## TCEQ EDWARDS AQUIFER ROADWAY APPLICATION

### CR 143 AT DRY BERRY CREEK

CSJ: 0914-05-203, ETC.

### Williamson County, Texas

**FEBRUARY 2025** 

Prepared For:



AUSTIN DISTRICT

Prepared By:



**Texas Registration F-928** 

**Kimley»Horn** 

# **SECTION 1**

# EDWARDS AQUIFER APPLICATION COVER PAGE (TCEQ-20705)



#### Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with <u>30 TAC 213</u>.

#### Administrative Review

1. <u>Edwards Aquifer applications</u> must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.

To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: <u>http://www.tceq.texas.gov/field/eapp</u>.

- 2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
- 3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
- 4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.

An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.

- 5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
- 6. If the geologic assessment was completed before October 1, 2004 and the site contains "possibly sensitive" features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

Technical Review

- 1. When an application is deemed administratively complete, the technical review period begins. The regional office will distribute copies of the application to the identified affected city, county, and groundwater conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
- 2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.

- 3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied the application fee will be forfeited.
- 4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

**Mid-Review Modifications** 

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a "Mid-Review Modification". Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ's Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ's San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: County Road (CR) 143 at Dry Berry Creek Bridge Replacement; CSJ 0914-05-203					2. Regulated Entity No.:			
3. Customer Name: T	xDOT				4. Customer No.: CN 600803456			
5. Project Type: (Please circle/check one)	New	Modification		Extension		Exception		
6. Plan Type: (Please circle/check one)	WPAP CZP	SCS	UST	AST	EXP	EXT	Technical Clarification	Optional Enhanced Measures
7. Land Use: (Please circle/check one)	Residential	Non-residential		8. Sit		e (acres):	4.53	
9. Application Fee:	N/A (TxDOT)	10. Permanent E			BMP(s):		Vegetative Filter Strips	
11. SCS (Linear Ft.):	N/A	12. AST/UST (No			o. Tanks):		N/A	
13. County:	Williamso n	14. W	/aters	hed:			Dry Berry Creek	

### Application Distribution

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Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the "Texas Groundwater Conservation Districts within the EAPP Boundaries" map found at:

http://www.tceq.texas.gov/assets/public/compliance/field\_ops/eapp/EAPP%20GWCD%20map.pdf

For more detailed boundaries, please contact the conservation district directly.

Austin Region						
County:	Hays	Travis	Williamson			
Original (1 req.)	_		_X_			
Region (1 req.)			_ <u>X</u> _			
County(ies)			_X_			
Groundwater Conservation District(s)	Edwards Aquifer Authority Barton Springs/ Edwards Aquifer Hays Trinity Plum Creek	Barton Springs/ Edwards Aquifer	NA			
City(ies) Jurisdiction	Austin Buda Dripping Springs Kyle Mountain City San Marcos Wimberley Woodcreek	Austin Bee Cave Pflugerville Rollingwood Round Rock Sunset Valley West Lake Hills	Austin Cedar Park Florence Georgetown Jerrell Leander Liberty Hill Pflugerville Round Rock			

San Antonio Region						
County:	Bexar	Comal	Kinney	Medina	Uvalde	
Original (1 req.)		_				
Region (1 req.)						
County(ies)						
Groundwater Conservation District(s)	Edwards Aquifer Authority Trinity-Glen Rose	Edwards Aquifer Authority			EAA Uvalde	
City(ies) Jurisdiction	Castle Hills Fair Oaks Ranch Helotes Hill Country Village Hollywood Park San Antonio (SAWS) Shavano Park	Bulverde Fair Oaks Ranch Garden Ridge New Braunfels Schertz	NA	San Antonio ETJ (SAWS)	NA	

ustin Region

I certify that to the best of my knowledge, that the application is complete and accurate. This application is hereby submitted to TCEQ for administrative review and technical review.

Elizabeth W. Wagner, P.E.

Print Name of Customer/Authorized Agent

-25-

2/4/2025

Signature of Customer/Authorized Agent

Date

**FOR TCEQ INTERNAL USE ONLY	*				
Date(s)Reviewed:	D	Date Administratively Complete:			
Received From:	Correct Number of Copies:				
Received By:	D	istributi	ion Date:		
EAPP File Number:	Complex:				
Admin. Review(s) (No.):	No. AR Rounds:				
Delinquent Fees (Y/N):	R	Review Time Spent:			
Lat./Long. Verified:		SOS Customer Verification:			
Agent Authorization Complete/Notarized (Y/N):		ee	Payable to TCEQ (Y/N):		
Core Data Form Complete (Y/N):		heck:	Signed (Y/N):		
Core Data Form Incomplete Nos.:	Data Form Incomplete Nos.: Less that		Less than 90 days of	s than 90 days old (Y/N):	

# **SECTION 2**

# EDWARDS AQUIFER PROTECTION PROGRAM ROADWAY APPLICATION (TCEQ-20872)

TCEQ – 20872 EAPP Roadway Application County Road (CR) 143 at Dry Berry Creek Bridge Replacement Kimley »Horn

### Edwards Aquifer Protection Program Roadway Application

Texas Commission on Environmental Quality

This application is intended only for projects which a major roadway is designed for construction, such as State highways, County roads, and City thoroughfares.

Designed for Regulated Activities on the Contributing Zone to the Edwards Aquifer in relation to 30 TAC §213.24, Regulated Activities on the Edwards Aquifer Recharge Zone, in relation to 30 TAC §213.5(b), Effective June 1, 1999.

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

#### Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer.

The application was prepared by:

Print Name of Customer/Agent: Elizabeth W. Wagner, P.E

Date: 2/4/24

Signature of Customer/Agent:

-25m

Project Information

- 1. Regulated Entity (Project) Name: <u>County Road (CR) 143 at Dry Berry Creek Bridge</u> <u>Replacement; CSJ 0914-05-203</u>
- 2. County: Williamson
- 3. Stream Basin(s): Dry Berry Creek
- 4. Groundwater Conservation District (if applicable): N/A
- 5. Customer (Applicant):

Contact Person: Lorena Roque Martinez, P.G. Entity: <u>Texas Department of Transportation - Austin District</u> Mailing Address: <u>7901 N Interstate Hwy 35</u> City, State: <u>Austin, TX</u> Zip: <u>78753</u> Telephone: <u>737-291-8601</u> Email Address: <u>lorena.roquemartinez@txdot.gov</u> 6. Agent (Representative):

Contact Person: <u>Elizabeth W. Wagner, P.E</u> Entity: <u>Kimley-Horn</u> Mailing Address: <u>10814 Jollyville Rd, Bldg 4, Ste 200</u> City, State: <u>Austin, TX</u> Zip: <u>78759</u> Telephone: <u>737-400-9846</u> Email Address: <u>elizabeth.wagner@kimley-horn.com</u>

 Landowner of R.O.W. (Right of Way) Person or entity responsible for maintenance of water quality Best Management Practices (BMPs), if not applicant.

Contact Person: John C. Peters, P.E. Entity: <u>Texas Department of Transportation - Georgetown Area Office</u> Mailing Address: <u>2727 S. Austin Ave</u> City, State: <u>Georgetown, TX</u> Zip: <u>78626</u> Telephone: <u>512-930-6002</u> Email Address: John.Peters@TxDOT.gov

8. The TCEQ must be able to inspect the project site or the application will be returned. Sufficient survey marking is provided on the project to allow TCEQ regional staff to locate the boundaries and alignment of any regulated activities and the geologic or manmade features noted in the Geologic Assessment.

Survey marking will be completed by this date: <u>Geological features will be identified</u> <u>during the site assessment using GPS coordinates.</u>

- 9. Attachment A Road Map. A road map showing directions to and the location of the project site is attached. The map clearly shows the boundary of the project site.
- 10. Attachment B USGS Quadrangle. A copy of the official 7 ½ minute USGS Quadrangle Map (Scale: 1" = 2000') is attached. The map(s) clearly show:

 $\square$  Project site boundaries

USGS Quadrangle Name(s)

 $\square$  All drainage paths from site to surface waters

- 11. This project extends into (Check all that apply):
  - Recharge Zone (RZ)

Transition Zone (TZ)

Contributing Zone (CZ)

Contributing Zone within

Transition Zone (CZ/TZ)

Zone not regulated by EAPP

TCEQ-20872 (7/27/2020)

12. Attachment C - Project Description. A detailed narrative description of the proposed project is attached. The project description is consistent throughout the application and contains, at a minimum, the following details:

otured
hase]
Existing commercial site
Existing industrial site
Existing residential site
Other:
Quality. A detailed description of all ched.
will be used as fill material.

Concrete

Asphaltic concrete pavement

Permeable Friction Course (PFC)

Other:

17. Right of Way (R.O.W.) and Pavement Area:

R.O.W. for project: <u>4.53</u> (ac.)
Length: <u>1614</u> ft.
Width: varies from <u>97</u> ft. to <u>136</u> ft.
Impervious cover (IC): 0.93 (ac.)
Total of Pavement area 0.93 (ac.) ÷ R.O.W. area 4.53 (ac.) x 100 = 21% IC.

 $\square$  CAD program was used to determine areas.

Number of travel lanes: proposed: <u>2 travel lanes</u>, existing: <u>2 travel lanes</u>

- $\square$  Typical widths of lanes: <u>12</u> (ft.)
- $\square$  Are intersections also being improved? (Y/N) <u>N</u>

### Site Plan Requirements

Items 18 - 28 must be included on the Site Plan.

- 18. The Site Plan must have a minimum scale of 1'' = 400'. Site Plan Scale: 1'' = 50'
- 19. 100-year floodplain boundaries:

$\boxtimes$ Some part(s) of the project site is located within the 100-year floodplain. The
floodplain is shown and labeled. The 100-year floodplain boundaries are based on the
following specific (including date of material) source(s): Federal Emergency Management
Agency Federal Insurance Rate Map #48491C0285F, dated December 20, 2019.

No part of the project site is located within the 100-year floodplain.

- 20. A layout of the development with existing and finished contours at appropriate, but not greater than ten-foot contour intervals is shown. Sensitive features, lots, wells, buildings, roads, culverts, etc. are shown on the site plan.
- 21.  $\square$  A figure (map) indicating all paths of drainage from the site to surface waters.

Name all stream crossings: Dry Berry Creek

 $\square$  Drainage patterns and approximate slopes.

There will be no discharge to surface waters.

- 22. Distinguish between areas of soil disturbance and areas which will not be disturbed.
- 23. Show locations of major structural and nonstructural controls. These are the temporary and permanent best management practices. Include the following:

Show design and location of any hazardous materials traps.

 $\boxtimes$  Show design at outfalls of major control structures and conveyances.

A description of the BMPs and measures that prevent pollutants from entering surface streams.

24. Show locations of staging areas or project specific locations (PSL). Are they:

Onsite, within project R.O.W.

Offsite.

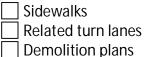
Not yet determined. (Requires future authorization)

- 25. Show locations where soil stabilization practices are expected to occur.
- 26.  $\boxtimes$  Show surface waters (including wetlands).
- 27. Temporary aboveground storage tank facilities:

Temporary aboveground storage tank facilities will be located on this site. Show on site plan.

Temporary aboveground storage tank facilities will not be located on this site.

28. Plan(s) also include:



Shared-use paths

Off-site improvements and staging areas

Utility relocations

Other improved areas:

### Permanent Best Management Practices (BMPs)

Description of practices and measures that will be used after construction is completed.

- 29. Permanent BMPs and measures have been designed, and will be constructed, operated, and maintained to ensure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity is removed. These quantities have been calculated in accordance with technical guidance accepted by the executive director.
  - The TCEQ Technical Guidance Manual (TGM) was used to design permanent BMPs and measures for this site.
  - A technical guidance other than the TCEQ TGM was used to design permanent BMPs and measures for this site. The complete citation for the technical guidance that was used:

30. Attachment E - BMPs for Upgradient (Offsite) Stormwater.

- A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is attached.
- No surface water, groundwater or stormwater originates upgradient from the site and flows across the site, and an explanation is attached.
- Permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site, and an explanation is attached.
- 31. 🛛 Attachment F BMPs for On-site Stormwater.
  - A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is attached.
  - Permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, and an explanation is attached.
- 32. Attachment G Construction Plans. Construction plans and design calculations for the proposed permanent BMPs and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer, and are signed, sealed, and dated. Construction plans for the proposed permanent BMPs and measures are attached and include all proposed structural plans and specifications, and appropriate details.
  - Major bridge cross-sections, and roadway plan and profiles
  - ☑ BMP plans and details
     ☑ Erosion control
     ☑ SW3P
     ☑ EPIC, as necessary

33. Attachment H - Inspection, Maintenance, Repair and Retrofit Plan. A site and BMP specific plan for the inspection, maintenance, repair, and, if necessary, retrofit of the permanent BMPs and measures is attached. The plan fulfills all the following:

Prepared and certified by the engineer designing the permanent BMPs and measures.
 Signed by the owner or responsible party.

Outlines specific procedures for documenting inspections, maintenance, repairs, and, if necessary, retrofit.

Contains a discussion of recordkeeping procedures.

34. Attachment I - Pilot-Scale Field Testing Plan. Pilot studies for BMPs that are not recognized by the Executive Director require prior approval from the TCEQ. A plan for pilot-scale field testing is attached.

🖂 N/A

35. Attachment J - Measures for Minimizing Surface Stream Contamination. A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows, and in-stream effects caused by the regulated activity which increase erosion or may result in water quality degradation.

Include permanent spill measures used to contain hydrocarbons or hazardous substances by way of traps, or response contingencies.

36. The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity.

If the applicant intends to transfer responsibility, check the box below.

Yes

A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days.

### Stormwater to be generated by the Proposed Project

Description of practices and measures that will be used during construction.

37. The site description, controls, maintenance, and inspection requirements for the Storm Water Pollution Prevention Plan (SWPPP or SW3P) developed under the Texas Pollutant Discharge Elimination System (TPDES) general permits for stormwater discharges have been submitted to fulfill paragraphs 30 TAC §213.24(1-5) & §213.5(b) of the technical report.

The Temporary Stormwater Section (TCEQ-0602) is included with the application. The SWPPP (SW3P) will serve as the Temporary Stormwater Section (TCEQ-0602).

38. Attachment K - Volume and Character of Stormwater. A detailed description of the volume (quantity) and character (quality) of the stormwater runoff expected to occur from the proposed project is attached. The estimates of stormwater runoff quality and quantity are based on area and type of impervious cover.

 $\square$  Include the pre-construction runoff coefficient.  $\square$  Include the post-construction runoff coefficient.

#### Administrative Information

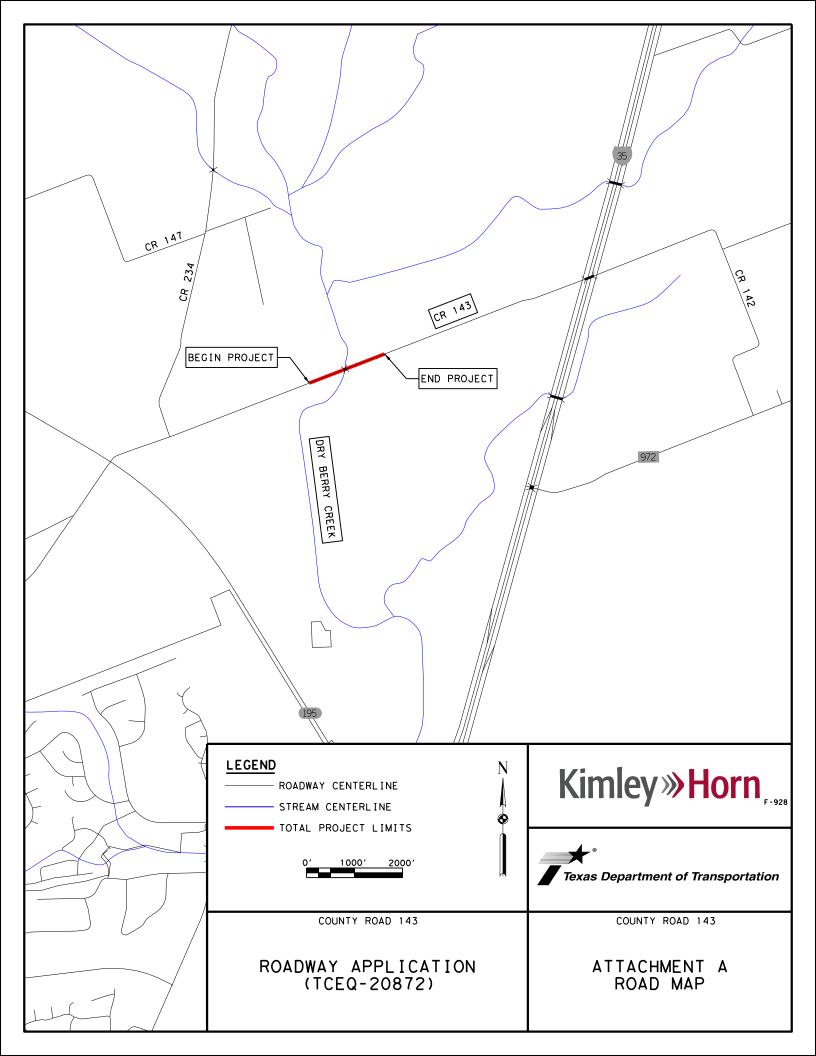
- 39. Submit one (1) original and one (1) copy of the application, plus one electronic copy as needed, for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ is required to distribute the additional copies to these jurisdictions.
- 40. The fee for the plan(s) is based on:
  - The total R.O.W. (as in Item 17).
  - TxDOT roadway project.

# ATTACHMENT A

### ROAD MAP

TCEQ – 20872 EAPP Roadway Application County Road (CR) 143 at Dry Berry Creek Bridge Replacement



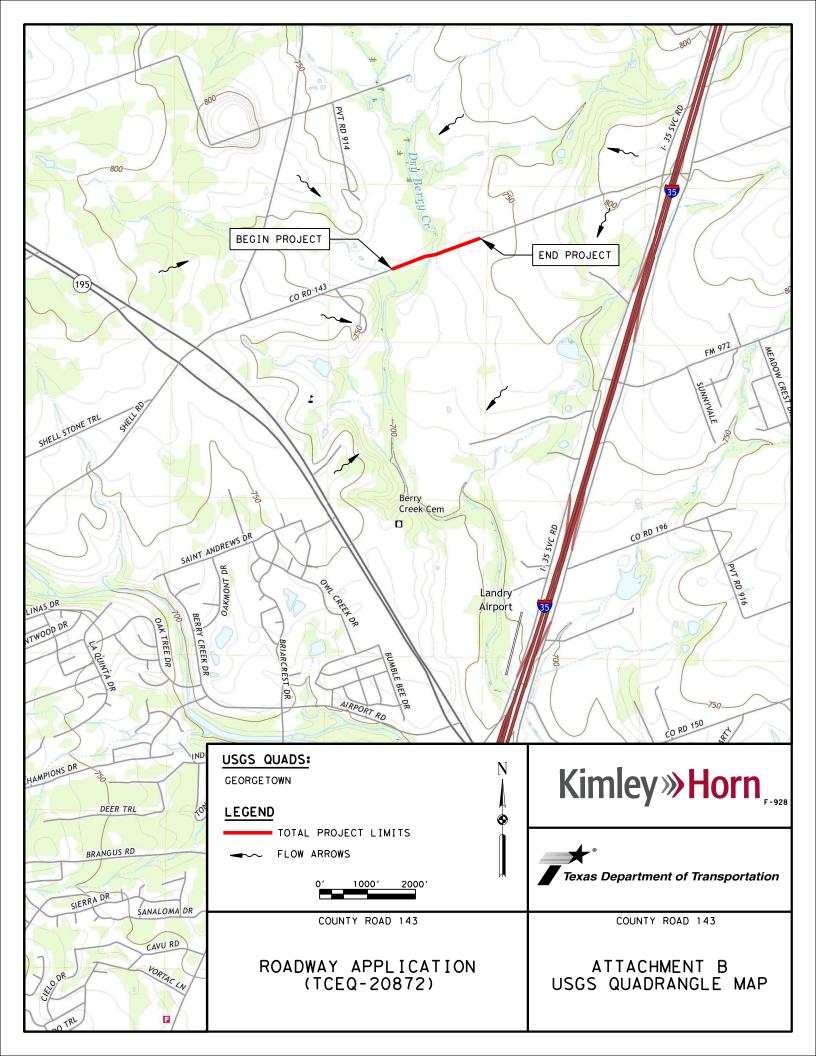


## ATTACHMENT B

### USGS QUADRANGLE MAP

TCEQ – 20872 EAPP Roadway Application County Road (CR) 143 at Dry Berry Creek Bridge Replacement





# ATTACHMENT C

### **PROJECT DESCRIPTION**

The Texas Department of Transportation (TxDOT) proposes improvements to County Road 143 (CR 143) in Williamson County approximately 7 miles northeast of Georgetown, Texas. The project begins 3,200 feet east of County Road 234 and ends 4,814 feet west of IH 35 Service Road (STA 100+56.00 to STA 116+70.00).

CR 143 is classified as local road, with the ultimate condition classified as a minor arterial. The existing roadway has a typical section of two 11-foot lanes with no shoulders. The proposed improvements include widening for two 12-foot lanes and 2-foot shoulders from station 100+56.00 to station 107+38.00 and from station 109+38.00 to station 116+70.00, and widening for two 12-foot lanes and 5-foot shoulders from station 107+38.00 to station 109+38.00, roadway signage and markings, and a bridge replacement. The project will increase the impervious cover by 0.40 acres, or 8%. The overall site area is approximately 4.53 acres based on the right-of-way to right-of-way limits of the project.

The project is located within the Dry Berry Creek watershed and includes the CR 143 Dry Berry Creek crossing. The project resides within the Edwards Aquifer Recharge Zone, regulated by the Texas Commission on Environmental Quality (TCEQ). Water quality Best Management Practices (BMPs) will be implemented to comply with TCEQ Edwards Aquifer Rules. The permanent BMPs designed to handle the impervious cover will be Vegetative Filter Strips (VFS). The total provided TSS removal per year is 358 lbs, which exceeds the required removal of 348 lbs.



# ATTACHMENT D

### FACTORS AFFECTING SURFACE WATER QUALITY

The following factors may affect water quality during both the construction and operation phases of the project:

- 1. The increase in impervious cover could result in an increase in runoff, potentially altering the quality and/or quantity of recharge to the aquifer.
- 2. Increase sediment loading in runoff due to erosion of sediment and pollutants from exposed soil due to preparation activities such as grading, excavating, and clearing vegetation. In addition to this disturbance of native soil, new soil will be brought onto the site for fill in the roadbed and other components of the project. Increased sediment loading may occur prior to full establishment of vegetation. Temporary erosion and sedimentation controls will be in place to minimize this effect.
- 3. Runoff from construction product staging, storage, and waste.
- 4. Runoff from hazardous material spills. These may contain metals, nutrients, bacteria, herbicides, hydrocarbons, and other toxic constituents. Pesticides, de-icing salts, paint, and fertilizers introduce pollutants into the runoff. Standard vehicle use may also add to pollutants through normal operations including braking and fuel combustion as well as through oil and fuel leaks. Runoff from automotive fluids, lubricants, and fuel leaks from standard vehicle use. Additionally, vehicles can transfer pollutants from source to source across the project site via vehicle tracking.



# ATTACHMENT E

### BMPS FOR UPGRADIENT STORMWATER

The surrounding topography generally drains from north to south. The mixing of offsite and untreated runoff is negligible. CR 143 is an elevated crowned roadway which prevents mixing of offsite and untreated onsite runoff. The BMPs are located in areas with little to no offsite water collected in the roadside ditches. Offsite runoff is contained primarily along the southbound lanes and conveyed through ditches which discharge to receiving waters. For locations with VFS, onsite runoff will sheet flow from the crown of the roadway across the VFS to the proposed ditches. For project drainage patterns see the Drainage Area map sheet in *Attachment G – Construction Plans*.



# ATTACHMENT F

### **BMPS FOR ON-SITE STORMWATER**

On-site stormwater will be treated as dictated by the TCEQ, complying with the Edwards Aquifer Rules Technical Guidance document on Best Management Practices (RG-348, dated July 2005). Per 30 Texas Administrative Code (TAC) Chapter 13, 80% of the Total Dissolved Solids (TSS) generated by the increase in impervious cover due to the project must be removed.

Following the calculation methodology in the TCEQ Technical Guidance RG-348, the minimum amount of TSS that must be removed due to the proposed roadway widening of CR 143 is 348 lbs. of TSS per year. Existing and proposed impervious cover were delineated for the full extent of the project within the existing Right-Of-Way (ROW). Existing impervious cover was delineated using the topographical survey edge of pavement for mainlanes and driveways. Proposed impervious cover was delineated based on the CR 143 roadway design and increase in impervious cover was calculated within project limits.

VFS were selected to meet removal requirements for the project due to their removal efficiency and low construction cost. VFS were designed with a 15 foot minimum width and a 5:1 max side slope in accordance with TCEQ design criteria. Contributing impervious cover for proposed VFS does not exceed the 72 foot maximum allowable.

VFS were placed on the southbound side of CR 143 adjacent to the proposed edge of pavement. Proposed VFS will treat 358 lbs of TSS per year, which exceeds the required removal of 348 lbs. Proposed RSS load removal was calculated following Equation 3.7 of TCEQ Technical Guidance RG-348.

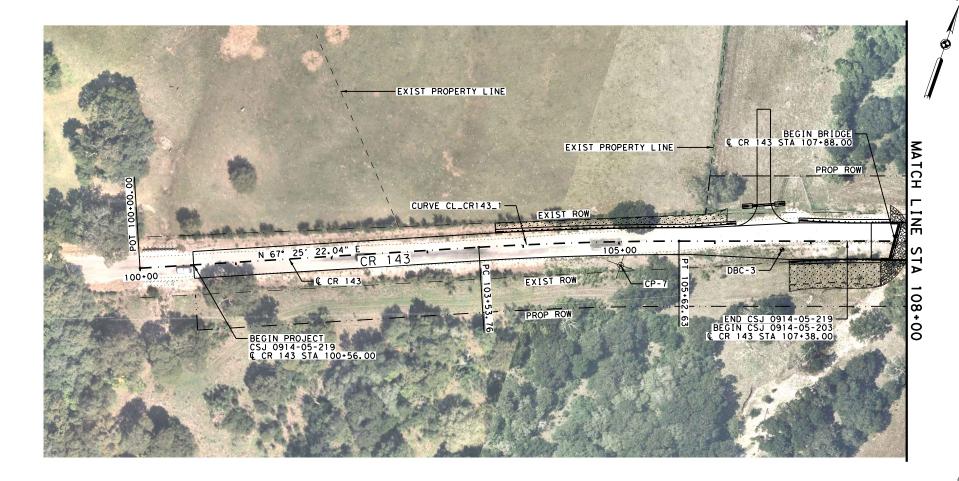
**Kimley»Horn** 

## ATTACHMENT G

### CONSTRUCTION PLANS

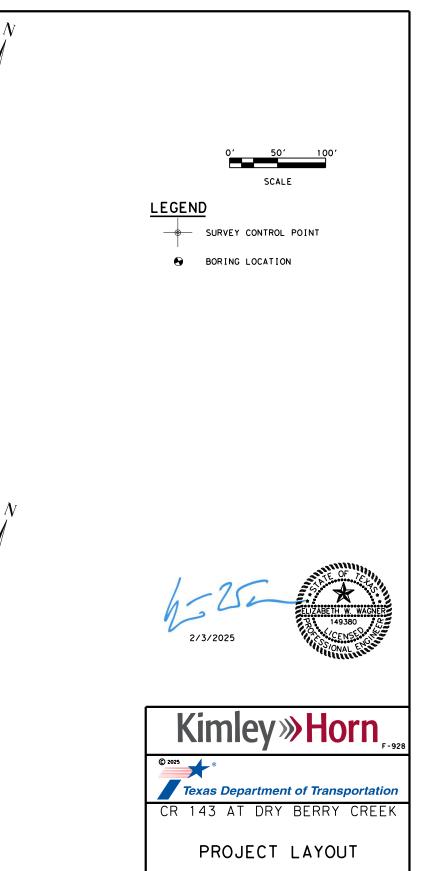
Please reference attached construction plans.



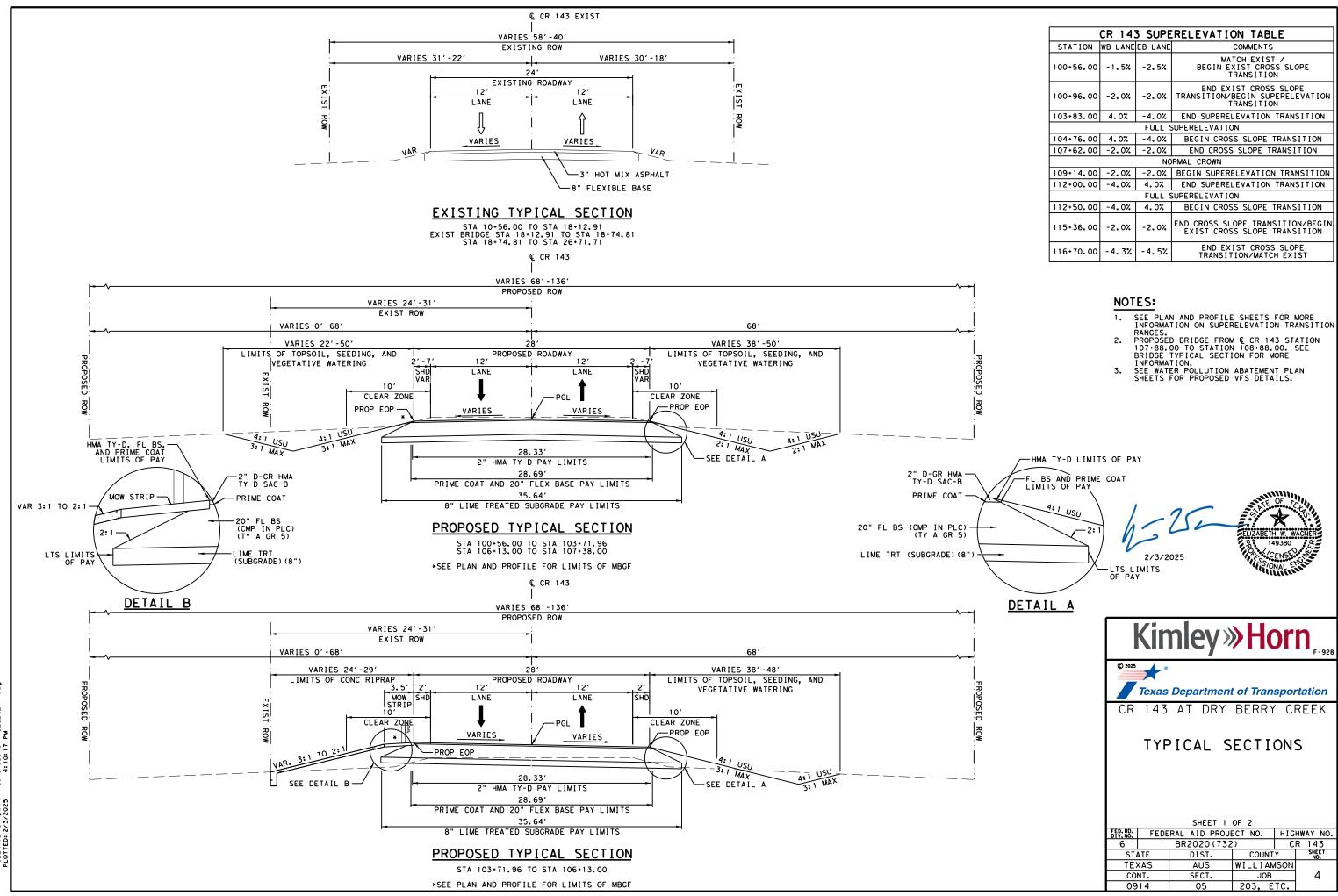




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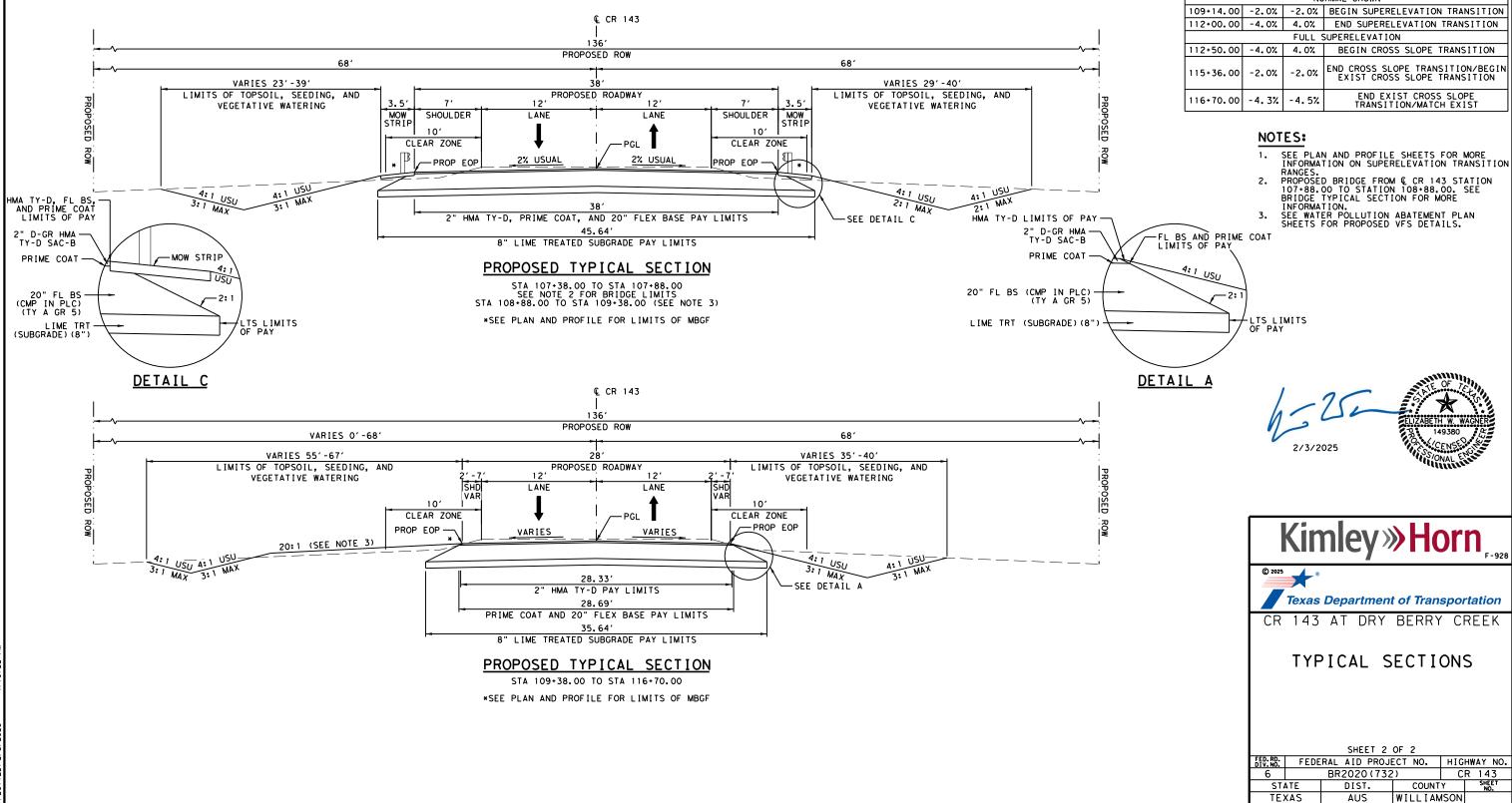


	SHEET 1 OF 1						
FED.RD. DIV.NO.	RD: FEDERAL AID PROJECT NO. HIGHWAY NO.						
6	BR2020(732) CR 143						
ST	ATE .	DIST.	COUNTY		SHEET NO.		
TEX	(AS	AUS	WILLIAMSON				
CONT.		SECT.	JOB		3		
0914		05	203, E	TC.			



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	CR 143 SUPERELEVATION TABLE				
STATION	WB LANE	EB LANE	COMMENTS		
100+56.00	-1.5%	-2.5%	MATCH EXIST / BEGIN EXIST CROSS SLOPE TRANSITION		
100+96.00	-2.0%	-2.0%	END EXIST CROSS SLOPE TRANSITION/BEGIN SUPERELEVATION TRANSITION		
103+83.00	4.0%	-4.0%	END SUPERELEVATION TRANSITION		
		FULL S	SUPERELEVATION		
104+76.00	4.0%	-4.0%	BEGIN CROSS SLOPE TRANSITION		
107+62.00	-2.0%	-2.0%	END CROSS SLOPE TRANSITION		
		NO	DRMAL CROWN		
109+14.00	-2.0%	-2.0%	BEGIN SUPERELEVATION TRANSITION		
112+00.00	-4.0%	4.0%	END SUPERELEVATION TRANSITION		
		FULL	SUPERELEVATION		
112+50.00	-4.0%	4.0%	BEGIN CROSS SLOPE TRANSITION		
115+36.00	-2.0%	-2.0%	END CROSS SLOPE TRANSITION/BEGIN EXIST CROSS SLOPE TRANSITION		
116+70.00	-4.3%	-4.5%	END EXIST CROSS SLOPE TRANSITION/MATCH EXIST		



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	CR 143 SUPERELEVATION TABLE				
STATION	STATION WB LANE EB LANE COMMENTS				
100+56.00	00 -1.5% -2.5% MATCH EXIST / BEGIN EXIST CROSS SLOPE TRANSITION				
100+96.00	-2.0%	-2.0%	END EXIST CROSS SLOPE TRANSITION/BEGIN SUPERELEVATION TRANSITION		
103+83.00	4.0%	-4.0%	END SUPERELEVATION TRANSITION		
		FULL S	SUPERELEVATION		
104+76.00	4.0%	-4.0%	BEGIN CROSS SLOPE TRANSITION		
107+62.00	-2.0%	-2.0%	END CROSS SLOPE TRANSITION		
		NO	RMAL CROWN		
109+14.00	-2.0%	-2.0%	BEGIN SUPERELEVATION TRANSITION		
112+00.00	-4.0%	4.0%	END SUPERELEVATION TRANSITION		
		FULL S	SUPERELEVATION		
112+50.00	-4.0%	4.0%	BEGIN CROSS SLOPE TRANSITION		
115+36.00	-2.0%	-2.0%	END CROSS SLOPE TRANSITION/BEGIN EXIST CROSS SLOPE TRANSITION		
116+70.00	-4.3%	-4.5%	END EXIST CROSS SLOPE TRANSITION/MATCH EXIST		

CONT.

0914

SECT.

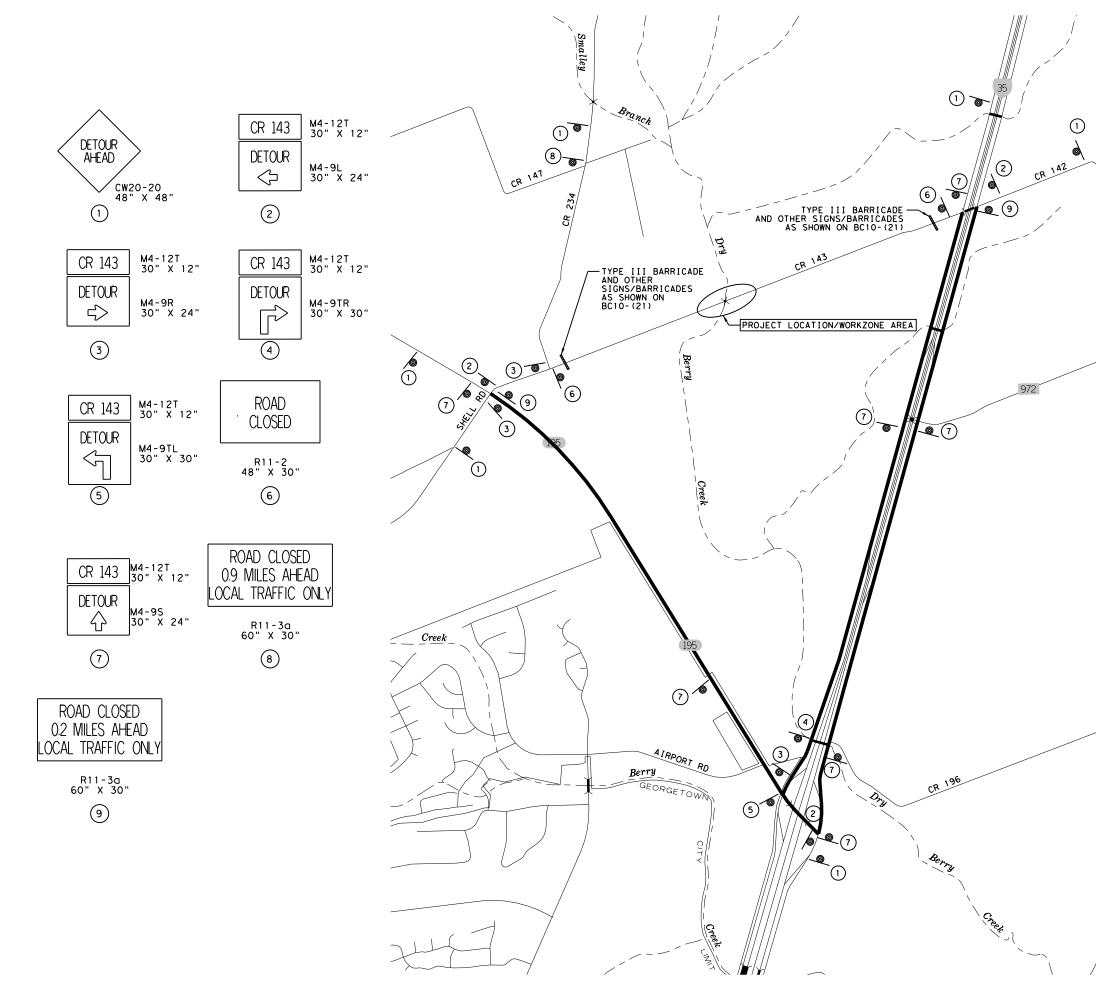
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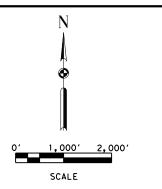
JOB

203. ETC.



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#### GENERAL NOTES

THE CONTRACTOR SHALL NOTIFY THE PROPER CITY, COUNTY, EMERGENCY MEDICAL SERVICES, FIRE DEPARTMENT, POLICE DEPARTMENT, TEXAS DEPARTMENT OF PUBLIC SAFETY, AND THE ENGINEER WHEN MAJOR TRAFFIC CHANGES ARE TO BE PERFORMED. THE NOTIFICATION MUST BE PROVIDED AT LEAST FOURTEEN (14) DAYS PRIOR TO THE CHANGE.

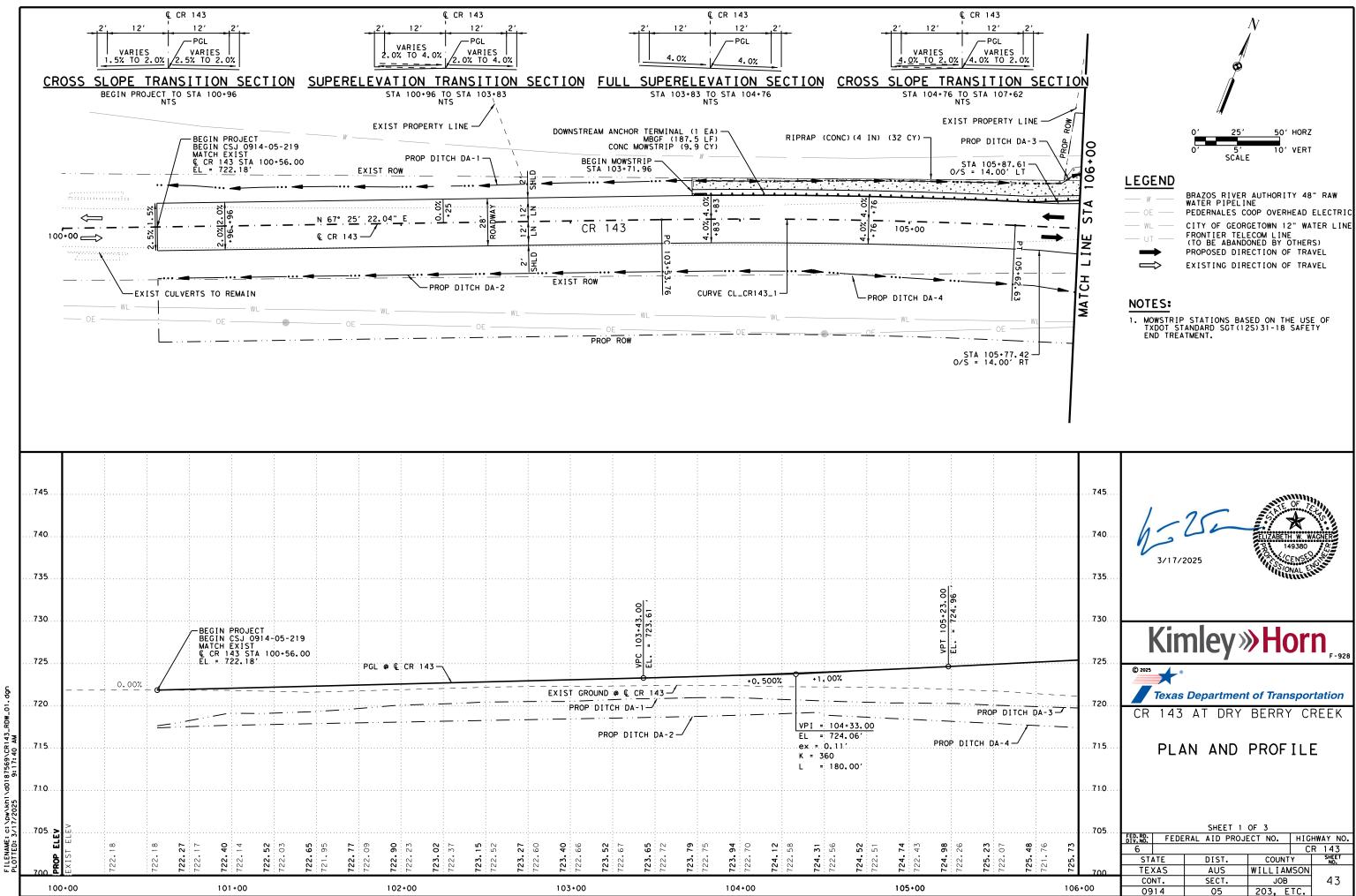
#### TCP NARRATIVE

- 1.
- 2.
- 3.
- PLACE WORK ZONE APPROACH SIGNAGE IN ACCORDANCE WITH BC STANDARD SHEETS. PLACE DETOUR SIGNAGE AND BARRICADES AS NOTED ON THE DETOUR LAYOUT. PLACE EROSION CONTROL MEASURES AS NOTED ON SW3P SHEETS. REMOVE EXISTING COUNTY ROAD 143 BRIDGE AND ROADWAY AS SHOWN IN THE PLANE 4.
- PLANS. 5.
- PLANS. CONSTRUCT PROPOSED COUNTY ROAD 143 BRIDGE AND ROADWAY AS SHOWN IN THE PLANS. REOPEN ROAD UPON COMPLETION OF CONSTRUCTION. 6.

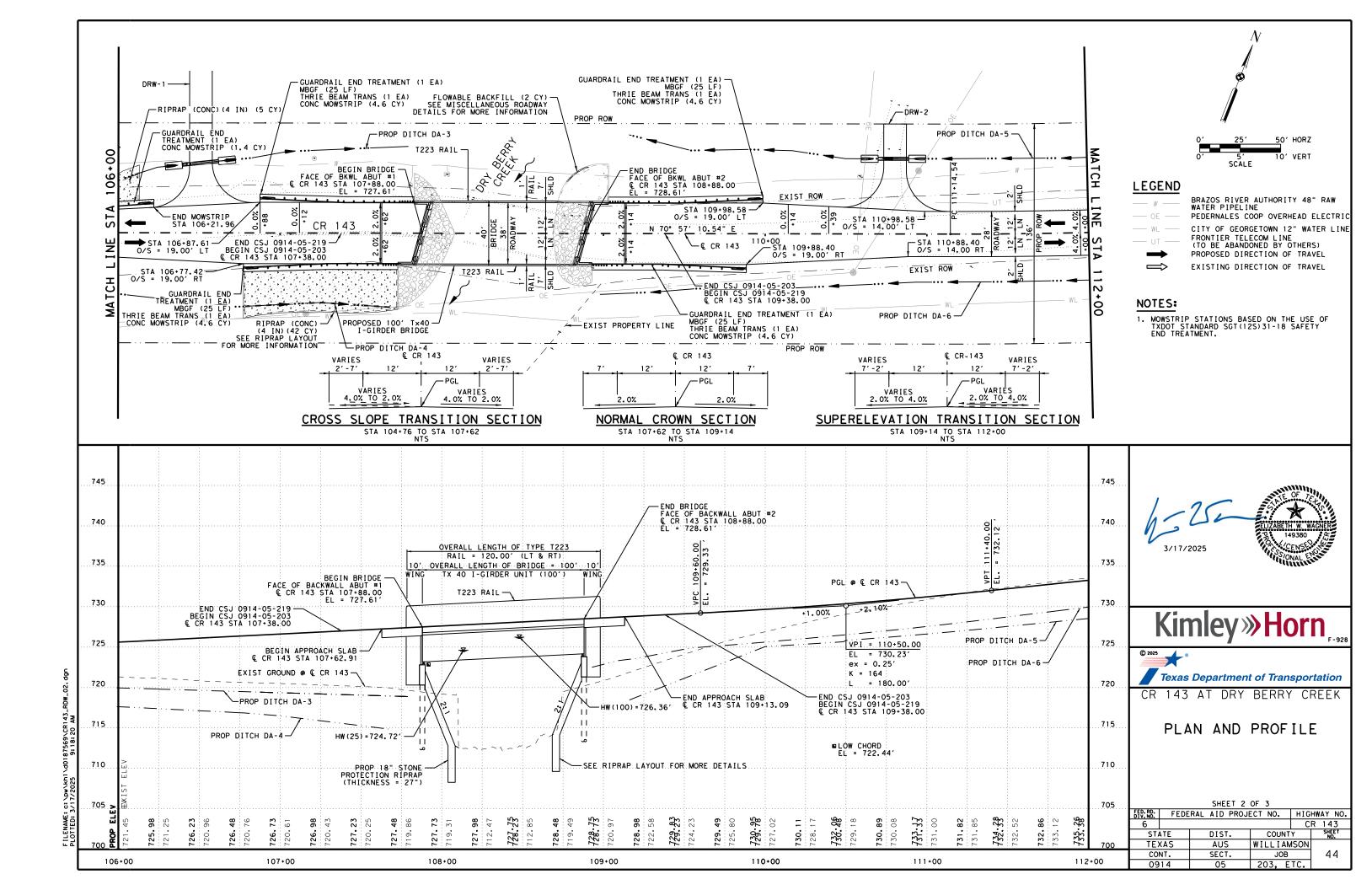
SIGNS MAY BE ADJUSTED TO FIT EXISTING DRIVEWAYS WITH PERMISSION OF ENGINEER. ACCESS TO ADJOINING DRIVEWAYS MUST BE MAINTAINED AT ALL TIMES.

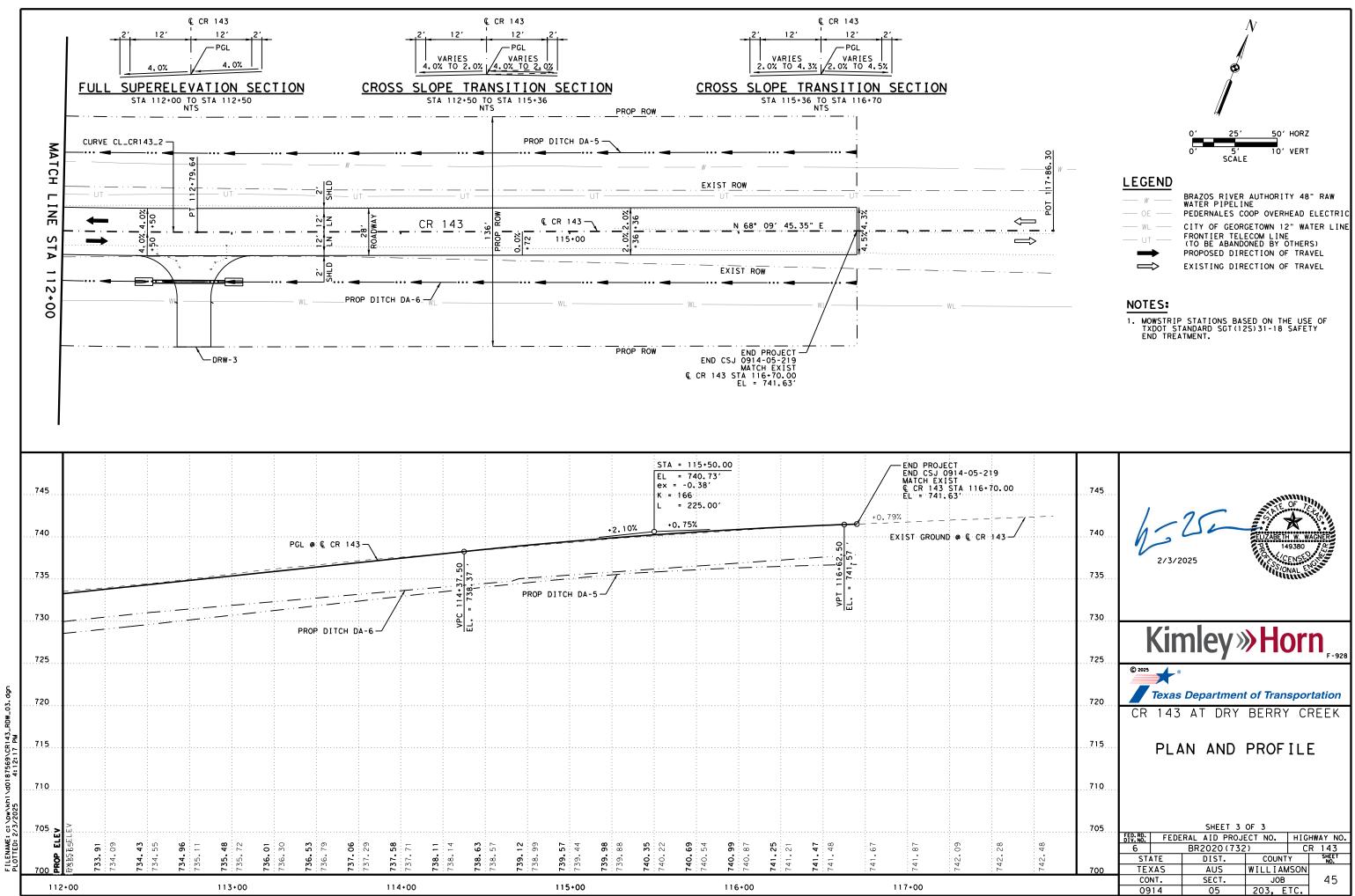
149380 2/3/2025 CENS



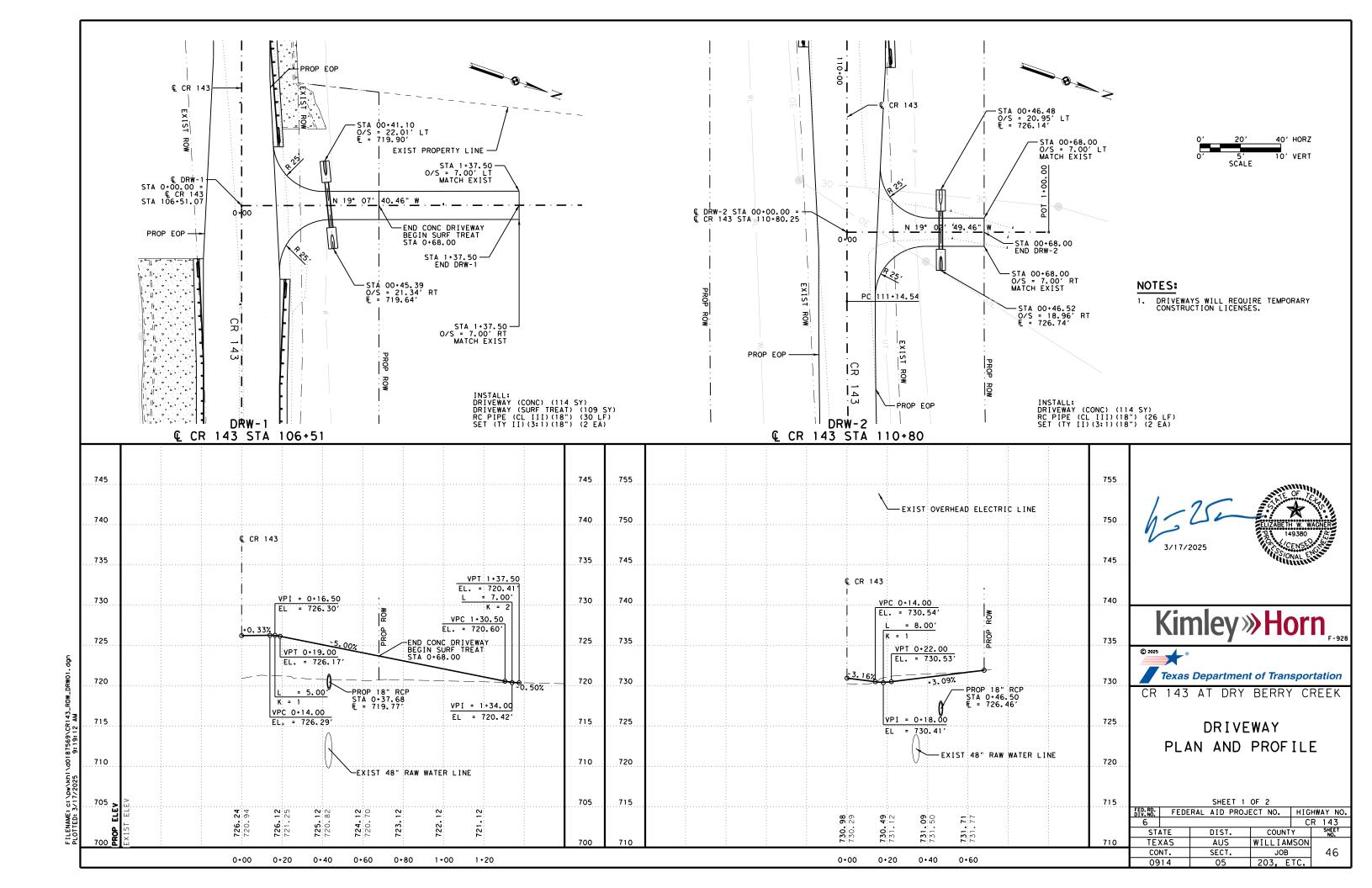


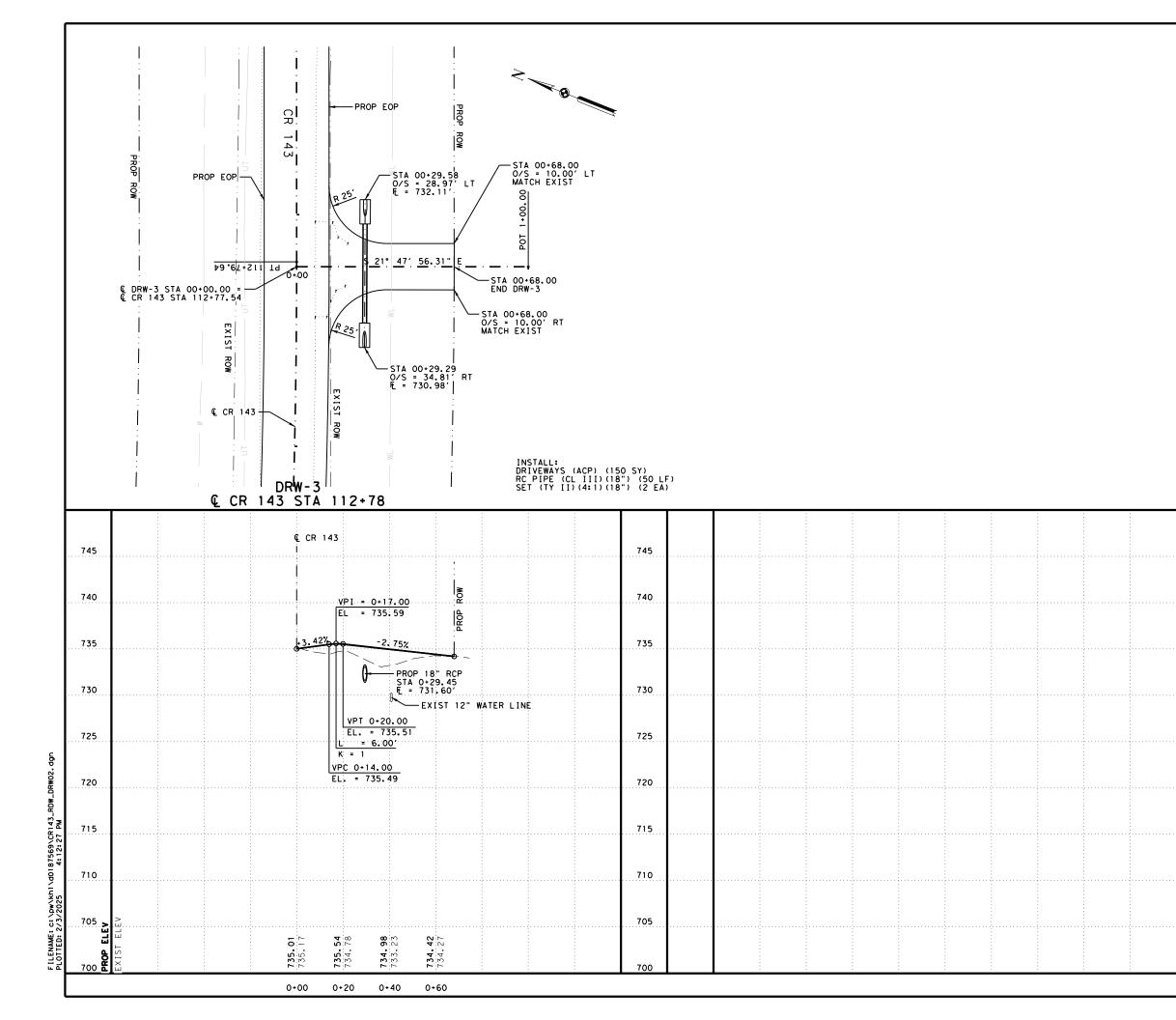
	3/17/2	025	ELIZABET	OF 72 W WA 19380 ENSE NAL EN	SI AND
	Kin © 2025	nley»	»Ho	ori	<b>1</b> F-928
OP DITCH DA-3 - 720		<b>Departmen</b> AT DRY	t of Tran BERRY	· ·	tation REEK
DA-4715	PLA	N AND	PROF	ILE	
705	FED. RD. FEDE	SHEET 1 RAL AID PROJ		нісн	WAY NO.
722.07 725.48 721.76 725.73	6			CR	143
722.07 725.48 721.76 725.73	STATE	DIST.	COUNT		SHEET NO.
	TEXAS CONT.	AUS SECT.	WILLIAN JOB	150N	43
106+00	0914	05	203, E	TC.	

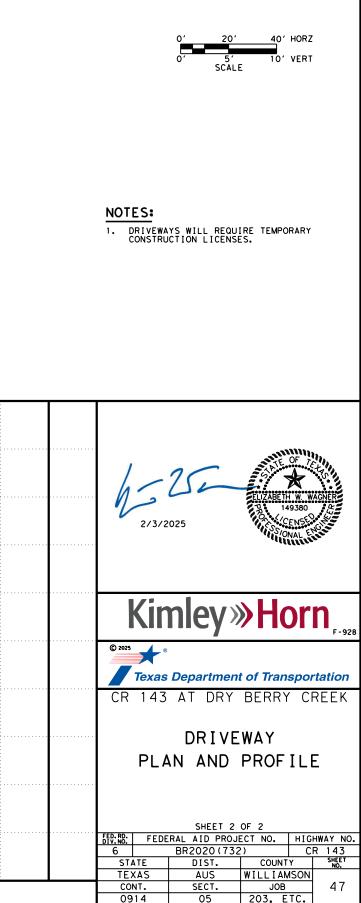


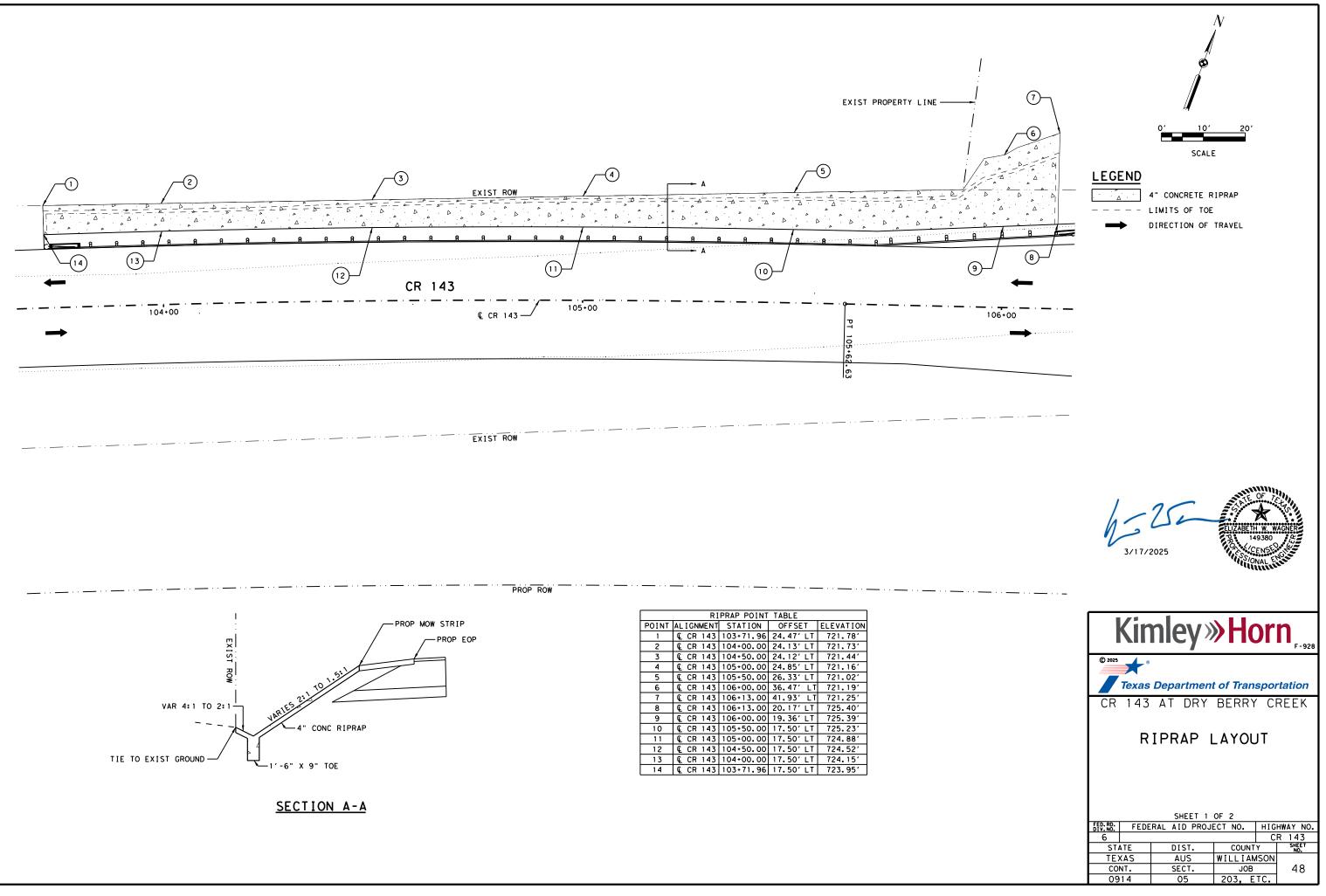


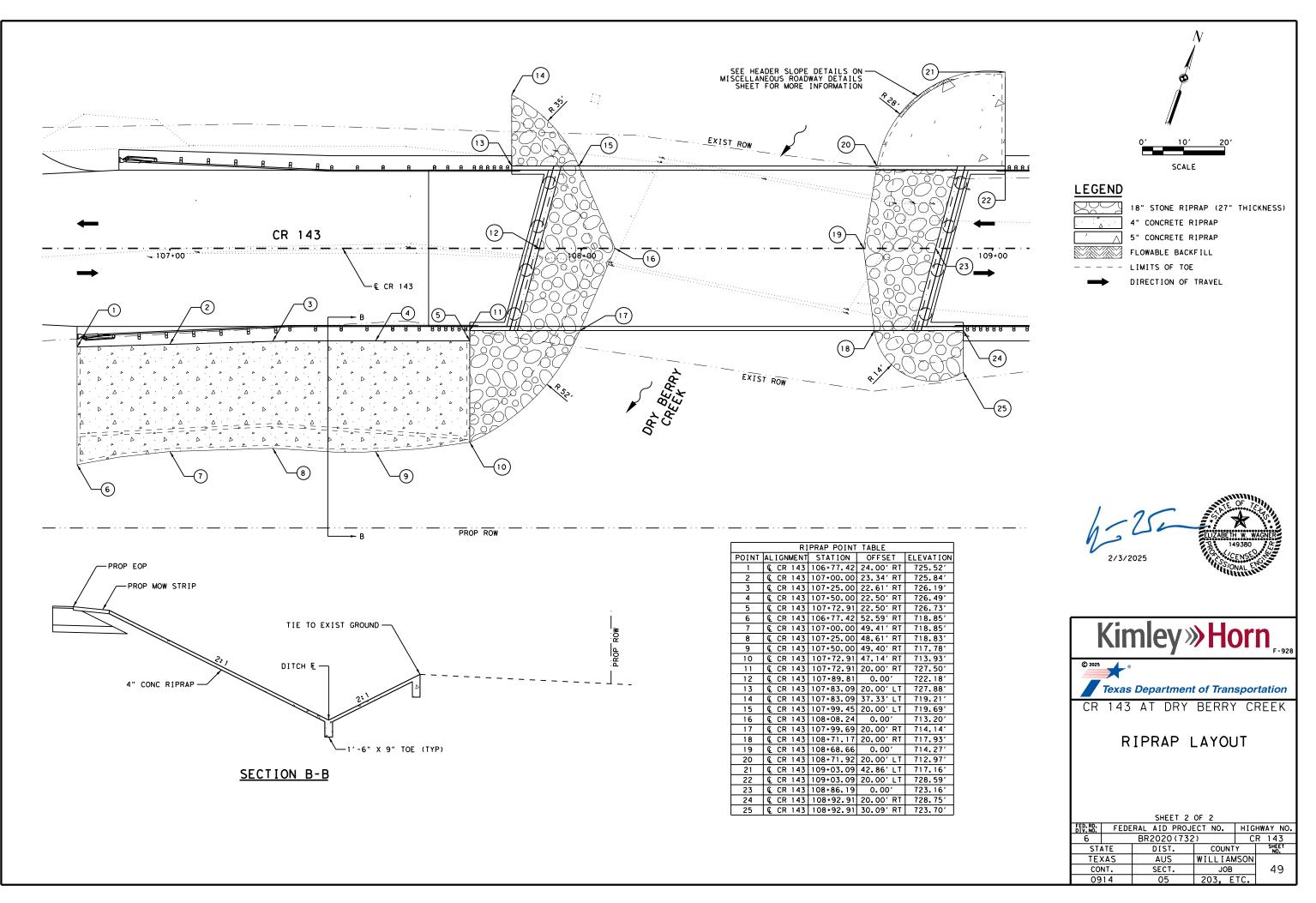
$\begin{array}{c} 19 \\ 70.00 \\ 7 \\ - \end{array}$	1-25- ELIZABETH W. WASHER
143740	ELIZABETH W. WACNER 149380 2/3/2025
	"WWWILLENE
730	
725	Kimley »Horn
720	CR 143 AT DRY BERRY CREEK
715	PLAN AND PROFILE
710	
	SHEET 3 OF 3 <u> <u> <u> </u> <u> </u></u></u>
742.	STATE DIST. COUNTY SHEET NO.
~ ~ 700	TEXAS AUS WILLIAMSON
	CONT. SECT. JOB 45
	0914 05 203, ETC.

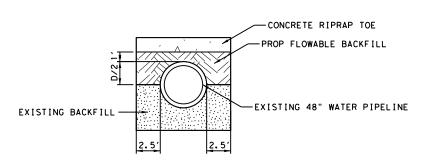




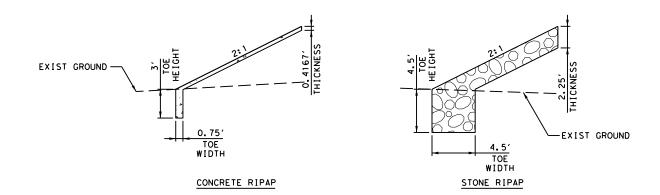








#### PIPELINE PROTECTION DETAIL

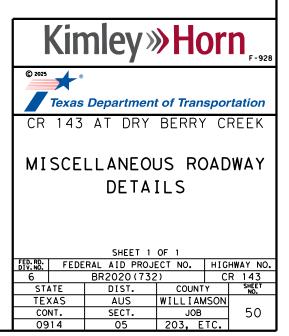


HEADER SLOPE DETAILS

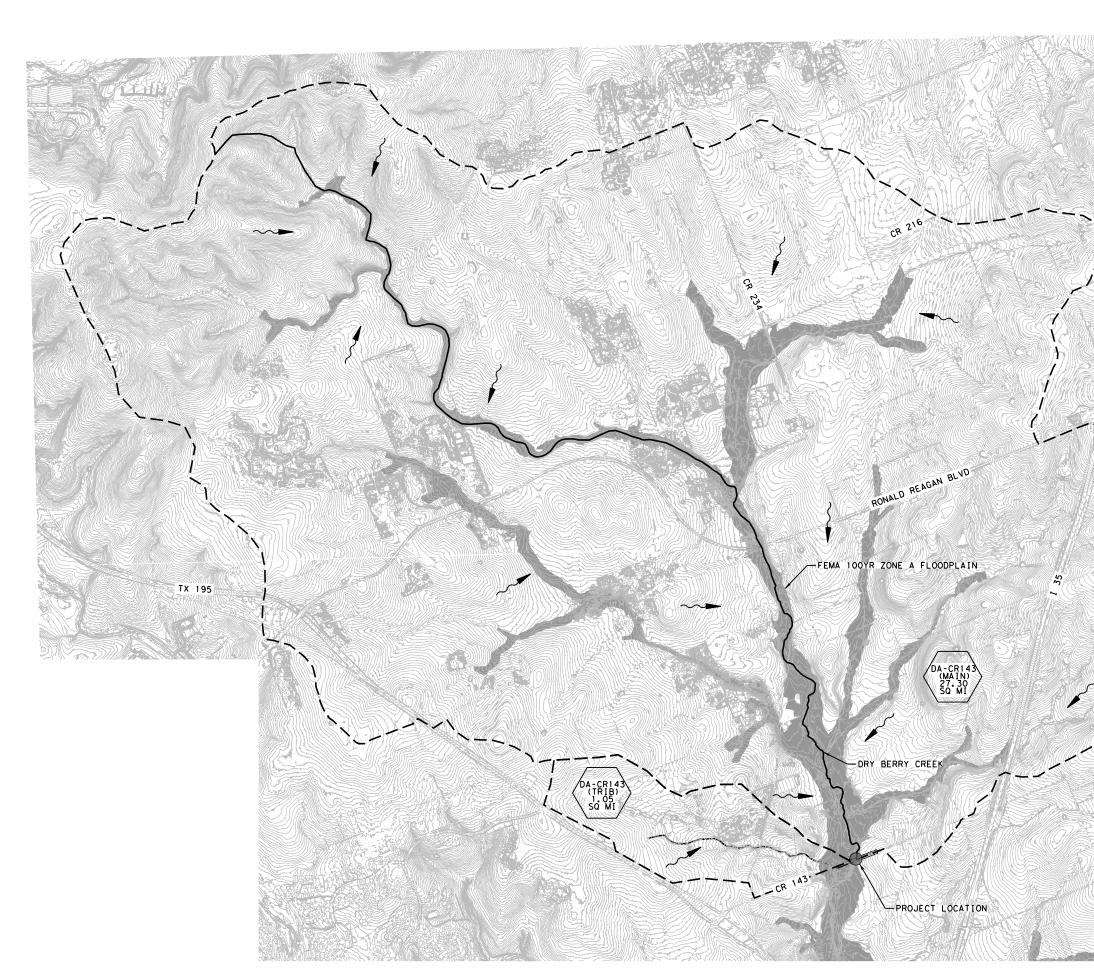
#### NOTES:

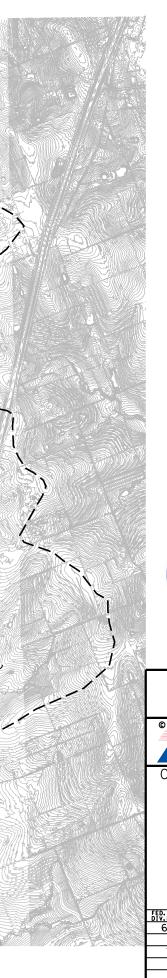
 CONTRACTOR TO EXTEND BRAZOS RIVER AUTHORITY BLOW OFF VALVE STEM AS NECESSARY, PAID FOR BY ITEM 0479 7007, ADJUSTING MANHOLES (WATER VALVE BOX). CONTACT BRA REPRESENTATIVE COLTON ADKINS AT (254)307-9836 PRIOR TO CONSTRUCTION.
 NO HEAVY EQUIPMENT ON TOP OF THE BRAZOS RIVER AUTHORITY RAW WATER LINE. CROSS AT 90 DEGREES USING MATTED CROSSINGS.











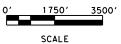
#### LEGEND

	DRAINAGE AREA BOUNDARY			
	LONGEST FLOW PATH			
	EXISTING CONTOURS			
	FEMA ZONE A			
$\sim$	FLOW ARROWS			

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#### NOTES:

- REFER TO THE HYDROLOGIC DATA SHEET FOR DETAILED CALCULATIONS.
- 2. DRY BERRY CREEK IS LOCATED IN A FEMA ZONE A FLOODPLAIN PER FIRM PANEL 48491C0285F DATED DECEMBER 20, 2019.
- 3. DRAINAGE AREA DELINEATED BASED ON TNRIS 2016 LIDAR 1 FT CONTOURS.



Daire 2/3/2025





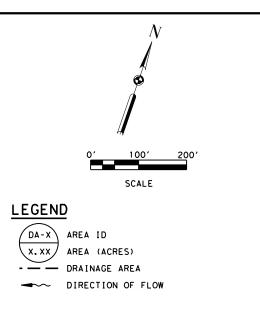


### EXTERNAL DRAINAGE AREA MAP

SHEET 1 OF 1						
FED.RD. DIV.NO.	FEDERAL AID PROJECT NO.			HIG	HWAY NO.	
6		BR2020(732)			CR 143	
STATE		DIST.	COUNTY		SHEET NO.	
TE	XAS	AUS	WILLIA	MSON		
CONT.		SECT.	JOB		63	
09	14	05	203,	ETC.		



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# NOTES:

1. PEAK DISCHARGE FOR DITCHES DETERMINED BY RATIONAL METHOD. ALL DITCHES DESIGNED TO THE 5 YEAR STORM USING ATLAS-14.

3/17/2025







# INTERNAL DRAINAGE AREA MAP

		SHEET 1	OF 1		
FED.RD. DIV.NO.	FEDE	RAL AID PROJ	ECT NO.	HIG	HWAY NO.
6				C	
ST	ATE .	DIST.	COUN	۲۱	SHEET NO.
TEX	(AS	AUS	WILLIA	VISON	
CO	NT.	SECT.	JOB		64
09	14	05	203, E	ETC.	

off	(CFS)
40	
02	
46	
76	
79	
07	

		HYD	ROLOGIC	MODEL	ING	INPUT	PARAME	TERS				
	BASI	N MODEL	PARAMETE	RS								
Name												
	Area	Area	Weighted	Тс	Tiag	2 yr	5 yr	10 yr	25 yr	50 yr	100 yr	500 yr
	(sq mi)	(ac)	Curve Number	(min)	(min)	(in)	(in)	(în)	(in)	(in)	(in)	(in)
DA-CR 143 (Main)	27.30	17475.00	66	249	149	3.93	5.14	6.28	8,01	9.49	11.20	16.00
DA-CR 143 (Trib)	1.05	671.00	68	76	46	3.93	5.14	6.28	8.01	9.49	11.20	16.00

Name			FLOW	COMPU	TATION	(Q)	
	2 yr	5 yr	10 yr	25 yr	50 yr	100 yr	500 yr
	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)
DA-CR 143 (Main)	3346	6098	9018	13781.6	18057	23073	37721
DA-CR 143 (Trib)	327	571	811	1181	1494	1844	2801
J - CR 143	3419	6220	9194	14045	18432	23587	38565

	SCS Curve Number	
Surface Method:	SCS Unit Hydrograph	
Precipitation:	SCS Storm - Atlas 14 Depths	

CR 143 (Main) Time of	f Concentration
Calculation Method	Kerby Kirpich Method
Kerby Varia	bles
Overland Flow Roughness	0.4
Slope (ft/ft)	0.052
Length (ft)	1200
Kerpich Flow V	ariables
Slope (ft/ft)	0.006
Length (ft)	44315
Time of Concentration (min)	249
T Lag (min)	149
CR 143 (Trib) Time of	f Concentration
Calculation Method	Kerby Kirpich Method
Kerby Varia	bles
Overland Flow Roughness	0.4
Slope (ft/ft)	0.015
Length (ft)	1200
Kerpich Flow V	ariables
Slope (ft/ft)	0,012
Length (ft)	6140
Time of Concentration (min)	76
T Lag (min)	46

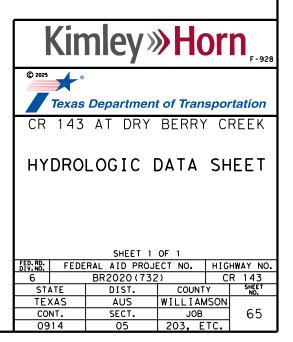
# NOTES:

- 1. HEC-HMS VERSION 4.5 WAS USED FOR HYDROLOGIC ANALYSIS.
- RAINFALL DEPTHS OBTAINED FROM NOAA ATLAS 14 TO MODEL FREQUENCY STORM.
- 25-YEAR DEISGN STORM WAS DETERMINED BY WILLIAMSON COUNTY BASED ON THE LONG RANGE TRANSPORTATION PLAN.

Daire N

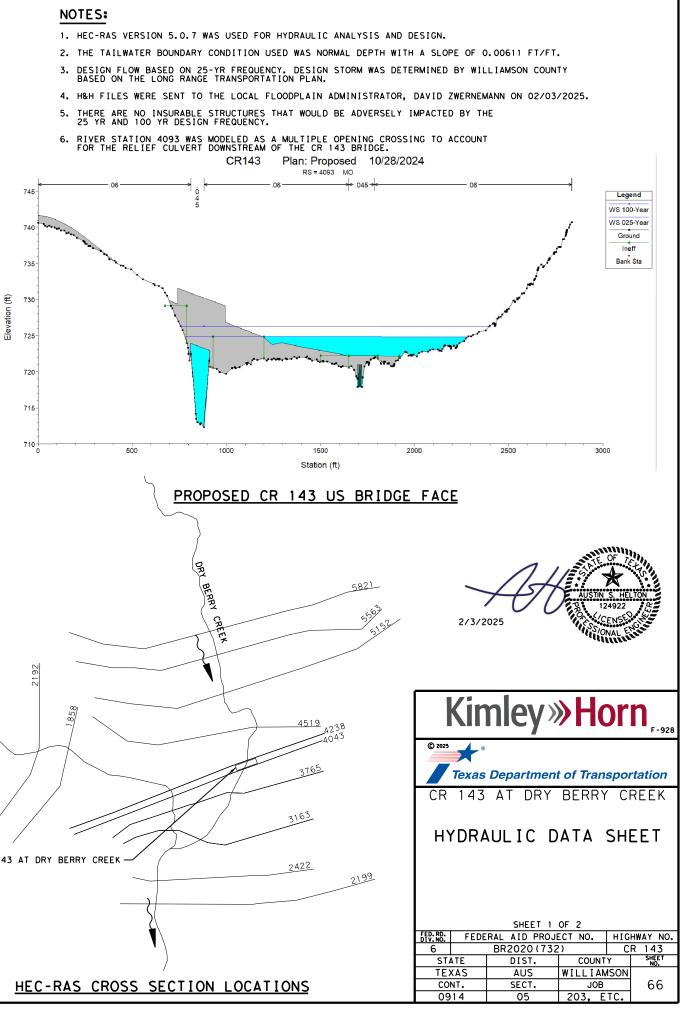
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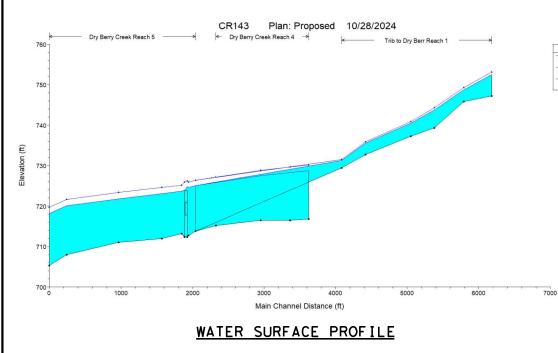


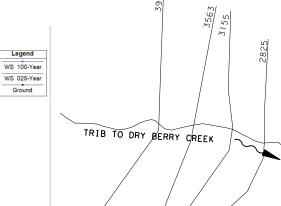


# HEC-RAS HYDRAULIC CALCULATIONS

Reach	River	Profile	Plan	Q Total	Min Ch El	W.S.Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chni	Flow Area	Top Width	Froude #
				(cfs)	(ft)	(f†)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(f†)	
Reach 4	5821	025-Year	Existing	13781.6	716.81	728.76		729.04	0.00236	7.57	4738.36	1590.27	0.41
Reach 4	5821	025-Year	Proposed	13781.6		728,76		729.04	0.00235	7.56	4744.96	1591.04	0,41
Reach 4	5821	100-Year	Existing	23073.0		730,11		730.39	0.00233	8,15	7119,17	1845.11	0.41
Reach 4	5821	100-Year	Proposed	23073.0		730.13		730.41	0.00231	8.12	7143,40	1847.88	0.41
Reach 4	5563	025-Year	Existing	13781.6	716.47	728,25		728.46	0.00194	6.61	5249.46	1672.33	0.36
Reach 4	5563	025-Year	Proposed	13781.6	716.47	728.26		728.47	0.00193	6.59	5260.99	1673.49	0.36
Reach 4	5563	100-Year	Existing	23073.0	716.47	729.60		729.83	0.00195	7.18	7633.60	1859.04	0.37
Reach 4	5563	100-Year	Proposed	23073.0	716.47	729.62		729.84	0.00192	7.15	7669.26	1862.14	0.37
Reach 4	5152	025-Year	Existing	13781.6	716.49	727.49		727.67	0.00189	6.15	5115.53	1488.21	0.36
Reach 4	5152	025-Year	Proposed	13781.6	716.49	727.50		727.68	0.00187	6.12	5137.07	1489.50	0.35
Reach 4	5152	100-Year	Existing			728.77		729.00	0.00211	7.08	7118.64	1634.75	0.38
Reach 4	5152	100-Year	Proposed	23073.0	716.49	728.80		729.03	0.00206	7.00	7179.76	1637.97	0.38
Reach 4	4519	025-Year	Existing	13781.6		725.76		726.08	0.00526	8.78	3592.79	1262.62	0.55
Reach 4	4519	025-Year	Proposed	13781.6		725.83		726.14	0.00501	8.62	3677.45	1273.93	0.54
Reach 4	4519	100-Year	Existing	23073.0		726.89		727.29	0.00546	9.78	5078.27	1371.35	0.58
Reach 4	4519	100-Year	Proposed	23073.0	715.19	727.12		727.47	0.00453	9.06	5404.77	1386.96	0.53
Reach 5	4238	025-Year	Existing	14045.0		724.92		725.13	0.00214	6.08	4796.65	1538.85	0.37
Reach 5	4238	025-Year	Proposed	14045.0		725.08		725.27	0.00185	5.74	5047.09	1549.95	0.35
Reach 5	4238	100-Year	Existing	23587.0		725.85		726.17	0.00283	7.52	6282.16	1633.84	0.44
Reach 5	4238	100-Year	Proposed	23587.0	713.83	726.39		726.62	0.00192	6.44	7165.73	1661.06	0.36
Reach 5	4141	025-Year	Existing	14045.0		723.85	723.85	724.72	0.00727	10,73	2879.76	1442.45	0.62
Reach 5	4141	025-Year	Proposed	14045.0		724.61	723.77	725.01	0.00338	7,71	3632.80	1492.28	0.43
Reach 5	4141	100-Year	Existing	23587.0		725.00	724.63	725.73	0.00639	10,88	4581.39	1530.02	0.60
Reach 5	4141	100-Year	Proposed	23587.0	712.69	725.89	724.38	726.35	0.00373	8.78	5549.71	1619.99	0.46
Reach 5	4093			Mult Oper	<u> </u>								
	40.47	0.05		1 40 45 0		707.66	700 00	707.00		6 76	4700.00		- 10
Reach 5	4043	025-Year	Existing	14045.0		723.66	722.60	723.98	0.00237	6.76	4388.90	1446.00	0.40
Reach 5	4043	025-Year	Proposed	14045.0		723.67	722.87	724.15	0.00325	7.92	3527.58	1446.53	0.47
Reach 5	4043	100-Year	Existing	23587.0		725.17	723.42	725.48	0.00211	7.10	6656.94	1547.93	0.39
Reach 5	4043	100-Year	Proposed	23587.0	713.16	725.17	723.76	725.49	0.00216	7.18	6486.40	1547.77	0.40
	3765	0.05		1 40 45 - 5	711.00	707.10		707 77			4160.15	1045 70	0.70
Reach 5	3765	025-Year	Existing	14045.0	711.99	723.10		723.37	0.00206	6.42	4162.16	1245.78	0.38
Reach 5	3765	025-Year	Proposed	14045.0		723.10		723.37	0.00206	6.42	4162.16	1245.78	0.38
Reach 5	3765	100-Year	Existing	23587.0	711.99	724.63		724.94	0.00200	7.02	6165.69	1389.58	0.38
Reach 5	3765	100-Year	Proposed	23587.0	711.99	724.63		724.94	0.00200	7.02	6165.69	1389.58	0.38







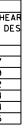
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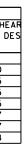
CR 143 AT DRY BERRY CREEK

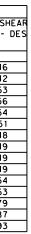
						P	ROPOSED DI	TCH ANALY	SIS - DA-1	1						
				DITCH		FRONT	BACK		DITCH	NORMAL	DESIGN	DESIGN	DELTA CAPACITY	ALLOW		DELTA SHEA
	FLOW LINE	DITCH	LINING	DEPTH	NORMAL DEPTH	SLOPE	SLOPE	CHANNEL	CAPACITY	CAPACITY	VELOCITY	Q(5)	CAP - DES	SHEAR	SHEAR	ALLOW - DE
STATION	ELEVATION	GRADE		(FT)	(FT)	(X:1)	(X:1)	n	(CFS)	(CFS)	(FPS)	(CFS)	(CFS)	STRESS	STRESS	
100+56	718.35															
101+00	719.44	2.48%	CLASS B	0.69	0.28	3	3	0.035	4.53	0.40	1.73	0.40	4.13	2.10	0.43	1.67
101+50	719.68	0.48%	CLASS B	1.01	0.38	3	3	0.035	5,51	0.40	0.93	0.40	5,11	2.10	0.11	1.99
102+00	720.40	1.44%	CLASS B	1.01	0.31	3	3	0.035	9.55	0.40	1.41	0.40	9.15	2,10	0.28	1.82
102+50	720.77	0.74%	CLASS B	1.00	0.35	3	3	0.035	6.66	0.40	1.10	0.40	6.26	2.10	0.16	1.94
103+00	720.87	0.20%	CLASS B	1.04	0.45	3	3	0.035	3.85	0.40	0.67	0.40	3.45	2.10	0.06	2.04
103+50	721.17	0.60%	CLASS B	0.64	0.36	3	3	0.035	1.83	0,40	1.02	0.40	1.43	2.10	0.14	1.96

FLOW LINE         DITCH GRADE         LINING         DITCH (FT)         NORMAL DEPTH (FT)         SLOPE (X:1)         SLOPE (X:1)         CHANNEL (FF)         CAPACITY (CFS)         VELOCITY (CFS)         Q(5) (CFS)         CAP - DES (CFS)         SHEAR STRESS         SHEAR STRESS <th></th>																	
FLOW LINE         DITCH GRADE         LINING         DITCH (FT)         NORMAL DEPTH (FT)         SLOPE (X:1)         CHANNEL (X:1)         CAPACITY (CFS)         CAPACITY (FFS)         VELOCITY (FFS)         Q (5) (CFS)         CAP - DES (CFS)         SHEAR STRESS							P	ROPOSED DI	TCH ANALY	SIS - DA-	3						
STATION         ELEVATION         GRADE         (FT)         (FT)         (X:1)         (X:1)         n         (CFS)         (CFS)         (CFS)         STRESS         STRESS         STRESS           104+00         721.26					DITCH		FRONT	BACK		DITCH	NORMAL	DESIGN	DESIGN	DELTA CAPACITY	ALLOW	DESIGN	DELTA SHE
104+00         721.26		FLOW LINE	DITCH	LINING	DITCH	NORMAL DEPTH	SLOPE	SLOPE	CHANNEL	CAPACITY	CAPACITY	VELOCITY	Q(5)	CAP - DES	SHEAR	SHEAR	ALLOW - D
104+50         720.90         0.72%         CLASS B         0.54         0.45         1.5         2.2         0.035         0.74         0.46         1.22         0.46         0.28         2.10         0.20           105+00         720.65         0.50%         CLASS B         0.51         0.49         1.5         2         0.035         0.50         0.46         1.07         0.46         0.04         2.10         0.15           105+50         720.39         0.52%         CLASS B         0.63         0.46         1.5         2.5         0.035         1.04         0.46         1.07         0.46         0.58         2.10         0.15           106+00         720.07         0.64%         CLASS B         1.12         0.37         2.3         4         0.035         8.81         0.46         1.07         0.46         8.35         2.10         0.15           106+50         719.77         0.60%         CLASS B         1.04         0.34         4         0.035         9.02         0.46         0.99         0.46         8.56         2.10         0.13	STATION	ELEVATION	GRADE		(FT)	(FT)	(X:1)	(X:1)	n	(CFS)	(CFS)	(FPS)	(CFS)	(CFS)	STRESS	STRESS	
105+00         720.65         0.50%         CLASS B         0.51         0.49         1.5         2         0.035         0.50         0.46         1.07         0.46         0.04         2.10         0.15           105+50         720.39         0.52%         CLASS B         0.63         0.46         1.5         2.5         0.035         1.04         0.46         1.07         0.46         0.58         2.10         0.15           106+00         720.07         0.64%         CLASS B         1.12         0.37         2.3         4         0.035         8.81         0.46         1.07         0.46         8.35         2.10         0.15           106+50         719.77         0.60%         CLASS B         1.04         0.34         4         4         0.035         9.02         0.46         0.99         0.46         8.56         2.10         0.13	104+00	721.26															
105+50         720.39         0.52%         CLASS B         0.63         0.46         1.5         2.5         0.035         1.04         0.46         1.07         0.46         0.58         2.10         0.15           106+00         720.07         0.64%         CLASS B         1.12         0.37         2.3         4         0.035         8.81         0.46         1.07         0.46         8.35         2.10         0.15           106+50         719.77         0.60%         CLASS B         1.04         0.34         4         4         0.035         9.02         0.46         0.99         0.46         8.56         2.10         0.13	104+50	720.90	0.72%	CLASS B	0.54	0.45	1.5	2.2	0.035	0.74	0.46	1.22	0.46	0.28	2.10	0.20	1.90
106+00         720.07         0.64%         CLASS B         1.12         0.37         2.3         4         0.035         8.81         0.46         1.07         0.46         8.35         2.10         0.15           106+50         719.77         0.60%         CLASS B         1.04         0.34         4         4         0.035         9.02         0.46         0.99         0.46         8.56         2.10         0.13	105+00	720.65	0.50%	CLASS B	0.51	0.49	1.5	2	0.035	0.50	0.46	1.07	0.46	0.04	2.10	0.15	1.95
106+50 719.77 0.60% CLASS B 1.04 0.34 4 4 0.035 9.02 0.46 0.99 0.46 8.56 2.10 0.13	105+50	720.39	0.52%	CLASS B	0.63	0.46	1.5	2.5	0.035	1.04	0.46	1.07	0.46	0.58	2.10	0.15	1.95
	106+00	720.07	0.64%	CLASS B	1.12	0.37	2.3	4	0.035	8.81	0.46	1.07	0.46	8.35	2.10	0.15	1.95
	106+50	719.77	0.60%	CLASS B	1.04	0.34	4	4	0.035	9.02	0.46	0.99	0.46	8,56	2.10	0.13	1.97
	107+00	719.47	0.60%	CLASS B	0.75	0.34	4	4	0.035	3.77	0.46	0.99	0.46	3.31	2.10	0.13	1.97
107+50 719.18 0.58% CLASS B 0.46 0.34 4 4 0.035 1.01 0.46 0.98 0.46 0.55 2.10 0.12	107+50	719.18	0.58%	CLASS B	0.46	0.34	4	4	0.035	1.01	0.46	0.98	0.46	0.55	2.10	0.12	1.98

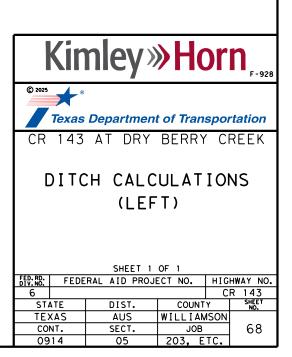
						P	ROPOSED DI	TCH ANALY	SIS - DA-	5						
				DITCH		FRONT	BACK		DITCH	NORMAL	DESIGN	DESIGN	DELTA CAPACITY	ALLOW	DESIGN	DELTA SHE
	FLOW LINE	DITCH	LINING	DEPTH	NORMAL DEPTH	SLOPE	SLOPE	CHANNEL	CAPACITY	CAPACITY	VELOCITY	Q(5)	CAP - DES	SHEAR	SHEAR	ALLOW - DE
STATION	ELEVATION	GRADE		(FT)	(FT)	(X:1)	(X:1)	n	(CFS)	(CFS)	(FPS)	(CFS)	(CFS)	STRESS	STRESS	
109+50	720.97															
110+00	724.69	7.44%	CLASS B	2.54	0.35	4	4	0.035	343.47	1.79	3.58	1.79	341.68	2.10	1.64	0.46
110+50	725.95	2.52%	CLASS B	4.12	0.43	4	4	0.035	726.06	1.79	2.38	1.79	724.27	2.10	0.68	1.42
111+00	726.74	1.58%	CLASS B	4.13	0.47	4	4	0.035	578.64	1.79	2.00	1.79	576.85	2.10	0.47	1.63
111+50	727.68	1.88%	CLASS B	4.14	0.46	4	4	0.035	635.27	1.79	2.13	1.79	633.48	2.10	0.54	1.56
112+00	728.66	1.96%	CLASS B	4.16	0.45	4	4	0.035	657.04	1.79	2.17	1.79	655.25	2.10	0.56	1.54
112+50	729.71	2.10%	CLASS B	4.03	0.45	4	4	0.035	624.89	1.79	2,23	1.79	623.10	2.10	0.59	1.51
113+00	730.83	2.24%	CLASS B	3.65	0.44	4	4	0.035	495.59	1.79	2.28	1.79	493.80	2.10	0.62	1.48
113+50	731.94	2.22%	CLASS B	3.28	0.44	4	4	0.035	371.01	1.79	2.27	1.79	369.22	2.10	0.61	1.49
114+00	733.05	2.22%	CLASS B	3.08	0.44	4	4	0.035	313.71	1.79	2.27	1.79	311.92	2.10	0.61	1.49
114+50	734.16	2.22%	CLASS B	2.59	0.44	4	4	0.035	197.63	1.79	2.27	1.79	195.84	2.10	0.61	1.49
115+00	735.16	2.00%	CLASS B	2.36	0.45	4	4	0.035	146.38	1.79	2.18	1.79	144.59	2.10	0.56	1.54
115+50	735.95	1.58%	CLASS B	1.92	0.47	4	4	0.035	75.05	1.79	2.00	1.79	73,26	2.10	0.47	1.63
116+00	736.43	0.96%	CLASS B	2.14	0.52	4	4	0.035	78.12	1.79	1.66	1.79	76.33	2.10	0.31	1.79
116+50	736.76	0.66%	CLASS B	2.41	0.56	4	4	0.035	88.93	1.79	1.44	1.79	87.14	2.10	0.23	1.87
116+70	736.85	0.45%	CLASS B	2.41	0.60	4	4	0.035	73.43	1.79	1.25	1.79	71.64	2.10	0,17	1.93







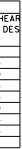


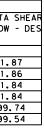


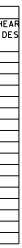
						Р	ROPOSED DI	TCH ANALY	SIS - DA-2	2						
				DITCH		FRONT	BACK		DITCH	NORMAL	DESIGN	DESIGN	DELTA CAPACITY	ALLOW	DESIGN	DELTA SHEA
	FLOW LINE	DITCH	LINING	DEPTH	NORMAL DEPTH	SLOPE	SLOPE	CHANNEL	CAPACITY	CAPACITY	VELOCITY	Q(5)	CAP - DES	SHEAR	SHEAR	ALLOW - DE
STATION	ELEVATION	GRADE		(FT)	(FT)	(X:1)	(X:1)	n	(CFS)	(CFS)	(FPS)	(CFS)	(CFS)	STRESS	STRESS	
100+56	717.82															
101+00	718.04	0.50%	CLASS B	1.96	0.48	4	4	0.035	44.60	1.03	1.13	1.02	43.58	2.10	0.15	1.95
101+50	718.22	0.36%	CLASS B	2.29	0.51	4	4	0.035	57.31	1.03	1.00	1.02	56.29	2.10	0.11	1.99
102+00	718.41	0.38%	CLASS B	2.22	0.50	4	4	0.035	54.21	1.03	1.02	1.02	53.19	2.10	0.12	1.98
102+50	718.60	0.38%	CLASS B	2.29	0.50	4	4	0.035	58.88	1.03	1.02	1.02	57.86	2.10	0.12	1.98
103+00	718.78	0.36%	CLASS B	2.14	0.51	4	4	0.035	47.84	1.03	1.00	1.02	46.82	2.10	0.11	1.99
103+50	718.97	0.38%	CLASS B	2.06	0.50	4	4	0.035	44.40	1.03	1.02	1.02	43.38	2.10	0.12	1,98
104+00	719.22	0.50%	CLASS B	1.87	0.48	4	4	0.035	39.35	1.03	1.13	1.02	38.33	2.10	0.15	1.95
104+50	719.55	0.66%	CLASS B	1.53	0.45	4	4	0.035	26.47	1.03	1.26	1.02	25.45	2.10	0.19	1.91

						PRO	POSED DIT	CH ANALYS	IS - DA-4							
				DITCH		FRONT	BACK		DITCH	NORMAL	DESIGN	DESIGN	DELTA CAPACITY	ALLOW	DESIGN	DELTA S
	FLOW LINE	DITCH	LINING	DEPTH	NORMAL DEPTH	SLOPE	SLOPE	CHANNEL	CAPACITY	CAPACITY	VELOCITY	Q(5)	CAP - DES	SHEAR	SHEAR	ALLOW -
STATION	ELEVATION	GRADE		(FT)	(FT)	(X:1)	(X:1)	n	(CFS)	(CFS)	(FPS)	(CFS)	(CFS)	STRESS	STRESS	
104+50	719.22															
105+00	718.73	0.98%	CLASS B	2.62	0.38	4	4	0.035	135,40	0.76	1.35	0.76	134.64	2.10	0.23	1.8
105+50	718.25	0.96%	CLASS B	3.42	0.40	3.4	3.4	0.035	230.11	0.76	1,39	0.76	229.35	2.10	0.24	1.8
106+00	717.75	1.00%	CLASS B	3.39	0.42	3	3	0.035	200.91	0.76	1,45	0.76	200.15	2.10	0.26	1.8
106+50	717.25	1.00%	CLASS B	2.22	0.42	3	3	0.035	64.97	0.76	1,45	0.76	64.21	2.10	0.26	1.8
107+00	716.62	1.26%	CONCRETE RIPRAP	2.23	0.33	2.3	2	0.015	119.74	0.76	3.16	0.76	118.98	100.00	0.26	99.7
107+50	715.41	2.42%	CONCRETE RIPRAP	2.37	0.31	2	2	0.015	179.97	0.76	4.08	0.76	179.21	100.00	0.46	99.5

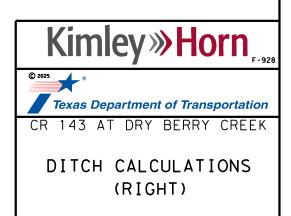
						P	ROPOSED DI	TCH ANALY	SIS - DA-G	5						
				DITCH		FRONT	BACK		DITCH	NORMAL	DESIGN	DESIGN	DELTA CAPACITY	ALLOW	DESIGN	DELTA SHEA
	FLOW LINE	DITCH	LINING	DEPTH	NORMAL DEPTH	SLOPE	SLOPE	CHANNEL	CAPACITY	CAPACITY	VELOCITY	Q(5)	CAP - DES	SHEAR	SHEAR	ALLOW - DE
STATION	ELEVATION	GRADE		(FT)	(FT)	(X:1)	(X:1)	n	(CFS)	(CFS)	(FPS)	(CFS)	(CFS)	STRESS	STRESS	
109+00	722.99															
109+50	724.38	6.62%	CLASS B	1.38	0.43	3	3	0.035	47.03	2.07	3.77	2.07	44.96	2.10	1.77	0.33
110+00	725.74	2.75%	CLASS B	1.23	0.50	3	3	0.035	22.31	2.07	2.71	2.07	20.24	2,10	0.87	1.23
110+50	726.65	1.82%	CLASS B	1.48	0.49	4	4	0.035	40.24	2.07	2.19	2.07	38.17	2.10	0.55	1.55
111+00	727.67	2.04%	CLASS B	1.76	0.48	4	4	0.035	67.62	2.07	2.28	2.07	65.55	2.10	0.61	1.49
111+50	728.85	2.36%	CLASS B	1.67	0.46	4	4	0.035	63.23	2.07	2.41	2.07	61.16	2.10	0.68	1.42
112+00	730.08	2.46%	CLASS B	1.54	0.46	4	4	0.035	52.01	2.07	2.45	2.07	49.94	2.10	0.71	1.39
112+50	731.13	2.10%	CLASS B	1.47	0.47	4	4	0.035	42.45	2.07	2.31	2.07	40.38	2.10	0.62	1.48
113+00	732.00	1.74%	CLASS B	1.57	0.49	4	4	0.035	46.05	2.07	2.15	2.07	43.98	2.10	0.53	1.57
113+50	732.87	1.74%	CLASS B	1.82	0.49	4	4	0.035	68.29	2.07	2.15	2.07	66.22	2.10	0.53	1.57
114+00	733.73	1.72%	CLASS B	1.84	0.49	4	4	0.035	69.90	2.07	2.14	2.07	67.83	2.10	0.53	1.57
114+50	734.59	1.72%	CLASS B	2.62	0.49	4	4	0.035	179.38	2.07	2.14	2.07	177.31	2.10	0.53	1.57
115+00	735.59	2.00%	CLASS B	2.65	0.48	4	4	0.035	199.40	2.07	2.27	2.07	197.33	2.10	0.60	1.50
115+50	736.30	1.42%	CLASS B	2.50	0,51	4	4	0.035	143.84	2.07	1.99	2.07	141.77	2.10	0.45	1.65
116+00	737.01	1.42%	CLASS B	2.44	0,51	4	4	0.035	134.81	2.07	1.99	2.07	132.74	2.10	0.45	1.65
116+50	737.72	1.42%	CLASS B	2.23	0,51	4	4	0.035	106.05	2.07	1.99	2.07	103.98	2.10	0.45	1.65
116+70	738.00	1.40%	CLASS B	2.14	0.51	4	4	0.035	94.34	2.07	1.98	2.07	92.27	2.10	0.45	1.65











	SHEET 1 OF 1									
FED.RD. DIV.NO.	FEDE	FEDERAL AID PROJECT NO. HIGHWAY NO.								
6		BR2020(732	C	R 143						
ST	<b>ATE</b>	DIST.	COUNT	ſΥ	SHEET NO.					
TEX	XAS	AUS	WILLIAMSON							
CONT.		SECT.	JOB		69					
09	14	05	203, E	TC.						

# CONTRACTION SCOUR ANALYSIS (PRESSURE FLOW)

 $\frac{1}{Q_{Ue}} = \frac{Q_1}{(h_{Ue}/h_U)^{8/7}} \text{ and } \frac{y_2/h_{Ue}}{(Q_2/Q_{Ue})^{6/7}(W_1/W_2)^{k_1}} \text{ and } \frac{1}{h_b} = 0.5(h_b*h_t/h_U^2)^{0.2}(1-h_w/h_t)^{-0.1}$ USING HEC-18 EQ 6.14

y<sub>s</sub>= y<sub>2</sub>++-h<sub>b</sub>

WHERE:

 $\mathtt{Q}_{\mathsf{U}\mathsf{e}^{\texttt{e}}}$  effective channel discharge for live bed conditions and bridge overtopping flows

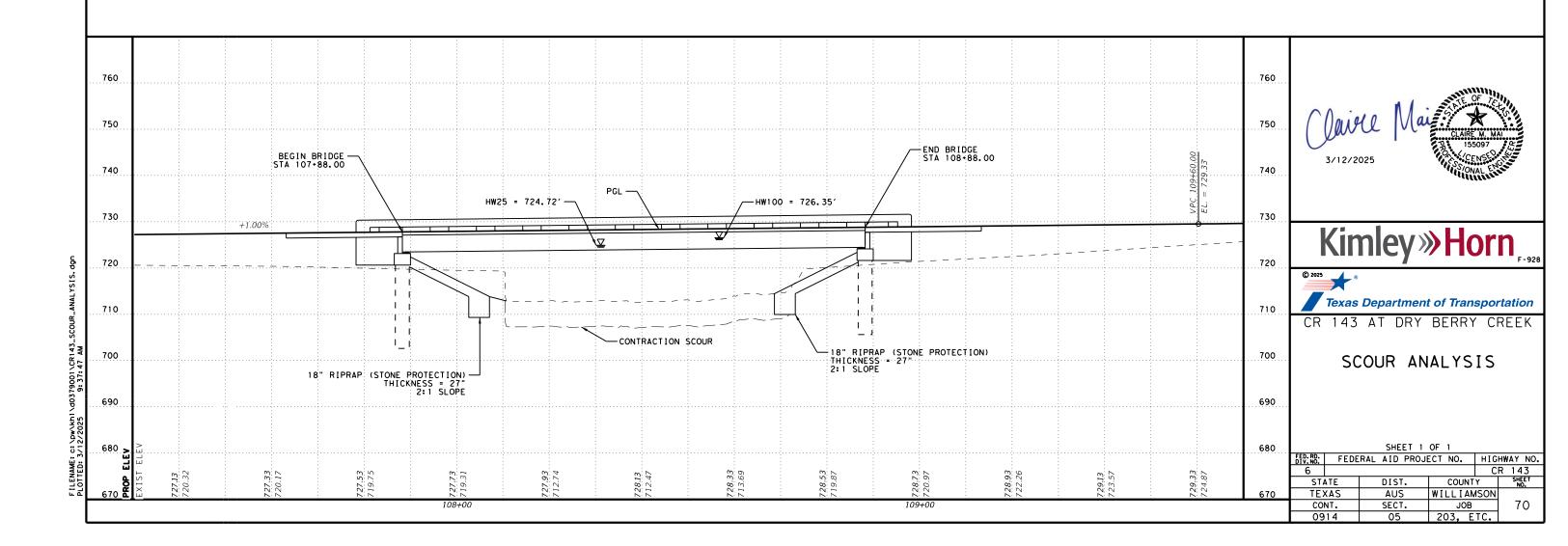
- Q1 = UPSTREAM CHANNEL DISCHARGE
- h<sub>u</sub> = UPSTREAM CHANNEL FLOW DEPTH
- hue<sup>=</sup> EFFECTIVE CHANNEL FLOW DEPTH FOR LIVE BED CONDITIONS AND BRIDGE OVERTOPPING FROM STAGNATION STREAMLINE TO AVERAGE CHANNEL BOTTOM ELEVATION
- y<sub>2</sub> = AVERAGE DEPTH OF FLOW
- Q<sub>2</sub> = FLOW THROUGH THE BRIDGE
- $w_1$  = width of flow contributing to transport of bed material
- $W_2$  = WIDTH OF FLOW CONTRIBUTING TO TRANSPORT OF BED MATERIAL IN UPSTREAM CROSS SECTION
- $\kappa_1$  = determined via relationship between sheer velocity and fall velocity
- + = SEPARATION ZONE THICKNESS
- $\mathbf{h}_{\mathbf{b}}$  = vertical size of bridge opening prior to scour
- $\mathbf{h}_{\texttt{T}}$  = DISTANCE FROM WATER SURFACE TO LOW CHORD
- h<sub>w</sub> = WEIR FLOW HEIGHT
- ys = AVERAGE CONTRACTION SCOUR DEPTH

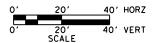
# 50 YEAR SCOUR CALCULATIONS CONTRACTION SCOUR COMPUTATION RESULTS

FREQ (yrs)	Q <sub>ue</sub>	Q1	hu	h <sub>ue</sub>	У <sub>2</sub>	0 <sub>2</sub>	W <sub>1</sub>	W2	k <sub>1</sub>
	6582.6	3751.49	9.11	14.9	11.76	5090.8	68	70	0.65
50	+	h <sub>D</sub>	h <sub>t</sub>	h <sub>w</sub>	У <sub>S</sub>				
	3.8	10.55	2.6	-4.1	5.50				

## 100 YEAR SCOUR CALCULATIONS CONTRACTION SCOUR COMPUTATION RESULTS

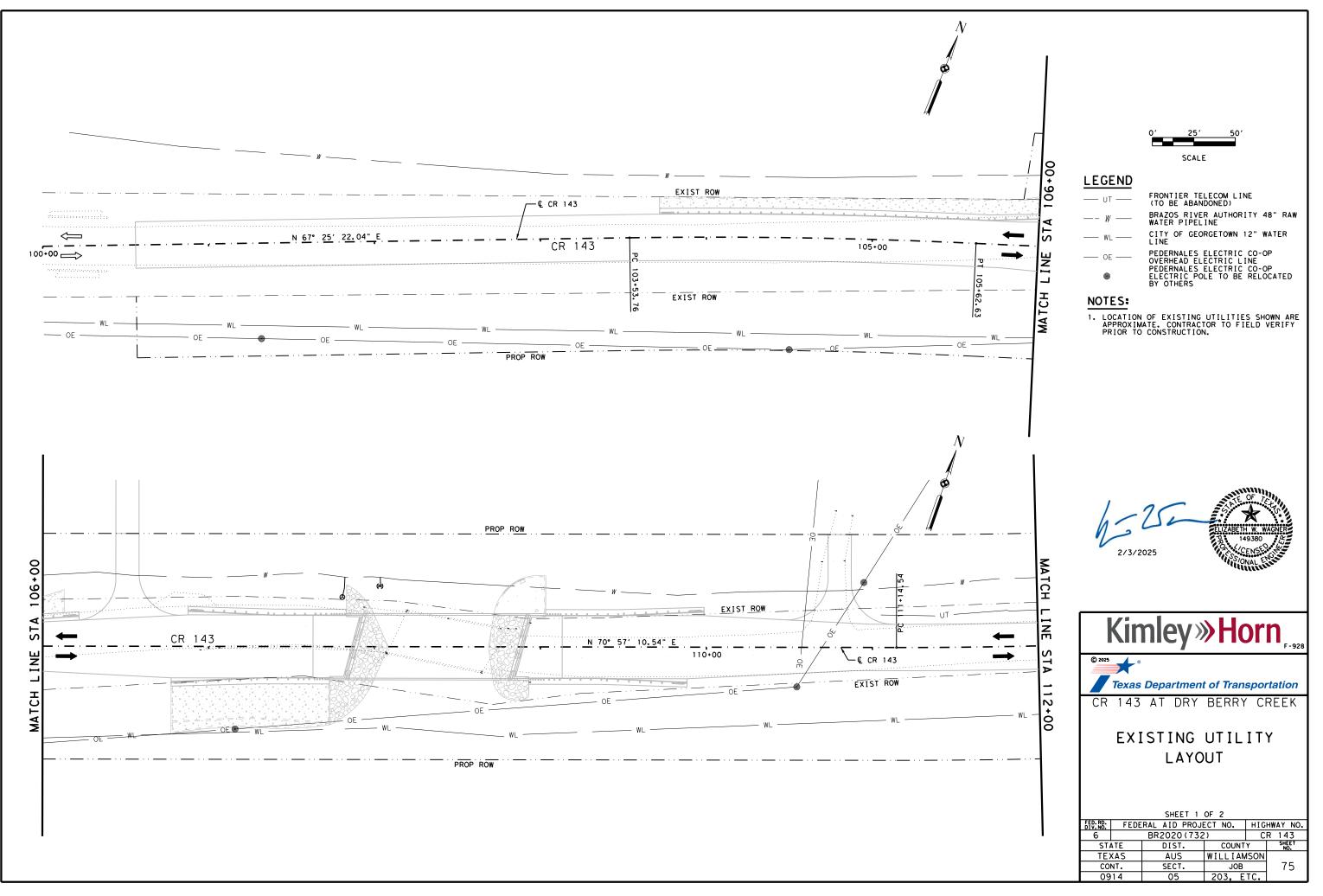
FREQ (yrs)	Q <sub>ue</sub>	Q1	hu	h <sub>ue</sub>	У <sub>2</sub>	Q2	w <sub>1</sub>	W2	ĸ <sub>1</sub>
	6965	4279.59	9.73	14.9	10.99	4976.4	68	70	0.65
100	+	hb	h <sub>t</sub>	hw	У <sub>S</sub>		-	-	-
	4.0	10.55	3.4	-3.59	4.50				



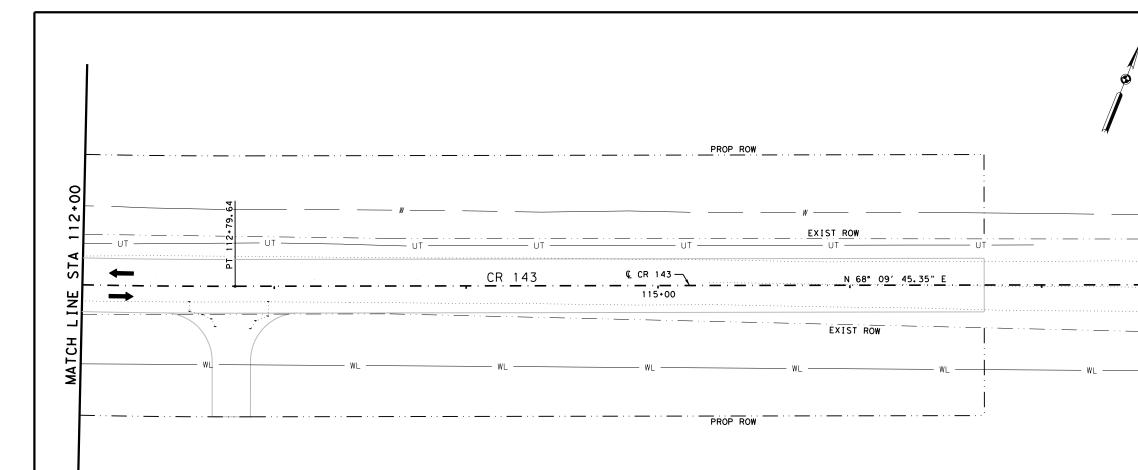


# NOTES:

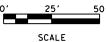
- 1. SCOUR ANALYSIS BASED ON TXDOT GEOTECHNICAL MANUAL (GM) AND FHWA H.E.C.-18 "EVALUATING SCOUR AT BRIDGES".
- THE D50 SOIL PARTICLE SIZE FOR THIS PROJECT WAS PROVIDED BY TERRACON IN THE GEOTECHNICAL REPORT, LISTED AS 4.907 MM OR 0.016 FT.
- 3. THE MAXIMUM CALCULATED SCOUR WAS DURING THE 50 YEAR STORM AT A DISHCHARGE OF 2104 CFS. THE BRIDGE WAS ANALYZED IN THE 50 AND 100 YR EVENTS.
- 4. SEE DRAINAGE REPORT FOR MORE INFORMATION ON SCOUR ANALYSIS.



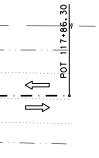
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— UT —	FRONTIER TELECOM LINE (TO BE ABANDONED)
—- W —	BRAZOS RIVER AUTHORITY 48" RAW WATER PIPELINE
WL	CITY OF GEORGETOWN 12" WATER LINE
OE	PEDERNALES ELECTRIC CO-OP OVERHEAD ELECTRIC LINE
۲	PEDERNALES ELECTRIC CO-OP ELECTRIC POLE TO BE RELOCATED BY OTHERS

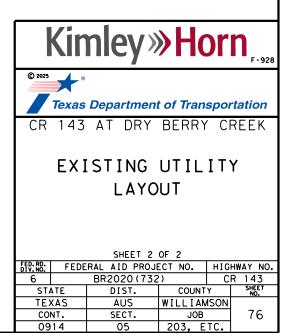
# NOTES:

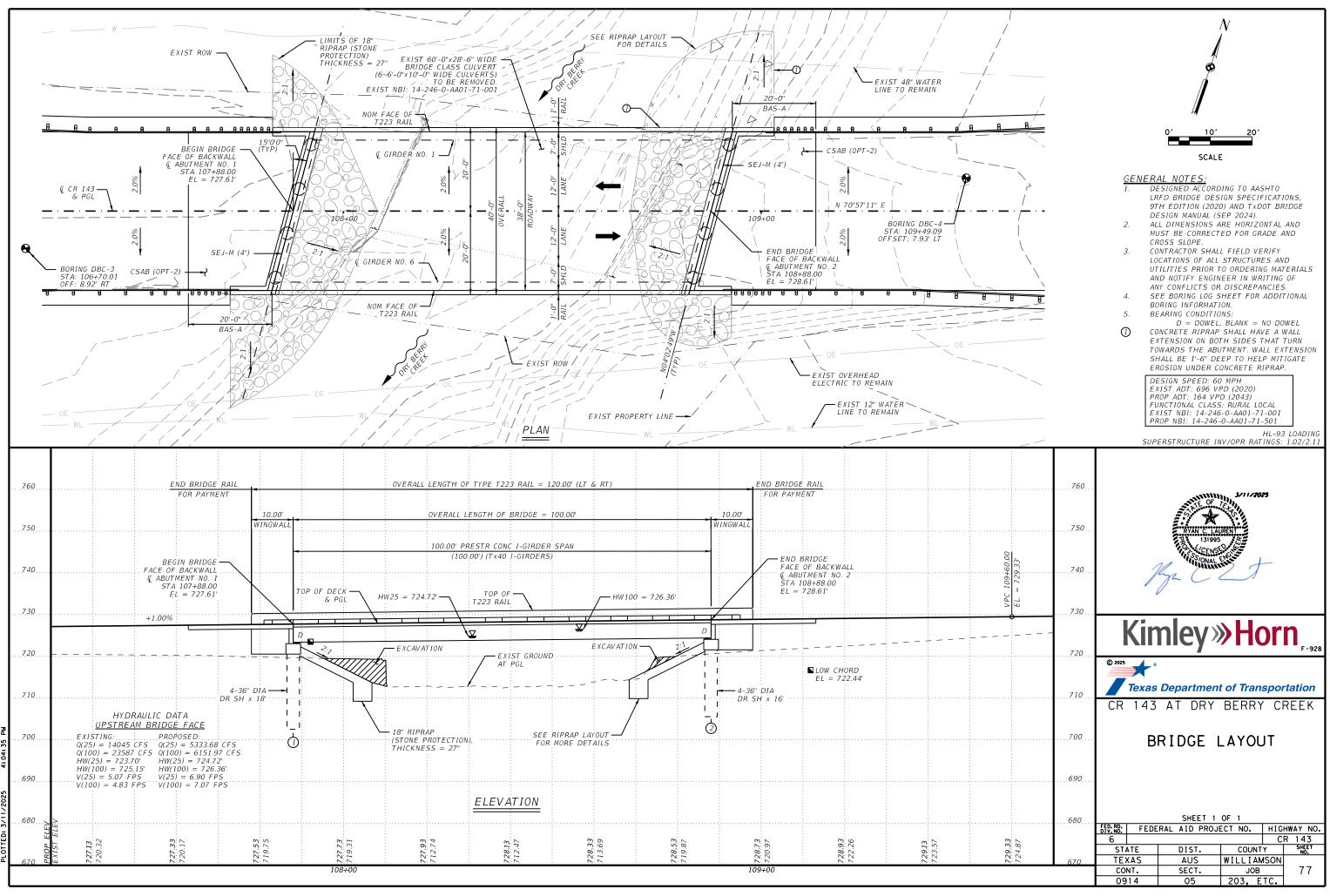
LEGEND

1. LOCATION OF EXISTING UTILITIES SHOWN ARE APPROXIMATE. CONTRACTOR TO FIELD VERIFY PRIOR TO CONSTRUCTION.



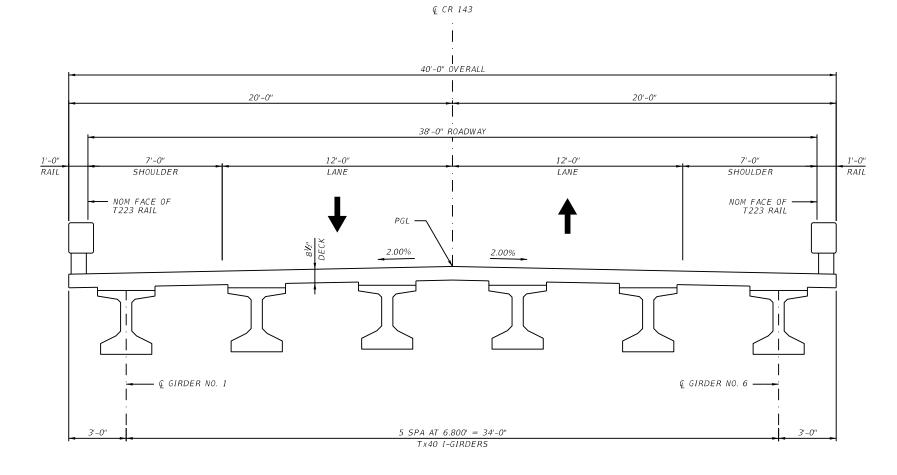






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# BRIDGE TYPICAL SECTION



0, 2.5, 5, SCALE
2/3/2025
RVANIC LAURENT 131995 SEINES MAL
Kimley »Horn
<b>Texas Department of Transportation</b> CR 143 AT DRY BERRY CREEK
BRIDGE TYPICAL SECTIONS
SHEET 1 OF 1FEDERAL AID PROJECT NO. HIGHWAY NO.6BR2020(732)CR 143STATEDIST.COUNTYSHEETTEXASAUSWILLIAMSONNO.CONT.SECT.JOB78091405203, ETC.78

		SUMMARY	OF ESTIMAT	ED QUANTIT		3 AT DRY B	ERRY CREEF	K			
CSJ: 0914-05-203	ITEM NO.	0400 7010	0416 7006	0420 7012	0422 7001	0422 7013	0425 7003	0432 7002	0432 7043	0450 7008	0454 7004
BRIDGE ELEMENT	BID ITEM DESCRIPTION		DRILL SHAFT (36 IN)	CL C CONC (ABUT)	REINF CONC SLAB	APPROACH SLAB	PRESTR CONC GIRDER (TX40)	RIPRAP (CONC) (5 IN)	RIPRAP (STONE PROTECTION)(18 IN)	RAIL (TY T223)	SEALED EXPANSION JOINT (4 IN) (SEJ - M)
NBI#: 14-246-0-AA01-71-501		CY	LF	СҮ	SF	СҮ	LF	СҮ	CY	LF	LF
2 - ABUTMENTS		191	136	51.2		77		16	306	40.0	82
1 - 100.00' PRESTR CONC Tx40 I-GIF	RDER UNIT				4,000		596.94			200.0	
ТОТ	- AL	191	136	51.2	4,000	77	596.94	16	306	240.0	82

- NOTES: 1. ALL QUANTITIES ON THIS SHEET PERTAIN TO CSJ 0914-05-203. 2. CL C CONC (ABUT) QUANTITY INCLUDES SHEAR KEY QUANTITY.

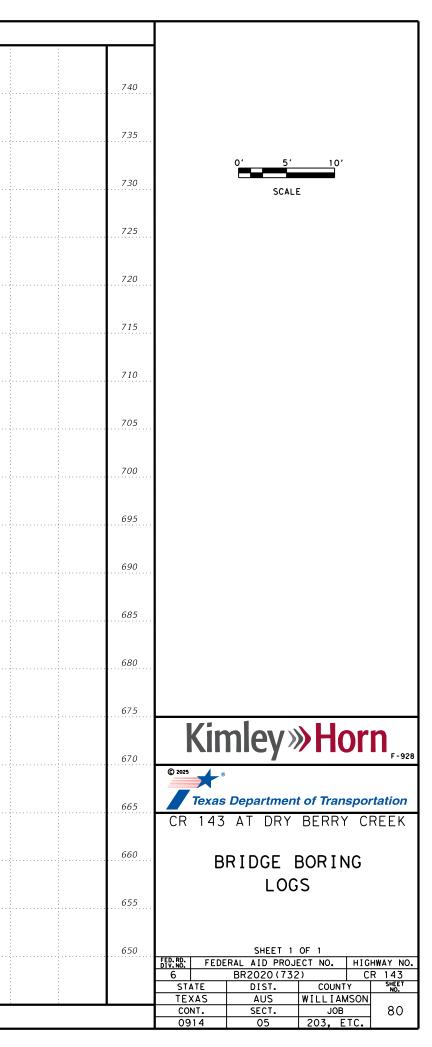


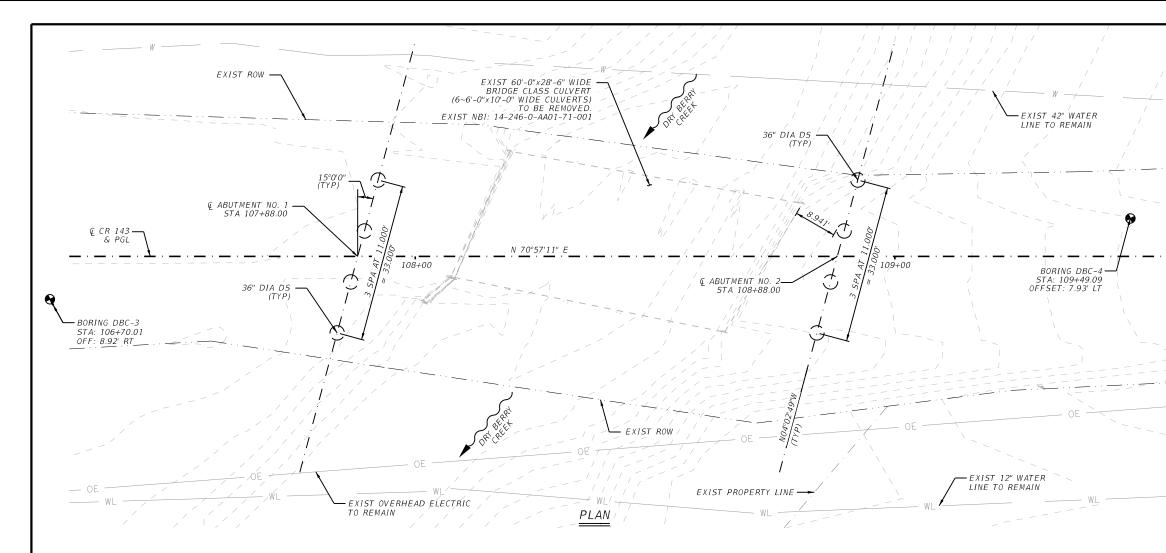
0914

JOB

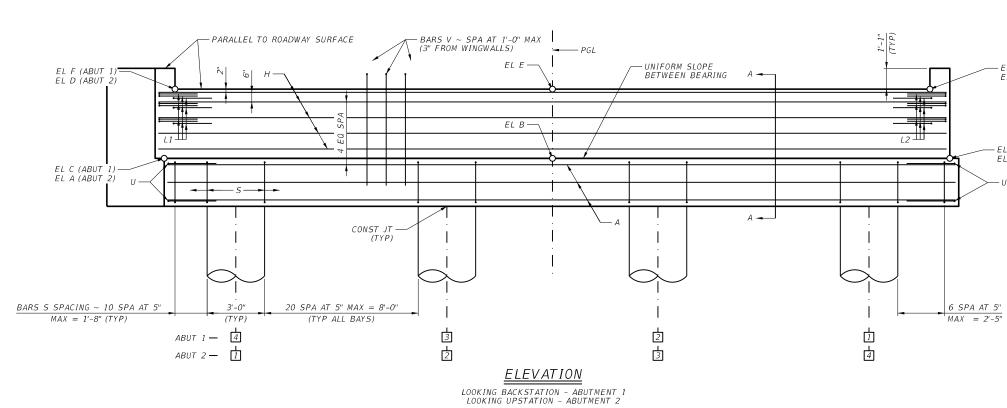
203, ETC.

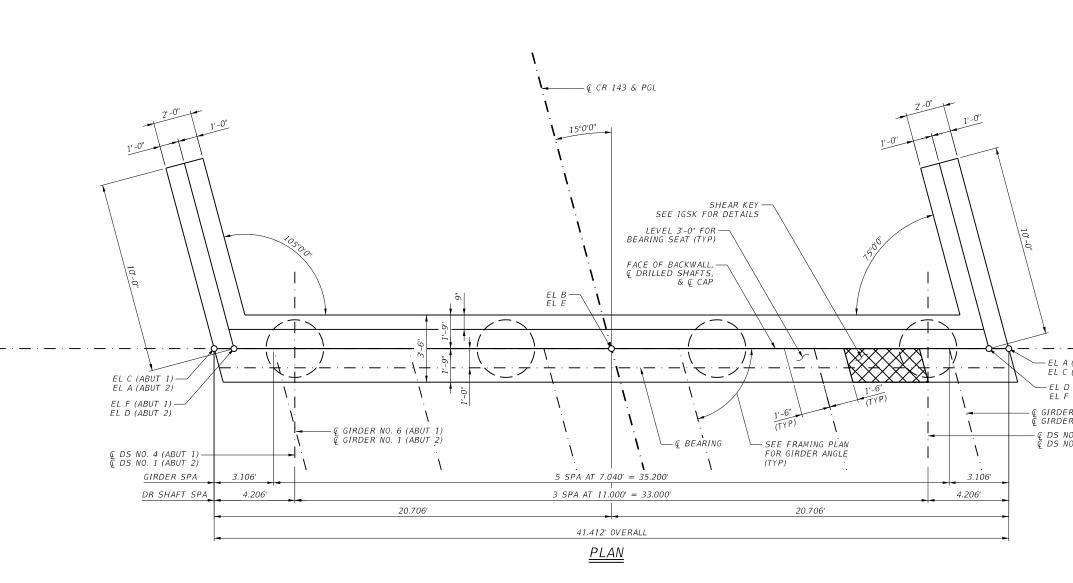
	BORING DBC-4 ELEV = 726.00'
	STA = 109+49.09 0FF = 7.93' LT
BORING DBC-3 ELEV = 721.00' STA = 106+70.01	
 OFF = 8.92' RT	<i>ASPHALT, 3" Asphalt, 8" Base</i> CLAY, Iean, sandy, slightly
	50 (2.5) 50 (0) 50 (2.
 SAND, clayey, loose, dark	with clayey seams
50 (0.5) 50 (1)	50 (0.5) 50 (0.5)
 LIMESTONE, very hard, light tan to pale brown	to light gray, moderately to highly fractured, with highrequent marly seams
50 (0) 50 (0)	50 (0.25) 50 (0.5)
LIMESTONE, very hard, gray to lightly gray, slightly to	
 50 (0) 50 (0) moderately fractured	50 (0.5) 50 (0)
	50 (0.25) 50 (0)
50 (0) 50 (0)	50 (0.5) 50 (0.25)
	50 (0) 50 (0)
50 (0.5) 50 (0)	50 (0.5) 50 (0)
50 (0.5) 50 (0.25) LIMESTONE, very hard, brown to light brown, highly fractured,	40 (6) 50 (4)
with sandy seams and chert deposits	LIMESTONE, very hard, brown to light brown, moderately fractured,
	50 (0) 50 (0) 1 Inght brown, moderately fractured, with sandy seams and chert deposits
50 (0 5) 50 (0)	
50 (0) 50 (0)	50 (0.5) 50 (0.5)
 B/H = 661.00'	B/H = 661.00'

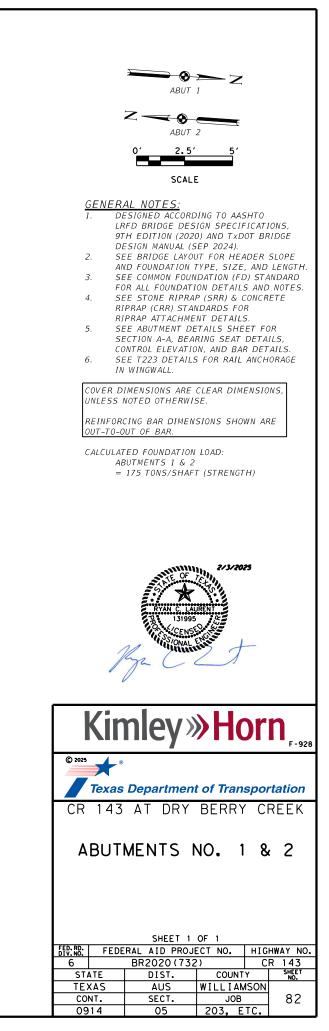




0' 10' 20' SCALE
AVAN E LAURENT 131995 CENSE CONAL ECONT
Kimley »Horn F-928 Texas Department of Transportation CR 143 AT DRY BERRY CREEK FOUNDATION LAYOUT
SHEET 1 OF 1DIV.NC.FEDERAL AID PROJECT NO.HIGHWAY NO.6BR2020(732)CR 143STATEDIST.COUNTYTEXASAUSWILLIAMSONCONT.SECT.JOB091405203, ETC.



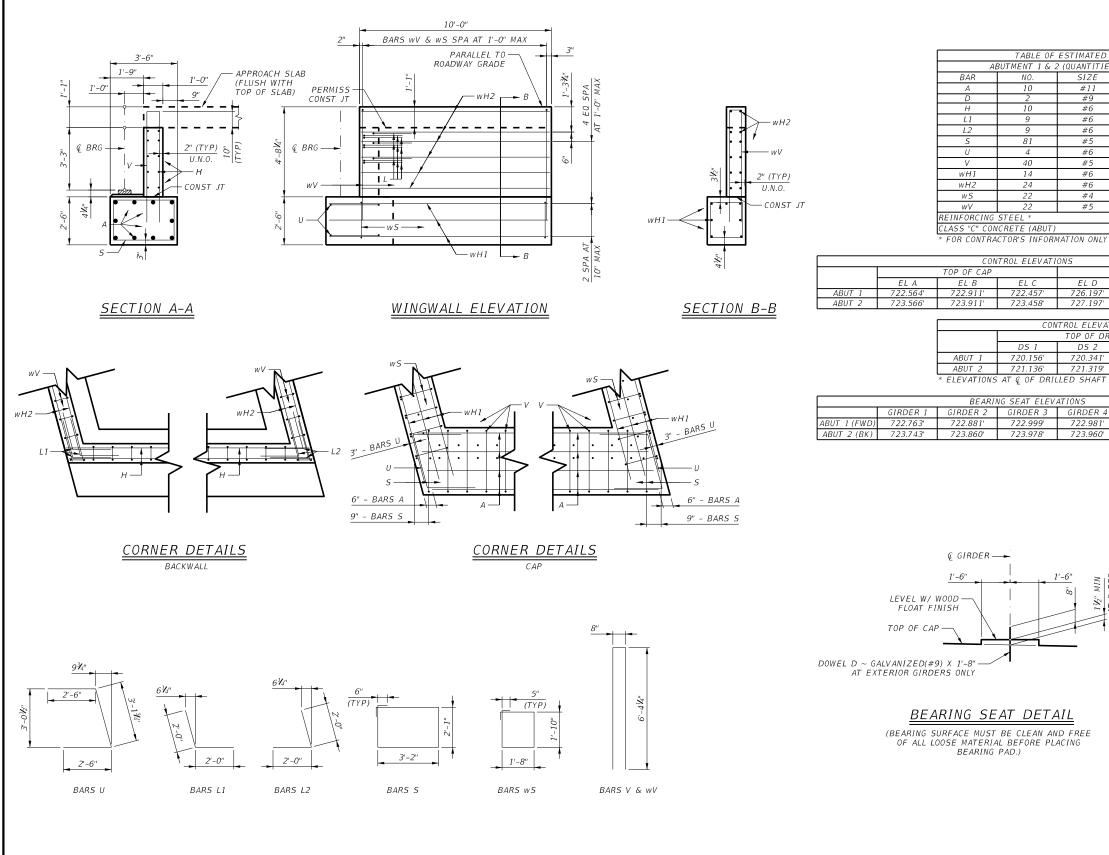




← EL A (ABUT 1) EL C (ABUT 2) ← EL D (ABUT 1) EL F (ABUT 2) GIRDER NO. 1 (ABUT 1) GIRDER NO. 6 (ABUT 2) G DS NO. 1 (ABUT 1) G DS NO. 4 (ABUT 2)

> - EL D (ABUT 1) EL F (ABUT 2)

EL A (ABUT 1) EL C (ABUT 2)



Q	JANTITIES					
ΞS	ES PER ABUTMENT)					
	LENGTH	WEIGHT				
	40'-5"	2,147				
	1'-8"	11				
	41'-1"	617				
	4'-0"	54				
	4'-0"	54				
	11'-6"	972				
	8'-2"	49				
	13'-4"	556				
	11'-5"	240				
	9'-8"	348				
	7'-10"	115				
	13'-4"	306				
	LB	5,469				
	СҮ	25.6				

SIZE

#11

#9

#6

#6

#6

#5

#6

#5

#6

#6

#4

#5

EL D

726.197'

727.197'

722.981'

721.319

ΤC	OP OF BACKWA	LL
	EL E	EL F
	726.527'	726.097'
	727.527'	727.097'

TROL ELEVATIONS								
TOP OF DRILLED SHAFT*								
DS 2	DS 3	DS 4						
720.341'	720.071'							
721.319' 721.291' 721.050'								

1	GIRDER 5	GIRDER 6
	722.826'	722.672'
	723.806'	723.652'

SCALE

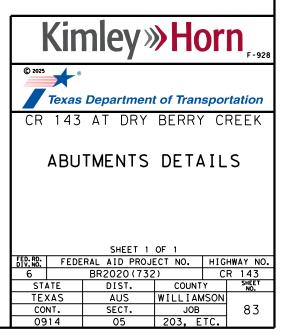
### MATERIAL NOTES:

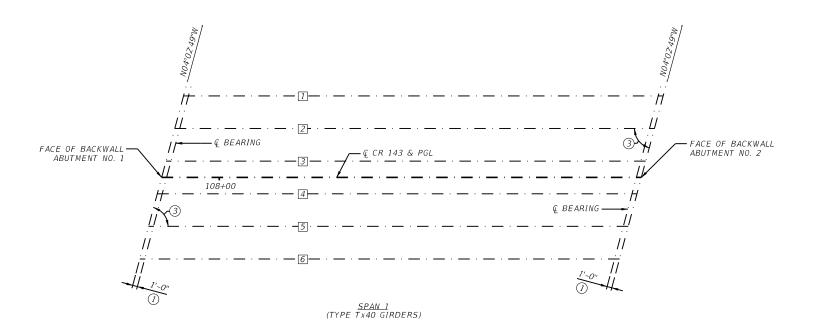
PROVIDE CLASS C CONCRETE, f'c = 3600 PSI PROVIDE GRADE 60 REINFORCING STEEL. GALVANIZE DOWEL BARS D.

COVER DIMENSIONS ARE CLEAR DIMENSIONS, UNLESS NOTED OTHERWISE.

REINFORCING BAR DIMENSIONS SHOWN ARE OUT-TO-OUT OF BAR.







# FRAMING PLAN

#### BENT REPORT

	BENT NO. 1 (N 4	1 2 49.46 W)	
DISTANCE	BETWEEN STATION		17.600 L
	BEAM SPA	C. BEAM ANGLE	
	(C.L. BENT)	DMS	
SPAN 1	BEAM 1 0.000	75 0 0	
	BEAM 2 7.040	75 0 0	
	BEAM 3 7.040	75 0 0	
	BEAM 4 7.040	75 0 0	
	BEAM 5 7.040	75 0 0	
	BEAM 6 7.040	75 0 0	
	TOTAL 35.200		
	BENT NO. 2 (N 4	1 2 49.46 W)	
DISTANCE	BETWEEN STATION	LINE AND BEAM 1,	17.600 L
	BEAM SPA	C. BEAM ANGLE	
	(C.L. BENT)	DMS	
SPAN 1	BEAM 1 0.000	75 0 0	

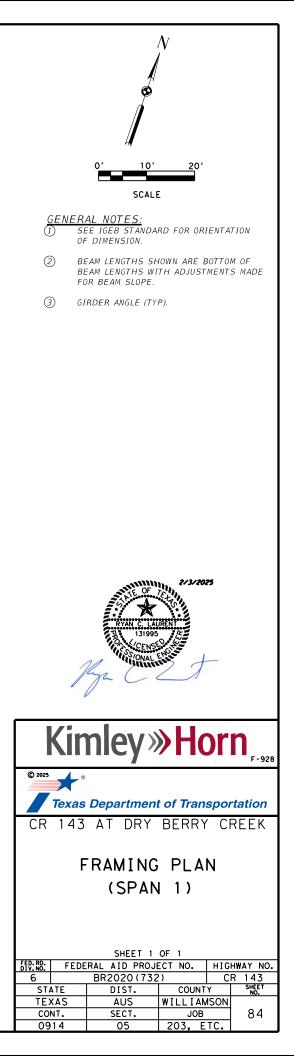
			(C.	L. BENT)	D	М	S	
PAN	1	BEAM	1	0.000	75	0	0	
		BEAM	2	7.040	75	0	0	
		BEAM	3	7.040	75	0	0	
		BEAM	4	7.040	75	0	0	
		BEAM	5	7.040	75	0	0	
		BEAM	6	7.040	75	0	0	
		TOTAL	_	35.200				

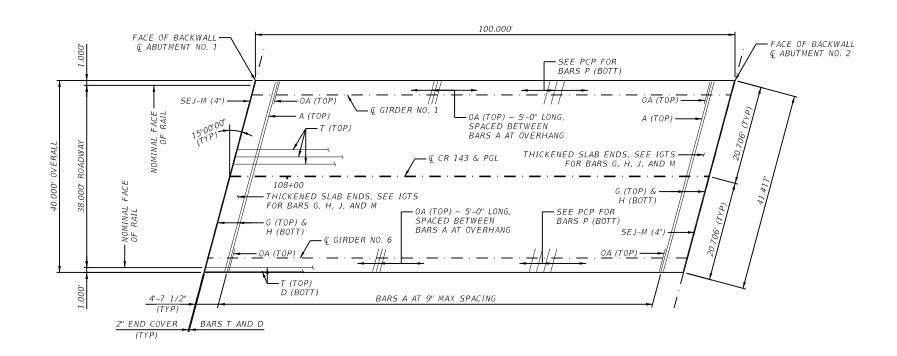
#### BEAM REPORT

#### BEAM REPORT, SPAN 1 HORIZONTAL DISTANCE TRUE DISTANCE BEAM C-C BENT C-C BRG. BOT. BM. FLG.(2) SLOPE

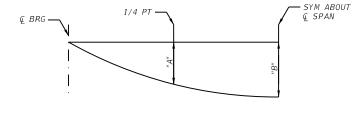
					$\bigcirc$
BEAM	1	100.000	97.929	99.49	0.0100
BEAM	2	100.000	97.929	99.49	0.0100
BEAM	3	100.000	97.929	99.49	0.0100
BEAM	4	100.000	97.929	99.49	0.0100
BEAM	5	100.000	97.929	99.49	0.0100
BEAM	6	100.000	97.929	99.49	0.0100

ğ





PLAN



<u>DEAD LOAD</u> DEFLECTION DIAGRAM CALCULATED DEFLECTIONS SHOWN ARE

CALOUATED DEFLECTIONS SHOWN ARE DUE TO THE CONCRETE SLAB ONLY (Ec = 5000 KSI). ADJUST VALUES AS REQUIRED FOR EXTERIOR GIRDERS AND IF OPTIONAL SLAB FORMING IS USED. THESE VALUES MAY REQUIRE FIELD VERIFICATION.

TABLE OF ESTIMATED QUANTITIES									
	REINF	PRESTRESSED	TOTAL						
	CONCRETE	CONCRETE	REINF						
	SLAB	GIRDERS	STEEL *						
		(Tx40) 1	2						
	SF	LF	LB						
SPAN 1	4,000	596.94	9,200						
* FOR CONTRACTORIC INFORMATION ONLY									

\* FOR CONTRACTOR'S INFORMATION ONLY

" <u></u> \"	
71	"B"
SPAN NO. FT	FT
1 0.132	0.185

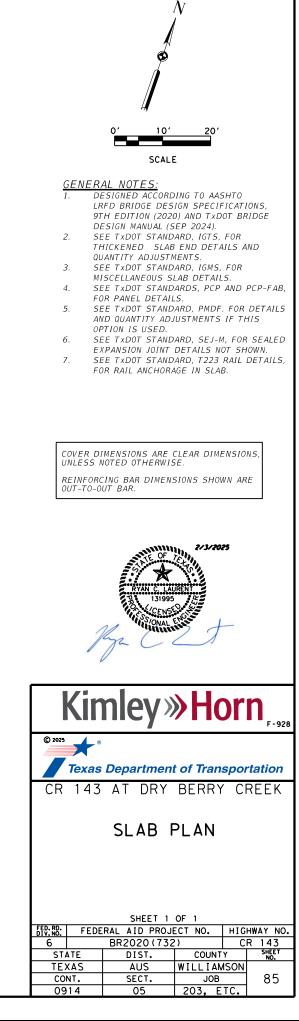
BAR 1	<i>FABLE</i>
BAR	SIZE
A	#4
D	#4
G	#4
Н	#4
J	#4
М	#4
0A	#5
Р	#4
Т	#4

# MATERIAL NOTES:

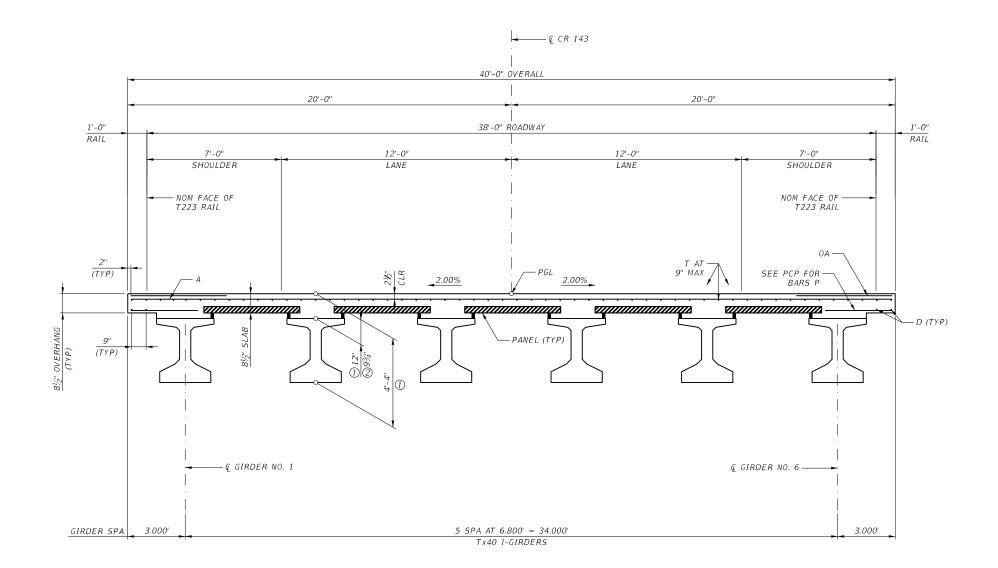
ALL CONCRETE SHALL BE CLASS S CONCRETE (f'c = 4,000 PSI)

ALL REINFORCING SHALL BE GRADE 60 REINFORCING STEEL.

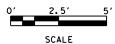
PROVIDE BAR LAPS, WHEN REQUIRED. UNCOATED ~ #4 - 1'-7".



LAPS IN BARS A AND T SHALL BE STAGGERED AND ALTERNATED TO MAXIMIZE THE DISTANCE BETWEEN ADJACENT SPLICES.

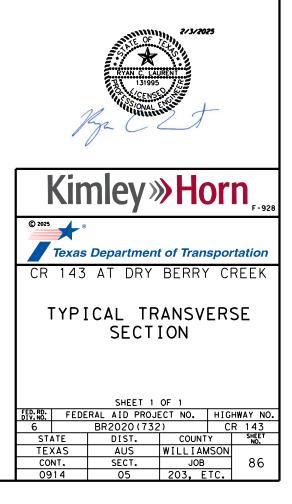


TYPICAL TRANSVERSE SECTION



## <u>NOTES:</u>

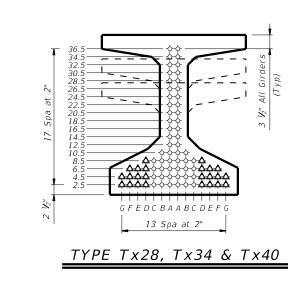
- ① VALUE IS MEASURED AT & OF BEARING
- 2 VALUE IS MEASURED AT Q OF SPAN

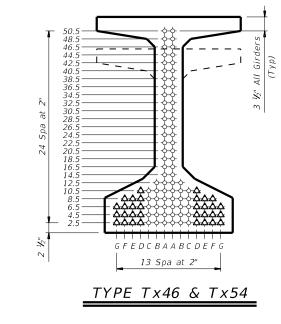


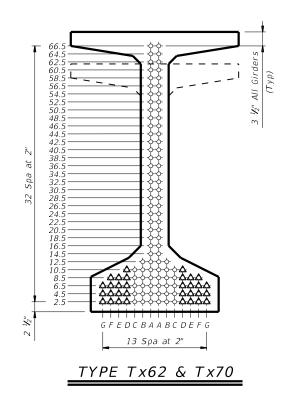
			D	ESIGN	ED GIR	DERS				DEP	RESSED	СОЛС	CRETE		OPTIO	NAL DESIG	δN		LC	DAD R	AT ING ORS
STRUCTURE	SPAN NO.	GIRDER NO.	GIRDER TYPE	NON- STD STRAND PATTERN	PR. TOTAL NO.	SIZE	SING ST STRGTH fpu	"e" ¢	"e" END	ST PAT NO.	RAND TERN	RELEASE STRGTH 1 f'ci	MINIMUM 28 DAY COMP STRGTH f'c	DESIGN LOAD COMP STRESS (TOP Q) (SERVICE I)	DESIGN LOAD TENSILE STRESS (BOTT Ç) (SERVICE III)	REQUIRED MINIMUM ULTIMATE MOMENT CAPACITY (STRENGTH I)	DISTR FAC	ELOAD IBUTION CTOR 2	STRE	NGTH I	SERVICE III
CR 143 AT DRY BERRY CREEK	1	1-6	T X 40		36	(in) 0.6	(ksi) 270	(in) 13.93	(in) 8.93	6	(in) 36.5	5.800	6.600	4.002	-4.439	5326	0.546	Shear 0.770	Inv 1.59		Inv 1.02

DISCLAIMER: The use of this standard is TXDOT assumes no responsil









# NON-STANDARD STRAND PATTERNS

PATTERN	STRAND ARRANGEMENT AT € OF GIRDER

1) Based on the following allowable stresses (ksi):

Compression = 0.65 f'ci

Tension =  $0.24\sqrt{f'ci}$ 

Optional designs must likewise conform.

(2) Portion of full HL93.

### DESIGN NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Load rated using Load and Resistance Factor Rating according to

AASHTO Manual for Bridge Evaluation. Optional designs for girders 120 feet or longer must have a calculated residual camber equal to or greater than that of the designed girder.

Prestress losses for the designed girders have been calculated for a relative humidity of 60 percent. Optional designs must likewise conform.

### FABRICATION NOTES:

Provide Class H concrete.

Provide Grade 60 reinforcing steel bars.

Use low relaxation strands, each pretensioned to 75 percent of fpu.

Strand debonding must comply with Item 424.4.2.2.2.4. Full-length debonded strands are only permitted in positions marked  $\Delta$ . Double wrap full-length debonded strands in outer most position of each row.

When shown on this sheet, the Fabricator has the option of furnishing either the designed girder or an approved optional design. All optional design submittals must be signed, sealed and dated by a Professional Engineer registered in the State of Texas.

Seal cracks in girder ends exceeding 0.005" in width as directed by the Engineer. The fabricator is permitted to decrease the spacing of Bars R and S by providing additional bars to help limit crack width provided the decreased spacing results in no less than 1" clear between bars. The fabricator must take an approved corrective action if cracks greater than 0.005" form on a repetitive basis.

## DEPRESSED STRAND DESIGNS:

Locate strands for the designed girder as low as possible on the 2" grid system unless a non-standard strand pattern is indicated. Fill row "2.5", then row "4.5", then row "6.5", etc., beginning each row in the "A" position and working outward until the required number of strands is reached. All strands in the "A" position must be depressed, maintaining the 2" spacing so that, at the girder ends, the upper two strands are in the position shown in the table.



# **STORMWATER POLLUTION PREVENTION PLAN (SWP3):**

This SWP3 has been developed in accordance with the TPDES Construction General Permit TXR150000 (CGP). The Texas Department of Transportation (TxDOT) ensures that project specifications include adequate best management practices (BMPs) for this project.

For all projects with soil disturbing activity and for projects that have Environmental, Permits, Issues, and Commitments (EPICs) dependent on stormwater controls and water quality measures TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office, Area Office, or electronically.

This SWP3 is consistent with requirements specified in applicable stormwater plans and the projects environmental permits, issues, and commitments (EPICs). A copy of the CGP is included in Attachment 2.12 of the SWP3 binder.

# **1.0 SITE/PROJECT DESCRIPTION**

**1.1 PROJECT CONTROL SECTION JOB (CSJ):** 0914-05-203, ETC.

# 1.2 PROJECT LIMITS:

From: ON CR 143 AT DRY BERRY CREEK

_	ETC.	
To:	., ETC.	

# **1.3 PROJECT COORDINATES:**

BEGIN:	(Lat)	30.7302	,(Long)	-97.658703
END:	(Lat)_	30.7316	,(Long)	-97.653825

4.53 1.4 TOTAL PROJECT AREA (Acres): \_

3.31 1.5 TOTAL AREA TO BE DISTURBED (Acres):

# **1.6 NATURE OF CONSTRUCTION ACTIVITY:**

CONSISTING OF REPLACING BRIDGE AND APPROACHES

# **1.7 MAJOR SOIL TYPES:**

Soil Type	Description
DENTON SILTY CLAY 0 TO 1% SLOPES	7.9%
KRUM SILTY CLAY 1 TO 3% SLOPES	20.2%
OAKALLA SILTY CLAY 0 TO 2% SLOPES	71.9% FREQUENTLY FLOODED

# **1.8 PROJECT SPECIFIC LOCATIONS (PSLs):**

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below:

- PSLs determined during preconstruction meeting
- X PSLs determined during construction
- No PSLs planned for construction

Туре	Sheet #s

All off-ROW PSLs required by the Contractor are the Contractor's responsibility. The Contractor shall secure all permits required by local, state, federal laws for off-ROW PSLs. The contractor shall provide diagrams, areas of disturbance, acreage, and BMPs for all off-ROW PSLs within one mile of the project.

#### **1.9 CONSTRUCTION ACTIVITIES:** • • • • • • • •

Other:

Use the following list as a starting point when developing the Construction Activity Schedule and Ceasing Record in
Attachment 2.5.)
Mobilization
Install sediment and erosion controls
I Blade existing topsoil into windrows, prep ROW, clear and grad to the second strain of t
Grading operations, excavation, and embankment
Excavate and prepare subgrade for proposed pavement widening
Remove existing culverts, safety end treatments (SETs)
Remove existing metal beam guard fence (MBGF), bridge rai
Install proposed pavement per plans
Install culverts, culvert extensions, SETs
Install mow strip, MBGF, bridge rail
Place flex base
Rework slopes, grade ditches
Blade windrowed material back across slopes
Revegetation of unpaved areas
Achieve site stabilization and remove sediment and
erosion control measures
Other:
Other:

# **1.10 POTENTIAL POLLUTANTS AND SOURCES:**

- X Sediment laden stormwater from stormwater conveyance over disturbed area
- X Fuels, oils, and lubricants from construction vehicles, equipment, and storage
- X Solvents, paints, adhesives, etc. from various construction activities
- X Transported soils from offsite vehicle tracking
- X Construction debris and waste from various construction activities
- X Contaminated water from excavation or dewatering pump-out water
- X Sanitary waste from onsite restroom facilities
- X Trash from various construction activities/receptacles
- □ Long-term stockpiles of material and waste
- X Discharges from concrete washout activities, runoff from concrete cutting activities, and other concrete related activities.

Other:

□ Other:	
Other:	

# **1.11 RECEIVING WATERS:**

Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for receiving waters.

	Tributaries	<b>Classified Waterbody</b>
	DRY BERRY CREEK	SAN GABRIEL RIVER (1214)
dı		
	* Add (*) for impaired waterbodies 1.12 ROLES AND RESPONSIE	
	X Development of plans and spectrum Submit Notice of Intent (NOI) to	
	Post Construction Site Notice	
	□ Submit NOI/CSN to local MS4	
	X Perform SWP3 inspections	
	X Maintain SWP3 records and up	
	<ul> <li>Complete and submit Notice of</li> <li>Maintain SWP3 records for 3 years</li> </ul>	
	□ Other:	
	Other:	
	□ Other:	

# **1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR**

X Day To Day Operational Control

- Submit Notice of Intent (NOI) to TCEQ (≥5 acres)
- Post Construction Site Notice
- Submit NOI/CSN to local MS4
- X Maintain schedule of major construction activities
- X Install, maintain and modify BMPs
- Complete and submit Notice of Termination to TCEQ
- □ Maintain SWP3 records for 3 vears
- ☐ Other: \_\_\_\_\_

Other:

Other:

# 1.14 LOCAL MUNICIPAL SEPARATE STORM SEWER SYSTEM (MS4) OPERATOR COORDINATION:

MS4 Entity

# **STORMWATER POLLUTION PREVENTION PLAN (SWP3)**



<sup>2023</sup> July 2023 Sheet 1 of 2

Texas Department of Transportation

FED. RD. DIV. NO.		PROJECT NO.						
6		SEE	TITLE	138				
STATE		STATE DIST.						
TEXAS	5	AUS	WILLIAMSON					
CONT.		SECT.	JOB		HIGHWAY I	٥٥.		
0914		05	203,	ETC.	CR 1	43		

# **STORMWATER POLLUTION PREVENTION PLAN (SWP3):**

# 2.0 BEST MANAGEMENT PRACTICES (BMPs) AND CONTROLS, INSPECTION, AND MAINTENANCE

The Contractor shall be the responsible party for implementing the BMPs described herein and for complying with the SWP3 for control of erosion and sedimentation during day-to-day operations. The Contractor shall implement changes to this SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.

# 2.1 EROSION CONTROL AND SOIL STABILIZATION BMPs:

# T/P

- □ □ Protection of Existing Vegetation
- □ □ Vegetated Buffer Zones
- □ □ Soil Retention Blankets
- Geotextiles
- □ □ Mulching/ Hydromulching
- □ □ Soil Surface Treatments
- X 🗆 Temporary Seeding
- □ X Permanent Planting, Sodding or Seeding
- □ □ Biodegradable Erosion Control Logs
- X 🛛 Rock Filter Dams/ Rock Check Dams
- Vertical Tracking
- Interceptor Swale
- 🗆 🗶 Riprap
- □ □ Diversion Dike
- Temporary Pipe Slope Drain
- □ □ Embankment for Erosion Control
- Paved Flumes
- □ □ Other:
- Other: \_\_\_\_\_\_
- □ □ Other:\_\_\_\_
- □ □ Other:

# 2.2 SEDIMENT CONTROL BMPs:

# T/P

- X 🗆 Biodegradable Erosion Control Logs
- Dewatering Controls
- □ □ Inlet Protection
- X 🛛 Rock Filter Dams/ Rock Check Dams
- □ □ Sandbag Berms
- X 🗆 Sediment Control Fence
- □ □ Stabilized Construction Exit
- 🗶 🗆 Floating Turbidity Barrier
- Vegetated Buffer Zones
- □ X Vegetated Filter Strips
- □ □ Other: \_\_\_\_
- □ □ Other:\_\_\_\_\_
- □ □ Other:\_\_\_\_\_
- □ □ Other: \_\_\_\_\_

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

Sediment control BMPs requiring design capacity calculations (See SWP3 Attachment 1.3.):

# T/P

- Sediment Trap
  - □ Calculated volume runoff from 2-year, 24-hour storm for each acre of disturbed area
  - □ 3,600 cubic feet of storage per acre drained
- Sedimentation Basin
  - X Not required (<10 acres disturbed)
  - □ Required (>10 acres) and implemented.
    - □ Calculated volume runoff from 2-year, 24-hour storm for each acre of disturbed area

Other:

- 3,600 cubic feet of storage per acre drained
- □ Required (>10 acres), but not feasible due to:
  - □ Available area/Site geometry
  - □ Site slope/Drainage patterns
  - □ Site soils/Geotechnical factors
  - Public safetv

# 2.3 PERMANENT CONTROLS:

(Coordinate post-construction BMPs with appropriate TxDOT maintenance sections.)

BMPs To Be Left In Place Post Construction:

Туро	Stationing				
Туре	From	То			
SEE ITEM 2.6 VEGETATED DETAILS FOR TCEQ EDWARI					

# 2.4 OFFSITE VEHICLE TRACKING CONTROLS:

- X Excess dirt/mud on road removed daily
- Haul roads dampened for dust control
- X Loaded haul trucks to be covered with tarpaulin
- X Stabilized construction exit Daily street sweeping
- Other:

Other:

Other:\_\_\_\_\_

# □ Other:

# 2.5 POLLUTION PREVENTION MEASURES:

- X Chemical Management
- X Concrete and Materials Waste Management
- X Debris and Trash Management
- X Dust Control
- X Sanitary Facilities
- Other:

□ Other:\_\_\_\_\_

Other:

# 2.6 VEGETATED BUFFER ZONES:

Natural vegetated buffers shall be maintained as feasible to protect adjacent surface waters. If vegetated natural buffer zones are not feasible due to site geometry, the appropriate additional sediment control measures have been incorporated into this SWP3.

Other:

	Turne	Stati	Stationing	
	Туре	From	То	
	VEGETATIVE FILTER STRIPS	109+54.10	116+70.00	
efer to the Environmental Layout Sheets/ SWP3 Layout Sheets cated in Attachment 1.2 of this SWP3				
	Refer to the Environmental Layout located in Attachment 1.2 of this S		_ayout She	

# 2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- X Fire hydrant flushings
- X Irrigation drainage
- X Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- X Potable water sources
- X Springs
- X Uncontaminated groundwater
- X Water used to wash vehicles or control dust
- X Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

# 2.8 DEWATERING:

Dewatering discharges of accumulated stormwater, groundwater, and surface water including discharges from dewatering of trenches, excavations, foundations, vaults, and other points of accumulation are prohibited unless managed by appropriate controls to prevent and minimize the offsite discharge of sediment and other pollutants.

# 2.9 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.5 of this SWP3.

When dewatering activities are present, a daily inspection will be conducted once per day during those activities and documented in accordance with CGP and TxDOT requirements.

**2.10 MAINTENANCE:** Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.5 of this SWP3.

# **STORMWATER POLLUTION PREVENTION PLAN (SWP3)**

<sup>2023</sup> July 2023 Sheet 2 of 2

Texas Department of Transportation

FED. RD. DIV. NO.			SHEET NO.			
6		SEE	TITLE	E SHE	ΕT	139
STATE		STATE DIST.				
TEXAS AUS			WILLIAMSON			
CONT.		SECT.	JOB		HIGHWAY I	٥٠.
0914		05	203,	ETC	CR 1	43

re		<sup>.</sup> Discharge Permit or Constr	uction Ceneral Permit			
I 1+	· · ·	or more acres disturbed so for erosion and sedimentati	il. Projects with any	archeological artifacts are fo	fications in the event historical issues or bund during construction. Upon discovery of 5. burnt rock, flint, pottery, etc.) cease	General (appli Comply with the Haz hazardous materials making workers awar
Li	ist MS4 Operator(s) that mo	by receive discharges from t		work in the immediate area and	d contact the Engineer immediately.	provided with perso
1.		d prior to construction acti	vities.	🛛 No Action Required	Required Action	Obtain and keep on- used on the project Paints, acids, solv
				Action No.		compounds or additi products which may
2.	No Action Required	Required Action		1.		Maintain an adequat
	Action No.			2.		In the event of a s in accordance with
1.		tion by controlling erosion mit TXR 150000	and sedimentation in	3.		immediately. The Co of all product spil
2.	Comply with the SW3P and required by the Engineer.	revise when necessary to co	ontrol pollution or	4.		Contact the Enginee * Dead or distr * Trash piles,
3.		otice (CSN) with SW3P inform	nation on or near	IV. VEGETATION RESOURCES		* Undesirable s * Evidence of I
		the public and TCEQ, EPA or			struction Specification Requirements Specs 162,	Does the project replacements (br
4.		specific locations (PSL's) i submit NOI to TCEQ and the			752 in order to comply with requirements for landscaping, and tree/brush removal commitments.	
	WORK IN OR NEAR STREA ACT SECTIONS 401 AND	MS, WATERBODIES AND WE 404	TLANDS CLEAN WATER	No Action Required	Required Action	If "Yes", then I Are the results
-	USACE Permit required for	filling, dredging, excavatio		Action No.		Yes
	The Contractor must adhere	ks, streams, wetlands or we to all of the terms and co		1. COMPLY WITH EXECUTIVE OF APPLICABLE	RDER 13112 ON INVASIVE SPECIES IF AND WHEN	If "Yes", then the notification
	the following permit(s):			2.		activities as ne 15 working days
	] No Permit Required			3.		If "No", then T scheduled demoli
Þ	Nationwide Permit 14 - F wetlands affected)	PCN not Required (less than	1/10th acre waters or	4.		In either case, activities and/o
	] Nationwide Permit 14 - F	PCN Required (1/10 to <1/2 c	acre, 1/3 in tidal waters)			asbestos consult
	] Individual 404 Permit Re ] Other Nationwide Permit	-			) THREATENED, ENDANGERED SPECIES, LISTED SPECIES, CANDIDATE SPECIES	Any other eviden on site. Hazard
R	equired Actions: List wate	rs of the US permit applies	to, location in project			No Action
	nd check Best Management P nd post-project TSS.	ractices planned to control	erosion, sedimentation	🛛 No Action Required	Required Action	Action No. 1.
1	. DRY BERRY CREEK - NWP #	14		Action No.		2.
2				1.		3.
3				2.		VII. OTHER ENVI
4				3.		(includes reg
+	o be performed in the wate	ry high water marks of any rs of the US requiring the		4.		No Action
- 70 –	ermit can be found on the Hest Management Practic				observed, cease work in the immediate area,	1. THE PROJECT IS LO NOTIFY THE LOCAL APPLICABLE RULES
E	rosion	Sedimentation	Post-Construction TSS	work may not remove active nests	and contact the Engineer immediately. The from bridges and other structures during	2. THE PROJECT IS LO ABATEMENT PLAN (1)
Σ	Temporary Vegetation	🛛 Silt Fence	🛛 Vegetative Filter Strips	are discovered, cease work in the	iated with the nests. If caves or sinkholes e immediate area, and contact the	THE TEXAS COMMIS
	]Blankets/Matting	Rock Berm	Retention/Irrigation Systems	Engineer immediately.		QUALITY (TCEQ) IS START OF CONSTRUC
	Mulch	Triangular Filter Dike	Extended Detention Basin			3. COMPLY WITH THE APPROVAL LETTER.
	Sodding	Sand Bag Berm	Constructed Wetlands	LIST OF	ABBREVIATIONS	4. MAINTAIN COPIES
	] Interceptor Swale	Straw Bale Dike	Wet Basin	BMP: Best Management Practice	SPCC: Spill Prevention Control and Countermeasure	APPROVAL LETTER ( AVAILABLE DURING
	] Diversion Dike	Brush Berms	Erosion Control Compost	CGP: Construction General Permit DSHS: Texas Department of State Health Serv		5. VOIDS ENCOUNTEREN
. L	]Erosion Control Compost ]Mulch Filter Berm and Socks	Erosion Control Compost     Mulch Filter Berm and Socks	Mulch Filter Berm and Socks	FHWA: Federal Highway Administration MOA: Memorandum of Agreement	PSL: Project Specific Location TCEQ: Texas Commission on Environmental Quality	CONSTRUCTION ARE BY THE TCEQ PER
1 -	I worder ringer berin and Socks			MOU: Memorandum of Understanding	TPDES: Texas Pollutant Discharge Elimination System	
	- Compost Filter Rerm and Scoke	Compost Filter Berm and Socks	Venetation Lined Ditches	MS4: Municipal Separate Stormwater Sewer S	vstem TPWD; Texas Parks and Wildlife Department	
	Compost Filter Berm and Socks	Compost Filter Berm and Socks	S	MS4: Municipal Separate Stormwater Sewer S MBTA: Migratory Bird Treaty Act NOT: Notice of Termination	ystem TPWD: Texas Porks and Wildlife Department TxDDT: Texas Department of Transportation T&E: Threatened and Endangered Species	REGARDING INSPEC

### ARDOUS MATERIALS OR CONTAMINATION ISSUES

ral (applies to all projects):

th the Hazard Communication Act (the Act) for personnel who will be working with materials by conducting safety meetings prior to beginning construction and rkers aware of potential hazards in the workplace. Ensure that all workers are with personal protective equipment appropriate for any hazardous materials used. d keep on-site Material Safety Data Sheets (MSDS) for all hazardous products he project, which may include, but are not limited to the following categories: cids, solvents, asphalt products, chemical additives, fuels and concrete curing or additives. Provide protected storage, off bare ground and covered, for which may be hazardous. Maintain product labelling as required by the Act.

an adequate supply of on-site spill response materials, as indicated in the MSDS. ent of a spill, take actions to mitigate the spill as indicated in the MSDS, ance with safe work practices, and contact the District Spill Coordinator ly. The Contractor shall be responsible for the proper containment and cleanup oduct spills.

he Engineer if any of the following are detected: d or distressed vegetation (not identified as normal) ish piles, drums, canister, barrels, etc. lesirable smells or odors dence of leaching or seepage of substances

the project involve any bridge class structure rehabilitation or

cements (bridge class structures not including box culverts)?

No No

o", then no further action is required. es", then TxDOT is responsible for completing asbestos assessment/inspection.

ne results of the asbestos inspection positive (is asbestos present)? ] Yes 🛛 🕅 No

es", then TxDOT must retain a DSHS licensed asbestos consultant to assist with otification, develop abatement/mitigation procedures, and perform management ities as necessary. The notification form to DSHS must be postmarked at least rking days prior to scheduled demolition.

", then TxDOT is still required to notify DSHS 15 working days prior to any uled demolition.

ther case, the Contractor is responsible for providing the date(s) for abatement ties and/or demolition with careful coordination between the Engineer and toos consultant in order to minimize construction delays and subsequent claims.

her evidence indicating possible hazardous materials or contamination discovered e. Hazardous Materials or Contamination Issues Specific to this Project:

No Action Required

Required Action

### HER ENVIRONMENTAL ISSUES

cludes regional issues such as Edwards Aquifer District, etc.)

No Action Required

Required Action

DJECT IS LOCATED IN A FEDERAL EMERGENCY MANAGEMENT AGENCY MAPPED FLOODPLAIN. THE LOCAL FLOODPLAIN ADMINISTRATOR AS NECESSARY AND COMPLY WITH ALL ABLE RULES AND REGULATIONS REGARDING HYDRAULIC DESIGN OF THE PROJECT. DJECT IS LOCATED ON THE EDWARDS AQUIFER RECHARGE ZONE. A WATER POLLUTION

ENT PLAN (WPAP) APPROVED BY XAS COMMISON ON ENVIRONMENTAL Y (TCEQ) IS REQUIRED BEFORE OF CONSTRUCTION. WITH THE WPAP AND WPAP

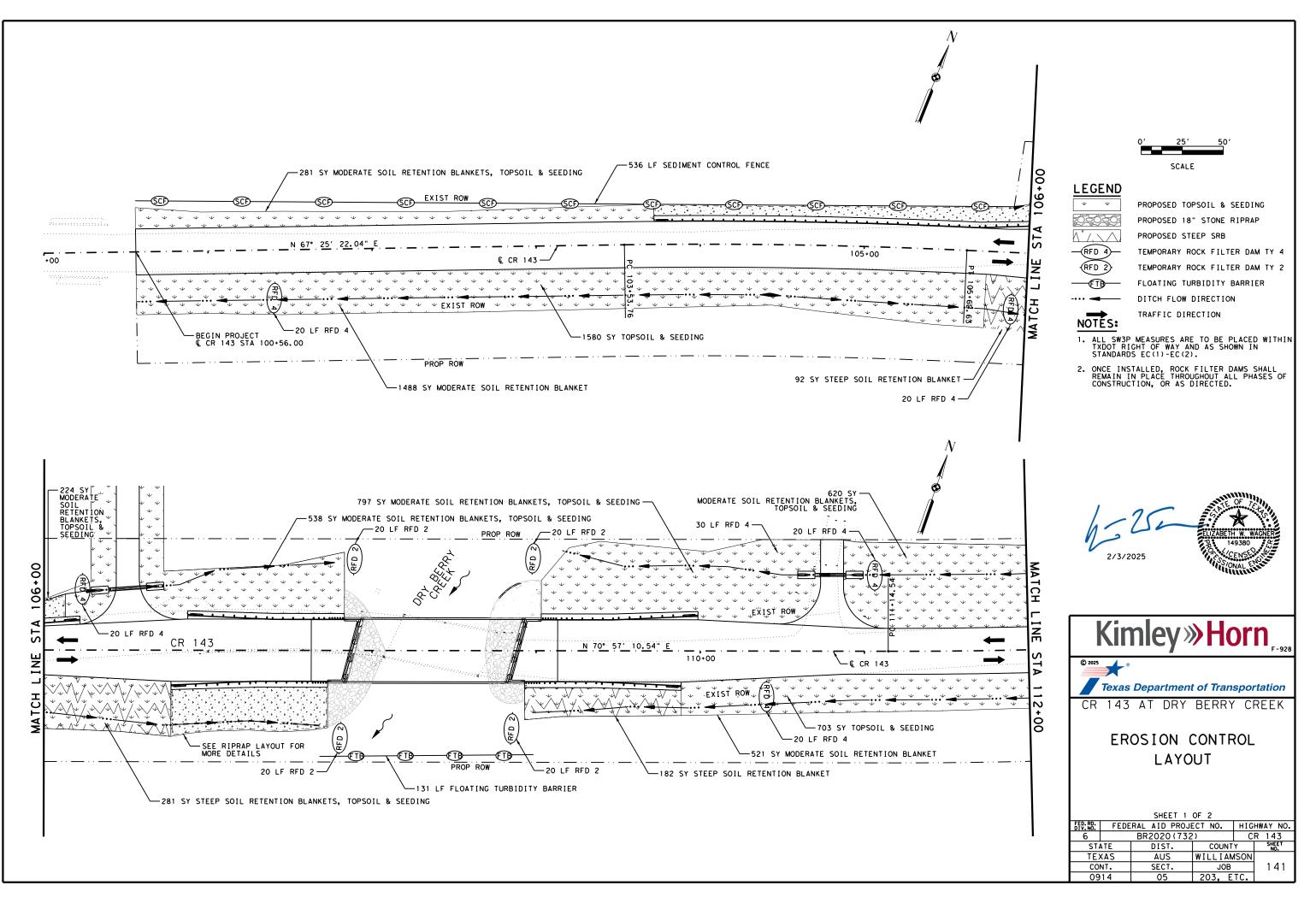
IN COPIES OF THE WPAP AND WPAP AL LETTER ONSITE OR IMMEDIATELY BLE DURING CONSTRUCTION. ENCOUNTERED DURING UCTION ARE SUBJECTED TO REVIEW TCEQ PER THE REQUIREMENTS OF WARDS AQUIFER RULES. DETAILS ING INSPECTION OF VOIDS AND NATION OF CLOSURE PLANS CEQ ARE INCLUDED IN THE ITIGATION DIAGRAMS (VMD-18).

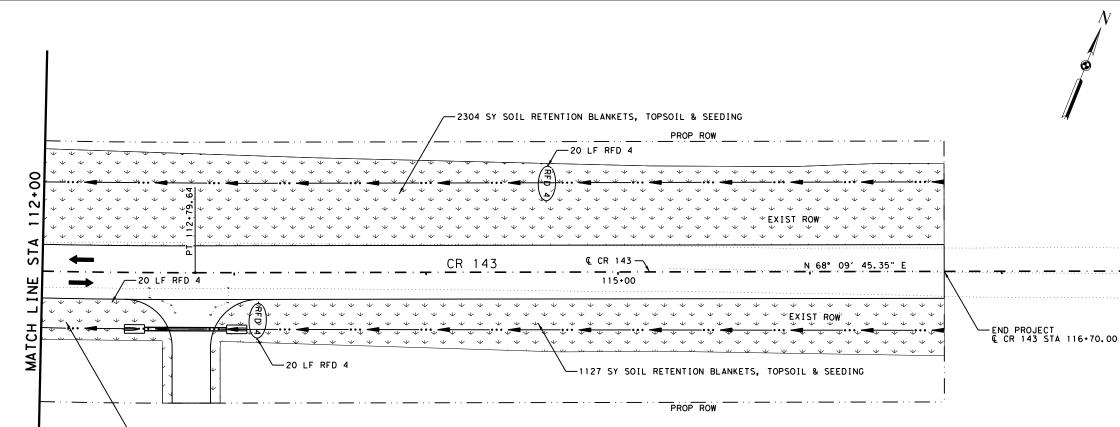


ENVIRONMENTAL PERMITS, ISSUES AND COMMITMENTS

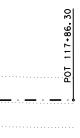
# EPIC

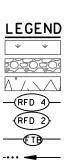
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Ĉ∣TxDOT: February 2015	CONT	SECT	JOB		н	GHWAY
REVISIONS 2-12-2011 (DS)	0914	05	203, E	TC.	CF	143
5-07-14 ADDED NOTE SECTION IV.	DIST		COUNT	Y		SHEET NO.
1-23-2015 SECTION I (CHANGED ITEM 1122 0 ITEM 506, ADDED GRASSY SWALES.	AUS	1	WILLIA	MSO	N	140











NOTES:

PROPOSED TOPSOIL & SEEDING PROPOSED 18" STONE RIPRAP PROPOSED STEEP SRB TEMPORARY ROCK FILTER DAM TY 4 TEMPORARY ROCK FILTER DAM TY 2 FLOATING TURBIDITY BARRIER DITCH FLOW DIRECTION

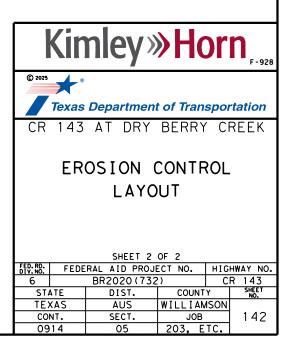
TRAFFIC DIRECTION

SCALE

- 1. ALL SW3P MEASURES ARE TO BE PLACED WITHIN TXDOT RIGHT OF WAY AND AS SHOWN IN STANDARDS EC(1)-EC(2).
- ONCE INSTALLED, ROCK FILTER DAMS SHALL REMAIN IN PLACE THROUGHOUT ALL PHASES OF CONSTRUCTION, OR AS DIRECTED.







# TEXAS COMMISSION ON ENVIRONMENTAL QUALITY WATER POLLUTION ABATEMENT PLAN GENERAL CONSTRUCTION NOTES:

- A written notice of construction must be submitted to the TCEQ regional office at least 48 hours prior to the start of any regulated activities. This notice must include:
   the name of the approved project;

  - the activity start date; and
    the contact information of the prime contractor.
- 2. All contractors conducting regulated activities associated with this project must be provided with complete copies of the approved Water Pollution Abatement Plan (WPAP) and the TCEQ letter indicating the specific conditions of its approval. During the course of these regulated activities, the contractors are required to keep on-site copies of the approved plan and approval letter
- If any sensitive feature(s) (caves, solution cavity, sink hole, etc.) is discovered during construction, all regulated activities near the sensitive feature must be suspended immediately. The appropriate TCEQ regional office must be immediately notified of any sensitive features encountered during construction. Construction activities may not be resumed until the TCEQ has reviewed and approved the appropriate protective measures in order to protect any sensitive feature and the Edwards Aquifer from potentially adverse impacts to water quality.
- No temporary or permanent hazardous substance storage tank shall be installed within 150 feet of a water supply source, distribution system, well, or sensitive feature.
- Prior to beginning any construction activity, all temporary erosion and sedimentation (E&S) control measures must be properly installed and maintained in accordance with the approved plans and manufacturers specifications. If inspections indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations. These controls must remain in place until the disturbed areas have been 5. permanently stabilized.
- Any sediment that escapes the construction site must be collected and properly disposed of before the next rain event to ensure it is not washed into surface streams, sensitive features, 6.
- Sediment must be removed from the sediment traps or sedimentation basins not later than TCEQ-0592 (Rev. July 15, 2015) Page 2 of 2 when it occupies 50% of the basin  $\eta_{\rm R}$  s design capacity. 7.
- Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from being discharged offsite.
- All spoils (excavated material) generated from the project site must be stored on-site with proper E&S controls. For storage or disposal of spoils at another site on the Edwards Aquifer Recharge Zone, the owner of the site must receive approval of a water pollution abatement plan for the placement of fill material or mass grading prior to the placement of spoils at the other site.
- 10. If portions of the site will have a temporary or permanent cease in construction activity lasting longer than 14 days, soil stabilization in those areas shall be initiated as soon as possible prior to the 14th day of inactivity. If activity will resume prior to the 21st day, stabilization measures are not required. If drought conditions or inclement weather prevent action by the 14th day, stabilization measures shall be initiated as soon as possible.
- The following records shall be maintained and made available to the TCEQ upon request:
   the dates when major grading activities occur;
   the dates when construction activities temporarily or permanently cease on a portion

  - of the site; and the dates when stabilization measures are initiated.
- 12. The holder of any approved Edward Aquifer protection plan must notify the appropriate regional office in writing and obtain approval from the executive director prior to initiating any the following:
  - A. any physical or operational modification of any water pollution abatement structure(s), including but not limited to ponds, dams, berms, sewage treatment plants, and diversionary structures;
  - B. any change in the nature or character of the regulated activity from that which was originally approved or a change which would significantly impact the ability of the plan to prevent pollution of the Edwards Aquifer;
  - C. any development of land previously identified as undeveloped in the original water pollution abatement plan.

Austin Regional Office	
12100 Park 35 Circle, Building Austin, Texas 78753-1808	A
Austin, Texas 78753-1808	
Phone (512) 339-2929 Fax (512) 339-3795	
Fax (512) 339-3795	

San Antonio Regional Office San Antonio Regional Ottice 14250 Judson Road San Antonio, Texas 78233-4480 Phone (210) 490-3096 Fax (210) 545-4329

## TCEQ WATER QUALITY CALCULATION:

Texas Commission on Environmental Quality TSS Removal Calculations

STEP ONE: Required TSS Removal

					EQU	ATION	3.3			
Lm	-	28.9	3 (An	x P)						
Lm	-	Requ	ired	TSS	Rer	noval	(poun	ds)		
An	=	Ne†	Incre	ase	in	Imper	vius /	Area	(acre	s)
P :	· /	lvera	ge Ar	nual	P	recipi	tatio	n (ir	ches)	

Total Project Area	4.53	Acres
Pre-Dev. Imp. Area	0.93	Acres
Post-Dev. Imp. Area	1.33	Acres
Imp. Cover Fraction	0.29	
P =	32	Inches
Lm =	348	Lbs

STEP TWO: Select an Appropriate BMP

Proposed BMP	VFS
Removal Efficiency	85%

STEP THREE: Calculate TSS Load Removed by BMPs

EQUATION 3.8					
Lr = (BMP Efficiency) x P x (Ai x 34.6 + Ap x 0.54)					
Lr = Load Removed by BMP					
BMP Efficiency = TSS Removal Efficiency					
Ai = Impervious Tributary Area to the BMP (ac)					
Ap = Pervious Tributary Area to the BMP (ac)					

Ai=	0.38		
Ap =	0.00		
Lr =	358 Ibs		

# TCEQ WATER QUALITY CALCULATION SUMMARY:

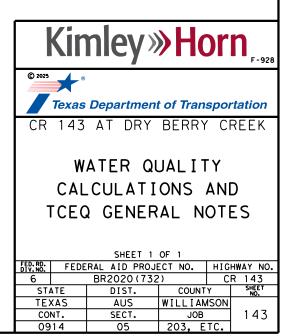
	TOTAL	PROJECT S	UMMARY		
SITE EXISTING AREA IMPERVIOUS AREA		PROPOSED IMPERVIO US AREA	REQUIRED ANNUAL TSS LOAD REMOVAL	PROVIDED ANNUAL TSS LOAD REMOVAL	
AC	AC	AC	LBS	LBS	
4.53	0.93	1.33	348	358	

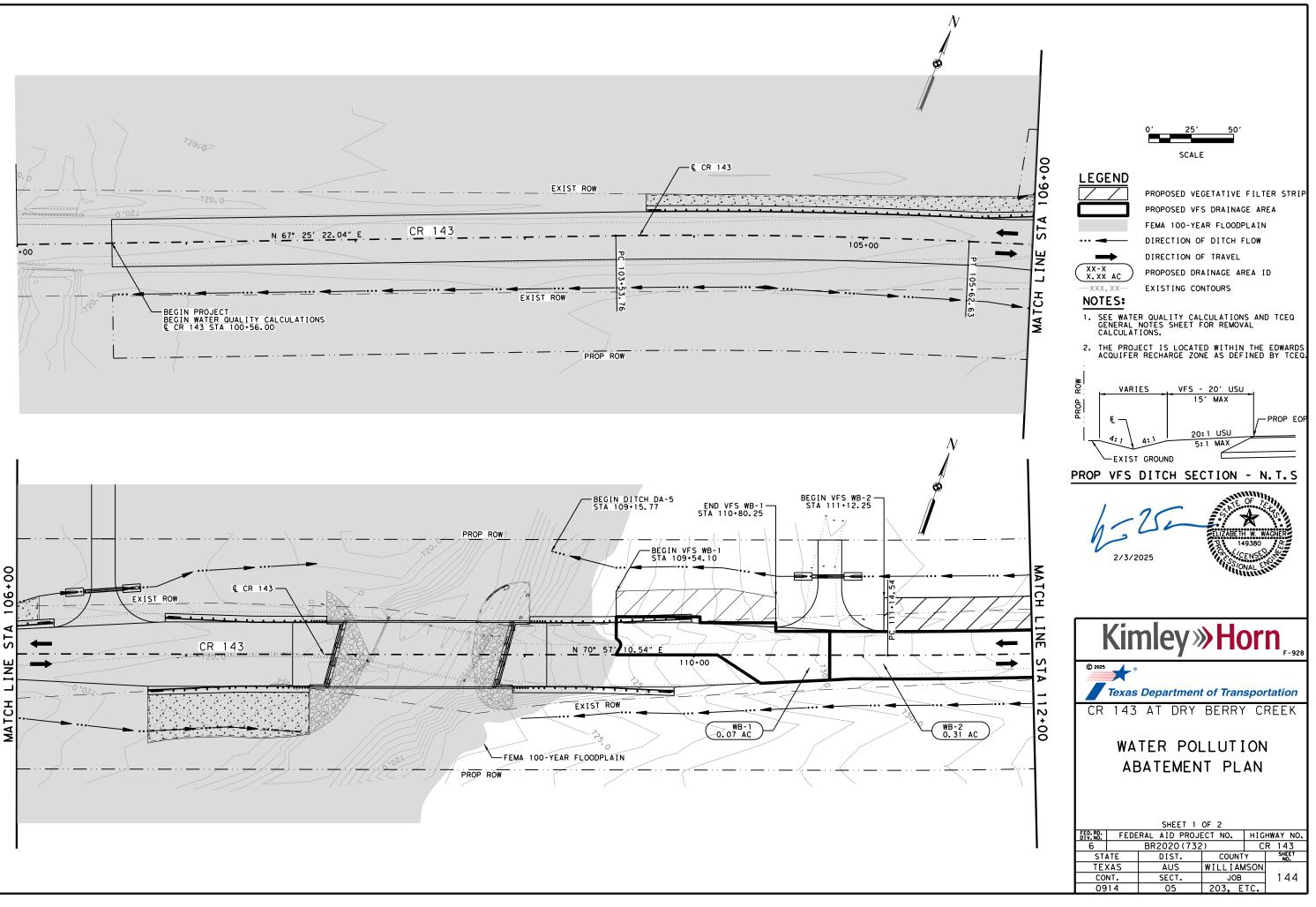
PR	PROVIDED LOAD REMOVAL / VEGETATIVE FILTER STRIP SUMMARY					
VFS ID	DRA I NAGE AREA	BEGIN STA	END STA	OFFSET	REMOVAL EFFICIENCY	PROVIDED TSS REMOVAL
	AC			RT/LT		LBS
WB-1	0.07	109+54.10	110+80.25	LT	85	66
WB-2	0.31	111+12.25	116+70.00	LT	85	292
TOTAL PROVIDED LOAD REMOVAL =					358	

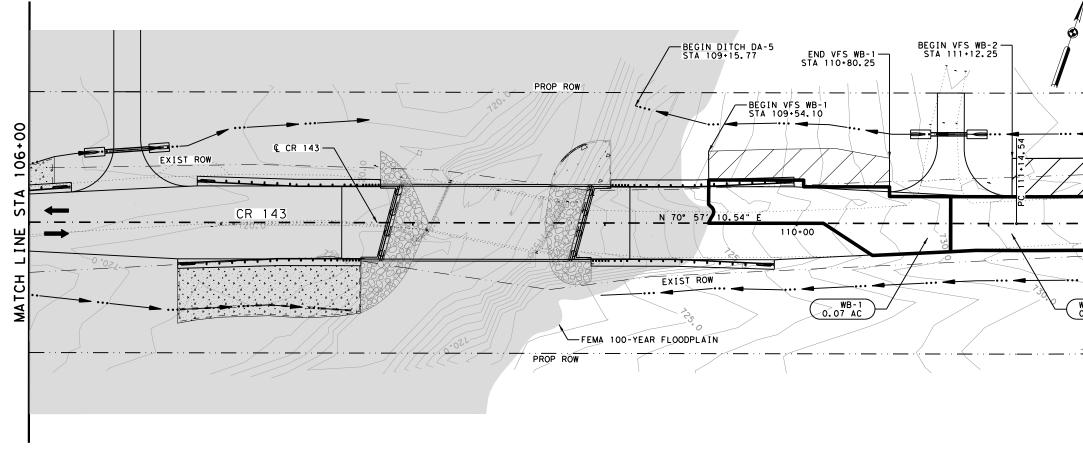
### NOTES:

- SEE CR 143 WATER QUALITY REPORT (KIMLEY-HORN) FOR DETAILED DISCUSSION ON WATER QUALITY CALCULATION METHODOLOGY.
- TOTAL REQUIRED LOAD REMOVALS ONLY COMPUTED WITHIN THE LIMITS OF THE ROADWAY WIDENING AS FOLLOWS: BEGIN STA 100+56.00 END STA 116+70.00



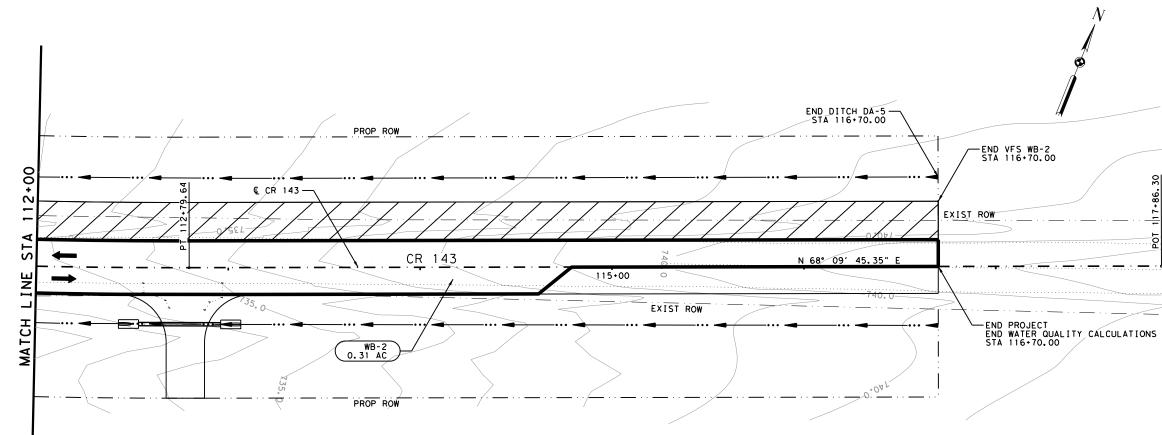






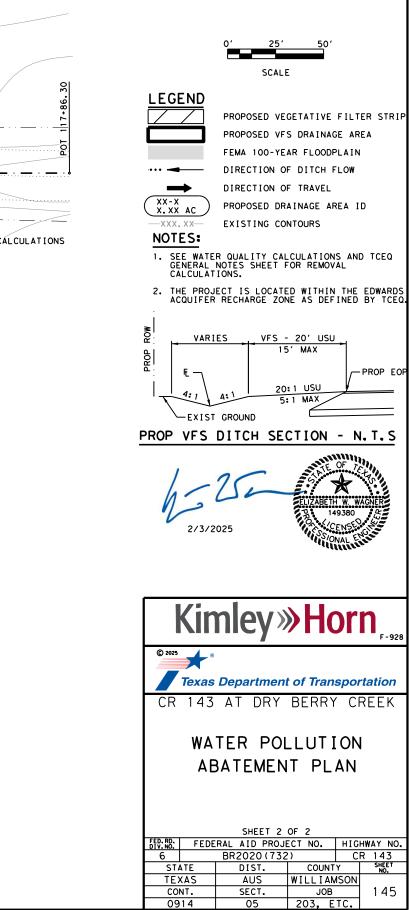
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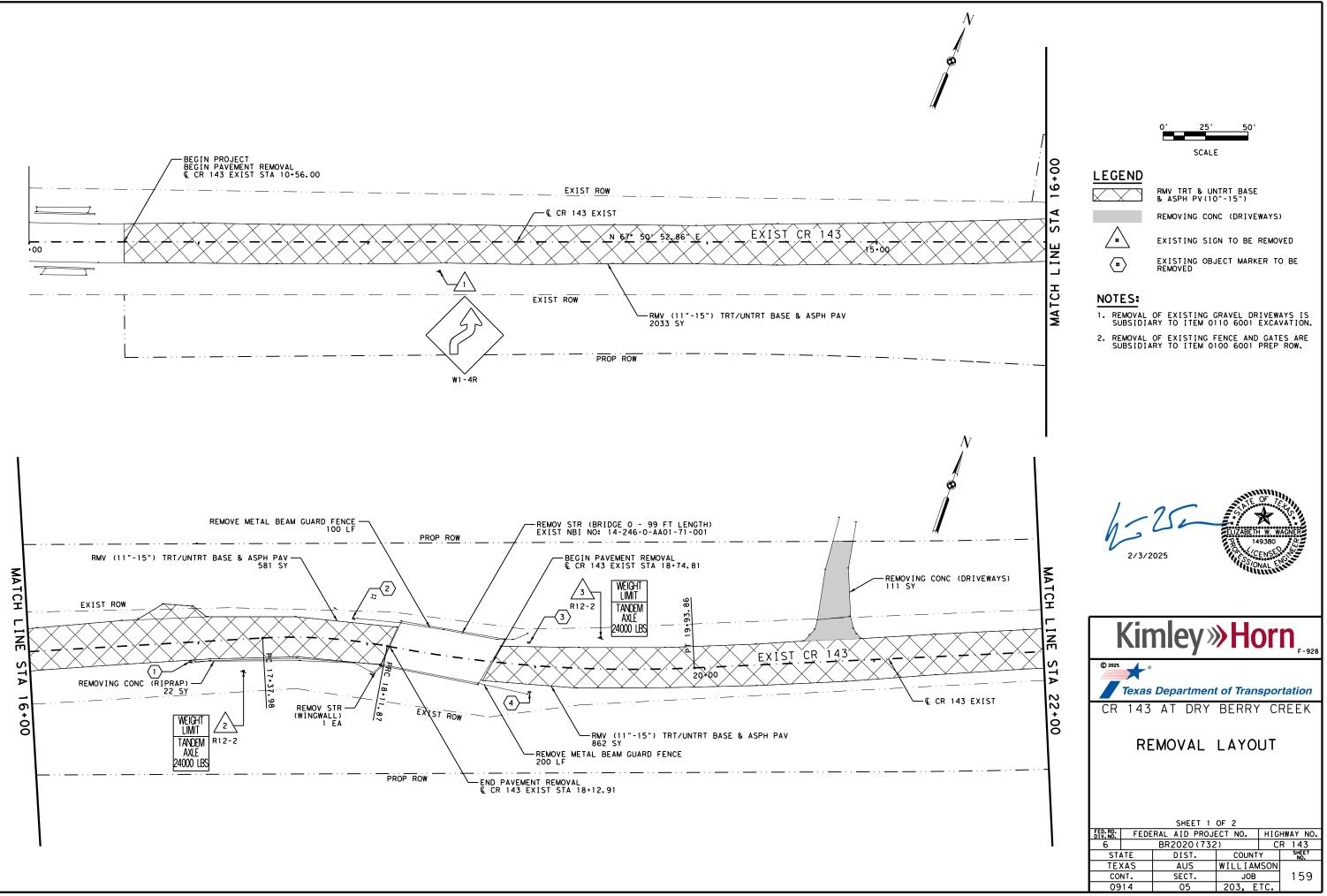
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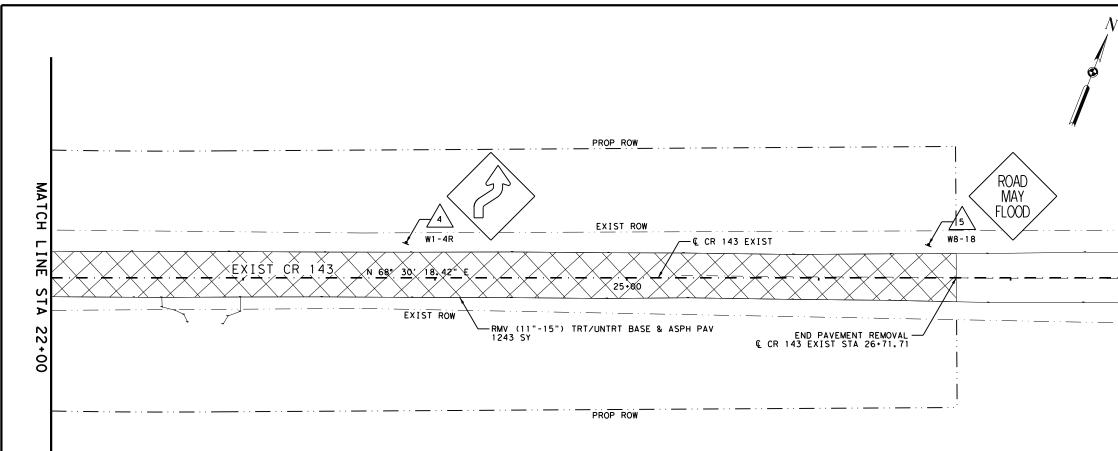




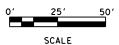




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 LEGEND
 RMV TRT & UNTRT BASE

 RMV TRT & UNTRT BASE
 & ASPH PV (10"-15")

 REMOVING CONC (DRIVEWAYS)
 EXISTING SIGN TO BE REMOVED

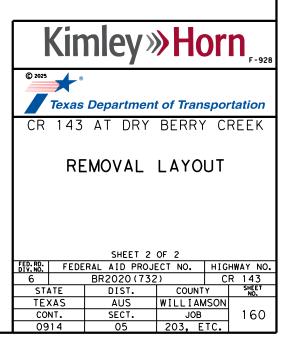
 \*
 EXISTING OBJECT MARKER TO BE

# NOTES:

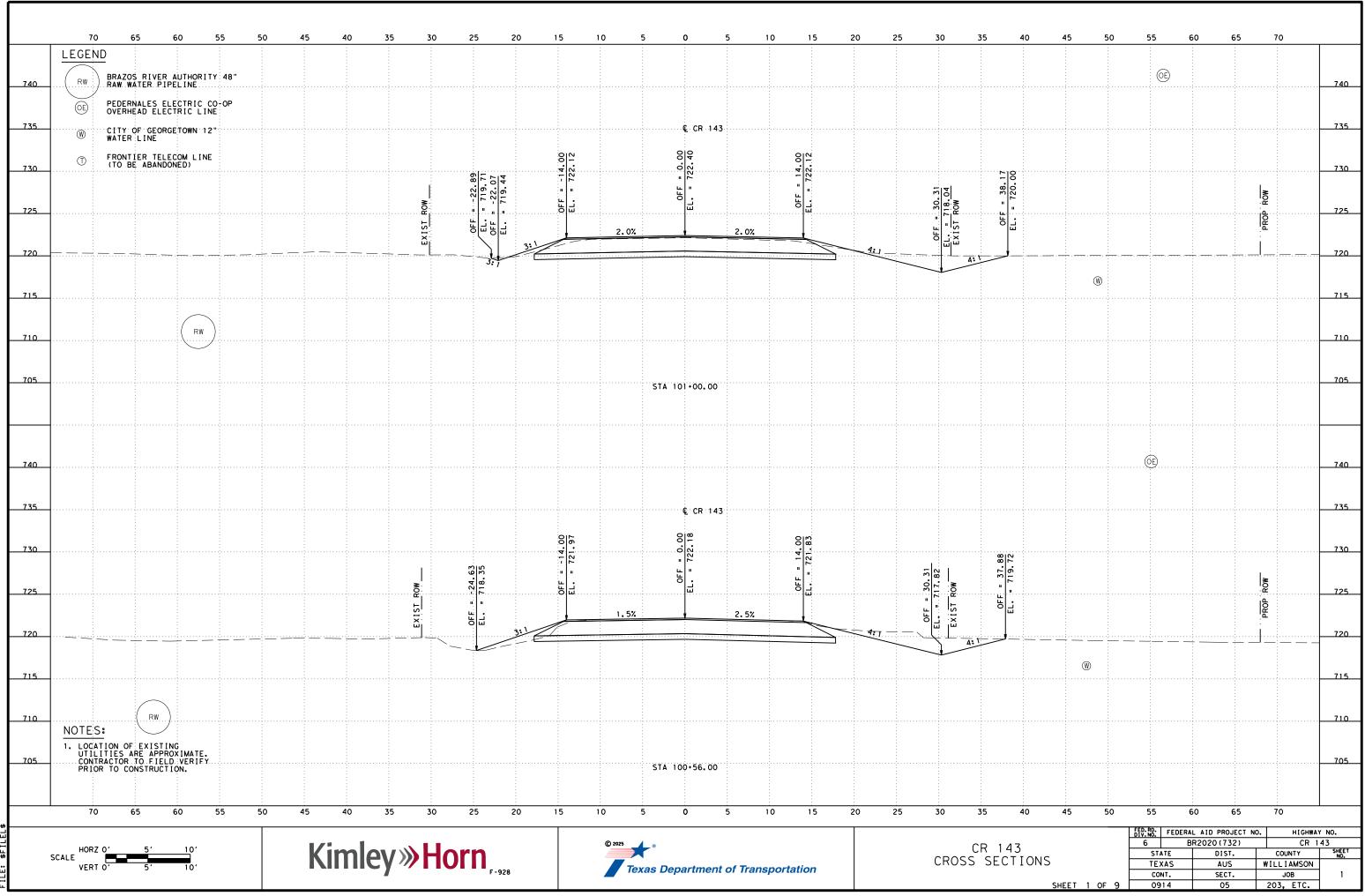
- 1. REMOVAL OF EXISTING GRAVEL DRIVEWAYS IS SUBSIDIARY TO ITEM 0110 6001 EXCAVATION.
- 2. REMOVAL OF EXISTING FENCE AND GATES ARE SUBSIDIARY TO ITEM 0100 6001 PREP ROW.

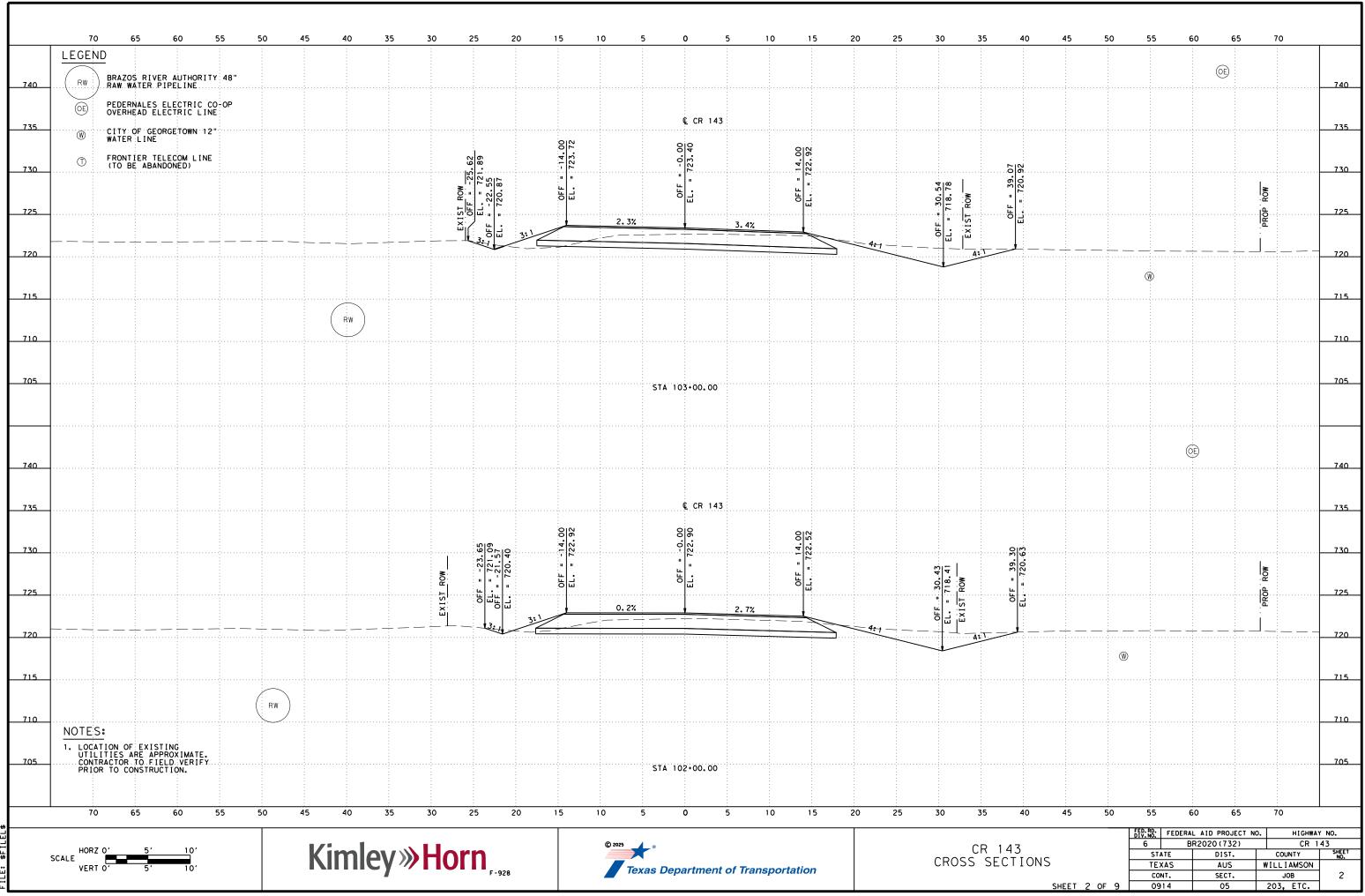


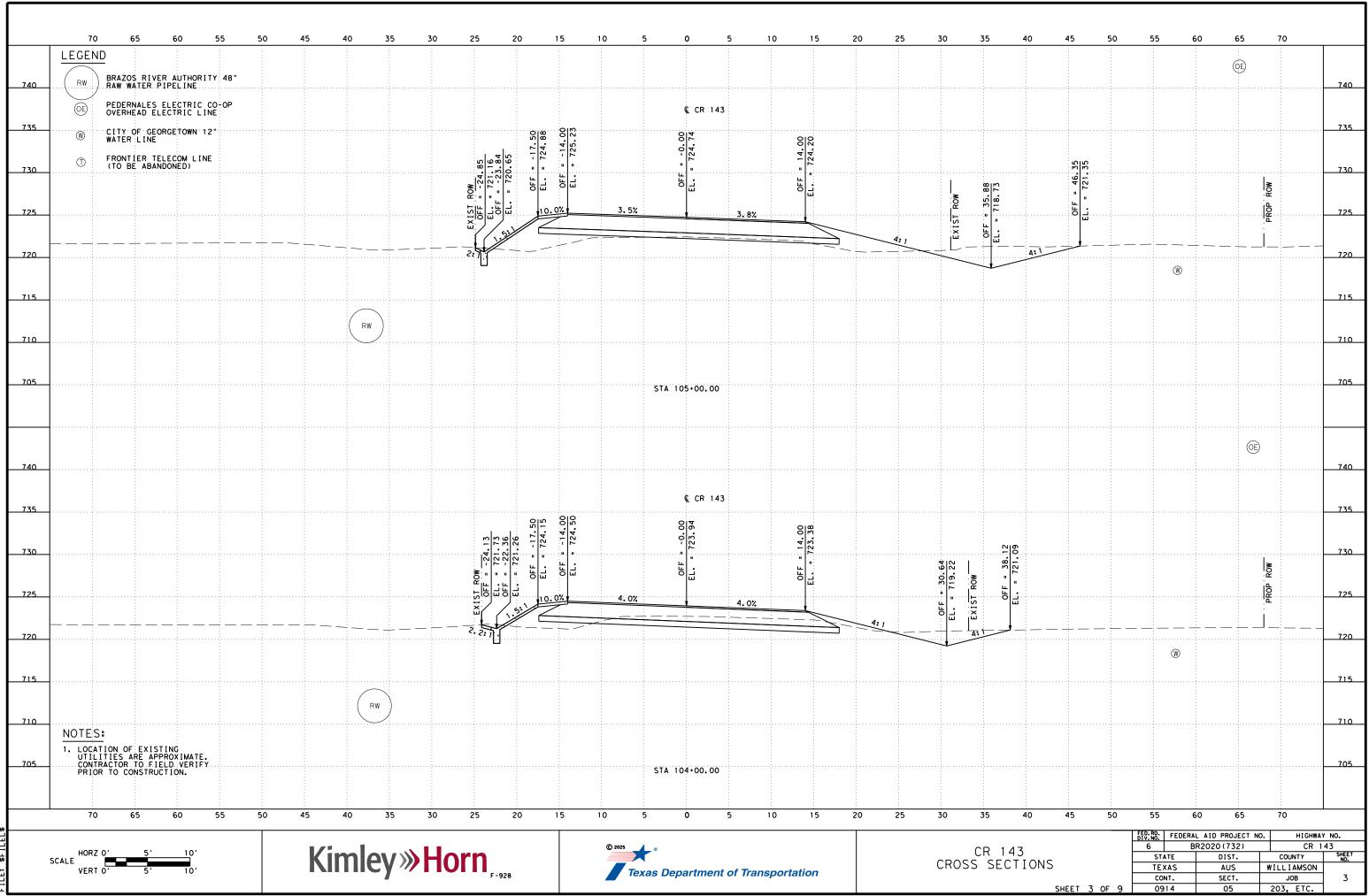




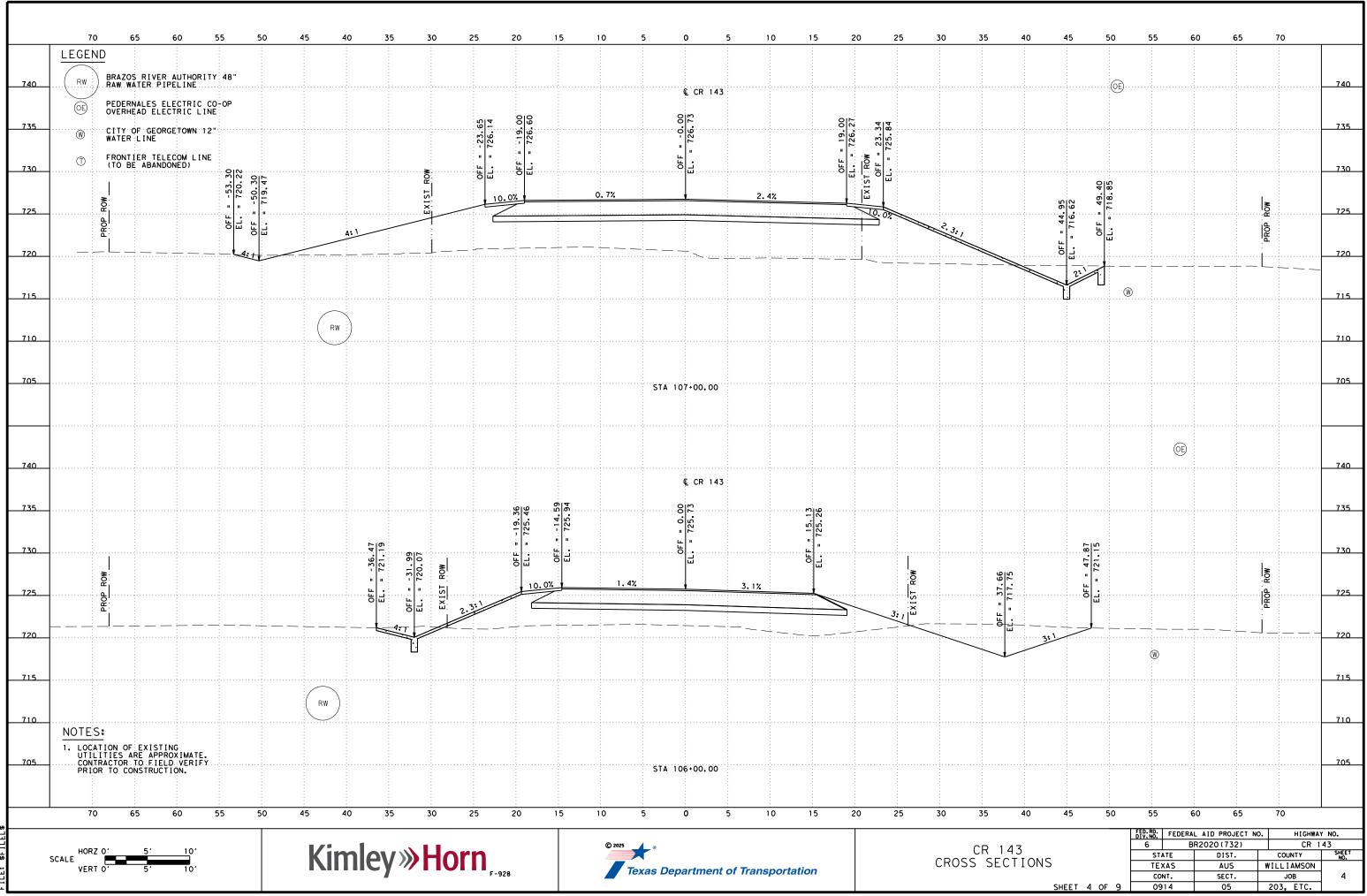
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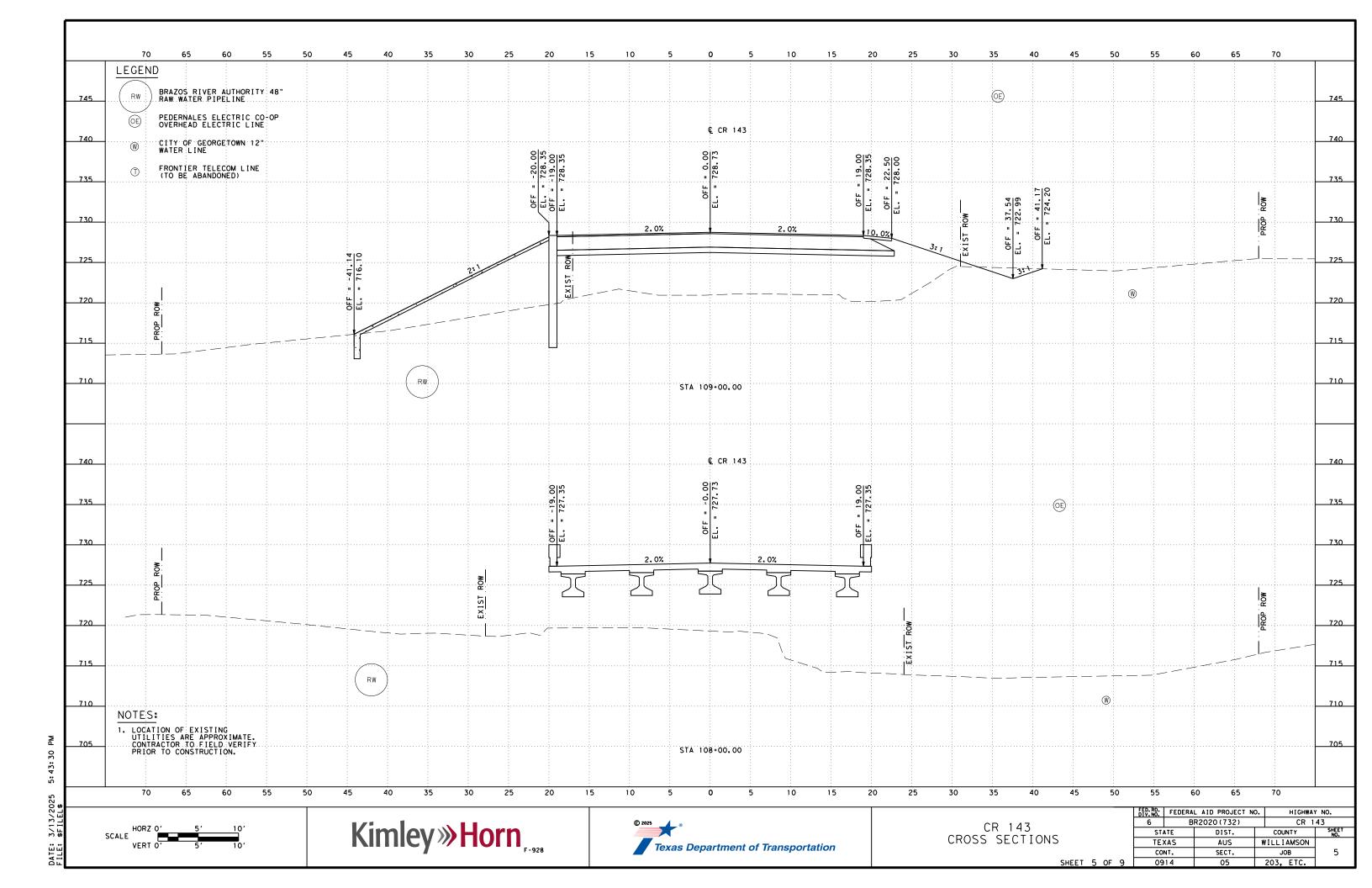


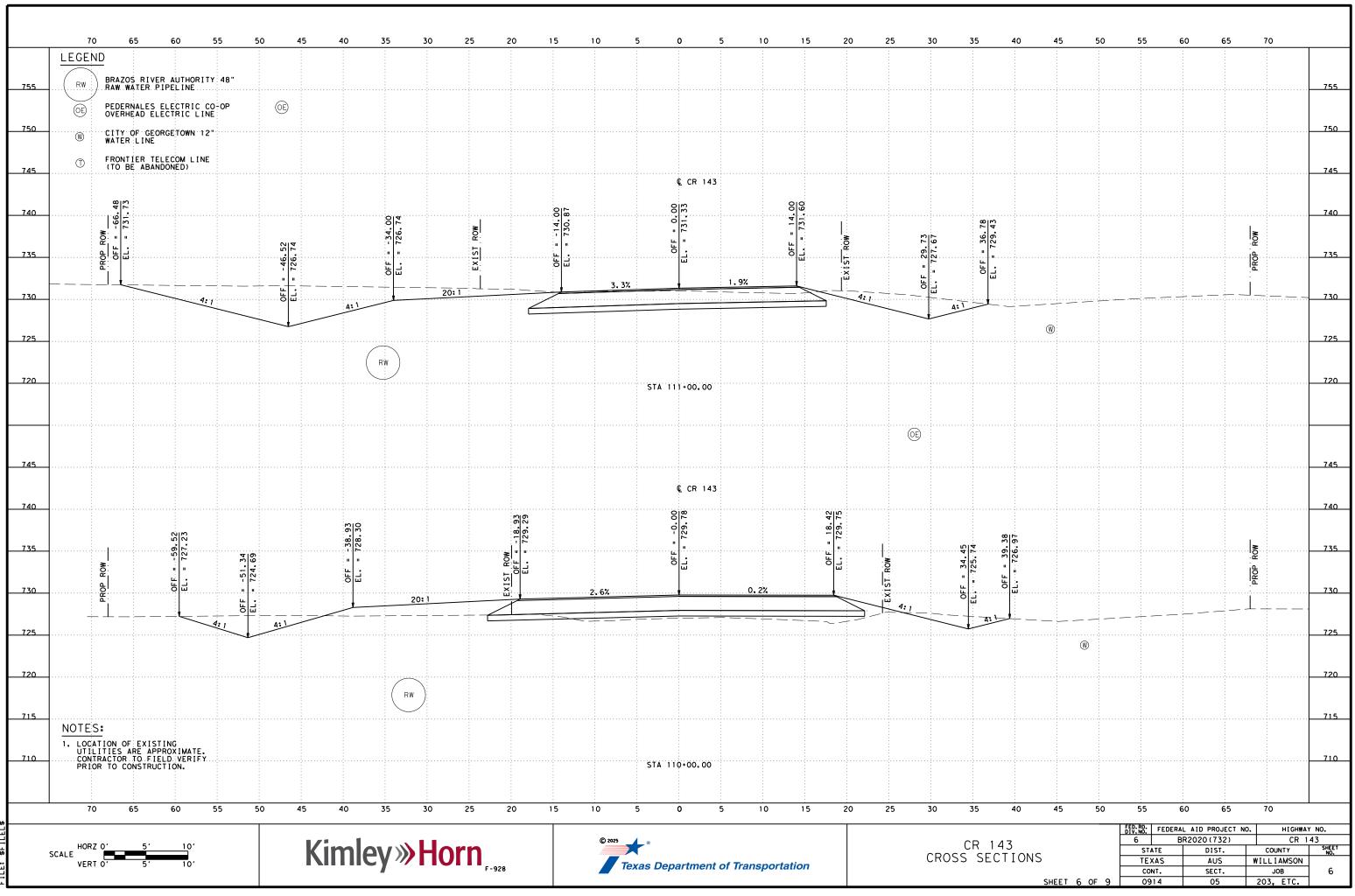


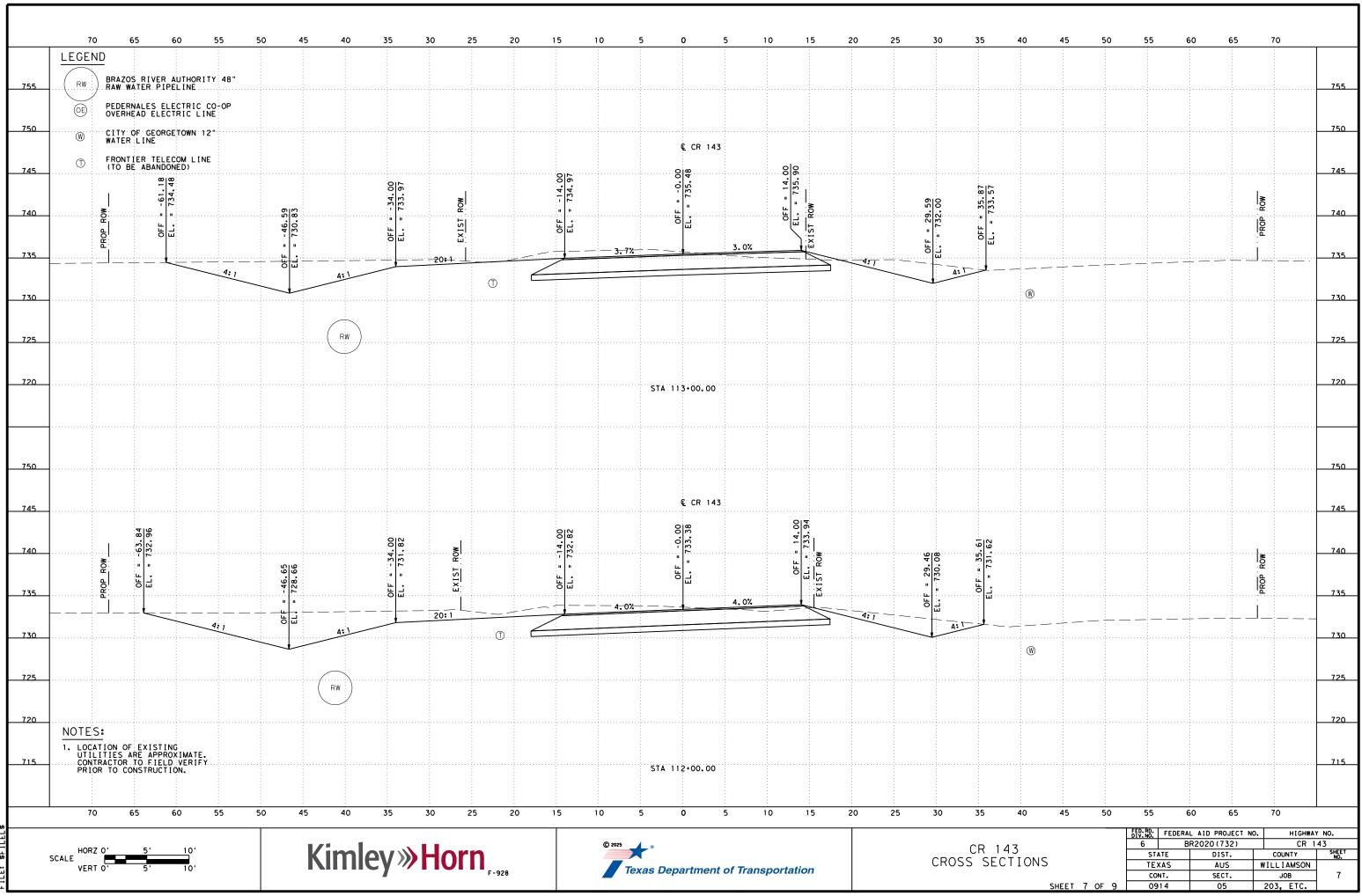
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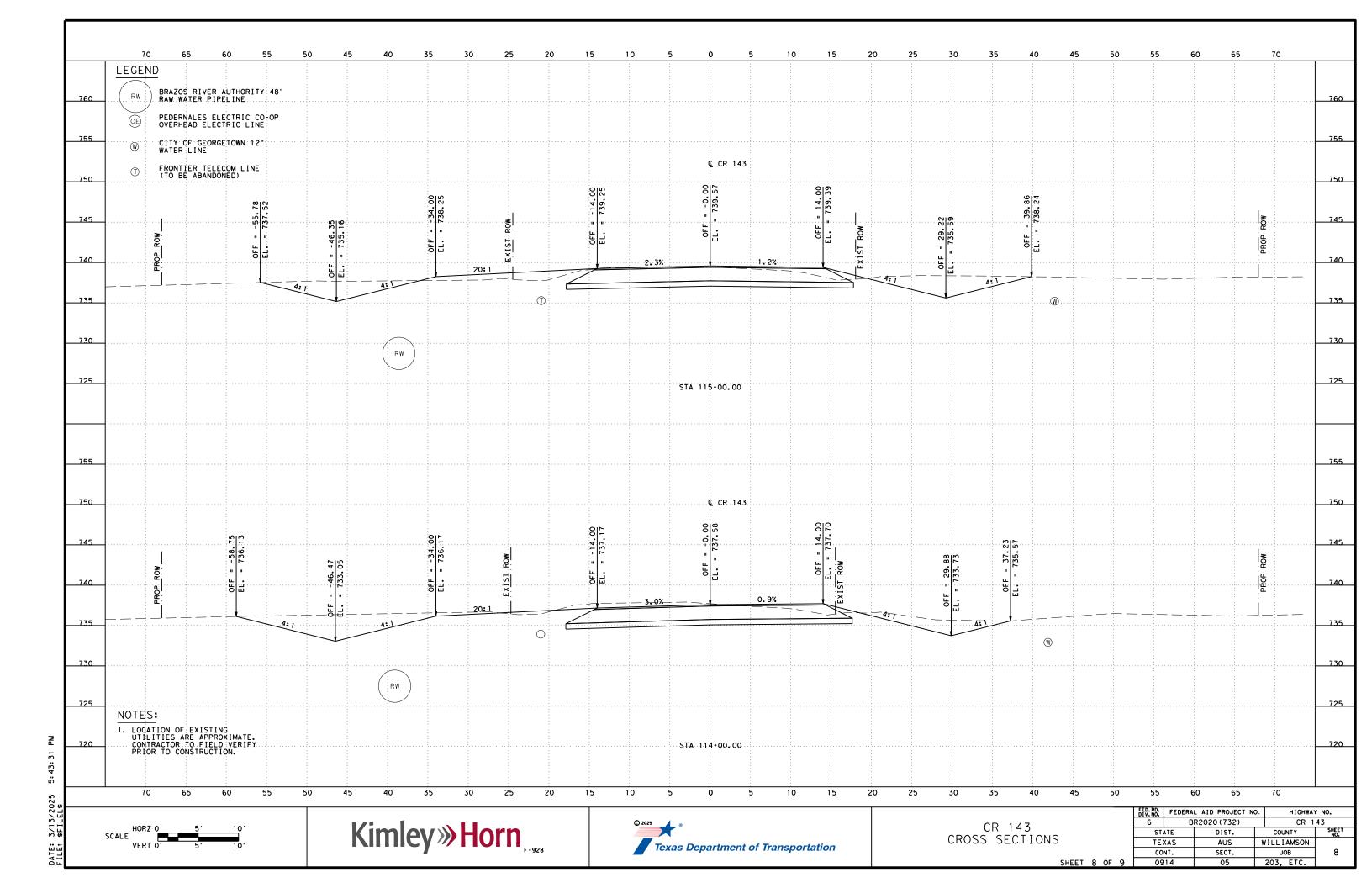
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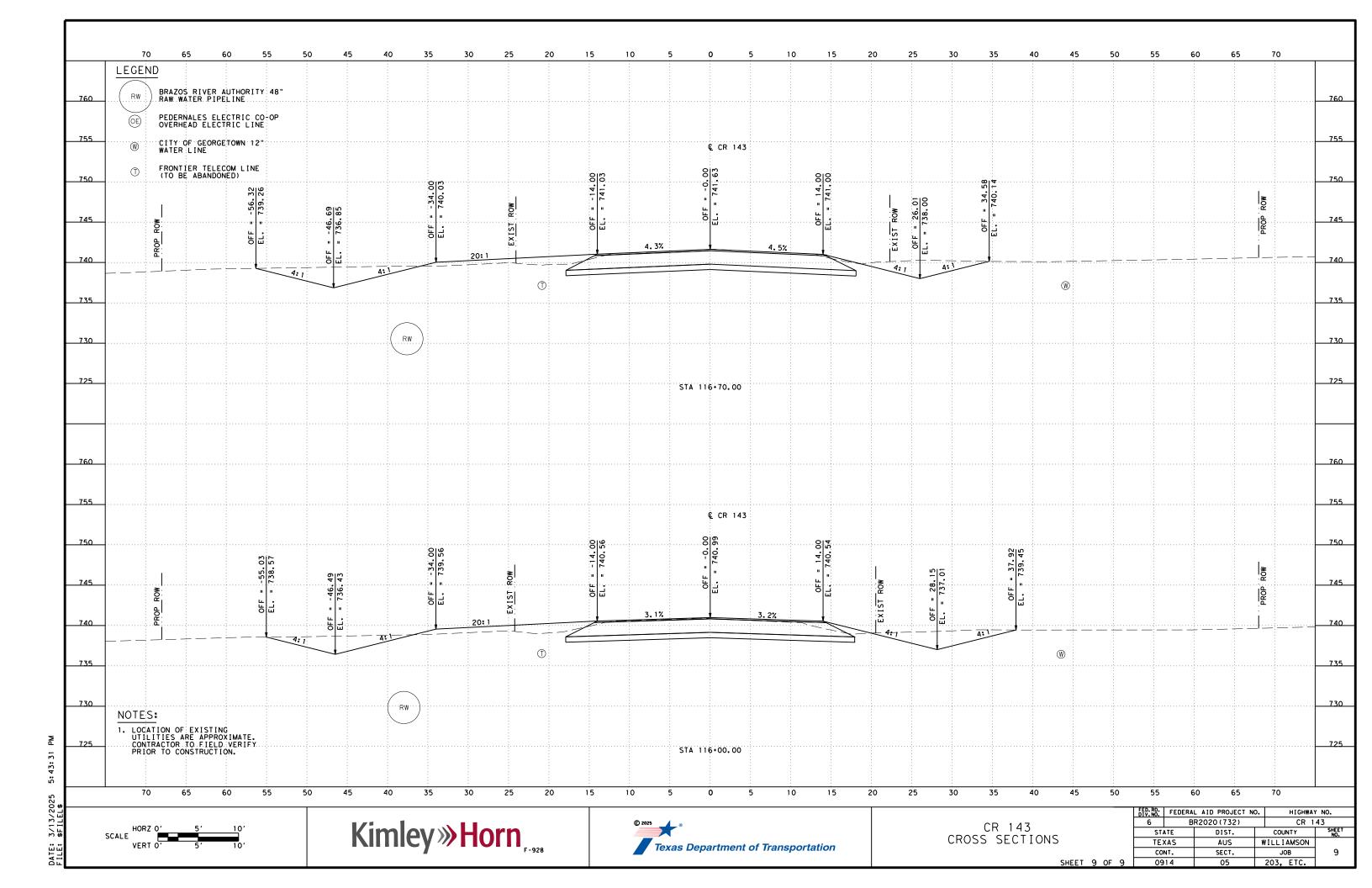






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## ATTACHMENT H

### INSPECTION, MAINTENANCE, REPAIR, AND RETROFIT PLAN

The following sections address inspection and maintenance taken from the TNRCC Manual "Complying with Edward Aquifer Rules: Technical Guidance on Best Management Practices."

### PEST MANAGEMENT

An Integrated Pest Management (IPM) Plan should be developed for vegetated areas. This plan should specify how problem insects and weeds will be controlled with minimal or no use of insecticides and herbicides.

### SEASONAL MOWING AND LAWN CARE

If the filter strip is made up of turf grass, it should be mowed as needed to limit vegetation height to 18 inches, using a mulching mower (or removal of clippings). If native grasses are used, the filter may require less frequent mowing, but a minimum of twice annually. Grass clippings and brush debris should not be deposited on vegetated filter strip areas. Regular mowing should also include weed control practices, however herbicide use should be kept to a minimum (Urbonas et al., 1992). Healthy grass can be maintained without using fertilizers because runoff usually contains sufficient nutrients. Irrigation of the site can help assure a dense and healthy vegetative cover.

### **INSPECTION**

Inspect filter strips at least twice annually for erosion or damage to vegetation; however, additional inspection after periods of heavy runoff is most desirable. The strip should be checked for uniformity of grass cover, debris and litter, and areas of sediment accumulation. More frequent inspections of the grass cover during the first few years after establishment will help to determine if any problems are developing, and to plan for long-term restorative maintenance needs. Bare spots and areas of erosion identified during semi-annual inspections must be replanted and 3-91 restored to meet specifications. Construction of a level spreader device may be necessary to reestablish shallow overland flow.

**Kimley Horn** 

#### **DEBRIS AND LITTER REMOVAL**

Trash tends to accumulate in vegetated areas, particularly along highways. Any filter strip structures (i.e. level spreaders) should be kept free of obstructions to reduce floatables being flushed downstream, and for aesthetic reasons. The need for this practice is determined through periodic inspection, but should be performed no less than 4 times per year.

#### SEDIMENT REMOVAL

Sediment removal is not normally required in filter strips, since the vegetation normally grows through it and binds it to the soil. However, sediment may accumulate along the upstream boundary of the strip preventing uniform overland flow. Excess sediment should be removed by hand or with flat-bottomed shovels.

### **GRASS RESEEDING AND MULCHING**

A healthy dense grass should be maintained on the filter strip. If areas are eroded, they should be filled, compacted, and reseeded so that the final grade is level. Grass damaged during the sediment removal process should be promptly replaced using the same seed mix used during filter strip establishment. If possible, flow should be diverted from the damaged areas until the grass is firmly established. Bare spots and areas of erosion identified during semi-annual inspections must be replanted and restored to meet specifications. Corrective maintenance, such as weeding or replanting should be done more frequently in the first two to three years after installation to ensure stabilization. Dense vegetation may require irrigation immediately after planting, and during particularly dry periods, particularly as the vegetation is initially established.

The TxDOT Area Engineer may be contacted for questions that pertain to the maintenance of this roadway. He has approved these guidelines and signed below.

Responsible Party for Maintenance: Doughs C. Havins
Address: 2727 South Austin Avenue
City, State, Zip: Georgetown TX. 78626
Signature of Responsible Party:

TCEQ – 20872 EAPP Roadway Application County Road (CR) 143 at Dry Berry Creek Bridge Replacement **Kimley**»Horn

# ATTACHMENT J

### MEASURES FOR MINIMIZING SURFACE STREAM CONTAMINATION

Temporary BMPs, such as rock filter dams and a floating turbidity barrier, will be installed to prevent onsite sediment and debris from entering surface streams during construction. Temporary BMPs will be placed before commencing any construction activities and will remain in place during construction.

A floating turbidity barrier will be installed at the downstream end of the project as a perimeter control and Type 2 and Type 4 rock filter dams will be placed at ditch outfall locations to filter onsite stormwater runoff before release to receiving waters. After construction, the project limits will be revegetated to prevent pollution of surface streams. Temporary BMPs will only be removed once sufficient vegetation is established.

In the event of a hazardous or hydrocarbon spill, spill response contingencies are in place for the project as included in *Attachment A – Spill Response Actions* of the Temporary Stormwater Section. This includes standard procedures for spill prevention, control, clean up, and reporting.



# ATTACHMENT K

### VOLUME AND CHARACTER OF STORMWATER

The proposed BMPs were designed and sized to treat the proposed onsite and offsite flows. The proposed improvements create a total of 1.33 acres of impervious cover, making up 29% of the overall site that drains into the proposed BMPs.

The character of the stormwater is not expected to change significantly from pre-project to postproject conditions. Vegetative filter strips will treat 80% of TSS generated by the project. All disturbed areas will be re-vegetated at the completion of the project; therefore, no significant degradation of stormwater quality is anticipated due to construction activities. For project drainage patterns see Drainage Area Map in *Attachment G – Construction Plans*.



## **SECTION 3**

# GEOLOGICAL ASSESSMENT FORM (TCEQ-0585)





## **Geologic Assessment**

# County Road (CR) 143 at Dry Berry Creek Bridge Replacement

CSJ Number: 0914-05-203

March 2024

Prepared for Texas Department of Transportation

The environmental review, consultation, and other actions required by applicable Federal environmental laws for this project are being, or have been, carried-out by TxDOT pursuant to 23 U.S.C. 327 and a Memorandum of Understanding dated 12-9-2019, and executed by FHWA and TxDOT.

### **Geologist Certification**

County Road (CR) 143 at Dry Berry Creek Bridge Replacement

Prepared for: Texas Department of Transportation Prepared by: Stantec Consulting, Inc. Date: 29 March 2024

In accordance with the Texas Board of Professional Geologists rules at 22 Texas Administrative Code, Part 39, Chapter 851, Subchapter C, §851.156, this report is signed and sealed on the title page to assure the user that the work has been performed by or directly supervised by the following professional geoscientists who take full responsibility for this work.

The computer-generated seals appearing on this document were authorized by Brian Cowan, P.G. 11180.



Brigh Coway

Brian D. Cowan, Texas Professional Geoscientist No. 11180 Stantec Consulting Services, Inc., Geoscience Firm No. 50120

## **Geologic Assessment**

**Texas Commission on Environmental Quality** 

For Regulated Activities on The Edwards Aquifer Recharge/transition Zones and Relating to 30 TAC §213.5(b)(3), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

## Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

Print Name of Geologist: <u>Brain Davis Cowan,</u> <u>P.G.</u> Telephone: 512-632-8409

Fax: \_\_\_\_\_

Date: 03/29/2024

Representing: <u>Stantec Consulting Services</u>, Inc. (#50120) (Name of Company and TBPG or TBPE registration number)

Signature of Geologist:

Brin Cours

**Regulated Entity Name:** <u>County Road (CR) 143 at Dry Berry Creek Bridge Replacement; CSJ</u> 0914-05-203

## **Project Information**

- 1. Date(s) Geologic Assessment was performed: 12/12/2023
- 2. Type of Project:

🔀 WPAP	AST
SCS	UST

3. Location of Project:

$\times$	Recharge Zone
	Transition Zone

Contributing Zone within the Transition Zone

- 4. Attachment A Geologic Assessment Table. Completed Geologic Assessment Table (Form TCEQ-0585-Table) is attached.
- 5. Soil cover on the project site is summarized in the table below and uses the SCS Hydrologic Soil Groups\* (Urban Hydrology for Small Watersheds, Technical Release No. 55, Appendix A, Soil Conservation Service, 1986). If there is more than one soil type on the project site, show each soil type on the site Geologic Map or a separate soils map.

Soil Name	Group*	Thickness(feet)
DnA	С	6.66
KrB	С	>6.66
OkA	В	>6.66

## Table 1 - Soil Units, InfiltrationCharacteristics and Thickness

- \* Soil Group Definitions (Abbreviated)
  - A. Soils having a high infiltration rate when thoroughly wetted.
  - B. Soils having a moderate infiltration rate when thoroughly wetted.
  - C. Soils having a slow infiltration rate when thoroughly wetted.
  - D. Soils having a very slow infiltration rate when thoroughly wetted.
- 6. Attachment B Stratigraphic Column. A stratigraphic column showing formations, members, and thicknesses is attached. The outcropping unit, if present, should be at the top of the stratigraphic column. Otherwise, the uppermost unit should be at the top of the stratigraphic column.
- 7. Attachment C Site Geology. A narrative description of the site specific geology including any features identified in the Geologic Assessment Table, a discussion of the potential for fluid movement to the Edwards Aquifer, stratigraphy, structure(s), and karst characteristics is attached.
- 8. Attachment D Site Geologic Map(s). The Site Geologic Map must be the same scale as the applicant's Site Plan. The minimum scale is 1": 400'

Applicant's Site Plan Scale: 1'' = 50'Site Geologic Map Scale: 1'' = 50'Site Soils Map Scale (if more than 1 soil type): 1'' = 400'

- 9. Method of collecting positional data:
  - Global Positioning System (GPS) technology.
  - Other method(s). Please describe method of data collection: <u>Via ESRI ArcGIS software</u> <u>for mapped features</u>
- 10. The project site and boundaries are clearly shown and labeled on the Site Geologic Map.

TCEQ-0585 (Rev.02-11-15)

- 11. Surface geologic units are shown and labeled on the Site Geologic Map.
- 12. Geologic or manmade features were discovered on the project site during the field investigation. They are shown and labeled on the Site Geologic Map and are described in the attached Geologic Assessment Table.

Geologic or manmade features were not discovered on the project site during the field investigation.

- 13. The Recharge Zone boundary is shown and labeled, if appropriate.
- 14. All known wells (test holes, water, oil, unplugged, capped and/or abandoned, etc.): If applicable, the information must agree with Item No. 20 of the WPAP Application Section.

There are  $\underline{1}$  (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply.)

The wells are not in use and have been properly abandoned.

] The wells are not in use and will be properly abandoned.

The wells are in use and comply with 16 TAC Chapter 76.

There are no wells or test holes of any kind known to exist on the project site.

## Administrative Information

15. Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

### **Attachment A: Geologic Assessment Table**

GEOLOGIC ASSESSMENT TABLE PROJECT NAME: Co							County Road (CR) 143 at Dry Berry Creek Bridge Replacement													
	LOCATION			FEATURE CHARACTERISTICS						EVALUATION			PHYSICAL SETTING							
1A	1B *	1C*	2A	2B	3		4		5	5A	6	7	8A	8B	9	1	10	1	1	12
FEATURE ID	LATITUDE	LONGITUDE	FEATURE TYPE	POINTS	FORMATION	DIMENSIONS (FEET)		EET)	TREND (DEGRE ES)	DOM	DENSITY (NO/FT)	APERTURE (FEET)	INFILL	RELATIVE INFILTRATION RATE	TOTAL	SENSITIVITY		SITIVITY CATCHMENT AREA (ACRES)		TOPOG RAPHY
						Х	Y	Z		10						<40	<u>≥40</u>	<1.6	<u>≥1.6</u>	
CR143-01	30.730683	-97.656998	MB	30	Qal	2	2	UNK	NA	NA	NA	NA	NA	5	35	x			x	Hillside
* DATUM: WG	* DATUM: WGS 1984																			
2A TYPE		TYPE		2A TYPE TYPE 2B POINTS 8A INFILLING																

2A TYPE	TYPE	2B POINTS			
С	Cave	30		Ν	None, e
SC	Solution cavity	20		С	Coarse
SF	Solution-enlarged fracture(s)	20		0	Loose o
F	Fault	20		F	Fines, c
0	Other natural bedrock features	5		V	Vegetat
MB	Manmade feature in bedrock	30		FS	Flowsto
SW	Swallow hole	30		Х	Other m
SH	Sinkhole	20	-		
CD	Non-karst closed depression	5			
Z	Zone, clustered or aligned features	30		Cliff	, Hilltop, H

exposed bedrock

- cobbles, breakdown, sand, gravel

or soft mud or soil, organics, leaves, sticks, dark colors

compacted clay-rich sediment, soil profile, gray or red colors

tion. Give details in narrative description

one, cements, cave deposits

naterials

12 TOPOGRAPHY Hillside, Drainage, Floodplain, Streambed



I have read, I understood, and I have followed the Texas Commission on Environmental Quality's Instructions to Geologists. The information presented here complies with that document and is a true representation of the conditions observed in the field. My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

Date:

Brin Cours

Sheet <u>1</u> of <u>1</u>

TCEQ-0585-Table (Rev. 10-01-04)

## Attachment B: Stratigraphic Column

Series	Group		Stratigraphic Unit	Outcrops in project area	
	Eagle Ford				
			Buda Limestone		
	Washita		Del Rio Clay		
			Georgetown Formation		
			Edwards Limestone		
	Fredericksburg		Comanche Peak Limestone		
Iche			Walnut Formation		
Comanche			Paluxy Formation		
Col		Glen	Upper Member		
	Trinity		Rose	Lower Member	
			Hensel Sand Member		
		Travis Peak	Cow Creek Limestone Member		
		vis F	Hammett Shale Member		
		Trav	Sligo Member		
			Hosston Member		

Adapted from Jones 2023

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### Introduction

The proposed project would replace the CR 143 culverts with a bridge and reconstruct the approaches over Dry Berry Creek. The project would require approximately 3.53 acres of additional right-of-way (ROW). The project limits extend from 3,200 feet east of CR 234 to 4,814 feet west of the IH 35 service road. The project is located with the TCEQ Edwards Aquifer Recharge Zone (EARZ). TxDOT conducted a Geologic Assessment (GA) on the project.

### Methodology

Before fieldwork, pertinent data was reviewed, including local and regional geology (Blome et al., 2005), soils (National Resource Conservation Service [NRCS] 2023), flood insurance rate maps (Federal Emergency Management Agency [FEMA] 2012), and well records (Texas Water Development Board [TWDB] 2023).

Pedestrian surveys were conducted on 12 December 2023 by walking in transects spaced no more than 25 feet apart while visually surveying for indications of karst features or other features reported in a GA. A reconnaissance investigation was performed on all potential features by Brian D. Cowan, a licensed Professional Geoscientist (TX: 11180) to evaluate the subsurface extent and infiltration potential. The sensitivity of each feature was ranked using the point system as defined in TCEQ 2004. All work was conducted and supervised by Brian D. Cowan, a licensed Professional Geoscientist in the State of Texas (#11180).

### **Results**

### Regional Hydrostratigraphy

The project area is in the Northern Segment of the Edwards Aquifer (NEA), which consists of the Comanche Peak Limestone, Edwards Limestone, and Georgetown Formation, collectively referred to as the Edwards and associated limestones. The NEA overlies the Walnut and Glen Rose formations and is overlain by the confining Del Rio Clay. The Walnut Formation is the oldest unit of the Fredericksburg Group and is underlain by the Glen Rose Formation. In most areas, the Walnut formation acts as a confining unit for the bottom of the NEA, but in some locations, it is composed of permeable shell beds (Kirkland et al. 1996). These beds are part of the NEA in the areas where they occur. The Walnut Formation is comprised of the Bull Creek, Bee Cave, and Cedar Park members. These units are nodular grainstones, except for the Bee Cave Member, which has a significant clay content (Kirkland et al. 1996). The Comanche Peak Formation is the next youngest unit in the Fredericksburg Group and is made up of chalky fossiliferous limestone interbedded with thin shale beds (USGS 2015). The Edwards Formation is the youngest unit in the Fredericksburg group and is comprised of massive, vuggy limestone. The Georgetown Formation represents the stratigraphic top of the units included in the NEA. The Georgetown Formation is a fossiliferous micritic limestone (Wilbert 1966). These units are overlain by the Del Rio Clay, which acts as an aquitard

(Kirkland et al. 1996). The following units are mapped within the project area: Georgetown Formation, Quaternary alluvium, and Quaternary undivided alluvium. A stratigraphic column showing the regional geology is included as Attachment B.

### Narrative Description of Project Area Geology

The project area is underlain by alluvial units and the Georgetown Limestone (Collins 2005; Attachment D, Figure 2). The project area is mostly covered by paved roadway and the unpaved portions consisted of thick sediments that are Quaternary in age. Massive nodular limestone bedrock outcrops occurred within the bed of Dry Berry Creek and in a drainage ditch to the east of Dry Berry Creek. The rest of the project area appears to be underlain by thick alluvium and limestone residuum.

The following descriptions are summarized from Collins 2005 and Blome et al. 2005:

- Georgetown Formation: massive, nodular limestone, reddish-brown and gray to lighttan, marly with a biomicritic texture. It is characterized by considerable jointing, which contributes to its hydrological properties, particularly in terms of groundwater flow and aquifer recharge. The Georgetown Formation is often hydrologically connected to the underlying Edwards Limestone.
- Quaternary alluvium: alluvium, gravel, sand, silt, and clay along streams and rivers (Collins, 2005).
- Quaternary undivided alluvium: sand, silt, clay, and some gravel. Includes terrace alluvium, local drainageway alluvium, and slope-wash alluvium (Collins, 2005).

### Soils

A review of the NRCS United States Department of Agriculture (USDA) Web Soil Survey (NRCS 2024) indicated that three soil types occur within the project area (Attachment D, Figure 3).

Map Unit Symbol	Map Unit Name	Acres in Project Area	Percent of Project Area	Thickness (ft)	Hydrologic Group	Description
DnA	Denton silty clay, 0 to 1 percent slopes	1.2	20.8	6.66	С	Deep, well drained, moderately low to moderately high saturated conductivity, formed in clayey slope alluvium and/or residuum over calcareous residuum weathered from limestone
KrB	Krum silty clay, 1 to 3 percent slopes	2.2	39.1	>6.66	С	Very deep, well drained, high saturated conductivity, clayey alluvium of Pleistocene age derived from mixed sources
OkA	Oakalla silty clay loam, 0 to 2 percent slopes, frequently flooded	2.2	40.1	>6.66	В	Very deep, well drained, moderately high to high saturated conductivity, loamy alluvium derived from limestone

### FEMA Flood Zones

The project area is shown on the FEMA flood map 48491C0285F effective 19 December 2019. Significant portions of the project area are within Zone A, the 100-Yr floodplain, which is the area where there is a one percent or greater annual chance of flooding. Parts of the project area are outside the FEMA flood zones (Attachment D, Figure 2).

### Water Well Records

According to the Texas Water Development Board (TWDB) Groundwater Database, Brackish

Resources Aquifer Characterization System Database, and Submitted Drillers Reports Database, no wells occur within the project area. A groundwater monitoring well was observed on the east side of Dry Berry Creek within the south CR-143 ROW, but it was not listed in any of the searched databases. Well depth, screened interval, completion date, and owner information are unknown.

### Feature Descriptions

One feature was identified within the project area. The feature is described in the Geologic Assessment Table (Attachment A) and below. The location of the feature is mapped on the Site Geologic Map (Attachment D, Figure 2). Feature photographs can be found in Appendix A.

Feature CR143-01 Manmade Feature in Bedrock -97.656998°, 30.730683° This feature is a monitoring well located on the east side of Dry Berry Creek within the south CR-143 ROW, but it was not listed in any of the searched databases. Well depth, screened interval, completion date, and owner information are unknown. The well has a cap and concrete pad and there is no indication of rapid infiltration of water into the subsurface near this feature, it is not considered sensitive according to the Edwards Aquifer Rules (30 TAC §213.5(b)(3)).

### **Discussion and Recommendations**

The potential for rapid recharge to the Edwards Aquifer within much of the project area is low due to the lack of features with downward trending voids that may act as a conduit for flow into the Edwards Aquifer. The feature identified was a monitoring well that appeared to be properly constructed with a cap and concrete pad and no indications of rapid infiltration of water into the subsurface near the feature. The feature was not listed in any TWDB database that was reviewed for this report. It is recommended that the owner of the well be identified so that more information on its construction and reason for groundwater monitoring can be ascertained. This well should be properly abandoned per applicable standards prior to roadway construction. It is important to note that visual observations alone cannot identify all karst features as they are often obscured by sediment, pavement, or dense vegetation.

Care should be taken during subsurface excavation within the EARZ as there is a potential to intersect a karst feature with no previous surface expression. A qualified Professional Geoscientist should inspect excavations for karst features. If a void is encountered during excavation within the EARZ, all work around it should cease immediately, and a qualified Professional Geoscientist should inspect the void and prepare a TCEQ Void Discovery Notification Form, if applicable.

### References

Blome, C.D., Faith, J.R., Pedraza, D.E., Ozuna, G.B., Cole, J.C., Clark, A.K., Small, T.A., and Morris, R.R., 2005, Geologic map of the Edwards aquifer recharge zone, south-central Texas, U.S. Geological Survey, Scientific Investigations Map SIM-2873, 1:200,000.

Collins, E.W., 2005, Geologic map of the west half of the Taylor, Texas, 30 X 60 minute quadrangle: central Texas urban corridor, encompassing Round Rock, Georgetown, Salado, Briggs, Liberty Hill, and Leander, University of Texas at Austin, Bureau of Economic Geology, Miscellaneous Map 43, 1:100,000. Online at https://ngmdb.usgs.gov/Prodesc/proddesc\_70045.htm

Federal Emergency Management Agency (FEMA). 2012. National Flood Insurance Program, Flood Insurance Rate Map (FIRM) panel: 48209C0370F - Effective 09/02/2005. Available Online at https://msc.fema.gov/portal/home. Accessed on November 29, 2023.

C. V. Proctor, Jr., T. E. Brown, J. H. McGowen, and N. B. Waechter, 1974, Geologic Atlas of Texas, Austin Sheet, Bureau of Economic Geology.

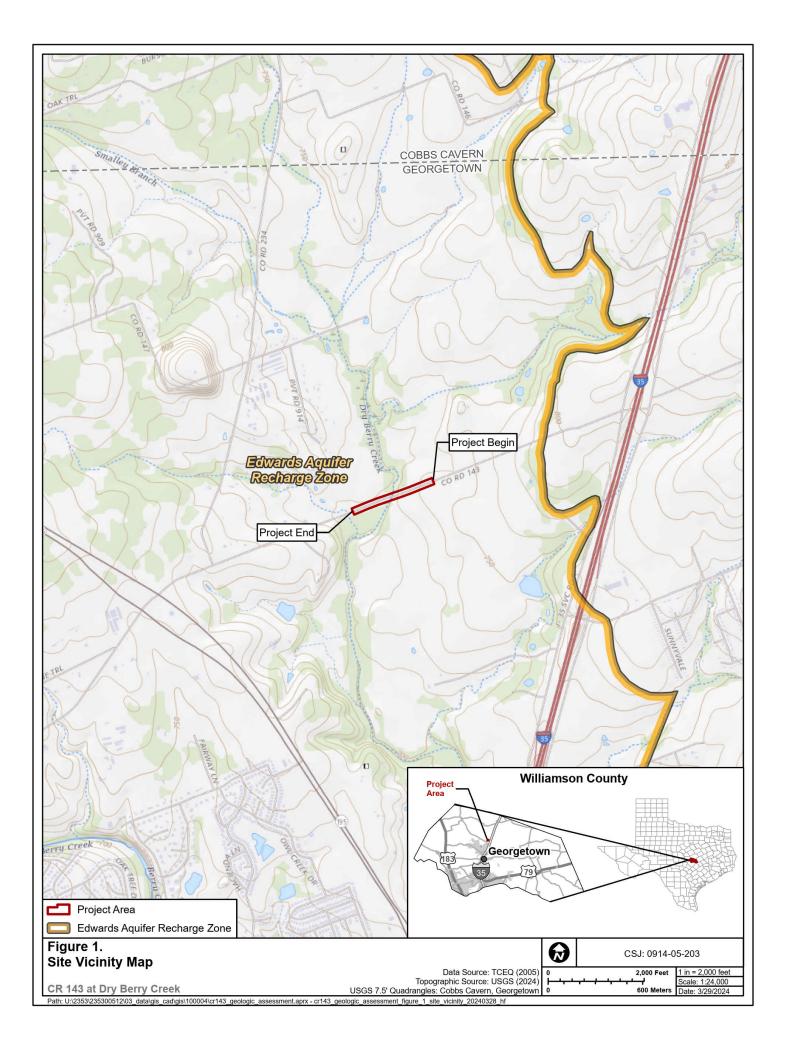
Jones, I.C., 2023, Conceptual Model: Northern Segment of the Edwards (Balcones Fault Zone) and Associated Trinity Aquifers of Texas, Texas Water Development Board Publication. February 6, 2023.

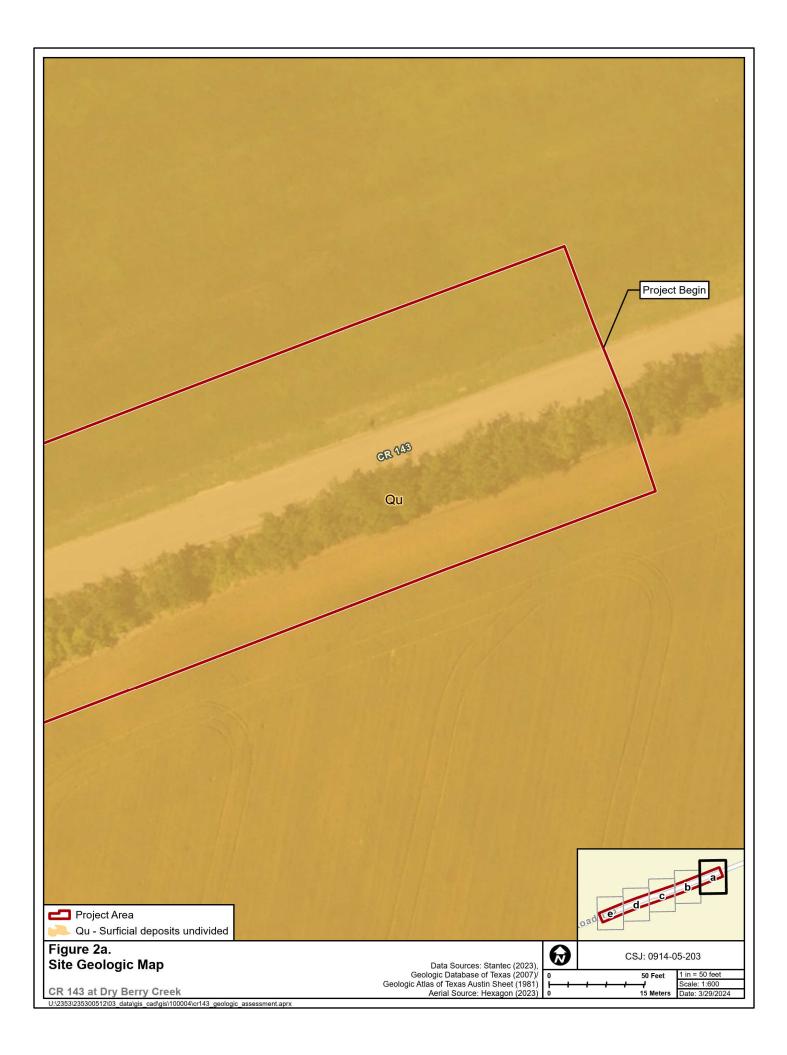
Texas Commission on Environmental Quality (TCEQ). 2004. Instructions to Geologists for Geologic Assessments on the Edwards Aquifer Recharge/Transition Zone. TCEQ RG-0508, 34 p., revised October 1, 2004.

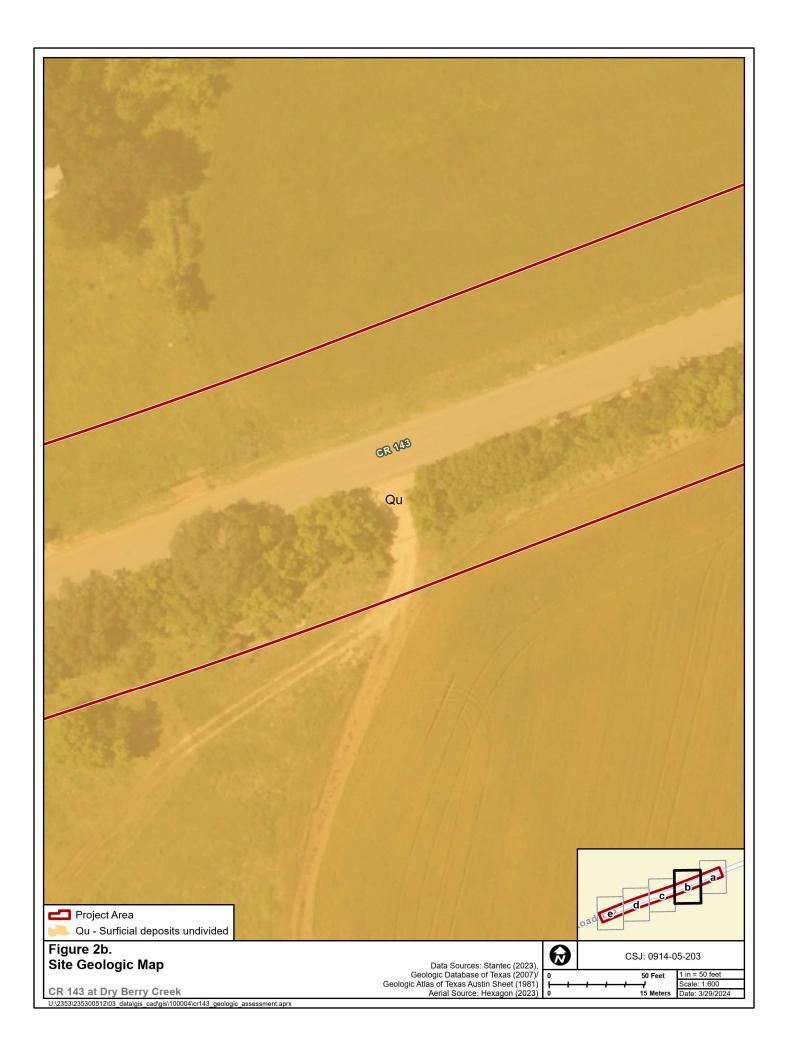
Texas Water Development Board (TWDB). 2023. Groundwater Data Viewer. Available online at https://www3.twdb.texas.gov/apps/waterdatainteractive/groundwaterdataviewer. Accessed on November 29, 2023.

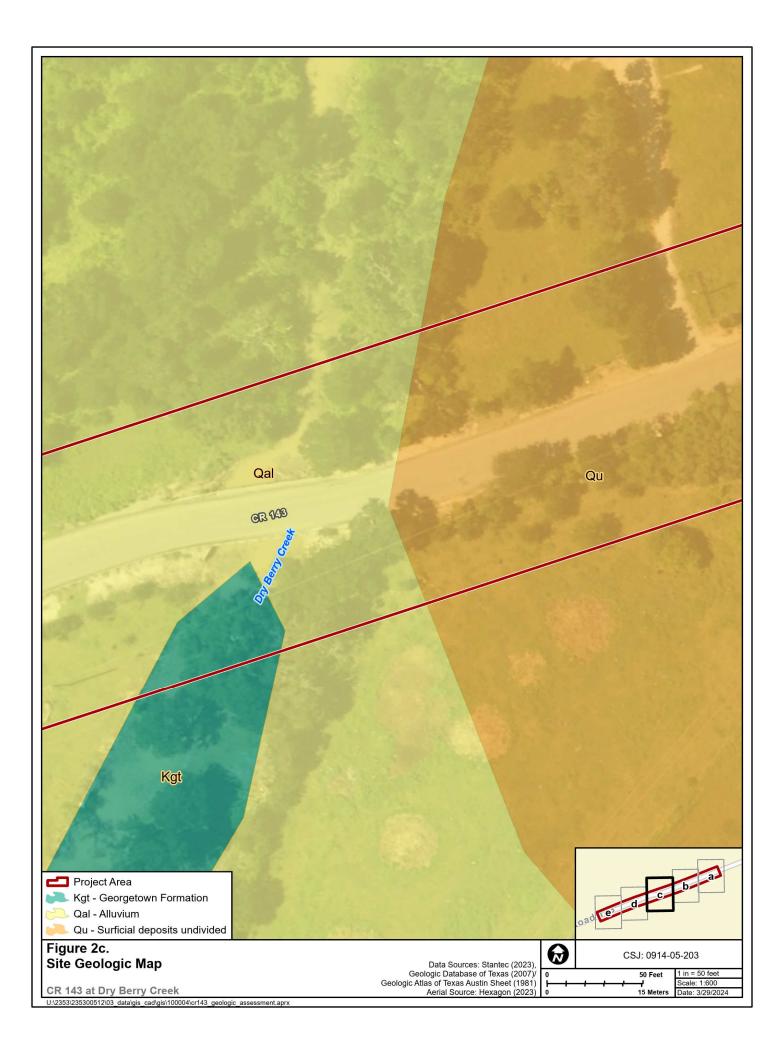
United States Department of Agriculture (USDA), Natural Resources Conservation Service, Soil Survey. 2019. Web Soil Survey. Available online at http://websoilsurvey.nrcs.usda.gov/. Accessed November 29, 2023.

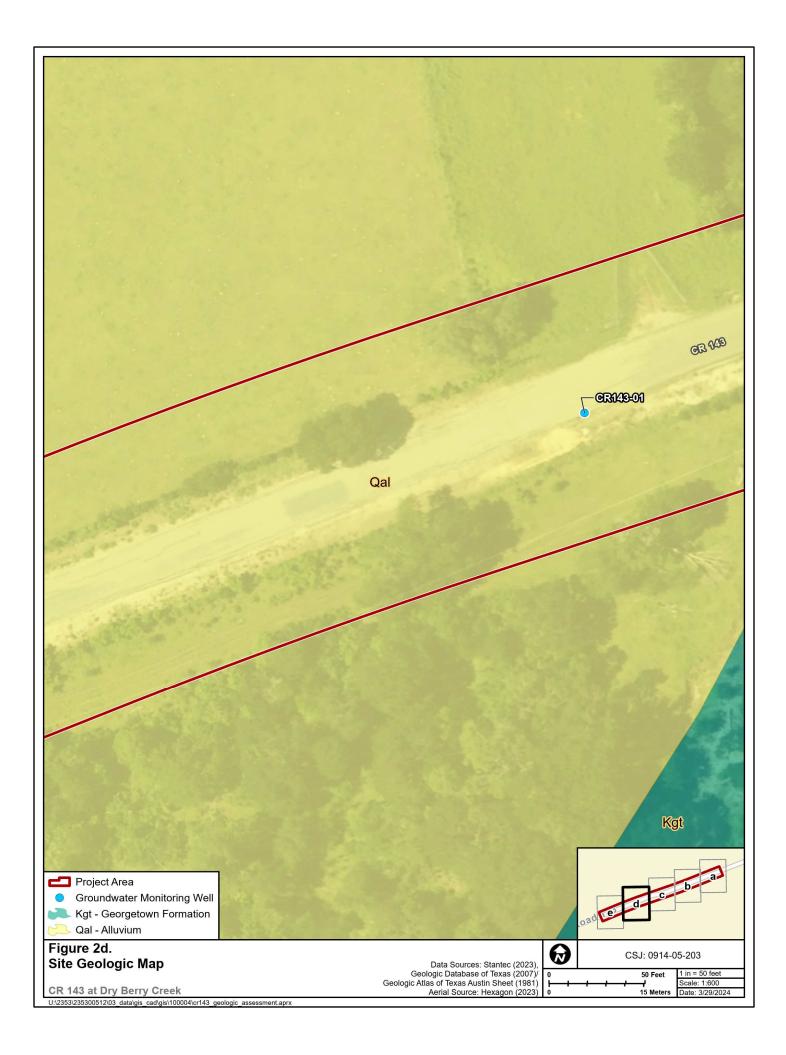
Attachment D: Project Location, Site Geology & Site Soils Maps

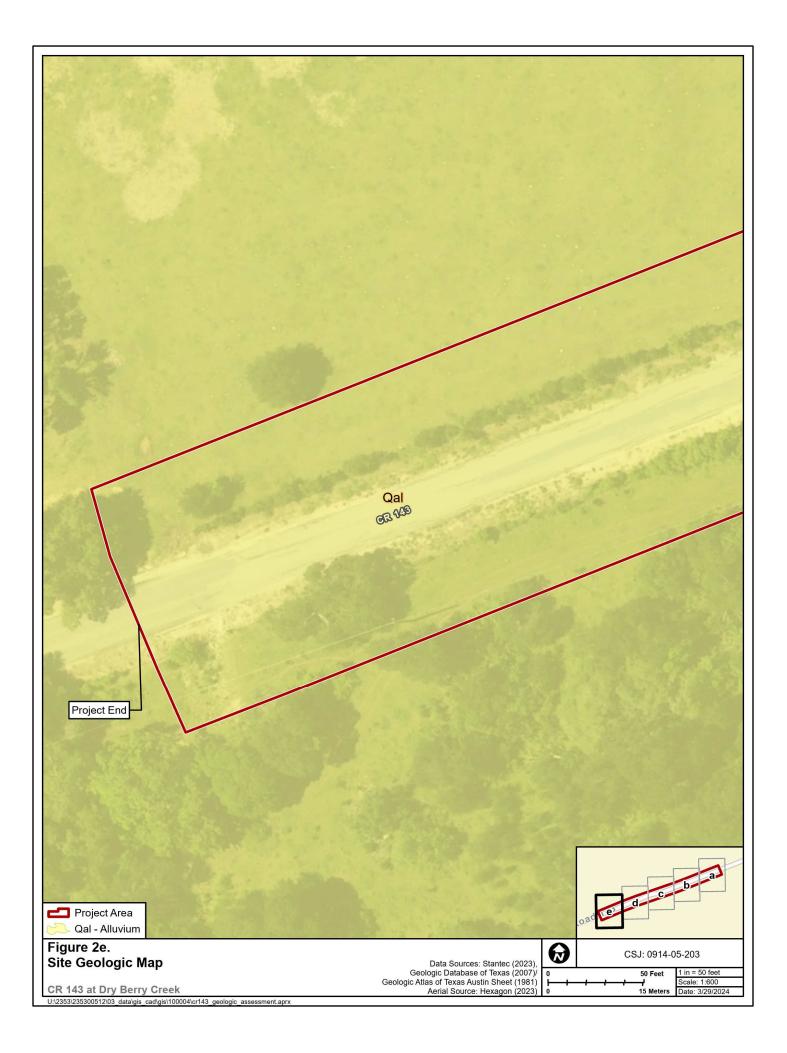


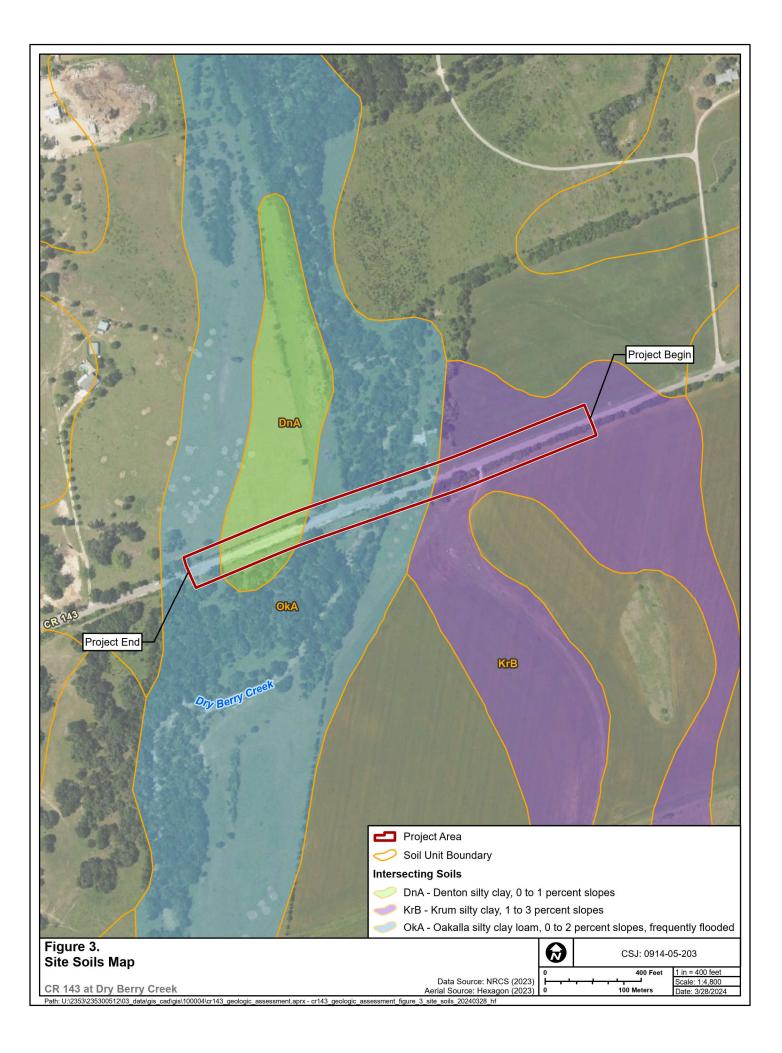












Appendix A: Field Photographs



Photo 01: View of project area from eastern project terminus. Facing west.



Photo 02: View of project eastern terminus. Facing east.



Photo 03: Adjacent properties are used as rural residences and agricultural with some abandoned buildings. Facing north.



Photo 04: Downstream view of Dry Berry Creek. Facing south.



Photo 05: Upstream view of Dry Berry Creek. Facing north.



Photo 06: A groundwater monitoring well is located within the project right of way southwest of the culvert. Facing west.



Photo 07: View of western project terminus from project area. Facing west.



Photo 08: View of project area from western project terminus. Facing east.



Photo 09: Utilities are present within proposed ROW. Facing north.



Photo 10: Limestone bedrock, consistent with the Georgetown Formation in creek on north side of the culvert. Facing west.



Photo 11: View underneath culvert facing upstream. Facing north.



Photo 12: View of bedrock outcrop in drainage ditch southeast of bridge. The only bedrock out crops are within the creek bed and in this ditch. Consistent with the Georgetown Formation. Facing east.

# **SECTION 4**

# TEMPORARY STORMWATER SECTION (TCEQ-0602)



## **Temporary Stormwater Section**

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

#### Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This Temporary Stormwater Section is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: Elizabeth W. Wagner, P.E.

Date: <u>2/4/2025</u> Signature of Customer/Agent:

Regulated Entity Name: Kimley-Horn and Associates, Inc.

Project Information

### Potential Sources of Contamination

*Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.* 

1. Fuels for construction equipment and hazardous substances which will be used during construction:

The following fuels and/or hazardous substances will be stored on the site: \_\_\_\_\_

These fuels and/or hazardous substances will be stored in:

Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year.

- Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
- Fuels and hazardous substances will not be stored on the site.
- 2. Attachment A Spill Response Actions. A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
- 3. Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
- 4. Attachment B Potential Sources of Contamination. A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.

#### Sequence of Construction

- 5. Attachment C Sequence of Major Activities. A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
  - For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given.
  - For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
- 6. Name the receiving water(s) at or near the site which will be disturbed or which will receive discharges from disturbed areas of the project: <u>Dry Berry Creek</u>

### Temporary Best Management Practices (TBMPs)

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

7. Attachment D – Temporary Best Management Practices and Measures. TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

	A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.
	A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.
	A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer.
	A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.
8.	The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
	Attachment E - Request to Temporarily Seal a Feature. A request to temporarily seal a feature is attached. The request includes justification as to why no reasonable and practicable alternative exists for each feature.
	There will be no temporary sealing of naturally-occurring sensitive features on the site.
9.	Attachment F - Structural Practices. A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.
10.	Attachment G - Drainage Area Map. A drainage area map supporting the following requirements is attached:
	<ul> <li>For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided.</li> </ul>
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used.
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area.
	<ul> <li>There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.</li> </ul>

There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. Erosion and sediment controls other than sediment basins or sediment traps within each disturbed drainage area will be used.

- 11. Attachment H Temporary Sediment Pond(s) Plans and Calculations. Temporary sediment pond or basin construction plans and design calculations for a proposed temporary BMP or measure have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information must be signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed temporary BMPs and measures are attached.
  - 🖂 N/A
- 12. Attachment I Inspection and Maintenance for BMPs. A plan for the inspection of each temporary BMP(s) and measure(s) and for their timely maintenance, repairs, and, if necessary, retrofit is attached. A description of the documentation procedures, recordkeeping practices, and inspection frequency are included in the plan and are specific to the site and/or BMP.
- 13. All control measures must be properly selected, installed, and maintained in accordance with the manufacturer's specifications and good engineering practices. If periodic inspections by the applicant or the executive director, or other information indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations.
- 14. If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain).
- 15. Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50%. A permanent stake will be provided that can indicate when the sediment occupies 50% of the basin volume.
- 16. Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source for stormwater discharges (e.g., screening outfalls, picked up daily).

#### Soil Stabilization Practices

*Examples:* establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

17. Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices. A schedule of the interim and permanent soil stabilization practices for the site is attached.

- 18. Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 19. Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

### Administrative Information

- 20. All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
- 21. If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
- 22. Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

# ATTACHMENT A

### SPILL RESPONSE ACTION

If there is an accidental spill on site, the contractor shall respond with appropriate action. The contractor will be required to contact the owner and in turn the owner will contact the TCEQ in the event of a spill on site. In addition to the following guidance, reference the latest version of TCEQ's Technical Guidance Manual (TGM) RG-348 Section 1.4.16. For guidance on reportable guantities of spills reference https://www.tceq.texas.gov/response/spills/spill\_rq.html

#### CLEANUP

- Clean up leaks and spills immediately.
- Use a rag for small spills on paved surfaces, a damp mop for general cleanup, and absorbent material for larger spills. If the spilled material is hazardous, then the used cleanup materials are also hazardous and must be disposed of as hazardous waste.
- Never hose down or bury dry material spills. Clean up as much of the material as possible and dispose of properly.

#### **MINOR SPILLS**

- Minor spills typically involve small quantities of oil, gasoline, paint, etc. which can be controlled by the first responder at the discovery of the spill.
- Use absorbent materials on small spills rather than hosing down or burying the spill.
- Absorbent materials should be promptly removed and disposed of properly.
- Follow the practice below for a minor spill:
  - Contain the spread of the spill.
  - Recover spilled materials.
  - Clean the contaminated area and properly dispose of contaminated materials.

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#### SEMI-SIGNIFICANT SPILLS

Semi-significant spills still can be controlled by the first responder along with the aid of other personnel such as laborers and the foreman, etc. This response may require the cessation of all other activities.

Spills should be cleaned up immediately:

- Contain spread of the spill.
- Notify the project foreman immediately.
- If the spill occurs on paved or impermeable surfaces, clean up using "dry" methods (absorbent materials, cat litter and/or rags). Contain the spill by encircling with absorbent materials and do not let the spill spread widely.
- If the spill occurs in dirt areas, immediately contain the spill by constructing an earthen dike. Dig up and properly dispose of contaminated soil.
- If the spill occurs during rain, cover spill with tarps or other material to prevent contaminating runoff.

#### SIGNIFICANT/HAZARDOUS SPILLS

For significant or hazardous spills that are in reportable quantities:

- Notify the TCEQ by telephone as soon as possible and within 24 hours at 512- 339-2929 (Austin) or 210-490-3096 (San Antonio) between 8 AM and 5 PM. After hours, contact the Environmental Release Hotline at 1-800-832-8224. It is the contractor's responsibility to have all emergency phone numbers at the construction site.
- For spills of federal reportable quantities, in conformance with the requirements in 40 CFR parts 110,119, and 302, the contractor should notify the National Response Center at (800) 424-8802.
- Notification should first be made by telephone and followed up with a written report.
- The services of a spills contractor or a Haz-Mat team should be obtained immediately. Construction personnel should not attempt to clean up until the appropriate and qualified staffs have arrived at the job site.
- Other agencies which may need to be consulted include, but are not limited to, the City Police Department, County Sheriff Office, Fire Departments, etc.

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## ATTACHMENT B

### POTENTIAL SOURCES OF CONTAMINATION

The potential sources of storm water pollution due to the roadway improvements include displaced soil, wastewater, dust, and miscellaneous construction waste. The potential pollution sources are listed below.

There is potential for soil to be displaced from the construction site from activities such as grading, clearing/grubbing, excavating, and filling. Other potential sources of stormwater pollution include wastewater from portable bathroom, litter generated during the construction process, additional dust and soils tracked onto roads by construction vehicles, miscellaneous construction products and waste, and imported soils.

There are also hazardous construction materials including fuel chemicals such as automotive fluids and lubricants, use of asphaltic products, and petroleum products from the operation of equipment on the site, all of which are potential sources of contamination. The clean up and containment of any fuels, hydraulic fluids, hydrocarbons, or other hazardous substances released on site will be the responsibility of the contractor. In the case that a spill occurs on the site the contractor must respond immediately following *Attachment A – Spill Response Actions*.

The primary storm water containment expected to be generated during the construction project is the entrained soils (soil particles) which will affect the turbidity of the runoff. From this project, disturbed soils will result from:

- Clearing, grubbing, and grading within the right-of-way
- Roadway embankment grading
- Driveway embankment grading
- Ditch excavation and embankment grading
- Roadway construction
- Placement and removal of temporary erosion and sediment controls
- Placement of topsoil and seeding

Increased sediment loading in stormwater can be attributed to a) direct impingement of rain onto disturbed soil areas, sand, gravel and rock areas where rains dislodge or entrain particles; b) erosion of disturbed soil areas, and c) the transfer of soils and particulate matter via equipment



or vehicle tires onto non-disturbed areas. All stormwater will be treated by BMPs such as floating turbidity barrier and rock filter dams before leaving the project site and discharging to receiving waters. BMPs are placed at the project perimeter and key outfall locations as shown in the Stormwater Pollution Prevention Plan (SW3P).



# ATTACHMENT C

### SEQUENCE OF MAJOR ACTIVITIES

The general order of soil disturbing activities is detailed below. The detailed construction sequence is included in Edwards Aquifer Protection Roadway Application, *Attachment G* – *Construction Plans.* 

Temporary erosion and sediment control measures include floating turbidity barrier and rock filter dams which will be placed before commencing any construction activities and will remain in place during all phases of construction. Temporary controls will be removed once sufficient vegetation is established.

#### PHASE 1

Close road, remove existing pavement and bridge and construct proposed pavement and bridge, and reopen road upon completion.

- 1. Place work zone approach signage.
- 2. Place detour signage and barricades.
- Install temporary erosion control measures and BMPs. BMPs to be inspected and maintained per TCEQ and WPAP requirements.
- 4. Clearing, grubbing, and grading at locations of proposed pavement.
- 5. Remove existing County Road 143 bridge and roadway as shown in the plans.
- 6. Construct proposed County Road 143 bridge and roadway as shown in the plans.
- 7. Placement of topsoil for final planting and seeding.
- 8. Reopen road upon completion of construction.



## ATTACHMENT D

### TEMPORARY BEST MANAGEMENT PRACTICES AND MEASURES

Temporary BMPs such as floating turbidity barrier and rock filter dams will be installed to prevent onsite sediment and debris from entering surface streams during construction. Temporary BMPs will be placed before commencing any construction activities and will remain in place during all phases of construction.

Floating turbidity barrier will be installed at the downstream end of the project as a perimeter control to filter onsite stormwater runoff before leaving the project site. Type 4 rock filter dams will be placed at ditch outfalls to intercept sediment from overland and/or concentrated flow upstream of culvert locations. Type 2 rock filter dams will be placed at ultimate outfall locations to intercept sediment from concentrated flow leaving the project site to protect receiving streams.

All disturbed areas shall be covered with temporary seeding as a means of dust control and to reduce wind erosion. After construction, the project limits will be revegetated to prevent pollution of surface streams. Temporary BMPs will be removed once sufficient vegetation is established. Temporary BMP placement and quantities, and additional measures are provided in the SW3P.



# ATTACHMENT F

### STRUCTUAL PRACTICES

Temporary structural practices used to limit transport of pollutants include floating turbidity barrier and Type 2 and Type 4 rock filter dams. These temporary BMPs are proposed to be placed along the perimeter of the channel and at key outfall locations.

The TCEQ guidelines included in Section 1.2 through Section 1.4 of RG-348 must be followed for installation and maintenance of temporary structural erosion and sediment control BMPs. Additional details on temporary structural practices can be found in the SW3P.



## ATTACHMENT G

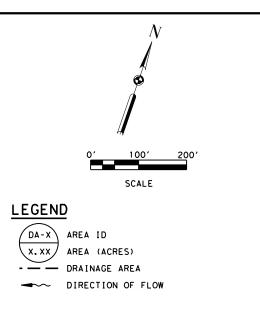
## DRAINAGE AREA MAPS

Please reference attached drainage area maps.





79001 \CR FILENAME: C: NpwNkh PLOTTED: 3/17/2025



#### NOTES:

1. PEAK DISCHARGE FOR DITCHES DETERMINED BY RATIONAL METHOD. ALL DITCHES DESIGNED TO THE 5 YEAR STORM USING ATLAS-14.

3/17/2025



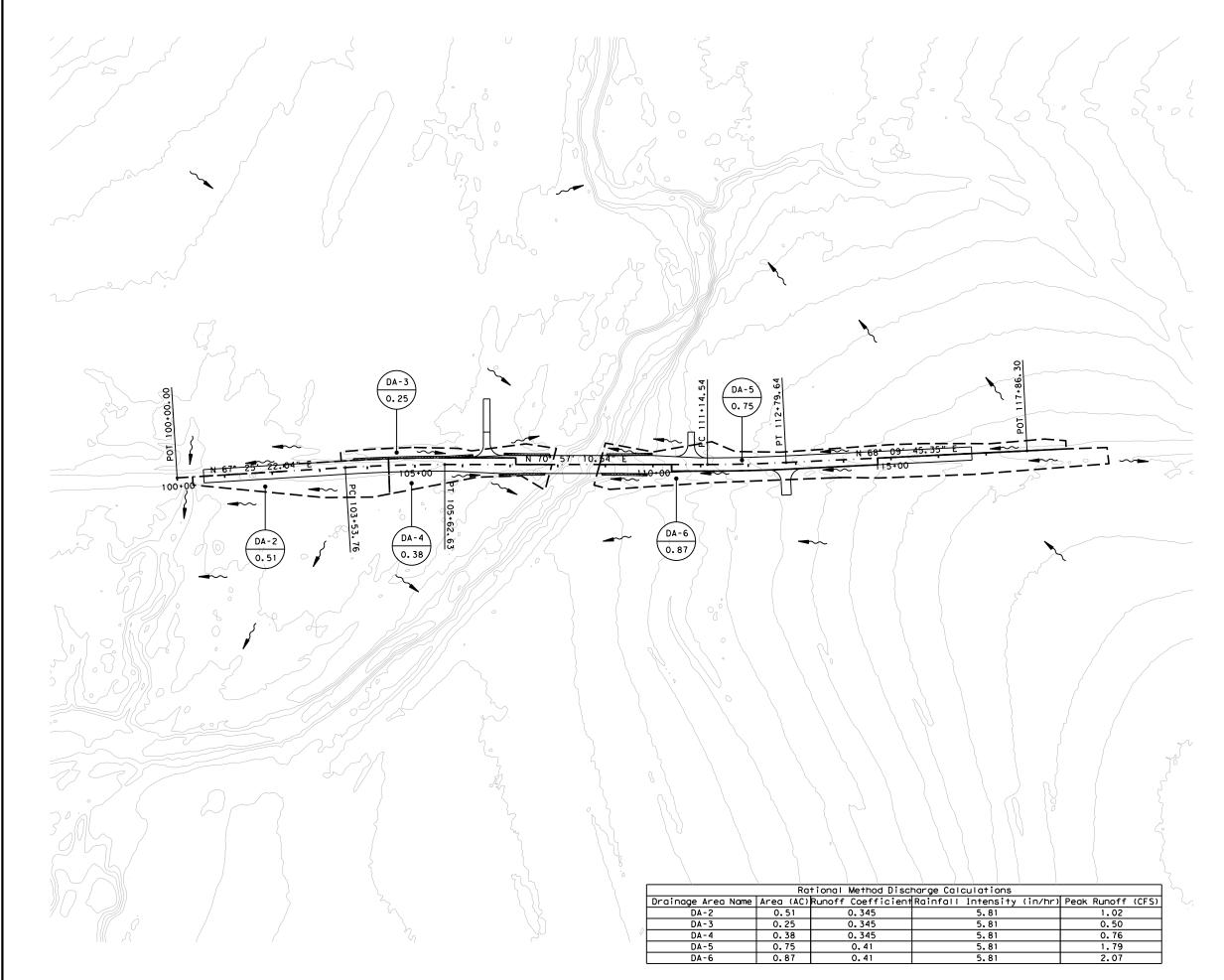


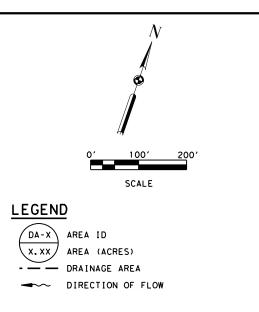


### INTERNAL DRAINAGE AREA MAP

SHEET 1 OF 1								
FED.RD. DIV.NO.	FEDE	FEDERAL AID PROJECT NO. HIGHWAY NO.						
6		CR 143						
ST	ATE .	DIST.	COUN	۲۱	SHEET NO.			
TEX	(AS	AUS	WILLIA	VISON				
CONT.		SECT.			64			
09	14	05	203, E	ETC.				

off	(CFS)
40	
02	
46	
76	
79	
07	





#### NOTES:

1. PEAK DISCHARGE FOR DITCHES DETERMINED BY RATIONAL METHOD. ALL DITCHES DESIGNED TO THE 5 YEAR STORM USING ATLAS-14.

2/3/2025







### INTERNAL DRAINAGE AREA MAP

SHEET 1 OF 1								
FED.RD. DIV.NO.	FEDE	FEDERAL AID PROJECT NO. HIGHWAY NO.						
6		BR2020(732) CR 143						
ST	<b>ATE</b>	DIST.	COUNTY		SHEET NO.			
TEX	XAS	AUS	WILLIAM	ISON				
CO	NT.	SECT.	JOB		64			
09	14	05	203, E	TC.				

# ATTACHMENT I

### INSPECTION AND MAINTENANCE FOR BMPS

All erosion and sediment control measures will be maintained in effective operating condition by following the project management procedures. The general maintenance and inspection requirements are provided in the SW3P. The maintenance plan for temporary BMPs meets the maintenance guidance provided in RG-348.

The contractor shall install and maintain the integrity of temporary erosion and sediment control devices to accumulate silt and debris until soil disturbing activities are complete, permanent erosion control features are in place, and sufficient vegetation has been established in accordance with contract documents including standard TxDOT Specification 506.

Maintenance, repairs, or retrofits will adhere to the project standards and details for the BMP. Damaged portions of BMPs shall be removed and replaced as needed to adhere to the contract documents. BMPs that cannot be adequately repaired or retrofitted to meet project requirements shall be removed and replaced entirely in accordance with the contract documents.

The maintenance documentation procedures and recordkeeping practices to be followed are summarized in the TCEQ Edwards Aquifer General Construction Notes, which are provided in the Roadway Application included as Attachment G.



# ATTACHMENT J

## SCHEDULE OF INTERIM AND PERMANENT SOIL STABILIZATION PRACTICES

The general order of soil disturbing construction activities is detailed below. The detailed construction sequence is included in the Traffic Control Plans in *Attachment G – Construction Plans*. Soil stabilization practices are **bold**.

#### PHASE 1

Close road, remove existing pavement and bridge and construct proposed pavement and bridge, and reopen road upon completion.

- 1. Install temporary erosion control measures and BMPs including floating turbidity barrier and rock filter dams.
- 2. Clearing, grubbing, and grading at locations of proposed pavement.
- 3. Construct proposed County Road 143 bridge and roadway as shown in the plans.
- 4. Placement of topsoil for final planting and seeding.

Where construction activity temporarily ceases for more than 21 days, these areas will be stabilized by the contractor with temporary seeding within 14 days of the last disturbance.

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Records will be kept at the site to document dated when:

- Major grading activities occur
- Construction activities temporarily cease
- Construction activities permanently cease; and
- Stabilization measures are initiated

## **SECTION 5**

## **ADDITIONAL FORMS**

TCEQ – 20872 EAPP Roadway Application County Road (CR) 143 at Dry Berry Creek Bridge Replacement Kimley **»Horn** 

#### Agent Authorization Form For Required Signature

Edwards Aquifer Protection Program Relating to 30 TAC Chapter 213 Effective June 1, 1999

t	Lorena Roque Martinez, P.G.	,
· · · · · · · · · · · · · · · · · · ·	Print Name	
	Environmental Project Planner	,
	Title - Owner/President/Other	
of	Texas Department of Transportation- Austin District Corporation/Partnership/Entity Name	,
have authorized	Elizabeth W. Wagner, P.E. Print Name of Agent/Engineer	
of	Kimley-Horn and Associates, Inc. Print Name of Firm	

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

- 1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
- 2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
- 3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
- 4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
- 5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:

pplicant's Signature

2025 Dat

THE STATE OF \_\_\_\_\_ § \_\_\_\_ § County of

BEFORE ME, the undersigned authority, on this day personally appeared \_\_\_\_\_known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this \_\_\_\_ day of \_\_\_\_\_, \_\_\_\_. NOTARY PUBLIC Typed or Printed Name of Notary MY COMMISSION EXPIRES: Hur at the full



## TCEQ Core Data Form

For detailed instructions on completing this form, please read the Core Data Form Instructions or call 512-239-5175.

#### SECTION I: General Information

1. Reason for Submission (If other is checked please describe in space provided.)						
New Permit, Registration or Authorization ( <i>Core Data Form should be submitted with the program application.</i> )						
Renewal (Core Data Form should be submitted with the	Other					
2. Customer Reference Number (if issued)	Follow this link to search for CN or RN numbers in	3. Regulated Entity Reference Number (if issued)				
CN 600803456	Central Registry**	RN				

#### SECTION II: Customer Information

4. General Cu	istomer Informa <b>ti</b> on	ustomer I	Informa <b>ti</b> on	Updates (mm/dd/	уууу)			
New Custor	ner 🛛 🛛 l egal Name (Verifiable with the Te	I Jpdate to Customer Inform xas Secretary of State or Te			ige in Regulated Ent Accounts)	ity Owne	ership	1
The Customer Name submitted here may be updated automatically based on what is current and active with the Texas Secretary of State (SOS) or Texas Comptroller of Public Accounts (CPA).								
6. Customer Legal Name (If an individual, print last name first: eg: Doe, John) <u>If new Customer, enter previous Customer below:</u>								
Texas Department of Transportation								
7. TX SOS/CP	A Filing Number	digits)		9. Federal Tax I (9 digits)	D	10. DUNS applicable)	Number <i>(if</i>	
11. Type of C	ustomer: Corpora	tion		🗌 Individ	lual	Partne	rship: 🗌 Gen	neral 🗌 Limited
Government:	🗋 City 🔲 County 🔲 Federal 🗌	Local 🛛 State 🗌 Other		Sole P	roprietorship	🗌 Otł	ner:	
12. Number of	of Employees				13. Independer	ntly Owr	ned and Ope	erated?
	21-100 🗌 101-250 🔲 251	<b>—</b> 5				🗙 No		
14. Customer	Role (Proposed or Actual) – as	it relates to the Regulated I	En <del>t</del> ity listed	on this form.	Please check one of	the follo	wing	
Owner	Operator Olimitation Operator Olimitation Operator Opera	⊠ Owner & Oper rty □ VCP/BSA Ap			Other:			
15. Mailing	John C. Peters, P.E. Texas Depar	tment of Transportation - (	Georgetowr	n Area Office				
Address:	2727 S. Austin Ave							
	City Georgetown	State	TX	ZIP	78626		ZIP + 4	
16. Country N	Mailing Informa <b>ti</b> on <i>(if outside</i>		17. E-Mail Address (if applicable)					
			j	john.peters@t	xdot.gov			

18. Telephone Number	19. Extension or Code	20. Fax Number (if applicable)
( ) -		( ) -

## SECTION III: Regulated Entity Information

	-	-						
21. General Regulated En	21. General Regulated Entity Information (If 'New Regulated Entity" is selected, a new permit application is also required.)							
New Regulated Entity	🛛 New Regulated Entity 🛛 Update to Regulated Entity Name 🔄 Update to Regulated Entity Information							
The Regulated En <b>ti</b> ty Name submi <b>tt</b> ed may be updated, in order to meet TCEQ Core Data Standards (removal of organiza <b>ti</b> onal endings such as Inc, LP, or LLC).								
22. Regulated Entity Name (Enter name of the site where the regulated action is taking place.)								
County Road (CR) 143 at Dry Berry Creek Bridge Replacement; CSJ 0914-05-203								
23. Street Address of	N/A							
the Regulated En <b>t</b> ity:						-		
<u>(No PO Boxes)</u>	City		State		ZIP		ZIP + 4	
24. County	Williamson							

#### If no Street Address is provided, fields 25-28 are required.

25. Descrip <b>ti</b> on to Physical Loca <b>ti</b> on:	CR 143 at Dry Berry Creek							
26. Nearest City						State	Ne	arest ZIP Code
Georgetown TX 78626							26	
La <b>ti</b> tude/Longitude are required and may be added/updated to meet TCEQ Core Data Standards. (Geocoding of the Physical Address may be used to supply coordinates where none have been provided or to gain accuracy).								
27. La <b>ti</b> tude (N) In Decim	al:			28. Lo	ongitude (N	V) In Decimal:		
Degrees	Minutes		Seconds	Degree	es	Minutes		Seconds
30		43	51.12		97	3	39	22.94
29. Primary SIC Code	30.	Secondary SIC C	Code	31. Primary NAICS Code 32. Secondary NAICS Code				ICS Code
(4 digits)	(4 d	igits)		(5 or 6 digit	s)	(5 or 6	digits)	
1611	N/A			237310		N/A	N/A	
33. What is the Primary E	Business of t	his en <b>ti</b> ty? <i>(Do</i>	not repeat the SIC o	r NAICS descri	ption.)	I		
Roadway								
	John C. Pe	ters, P.E. Texas De	partment of Transpo	rta <b>ti</b> on - Geor	rgetown Are	ea O <b>ffi</b> ce		
34. Mailing	2727 S. Austin Ave							
Address:	City	Georgetown	State	TX	ZIP	78626	ZIP + 4	
35. E-Mail Address:	johr	n.peters@txdot.go	V					
36. Telephone Number			37. Extension or	Code	38. F	ax Number (if appli	cable)	
(512)930-6002					(	) -		

39. TCEQ Programs and ID Numbers Check all Programs and write in the permits/registration numbers that will be affected by the updates submitted on this form. See the Core Data Form instructions for additional guidance.

🗌 Dam Safety	Districts	Edwards Aquifer	Emissions Inventory Air	Industrial Hazardous Waste
	New Source			
Municipal Solid Waste	Review Air	□ OSSF	Petroleum Storage Tank	PWS
	Noviou / III			
Sludge	Storm Water	Title V Air	Tires	Used Oil
🔲 Voluntary Cleanup	Wastewater	Wastewater Agriculture	Water Rights	Other:

#### SECTION IV: Preparer Information

40. Name:	Elizabeth Wagner, P.E.			41. Title:	Project Manager
42. Telephone Number		43. Ext./Code	44. Fax Number	45. E-Mail A	Address
( 737 ) 400-9846			( ) -	elizabeth.wag	gner@kimley-horn.com

#### SECTION V: Authorized Signature

46. By my signature below, I certify, to the best of my knowledge, that the information provided in this form is true and complete, and that I have signature authority to submit this form on behalf of the entity specified in Section II, Field 6 and/or as required for the updates to the ID numbers identified in field 39.

Company:	Kimley-Horn and Associates, Inc.	Job Title:	Civil Engineer		
Name (In Print):	Elizabeth W. Wagner, P.E.			Phone:	( 737 ) 400- 9846
Signature:	1-25-			Date:	2/3/2025