMENT	•									
Name of Proposed Regulated En	tity: <u>Saddleback at Santa</u>	Rita Ranch Phase 1 Se	ction 3							
Regulated Entity Location: South		of Grovelad Drive								
Name of Customer: Pulte Homes	s of Texas, LP									
Contact Person: Stephen Ashloc	<u>k</u> Phone	e: <u>(512)532-3355</u>								
Customer Reference Number (if	Customer Reference Number (if issued):CN <u>603410499</u>									
Regulated Entity Reference Num	ber (if issued):RN									
Austin Regional Office (3373)										
Hays	Travis	⊠ Wil	liamson							
San Antonio Regional Office (33	62)									
Bexar	Medina	Uva	alde							
Comal	Kinney									
Application fees must be paid by		monov ordor navahl	o to the Toyas							
Commission on Environmental										
form must be submitted with ye	-	•	•							
		_								
Austin Regional Office		n Antonio Regional Of								
Mailed to: TCEQ - Cashier		vernight Delivery to: To	CEQ - Cashier							
Revenues Section		100 Park 35 Circle								
Mail Code 214	Bu	ıilding A, 3rd Floor								
P.O. Box 13088	Αι	ıstin, TX 78753								
Austin, TX 78711-3088	(5	12)239-0357								
Site Location (Check All That Ap	ply):									
Recharge Zone	Contributing Zone	Transition Zone								
Type of P	lan	Size	Fee Due							
Water Pollution Abatement Pla	n, Contributing Zone									
Plan: One Single Family Resider	ntial Dwelling	Acres	\$							
Water Pollution Abatement Pla	n, Contributing Zone									
Plan: Multiple Single Family Res	sidential and Parks	37.02 Acres	\$ 4,000.00							
Water Pollution Abatement Pla	n, Contributing Zone									
Plan: Non-residential	Acres	\$								
Sewage Collection System	L.F.	\$								
Lift Stations without sewer line	Acres	\$								
Underground or Aboveground	Tanks	\$								
Piping System(s)(only)		Each	\$							
Exception		Each	\$							
Extension of Time		Each	\$							

gnature:

Date: 10/01/2023

Application Fee Schedule

Texas Commission on Environmental Quality

Edwards Aquifer Protection Program 30 TAC Chapter 213 (effective 05/01/2008)

Water Pollution Abatement Plans and Modifications

Contributing Zone Plans and Modifications

	Project Area in	
Project	Acres	Fee
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5	\$1,500
	5 < 10	\$3,000
	10 < 40	\$4,000
	40 < 100	\$6,500
	100 < 500	\$8,000
	≥ 500	\$10,000
Non-residential (Commercial, industrial,	< 1	\$3,000
institutional, multi-family residential, schools, and	1 < 5	\$4,000
other sites where regulated activities will occur)	5 < 10	\$5,000
	10 < 40	\$6,500
	40 < 100	\$8,000
	≥ 100	\$10,000

Organized Sewage Collection Systems and Modifications

Project	Cost per Linear Foot	Minimum Fee- Maximum Fee		
Sewage Collection Systems	\$0.50	\$650 - \$6,500		

Underground and Aboveground Storage Tank System Facility Plans and Modifications

Project	Cost per Tank or Piping System	Minimum Fee- Maximum Fee	
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500	

Exception Requests

Project	Fee
Exception Request	\$500

Extension of Time Requests

Project	Fee
Extension of Time Request	\$150



TCEQ Core Data Form

TCEQ Use Only

For detailed instructions regarding completion of this form, please read the Core Data Form Instructions or call 512-239-5175.

	- 12	
SECTION I: General Inform	いつけいい	

1. Reason for Submission (If other is checked please describe in space provided.) New Permit, Registration or Authorization (Core Data Form should be submitted with the program application.)													
Renewal	(Core Da	ata Form should	be submitted	with the	renew	al for	m)		Othe	r	<u> </u>	<u>, </u>	
	•	Number (if issue						3.	Regu	ulate	ed Entity Referer	ce Numbe	r (if issued)
CN 603	3410499)		for C	Follow this link to search for CN or RN numbers in Central Registry**								
SECTION II	: Custor	mer Informati	on	Cel	<u>ntrai i</u>	Kegi	stry						
4. General Customer Information 5. Effective Date for Customer Information Updates (mm/dd/yyyy) 12/01/2023								/2023					
New Custo		ne (Verifiable wit		•	to Cus					r of	•	•	d Entity Ownership
The Custon	mer Nam		here may b	е ирс	dated	auto	matic	ally	base	ed o	on what is cu		d active with the
6. Customer L	_egal Nam	e (If an individual,	print last name	first: e.ç	յ.: Doe,	John)		<u> 1</u>	fnew	Cus	stomer, enter prev	rious Custo	ner below:
Pulte Homes	of Texa	s. LP											
7. TX SOS/CF		•	8. TX State	Tax ID	(11 digits))		- (9. Fed	leral	I Tax ID (9 digits)	10. DU	NS Number (if applicable)
0010034910)		175272012	275					7527	201	127		
11. Type of Co	ustomer:		on			Individ	lual	ļ		Part	tnership: 🗌 Gene	ral Limited	t
Government: [City (County 🔲 Federal [State Other			Sole F	ropriet	orship) c	X	Other: Limited F	Partnership	
12. Number of X 0-20	f Employed	es 101-250	<u>251-500</u>		501 and	d high	er	[13. Inc		endently Owned No	and Opera	ted?
14. Customer	Role (Prop	oosed or Actual) -	as it relates to t	he Regi	ulated E	ntity li	sted on	this fo	rm. Ple	ease	e check one of the	following:	
Owner Occupation	nal License	Opera	itor Insible Party				& Opera		nnlica	ant	Other:		
· -		berglen Blvd.	moioro i urty			, arritar	y Oloui	тар 7	фрпос	4110			
∣ 15. Mailing ⊢	Bldg. I, S												
Address.		Austin		St	tate	TX		ZIP	78	3729	9	ZIP + 4	
16 Country M		rmation (if outside	ΠΖΔ			171	17 F	-Mail			(if applicable)		
To: Country in	idiii ig ii iio	THICKOTT (III OULDIGO	00/1/								@PulteGroup	.com	
18. Telephone	e Number			19. Ex	ktension	n or C					20. Fax Number		ıble)
(512)	532 - 33	55									()	-	
SECTION II	I: Regul	ated Entity Ir	nformation										
21. General Re	egulated E	Intity Information	(If `New Regu	ulated E	Entity" i	s sele	cted be	low t	his for	rm s	should be accom	panied by	a permit application)
New Regulated Entity													
The Regulated Entity Name submitted may be updated in order to meet TCEQ Agency Data Standards (removal of organizational endings such as Inc, LP, or LLC).													
_		me (Enter name o		-	•	ction is	taking	place.	.)				
Saddleback	at Santa	a Rita Ranch F	Phase 1 Sec	tion 3									

TCEQ-10400 (04/15) Page 1of 2

Regulated Entity: (No PO Boxes)																		
		City				State		ZIP					ZI	ZIP + 4				
24. County																		
				Enter Physical	Loca	tion Description	if	no street	addre	ss is p	orovid	ed.						
25. Description Physical Local																		
26. Nearest C	ity									State				Nearest ZIP Code				
Liberty Hill										TX				78642				
27. Latitude (al: 30.670536						28. Longitude (V						-97.851442					
Degrees		Minutes				onds	Degrees		Minutes			Seconds						
30		40	40			93	97				51			5.19				
29. Primary S	its) 30. Secondary SIC C				ode (4 digits) 31. Prima (5 or 6 digits									econdary NAICS Code 6 digits)				
1521																		
33. What is th	e Primary Bus	siness	of this	s entity? (Do n	ot rep	eat the SIC or NAIC	S d	escription.)										
34. Mailing Address:		Pulte	Grou	nb														
		9401	Amb	erglen Blvd. Blo														
		City	City Liberty Hill			State	1	ГХ	ZIP		78729			Z	ZIP + 4			
35. E-N	Mail Address:	Stephen.Ashlock@Pu				lteGroup.com												
36. Telephone Number						37. Extensi	38. Fax Number (if applicable)											
	532 - 3355								() -									
39. TCEQ Progr			eck all	Programs and writ	e in th	e permits/registration	on n	umbers tha	t will be	e affecte	ed by th	e updat	es subm	nitted on	this form	. See	the Co	re Data
☐ Dam Safety		Districts				Edwards Aquifer				Emissions Inventory Air					Industrial Hazardous Waste			
					E	EAPP # 11002565												
Municipal Solid Waste		☐ New Source Review		Air	OSSF		□Р	Petroleum Storage Ta			Tank	k PWS						
Sludge		Storm Water				☐ Title V Air			Tires						Used C)il		
☐ Voluntary Cleanup		☐ Waste Water				☐Wastewater Agriculture			☐ Water Rights				Other:					
					1				<u> </u>									
SECTION I	V: Preparer	Inforn	natio	n														
40. Name: Steven P. Cates, P.E.									41. Title: Senior Project Manager									
42. Telephone	43. Ext./Code				44. Fax Number				45. E-Mail Address									
(512)28					(512)280-5165			steve@cbdeng.com										
	iture below, I ce	rtify, to	the be	est of my knowled		hat the informatio d 6 and/or as req										gnat	ure aut	hority
Company:	Carlson, Brig	ance a	ance and Doering, Inc.							Title:	Senio	r Proje	ect Ma	nager				
Name(In Print):	Steven P. Ca	es								ne:	(512	2)280	516	0				

TCEQ-10400 (04/15) Page 2 of 2

Signature:

two Cate

12-18-2023

Date:

SADDLEBACK AT SANTA RITA RANCH PHASE 1 SECTION 3

ALL RESPONSIBILITY FOR THE ADEQUACY OF THESE PLANS REMAINS WITH THE ENGINEER WHO PREPARED THEM. IN ACCEPTING THESE PLANS, WILLIAMSON COUNTY MUST RELY UPON THE ADEQUACY OF THE WORK OF THE DESIGN ENGINEER

THESE WATER SYSTEM PLANS WERE PREPARED, SEALED, SIGNED AND DATED BY A TEXAS LICENSED PROFESSIONAL CONSTRUCTION SPECIFICATIONS AND DETAILS MANUAL AND ALL OTHER APPLICABLE CITY, STATE AND FEDERAL REQUIREMENTS AND CODES.

SUBMITTED BY:

12-1-2023

STEVEN P. CATES, P.E. REGISTERED PROFESSIONAL ENGINEER No. 93648

DATE

DATE

ACCEPTED FOR CONSTRUCTION:

CITY OF GEORGETOWN (WATER SYSTEM ONLY)	DATE
	5.487
DAVID THOMISON, PUBLIC WORKS DIRECTOR (WASTEWATER SYSTEM PLAN)	DATE

PAUL BRANDENBURG, CITY MANAGER (WASTEWATER SYSTEM PLAN)

WILLIAMSON COUNTY M.U.D. #19H

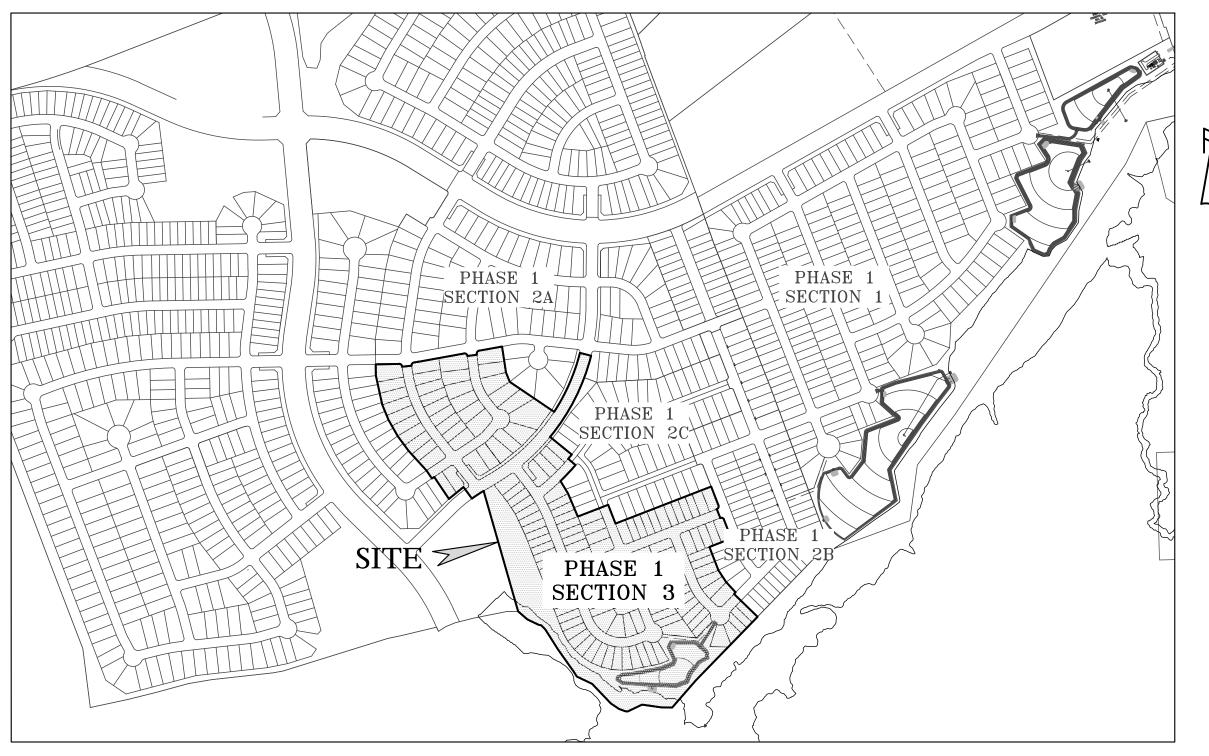
Based on the design engineer's certification of compliance with all applicable City, State and Federal regulations the plans and specifications contained herein have been reviewed and are found to be in compliance with the requirements of the City of Liberty Hill.

REVIEWED FOR COMPLIANCE WITH COUNTY REQUIREMENTS:

FOR WILLIAMSON COUNTY	DATI

REV. NO. SHT. NO. **ACCEPTED** DATE DATE ACCEPTED DATE ACCEPTED DESCRIPTION OF REVISION ACCEPTED CITY OF GEORGETOWN CITY OF LIBERTY HILL WILLIAMSON COUNTY W.C. M.U.D. #19

WILLIAMSON COUNTY, TEXAS CONSTRUCTION PLANS



LOCATION MAP SCALE: 1" = 500'

THIS PROJECT IS LOCATED WITHIN THE EDWARDS AQUIFER CONTRIBUTING ZONE A CONTRIBUTING ZONE PLAN WAS APPROVED BY TCEQ ON ______ 30 TEXAS ADMINISTRATIVE CODE (TAC) CHAPTER 213 EDWARDS AQUIFER PROTECTION PROGRAM ID NO. 11003083 THE WASTEWATER SYSTEM WAS APPROVED BY THE TCEQ ON ______ 30 TEXAS ADMINISTRATIVE CODE (TAC) CHAPTER 217 PERMIT NO. _____ WWPR LOG NO. _____

OWNER:

PULTE HOMES OF TEXAS, LP 9401 AMBERGLEN BLVD. BLDG. I, SUITE 150 AUSTIN, TX. 78729

ENGINEER & SURVEYOR: CARLSON, BRIGANCE & DOERING, INC. 5501 WEST WILLIAM CANNON DRIVE AUSTIN, TEXAS 78749 (512) 280-5160 phone (512) 280-5165 fax

TOTAL ACREAGE: 37.018 AC SURVEY: B. MANLOVE SURVEY, ABSTRACT NO. 417

F.E.M.A. MAP NO. 48491C 0275E WILLIAMSON COUNTY, TEXAS AND INCORPORATED AREAS. DATED: SEPTEMBER 26, 2008



STEVEN P. CATES CARLSON, BRIGANCE & DOERING, INC.

1D# F3791

> OCT 2023 5413

12-1-2023

of 73

SHEET INDEX

1 - COVER SHEET
2 - SHEET INDEX
3 - GENERAL NOTES (1 OF 2)
4 - GENERAL NOTES (2 OF 2)
5 - FINAL PLAT
6 - FINAL PLAT
7 - FINAL PLAT
8 - EROSION CONTROL PLAN (1 OF 2)
9 - EROSION CONTROL PLAN (2 OF 2)
10 - EROSION CONTROL NOTES & DETAILS 11 - EXISTING HYDROLOGY MAP
12 - DEVELOPED HYDROLOGY MAP
13 - DRAINAGE AREA PLAN (1 OF 2)
14 - DRAINAGE AREA PLAN (2 OF 2)
15 - DRAINAGE AREA CALCS
16 - TCEQ PROJECT AND DRAINAGE AREA MAP
17 - GRADING PLAN (1 OF 2
18 - GRADING PLAN (2 OF 2)
19 - TRAFFIC CONTROL PLAN (1 OF 2)
20 - TRAFFIC CONTROL PLAN (2 OF 2)
21 - BOWIE BEND (0+00 TO 4+00)
22 - BOWIE BEND (4+00 TO 8+00)
23 - BOWIE BEND (8+00 TO 11+00)
24 - BOWIE BEND (11+00 TO 14+50)
25 - BOWIE BEND (14+50 TO 17+50)
26 - BOWIE BEND (17+50 TO END) 27 - BRISTLECONE BEND (0+00 TO 3+00)
28 - BRISTLECONE BEND (0+00 TO 5+00) 28 - BRISTLECONE BEND (3+00 TO END)
29 - HODGE LANE (0+00 TO 4+00)
30 - HODGE LANE (4+00 TO END)
31 - ARCHWAY LANE (0+00 TO 3+50)
32 - ARCHWAY LANE (3+50 TO END)
33 - LILLY WAY (0+00 TO 2+50)
34 - LILLY WAY (2+50 TO 5+00)
35 - LILLY WAY (5+00 TO END)
36 - MATERA PASS (0+00 TO END)
37 - BARNETT PASS (0+00 TO 4+00)
38 - BARNETT PASS (4+00 TO 7+50)
39 - BARNETT PASS (7+50 TO END)
40 - POND PLAN
41 - POND DETAILS
42 - OVERALL STORMSEWER PLAN (1 OF 2)
43 - OVERALL STORMSEWER PLAN (2 OF 2)
44 - STORMSEWER LINE A (0+00 TO 6+00) 45 - STORMSEWER LINE A (6+00 TO 14+00)
46 - STORMSEWER LINE A (0+00 TO 14+00) 46 - STORMSEWER LINE A (14+00 TO END)
47 - STORMSEWER LINE B (0+00 TO END)
48 - STORMSEWER LINE C & D (0+00 TO END)
49 - STORMSEWER LINE E (0+00 TO END)
50 - STORMSEWER LINE F & G (0+00 TO END)
51 - STORMSEWER LINE H (0+00 TO END)
52 - STORMSEWER LATERALS (1 OF 2)
53 - STORMSEWER LATERALS (2 OF 2)
54 - OVERALL WATER PLAN (1 OF 2)
55 - OVERALL WATER PLAN (2 OF 2)
56 - OVERALL WASTEWATER PLAN (1 OF 2)
57 - OVERALL WASTEWATER PLAN (2 OF 2)
58 - WASTEWATER LINE A (0+00 TO 8+50)
59 - WASTEWATER LINE A (8+50 TO 17+00)
60 - WASTEWATER LINE A (17+00 TO END)
61 - WASTEWATER LINE B (0+00 TO END)

61 - WASTEWATER LINE B (0+00 TO END)

62	_	WASTEWATER LINE C (0+00 TO END)
63	_	WASTEWATER LINE D AND E (0+00 TO END
64	_	WASTEWATER LINE F (0+00 TO END)
65	_	WASTEWATER LINE G (0+00 TO END)
66	_	WASTEWATER LINE H (0+00 TO END)
67	_	WASTEWATER LINE J (0+00 TO END)
68	_	WASTEWATER LINE K (0+00 TO END)
69	_	CONSTRUCTION DETAILS (1 OF 3)
70	_	CONSTRUCTION DETAILS (2 OF 3)
71	_	CONSTRUCTION DETAILS (3 OF 3)
72	_	WATER DETAILS
73	_	WASTEWATER DETAILS



12-1-2023

OCT 2023 5413 SHEET 2 OF 73

- GENERAL CONTRACTOR TO INSTALL AND MAINTAIN EROSION CONTROLS AND TREE PROTECTION PER APPROVED PLANS.
- 2. HOLD PRE-CONSTRUCTION CONFERENCE. PROVIDE 72 HOUR NOTIFICATION TO THE OWNER, THE DESIGN ENGINEER, THE CONTRACTOR AND SUBCONTRACTORS, THE M.U.D. ENGINEER (512) 836-4817, THE CITY OF LIBERTY HILL (512) 778-5449, (WAYNE BONNET, DIRECTOR OF PUBLIC WORKS), WILLIAMSON COUNTY INSPECTIONS SUPERVISOR, GEORGE MAYFIELD (512) 943-3324, AND THE CITY OF GEORGETOWN UTILITY SYSTEM (512)930-3640. SEE WILLIAMSON COUNTY SUBDIVISION REGULATIONS CONSTRUCTION-GENERAL NOTE #1 ON THIS
- 3. ROUGH CUT ALL REQUIRED OR NECESSARY PONDS. EITHER THE PERMANENT OUTLET STRUCTURE OR A TEMPORARY OUTLET MUST BE CONSTRUCTED PRIOR TO DEVELOPMENT OF ANY EMBANKMENT OR EXCAVATION THAT LEADS TO PONDING CONDITIONS. THE OUTLET SYSTEM MUST CONSIST OF A LOW-LEVEL OUTLET AND AN EMERGENCY OVERFLOW. THE OUTLET SYSTEM SHALL BE PROTECTED FROM EROSION AND SHALL BE MAINTAINED THROUGHOUT THE COURSE OF CONSTRUCTION UNTIL FINAL RESTORATION IS ACHIEVED.
- 4. ROUGH GRADE STREETS. NO DEVELOPMENT OF EMBANKMENT WILL BE PERMITTED AT THIS TIME. ONCE STREETS ARE ROUGH CUT, THE GEOTECHNICAL ENGINEER IS TO FIELD VERIFY PAVEMENT DESIGN IS APPROPRIATE, AND MODIFY RECOMMENDATIONS ACCORDINGLY.
- 6. BEGIN INSTALLATION OF STORM SEWER LINES. UPON COMPLETION, RESTORE AS MUCH DISTURBED AREA AS MUCH AS POSSIBLE, PARTICULARLY CHANNELS AND LARGE OPEN AREAS.
- REGRADE STREETS TO SUBGRADE. 8. INSURE THAT ALL UNDERGROUND UTILITY CROSSINGS ARE COMPLETED. LAY FIRST COURSE BASE MATERIAL ON ALL STREETS.
- 9. INSTALL CURB AND GUTTER.
- 10. LAY FINAL BASE COURSE ON ALL STREETS. 11. LAY ASPHALT.
- 12. COMPLETE ALL UNDERGROUND INSTALLATIONS WITHIN THE R.O.W.

5. INSTALL ALL UTILITIES TO BE LOCATED UNDER THE PROPOSED PAVEMENT.

- 13. COMPLETE PERMANENT EROSION CONTROL AND RESTORATION OF SITE VEGETATION. 14. THE PROJECT ENGINEER INSPECTS JOB AND WRITES CONCURRENCE LETTER TO THE CITY. FINAL INSPECTION IS SCHEDULED UPON RECEIPT OF LETTER. FINAL INSPECTION WITH THE M.U.D. ENGINEER, EV INSPECTOR, THE CITY OF LIBERTY HILL, WILLIAMSON COUNTY, AND THE CITY OF GEORGETOWN PRIOR TO THE REMOVAL OF EROSION CONTROLS. 15. REMOVE AND DISPOSE OF TEMPORARY EROSION CONTROLS. TREE PROTECTION SHALL BE REQUIRED TO BE MAINTAINED AND REMAIN IN PLACE FOR EACH RESIDENTIAL LOT THROUGH
- RECEIPT OF THE CONCURRENCE LETTER TO THE RESIDENTIAL CERTIFICATE OF OCCUPANCY. 16. COMPLETE ANY NECESSARY FINAL DRESS UP OF AREAS DISTURBED BY ITEM 15.

GEORGETOWN UTILITY SYSTEMS GENERAL NOTES:

- THESE CONSTRUCTION PLANS WERE PREPARED, SEALED AND DATED BY A TEXAS LICENSED PROFESSIONAL ENGINEER. THEREFORE BASED ON THE ENGINEER'S CONCURRENCE OF COMPLIANCE, THE CONSTRUCTION PLANS FOR CONSTRUCTION OF THE PROPOSED PROJECT ARE HEREBY APPROVED SUBJECT TO THE STANDARD CONSTRUCTION SPECIFICATIONS AND
- DETAILS MANUAL AND ALL OTHER APPLICABLE CITY, STATE AND FEDERAL REQUIREMENTS AND CODES. THIS PROJECT IS SUBJECT TO ALL CITY STANDARD SPECIFICATIONS AND DETAILS IN EFFECT AT THE TIME OF SUBMITTAL OF THE PROJECT OF THE CITY.
- THE SITE CONSTRUCTION PLANS SHALL MEET ALL REQUIREMENTS OF THE APPROVED SITE PLAN.
- PRIVATE WATER SYSTEM FIRE LINES SHALL BE TESTED BY THE CONTRACTOR TO 200 PSI FOR 2 HOURS.
- PRIVATE WATER SYSTEM FIRE LINES SHALL BE DUCTILE IRON PIPING FROM THE WATER MAIN TO THE BUILDING SPRINKLER SYSTEM, AND 200 PSI C900 PVC FOR ALL OTHERS.
- PUBLIC WATER SYSTEM MAINS SHALL BE 150 PSI C900 PVC AND TESTED BY THE CONTRACTOR AT 150 PSI FOR 4 HOURS.
- ALL BENDS AND CHANGES IN DIRECTION ON WATER MAINS SHALL BE RESTRINED AND THRUST BLOCKED. LONG FIRE HYDRANT LEADS SHALL BE RESTRAINED.
- 9. ALL WATER LINES ARE TO BE BACTERIA TESTED BY THE CONTRACTOR ACCORDING TO THE CITY STANDARDS AND SPECIFICATIONS.
- 10. WATER AND SEWER MAIN CROSSINGS SHALL MEET ALL REQUIREMENTS OF THE TCEQ AND THE CITY. 11. A MAINTENANCE BOND IS REQUIRED TO BE SUBMITTED TO TEH CITY PRIOR TO ACCEPTANCE OF THE PUBLIC IMPROVMENTS. THIS BOND SHALL BE ESTABLISHED FOR 2 YEARS IN THE AMOUNT OF 10% OF THE COST OF THE PUBLIC IMPROVEMENTS AND SHALL FOLLOW THE CITY FORMAT.
- 12. RECORD DRAWINGS OF THE PUBLIC IMPROVMENTS SHALL BE SUBMITTED TO THE CITY BY THE DESIGN ENGINEER PRIOR TO ACCEPTACNE OF THE PROJECT. THESE DRAWINGS SHALL BE SUBMITTED AS A PDF (300P DPI) ON A FLASH DRIVE, OR BY A CLOUD SOURCE.

STREET AND DRAINAGE NOTES:

- 1. ROADWAY CONSTRUCTION SHALL BE IN ACCORDANCE WITH CURRENT "WILLIAMSON COUNTY SUBDIVISION REGULATIONS," AS APPLICABLE. 2. ALL MATERIALS SHALL BE SAMPLED AND TESTED BY AN INDEPENDENT TESTING LABORATORY IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE COUNTY ENGINEER. THE OWNER SHALL PAY FOR ALL TESTING SERVICES AND SHALL FURNISH THE COUNTY ENGINEER WITH CERTIFIED COPIES OF THESE TEST RESULTS. ANY RETESTING SHALL BE PAID FOR BY THE CONTRACTOR. THE COUNTY ENGINEER MUST APPROVE THE TEST RESULTS PRIOR TO CONSTRUCTING THE NEXT COURSE OF THE ROADWAY STRUCTURE. ANY MATERIAL WHICH DOES NOT MEET THE MINIMUM REQUIRED TEST SPECIFICATIONS SHALL BE REMOVED AND RECOMPACTED OR REPLACED UNLESS ALTERNATIVE
- REMEDIAL ACTION IS APPROVED IN WRITING FROM THE COUNTY ENGINEER. 3. BACKFILL BEHIND THE CURB SHALL BE COMPACTED TO OBTAIN A MINIMUM OF 95% MAXIMUM DENSITY TO WITHIN 3" OF TOP OF CURB. MATERIAL USED SHALL BE PRIMARILY GRANULAR WITH NO ROCKS LARGER THAN 6" IN THE GREATEST DIMENSION. THE REMAINING 3" SHALL BE CLEAN TOPSOIL FREE FROM ALL CLODS AND SUITABLE FOR SUSTAINING
- 4. DEPTH OF COVER FOR ALL CROSSINGS UNDER PAVEMENT INCLUDING GAS, ELECTRIC, TELEPHONE, CABLE TV, WATER SERVICES, ETC., SHALL BE A MINIMUM OF 30" BELOW
- 5. STREET RIGHTS-OF-WAY SHALL BE GRADED AT A SLOPE OF 1/4" PER FOOT TOWARD THE CURB UNLESS OTHERWISE INDICATED. HOWEVER, IN NO CASE SHALL THE WIDTH OF RIGHT-OF-WAY AT 1/4" PER FOOT SLOPE BE LESS THAN 10 FEET UNLESS A SPECIFIC REQUEST FOR AN ALTERNATE GRADING SCHEME IS MADE TO AND ACCEPTED BY THE COUNTY ENGINEER. BARRICADES BUILT TO WILLIAMSON COUNTY STANDARDS SHALL BE CONSTRUCTED ON ALL DEAD—END STREETS AND AS NECESSARY DURING CONSTRUCTION TO
- 6. ALL R.C.P. SHALL BE MINIMUM CLASS III, UNLESS OTHERWISE NOTED. 7. THE PREPARATION OF SUBGRADE SHALL FOLLOW GOOD ENGINEERING PRACTICES AS DIRECTED BY THE COUNTY ENGINEER AND IN CONJUNCTION WITH THE OUTLINED IN THE
- GEOTECHNICAL REPORT BY MLA LABS, INC., DATED . THE PAVING SECTIONS ARE TO BE CONSTRUCTED AS FOLLOWS:

RECOMMENDATIONS PAVEMENT THICKNESS SECTIONS												
Street Classification	Subgrade Material	Hot Mix Asphaltic Concrete, in	Crushed Limestone Base, in	Lime Stabilized Subgrade, in								
Neighborhood Collectors	Subgrade PI > 20	3.0	13	8								
Local Streets	Subgrade PI > 20	2.0	8	8								
Residential Collectors	Subgrade PI > 20	2.0	12	8								

- 2. The surface clay must first be tested for sulfate reaction and a mix design should be completed to determine the proper lime content, lime bype, mixing procedure and
- 3. The subgrade improvement should be extended 3 feet beyond the back of the curb
- 6. If pavement designs other than those listed above are desired, please contact ETTL
- THE GEOTECHNICAL ENGINEER SHALL INSPECT THE SUBGRADE FOR COMPLIANCE WITH THE DESIGN ASSUMPTIONS MADE DURING PREPARATION OF THE SOILS REPORT. ANY
- APPROPRIATE SUBGRADE STABILIZATION IF SULFATES ARE DETERMINED TO BE PRESENT. SEE THE GEOTECHNICAL REPORT FOR DESIGN GUIDES FOR DIFFERENT PI VALUES.
- 9. CONTRACTOR IS TO AVOID INSTALLATION OF IRRIGATION, PLANTINGS, SILT FENCE, ETC. IN THE SUBGRADE IMPROVEMENT EXTENDED BEHIND THE CURB.

COUNTY REQUIRES THE 2 INCH WEARING SURFACE TO BE "VIRGIN MIX"

GENERAL NOTES:

- 1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH WILLIAMSON COUNTY, CITY OF ROUND ROCK (WASTEWATER), AND GEORGETOWN UTILITY SYSTEMS (WATER)
- SPECIFICATIONS 2. DESIGN PROCEDURES ARE IN COMPLETE COMPLIANCE WITH THE CITY OF AUSTIN DRAINAGE CRITERIA MANUAL AND ALL VARIANCES TO THE MANUAL ARE NOTED.
- VARIANCES REQUESTED: (NONE) ANY EXISTING UTILITIES, PAVEMENT, CURBS, SIDEWALKS, STRUCTURES, TREES, ETC., NOT PLANNED FOR DESTRUCTION OR REMOVAL THAT ARE DAMAGED OR REMOVED SHALL BE REPAIRED OR REPLACED AT HIS EXPENSE.
- 4. THE CONTRACTOR SHALL VERIFY ALL DEPTHS AND LOCATIONS OF EXISTING UTILITIES PRIOR TO ANY CONSTRUCTION. ANY DISCREPANCIES WITH THE CONSTRUCTION PLANS FOUND IN THE FIELD SHALL BE BROUGHT IMMEDIATELY TO THE ATTENTION OF THE ENGINEER WHO SHALL BE RESPONSIBLE FOR REVISING THE PLANS ARE
- MANHOLE FRAMES, COVERS, VALVES, CLEANOUTS, ETC. SHALL BE RAISED TO FINISHED GRADE PRIOR TO FINAL PAVING CONSTRUCTION. THE CONTRACTOR SHALL GIVE THE CITY OF LIBERTY HILL 48 HOURS NOTICE BEFORE BEGINNING EACH PHASE OF CONSTRUCTION. TELEPHONE 512-778-5449
- (PLANNING & DEVELOPMENT DEPARTMENT) ALL AREAS DISTURBED O EXPOSED DURING CONSTRUCTION SHALL BE REVEGETATED IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. REVEGETATION MUST EQUAL OR EXCEED THE TYPE OF VEGETATION PRESENT BEFORE CONSTRUCTION.
- 8. THE CONTRACTOR AND THE ENGINEER SHALL KEEP ACCURATE RECORDS OF ALL CONSTRUCTION THAT DEVIATES FROM THE PLANS. THE ENGINEERR SHALL FURNISH THE CITY OF LIBERTY HILL ACCURATE "AS-BUILT" DRAWINGS FOLLOWING COMPLETION OF ALL CONSTRUCTION, THESE "AS-BUILT" DRAWINGS SHALL MEET WITH THE
- SATISFACTION OF THE PLANNING & DEVELOPMENT DEPARTMENT PRIOR TO FINAL ACCEPTANCE 9. THE LIBERTY HILL CITY COUNCIL SHALL NOT BE PETITIONED FOR ACCEPTANCE UNTIL ALL NECESSARY EASEMENT DOCUMENTS HAVE BEEN SIGNED AND RECORDED. 10. WHEN CONSTRUCTION IS BEING CARRIED OUT WITHIN EASEMENTS, THE CONTRACTOR SHALL CONFINE HIS WORK TO WITHIN THE PERMANENT AND ANY TEMPORARY EASEMENTS. PRIOR TO FINAL ACCEPTANCE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING ALL TRASH AND DEBRIS WITHIN THE PERMANENT AND
- TEMPORARY EASEMENTS. CLEANUP SHALL BE TO THE SATISFACTION OF THE COUNTY ENGINEER. 11. PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR SHALL APPLY FOR AND SECURE ALL PROPER PERMITS FROM THE APPROPRIATE AUTHORITIES.

12. AVAILABLE BENCHMARKS THAT MAY BE UTILIZED FOR THE CONSTRUCTION OF THIS PROJECT ARE DESCRIBED AS FOLLOWS:

BENCHMARKS:

DESCRIPTION	ELEVATION	NORTHING	EASTING
MAG NAIL @ TOP OF CURB ON THE			
NORTH WEST SIDE OF THE			
INTERSECTION OF INSPIRATION DRIVE			
AND KRUPP AVENUE	1,034.95	10,215,305.2832	3,082,177.5880
MAG NAIL W/ SHINER ON HEADWALL ON			
NORTH SIDE OF MAGDALENE			
WAY/KRUPP AVENUE KNUCKLE	1,025.14	10,216,145.1382	3,083,291.4400
MAG NAIL W/ SHINER IN THE EASTERN			
ROW OF RONALD REAGAN BLVD			
APPROXIMATELY 600' SOUTH OF THE			
BRIDGE OVER SOWES BRANCH	908.25	10,218,449.9552	3,080,132.8670

TRENCH SAFETY NOTES:

- 1. IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U. S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, ALI TRENCHES OVER 5 FEET IN DEPTH IN EITHER HARD AND COMPACT OR SOFT AND UNSTABLE SOIL SHALL BE SLOPED, SHORED, SHEETED, BRACED OR OTHERWISE SUPPORTED. FURTHERMORE, ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT WILL BE PROVIDED BY THE
- 2. IN ACCORDANCE WITH THE U. S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4-FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED SO AS TO REQUIRE NO MORE THAN
- 3. IF TRENCH SAFETY SYSTEM DETAILS WERE NOT PROVIDED IN THE PLANS BECAUSE TRENCHES WERE ANTICIPATED TO BE LESS THAN 5 FEET IN DEPTH AND DURING CONSTRUCTION IT IS FOUND THAT TRENCHES ARE IN FACT 5 FEET OR MORE IN DEPTH OR TRENCHES LESS THAN 5 FEET IN DEPTH ARE IN AN AREA WHERE HAZARDOUS GROUND MOVEMENT IS EXPECTED, ALL CONSTRUCTION SHALL CEASE, THE TRENCHED AREA SHALL BE BARRICADED AND THE ENGINEER NOTIFIED IMMEDIATELY. CONSTRUCTION SHALL NOT RESUME UNTIL APPROPRIATE TRENCH SAFETY SYSTEM DETAILS, AS DESIGNED BY A PROFESSIONAL ENGINEER, ARE RETAINED AND COPIES SUBMITTED TO THE CITY OF LIBERTY HILL.

TRAFFIC MARKING NOTES:

- 1. ANY METHODS, STREET MARKINGS AND SIGNAGE NECESSARY FOR WARNING MOTORISTS, WARNING PEDESTRIANS OR DIVERTING TRAFFIC DURING CONSTRUCTION SHALL CONFORM TO THE TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS, LATEST EDITION.
- 2. ALL PAVEMENT MARKINGS, MARKERS, PAINT, TRAFFIC BUTTONS, TRAFFIC CONTROLS AND SIGNS SHALL BE INSTALLED IN ACCORDANCE WITH THE TEXAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR CONSTRUCTION OF HIGHWAYS, STREETS AND BRIDGES AND, THE TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS, LATEST EDITIONS.

EROSION AND SEDIMENTATION CONTROL NOTES:

- 1. EROSION CONTROL MEASURES, SITE WORK AND RESTORATION WORK SHALL BE IN ACCORDANCE WITH THE CITY OF LIBERTY HILL EROSION AND SEDIMENTATION CONTROL ORDINANCE
- 2. ALL SLOPES SHALL BE SODDED OR SEEDED WITH APPROVED GRASS, GRASS MIXTURES OR GROUND COVER SUITABLE TO THE AREA AND SEASON IN WHICH THEY ARE APPLIED.
- 3. SILT FENCES, ROCK BERMS, SEDIMENTATION BASINS AND SIMILARLY RECOGNIZED TECHNIQUES AND MATERIALS SHALL BE EMPLOYED DURING CONSTRUCTION TO PREVENT POINT SOURCE SEDIMENTATION LOADING OF DOWNSTREAM FACILITIES. SUCH INSTALLATION SHALL BE REGULARLY INSPECTED BY THE CITY OF LIBERTY HILL FOR EFFECTIVENESS. ADDITIONAL MEASURES MAY BE REQUIRED IF. IN THE OPINION OF THE CITY ENGINEER. THEY ARE WARRANTED.
- 4. ALL TEMPORARY EROSION CONTROL MEASURES SHALL NOT BE REMOVED UNTIL FINAL INSPECTION AND APPROVAL OF THE PROJECT BY THE ENGINEER. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN ALL TEMPORARY EROSION CONTROL STRUCTURES AND TO REMOVE EACH STRUCTURE
- 5. ALL MUD, DIRT, ROCKS, DEBRIS, ETC., SPILLED, TRACKED OR OTHERWISE DEPOSITED ON EXISTING PAVED STREETS, DRIVES AND AREAS USED BY THE PUBLIC SHALL BE CLEANED UP IMMEDIATELY.

WILLIAMSON COUNTY SUBDIVISION REGULATIONS

APPENDIX B

ADOPTED AND EFFECTIVE AS OF JANUARY 18, 2018

- 1. A PRECONSTRUCTION MEETING SHALL BE SCHEDULED PRIOR TO THE START OF CONSTRUCTION. THE DESIGN ENGINEER, M.U.D. ENGINEER, OWNER, CONTRACTOR, SUBCONTRACTORS, AND COUNTY ENGINEER SHALL ATTEND THIS MEETING. ALL ROADS ARE TO BE CONSTRUCTED IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS AS APPROVED BY THE COUNTY ENGINEER AND IN ACCORDANCE WITH THE SPECIFICATIONS FOUND IN THE CURRENT VERSION OF THE "TEXAS DEPARTMENT OF TRANSPORTATION MANUAL STANDARD SPECIFICATIONS FOR CONSTRUCTION OF HIGHWAYS, STREETS, AND BRIDGES" UNLESS OTHERWISE STATED ON THE CONSTRUCTION DOCUMENTS APPROVED BY THE COUNTY ENGINEER.
- 2. ALL MATERIALS SHALL BE SAMPLED AND TESTED BY AN INDEPENDENT TESTING LABORATORY IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE COUNTY ENGINEER. THE OWNER SHALL PAY FOR ALL TESTING SERVICES AND SHALL FURNISH THE COUNTY ENGINEER WITH CERTIFIED COPIES OF THESE TEST RESULTS. THE COUNTY ENGINEER MUST APPROVE THE TEST RESULTS PRIOR TO CONSTRUCTING THE NEXT COURSE OF THE ROADWAY STRUCTURE. ANY MATERIAL WHICH DOES NOT MEET THE MINIMUM REQUIRED TEST SPECIFICATIONS SHALL BE REMOVED AND RECOMPACTED OR REPLACED UNLESS ALTERNATIVE REMEDIAL ACTION IS APPROVED IN WRITING FROM THE COUNTY ENGINEER.
- 3. EXCEPT FOR ELECTRICAL LINES, ALL UNDERGROUND NONFERROUS UTILITIES WITHIN A RIGHT-OF-WAY OR EASEMENT MUST BE ACCOMPANIED BY FERROUS METAL LINES TO AID IN TRACING THE LOCATION OF SAID UTILITIES THROUGH THE USE OF A METAL
- 4. ALL PAVEMENTS ARE TO BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER. THE DESIGN SHALL BE BASED ON A 20-YEAR DESIGN LIFE AND IN CONJUNCTION WITH RECOMMENDATIONS BASED UPON A SOILS REPORT OF SAMPLES TAKEN ALONG THE PROPOSED ROADWAYS. TEST BORINGS SHALL BE PLACED AT A MAXIMUM SPACING OF 500 FEET OR OTHER SAMPLING FREQUENCY APPROVED BY THE COUNTY ENGINEER BASED ON RECOMMENDATIONS PROVIDED BY THE GEOTECHNICAL ENGINEER. THE SOILS REPORT AND PAVEMENT DESIGN SHALL BE SUBMITTED TO THE COUNTY ENGINEER FOR REVIEW. THE PAVEMENT DESIGN MUST BE APPROVED BY THE COUNTY ENGINEER PRIOR TO OR CONCURRENTLY WITH THE REVIEW AND APPROVAL OF THE CONSTRUCTION PLANS. IN ADDITION TO THE BASIS OF THE PAVEMENT DESIGN, THE SOILS REPORT SHALL CONTAIN THE RESULTS OF SAMPLED AND TESTED SUBGRADE FOR PLASTICITY INDEX, PH, SULFATE CONTENT, AND MAXIMUM DENSITY.

- 1. THE PREPARATION OF THE SUBGRADE SHALL FOLLOW GOOD ENGINEERING PRACTICES AS DIRECTED BY THE COUNTY ENGINEER IN CONJUNCTION WITH RECOMMENDATIONS OUTLINED IN THE GEOTECHNICAL REPORT. WHEN THE PLASTICITY INDEX (PI) IS GREATER THAN 20, A SUFFICIENT AMOUNT OF LIME SHALL BE ADDED AS DESCRIBED IN ITEM 260 OF THE CURRENT EDITION OF THE TXDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION UNTIL THE PI IS LESS THAN 20. IF THE ADDITION OF LIME AS DESCRIBED IN ITEM 260 IS NOT FEASIBLE, AN ALTERNATE STABILIZING DESIGN SHALL BE PROPOSED AND SUBMITTED TO THE COUNTY ENGINEER FOR APPROVAL. THE SUBGRADE SHALL BE PREPARED AND COMPACTED TO ACHIEVE A DRY DENSITY PER TXDOT ITEM 132. IN ADDITION, PROOF ROLLING MAY BE REQUIRED BY THE COUNTY ENGINEER
- 2. THE SUBGRADE SHALL BE INSPECTED AND APPROVED BY AN INDEPENDENT TESTING LABORATORY AND A CERTIFIED COPY OF ALL INSPECTION REPORTS FURNISHED TO THE COUNTY ENGINEER, WHO MUST APPROVE THE REPORT PRIOR TO APPLICATION OF THE BASE MATERIAL. ALL DENSITY TEST REPORTS SHALL INCLUDE A COPY OF THE WORK SHEET SHOWING THE PERCENTAGE OF THE MAXIMUM DRY (PROCTOR) DENSITY. THE NUMBER AND LOCATION OF ALL SUBGRADE TESTS SHALL BE DETERMINED BY THE COUNTY

- 1. BASE MATERIAL SHALL CONFORM TO ITEM 247 OF THE CURRENT EDITION OF THE TXDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION, "FLEXIBLE BASE". THE BASE MATERIAL SHALL BE TYPE A GRADE 1, TYPE A GRADE 2, OR AS APPROVED BY THE COUNTY
- 2. EACH LAYER OF BASE COURSE SHALL BE TESTED FOR IN-PLACE DRY DENSITY AND MEASURED FOR COMPACTED THICKNESS. THE NUMBER AND LOCATION OF ALL BASE TEST SAMPLES SHALL BE DETERMINED BY THE COUNTY ENGINEER.
- 3. THE BASE SHALL BE PREPARED AND COMPACTED TO ACHIEVE A MINIMUM OF 98% OF THE MAXIMUM (PROCTOR) DRY DENSITY OR AS APPROVED BY THE COUNTY ENGINEER UPON RECOMMENDATION BY THE TESTING LABORATORY. THE MAXIMUM LIFT SHALL NOT EXCEED SIX INCHES. THE BASE MUST BE INSPECTED AND APPROVED BY AN INDEPENDENT TESTING LABORATORY AND A CERTIFIED COPY OF THE TEST RESULTS FURNISHED TO THE COUNTY ENGINEER FOR APPROVAL. PRIOR TO THE PLACEMENT OF THE FIRST LIFT OF BASE, THE STOCKPILE SHALL BE TESTED FOR THE SPECIFICATIONS FOUND IN ITEM 247 TABLE 1 AND THE RESULT FURNISHED TO THE COUNTY ENGINEER FOR APPROVAL.

- 1. URBAN ROADS REQUIRE A MINIMUM 2 INCH WEARING SURFACE OF HMAC TXDOT TYPE D. THE MIX SHALL BE FROM A TXDOT CERTIFIED PLANT. THE MIX DESIGN SHALL BE SUBMITTED TO THE COUNTY ENGINEER FOR APPROVAL PRIOR TO PLACEMENT OF THE MATERIAL. CONTRACTOR'S QUALITY CONTROL (CQC) TEST REPORTS SHALL BE SUBMITTED TO THE COUNTY ENGINEER ON A DAILY BASIS. AS A MINIMUM, DAILY CQC TESTING ON THE PRODUCED MIX SHALL INCLUDE: SIEVE ANALYSIS TEX-200-F, ASPHALT CONTENT TEX-210-F, HVEEM STABILITY TEX-208-F, LABORATORY COMPACTED DENSITY TEX-207-F, AND MAXIMUM SPECIFIC GRAVITY TEX-227-F. THE NUMBER AND LOCATION OF ALL HMAC TESTS SHALL BE DETERMINED BY THE COUNTY ENGINEER WITH A MINIMUM OF THREE, 6-INCH DIAMETER FIELD CORES SECURED AND TESTED BY THE CONTRACTOR FROM EACH DAY'S PAVING. EACH HMAC COURSE SHALL BE TESTED FOR IN-PLACE DENSITY, BITUMINOUS CONTENT AND AGGREGATE GRADATION, AND SHALL BE MEASURED FOR COMPACTED THICKNESS. THE NUMBER AND LOCATION OF ALL HMAC TEST SAMPLES SHALL BE DETERMINED BY THE COUNTY ENGINEER.
- 2. RURAL ROADS MAY USE EITHER THE SPECIFICATIONS FOUND IN SECTION B7.1 OR A TWO-COURSE SURFACE IN ACCORDANCE WITH ITEM 316, TREATMENT WEARING SURFACE, OF THE CURRENT EDITION OF THE TXDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION. THE TYPE AND RATE OF ASPHALT AND AGGREGATE SHALL BE INDICATED ON THE PLANS AS A BASIS OF ESTIMATE AND SHALL BE DETERMINED AT THE PRECONSTRUCTION CONFERENCE. AGGREGATE USED IN THE MIX SHALL BE ON THE TXDOT QUALITY MONITORING SCHEDULE. AGGREGATE SHALL BE TYPE B GRADE 4. GRADATION TESTS SHALL BE REQUIRED FOR EACH 300 CUBIC YARDS OF MATERIAL PLACED WITH A MINIMUM OF TWO TESTS PER EACH GRADE PER EACH PROJECT. TEST RESULTS SHALL BE REVIEWED BY THE COUNTY ENGINEER PRIOR TO APPLICATION OF THE MATERIAL.

IN LIEU OF BITUMINOUS PAVEMENT, PORTLAND CEMENT CONCRETE PAVEMENT MAY BE USED. IN SUCH CASES, THE PAVEMENT THICKNESS SHALL BE A MINIMUM OF 9 INCHES OF CONCRETE, AND SHALL BE JOINTED AND REINFORCED IN ACCORDANCE WITH THE DETAIL INCLUDED IN APPENDIX J. THE MIX SHALL BE FROM A TXDOT CERTIFIED PLANT. THE MIX DESIGN SHALL BE SUBMITTED TO THE COUNTY ENGINEER FOR APPROVAL PRIOR TO PLACEMENT OF THE MATERIAL.

- 1. UNLESS OTHERWISE SPECIFIED, CONCRETE SHALL BE IN ACCORDANCE WITH ITEM 421 OF THE CURRENT EDITION OF THE TXDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION AND BE PLACED IN ACCORDANCE WITH THE APPLICABLE ITEM.
- 2. ALL CONCRETE SHALL BE TESTED FOR COMPRESSIVE STRENGTH. ONE SET OF THREE CONCRETE TEST CYLINDERS SHALL BE MOLDED FOR EVERY 50 CUBIC YARDS OF CONCRETE PLACED FOR EACH CLASS OF CONCRETE PER DAY, OR AT ANY OTHER INTERVAL AS DETERMINED BY THE COUNTY ENGINEER. A SLUMP TEST SHALL BE REQUIRED WITH EACH SET OF TEST CYLINDERS. ONE CYLINDER SHALL BE TESTED FOR COMPRESSIVE STRENGTH AT AN AGE OF SEVEN DAYS AND THE REMAINING TWO CYLINDERS SHALL BE TESTED AT 28 DAYS OF AGE.

- 1. ALL ROADS SHALL BE NAMED, WITH PRIOR APPROVAL FOR SAID NAME FROM THE WILLIAMSON COUNTY 911 ADDRESSING COORDINATOR. ROADS MUST BE NAMED IN A MANNER TO AVOID CONFUSION IN IDENTIFICATION. ROADS THAT ARE EXTENSIONS OF EXISTING ROADS MUST CARRY THE NAMES OF THOSE IN EXISTENCE. ROADS THAT ARE NOT CONTINUOUS, OR WHICH HAVE 90 DEGREE TURNS, SHALL HAVE DIFFERENT NAMES. THE OWNER SHALL PROVIDE THE COORDINATOR WITH TWO DIGITAL FILES OF THE PLAT. ONE FILE SHALL BE IN AN ADOBE .PDF FORMAT, AND THE OTHER FILE SHALL BE IN AN AUTOCAD .DWG FORMAT GEOREFERENCED TO NAD 1983 STATE PLANE GRID COORDINATE SYSTEM, TEXAS CENTRAL ZONE (4203), WITH DRAWING UNITS OF US FEET. THE ROAD NAMES SHALL BE DISPLAYED ON STANDARD INTERSECTION ROAD MARKER SIGNS ERECTED BY THE OWNER IN COMPLIANCE WITH THE TXMUTCD "STREET NAME SIGNS" AND AT THE LOCATIONS AS INDICATED ON THE CONSTRUCTION PLANS.
- 2. TRAFFIC CONTROL SIGNS (SUCH AS STOP, YIELD, AND SPEED LIMIT SIGNS) SHALL BE INSTALLED BY THE OWNER OF SAID SUBDIVISION IN COMPLIANCE WITH THE TXMUTCD AND AT THE LOCATIONS AS INDICATED ON THE APPROVED CONSTRUCTION PLANS. OTHER TRAFFIC CONTROL SIGNS, AS SHOWN ON THE CONSTRUCTION PLANS, SHALL BE INSTALLED TO INDICATE ANY UNUSUAL TRAFFIC OR ROAD HAZARD OR CONDITIONS THAT MAY EXIST. ALL TRAFFIC CONTROL DEVICES SHALL BE PLACED IN COMPLIANCE WITH THE TXMUTCD AND THE CONSTRUCTION COST SHALL BE BORNE BY THE OWNER.
- 3. A SPEED LIMIT OF 25 MPH FOR LOCAL ROADS, 30 MPH FOR COLLECTOR ROADS AND 40 MPH FOR ARTERIAL ROADS WITHIN ALL PLATTED SUBDIVISIONS IS HEREBY ADOPTED. THIS LIMIT MAY BE CHANGED ONLY BY COMMISSIONER'S COURT UPON THE BASIS OF AN ENGINEERING AND TRAFFIC INVESTIGATION SHOWING THAT THE PRIMA FACIE MAXIMUM REASONABLE AND PRUDENT SPEED FOR A PARTICULAR ROAD (OR PART OF A ROAD) SHOULD BE DIFFERENT. THE PLACEMENT OF A STOP SIGN OR A YIELD SIGN ON THE MINOR ROAD AT INTERSECTIONS SHALL BE EVALUATED ON A CASE—BY—CASE BASIS IN ACCORDANCE WITH THE TXMUTCD.
- 4. FOR ANY ROAD THAT IS PROPOSED TO BE EXTENDED AT SOME TIME IN THE FUTURE, A MINIMUM OF FIVE METAL CHANNEL POSTS. EQUALLY SPACED, SHALL BE PLACED AT THE END OF THE ROAD. EACH POST SHALL HAVE AN 18"X18" RED DIAMOND OBJECT MARKER SIGN (TYPE OM-4 PER TXMUTCD) PLACED FOUR FEET ABOVE THE EXISTING GROUND.
- 5. A FUTURE ROAD EXTENSION SIGN SHALL BE PLACED AT THE END OF ALL ROADS AND TEMPORARY CUL-DE-SACS THAT ARE PROPOSED TO BE EXTENDED AT SOME TIME IN THE FUTURE. THE SIGN SHALL STATE THE FOLLOWING: FUTURE EXTENSION OF <NAME OF
- 6. SIGNAGE THAT DIFFERS FROM THE STANDARD SIGNAGE THAT IS MAINTAINED BY THE COUNTY SHALL BE MAINTAINED BY THE OWNER. THE SIGNAGE SHALL BE MAINTAINED IN SUCH A FASHION TO COMPLY WITH THE TXMUTCD REQUIREMENTS.

DRAINAGE AND FLOOD CONTROL:

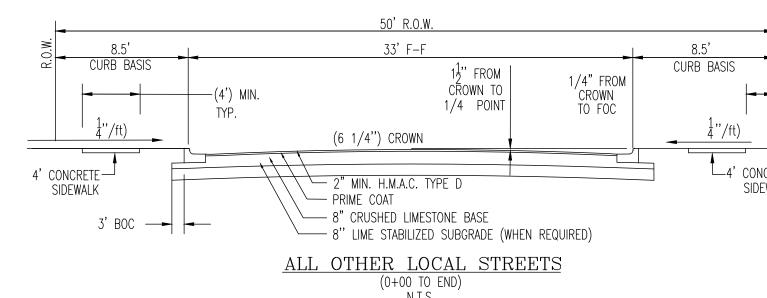
- . STORM WATER MANAGEMENT CONTROLS, WHEN NEEDED, SHALL BE DESIGNED, CONSTRUCTED AND MAINTAINED TO RESTRICT THE RATE OF DRAINAGE FROM THE PLATTED AREA TO THE RATE OF DRAINAGE OF THE LAND IN ITS NATURAL STATE. WHEN A DEVELOPMENT SHALL HAVE SEVERAL SECTIONS, STORM WATER MANAGEMENT CONTROLS FOR THE ULTIMATE DEVELOPED AREA SHALL BE CONSTRUCTED IF NOT LOCATED IN THE FIRST PLATTED SECTION. STORM WATER MANAGEMENT CONTROLS ARE TO BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER USING A BASIS OF A 2, 10, AND 100-YEAR STORM. EXCEPTIONS MAY BE ALLOWED WHEN THE OWNER CAN DEMONSTRATE THAT DOWNSTREAM PROPERTY SHALL NOT BE ADVERSELY AFFECTED.
- 2. DRAINAGE CALCULATIONS SHALL BE MADE USING THE EDITION OF THE CITY OF AUSTIN'S DRAINAGE CRITERIA MANUAL IN EFFECT AS OF THE DATE OF THESE REGULATIONS OR OTHER METHODS SATISFACTORY TO THE COUNTY ENGINEER. ALL DATA AND CALCULATIONS MUST BE PRESENTED TO THE COUNTY ENGINEER AS PART OF THE CONSTRUCTION PLANS. THE FOLLOWING REQUIREMENTS SHALL BE INCORPORATED INTO THE DESIGN: (a) BRIDGES AND CROSS DRAINAGE STRUCTURES FOR ARTERIAL AND COLLECTOR ROADS SHALL BE DESIGNED TO CONVEY THE 25-YEAR STORM WITHOUT OVERTOPPING THE FACILITY.
- (b) BRIDGES AND CROSS DRAINAGE STRUCTURES FOR LOCAL ROADS SHALL BE DESIGNED TO CONVEY THE 10-YEAR STORM WITHOUT OVERTOPPING THE FACILITY. (c) ALL LONGITUDINAL DRAINAGE STRUCTURES SHALL BE DESIGNED TO CONVEY THE 10-YEAR STORM.

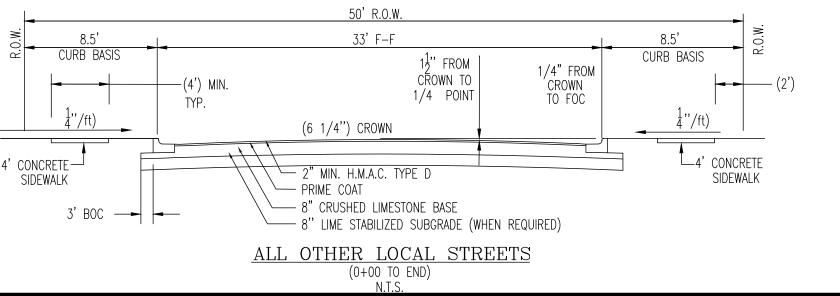
DRIVEWAY CULVERT PIPE(S). THE SIDE SLOPES OF THE DITCHES ARE TO BE 3:1 OR FLATTER.

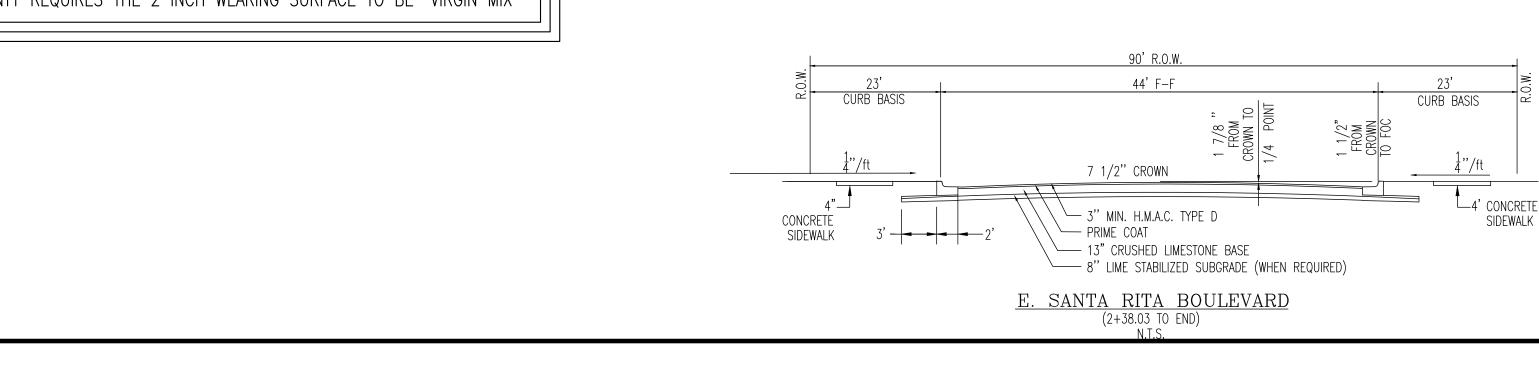
- (d) ON CURB AND GUTTER ROADWAYS, THE ROADWAY SHALL BE DESIGNED SO THAT NO MORE THAN ONE HALF OF ONE TRAVEL LANE SHALL BE INUNDATED BY THE 10 YEAR STORM.
- 3. ALL DRAINAGE STRUCTURES AND APPURTENANCES SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER. A PROFILE SHALL BE SHOWN IN THE CONSTRUCTION PLANS FOR ALL DRAINAGE STRUCTURES. EACH PROFILE SHALL SHOW THE DESIGN FLOW, VELOCITY, INVERT ELEVATIONS, AND THE HYDRAULIC GRADE LINE.
- 4. DRIVEWAY CULVERTS SHALL HAVE A MINIMUM INTERIOR DIAMETER OF 18" OR EQUAL AND A MINIMUM LENGTH OF 22 FEET, AND SHALL INCLUDE A CONCRETE APRON SAFETY TREATMENT IN ACCORDANCE WITH CITY OF AUSTIN STANDARD DETAIL 508S-20, "STORMDRAIN OUTFALL PROTECTION CULVERT UNDER ROADWAY/INLINE". LARGER OR LONGER CULVERTS SHALL BE INSTALLED IF NECESSARY TO ACCOMMODATE DRAINAGE BASED UPON A 10-YEAR FLOW FREQUENCY. ALL DRIVEWAY CULVERTS SHALL HAVE SAFETY END TREATMENTS WITH CONCRETE APRONS.
- 5. AT SOME POINT WITHIN THE FIRST TEN FEET FROM THE EDGE OF THE ROADWAY GUTTER, THE ENTIRE WIDTH OF A DRIVEWAY SHALL HAVE THE SAME OR GREATER ELEVATION AS THE TOP OF THE CURB AT THE EDGE OF THE ROADWAY.
- 6. MAINTENANCE RESPONSIBILITY FOR DRAINAGE WILL NOT TO BE ACCEPTED BY THE COUNTY OTHER THAN THAT ACCEPTED IN CONNECTION WITH DRAINING OR PROTECTING THE ROAD SYSTEM. MAINTENANCE RESPONSIBILITY FOR STORM WATER MANAGEMENT CONTROLS WILL REMAIN WITH THE OWNER.
- UTILITIES EASEMENT". IN GENERAL, A "DRAINAGE EASEMENT" SHALL BE A MINIMUM OF 20 FEET IN WIDTH AND A "DRAINAGE AND UNDERGROUND UTILITIES EASEMENT" SHALL BE A MINIMUM OF 30 FEET IN WIDTH. 8. ALL ROADSIDE DITCHES SHALL HAVE A MINIMUM DEPTH, AS MEASURED FROM THE EDGE OF THE ROAD PAVEMENT, EQUAL TO THE DIAMETER OF THE DRIVEWAY CULVERT PIPE(S) PLUS NINE INCHES, AND A BOTTOM WIDTH EQUAL TO THE DIAMETER OF THE

7. EASEMENTS SHALL BE PROVIDED, WHERE NECESSARY, FOR ALL DRAINAGE COURSES IN AND ACROSS PROPERTY TO BE PLATTED. THE LOCATION AND WIDTH SHALL BE SHOWN ON THE PLAT AND MARKED "DRAINAGE EASEMENT" OR "DRAINAGE AND UNDERGROUND

9. ROADSIDE DITCHES MAY BE ELIMINATED WITHIN A RURAL SUBDIVISION PROVIDED THAT THE ROAD HAS 18-INCH RIBBON CURBS, THE ROADWAY SURFACE HAS AN ADEQUATE CROSS SLOPE, AND THE OVERALL DRAINAGE PATTERNS THROUGHOUT THE SUBDIVISION REMAIN AS IN AN UNDEVELOPED STATE.







residential confectors | Subgrade PI > 20 | c.0 | 1c | 0 1. Where the subgrade is comprised of limestone or low PI clay (PI < 20), lime stabilization may be omitted. curing conditions required. 4. These pavement thickness designs are intended to transfer the load from the anticipated traffic conditions. 5. The responsibility of assigning street classification to the streets in this project is left to the civil engineer. ENGINEERS & CONSULTANTS ADJUSTMENTS THAT ARE REQUIRED SHALL BE MADE THROUGH REVISION OF THE CONSTRUCTION PLANS.

8. WHERE PI'S ARE OVER 20, SUBGRADES MUST BE STABILIZED UTILIZING A METHOD ACCEPTABLE TO THE COUNTY ENGINEER. THE GEOTECHNICAL ENGINEER SHALL RECOMMEND AN

PER THE WILLIAMSON COUNTY ENGINEER, THERE MUST BE A TXDOT HMAC WEARTING SURFACE 340 TYPE D, WITHOUT RAP OR RAZ. WILLIAMSON

GENER

* STEVEN P. CATES 93040 VCENSED CARLSON, BRIGANCE & DOERING, INC ID# F3791

two . Cate 12-1-2023 OCT 2023 B NUMBER 5413

OF 73 HEET NO.

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY WATER POLLUTION ABATEMENT PLAN GENERAL CONSTRUCTION NOTES

EDWARDS AQUIFER PROTECTION PROGRAM CONSTRUCTION NOTES - LEGAL DISCLAIME

THE FOLLOWING/LISTED "CONSTRUCTION NOTES" ARE INTENDED TO BE ADVISORY IN NATURE ONLY AND DO NOT CONSTITUTE AN APPROVAL OR CONDITIONAL APPROVAL BY THE EXECUTIVE DIRECTOR (ED), NOR DO THEY CONSTITUTE A COMPREHENSIVE LISTING OF RULES OR CONDITIONS TO BE FOLLOWED DURING CONSTRUCTION. FURTHER ACTIONS MAY BE REQUIRED TO ACHIEVE COMPLIANCE WITH TCEQ REGULATIONS FOUND IN TITLE 30, TEXAS ADMINISTRATIVE CODE (TAC), CHAPTERS 213 AND 217, AS WELL AS LOCAL ORDINANCES AND REGULATIONS PROVIDING FOR THE PROTECTION OF WATER QUALITY. ADDITIONALLY, NOTHING CONTAINED IN THE FOLLOWING/LISTED "CONSTRUCTION NOTES" RESTRICTS THE POWERS OF THE ED, THE COMMISSION OR ANY OTHER GOVERNMENTAL ENTITY TO PREVENT, CORRECT, OR CURTAIL ACTIVITIES THAT RESULT OR MAY RESULT IN POLLUTION OF THE EDWARDS AQUIFER OR HYDROLOGICALLY CONNECTED SURFACE WATERS. THE HOLDER OF ANY EDWARDS AQUIFER PROTECTION PLAN CONTAINING "CONSTRUCTION NOTES" IS STILL RESPONSIBLE FOR COMPLIANCE WITH TITLE 30, TAC, CHAPTERS 213 OR ANY OTHER APPLICABLE TCEQ REGULATION, AS WELL AS ALL CONDITIONS OF AN EDWARDS AQUIFER PROTECTION PLAN THROUGH ALL PHASES OF PLAN IMPLEMENTATION. FAILURE TO COMPLY WITH ANY CONDITION OF THE ED'S APPROVAL, WHETHER OR NOT IN CONTRADICTION OF ANY "CONSTRUCTION NOTES," IS A VIOLATION OF TCEQ REGULATIONS AND ANY VIOLATION IS SUBJECT TO ADMINISTRATIVE RULES, ORDERS, AND PENALTIES AS PROVIDED UNDER TITLE 30, TAC § 213.10 (RELATING TO ENFORCEMENT). SUCH VIOLATIONS MAY ALSO BE SUBJECT TO CIVIL PENALTIES AND INJUNCTION. THE FOLLOWING/LISTED "CONSTRUCTION NOTES" IN NO WAY REPRESENT AN APPROVED EXCEPTION BY THE ED TO ANY PART OF TITLE 30 TAC, CHAPTERS 213 AND 217, OR ANY OTHER TCEQ APPLICABLE REGULATION

- A WRITTEN NOTICE OF CONSTRUCTION MUST BE SUBMITTED TO THE TCEQ REGIONAL OFFICE AT LEAST 48 HOURS PRIOR TO THE START OF ANY REGULATED ACTIVITIES. THIS NOTICE MUST INCLUDE:
 - THE NAME OF THE APPROVED PROJECT;
 - THE ACTIVITY START DATE; AND
 THE CONTACT INFORMATION OF THE PRIME CONTRACTOR.
- 2. ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROJECT MUST BE PROVIDED WITH COMPLETE COPIES OF THE APPROVED WATER POLLUTION ABATEMENT PLAN (WPAP) AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS ARE REQUIRED TO KEEP ON-SITE COPIES OF THE APPROVED PLAN AND APPROVAL LETTER.
- IF ANY SENSITIVE FEATURE(S) (CAVES, SOLUTION CAVITY, SINK HOLE, ETC.) IS DISCOVERED DURING CONSTRUCTION, ALL REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE SUSPENDED IMMEDIATELY. THE APPROPRIATE TCEQ REGIONAL OFFICE MUST BE IMMEDIATELY NOTIFIED OF ANY SENSITIVE FEATURES ENCOUNTERED DURING CONSTRUCTION. CONSTRUCTION ACTIVITIES MAY NOT BE RESUMED UNTIL THE TCEQ HAS REVIEWED AND APPROVED THE APPROPRIATE PROTECTIVE MEASURES IN ORDER TO PROTECT ANY SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM POTENTIALLY ADVERSE IMPACTS TO WATER
- 4. NO TEMPORARY OR PERMANENT HAZARDOUS SUBSTANCE STORAGE TANK SHALL BE INSTALLED WITHIN 150 FEET OF A WATER SUPPLY SOURCE, DISTRIBUTION SYSTEM, WELL, OR SENSITIVE FEATURE.
- PRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITY, ALL TEMPORARY EROSION AND SEDIMENTATION (E&S) CONTROL MEASURES MUST BE PROPERLY INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE APPROVED PLANS AND MANUFACTURERS SPECIFICATIONS. IF INSPECTIONS INDICATE A CONTROL HAS BEEN USED INAPPROPRIATELY, OR INCORRECTLY, THE APPLICANT MUST REPLACE OR MODIFY THE CONTROL FOR SITE SITUATIONS. THESE CONTROLS MUST REMAIN IN PLACE UNTIL THE DISTURBED AREAS HAVE BEEN PERMANENTLY STABILIZED.
- 6. ANY SEDIMENT THAT ESCAPES THE CONSTRUCTION SITE MUST BE COLLECTED AND PROPERLY DISPOSED OF BEFORE THE NEXT RAIN EVENT TO ENSURE IT IS NOT WASHED INTO SURFACE STREAMS, SENSITIVE FEATURES, ETC.
- 7. SEDIMENT MUST BE REMOVED FROM THE SEDIMENT TRAPS OR SEDIMENTATION BASINS NOT LATER THAN WHEN IT OCCUPIES 50% OF THE BASIN'S DESIGN CAPACITY.
- 8. LITTER, CONSTRUCTION DEBRIS, AND CONSTRUCTION CHEMICALS EXPOSED TO STORMWATER SHALL BE PREVENTED FROM BEING
- 9. ALL SPOILS (EXCAVATED MATERIAL) GENERATED FROM THE PROJECT SITE MUST BE STORED ON-SITE WITH PROPER E&S CONTROLS. FOR STORAGE OR DISPOSAL OF SPOILS AT ANOTHER SITE ON THE EDWARDS AQUIFER RECHARGE ZONE, THE OWNER OF THE SITE MUST RECEIVE APPROVAL OF A WATER POLLUTION ABATEMENT PLAN FOR THE PLACEMENT OF FILL MATERIAL OR MASS GRADING PRIOR TO THE PLACEMENT OF SPOILS AT THE OTHER SITE.
- 10. IF PORTIONS OF THE SITE WILL HAVE A TEMPORARY OR PERMANENT CEASE IN CONSTRUCTION ACTIVITY LASTING LONGER THAN 14 DAYS, SOIL STABILIZATION IN THOSE AREAS SHALL BE INITIATED AS SOON AS POSSIBLE PRIOR TO THE 14TH DAY OF INACTIVITY. IF ACTIVITY WILL RESUME PRIOR TO THE 21ST DAY, STABILIZATION MEASURES ARE NOT REQUIRED. IF DROUGHT CONDITIONS OR INCLEMENT WEATHER PREVENT ACTION BY THE 14TH DAY, STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS POSSIBLE.
- 11. THE FOLLOWING RECORDS SHALL BE MAINTAINED AND MADE AVAILABLE TO THE TCEQ UPON REQUEST:
 - THE DATES WHEN MAJOR GRADING ACTIVITIES OCCUR;

TCEQ-0592 (REV. JULY 15, 2015)

- THE DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE: AND
- THE DATES WHEN STABILIZATION MEASURES ARE INITIATED.
- 12. THE HOLDER OF ANY APPROVED EDWARD AQUIFER PROTECTION PLAN MUST NOTIFY THE APPROPRIATE REGIONAL OFFICE IN WRITING AND OBTAIN APPROVAL FROM THE EXECUTIVE DIRECTOR PRIOR TO INITIATING ANY OF THE FOLLOWING:
- A. ANY PHYSICAL OR OPERATIONAL MODIFICATION OF ANY WATER POLLUTION ABATEMENT STRUCTURE(S), INCLUDING BUT NOT LIMITED TO PONDS, DAMS, BERMS, SEWAGE TREATMENT PLANTS, AND DIVERSIONARY STRUCTURES;
- B. ANY CHANGE IN THE NATURE OR CHARACTER OF THE REGULATED ACTIVITY FROM THAT WHICH WAS ORIGINALLY APPROVED OR A CHANGE WHICH WOULD SIGNIFICANTLY IMPACT THE ABILITY OF THE PLAN TO PREVENT POLLUTION OF THE EDWARDS ACLIFER.
- C. ANY DEVELOPMENT OF LAND PREVIOUSLY IDENTIFIED AS UNDEVELOPED IN THE ORIGINAL WATER POLLUTION ABATEMENT PLAN

AUSTIN REGIONAL OFFICE 12100 PARK 35 CIRCLE, BUILDING A AUSTIN, TEXAS 78753-1808 PHONE (512) 339-2929 FAX (512) 339-3795

SAN ANTONIO REGIONAL OFFICE 14250 JUDSON ROAD SAN ANTONIO, TEXAS 78233-4480 PHONE (210) 490-3096

THESE GENERAL CONSTRUCTION NOTES MUST BE INCLUDED ON THE CONSTRUCTION PLANS PROVIDED TO THE CONTRACTOR AND ALL SUBCONTRACTORS.

FAX (210) 545-4329

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY ORGANIZED SEWAGE COLLECTION SYSTEM GENERAL CONSTRUCTION NOTES

EDWARDS AQUIFER PROTECTION PROGRAM CONSTRUCTION NOTES - LEGAL DISCLAIMER

THE FOLLOWING/LISTED "CONSTRUCTION NOTES" ARE INTENDED TO BE ADVISORY IN NATURE ONLY AND DO NOT CONSTITUTE AN APPROVAL OR CONDITIONAL APPROVAL BY THE EXECUTIVE DIRECTOR, NOR DO THEY CONSTITUTE A COMPREHENSIVE LISTING OF RULES OR CONDITIONS TO BE FOLLOWED DURING CONSTRUCTION. FURTHER ACTIONS MAY BE REQUIRED TO ACHIEVE COMPLIANCE WITH TCEQ REGULATIONS FOUND IN TITLE 30, TEXAS ADMINISTRATIVE CODE, CHAPTERS 213 AND 217, AS WELL AS LOCAL ORDINANCES AND REGULATIONS PROVIDING FOR THE PROTECTION OF WATER QUALITY. ADDITIONALLY, NOTHING CONTAINED IN THE FOLLOWING/LISTED "CONSTRUCTION NOTES" RESTRICTS THE POWERS OF THE EXECUTIVE DIRECTOR, THE COMMISSION OR ANY OTHER GOVERNMENTAL ENTITY TO PREVENT, CORRECT, OR CURTAIL ACTIVITIES THAT RESULT OR MAY RESULT IN POLLUTION OF THE EDWARDS AQUIFER OR HYDROLOGICALLY CONNECTED SURFACE WATERS. THE HOLDER OF ANY EDWARDS AQUIFER PROTECTION PLAN CONTAINING "CONSTRUCTION NOTES" IS STILL RESPONSIBLE FOR COMPLIANCE WITH TITLE 30, TEXAS ADMINISTRATIVE CODE, CHAPTERS 213 OR ANY OTHER APPLICABLE TCEQ REGULATION, AS WELL AS ALL CONDITIONS OF AN EDWARDS AQUIFER PROTECTION PLAN THROUGH ALL PHASES OF PLAN IMPLEMENTATION. FAILURE TO COMPLY WITH ANY CONDITION OF THE EXECUTIVE DIRECTOR'S APPROVAL, WHETHER OR NOT IN CONTRADICTION OF ANY "CONSTRUCTION NOTES," IS A VIOLATION OF TCEQ REGULATIONS AND ANY VIOLATIONS AND ANY VIOLATION SUBJECT TO ADMINISTRATIVE RULES, ORDERS, AND PENALTIES AS PROVIDED UNDER TITLE 30, TEXAS ADMINISTRATIVE CODE \$ 213.10 (RELATING TO ENFORCEMENT). SUCH VIOLATIONS MAY ALSO BE SUBJECT TO CIVIL PENALTIES AND INJUNCTION. THE FOLLOWING/LISTED "CONSTRUCTION NOTES" IN NO WAY REPRESENT AN APPROVED EXCEPTION BY THE EXECUTIVE DIRECTOR TO ANY PART OF TITLE 30 TEXAS ADMINISTRATIVE CODE. CHAPTERS 213 AND 217. OR ANY OTHER TCEQ APPLICABLE REGULATION.

- 1. THIS ORGANIZED SEWAGE COLLECTION SYSTEM (SCS) MUST BE CONSTRUCTED IN ACCORDANCE WITH 30 TEXAS ADMINISTRATIVE CODE (TAC) §213.5(C), THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY'S (TCEQ) EDWARDS AQUIFER RULES AND ANY LOCAL GOVERNMENT STANDARD SPECIFICATIONS.
- 2. ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROPOSED REGULATED PROJECT MUST BE PROVIDED WITH COPIES OF THE SCS PLAN AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS MUST BE REQUIRED TO KEEP ON-SITE COPIES OF THE PLAN AND THE APPROVAL LETTER.
- A WRITTEN NOTICE OF CONSTRUCTION MUST BE SUBMITTED TO THE PRESIDING TCEQ REGIONAL OFFICE AT LEAST 48 HOURS PRIOR TO THE START OF
 ANY REGULATED ACTIVITIES. THIS NOTICE MUST INCLUDE:
 THE NAME OF THE APPROVED PROJECT;
- THE ACTIVITY START DATE; AND
 THE CONTACT INFORMATION OF THE PRIME CONTRACTOR.
- 4. ANY MODIFICATION TO THE ACTIVITIES DESCRIBED IN THE REFERENCED SCS APPLICATION FOLLOWING THE DATE OF APPROVAL MAY REQUIRE THE SUBMITTAL OF AN SCS APPLICATION TO MODIFY THIS APPROVAL, INCLUDING THE PAYMENT OF APPROPRIATE FEES AND ALL INFORMATION NECESSARY FOR ITS REVIEW AND APPROVAL.
- 5. PRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITY, ALL TEMPORARY EROSION AND SEDIMENTATION (E&S) CONTROL MEASURES MUST BE PROPERLY INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS. THESE CONTROLS MUST REMAIN IN PLACE UNTIL THE DISTURBED AREAS HAVE BEEN PERMANENTLY STABILIZED.
- IF ANY SENSITIVE FEATURES ARE DISCOVERED DURING THE WASTEWATER LINE TRENCHING ACTIVITIES, ALL REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE SUSPENDED IMMEDIATELY. THE APPLICANT MUST IMMEDIATELY NOTIFY THE APPROPRIATE REGIONAL OFFICE OF THE TCEQ OF THE FEATURE DISCOVERED. A GEOLOGIST'S ASSESSMENT OF THE LOCATION AND EXTENT OF THE FEATURE DISCOVERED MUST BE REPORTED TO THAT REGIONAL OFFICE IN WRITING AND THE APPLICANT MUST SUBMIT A PLAN FOR ENSURING THE STRUCTURAL INTEGRITY OF THE SEWER LINE OR FOR MODIFYING THE PROPOSED COLLECTION SYSTEM ALIGNMENT AROUND THE FEATURE. THE REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MAY NOT PROCEED UNTIL THE EXECUTIVE DIRECTOR HAS REVIEWED AND APPROVED THE METHODS PROPOSED TO PROTECT THE SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM ANY POTENTIALLY ADVERSE IMPACTS TO WATER QUALITY WHILE MAINTAINING THE STRUCTURAL INTEGRITY OF THE LINE.
- 7. SEWER LINES LOCATED WITHIN OR CROSSING THE 5-YEAR FLOODPLAIN OF A DRAINAGE WAY WILL BE PROTECTED FROM INUNDATION AND STREAM VELOCITIES WHICH COULD CAUSE EROSION AND SCOURING OF BACKFILL. THE TRENCH MUST BE CAPPED WITH CONCRETE TO PREVENT SCOURING OF BACKFILL, OR THE SEWER LINES MUST BE ENCASED IN CONCRETE. ALL CONCRETE SHALL HAVE A MINIMUM THICKNESS OF 6 INCHES.
- 8. BLASTING PROCEDURES FOR PROTECTION OF EXISTING SEWER LINES AND OTHER UTILITIES WILL BE IN ACCORDANCE WITH THE NATIONAL FIRE PROTECTION ASSOCIATION CRITERIA. SAND IS NOT ALLOWED AS BEDDING OR BACKFILL IN TRENCHES THAT HAVE BEEN BLASTED. IF ANY EXISTING SEWER LINES ARE DAMAGED, THE LINES MUST BE REPAIRED AND RETESTED.
- 9. ALL MANHOLES CONSTRUCTED OR REHABILITATED ON THIS PROJECT MUST HAVE WATERTIGHT SIZE ON SIZE RESILIENT CONNECTORS ALLOWING FOR DIFFERENTIAL SETTLEMENT. IF MANHOLES ARE CONSTRUCTED WITHIN THE 100-YEAR FLOODPLAIN, THE COVER MUST HAVE A GASKET AND BE BOLTED TO THE RING. WHERE GASKETED MANHOLE COVERS ARE REQUIRED FOR MORE THAN THREE MANHOLES IN SEQUENCE OR FOR MORE THAN 1500 FEET, ALTERNATE MEANS OF VENTING WILL BE PROVIDED. BRICKS ARE NOT AN ACCEPTABLE CONSTRUCTION MATERIAL FOR ANY PORTION OF THE MANHOLE.
- THE DIAMETER OF THE MANHOLES MUST BE A MINIMUM OF FOUR FEET AND THE MANHOLE FOR ENTRY MUST HAVE A MINIMUM CLEAR OPENING DIAMETER OF 30 INCHES. THESE DIMENSIONS AND OTHER DETAILS SHOWING COMPLIANCE WITH THE COMMISSION'S RULES CONCERNING MANHOLES AND SEWER LINE/MANHOLE INVERTS DESCRIBED IN 30 TAC §217.55 ARE INCLUDED ON PLAN SHEET 58 68 OF 73.

IT IS SUGGESTED THAT ENTRANCE INTO MANHOLES IN EXCESS OF FOUR FEET DEEP BE ACCOMPLISHED BY MEANS OF A PORTABLE LADDER. THE INCLUSION OF STEPS IN A MANHOLE IS PROHIBITED.

- WHERE WATER LINES AND NEW SEWER LINE ARE INSTALLED WITH A SEPARATION DISTANCE CLOSER THAN NINE FEET (I.E., WATER LINES CROSSING WASTEWATER LINES, WATER LINES PARALLELING WASTEWATER LINES, OR WATER LINES NEXT TO MANHOLES) THE INSTALLATION MUST MEET THE REQUIREMENTS OF 30 TAC §217.53(D) (PIPE DESIGN) AND 30 TAC §290.44(E) (WATER DISTRIBUTION).
- 11. WHERE SEWERS LINES DEVIATE FROM STRAIGHT ALIGNMENT AND UNIFORM GRADE ALL CURVATURE OF SEWER PIPE MUST BE ACHIEVED BY THE FOLLOWING PROCEDURE WHICH IS RECOMMENDED BY THE PIPE MANUFACTURER: THERE SHALL BE NO CURVATURE OF SANITARY SEWER LINE PIPES.

IF PIPE FLEXURE IS PROPOSED, THE FOLLOWING METHOD OF PREVENTING DEFLECTION OF THE JOINT MUST BE USED: THERE SHALL BE NO FLEXURE OF SANITARY SEER LINE PIPES.

SPECIFIC CARE MUST BE TAKEN TO ENSURE THAT THE JOINT IS PLACED IN THE CENTER OF THE TRENCH AND PROPERLY BEDDED IN ACCORDANCE WITH 30 TAC §217.54.

NEW SEWAGE COLLECTION SYSTEM LINES MUST BE CONSTRUCTED WITH STUB OUTS FOR THE CONNECTION OF ANTICIPATED EXTENSIONS. THE LOCATION OF SUCH STUB OUTS MUST BE MARKED ON THE GROUND SUCH THAT THEIR LOCATION CAN BE EASILY DETERMINED AT THE TIME OF CONNECTION OF THE EXTENSIONS. SUCH STUB OUTS MUST BE MANUFACTURED WYES OR TEES THAT ARE COMPATIBLE IN SIZE AND MATERIAL WITH BOTH THE SEWER LINE AND THE EXTENSION. AT THE TIME OF ORIGINAL CONSTRUCTION, NEW STUB-OUTS MUST BE CONSTRUCTED SUFFICIENTLY TO EXTEND BEYOND THE END OF THE STREET PAVEMENT. ALL STUB-OUTS MUST BE SEALED WITH A MANUFACTURED CAP TO PREVENT LEAKAGE. EXTENSIONS THAT WERE NOT ANTICIPATED AT THE TIME OF ORIGINAL CONSTRUCTION OR THAT ARE TO BE CONNECTED TO AN EXISTING SEWER LINE NOT FURNISHED WITH STUB OUTS MUST BE CONNECTED USING A MANUFACTURED SADDLE AND IN ACCORDANCE WITH ACCEPTED PLUMBING TECHNIQUES. IF NO STUB-OUT IS PRESENT AN ALTERNATE METHOD OF JOINING LATERALS IS SHOWN IN THE DETAIL ON PLAN SHEET OF (FOR POTENTIAL FUTURE LATERALS). (NOT APPLICABLE)

THE PRIVATE SERVICE LATERAL STUB-OUTS MUST BE INSTALLED AS SHOWN ON THE PLAN AND PROFILE SHEETS ON PLAN SHEET 73 OF 73 AND MARKED AFTER BACKFILLING AS SHOWN IN THE DETAIL ON PLAN SHEET 73 OF 73.

- 13. TRENCHING, BEDDING AND BACKFILL MUST CONFORM WITH 30 TAC §217.54. THE BEDDING AND BACKFILL FOR FLEXIBLE PIPE MUST COMPLY WITH THE STANDARDS OF ASTM D-2321, CLASSES IA, IB, II OR III. RIGID PIPE BEDDING MUST COMPLY WITH THE REQUIREMENTS OF ASTM C 12 (ANSI A 106.2) CLASSES A, B OR C.
- 14. SEWER LINES MUST BE TESTED FROM MANHOLE TO MANHOLE. WHEN A NEW SEWER LINE IS CONNECTED TO AN EXISTING STUB OR CLEAN-OUT, IT MUST BE TESTED FROM EXISTING MANHOLE TO NEW MANHOLE. IF A STUB OR CLEAN-OUT IS USED AT THE END OF THE PROPOSED SEWER LINE, NO PRIVATE SERVICE ATTACHMENTS MAY BE CONNECTED BETWEEN THE LAST MANHOLE AND THE CLEANOUT UNLESS IT CAN BE CERTIFIED AS CONFORMING WITH THE PROVISIONS OF 30 TAC §213.5(C)(3)(E).
- 15. ALL SEWER LINES MUST BE TESTED IN ACCORDANCE WITH 30 TAC §217.57. THE ENGINEER MUST RETAIN COPIES OF ALL TEST RESULTS WHICH MUST BE MADE AVAILABLE TO THE EXECUTIVE DIRECTOR UPON REQUEST. THE ENGINEER MUST CERTIFY IN WRITING THAT ALL WASTEWATER LINES HAVE PASSED ALL REQUIRED TESTING TO THE APPROPRIATE REGIONAL OFFICE WITHIN 30 DAYS OF TEST COMPLETION AND PRIOR TO USE OF THE NEW COLLECTION SYSTEM. TESTING METHOD WILL BE:

 (a. FOR A COLLECTION SYSTEM PIPE THAT WILL TRANSPORT WASTEWATER BY GRAVITY FLOW, THE DESIGN MUST SPECIFY AN INFILTRATION AND
- EXFILTRATION TEST OR A LOW-PRESSURE AIR TEST. A TEST MUST CONFORM TO THE FOLLOWING REQUIREMENTS:

 (1) LOW PRESSURE AIR TEST.

 (A) A LOW PRESSURE AIR TEST MUST FOLLOW THE PROCEDURES DESCRIBED IN AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM)
- C-828, ASTM C-924, OR ASTM F-1417 OR OTHER PROCEDURE APPROVED BY THE EXECUTIVE DIRECTOR, EXCEPT AS TO TESTING TIMES AS REQUIRED IN TABLE C.3 IN SUBPARAGRAPH (C) OF THIS PARAGRAPH OR EQUATION C.3 IN SUBPARAGRAPH (B)(II) OF THIS PARAGRAPH.

 (B) FOR SECTIONS OF COLLECTION SYSTEM PIPE LESS THAN 36 INCH AVERAGE INSIDE DIAMETER, THE FOLLOWING PROCEDURE MUST APPLY, UNLESS A PIPE IS TO BE TESTED AS REQUIRED BY PARAGRAPH (2) OF THIS SUBSECTION.
- (i) A PIPE MUST BE PRESSURIZED TO 3.5 POUNDS PER SQUARE INCH (PSI) GREATER THAN THE PRESSURE EXERTED BY GROUNDWATER ABOVE THE
- (ii) ONCE THE PRESSURE IS STABILIZED, THE MINIMUM TIME ALLOWABLE FOR THE PRESSURE TO DROP FROM 3.5 PSI GAUGE TO 2.5 PSI GAUGE IS COMPUTED FROM THE FOLLOWING EQUATION:

EQUATION C.3 $T = \frac{0.0}{2}$

WHERE:

- T = TIME FOR PRESSURE TO DROP 1.0 POUND PER SQUARE INCH GAUGE IN SECONDS
- K = 0.000419 X D X L, BUT NOT LESS THAN 1.0 D = AVERAGE INSIDE PIPE DIAMETER IN INCHES
- L = LENGTH OF LINE OF SAME SIZE BEING TESTED, IN FEET

 Q = RATE OF LOSS, 0.0015 CUBIC FEET PER MINUTE PER SQUARE FOOT INTERNAL SURFACE

(C) SINCE A K VALUE OF LESS THAN 1.0 MAY NOT BE USED, THE MINIMUM TESTING TIME FOR EACH PIPE DIAMETER IS SHOWN IN THE

PIPE DIAMETER (INCHES)	Minimum Time (seconds)	Maximum Length for Minimum Time (feet)	Time for Longer Length (seconds/foot)
6	340	398	0.8550
8	454	298	1.5200
10	567	239	2.3740
12	680	199	3.4190
15	850	159	5.3420
18	1020	133	7.6930
21	1190	114	10.4710
24	1360	100	13.6760
27	1530	88	17.3090
30	1700	80	21.3690
33	1870	72	25.8560

- (D) AN OWNER MAY STOP A TEST IF NO PRESSURE LOSS HAS OCCURRED DURING THE FIRST 25% OF THE CALCULATED TESTING TIME. IF ANY PRESSURE LOSS OR LEAKAGE HAS OCCURRED DURING THE FIRST 25% OF A TESTING PERIOD, THEN THE TEST MUST CONTINUE FOR THE ENTIRE TEST DURATION AS OUTLINED ABOVE OR UNTIL FAILURE.
- (E) WASTEWATER COLLECTION SYSTEM PIPES WITH A 27 INCH OR LARGER AVERAGE INSIDE DIAMETER MAY BE AIR TESTED AT EACH JOINT INSTEAD OF FOLLOWING THE PROCEDURE OUTLINED IN THIS SECTION.
- (F) A TESTING PROCEDURE FOR PIPE WITH AN INSIDE DIAMETER GREATER THAN 33 INCHES MUST BE APPROVED BY THE EXECUTIVE DIRECTOR.

 (2) INFILITRATION/FXFII TRATION TEST.
- (2) INPILITATION/EXPILITATION TEST.

 (A) THE TOTAL EXFILTRATION, AS DETERMINED BY A HYDROSTATIC HEAD TEST, MUST NOT EXCEED 50 GALLONS PER INCH OF DIAMETER PER MILE OF PIPE PER 24 HOURS AT A MINIMUM TEST HEAD OF 2.0 FEET ABOVE THE CROWN OF A PIPE AT AN UPSTREAM MANHOLE.
- (B) AN OWNER SHALL USE AN INFILTRATION TEST IN LIEU OF AN EXFILTRATION TEST WHEN PIPES ARE INSTALLED BELOW THE GROUNDWATER LEVEL.

 (C) THE TOTAL EXFILTRATION, AS DETERMINED BY A HYDROSTATIC HEAD TEST, MUST NOT EXCEED 50 GALLONS PER INCH DIAMETER PER
- MILE OF PIPE PER 24 HOURS AT A MINIMUM TEST HEAD OF TWO FEET ABOVE THE CROWN OF A PIPE AT AN UPSTREAM MANHOLE, OR AT LEAST TWO FEET ABOVE EXISTING GROUNDWATER LEVEL, WHICHEVER IS GREATER.

 (D) FOR CONSTRUCTION WITHIN A 25-YEAR FLOOD PLAIN, THE INFILTRATION OR EXFILTRATION MUST NOT EXCEED 10 GALLONS PER INCH
- (E) IF THE QUANTITY OF INFILTRATION OR EXFILTRATION EXCEEDS THE MAXIMUM QUANTITY SPECIFIED, AN OWNER SHALL UNDERTAKE REMEDIAL ACTION IN ORDER TO REDUCE THE INFILTRATION OR EXFILTRATION TO AN AMOUNT WITHIN THE LIMITS SPECIFIED. AN OWNER SHALL RETEST A PIPE FOLLOWING A REMEDIATION ACTION.

DIAMETER PER MILE OF PIPE PER 24 HOURS AT THE SAME MINIMUM TEST HEAD AS IN SUBPARAGRAPH (C) OF THIS PARAGRAPH.

- (b) IF A GRAVITY COLLECTION PIPE IS COMPOSED OF FLEXIBLE PIPE, DEFLECTION TESTING IS ALSO REQUIRED. THE FOLLOWING PROCEDURES MUST BE FOLLOWED:

 (1) FOR A COLLECTION PIPE WITH INSIDE DIAMETER LESS THAN 27 INCHES, DEFLECTION MEASUREMENT REQUIRES A RIGID MANDREL.
- (1) FOR A COLLECTION PIPE WITH INSIDE DIAMETER LESS THAN 27 INCHES, DEFLECTION MEASUREMENT REQUIRES A RIGID MANDI (A) MANDREL SIZING.

 (i) A RIGID MANDREL MUST HAVE AN OUTSIDE DIAMETER (OD) NOT LESS THAN 95% OF THE BASE INSIDE DIAMETER (ID) OR
 - AVERAGE ID OF A PIPE, AS SPECIFIED IN THE APPROPRIATE STANDARD BY THE ASTMS, AMERICAN WATER WORKS ASSOCIATION, UNI-BELL, OR AMERICAN NATIONAL STANDARDS INSTITUTE, OR ANY RELATED APPENDIX.

 (ii) IF A MANDREL SIZING DIAMETER IS NOT SPECIFIED IN THE APPROPRIATE STANDARD, THE MANDREL MUST HAVE AN OD EQUAL TO 95% OF THE ID OF A PIPE. IN THIS CASE, THE ID OF THE PIPE, FOR THE PURPOSE OF DETERMINING THE OD OF
 - THE MANDREL, MUST EQUAL BE THE AVERAGE OUTSIDE DIAMETER MINUS TWO MINIMUM WALL THICKNESSES FOR OD CONTROLLED PIPE AND THE AVERAGE INSIDE DIAMETER FOR ID CONTROLLED PIPE.

 (iii) ALL DIMENSIONS MUST MEET THE APPROPRIATE STANDARD.

 (B) MANDREL DESIGN.
 - (I) A RIGID MANDREL MUST BE CONSTRUCTED OF A METAL OR A RIGID PLASTIC MATERIAL THAT CAN WITHSTAND 200 PSI WITHOUT BEING DEFORMED.
 - (ii) A MANDREL MUST HAVE NINE OR MORE ODD NUMBER OF RUNNERS OR LEGS. (iii) A BARREL SECTION LENGTH MUST EQUAL AT LEAST 75% OF THE INSIDE DIAMETER OF A PIPE.
 - (iii) A BARREL SECTION LENGTH MOST EQUAL AT LEAST 75% OF THE INSIDE DIAMETER OF A P
 (iv) EACH SIZE MANDREL MUST USE A SEPARATE PROVING RING.
 - (i) AN ADJUSTABLE OR FLEXIBLE MANDREL IS PROHIBITED.
 - (ii) A TEST MAY NOT USE TELEVISION INSPECTION AS A SUBSTITUTE FOR A DEFLECTION TEST.
 - (iii) IF REQUESTED, THE EXECUTIVE DIRECTOR MAY APPROVE THE USE OF A DEFLECTOMETER OR A MANDREL WITH REMOVABLE LEGS OR RUNNERS ON A CASE-BY-CASE BASIS.

 (2) FOR A GRAVITY COLLECTION SYSTEM PIPE WITH AN INSIDE DIAMETER 27 INCHES AND GREATER, OTHER TEST METHODS MAY BE
 - USED TO DETERMINE VERTICAL DEFLECTION.

 (3) A DEFLECTION TEST METHOD MUST BE ACCURATE TO WITHIN PLUS OR MINUS 0.2% DEFLECTION.
 - (3) A DEFLECTION TEST METHOD MUST BE ACCURATE TO WITHIN PLUS OR MINUS 0.2% DEFLECTION.(4) AN OWNER SHALL NOT CONDUCT A DEFLECTION TEST UNTIL AT LEAST 30 DAYS AFTER THE FINAL BACKFILL.
 - (4) AN OWNER SHALL NOT CONDUCT A DEFLECTION TEST UNTIL AT LEAST 30 DAYS AFTER THE FINAL BACKFILL(5) GRAVITY COLLECTION SYSTEM PIPE DEFLECTION MUST NOT EXCEED FIVE PERCENT (5%),
 - (6) IF A PIPE SECTION FAILS A DEFLECTION TEST, AN OWNER SHALL CORRECT THE PROBLEM AND CONDUCT A SECOND TEST AFTER THE FINAL BACKFILL HAS BEEN IN PLACE AT LEAST 30 DAYS.
- ALL MANHOLES MUST BE TESTED TO MEET OR EXCEED THE REQUIREMENTS OF 30 TAC §217.58.
 (a) ALL MANHOLES MUST PASS A LEAKAGE TEST.
- (b) AN OWNER SHALL TEST EACH MANHOLE (AFTER ASSEMBLY AND BACKFILLING) FOR LEAKAGE, SEPARATE AND INDEPENDENT OF THE COLLECTION SYSTEM PIPES, BY HYDROSTATIC EXFILTRATION TESTING, VACUUM TESTING, OR OTHER METHOD APPROVED BY THE EXECUTIVE DIRECTOR.
 - (A) THE MAXIMUM LEAKAGE FOR HYDROSTATIC TESTING OR ANY ALTERNATIVE TEST METHODS IS 0.025 GALLONS PER FOOT DIAMETER PER FOOT OF MANHOLE DEPTH PER HOUR.

 (B) TO PERFORM A HYDROSTATIC EXPLICATION TEST, AN OWNER SHALL SEAL ALL WASTEWATER PIPES COMING INTO A MANHOLE WITH AN
 - (B) TO PERFORM A HYDROSTATIC EXFILTRATION TEST, AN OWNER SHALL SEAL ALL WASTEWATER PIPES COMING INTO A MANHOLE WITH AN INTERNAL PIPE PLUG, FILL THE MANHOLE WITH WATER, AND MAINTAIN THE TEST FOR AT LEAST ONE HOUR.
 (C) A TEST FOR CONCRETE MANHOLES MAY USE A 24-HOUR WETTING PERIOD BEFORE TESTING TO ALLOW SATURATION OF THE CONCRETE.
 - (2) VACUUM TESTING.(A) TO PERFORM A VACUUM TEST, AN OWNER SHALL PLUG ALL LIFT HOLES AND EXTERIOR JOINTS WITH A NON-SHRINK GROUT AND PLUG ALL PIPES ENTERING A MANHOLE.
 - PIPES ENTERING A MANHOLE.

 (B) NO GROUT MUST BE PLACED IN HORIZONTAL JOINTS BEFORE TESTING.
- (B) NO GROUT MUST BE PLACED IN HORIZONTAL JOINTS BEFORE TESTING.

 (C) STUB-OUTS, MANHOLE BOOTS, AND PIPE PLUGS MUST BE SECURED TO PREVENT MOVEMENT WHILE A VACUUM IS DRAWN.

 (D) AN OWNER SHALL USE A MINIMUM 60 INCH/LB TORQUE WRENCH TO TIGHTEN THE EXTERNAL CLAMPS THAT SECURE A TEST COVER TO
- THE TOP OF A MANHOLE.

 (E) A TEST HEAD MUST BE PLACED AT THE INSIDE OF THE TOP OF A CONE SECTION, AND THE SEAL INFLATED IN ACCORDANCE WITH THE
- (E) A TEST HEAD MUST BE PLACED AT THE INSIDE OF THE TOP OF A CONE SECTION, AND THE SEAL INFLATED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- (F) THERE MUST BE A VACUUM OF 10 INCHES OF MERCURY INSIDE A MANHOLE TO PERFORM A VALID TEST.
- (G) A TEST DOES NOT BEGIN UNTIL AFTER THE VACUUM PUMP IS OFF.

 (H) A MANHOLE PASSES THE TEST IF AFTER 2.0 MINUTES AND WITH ALL VALVES CLOSED, THE VACUUM IS AT LEAST 9.0 INCHES OF MERCURY
- 2. ALL PRIVATE SERVICE LATERALS MUST BE INSPECTED AND CERTIFIED IN ACCORDANCE WITH 30 TAC §213.5(C)(3)(I). AFTER INSTALLATION OF AND, PRIOR TO COVERING AND CONNECTING A PRIVATE SERVICE LATERAL TO AN EXISTING ORGANIZED SEWAGE COLLECTION SYSTEM, A TEXAS LICENSED PROFESSIONAL ENGINEER, TEXAS REGISTERED SANITARIAN, OR APPROPRIATE CITY INSPECTOR MUST VISUALLY INSPECT THE PRIVATE SERVICE LATERAL AND THE CONNECTION TO THE SEWAGE COLLECTION SYSTEM, AND CERTIFY THAT IT IS CONSTRUCTED IN CONFORMITY WITH THE APPLICABLE PROVISIONS OF THIS SECTION. THE OWNER OF THE COLLECTION SYSTEM MUST MAINTAIN SUCH CERTIFICATIONS FOR FIVE YEARS AND FORWARD COPIES TO THE APPROPRIATE REGIONAL OFFICE UPON REQUEST. CONNECTIONS MAY ONLY BE MADE TO AN APPROVED SEWAGE COLLECTION SYSTEM.

AUSTIN REGIONAL OFFICE 12100 PARK 35 CIRCLE, BUILDING A AUSTIN, TEXAS 78753-1808 PHONE (512) 339-2929 FAX (512) 339-3795

SAN ANTONIO REGIONAL OFFICE 14250 JUDSON ROAD SAN ANTONIO, TEXAS 78233-4480 PHONE (210) 490-3096 FAX (210) 545-4329

THESE GENERAL CONSTRUCTION NOTES MUST BE INCLUDED ON THE CONSTRUCTION PLANS PROVIDED TO THE CONTRACTOR AND ALL SUBCONTRACTORS.

TCEQ-0596 (REV. JULY 15, 2015)

TCEQ WATER DISTRIBUTION SYSTEM GENERAL CONSTRUCTION NOTES

- 1. This water distribution system must be constructed in accordance with the current Texas Commission on Environmental Quality (TCEQ) Rules and Regulations for Public Water Systems 30 Texas Administrative Code (TAC) Chapter 290 Subchapter D. When conflicts are noted with local standards, the more stringent requirement shall be applied. At a minimum, construction for public water systems must always meet TCEQ's "Rules and Regulations for Public Water Systems."
- 2. All newly installed pipes and related products must conform to American National Standards Institute (ANSI)/NSF International Standard 61 and must be certified by an organization accredited by ANSI [§290.44(a)(1)].
- 3. Plastic pipe for use in public water systems must bear the NSF International Seal of Approval (NSF-pw) and have an ASTM design pressure rating of at least 150 psi or a standard dimension ratio of 26 or less [§290.44(a)(2)].
- 4. No pipe which has been used for any purpose other than the conveyance of drinking water shall be accepted or relocated for use in any public drinking water supply [§290.44(a)(3)].
- 5. All water line crossings of wastewater mains shall be perpendicular [§290.44(e)(4)(B)].
- 6. Water transmission and distribution lines shall be installed in accordance with the manufacturer's instructions. However, the top of the water line must be located below the frost line and in no case shall the top of the water line be less than 24 inches below ground surface [§290.44(a)(4)].
- 7. The maximum allowable lead content of pipes, pipe fittings, plumbing fittings, and fixtures is 0.25 percent [§290.44(b)].
- 8. The contractor shall install appropriate air release devices with vent openings to the atmosphere covered with 16-mesh or finer, corrosion resistant screening material or an acceptable equivalent [§290.44(d)(1)].
- 9. The contractor shall not place the pipe in water or where it can be flooded with water or sewage during its storage or installation [§290.44(f)(1)].
- 10. When waterlines are laid under any flowing or intermittent stream or semi-permanent body of water the waterline shall be installed in a separate watertight pipe encasement. Valves must be provided on each side of the crossing with facilities to allow the underwater portion of the system to be isolated and tested [§290.44(f)(2)].
- 11. Pursuant to 30 TAC §290.44(a)(5), the hydrostatic leakage rate shall not exceed the amount allowed or recommended by the most current AWWA formulas for PVC pipe, cast iron and ductile iron pipe. Include the formulas in the notes on the plans.
- o The hydrostatic leakage rate for polyvinyl chloride (PVC) pipe and appurtenances shall not exceed the amount allowed or recommended by formulas in America Water Works Association (AWWA) C-605 as required in 30 TAC §290.44(a)(5). Please ensure that the formula for this calculation is correct and most current formula is in use;

 $Q = \frac{LD\sqrt{P}}{148,000}$

*W*here:

- Q = the quantity of makeup water in gallons per hour,
- L = the length of the pipe section being tested, in feet,
- D = the nominal diameter of the pipe in inches, and
 P = the average test pressure during the hydrostatic test in pounds per square inch
- o The hydrostatic leakage rate for ductile iron (DI) pipe and appurtenances shall not exceed the amount allowed or recommended by formulas in America Water Works Association (AWWA) C-600 as required in 30 TAC §290.44(a)(5). Please ensure that the formula for this calculation is correct and most current formula is in use;

 $L = \frac{SD\sqrt{P}}{148000}$

Where:

- L = the quantity of makeup water in gallons per hour,
- S = the length of the pipe section being tested, in feet,
 D = the nominal diameter of the pipe in inches, and
- D = the nominal diameter of the pipe in inches, and
 P = the average test pressure during the hydrostatic test in pounds per square inch
- 12. The contractor shall maintain a minimum separation distance in all directions of nine feet between the proposed waterline and wastewater collection facilities including manholes. If this distance cannot be maintained, the contractor must immediately notify the project engineer for further direction. Separation distances, installation methods, and materials utilized must meet §290.44(e)(1)-(4).
- 13. The separation distance from a potable waterline to a wastewater main or lateral manhole or cleanout shall be a minimum of nine feet. Where the nine-foot separation distance cannot be achieved, the potable waterline shall be encased in a joint of at least 150 psi pressure class pipe at least 18 feet long and two nominal sizes larger than the new conveyance. The space around the carrier pipe shall be supported at five-foot intervals with spacers or be filled to the springline with washed sand. The encasement pipe shall be centered on the crossing and both ends sealed with cement grout or manufactured sealant [§290.44(e)(5)].
- 14. Fire hydrants shall not be installed within nine feet vertically or horizontally of any wastewater line, wastewater lateral, or wastewater service line regardless of construction [§290.44(e)(6)].
- 15. Suction mains to pumping equipment shall not cross wastewater mains, wastewater laterals, or wastewater service lines. Raw water supply lines shall not be installed within five feet of any tile or concrete wastewater main, wastewater lateral, or wastewater service line [§290.44(e)(7)].
- 16. Waterlines shall not be installed closer than ten feet to septic tank drainfields [§290.44(e)(8)].
- 17. The contractor shall disinfect the new waterlines in accordance with AWWA Standard C-651-14 or most recent, then flush and sample the lines before being placed into service. Samples shall be collected for microbiological analysis to check the effectiveness of the disinfection procedure which shall be repeated if contamination persists. A minimum of one sample for each 1,000 feet of completed waterline will be required or at the next available sampling point beyond 1,000 feet as designated by the design engineer [§290.44(f)(3)].
- 18. Dechlorination of disinfecting water shall be in strict accordance with current AWWA Standard C655-09 or most recent.

Revised October 2017

ing, Inc.

Sec. Cer.

Soo. 6. 4.25

Designed BA:

Soo. 6.

Carlson, Brigance & Doerin

Civil Engineering Surveying

FIRM ID #F3791

Main Office

5501 West William Cannon Dr. 12129 RR 620 N

Austin, Texas 78749

Austin, Texas

ADDLEBACK AT SANTA RITA RANCH
PHASE 1 SECTION 3

STEVEN P. CATES

93648

CARLSON, BRIGANCE & DOERING, INC.

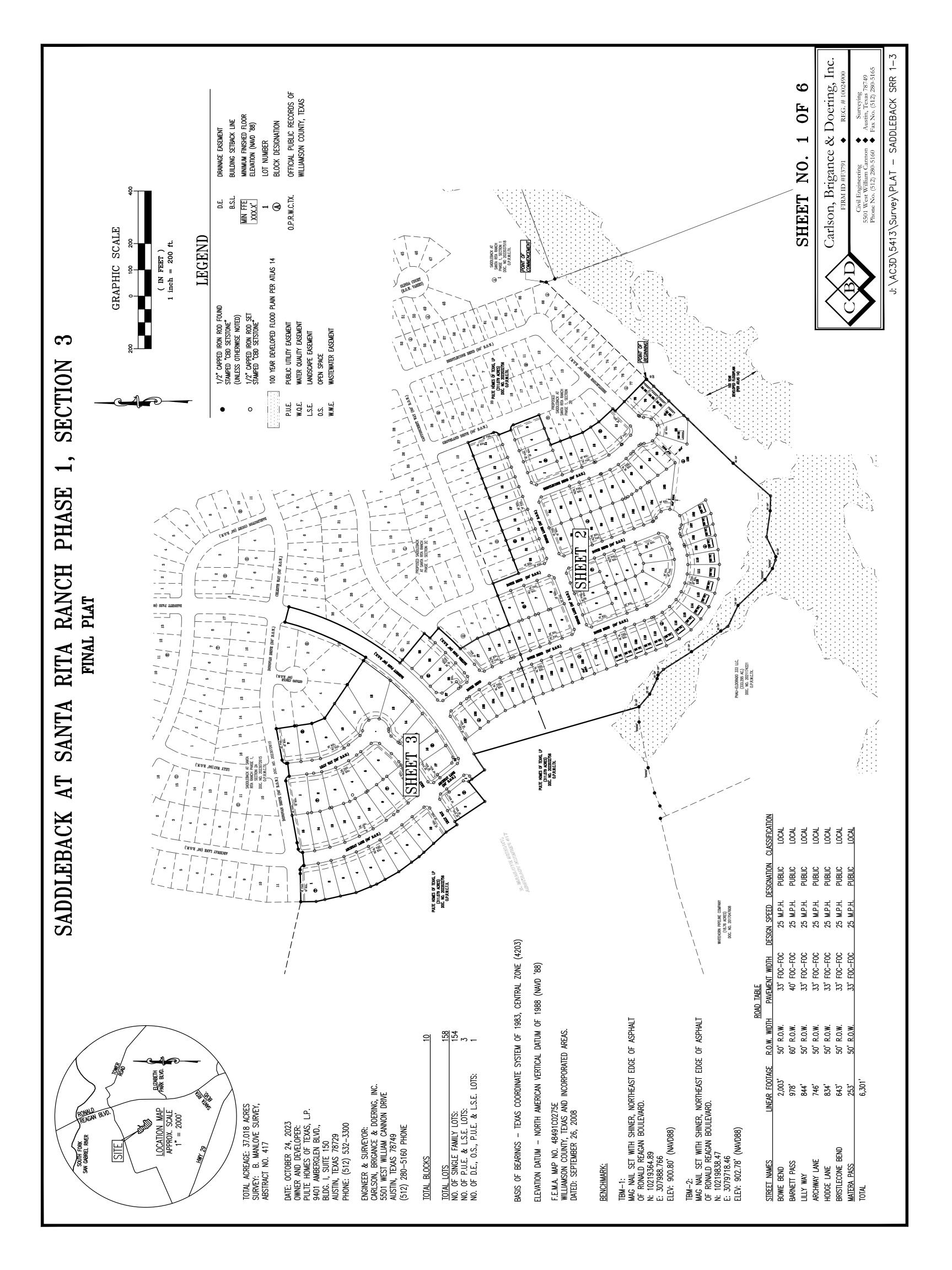
10# F3791

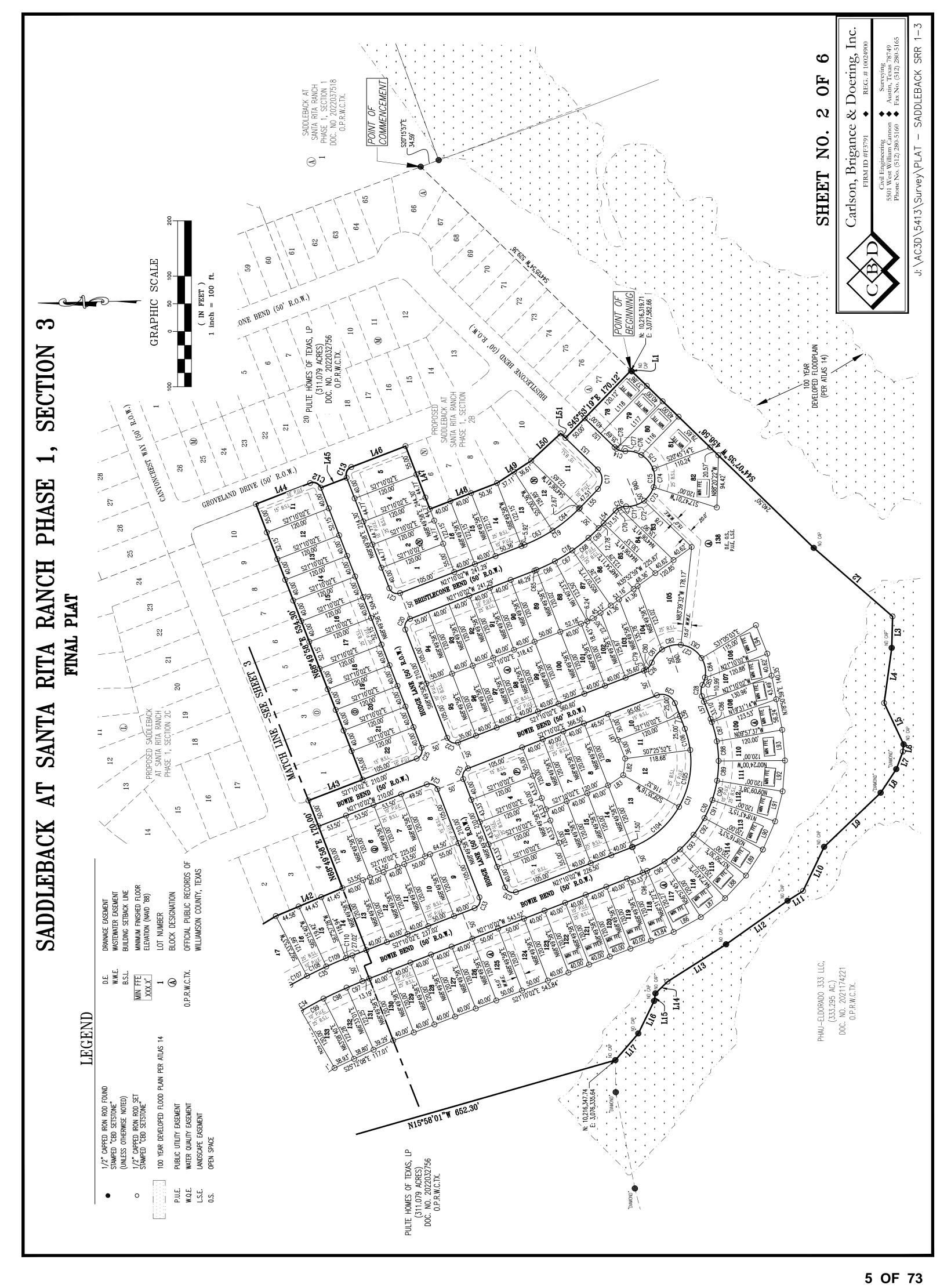
OCT 2023
OB NUMBER
5413

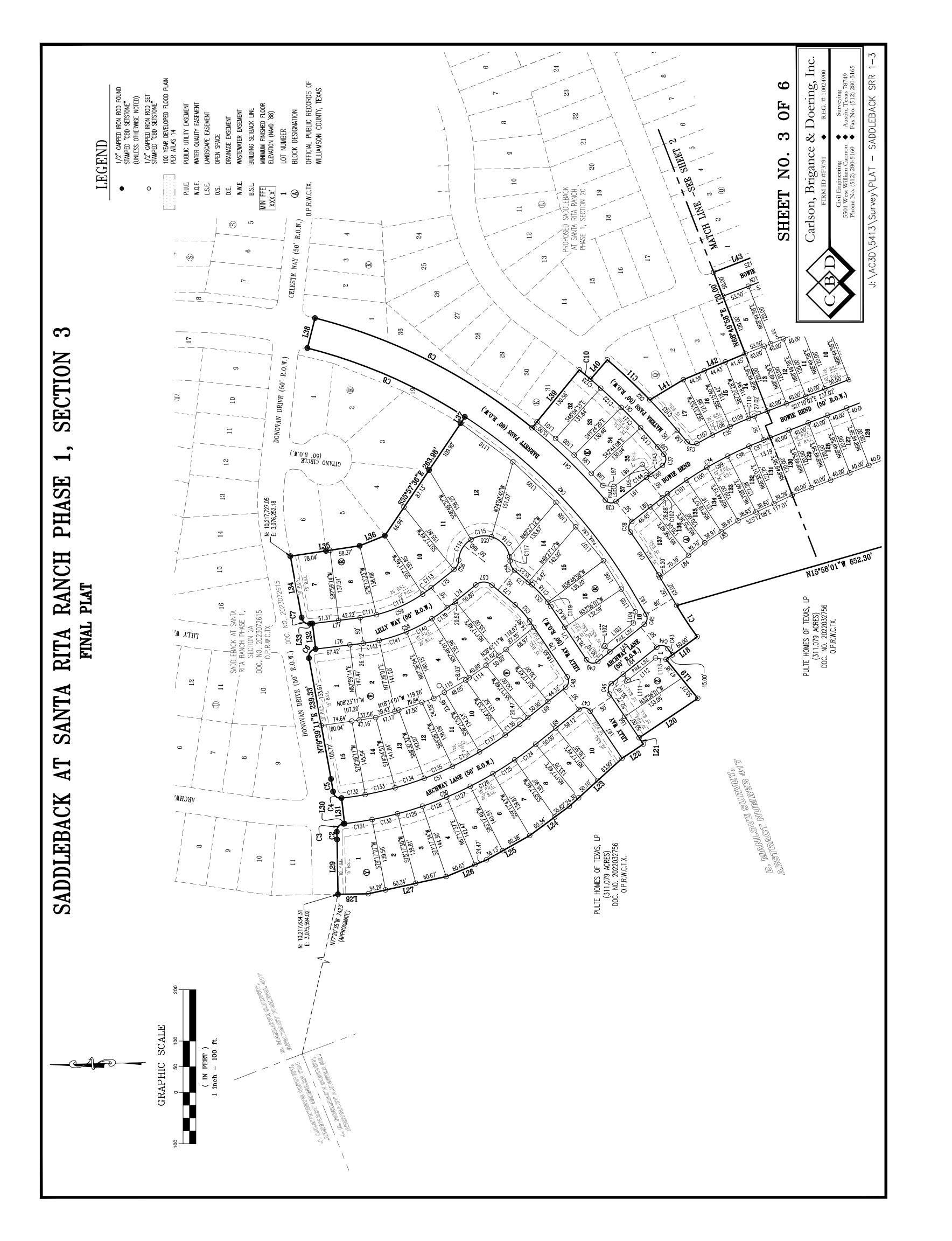
Store Cate

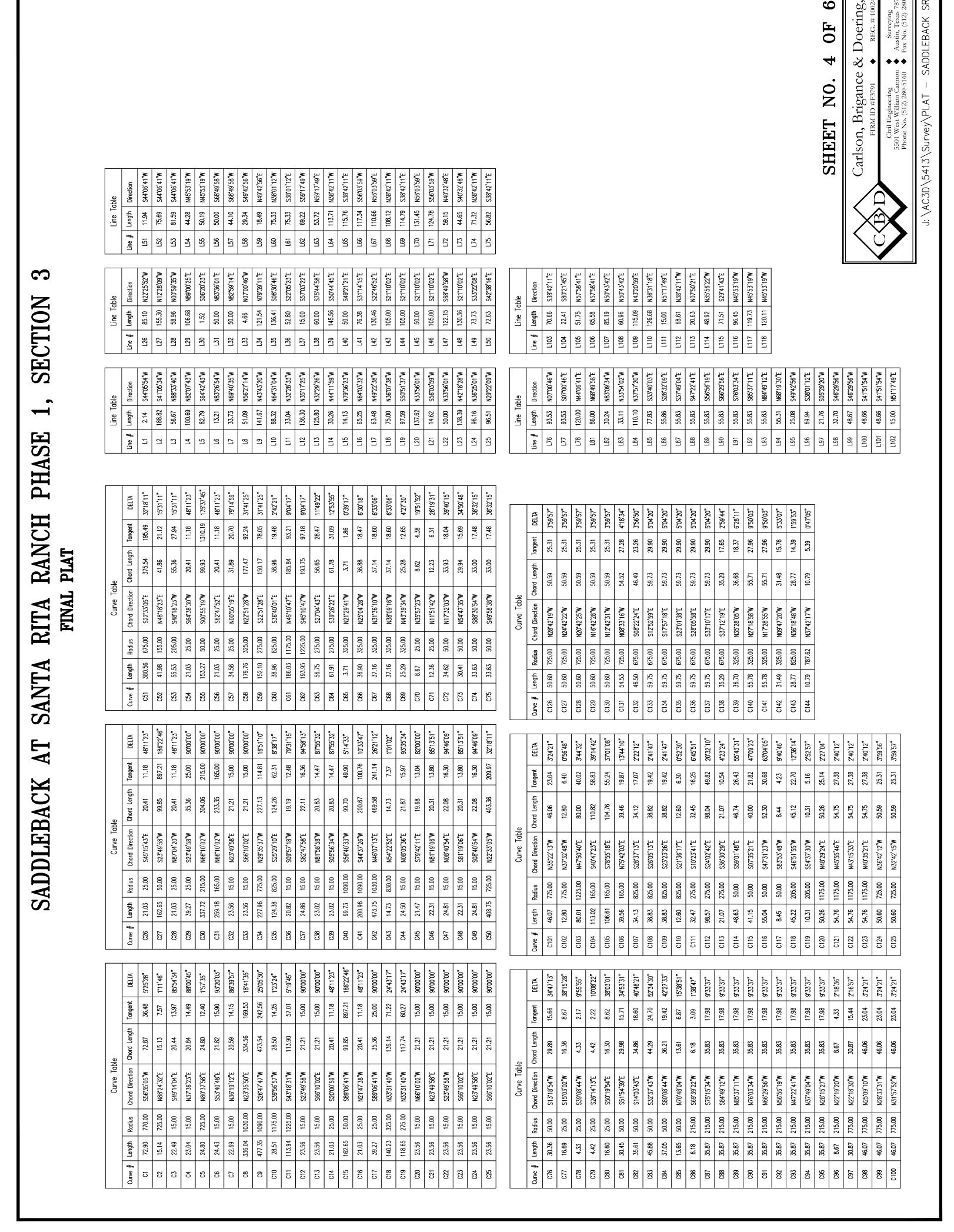
SHEET NO.

4









SECTION \blacksquare PHASE RITA RANCH FINAL PLAT SANTA AT SADDLEBACK

ATER AND WASTEWATER:

1. NO LOT IN THIS SUBDIVISION SHALL BE OCCUPIED UNTIL CONNECTION IS MADE TO AN APPROVED PUBLIC SEWER SYSTEM. NO LOT

1. NO LOT IN THIS SUBDIVISION SHALL BE OCCUPIED UNTIL CONNECTION IS AVAILABLE FROM A SOURCE IN ADEQUATE AND SUFFICIENT SUPPLY.

3. WATER SERVICE IS PROVIDED BY: WILLIAMSON COUNTY MUD 19H/GEORGETOWN UTILITY SYSTEMS.

4. WASTEWATER SERVICE IS PROVIDED BY: WILLIAMSON COUNTY MUD 19H/CITY OF LIBERTY HILL.

ING A PORTION OF A CA ACRE TRACT OF LAND E SAME BEING THE NORTH , A DISTANCE OF 2.13 F 311.079 / ROD FOUN

1. \$44'05'54"W, A DISTANCE OF 2.14 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
2. \$44'07'35"W, A DISTANCE OF 458.58 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
3. \$41'05'34"W, A DISTANCE OF 188.82 FEET TO A 1/2 INCH IRON ROD SET STAMPED "CBID SETSTONE" FOR CORNER;
4. N887340"W, A DISTANCE OF 10.69 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
5. N82'07'43"W, A DISTANCE OF 10.69 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
6. \$6442'43"W, A DISTANCE OF 13.21 FEET TO A 1/2 INCH IRON ROD FOUND SET STAMPED "CBID SETSTONE" FOR CORNER;
7. N872'65"W, A DISTANCE OF 13.21 FEET TO A CAPPED 1/2 INCH IRON ROD FOUND STAMPED "DIAMOND" FOR CORNER;
8. N69'40'35"W, A DISTANCE OF 51.09 FEET TO A CAPPED 1/2 INCH IRON ROD FOUND STAMPED "DIAMOND" FOR CORNER;
9. N56'27'14"W, A DISTANCE OF 51.09 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
11. N64'31'04"W, A DISTANCE OF 141.67 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
12. N32'28'33"W, A DISTANCE OF 135.30 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
13. N35'17'25"W, A DISTANCE OF 136.30 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
14. N32'29'26"W, A DISTANCE OF 136.30 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
15. N44'11'59"W, A DISTANCE OF 136.30 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
16. N79'36'23"W, A DISTANCE OF 14.13 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
17. N64'03'32"W, A DISTANCE OF 65.25 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
18. N49'22'38"W, A DISTANCE OF 63.48 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;
19. N49'22'38"W, A DISTANCE OF 65.25 FEET TO A 1/2 INCH IRON ROD FOUND FOR CORNER;

S. BERNOR SENDE FOR THE PAIR FOUNDS HERE.

BOADWAY AND BOSTL-GE-WIX.

1. IN PARNOW HE S. PAIR OF THE COMESTONES CLIRK OF WILLMSON COUNT, TEXS, II IS UNDERSTOOD THAT HE BUILDNG OF ALL ROWES, ON THE PROJECT OF WILLMSON COUNT, TEXS, SAD CONFED BY THE PAIR IN THE COUNTY SHARES AND CONFED BY THE PAIR IN THE COUNTY SHARES AND CONFED BY THE PAIR IN THE COUNTY SHARES AND CONFED BY THE PAIR IN THE COUNTY SHARES AND CONFED BY THE PAIR IN THE COUNTY SHARES AND CONFED BY THE PAIR IN THE COUNTY SHARES AND CONFED BY THE PAIR IN THE COUNTY SHARES AND CONFED BY THE WILL SHARES AND CONFED BY THE PAIR IN THE COUNTY SHARES AND CONFED BY THE WILL SHARES AND WILL

1. N15'58'01"W, A DISTANCE OF 652.30 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 3. N36'07'38"W, A DISTANCE OF 75.00 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 4. S50'51'37"W, A DISTANCE OF 75.00 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 5. N33'56'01"W, A DISTANCE OF 137.62 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 6. S56'03'59"W, A DISTANCE OF 14.62 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 7. N33'56'01"W, A DISTANCE OF 50.00 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 8. N42'18'28"W, A DISTANCE OF 96.16 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 10. N29'22'09"W, A DISTANCE OF 96.16 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 11. N22'25'52"W, A DISTANCE OF 96.51 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 12. N12'28'09"W, A DISTANCE OF 98.51 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 13. N00'59'35"W, A DISTANCE OF 58.96 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" FOR CORNER; 13. N00'59'35"W, A DISTANCE OF 58.96 FEET TO A CAPPED 1/2 INCH IRON ROD SET STAMPED "CBD SETSTONE" ON THE SOUTH LIN 2A, A SUBDIVISION RECORDED IN DOCUMENT NO. 2023072615, 0.P.R.W.C.TX.,

Heric Good workers and introducing Location.

Heric Good workers and introducing Location and any art is such that is about the St. St. Than No. 1990 to 1990

OF 2 NO. SHEET

Inc x Doering, S ★ REC.#1002490 ******* Carlson, Brigance

SECTION 1, **PHASE** RANCH FINAL PLAT RITA SANTA AT

SADDLEBACK

NER OF THAT CALLED 311.079 ACRE TRACT OF LAND DS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY O HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS OADS, ALLEYS, RIGHTS—OF—WAY, EASEMENTS AND PUBLIC Y MAY DEEM APPROPRIATE AND DO HEREBY STATE THAT DE LIENS. STEPHEN ASHLOCK, VICE PRESIDE AVEYED IN DOCUMENT NUMBER SDIVIDE 37.018 ACRES OUT OF SOMN HEREON, AND DO HEREBY FC CES SHOWN HEREON FOR SUCH FUBLIC ROADWAYS AND EASEMENTS SUBDIVISION IS TO BE KNOWN A

TO ME TO ME THEREIN

l≥

THIS FLOOD STATEMENT, AS DETERMINED BY A H.U.D.-OR THE IMPROVEMENTS THEREON WILL BE FREE FROM AND WILL OCCUR, AND FLOOD HEIGHTS MAY INCREASE

STATE OF TEXAS:

PF

THE STATE COMPLIES

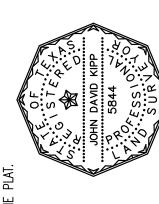
LAWS OF SION PLAT I, STEVEN P. CATES, P.E., AM AUTHORIZE ENGINEERING, AND HEREBY CERTIFY THAT

STEVEN P. CATE
35/0/AL EVEN

STEVEN P. CATE
35/0/AL EVEN

SAN SRIGANCE, & DOE

N DAVID KIPP, R.P.L.S., A AND HEREBY CERTIFY 1 LISTED ON TITLE REPORT 27 2021 WHICH AFFE



THE COUNTY ASSUMES NO RESPONSIBILITY FOR THE ACCURACY OF REPRESENTATIONS BY OTHER PARTIES IN THIS PLAT. FLOOD PLAIN DATA IN PARTICULAR, MAY CHANGE DEPENDING ON SUBSEQUENT DEVELOPMENT. IT IS FURTHER UNDERSTOOD THAT THE OWNERS OF THE TRACT OF LAND COVERED BY THIS PLAT MUST INSTALL AT THEIR OWN EXPENSE ALL TRAFFIC CONTROL DEVICES AND SIGNAGE THAT MAY BE REQUIRED BEFORE THE STREETS IN THE SUBDIVISION HAVE FINALLY BEEN ACCEPTED FOR MAINTENANCE BY THE COUNTY. THE CITY OF LIBERTY HILL, TEXAS ACKNOWLEDGES RECEIPT OF THIS PLAT FOR REVIEW AND/OR APPROVAL IN CONJUNCTION WITH

လာ လာ လာ STATE OF

LIAMSON COUNTY A

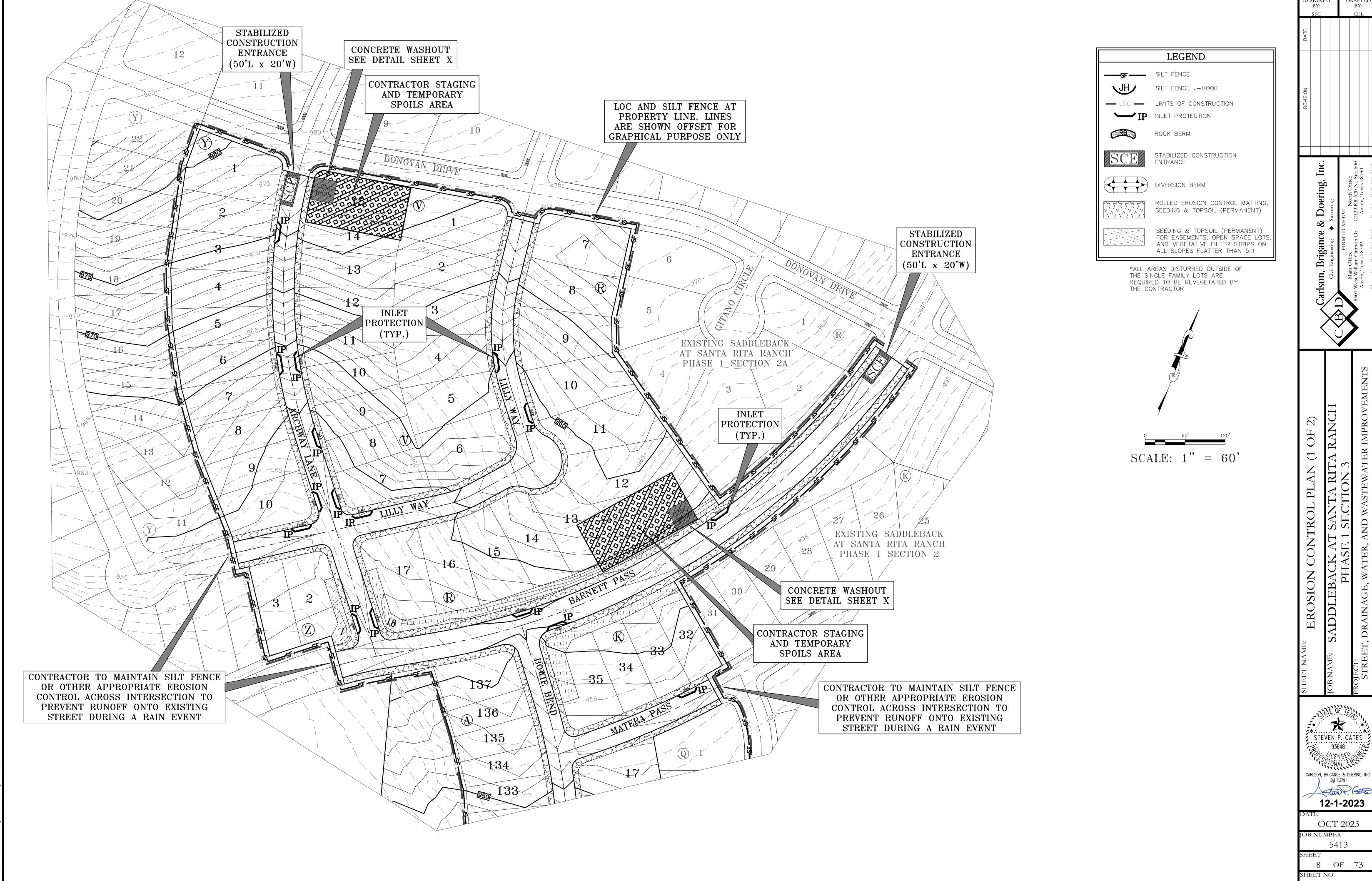
NTY JUDGE OF WILLIAMSON COUNTY, TEXAS, DO HEREBY CERTIFY THAT THIS MAP OR PLAT, WITH FIELD BDIVISION HAVING BEEN FULLY PRESENTED TO THE COMMISSIONERS' COURT OF WILLIAMSON COUNTY, TEXAS, DULY CONSIDERED, WERE ON THIS DAY APPROVED AND PLAT IS AUTHORIZED TO BE REGISTERED AND RECORDS OF THE COUNTY CLERK OF WILLIAMSON COUNTY, TEXAS. L GRAVELL, JR., S HEREON, THAT & BY THE SAID CO RDED IN THE PROI

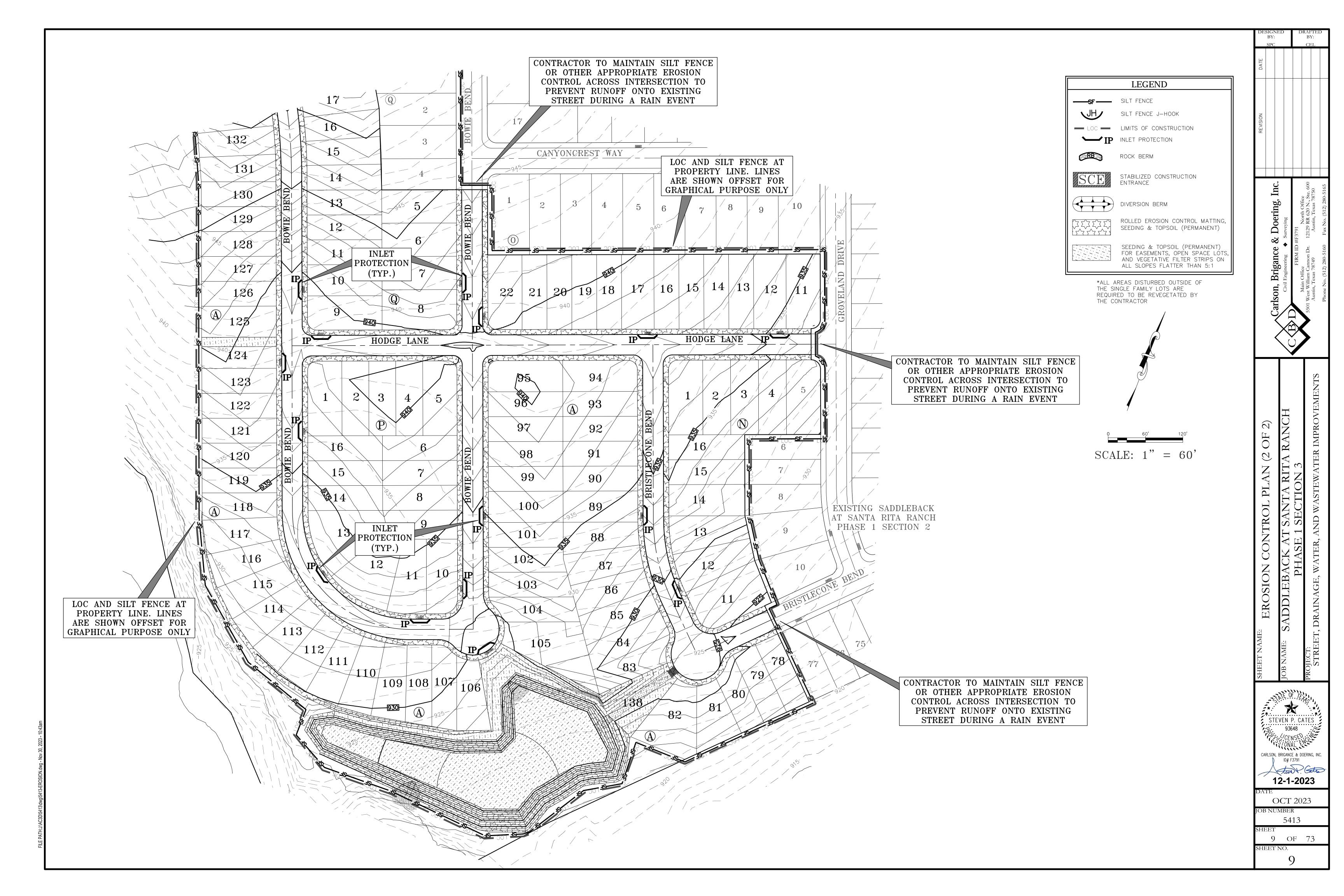
STATE OF TEXAS

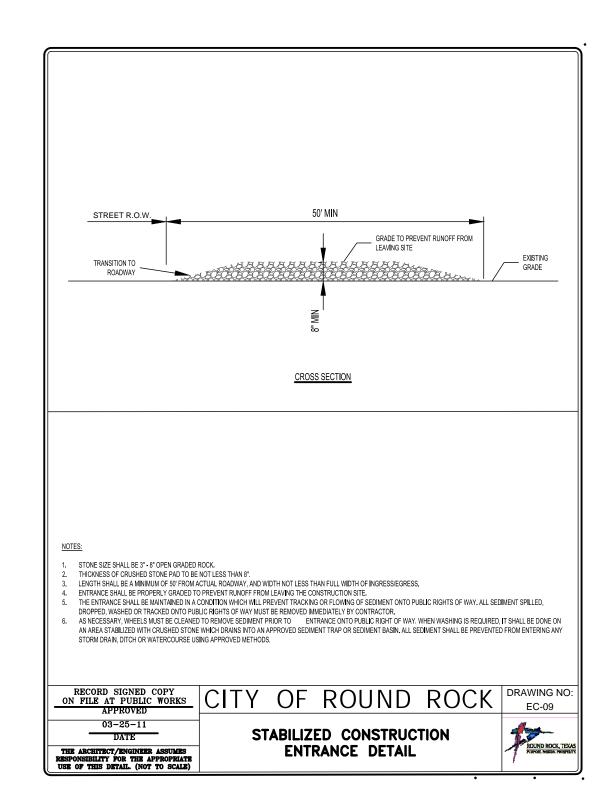
TO CERTIFY WHICH, WITNESS MY HAND THE DATE LAST SHOWN ABOVE WRITTEN

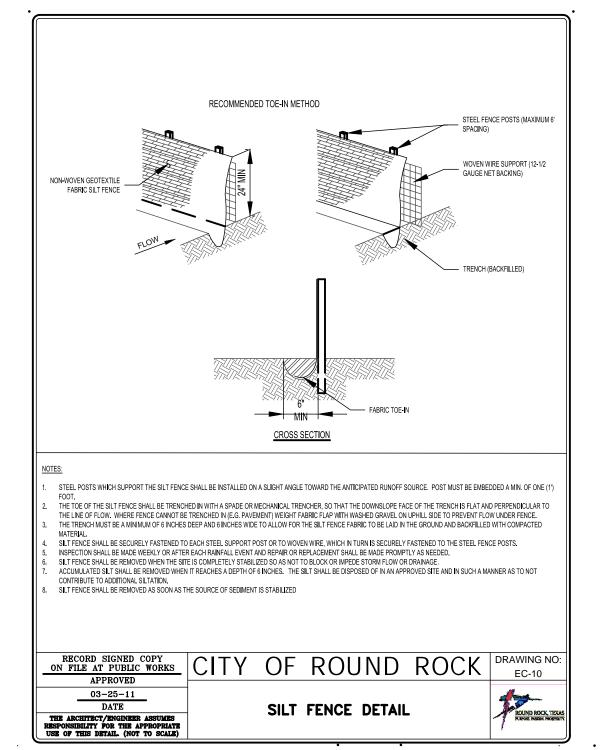
OF 9 SHEET NO. Carlson, Brigance & Doering, Inc

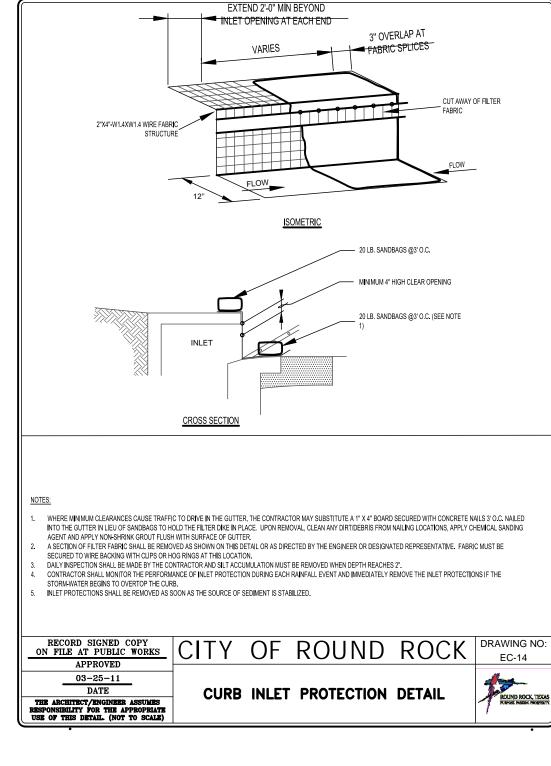
on 💠 Aus

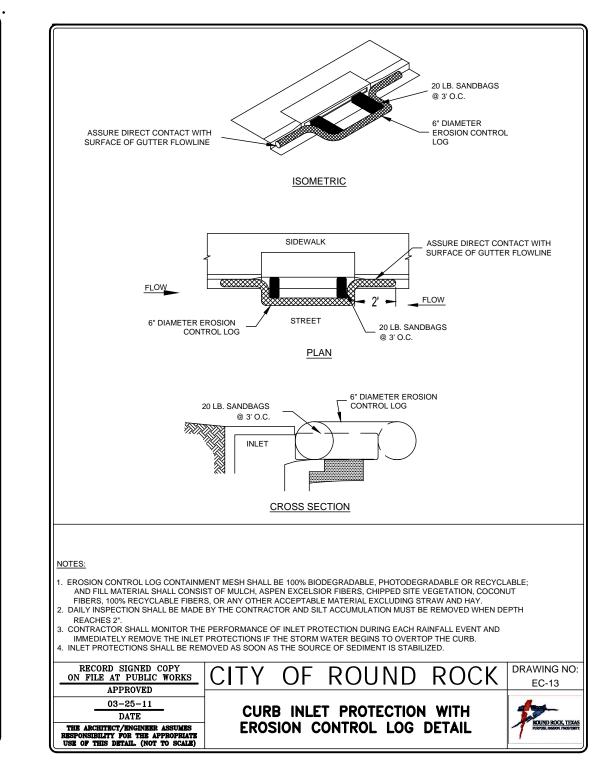


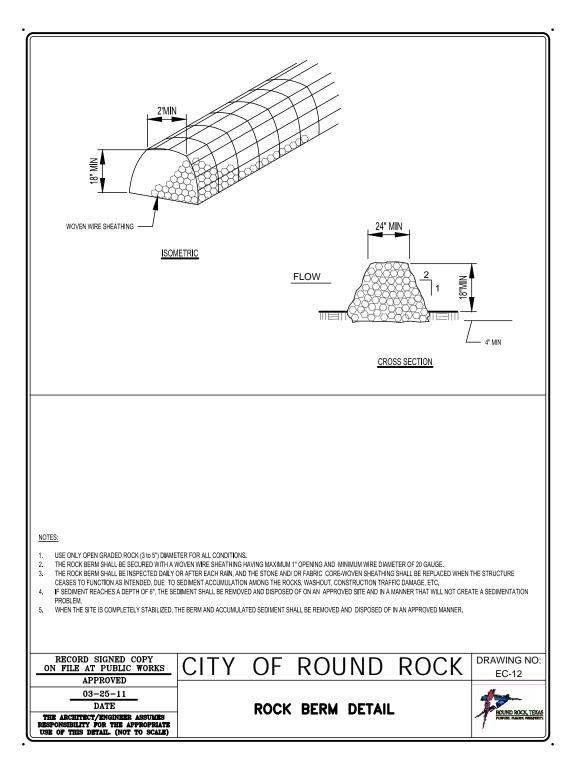


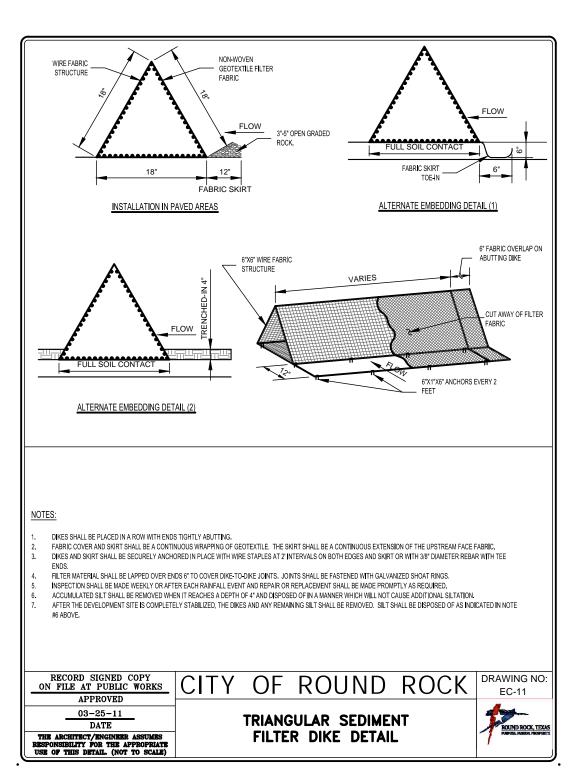


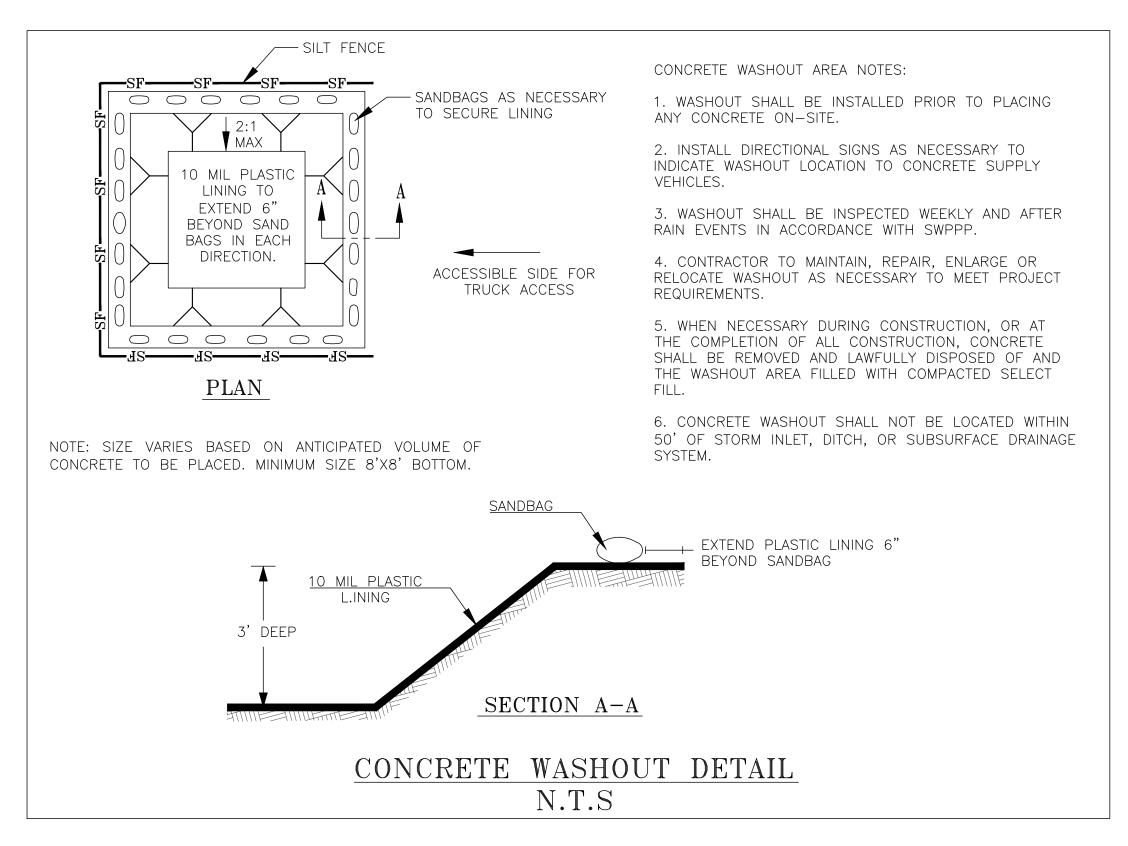


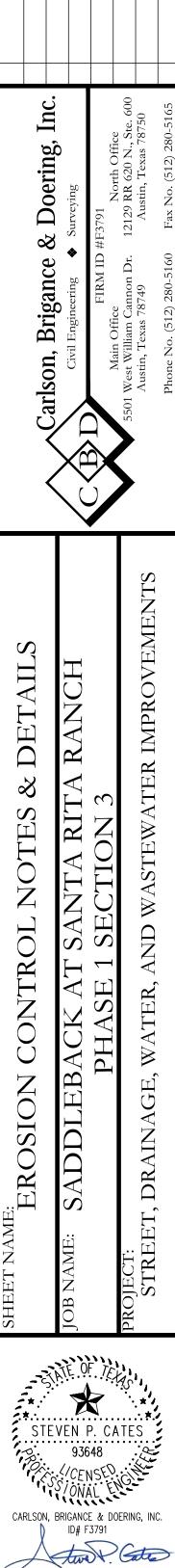












12-1-2023

OCT 2023

5413

10 OF 73



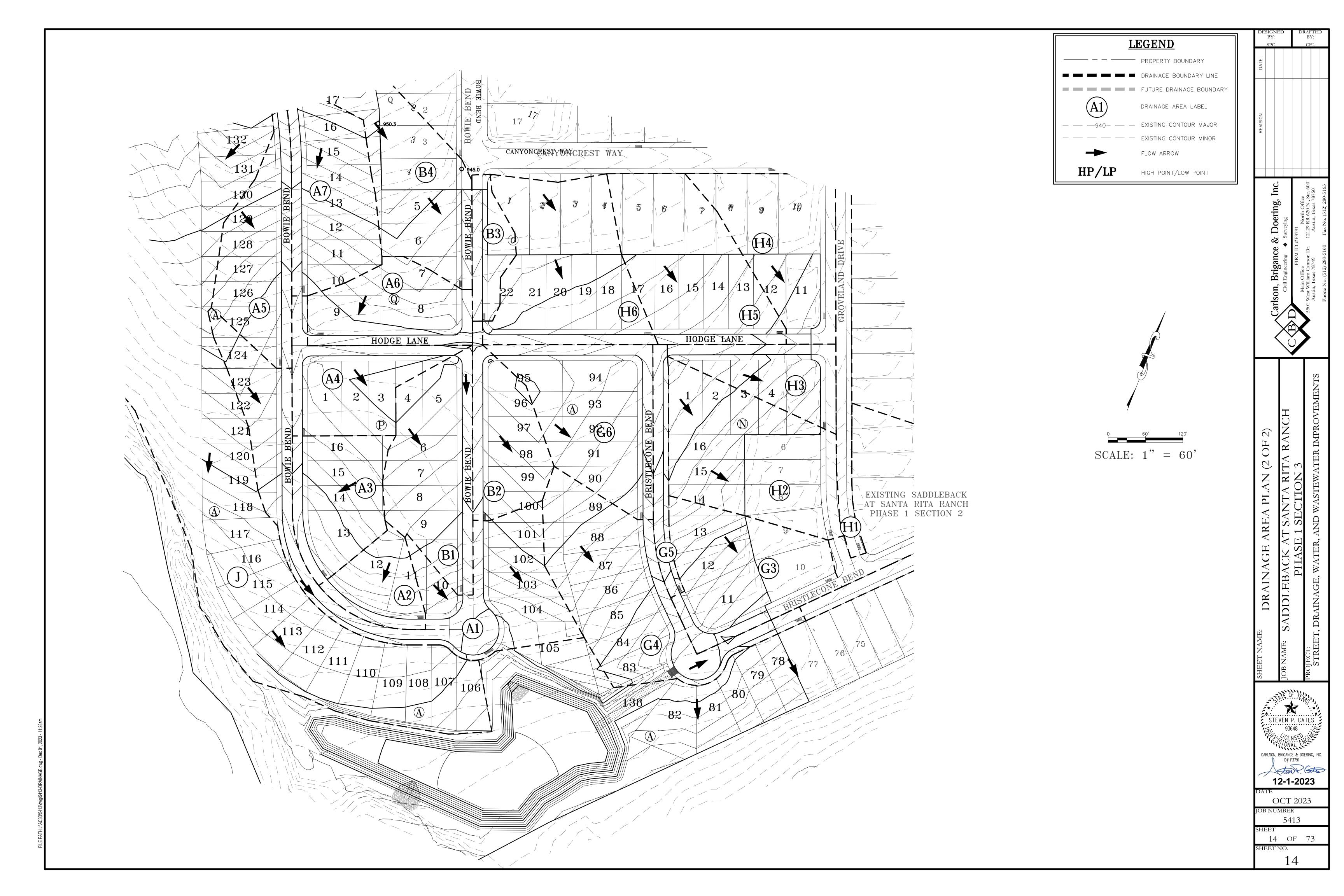
TER IMPROVEMENTS

STEVEN P. CATES

12-1-2023

OCT 2023

13 OF 73



No. A1 A2 A3 A4 A5	(Acre) 1.58 0.39 0.69	(Min.)	(%)	1			С	I ₁₀	I ₂₅	I ₁₀₀	Q ₁₀	Q ₂₅	Q ₁₀₀
A2 A3 A4 A5	0.39 0.69	14	(70)	(%)	10	25	100	In/Hr	In/Hr	In/Hr	CFS	CFS	CFS
A3 A4 A5	0.69		44	56	0.61	0.65	0.73	6.37	7.74	10.00	6.1	8.0	11.6
A4 A5		11	45	55	0.60	0.65	0.73	7.01	8.49	10.89	1.6	2.1	3.1
A5		12	47	53	0.59	0.64	0.72	6.88	8.35	10.72	2.8	3.7	5.3
	0.70	17	40	60	0.59	0.63	0.72	5.81	7.09	9.21	2.4	3.1	4.6
	1.03	10	36	64	0.64	0.69	0.77	7.26	8.78	11.23	4.8	6.3	9.0
A6	0.68	13	44	56	0.61	0.65	0.73	6.58	8.00	10.30	2.7	3.6	5.2
Α7	1.05	10	43	57	0.61	0.66	0.74	7.26	8.78	11.23	4.7	6.1	8.7
A8	1.87	11	48	52	0.59	0.63	0.71	7.10	8.60	11.02	7.8	10.2	14.7
A9	0.89	13	42	58	0.61	0.66	0.74	6.64	8.07	10.39	3.6	4.7	6.8
A10	0.17	10	16	84	0.72	0.77	0.85	7.26	8.78	11.23	0.9	1.1	1.6
A11	0.67	10	51	49	0.58	0.62	0.70	7.17	8.68	11.12	2.8	3.6	5.2
A12	1.13	10	51	49	0.57	0.62	0.70	7.26	8.78	11.23	4.7	6.1	8.9
A13	0.47	10	50	50	0.58	0.63	0.71	7.26	8.78	11.23	2.0	2.6	3.7
A14	0.81	12	48	52	0.54	0.59	0.67	6.77	8.21	10.56	3.0	3.9	5.7
A15	1.11	10	51	49	0.57	0.62	0.70	7.19	8.70	11.14	4.6	6.0	8.7
A16	0.66	11	52	48	0.57	0.62	0.70	7.10	8.60	11.02	2.7	3.5	5.1
B1	0.97	12	46	54	0.60	0.64	0.72	6.81	8.27	10.63	3.9	5.1	7.4
B2	0.62	10	44	56	0.61	0.65	0.73	7.22	8.74	11.18	2.7	3.5	5.1
B3	0.27	11	37	63	0.64	0.69	0.77	7.10	8.60	11.02	1.2	1.6	2.3
B4	1.03	12	51	49	0.57	0.62	0.70	6.83	8.28	10.65	4.0	5.3	7.6
C1	0.68	13	49	51	0.53	0.58	0.66	6.54	7.94	10.24	2.4	3.1	4.6
D1	0.46	10	25	75	0.67	0.72	0.80	7.23	8.75	11.20	2.2	2.9	4.1
D2	1.52	16	50	50	0.58	0.62	0.70	6.05	7.37	9.55	5.3	7.0	10.2
E1	0.90	11	44	56	0.61	0.65	0.73	7.02	8.51	10.91	3.8	5.0	7.2
E2	0.82	10	47	53	0.59	0.64	0.72	7.26	8.78	11.23	3.5	4.6	6.6
E3	0.76	11	49	51	0.58	0.63	0.71	6.96	8.43	10.82	3.1	4.1	5.9
F1	1.74	11	47	53	0.59	0.64	0.72	7.10	8.60	11.02	7.3	9.6	13.8
G1	0.60	10	21	79	0.71	0.76	0.85	7.26	8.78	11.23	3.1	4.0	5.7
G2	1.10	12	50	50	0.58	0.63	0.71	6.84	8.30	10.66	4.4	5.7	8.3
G3	1.00	10	49	51	0.58	0.63	0.71	7.26	8.78	11.23	4.2	5.5	8.0
G4	1.32	13	49	51	0.59	0.63	0.71	6.67	8.09	10.42	5.2	6.8	9.8
G5	0.23	10	18	82	0.73	0.78	0.86	7.26	8.78	11.23	1.2	1.6	2.2
G6	1.08	14	44	56	0.56	0.61	0.69	6.40	7.78	10.05	3.9	5.1	7.5
H1	0.36	10	21	79	0.71	0.76	0.85	7.26	8.78	11.23	1.9	2.4	3.5
H2	1.23	12	52 41	48 59	0.57	0.62	0.70	6.75	8.20	10.54	4.7	6.2	9.0
H3	0.51	13 15	48	59	0.58 0.54	0.62 0.58	0.71 0.67	6.57	7.98	10.28	1.9 3.1	2.5	
H4	0.93	16	51	49	0.54	0.56	0.67	6.14 6.01	7.48 7.32	9.68 9.49	3.1	4.1 4.9	6.0 7.2
H5	1.17	16	51	49	0.52	0.57					3. <i>1</i> 4.7		
H6 J	1.26 7.40	11	54	49	0.56	0.62	0.70 0.68	6.45 6.89	7.84 8.36	10.12 10.74	28.6	6.2 37.4	9.0 54.4

							25 - YEA	RINLE	T FLOW CALCU	LATION TABLE							
INLET	DRAINAGE	Q	Q PASS	Q SPILL	Q ADD	Q TOTAL	SLOPE	а	Yo	PAVEMENT	PONDED	Qa/La	La	LENGTH	L/La	a/Yo	Q/Qa
NUMBER	AREA NO.	(CFS)	(CFS)	(CFS)	(CFS)	(QA) (CFS)	(%)	(FT)	(FT)	WIDTH	WIDTH (FT)		(FT)	(FT)			
A1	A1	8.0	0.0	0.0	0.0	8.0	LP	0.42	0.32	33	-	-	-	15	-	1.33	1.00
A2	A2	2.1	0.0	0.0	0.0	2.1	0.50	0.42	0.16	33	2.88	0.62	3.44	10	2.91	2.62	1.00
A3	А3	3.7	0.0	0.0	0.0	3.7	1.30	0.42	0.16	33	2.94	0.62	5.86	10	1.71	2.57	1.00
A4	A4	3.1	0.0	0.0	0.0	3.1	1.40	0.42	0.15	33	2.75	0.61	5.10	10	1.96	2.74	1.00
A5	A5	6.3	0.0	0.0	0.0	6.3	1.10	0.42	0.20	33	3.69	0.66	9.50	10	1.05	2.08	1.00
A6	A6	3.6	0.0	0.0	0.0	3.6	0.90	0.42	0.17	33	3.11	0.63	5.64	10	1.77	2.44	1.00
A7	Α7	6.1	0.0	0.0	0.0	6.1	3.40	0.42	0.16	33	2.97	0.63	9.69	10	1.03	2.55	1.00
A8	A8	10.2	0.0	0.0	0.1	10.3	LP	0.42	0.37	33	-	-	-	15	-	1.12	1.00
A9	A9	4.7	0.0	0.0	0.0	4.7	1.90	0.42	0.17	33	3.01	0.63	7.52	10	1.33	2.51	1.00
A10	A10	1.1	0.0	0.0	3.2	4.3	0.80	0.42	0.19	33	3.41	0.65	6.68	10	1.50	2.24	1.00
A11	A11	3.6	0.0	0.0	0.0	3.6	1.10	0.42	0.17	33	3.02	0.63	5.78	10	1.73	2.50	1.00
A12	A12	6.1	0.0	0.0	0.0	6.1	2.40	0.42	0.18	33	3.17	0.64	9.65	10	1.04	2.39	1.00
A13	A13	2.6	0.0	0.0	0.0	2.6	3.60	0.42	0.12	33	2.18	0.59	4.38	10	2.28	3.43	1.00
A14	A14	3.9	0.0	0.0	0.0	3.9	6.10	0.42	0.13	33	2.30	0.59	6.61	10	1.51	3.26	1.00
A15	A15	6.0	0.0	0.0	0.0	6.0	6.10	0.42	0.15	33	2.67	0.61	9.83	10	1.02	2.82	1.00
A16	A16	3.5	0.0	0.0	0.0	3.5	3.10	0.42	0.14	33	2.49	0.60	5.81	10	1.72	3.02	1.00
B1	B1	5.1	0.0	0.0	0.0	5.1	2.20	0.42	0.17	33	3.02	0.63	8.16	10	1.23	2.51	1.00
B2	B2	3.5	0.0	0.0	0.0	3.5	2.00	0.42	0.15	33	2.69	0.61	5.76	10	1.74	2.80	1.00
B3	B3	1.6	0.0	0.0	0.0	1.6	1.20	0.42	0.13	33	2.24	0.59	2.71	10	3.69	3.35	1.00
B4	B4	5.3	0.0	0.0	0.0	5.3	2.30	0.42	0.17	33	3.03	0.63	8.38	10	1.19	2.50	1.00
C1	C1	3.1	0.0	0.0	0.0	3.1	0.80	0.42	0.17	33	3.03	0.63	4.96	10	2.02	2.50	1.00
D1	D1	2.9	0.0	0.0	0.0	2.9	0.40	0.42	0.18	33	3.34	0.64	4.50	10	2.22	2.28	1.00
D2	D2	7.0	0.1	0.0	0.0	6.9	0.50	0.42	0.24	33	4.43	0.70	9.89	10	1.01	1.77	1.00
E1	E1	5.0	0.0	0.0	0.0	5.0	2.40	0.42	0.16	33	2.95	0.62	8.01	10	1.25	2.56	1.00
E2	E2	4.6	0.0	0.0	0.0	4.6	1.40	0.42	0.17	33	3.15	0.63	7.22	10	1.38	2.41	1.00
E3	E3	4.1	0.0	0.0	0.0	4.1	4.00	0.42	0.14	33	2.51	0.60	6.74	10	1.48	3.00	1.00
F1	F1	9.6	3.2	0.0	0.0	6.4	2.20	0.42	0.18	33	3.27	0.64	9.96	10	1.00	2.33	1.00
G1	G1	4.0	0.0	0.0	0.4	4.4	LP	0.42	0.21	33	-	-	-	15	-	1.98	1.00
G2	G2	5.7	0.0	0.0	0.0	5.7	1.60	0.42	0.18	33	3.32	0.64	8.87	10	1.13	2.29	1.00
G3	G3	5.5	0.0	0.0	0.0	5.5	1.50	0.42	0.18	33	3.33	0.64	8.60	10	1.16	2.29	1.00
G4	G4	6.8	0.4	0.0	0.0	6.4	1.89	0.42	0.18	33	3.36	0.64	9.89	10	1.01	2.27	1.00
G5	G5	1.6	0.0	0.0	0.0	1.6	2.00	0.42	0.11	33	2.05	0.58	2.72	10	3.67	3.66	1.00
G6	G6	5.1	0.0	0.0	0.0	5.1	1.90	0.42	0.17	33	3.10	0.63	8.10	10	1.23	2.45	1.00
H1	H1	2.4	0.0	0.0	0.0	2.4	2.10	0.42	0.13	33	2.35	0.59	4.10	10	2.44	3.19	1.00
H2	H2	6.2	0.0	0.0	0.0	6.2	3.40	0.42	0.17	33	3.00	0.63	9.92	10	1.01	2.53	1.00
H3	H3	2.5	0.0	0.0	0.0	2.5	2.53	0.42	0.13	33	2.31	0.59	4.29	10	2.33	3.25	1.00
H4	H4	4.1	0.0	0.0	0.0	4.1	1.53	0.42	0.17	33	2.97	0.63	6.51	10	1.54	2.55	1.00
H5	H5	4.9	0.0	0.0	0.0	4.9	1.30	0.42	0.18	33	3.26	0.64	7.64	10	1.31	2.33	1.00
H6	H6	6.2	0.0	0.0	0.0	6.2	1.00	0.42	0.20	33	3.72	0.66	9.29	10	1.08	2.06	1.00

AREAS	T _c	C ₁₀ *A	C ₂₅ *A	C ₁₀₀ *A	I ₁₀	I ₂₅	I 100	Q ₁₀	Q ₂₅	Q ₁₀₀
COMBINED	(Min.)				In/Hr	In/Hr	In/Hr	CFS	CFS	CFS
A15-A16	11	1.02	1.09	1.24	7.10	8.60	11.02	7.2	9.4	13.6
A14-A16	12	1.45	1.57	1.78	6.77	8.21	10.56	9.8	12.9	18.8
A13-A16	12	1.73	1.86	2.11	6.77	8.21	10.56	11.7	15.3	22.2
A12-A16	12	2.37	2.56	2.90	6.77	8.21	10.56	16.1	21.0	30.6
A11-A16	12	2.76	2.98	3.37	6.77	8.21	10.56	18.7	24.5	35.5
E2-E3	11	0.93	1.00	1.13	6.96	8.43	10.82	6.5	8.5	12.2
E1-E3	11	1.48	1.59	1.79	6.96	8.43	10.82	10.3	13.4	19.4
F	11	2.51	2.70	3.04	6.96	8.43	10.82	17.5	22.8	32.9
J4: A12-A17,E,F	12	6.75	7.27	8.20	6.77	8.21	10.56	45.7	59.7	86.6
A10-A16	12	6.87	7.40	8.34	6.77	8.21	10.56	46.5	60.8	88.1
A9-A16	13	7.42	7.99	9.00	6.64	8.07	10.39	49.3	64.4	93.5
A8-A16	13	8.52	9.17	10.34	6.64	8.07	10.39	56.6	74.0	107.4
D1-D2	16	1.19	1.28	1.44	6.05	7.37	9.55	7.2	9.4	13.7
J3:A8-A16,D	16	9.70	10.45	11.78	6.05	7.37	9.55	58.7	77.0	112.5
J2:A8-A16,C	16	10.07	10.84	12.22	6.05	7.37	9.55	60.9	79.9	116.8
A7-A16	16	10.71	11.53	13.00	6.05	7.37	9.55	64.8	85.0	124.2
A6-A16	16	11.12	11.98	13.50	6.05	7.37	9.55	67.3	88.3	129.0
A5-A16	16	11.79	12.69	14.30	6.05	7.37	9.55	71.3	93.6	136.6
A4-A16	17	12.20	13.14	14.80	5.81	7.09	9.21	70.9	93.1	136.3
A3-A16	17	12.61	13.57	15.29	5.81	7.09	9.21	73.3	96.3	140.9
A2-A16	17	12.84	13.82	15.57	5.81	7.09	9.21	74.6	98.0	143.5
B3-B4	12	0.76	0.82	0.93	6.83	8.28	10.65	5.2	6.8	9.9
B2-B4	12	1.14	1.22	1.38	6.83	8.28	10.65	7.8	10.1	14.7
B1-B4	12	1.71	1.84	2.08	6.81	8.27	10.63	11.7	15.2	22.1
J1: A2-A7, B	17	14.55	15.67	17.65	5.81	7.09	9.21	84.6	111.1	162.6
OUTFALL: A, B, C, D, E, F	17	15.51	16.70	18.81	5.81	7.09	9.21	90.2	118.4	173.3
G5-G6	14	0.78	0.84	0.94	6.40	7.78	10.05	5.0	6.5	9.5
G4-G6	14	1.55	1.67	1.89	6.40	7.78	10.05	9.9	13.0	19.0
G3-G6	14	2.14	2.30	2.60	6.40	7.78	10.05	13.7	17.9	26.1
H5-H6	16	1.34	1.45	1.65	6.01	7.32	9.49	8.1	10.6	15.6
H4-H6	16	1.85	2.00	2.27	6.01	7.32	9.49	11.1	14.6	21.5
H3-H6	16	2.14	2.32	2.63	6.01	7.32	9.49	12.9	17.0	24.9
H2-H6	16	2.84	3.07	3.48	6.01	7.32	9.49	17.1	22.5	33.1
H1-H6	16	3.10	3.35	3.79	6.01	7.32	9.49	18.6	24.5	36.0
J5: G3-G6, H	16	5.24	5.66	6.39	6.01	7.32	9.49	31.5	41.4	60.7
G2-G6	16	5.88	6.34	7.17	6.01	7.32	9.49	35.3	46.4	68.0
OUTFALL: G, H	16	6.31	6.80	7.67	6.01	7.32	9.49	37.9	49.8	72.8

	L								1							_	
							100 - VE	A D I NII I	ET ELOW CALCI	JLATION TABLE							
INLET	DRAINAGE	Q	Q PASS	Q SPILL	Q ADD	Q TOTAL	SLOPE	a	Yo	PAVEMENT	PONDED	Qa/La	La	LENGTH	L/La	a/Yo	Q/Q
NUM BER	AREA NO.	(CFS)	(CFS)	(CFS)	(CFS)	(QA) (CFS)	(%)	(FT)	(FT)	WIDTH	WIDTH (FT)	Qu. 2	(FT)	(FT)			~~
A1	A1	11.6	0.0	0.0	8.0	19.6	LP	0.42	0.57	33	-	_	-	15	_	0.73	1.0
A2	A2	3.1	4.5	0.0	8.3	6.9	0.50	0.42	0.24	33	4.42	0.70	9.88	10	1.01	1.77	1.0
A2 A3	A2 A3	5.3	8.3	0.0	9.5	6.5	1.30	0.42	0.24	33	3.62	0.70	9.87	10	1.01	2.12	1.0
A3 A4	A3	4.6	9.5	0.0	11.4	6.5	1.40	0.42	0.20	33	3.57	0.66	9.91	10	1.01	2.12	1.0
A5	A5	9.0	2.4	0.0	0.0	6.6	1.40	0.42	0.20	33	3.75	0.66	9.93	10	1.01	2.15	1.0
A6	A5 A6	5.2	0.0	0.0	1.3	6.5	0.90	0.42	0.21	33	3.87	0.67	9.66	10	1.04	1.99	1.0
A7	A7	8.7	11.4	0.0	8.9	6.2	3.40	0.42	0.17	33	2.99	0.63	9.91	10	1.01	2.53	1.0
A8	A7	14.7	6.4	2.5	12.2	18.0	LP	0.42	0.54	33	2.99	- 0.03	9.91	15	1.01	0.77	1.0
A9	A9	6.8	1.4	0.0	1.0	6.4	1.90	0.42	0.19	33	3.36	0.65	9.96	10	1.00	2.27	1.0
A10	A10	1.6	7.5	0.0	12.6	6.7	0.80	0.42	0.19	33	4.01	0.68	9.91	10	1.00	1.93	1.00
A11	A11	5.2	0.0	0.0	0.0	5.2	1.10	0.42	0.19	33	3.45	0.65	8.08	10	1.24	2.21	1.00
A12	A12	8.9	5.1	0.0	2.6	6.4	2.40	0.42	0.18	33	3.21	0.64	9.97	10	1.00	2.37	1.00
A13	A13	3.7	0.0	0.0	0.0	3.7	3.60	0.42	0.14	33	2.47	0.60	6.16	10	1.62	3.04	1.0
A14	A14	5.7	0.0	0.0	0.0	5.7	6.10	0.42	0.15	33	2.62	0.61	9.40	10	1.06	2.87	1.0
A15	A15	8.7	2.6	0.0	0.0	6.1	6.10	0.42	0.15	33	2.68	0.61	9.93	10	1.01	2.81	1.0
A16	A16	5.1	0.0	0.0	0.0	5.1	3.10	0.42	0.16	33	2.83	0.62	8.18	10	1.22	2.67	1.0
B1	B1	7.4	1.1	0.0	0.0	6.3	2.20	0.42	0.18	33	3.26	0.64	9.87	10	1.01	2.34	1.0
B2	B2	5.1	0.0	0.0	0.0	5.1	2.00	0.42	0.17	33	3.06	0.63	8.05	10	1.24	2.48	1.0
B3	B3	2.3	0.0	0.0	0.0	2.3	1.20	0.42	0.14	33	2.53	0.60	3.80	10	2.63	2.97	1.0
B4	B4	7.6	1.3	0.0	0.0	6.3	2.30	0.42	0.18	33	3.24	0.64	9.93	10	1.01	2.35	1.0
C1	C1	4.6	0.0	0.0	0.0	4.6	0.80	0.42	0.19	33	3.48	0.65	7.03	10	1.42	2.20	1.0
D1	D1	4.1	0.0	0.0	0.0	4.1	0.40	0.42	0.21	33	3.81	0.67	6.22	10	1.61	2.02	1.0
D2	D2	10.2	3.3	0.0	0.0	6.9	0.50	0.42	0.24	33	4.43	0.70	9.93	10	1.01	1.77	1.0
E1	<u>5_</u> E1	7.2	0.9	0.0	0.0	6.3	2.40	0.42	0.18	33	3.20	0.64	9.91	10	1.01	2.37	1.0
E2	E2	6.6	0.1	0.0	0.0	6.5	1.40	0.42	0.20	33	3.57	0.66	9.92	10	1.01	2.14	1.0
E3	E3	5.9	0.0	0.0	0.0	5.9	4.00	0.42	0.16	33	2.85	0.62	9.47	10	1.06	2.65	1.0
F1	F1	13.8	7.5	0.0	0.0	6.3	2.20	0.42	0.18	33	3.26	0.64	9.88	10	1.01	2.34	1.0
G1	G1	5.7	0.0	0.0	10.6	16.3	LP	0.42	0.51	33	_	_	-	15	-	0.83	1.0
G2	G2	8.3	6.1	0.0	4.3	6.5	1.60	0.42	0.19	33	3.48	0.65	9.93	10	1.01	2.20	1.0
G3	G3	8.0	1.5	0.0	0.0	6.5	1.50	0.42	0.19	33	3.52	0.65	9.92	10	1.01	2.17	1.0
G4	G4	9.8	4.5	0.0	1.1	6.4	1.89	0.42	0.19	33	3.37	0.65	9.96	10	1.00	2.26	1.0
G5	G5	2.2	0.0	0.0	0.0	2.2	2.00	0.42	0.13	33	2.30	0.59	3.79	10	2.64	3.26	1.0
G6	G6	7.5	1.1	0.0	0.0	6.4	1.90	0.42	0.19	33	3.36	0.64	9.92	10	1.01	2.27	1.0
H1	H1	3.5	0.0	0.0	0.0	3.5	2.10	0.42	0.15	33	2.66	0.61	5.69	10	1.76	2.83	1.0
H2	H2	9.0	2.8	0.0	0.0	6.2	3.40	0.42	0.17	33	3.00	0.63	9.93	10	1.01	2.53	1.0
H3	H3	3.7	0.0	0.0	0.0	3.7	2.53	0.42	0.15	33	2.63	0.61	6.08	10	1.64	2.86	1.0
H4	H4	6.0	0.0	0.0	0.0	6.0	1.53	0.42	0.19	33	3.41	0.65	9.27	10	1.08	2.24	1.0
H5	H5	7.2	3.0	0.0	2.3	6.5	1.30	0.42	0.20	33	3.63	0.66	9.92	10	1.01	2.12	1.00
H6	H6	9.0	2.3	0.0	0.0	6.7	1.00	0.42	0.21	33	3.84	0.67	9.98	10	1.00	2.01	1.00

REVISION							
	O. Dooming Inc	soli, brigalice & Doering, ilic.	Surveying		North Office 12129 RR 620 N., Ste. 600	Austin, Texas 78750	Fax No. (512) 280-5165
	Confeed Discourse		Civil Engineering Surveying	FIRM ID #F3791	Main Office 5501 West William Cannon Dr.	Austin, Texas 78749	Phone No. (512) 280-5160

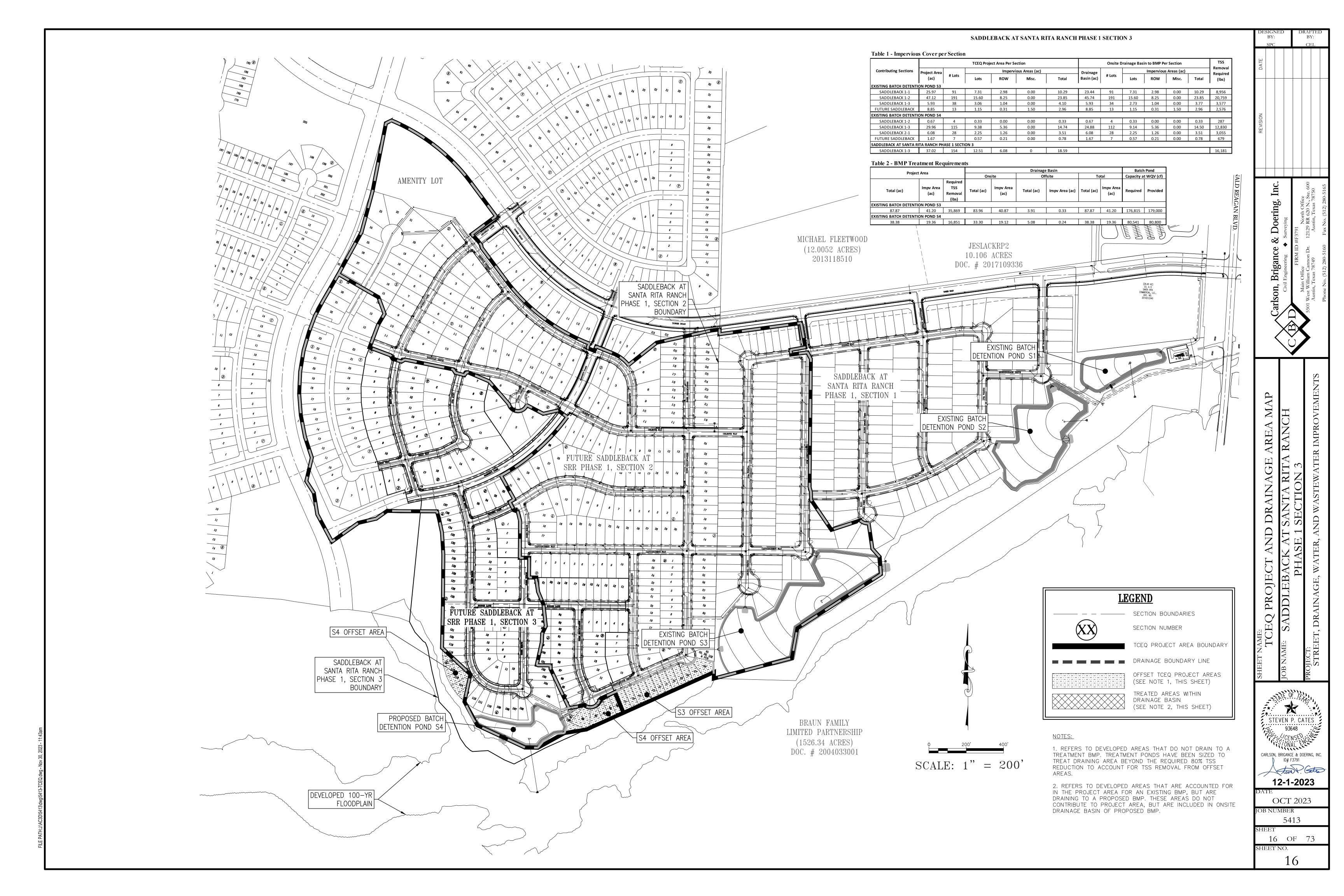
TER IMPROVEMENTS

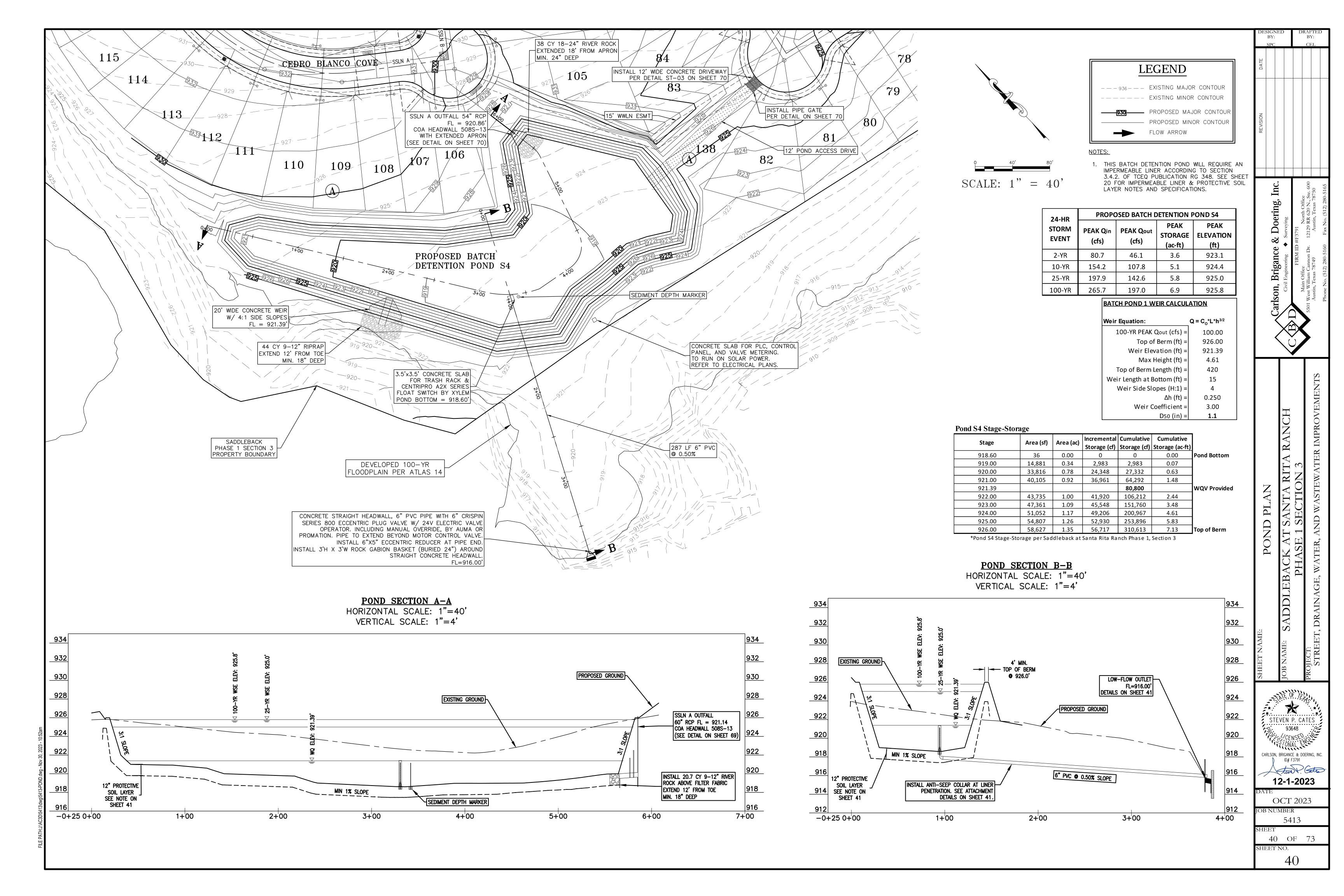
STE	SHEET NAME: DRAINAGE AREA CAI
VEN 936 SOON, BRIGANO	JOB NAME: SADDLEBACK AT SANTA RIT
648 NSED AL	PHASE 1 SECTION 3
\$\$. *	PROJECT: STREET, DRAINAGE, WATER, AND WASTEWAT

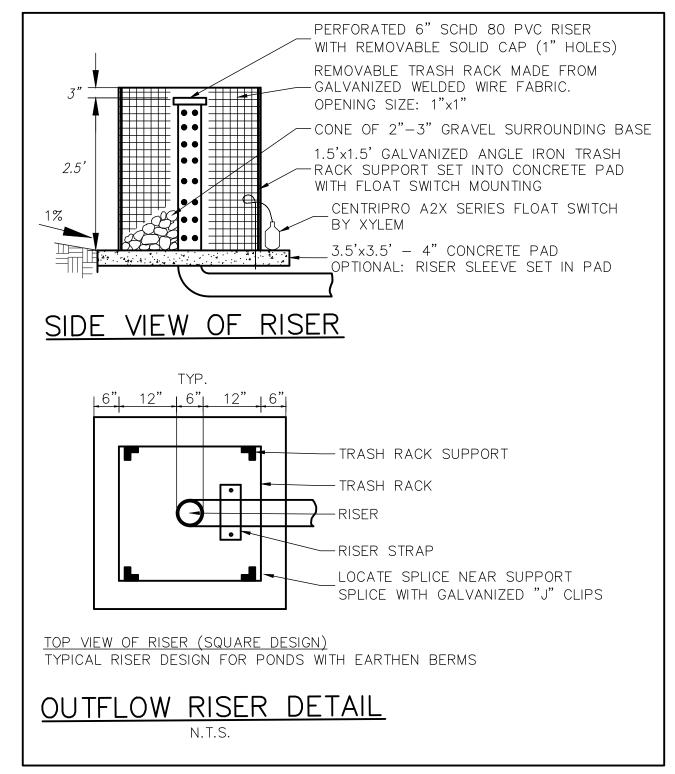
SADDLEBACK AT SANTA RITA RANCH
PHASE 1 SECTION 3

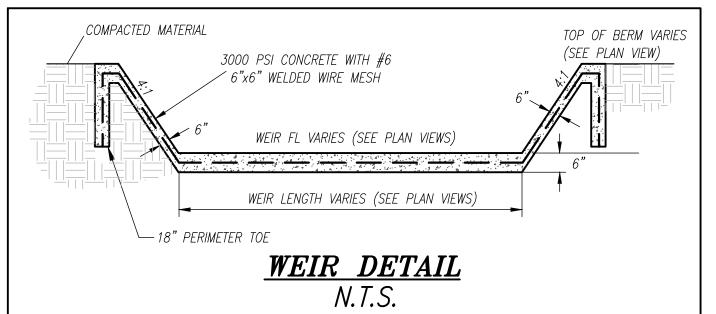
12-1-2023 OCT 2023 5413 15 OF 73

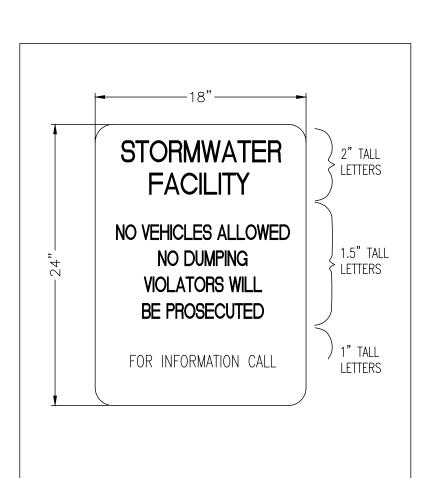
15

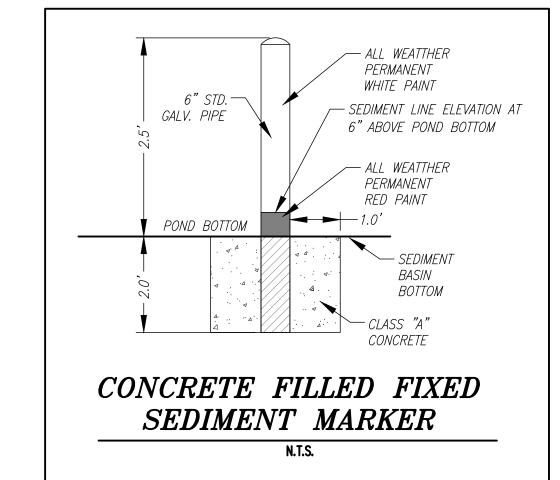


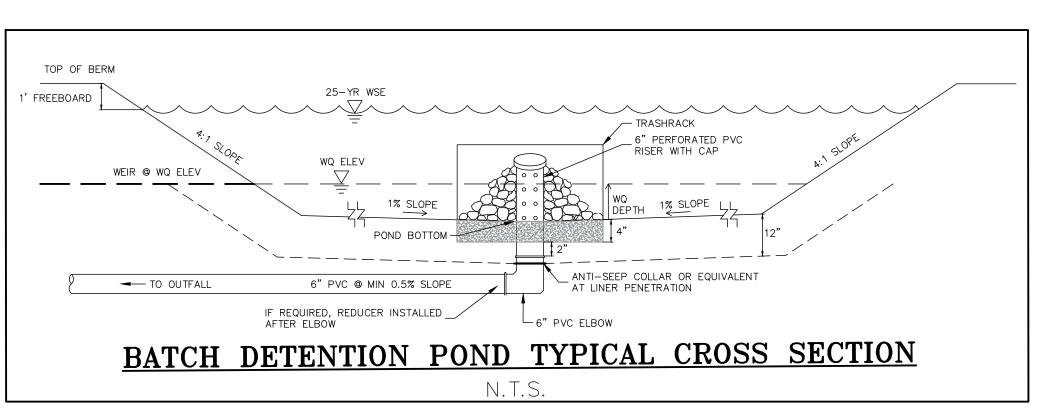


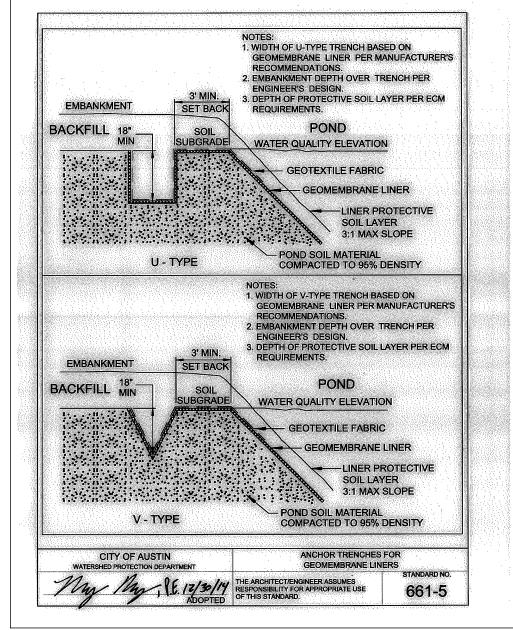


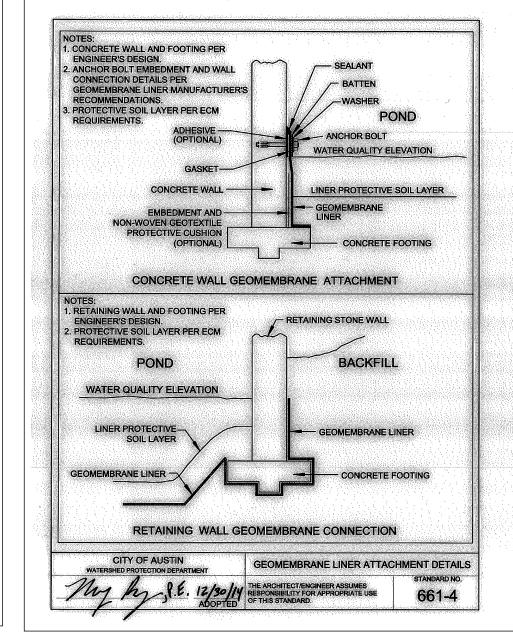












INSPECTION, MAINTENANCE, REPAIR AND RETROFIT PLAN:

TEMPORARY BMP'S:

BEST MANAGEMENT PRACTICES (BMP'S) INSTALLED DURING CONSTRUCTION WILL BE MAINTAINED IN ACCORDANCE WITH THE REQUIREMENTS OF THE EPA'S NPDES STORMWATER POLLUTION PREVENTION PROGRAM. THE CONSTRUCTION SUPERINTENDENT WILL INSPECT TEMPORARY EROSION CONTROLS ON A REGULAR BASIS AND ADJUST THE CONTROLS AND/OR REMOVE ANY SEDIMENT BUILDUP IN ACCORDANCE WITH THE EROSION/SEDIMENTATION CONTROL NOTES AND AS OTHERWISE DIRECTED BY THE OWNER OR HIS DESIGNATED REPRESENTATIVE. TEMPORARY EROSION CONTROLS SHOULD BE INSPECTED, MAINTAINED, AND REPAIRED, AT A MINIMUM, EVERY SEVEN (7) DAYS AND WITHIN 24 HOURS OF A STORM OF 0.5 INCHES OR MORE RAINFALL DEPTH. SEDIMENT SHALL BE REMOVED FROM CONTROLS WHEN 50% OF THE DESIGN HEIGHT IS EXCEEDED. FOLLOWING INSPECTION OF THE BMP'S, DEFICIENCIES SHALL BE NOTED AND CORRECTED BY THE CONTRACTOR.

PERMANENT BMP:

BATCH DETENTION BASINS

· INSPECTIONS. BASINS SHOULD BE INSPECTED AT LEAST TWICE A YEAR (ONCE DURING OR IMMEDIATELY FOLLOWING WET WEATHER) TO EVALUATE FACILITY OPERATION. WHEN POSSIBLE, INSPECTIONS SHOULD BE CONDUCTED DURING WET WEATHER TO DETERMINE IF THE POND IS MEETING THE TARGET DETENTION TIME OF 12 HOURS AND A DRAWDOWN OF NO MORE THAN 48 HOURS. IN PARTICULAR, THE DETENTION CONTROL DEVICE SHOULD BE REGULARLY INSPECTED FOR EVIDENCE OF CLOGGING, OR CONVERSELY, FOR TOO RAPID A RELEASE. IF THE DETENTION TIME IS LESS THAN 12 HOURS OR THE DRAWDOWN TIME IS EXCEEDED BY MORE THAN 24 HOURS, THEN REPAIRS SHOULD BE SCHEDULED IMMEDIATELY. DURING EACH INSPECTION, EROSION AREAS INSIDE AND DOWNSTREAM OF THE BMP SHOULD BE IDENTIFIED AND REPAIRED OR REVEGETATED IMMEDIATELY.

MOWING. THE BASIN, SIDE SLOPES, EMBANKMENT, AND EMERGENCY SPILLWAY OF A BATCH DETENTION BASIN MUST BE MOWED REGULARLY TO DISCOURAGE WOODY GROWTH AND CONTROL WEEDS. GRASS AREAS IN AND AROUND BASINS SHOULD BE MOWED AT LEAST TWICE ANNUALLY TO LIMIT VEGETATION HEIGHT TO 18 INCHES. MORE FREQUENT MOWING TO MAINTAIN AESTHETIC APPEAL MAY BE NECESSARY IN LANDSCAPED AREAS. WHEN MOWING OF GRASS IS PERFORMED, A MULCHING MOWER SHOULD BE USED, OR GRASS CLIPPINGS SHOULD BE CAUGHT AND REMOVED.

DEBRIS AND LITTER REMOVAL. DEBRIS AND LITTER WILL ACCUMULATE NEAR THE DETENTION CONTROL DEVICE AND SHOULD BE REMOVED DURING REGULAR MOWING OPERATIONS AND INSPECTIONS. PARTICULAR ATTENTION SHOULD BE PAID TO FLOATING DEBRIS THAT CAN EVENTUALLY CLOG THE CONTROL DEVICE OR RISER.

EROSION CONTROL. THE BASIN SIDE SLOPES, EMERGENCY SPILLWAY, AND EMBANKMENT ALL MAY PERIODICALLY SUFFER FROM SLUMPING AND EROSION, ALTHOUGH THIS SHOULD NOT OCCUR OFTEN IF THE SOILS ARE PROPERLY COMPACTED DURING CONSTRUCTION. REGRADING AND REVEGETATION MAY BE REQUIRED TO CORRECT THE PROBLEMS.

STRUCTURAL REPAIRS AND REPLACEMENT. WITH EACH INSPECTION, ANY DAMAGE TO THE STRUCTURAL ELEMENTS OF THE SYSTEM (PIPES, CONCRETE DRAINAGE STRUCTURES, RETAINING WALLS, ETC.) SHOULD BE IDENTIFIED AND REPAIRED IMMEDIATELY. THESE REPAIRS SHOULD INCLUDE PATCHING OF CRACKED CONCRETE, SEALING OF VOIDS, AND REMOVAL OF VEGETATION FROM CRACKS AND JOINTS. THE VARIOUS INLET/OUTLET AND RISER WORKS IN A BASIN WILL EVENTUALLY DETERIORATE AND MUST BE REPLACED. PUBLIC WORKS EXPERTS HAVE ESTIMATED THAT CORRUGATED METAL PIPE (CMP) HAS A USEFUL LIFE OF ABOUT 25 YR, WHEREAS REINFORCED CONCRETE BARRELS AND RISERS MAY LAST FROM 50 TO 75 YR.

NUISANCE CONTROL. STANDING WATER MAY OCCUR AFTER A STORM EVENT SINCE THE VALVE MAY CLOSE WITH 2 TO 3 INCHES OF WATER IN THE BASIN. FLOW SUCH AS SPRING FLOW AND RESIDENTIAL WATER USE MAY DISCHARGE INTO THE BASIN BETWEEN STORM EVENTS THAT DO NOT ENGAGE THE LEVEL SENSOR. STANDING WATER CAN CREATE NUISANCE CONDITIONS FOR NEARBY RESIDENTS. ODORS, MOSQUITOES, WEEDS, AND LITTER ARE ALL OCCASIONALLY PERCEIVED TO BE PROBLEMS. MOST OF THESE PROBLEMS ARE GENERALLY A SIGN THAT REGULAR INSPECTIONS AND MAINTENANCE ARE NOT BEING PERFORMED (E.G., MOWING, DEBRIS REMOVAL, CLEARING THE OUTLET CONTROL DEVICE). TWICE A YEAR, THE FACILITY SHOULD BE EVALUATED FOR NUISANCE CONTROL.

SEDIMENT REMOVAL. WHEN PROPERLY DESIGNED, BATCH DETENTION BASINS WILL ACCUMULATE QUANTITIES OF SEDIMENT OVER TIME. SEDIMENT ACCUMULATION IS A SERIOUS MAINTENANCE CONCERN IN BATCH DETENTION PONDS FOR SEVERAL REASONS. FIRST, SEDIMENT ACCUMULATION CAN MAKE BATCH DETENTION BASINS VERY UNSIGHTLY. SECOND, THE SEDIMENT DEPOSITION GRADUALLY REDUCES AVAILABLE STORAGE CAPACITY RESERVED FOR POLLUTANT REMOVAL AND DETENTION WITHIN THE BASIN. THIRD, SEDIMENT TENDS TO ACCUMULATE AROUND THE OUTLET CONTROL DEVICE. THIS INCREASES THE RISK OF CLOGGING THE ORIFICE OR INTERFERING WITH THE LEVEL SENSOR, WHICH CAN IN TURN REDUCE THE TREATMENT EFFICACY OF THE FACILITY. SEDIMENT CAN BE RESUSPENDED IF ALLOWED TO ACCUMULATE OVER TIME AND ESCAPE THROUGH THE HYDRAULIC CONTROL TO DOWNSTREAM CHANNELS AND STREAMS. FOR THESE REASONS, ACCUMULATED SEDIMENT NEEDS TO BE REMOVED FROM THE BASIN WHEN SEDIMENT DEPTH EXCEEDS 6 INCHES, WHEN SEDIMENT INTERFERES WITH THE LEVEL SENSOR, WHEN THE BASIN DOES NOT DRAIN WITHIN 48 HOURS, OR AT LEAST EVERY 5 YEARS.

LOGIC CONTROLLER. THE LOGIC CONTROLLER SHOULD BE INSPECTED AS PART OF THE TWICE YEARLY INSPECTIONS. VERIFY THAT THE EXTERNAL INDICATORS (ACTIVE, CYCLE IN PROGRESS) ARE OPERATING PROPERLY BY TURNING THE CONTROLLER OFF AND ON, AND BY INITIATING A CYCLE BY TRIGGERING THE LEVEL SENSOR IN THE BASIN. THE VALVE SHOULD BE MANUALLY OPENED AND CLOSED USING THE OPEN/CLOSE SWITCH TO VERIFY VALVE OPERATION AND TO ASSIST IN INSPECTING THE VALVE FOR DEBRIS. THE SOLAR PANEL SHOULD BE INSPECTED AND ANY DUST OR DEBRIS ON THE PANEL SHOULD BE CAREFULLY REMOVED. THE CONTROLLER AND ALL OTHER CIRCUITRY AND WIRING SHOULD BE INSPECTED FOR SIGNS OF CORROSION, DAMAGE FROM INSECTS, WATER LEAKS, OR OTHER DAMAGE. AT THE END OF THE INSPECTION, THE CONTROLLER SHOULD BE RESET.

ULTIMATELY, THESE FACILITIES WILL BE OWNED, OPERATED AND MAINTAINED BY THE PROPOSED WILLIAMSON COUNTY MUD NO. 19C UNTIL THE OWNERSHIP OF FACILITIES IS TRANSFERRED TO THE MUD, SANTA RITA C7 INVESTMENTS LLC WILL BE RESPONSIBLE FOR MAINTENANCE OF THESE FACILITIES IN ACCORDANCE WITH THE ABOVE STATED REQUIREMENTS.

IMPERMEABLE LINER NOTES

IMPERMEABLE LINERS SHOULD BE USED FOR WATER QUALITY BASINS (RETENTION, EXTENDED DETENTION, SAND FILTERS, WET PONDS, CONSTRUCTED WETLANDS, AND BATCH DETENTION PONDS) LOCATED OVER THE RECHARGE ZONE AND IN AREAS WITH THE POTENTIAL FOR GROUNDWATER CONTAMINATION. IMPERMEABLE LINERS MAY BE CLAY, CONCRETE OR GEOMEMBRANE. IF GEOMEMBRANE IS USED, SUITABLE GEOTEXTILE FABRIC SHOULD BE PLACED ON THE TOP AND BOTTOM OF THE MEMBRANE FOR PUNCTURE PROTECTION AND THE LINERS COVERED WITH A MINIMUM OF 6 INCHES OF COMPACTED TOPSOIL. THE TOPSOIL SHOULD BE STABILIZED WITH APPROPRIATE VEGETATION. CLAY LINERS SHOULD MEET THE SPECIFICATIONS IN TABLE 3-6 AND HAVE A MINIMUM THICKNESS OF 12 INCHES.

TABLE 3-6 CLAY LINER SPECIFICATIONS (COA, 2004)					
PROPERTY	TEST METHOD	UNIT	SPECIFICATION (MIN.)		
PERMEABILITY	ASTM D-2434	%	1 x 10 ⁻⁶		
PLASTICITY INDEX OF CLAY	ASTM D-423 & D-424	%	NOT LESS THAN 15		
LIQUID LIMIT OF CLAY	ASTM D-2216	%	NOT LESS THAN 30		
CLAY PARTICLES PASSING	ASTM D-422	%	NOT LESS THAN 30		
CLAY COMPACTION	ASTM D-2216	%	95% OF STANDARD PROCTOR DENSITY		

IF A GEOMEMBRANE LINER IS USED IT SHOULD HAVE A MINIMUM THICKNESS OF 30 MILS AND BE ULTRAVIOLET RESISTANT. THE GEOTEXTILE FABRIC (FOR PROTECTION OF GEOMEMBRANE) SHOULD BE NONWOVEN GEOTEXTILE FABRIC AND MEET THE SPECIFICATIONS IN TABLE 3-7.

TABLE 3-7 GEOTEXTILE FABRIC SPECIFICATIONS (COA, 2004)						
PROPERTY	TEST METHOD	UNIT	SPECIFICATION (MIN.)			
UNIT WEIGHT		OZ/YD²	8			
FILTRATION RATE		IN/SEC	0.0800			
PUNCTURE STRENGTH	ASTM D-751*	lb	125			
MULLEN BURST STRENGTH	ASTM-D751	psi	400			
TENSILE STRENGTH	ASTM D-1682	lb	200			
EQUIV. OPENING SIZE	US STANDARD SIEVE	No.	80			
	*MODIFIED					

PROTECTIVE SOIL LAYER NOTES

THE WET AND BATCH DETENTION PONDS WILL BE CONSTRUCTED WITH A MINIMUM OF 12" OF A PROTECTION SOIL LAYER ABOVE THE SELECTED IMPERMEABLE LINER SO THAT PLANTINGS CAN BE PROPERLY INSTALLED ABOVE THE LINER AND LINER INTEGRITY CAN BE MAINTAINED.

MATERIAL PLACED OVER LINER TO CONSIST OF SOIL THAT CLASSIFIES PER USDA STANDARDS AS A SANDY CLAY LOAM, SANDY CLAY, OR CLAY LOAM. THE SOIL SHALL BE FREE OF ORGANICS AND ANGULAR/SHARP MATERIALS, AND SHALL CONTAIN NO MATERIALS GREATER THAN 1" IN DIAMETER. MATERIAL TO BE PLACED AND COMPACTED IN TWO UNIFORM LIFTS.

DESIGNED BY:
SPC CEL

Carlson, Brigance & Doering, Inc.

Civil Engineering & Surveying

FIRM ID #F3791

Main Office

5501 West William Cannon Dr. 12129 RR 620 N., Ste. 600

Austin, Texas 78749

Austin, Texas 78749

Austin, Texas 78750

POND DETAILS

DLEBACK AT SANTA RITA RANCH
PHASE 1 SECTION 3

STEVEN P. CATES

93648

CARLSON, BRIGANCE & DOERING, INC.

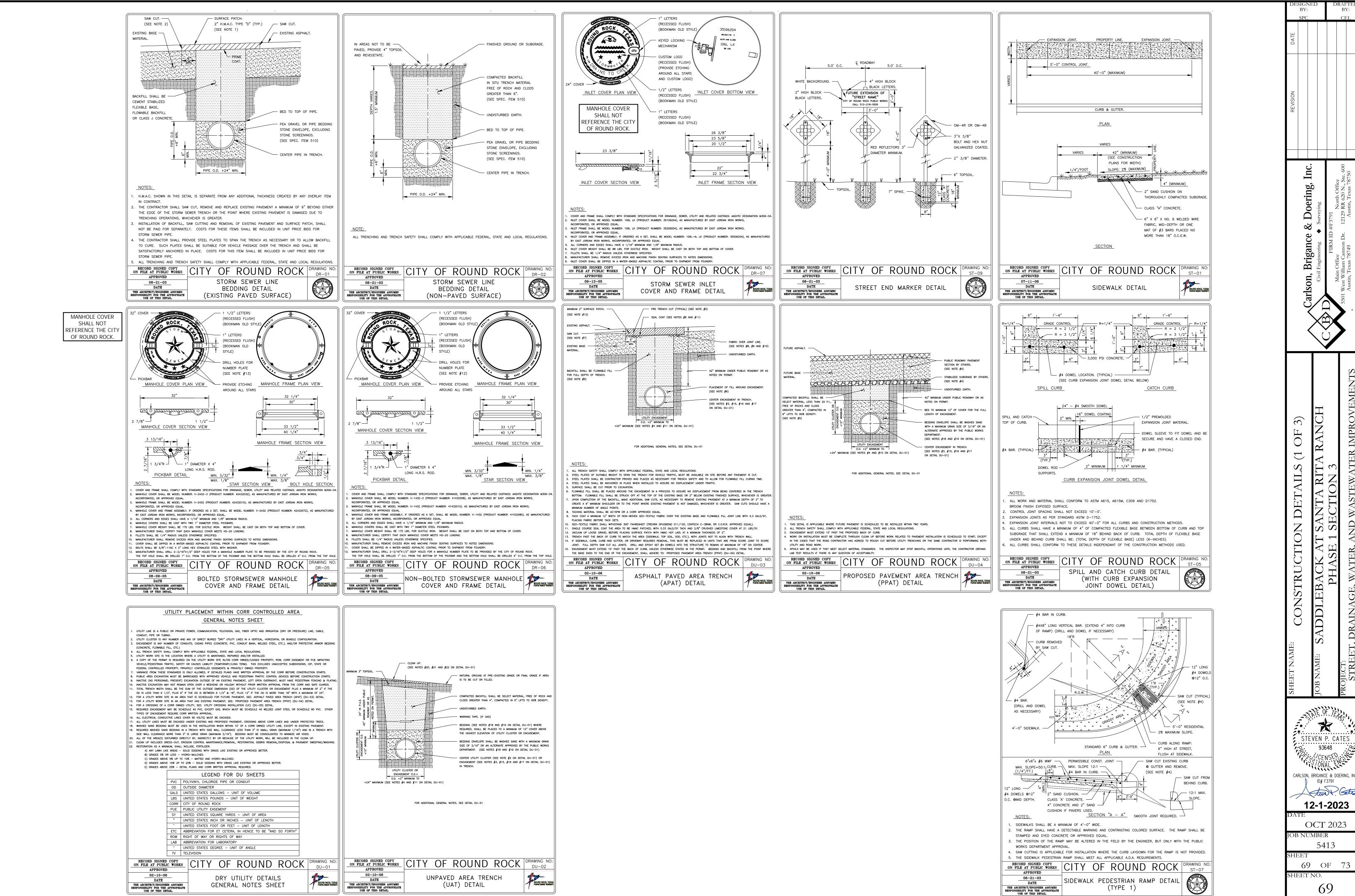
1D# F3791

12-1-2023
DATE
OCT 2023
OB NUMBER
5413

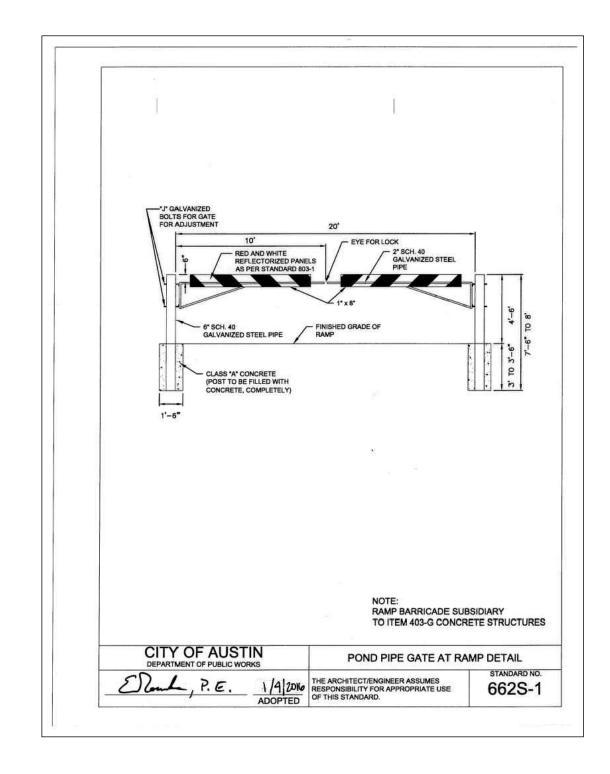
Store Cate

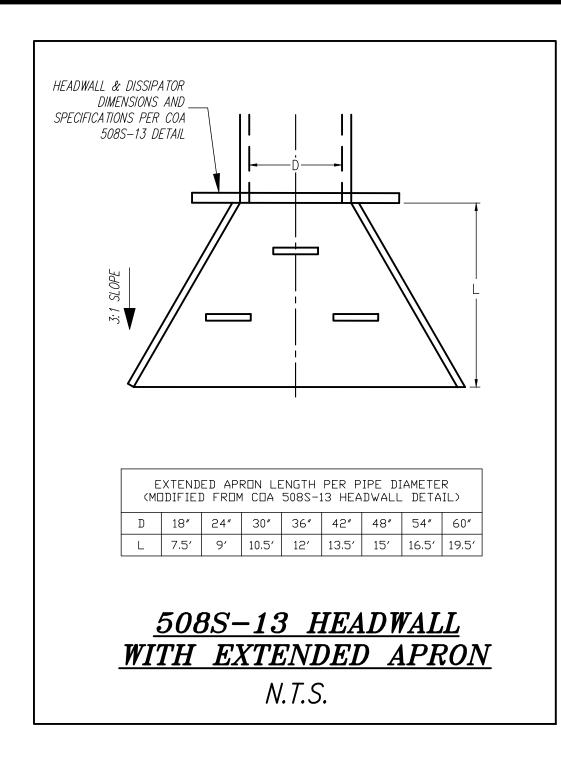
5413 SHEET 41 OF 73

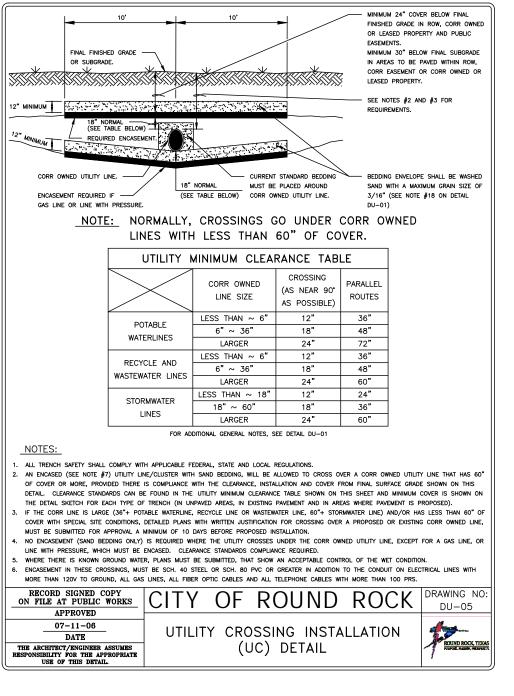
неет no. **41**

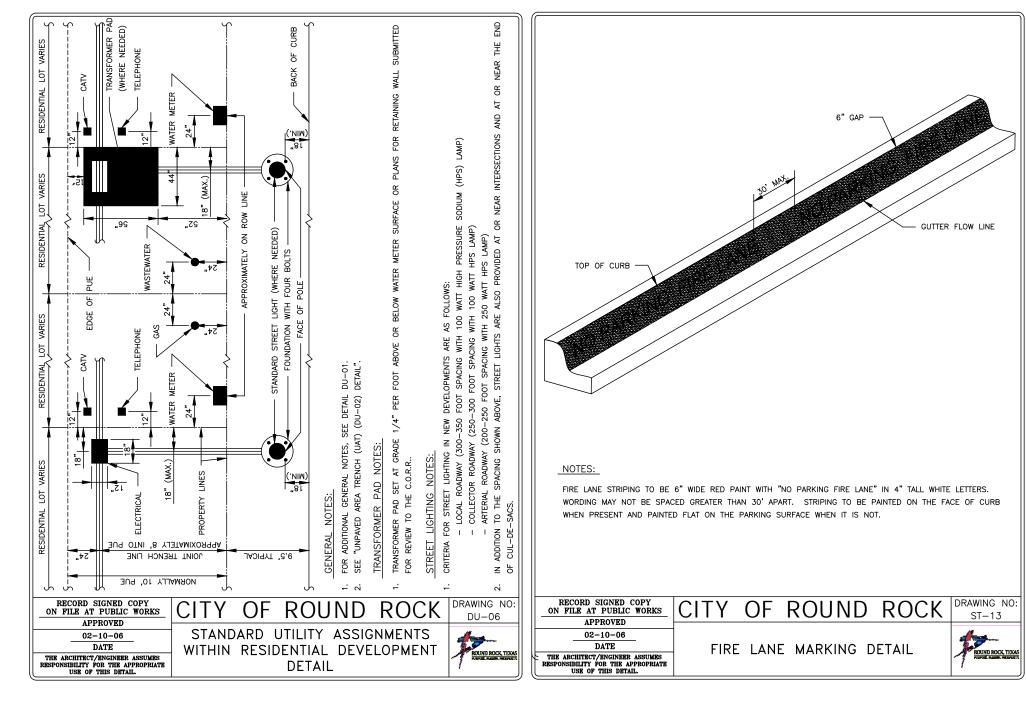


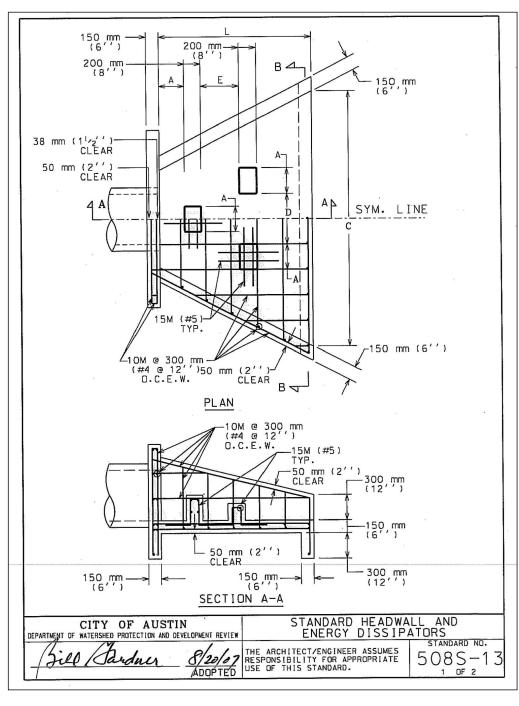
CARLSON, BRIGANCE & DOERING, INC two Cate

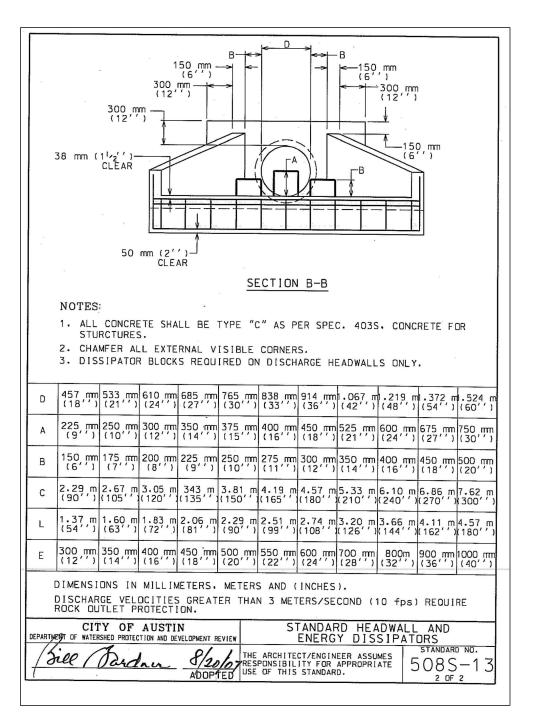


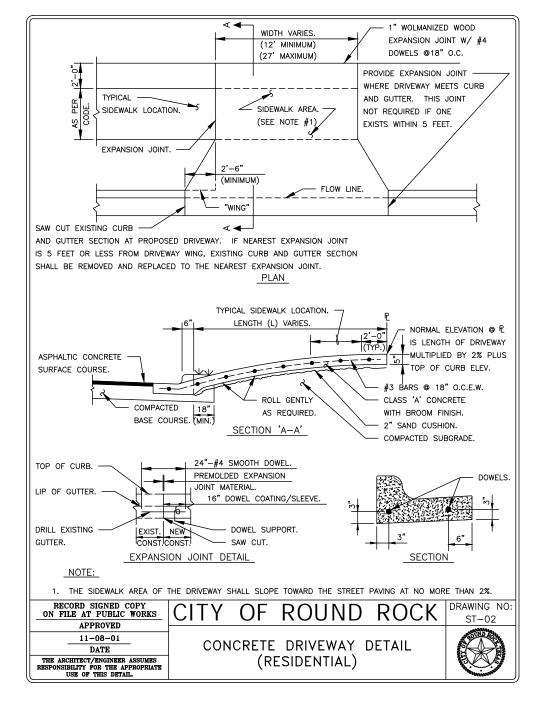


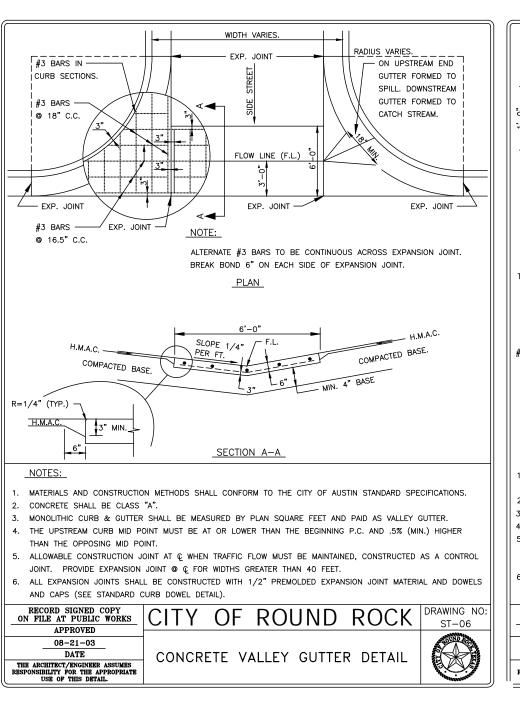


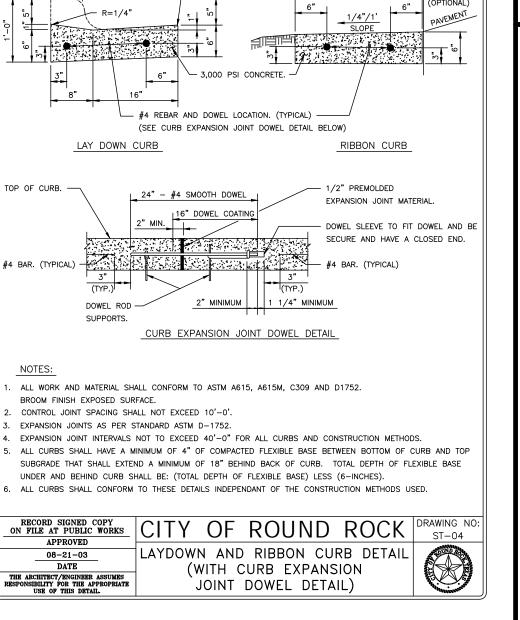




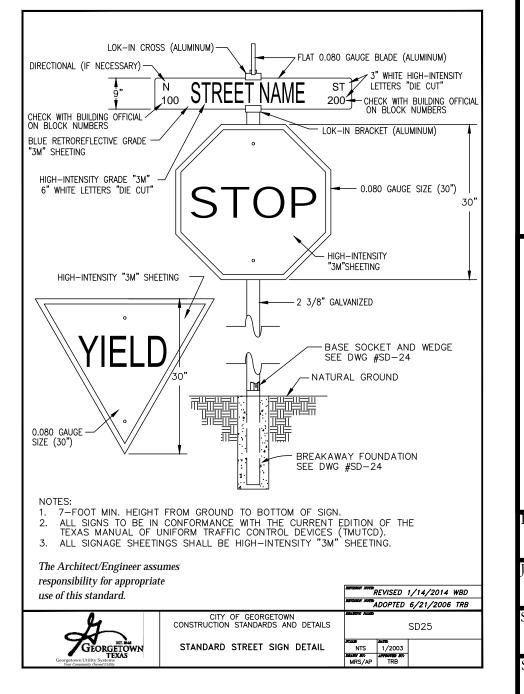








WIDTH AS SPECIFIED



ILE PATH:J:\AC3D\5413\dwg\5413-CONSTRUCTION DETAILS.dwg - Nov 30, 2023 - 11:21am

OCT 2023

JOB NUMBER

5413

SHEET

70 OF 73

STEVEN P. CATES

93648 .2 CENSED GINE

CARLSON, BRIGANCE & DOERING, INC.

12-1-2023

ID# F3791

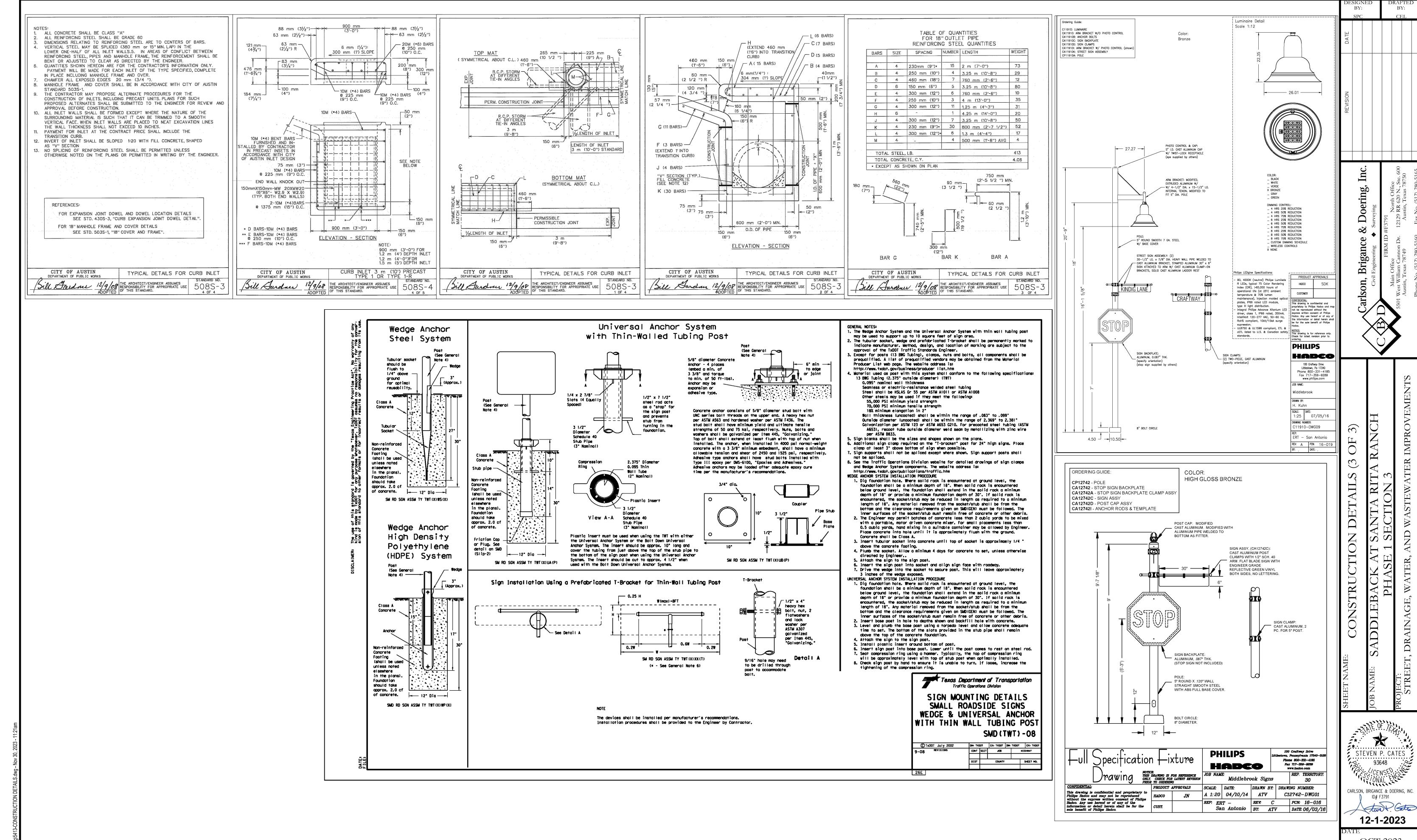
RI ON

SADDLEBACK PHAS

DETAIL

CONSTRUC

sheet no.



12-1-2023
OATE
OCT 2023
OB NUMBER
5413
HEET
71 OF 73

бнеет NO. 71