# ORGANIZED SEWAGE COLLECTION SYSTEM MODIFICATION

2525 CR 172 INDUSTRIAL 2525 CR 172, ROUND ROCK, TX 78681 ROUND ROCK, WILLIAMSON COUNTY, TEXAS

Prepared For:

**AMAZON.COM SERVICES, LLC** 

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### Prepared By:

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Firm No. 928 KHA Project No. 069284918

May 2025

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# SECTION 1: EDWARDS AQUIFER APPLICATION COVER PAGE

### **Texas Commission on Environmental Quality**

# **Edwards Aquifer Application Cover Page**

### **Our Review of Your Application**

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with 30 TAC 213.

#### **Administrative Review**

- 1. <u>Edwards Aquifer applications</u> must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.
  - To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: <a href="http://www.tceq.texas.gov/field/eapp">http://www.tceq.texas.gov/field/eapp</a>.
- 2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
- 3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
- 4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.
  - An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.
- 5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
- 6. If the geologic assessment was completed before October 1, 2004 and the site contains "possibly sensitive" features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

### **Technical Review**

- When an application is deemed administratively complete, the technical review period begins. The regional
  office will distribute copies of the application to the identified affected city, county, and groundwater
  conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days
  to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
- 2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

- clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.
- 3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited**.
- 4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

#### **Mid-Review Modifications**

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a "Mid-Review Modification". Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ's Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ's San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: 2525 CR 172 Industrial				2. Regulated Entity No.: RN112000070					
3. Customer Name: Amazon.com Services, LLC			<b>4. Customer No.:</b> CN605619519						
5. Project Type: (Please circle/check one)	New	New <u>Modification</u>		D	Extension Exception		Exception		
6. Plan Type: (Please circle/check one)	WPAP	CZP (	SCS	UST	AST	EXP	EXT	Technical Clarification	Optional Enhanced Measures
7. Land Use: (Please circle/check one)	Resider	ntial	Non-r	Non-residential			8. Sit	e (acres):	69.56 acres
9. Application Fee:	\$ 650		10. P	10. Permanent BMP(		s):	StormTrap Up-	Flo Filter	
11. SCS (Linear Ft.):	494		12. A	12. AST/UST (No. Tank			ıks):	N/A	
13. County:	William	son	14. W	14. Watershed:				Lake Creek	

# **Application Distribution**

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the "Texas Groundwater Conservation Districts within the EAPP Boundaries" map found at:

http://www.tceq.texas.gov/assets/public/compliance/field\_ops/eapp/EAPP%20GWCD%20map.pdf

For more detailed boundaries, please contact the conservation district directly.

Austin Region			
County:	Hays	Travis	Williamson
Original (1 req.)			<u>X</u>
Region (1 req.)	_	_	<u>X</u>
County(ies)			<u>X</u>
Groundwater Conservation District(s)	Edwards Aquifer AuthorityBarton Springs/ Edwards AquiferHays TrinityPlum Creek	Barton Springs/ Edwards Aquifer	NA
City(ies) Jurisdiction	AustinBudaDripping SpringsKyleMountain CitySan MarcosWimberleyWoodcreek	AustinBee CavePflugervilleRollingwoodRound RockSunset ValleyWest Lake Hills	AustinCedar ParkFlorenceGeorgetownJerrellLeanderLiberty HillPflugerville X_Round Rock

	Sa	an Antonio Region			
County:	Bexar	Comal	Kinney	Medina	Uvalde
Original (1 req.)					
Region (1 req.)			_		
County(ies)					
Groundwater Conservation District(s)	Edwards Aquifer Authority Trinity-Glen Rose	Edwards Aquifer Authority	Kinney	EAA Medina	EAA Uvalde
City(ies) Jurisdiction	Castle HillsFair Oaks RanchHelotesHill Country VillageHollywood ParkSan Antonio (SAWS)Shavano Park	Bulverde Fair Oaks Ranch Garden Ridge New Braunfels Schertz	NA	San Antonio ETJ (SAWS)	NA

I certify that to the best of my knowledge, that the application is complete and accurate. This application is hereby submitted to TCEQ for administrative review and technical review.		
Nicholas C. Brown, P.E.		
Print Name of Customer/Authorized Agent		
July -	05/15/2025	
Signature of Customer/Authorized Agent	Date	

**FOR TCEQ INTERNAL USE ONLY**			
Date(s)Reviewed:	Date Administ	tratively Complete:	
Received From:	Correct Numb	er of Copies:	
Received By:	Distribution D	Date:	
EAPP File Number:	Complex:		
Admin. Review(s) (No.):	No. AR Round	ls:	
Delinquent Fees (Y/N):	Review Time S	Spent:	
Lat./Long. Verified:	SOS Customer	r Verification:	
Agent Authorization Complete/Notarized (Y/N):	Fee Paya	able to TCEQ (Y/N):	
Core Data Form Complete (Y/N):		ned (Y/N):	
Core Data Form Incomplete Nos.:	Less	Less than 90 days old (Y/N):	



# SECTION 2: GENERAL INFORMATION FORM

# **General Information Form**

Print Name of Customer/Agent: Nicholas C. Brown, P.E.

### **Texas Commission on Environmental Quality**

For Regulated Activities on the Edwards Aquifer Recharge and Transition Zones and Relating to 30 TAC §213.4(b) & §213.5(b)(2)(A), (B) Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

### Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **General Information Form** is hereby submitted for TCEQ review. The application was prepared by:

Da	Date: <u>05/15/2025</u>			
Sig	Signature of Customer/Agent:			
_	Jilly			
Pi	Project Information			
1.	L. Regulated Entity Name: 2525 CR 172 Industrial			
2.	2. County: <u>Williamson</u>			
3.	3. Stream Basin: <u>Lake Creek</u>			
4.	1. Groundwater Conservation District (If applicable): $N$	<u>A</u>		
5.	5. Edwards Aquifer Zone:			
	X Recharge Zone Transition Zone			
6.	5. Plan Type:			
	WPAP X SCS Modification	AST UST Exception Request		

- 7. Customer (Applicant): Contact Person: David Barnett Entity: Amazon.com Services, LLC Mailing Address: Amazon Tower 1, 101 Platform Way N Zip: 37203 City, State: Nashville, TN FAX: N/A Telephone: 901-438-4156 Email Address: barnettu@amazon.com 8. Agent/Representative (If any): Contact Person: Nicholas C. Brown, P.E. Entity: Kimley-Horn and Associates Mailing Address: 10814 Jollyville Rd, Avallon IV, Suite 200 City, State: Austin, TX Zip: <u>78759</u> Telephone: 512-418-1771 FAX: N/A Email Address: nick.brown@kimley-horn.com 9. Project Location: The project site is located inside the city limits of \_\_\_\_\_\_. |X| The project site is located outside the city limits but inside the ETJ (extra-territorial jurisdiction) of Round Rock. The project site is not located within any city's limits or ETJ. 10. X The location of the project site is described below. The description provides sufficient detail and clarity so that the TCEQ's Regional staff can easily locate the project and site boundaries for a field investigation. 2525 CR 172 11. X Attachment A – Road Map. A road map showing directions to and the location of the project site is attached. The project location and site boundaries are clearly shown on the map. 12. X Attachment B - USGS / Edwards Recharge Zone Map. A copy of the official 7 ½ minute USGS Quadrangle Map (Scale: 1" = 2000') of the Edwards Recharge Zone is attached. The map(s) clearly show:
  - X Project site boundaries.
  - X USGS Quadrangle Name(s).
  - X Boundaries of the Recharge Zone (and Transition Zone, if applicable).
  - $\boxed{X}$  Drainage path from the project site to the boundary of the Recharge Zone.
- 13. X The TCEQ must be able to inspect the project site or the application will be returned. Sufficient survey staking is provided on the project to allow TCEQ regional staff to locate the boundaries and alignment of the regulated activities and the geologic or manmade features noted in the Geologic Assessment.
  - X Survey staking will be completed by this date: 05/31/2024

<u></u>	
narra	tive description of the proposed project. The project description is consistent ighout the application and contains, at a minimum, the following details:
X O X In X Pe X Pi X Si X Pi	rea of the site  ffsite areas  npervious cover  ermanent BMP(s)  roposed site use  te history  revious development  rea(s) to be demolished
15. Existing p	project site conditions are noted below:
Ex   Ex   Ex   U   X U	kisting commercial site kisting industrial site kisting residential site kisting paved and/or unpaved roads ndeveloped (Cleared) ndeveloped (Undisturbed/Uncleared) ther:
Prohibit	ted Activities
	aware that the following activities are prohibited on the Recharge Zone and are not osed for this project:
	/aste disposal wells regulated under 30 TAC Chapter 331 of this title (relating to nderground Injection Control);
(2) N	ew feedlot/concentrated animal feeding operations, as defined in 30 TAC §213.3;
(3) La	and disposal of Class I wastes, as defined in 30 TAC §335.1;
(4) TI	he use of sewage holding tanks as parts of organized collection systems; and
st	ew municipal solid waste landfill facilities required to meet and comply with Type I and ards which are defined in §330.41(b), (c), and (d) of this title (relating to Types f Municipal Solid Waste Facilities).
	ew municipal and industrial wastewater discharges into or adjacent to water in the rate that would create additional pollutant loading.
·	aware that the following activities are prohibited on the Transition Zone and are roposed for this project:

(1) Waste disposal wells regulated under 30 TAC Chapter 331 (relating to Underground

(2) Land disposal of Class I wastes, as defined in 30 TAC §335.1; and

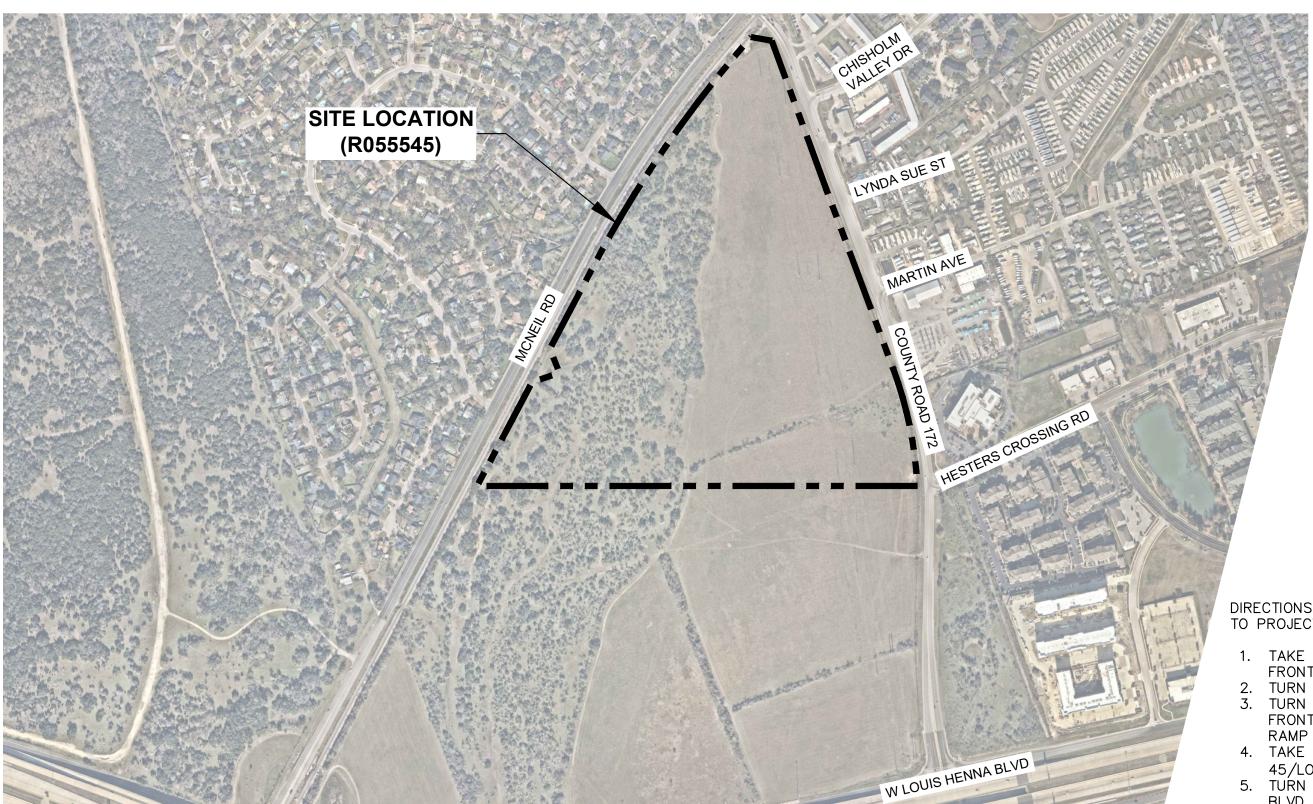
Injection Control);

(3) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41 (b), (c), and (d) of this title.

### Administrative Information

18. The	e fee for the plan(s) is based on:
	For a Water Pollution Abatement Plan or Modification, the total acreage of the site where regulated activities will occur.  For an Organized Sewage Collection System Plan or Modification, the total linear footage of all collection system lines.  For a UST Facility Plan or Modification or an AST Facility Plan or Modification, the total number of tanks or piping systems.  A request for an exception to any substantive portion of the regulations related to the protection of water quality.  A request for an extension to a previously approved plan.
19. X	Application fees are due and payable at the time the application is filed. If the correct fee is not submitted, the TCEQ is not required to consider the application until the correct fee is submitted. Both the fee and the Edwards Aquifer Fee Form have been sent to the Commission's:
	<ul> <li>TCEQ cashier</li> <li>Austin Regional Office (for projects in Hays, Travis, and Williamson Counties)</li> <li>San Antonio Regional Office (for projects in Bexar, Comal, Kinney, Medina, and Uvalde Counties)</li> </ul>
20. X	Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
21. X	No person shall commence any regulated activity until the Edwards Aquifer Protection Plan(s) for the activity has been filed with and approved by the Executive Director.

# **ROAD MAP**



**ROAD MAP** 

# 2525 CR 172 INDUSTRIAL

ROUND ROCK, TEXAS MAY 2024

DIRECTIONS FROM TCEQ HEADQUARTERS TO PROJECT SITE:

- 1. TAKE PARK 35 CIR TO S I-35 FRONTAGE RD.
- 2. TURN LEFT ONTO E BRAKER LN.
- 3. TURN LEFT ONTO N I-35 FRONTAGE RD AND TAKE THE RAMP ONTO I-35 N.
- 4. TAKE EXIT 250 TOWARDS TEXAS 45/LOUIS HENNA BLVD.
- 5. TURN LEFT ONTO W LOUIS HENNA
- 6. TURN RIGHT ONTO COUNTY ROAD

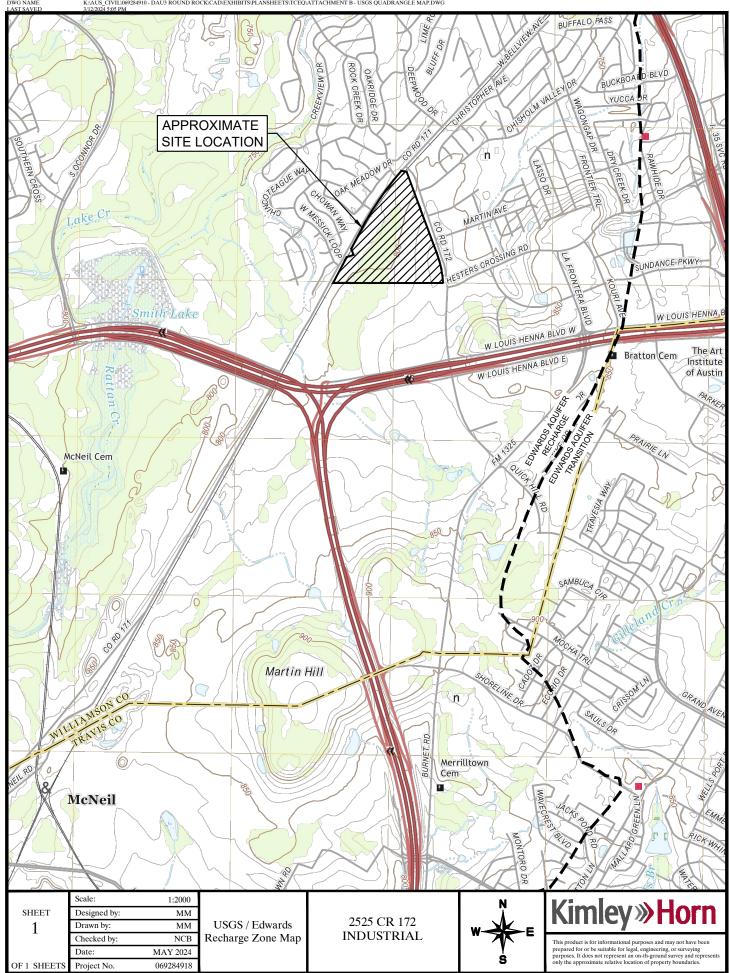


10814 Jollyville Road Campus IV, Suite 200 Austin, TX 78759 512-418-1771

State of Texas Registration No. F-928

# **USGS / EDWARDS RECHARGE ZONE MAP**

MONEGRO, MELISSA 3/12/2024 5:08 PM K:\aug\_cty/tl\069284910 - Dau3 ROUND ROCK\(CAD\)\(EXH\)BITS\(PLANSHEETS\(TCE\)\(TCE\)\(TACHMENT\(B - USG\)SQUADRANGLE MAP.DWG



### PROJECT DESCRIPTION

The proposed 2525 CR 172 Industrial project located at 2525 CR 172 in Round Rock, Texas is located south of the McNeil Rd and County Road 172 intersection within the Round Rock City Limits. The property is currently zoned Planned Unit Development (PUD) 153. The current zoning supports the use of the proposed development. The proposed existing property is approximately 69.56-acres and the site is currently undeveloped. The proposed land use would be classified as Industrial.

The proposed improvements include one (1) story building along with pedestrian improvements, private drives, loading docks, parking, and associated site improvements. Parking will be provided onsite throughout the property and access is provided on CR 172. A WPAP will be submitted separately to address any developments outside of this SCS wastewater improvements. The limits of construction for the wastewater improvements will consist of 1.01 acres out of the 69.56-total site acres.

This project is located within the Lake Creek Watershed. A portion of this site is located in the Federal Emergency Management Agency's 100-year floodplain according to FIRM 48491C0630F. The site is located within the Edwards Aquifer Recharge Zone according to the TCEQ Edward's Aquifer Maps. A Critical Environmental Feature (CEF) is located on the property, however, no wastewater or site improvements are proposed within the CEF or associated setback.

The wastewater extension will serve a proposed industrial building and miscellaneous smaller buildings located on the property.

Demolition activity will be limited to tree removal and site grading.



# SECTION 3: GEOLOGICAL ASSESSMENT FORM

# **Geologic Assessment**

**Texas Commission on Environmental Quality** 

For Regulated Activities on The Edwards Aquifer Recharge/transition Zones and Relating to 30 TAC §213.5(b)(3), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

### Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

Print Name of Geologist: <u>Dave Hill</u>	Telephone: <u>512-832</u>	<u>-8005</u>
Date: February 22, 2024	Fax: <u>512-837-8221</u>	
Representing: ECS Southwest, LLP (Name of	f Company and TBPG or TBPE	Eregistration number)
Signature of Geologist:		8
Katherine Moore TBPG   Regulated Entity Name: 2525 CR 172 Indus   Project Information 1. Date(s) Geologic Assessment was performant was performant.   2. Type of Project:		KATHERINE MOORE *  GEOLOGY 15137 15137 1500000 150000 150000 150000 150000 150000 150000 150000 150000 1500000 150000 150000 150000 150000 150000 150000 150000 150000 1500000 150000 150000 150000 150000 150000 150000 150000 150000 1500000 150000 150000 150000 150000 150000 150000 150000 150000 1500000 150000 150000 150000 150000 150000 150000 150000 150000 1500000 150000 150000 150000 150000 150000 150000 150000 150000 1500000 150000 150000 150000 150000 150000 150000 150000 150000 1500000 150000 150000 150000 150000 150000 150000 150000 150000 1500000 1500000 1500000 1500000 1500000 1500000 1500000 1500000 15000000 15000000 15000000 150000000 1500000000
Contributing Zone within the Transit	ion Zone	

┿.			)585-Table) is atta	•	a deologic Assessment Table
5.	Hyd 55,	drologic So Appendix	oil Groups* (Urbar A, Soil Conservati	n Hydrology for Small W ion Service, 1986). If the	e below and uses the SCS atersheds, Technical Release No. ere is more than one soil type on the sic Map or a separate soils map.
Cł			s, Infiltration d Thickness  Thickness(feet)	<i>A.</i>	Group Definitions (Abbreviated) Soils having a high infiltration rate when thoroughly wetted.
_		Стопр		В.	Soils having a moderate infiltration rate when thoroughly wetted.
				С.	Soils having a slow infiltration
				D.	rate when thoroughly wetted. Soils having a very slow infiltration rate when thoroughly wetted.
6. 7.	me top the Attaincl pot	mbers, an of the str stratigrap achment ( uding any ential for	d thicknesses is at atigraphic column thic column. C – <b>Site Geology</b> . A features identifie	ttached. The outcroppin  Otherwise, the upper  A narrative description of  d in the Geologic Assess  o the Edwards Aquifer, s	column showing formations, g unit, if present, should be at the most unit should be at the top of of the site specific geology sment Table, a discussion of the stratigraphy, structure(s), and
8.			_	Map(s). The Site Geolog ninimum scale is 1": 400	ic Map must be the same scale as  "
			te Plan Scale: 1" = Map Scale: 1" = _ Scale (if more th	=' nan 1 soil type): 1" =	
9.	Metho	d of collec	ting positional da	ta:	
			oning System (GPS d(s). Please descri	6) technology. ibe method of data colle	ction:
10	). 🔀 The	project si	te and boundarie	s are clearly shown and	labeled on the Site Geologic Map.
11	🔀 Sur	face geolo	gic units are show	vn and labeled on the Si	te Geologic Map.

12. [≥	Geologic or manmade features were discovered on the project site during the field investigation. They are shown and labeled on the Site Geologic Map and are described in the attached Geologic Assessment Table.
	Geologic or manmade features were not discovered on the project site during the field investigation.
13. 🛭	The Recharge Zone boundary is shown and labeled, if appropriate.
	Il known wells (test holes, water, oil, unplugged, capped and/or abandoned, etc.): If pplicable, the information must agree with Item No. 20 of the WPAP Application Section.
	There are 1 (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply.)  The wells are not in use and have been properly abandoned.  The wells are not in use and will be properly abandoned.  The wells are in use and comply with 16 TAC Chapter 76.  There are no wells or test holes of any kind known to exist on the project site.

### **Administrative Information**

15. Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

# **GEOLOGICAL ASSESSMENT TABLE**

GEOLOGI	C ASSESS	MENT TAE	BLE				PR	OJE	CT NA	ME	51-39	92 McN	eil and	I CR 107						
LOCATION					FEATURE CHARACTERISTICS								EVALUATION		PHYSICAL		_ SETTING			
1A	1B *	1C*	2A	2B	3		4		5	5A	6	7	8A	8B	9		10	1	11	12
FEATURE ID	LATITUDE	LONGITUDE	FEATURE TYPE	POINTS	FORMATION	DIME	NSIONS (	FEET)	TREND (DEGREES)	DOM	DENSITY (NO/FT)	APERTURE (FEET)	INFILL	RELATIVE INFILTRATION RATE	TOTAL	SENS	ITIVITY		ENT AREA RES)	TOPOGRAPHY
						Х	Υ	Z		10						<40	<u>&gt;40</u>	<1.6	<u>&gt;1.6</u>	
Feature 1	30.478333	-97.704167	0	5		40	590	5					N	15	20	Χ				Drainage
Feature 2	30.478333	-97.701944	0	5		1200	70	3					0	15	20	Χ				Drainage
Feature 3	30.480833	-97.702778	0	5		35	73	2					N	15	20	Χ				Drainage
Feature 4	30.291	97.4151	SC	20		4	7	6					0	10	30	Χ				Hilltop
Feature 5	30.291	97.4152	SC	20		14	9	6					0	10	30	Χ				Hilltop
Water Well	30.2916	97.4151	МВ	30		Χ	Х	Χ					N	5	35	Χ				

DATIM.	

2A TYPE	TYPE	2B POINT
С	Cave	3
sc	Solution cavity	2
SF	Solution-enlarged fracture(s)	2
F	Fault	2
0	Other natural bedrock features	
МВ	Manmade feature in bedrock	3
sw	Swallow hole	3
SH	Sinkhole	2
CD	Non-karst closed depression	
Z	Zone, clustered or aligned features	3

	8A INFILLING
N	None, exposed bedrock
С	Coarse - cobbles, breakdown, sand, gravel
0	Loose or soft mud or soil, organics, leaves, sticks, dark colors
F	Fines, compacted clay-rich sediment, soil profile, gray or red colors
V	Vegetation. Give details in narrative description
FS	Flowstone, cements, cave deposits
х	Other materials

12 TOPOGRAPHY	
Cliff, Hilltop, Hillside, Drainage, Floodplain,	Streambed

I have read, I understood, and I have followed the Texas Commission on Environmental Quality's Instructions to Geologists. The information presented here complies with that document and is a true representation of the conditions observed in the field.

My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

\_\_\_\_\_\_ Date: February 22, 2024

Sheet \_\_1\_\_\_ of \_\_\_\_\_

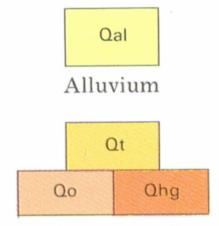
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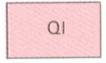
# STRATIGRAPHIC COLUMN

## Stratigraphic Column McNeil and CR 107 Roundrock, Williamson County, Texas





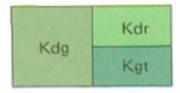
Fluviatile terrace deposits



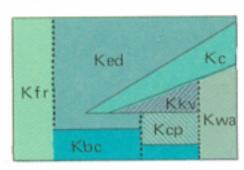
Lissie Formation undivided



Eagle Ford Group and Buda Limestone



Del Rio Clay ("Grayson Marl") and Georgetown Formation



Fredericksburg Group

# **SITE GEOLOGY**

### **GEOLOGIC ASSESSMENT**



PROPOSED AUSTIN ROUND ROCK DEVLOPMENT SOUTH CORNER OF CR 172 & MCNEIL ROAD ROUND ROCK, WILLIAMSON COUNTY, TEXAS 78759

ECS PROJECT NO. 51:3992

**FOR** 

**KIMLEY-HORN** 

MARCH 6, 2024





Geotechnical • Construction Materials • Environmental • Facilities

March 6, 2024

Mr.Nick Brown Kimley-Horn 10814 Jollyville Road, Avallon IV Austin, Texas, 78759

ECS Project No. 51:3992

Reference: Geologic Assessment Report, Proposed Austin Round Rock Devlopment, South Corner of CR 172 & McNeil Road, Round Rock, Williamson County Texas

Dear Mr. Brown:

ECS Southwest, LLP (ECS) is pleased to provide Kimley-Horn with a Geologic Assessment for the above referenced property. ECS' services were conducted in accordance with the services outlined in ECS Proposal No. 51:5593 authorized on February 13, 2024

ECS did not observe recharge features on the site. ECS would like to thank Kimley-Horn for the opportunity to provide you with this Geologic Assessment. We look forward to assisting you further with this project and other environmental concerns you may have. If you have any questions, please feel free to contact us at any time at 512-837-8005.

Sincerely,

ECS Southwest, LLP

John Jones Assistant Staff Project Manager

jjones5@ecslimited.com 512-837-8005

Katherine Moore, P.G. Geologic Assessment klmoore@ecslimited.com

512-837-8005

#### **1.0 INTRODUCTION**

The geologic assessment provided here, as part of the applicant's plan, addresses the required items as cited in Title 30 of the Texas Administrative Code (TAC), Part 1, Chapter 213, Subchapter A, Rule 213.5, relating to development over the Edwards Aquifer This report identifies observed potential pathways for contaminant movement into the underlying Edwards Aquifer as required by the Texas Commission on Environmental Quality (TCEQ).

The subject property is comprised of approximately 193.289 acres of undeveloped land. The subject property is identified by the Williamson County Central Appraisal District as portions of parcels R055545, located at South Corner of CR 172 & McNeil Road in Round Rock, Texas. The subject property is located over the Edwards Aquifer Recharge Zone.

The purpose of this Geologic Assessment is to fulfill the requirements for the applicant's plan for site improvements on the property. This report will describe surficial geologic units and identify the locations and extent of significant features that may impact the underlying Edwards Aquifer Recharge Zone.



### 2.0 SOIL UNITS

According to the United States Department of Agriculture (USDA) Soil Survey of Williamson County, Texas, there are nine (9) soil units mapped on the site (Figure 4). The soils on site consist of: Denton silty clay, 1 to 3 percent slopes, (DcB), Denton silty clay, 3 to 5 percent slopes, (DnC), Doss silty clay, moist, 1 to 5 percent slopes, (DoC), Eckrant cobbly clay, 1 to 8 percent slopes, (EaD), Eckrant stony clay, 0 to 3 percent slopes, stony, (EeB), Fairlie clay, 0 to 1 percent slopes, (FaA), Fairlie clay, 1 to 2 percent slopes, (FaB), Georgetown stony clay loam, 1 to 3 percent slopes, (GsB), and Heiden clay, 2 to 5 percent slopes, moderately eroded, (HedC2).

Denton silty clay, 1 to 3 percent slopes (DcB), and Denton silty clay, 3 to 5% slopes (DnC), The Hydrologic Soil Group is listed as D, The Denton series consists of deep, well-drained, slowly permeable soils that formed in clayey materials over residuum weathered from limestone bedrock of lower Cretaceous age (USDA, 2023). Flooding or ponding is reported as "none." The depth to a restrictive layer is reported to be 22 to 60 inches to lithic bedrock, and the available water supply is listed as low, 0 to 60 inches.

Doss silty clay, moist, 1 to 5 percent slopes, (DoC). The Hydrologic Soil Group is listed as D, The Doss series consists of shallow to weakly cemented limestone, well drained, moderately slow permeable soils that formed in calcareous loamy and clayey residuum derived from marls and limestone. (USDA, 2023). Flooding or ponding is reported as "none." The depth to a restrictive layer is reported to be 11 to 20 inches to lithic bedrock, and the available water supply is listed as very low, 0 to 60 inches.

Eckrant cobbly clay, 1 to 8 percent slopes, (EaD) and Eckrant cobbly clay, 0 to 3 percent slopes, stony (EeB), is formed on ridges derived from residuum weathered from limestone, very stony, (USDA, 2023). The Hydrologic Soil Group is listed as D, and the soil is well drained. Flooding or ponding is reported as "none." The depth to a restrictive layer is reported to be 4 to 20 inches to lithic bedrock, and the available water supply is listed as very low, 0 to 60 inches.

Fairlie clay, 0 to 1 percent slopes, (FaA), and Fairlie clay, 1 to 3 percent slopes (FaB). The Fairlie series consists of deep, moderately well-drained, very slowly permeable soils (USDA, 2023). The Hydrologic Soil Group is listed as D, and the soil is well drained. Flooding or ponding is reported as "none." The depth to a restrictive layer is reported to be 40 to 60 inches to paralithic bedrock, and the available water supply is listed as moderate, 0 to 60 inches.

Georgetown stony clay loam, 1 to 3 percent slopes, (GsB), The Georgetown series consists of moderately deep, well drained, very slowly permeable soils that have formed over indurated limestone of Cretaceous age (USDA, 2023). The Hydrologic Soil Group is listed as D, and the soil is well drained. Flooding or ponding is reported as "none." The depth to a restrictive layer is reported to be 20 to 40 inches to lithic bedrock, and the available water supply is listed as low, 0 to 60 inches.

Heiden clay, 2 to 5 percent slopes, moderately eroded, (HedC2), The Heiden series consists of deep and very deep to mudstone, well drained, very slowly permeable soils that formed in clayey residuum weathered from mudstone (USDA, 2023). The Hydrologic Soil Group is listed as D, and the soil is well drained. Flooding or ponding is reported as "none." The depth to a restrictive layer is reported to be 40 to 65 inches to densic material, and the available water supply is listed as very high, 0 to 60 inches



#### 3.0 GEOLOGY

### 3.1 Regional Geology

Ranging from north to south, two primary physiographic provinces are present in Williamson County: the Great Plain and the Gulf Coastal Plain. The Gulf Coastal Plain is comprised mainly of Blackland prairie.

The Great Plain is comprised chiefly of limestone plains, which locally merges with the Edwards Plateau in the vicinity of the Colorado River.

Groundwater recharge and flow are controlled by faulted Edwards Aquifer and adjacent strata. Water enters the aquifer by means of solution features controlled by faults, fractures and solution conduits. Solution features are created by the dissolution of limestone primarily from rainwater and groundwater. Deformation of the Balcones fault system controls both the large- and small-scale flow barriers and pathways present in the Edwards Aquifer.

### 3.2 Site Geology

Geological information pertaining to the area was obtained from the Geologic Atlas of Texas, Austin Sheet, published by University of Texas at Austin, Bureau of Economic Geology (BEG), 1997. The subject property is situated on Del Rio (Kdr) (Figure 6).

BEG describes Kdr as "calcareous and gypsiferous becoming less calcareous and more gypsiferous upward, pyrite common, blocky medium gray, some thin lenticular beds of highly calcareous siltstone; marine megafossils include abundant Exogyra arietina and other pelecypods; thickness 40-70 feet"

Obvious signs of faulting were not mapped or observed on the subject property.



### **4.0 HYDROLOGY**

### 4.1 Site Hydrology

Based upon interpretation of the United States Geological Survey 7.5 Minute Series topographic quadrangle map, Pflugerville West, Texas Quadrangle (2022), and the onsite reconnaissance, the estimated onsite shallow groundwater flow direction is south to an unnamed tributary of Lake Creek, and Ratten Creek Figure 2). It should be noted that shallow groundwater flow direction is estimated based on a review of published maps, surface topography, and site reconnaissance. Local conditions that may influence the subsurface hydrology would be local topography (hills and valleys), geologic anomalies, utilities, and nearby wells or sumps.

### 4.2 Surface Water Hydrology

Site drainage appears to slope generally to the southeast from Lake Creek, and Ratten Creek towards unknown tributary (Figure 2). Three (3) drainage features were observed at the subject site.



#### **5.0 SITE INVESTIGATION**

The site reconnaissance was performed on February 22, 2024. The site investigation was performed by traversing the subject property in meandering transects, spaced 10 to 15 meters apart depending on thickness of vegetation. Photographs were taken to document any features observed during the reconnaissance.

The subject property is currently undeveloped land. Five (5) potential site recharge features (S-1, S-2, S-3, S-4, and S-5) were observed on the subject property. S-1 and S-2 were near the southwest boundary, S-3 was located slightly north off the southern boundary, and S-4 and S-5 were located on the northern part of the subject property. These are marked on the Site Geologic Map (Figure 6C). S-1, S-2, and S-3 appeared to be natural drainage, S-1 and S-2 are karst like features. One (1) water well was observed on the northern part of the subject property. These features are described in further detail in the Geologic Assessment Table.

Other potential natural recharge features such as caves, faults, or lineaments were not observed on the subject property.



#### **6.0 SUMMARY**

The subject property is comprised of approximately 193.289 acres of developed land. The subject property is identified by the Williamson County Central Appraisal District as Parcel R055545, located at South Corner of CR 172 & McNeil Road in Round Rock, Texas. The subject property is located over the Edwards Aquifer Recharge Zone.

The subject property is currently undeveloped land. Five (5) potential site recharge features (S-1, S-2, S-3, S-4, and S-5) were observed on the subject property. S-1 and S-2 were near the southwest boundary, S-3 was located slightly north off the southern boundary, and S-4 and S-5 were located on the northern part of the subject property. These are marked on the Site Geologic Map (Figure 6C). S-1, S-2, and S-3 appeared to be natural drainage, S-1 and S-2 are karst like features. One (1) water well was observed on the northern part of the subject property. These features are described in further detail in the Geologic Assessment Table, and in the Site Narrative.

Caves, were not observed on the subject property at the time of the site reconnaissance.



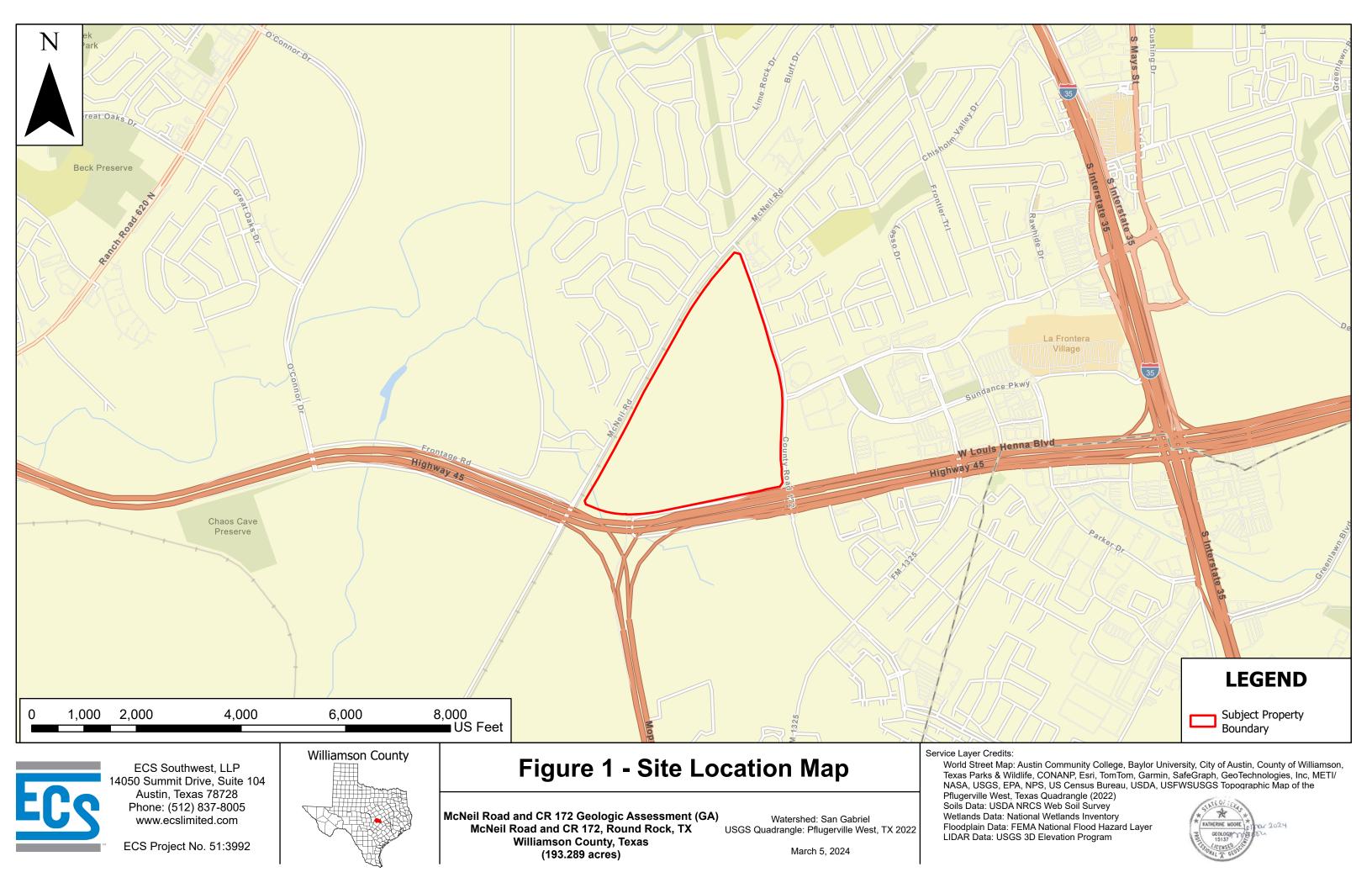
### **7.0 REFERENCES**

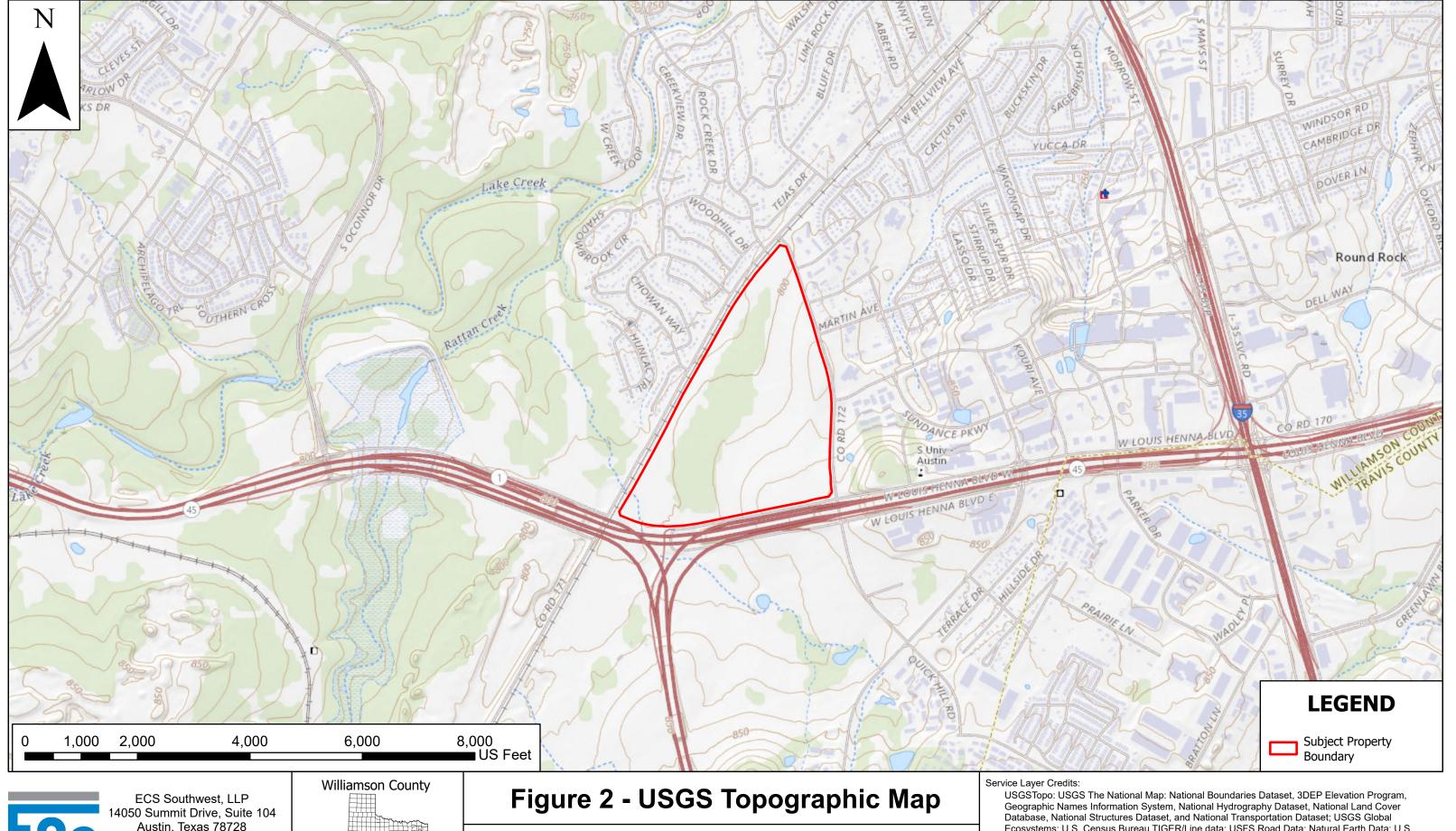
(BEG) The University of Texas at Austin Bureau of Economic Geology, Geologic Map of Texas, Austin Sheet, 1997.

(USDA) United States Department of Agriculture (USDA) Custom Soil Survey of Williamson County, 2022.

(USGS) United States Geologic Survey (USGS), 7.5- Minute Topographic Quadrangle Georgetown, Texas. 2019.









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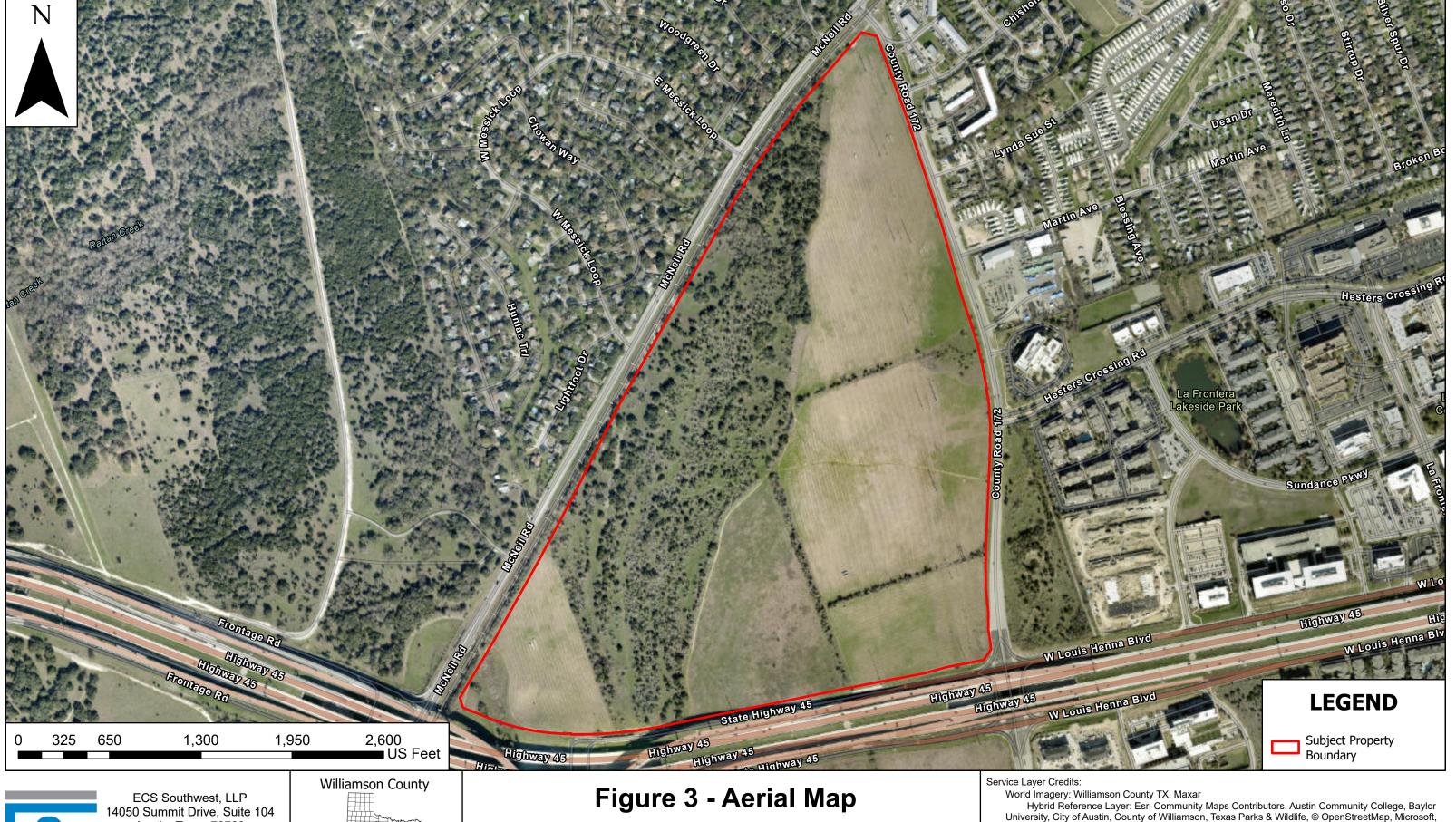
McNeil Road and CR 172 Geologic Assessment (GA) McNeil Road and CR 172, Round Rock, TX Williamson County, Texas (193.289 acres)

Watershed: San Gabriel USGS Quadrangle: Pflugerville West, TX 2022

March 5, 2024

Database, National Structures Dataset, and National Transportation Dataset; USGS Global Ecosystems; U.S. Census Bureau TIGER/Line data; USFS Road Data; Natural Earth Data; U.S. Department of State Humanitarian Information Unit; and NOAA National Centers for Environmental Information, U.S. Coastal Relief Model. Data refreshed April, 2023.USGS Topographic Map of the

Pflugerville West, Texas Quadrangle (2022) Soils Data: USDA NRCS Web Soil Survey Wetlands Data: National Wetlands Inventory Floodplain Data: FEMA National Flood Hazard Layer LIDAR Data: USGS 3D Elevation Program





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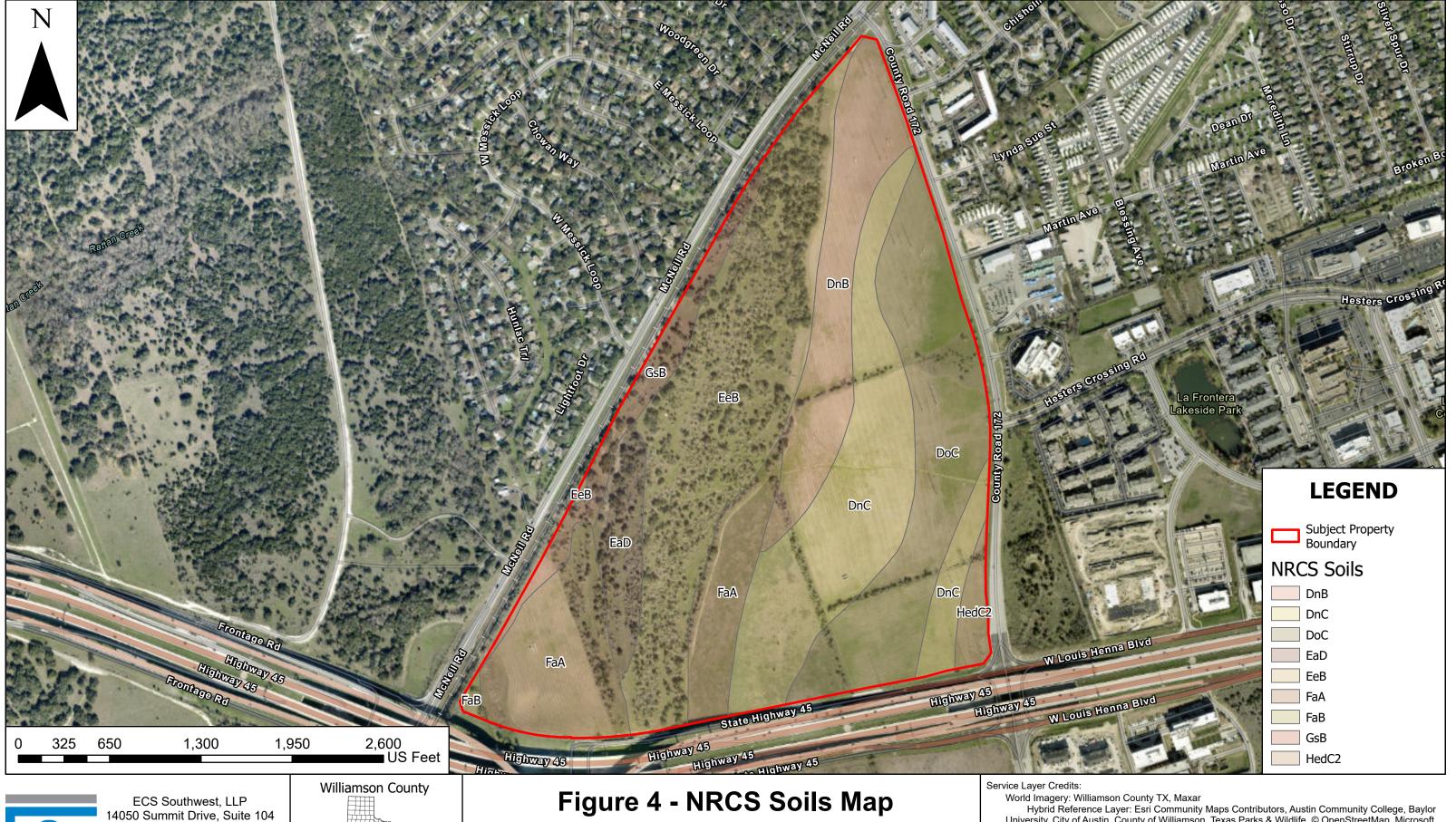
Watershed: San Gabriel
USGS Quadrangle: Pflugerville West, TX 2022

March 5, 2024

Hybrid Reference Layer: Esri Community Maps Contributors, Austin Community College, Baylor University, City of Austin, County of Williamson, Texas Parks & Wildlife, © OpenStreetMap, Microsoft, CONANP, Esri, TomTom, Garmin, SafeGraph, GeoTechnologies, Inc, METI/NASA, USGS, EPA, NPS, US Census Bureau, USDA, USFWSUSGS Topographic Map of the Pflugerville West, Texas Quadrangle (2022)
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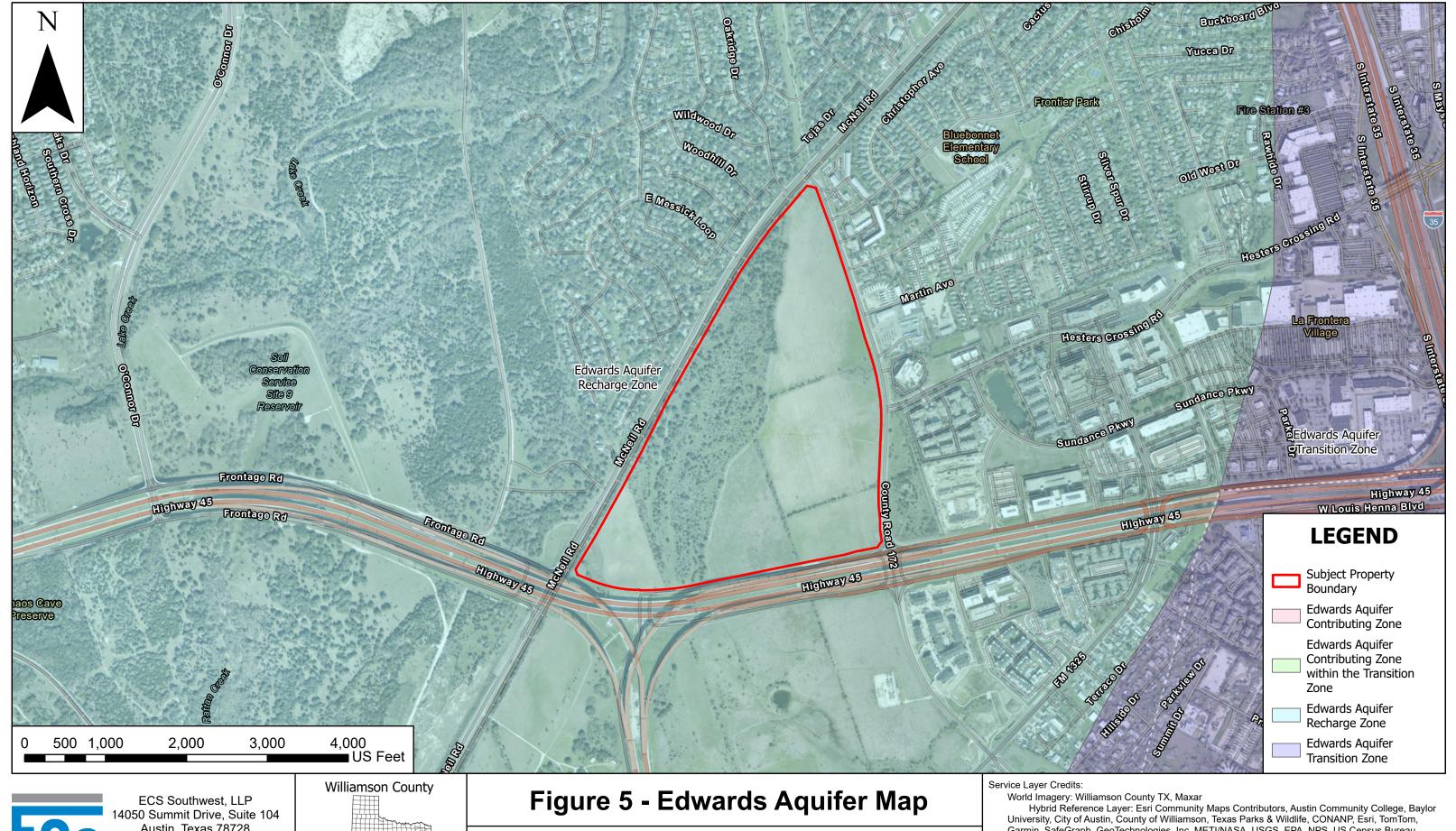
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March 5, 2024

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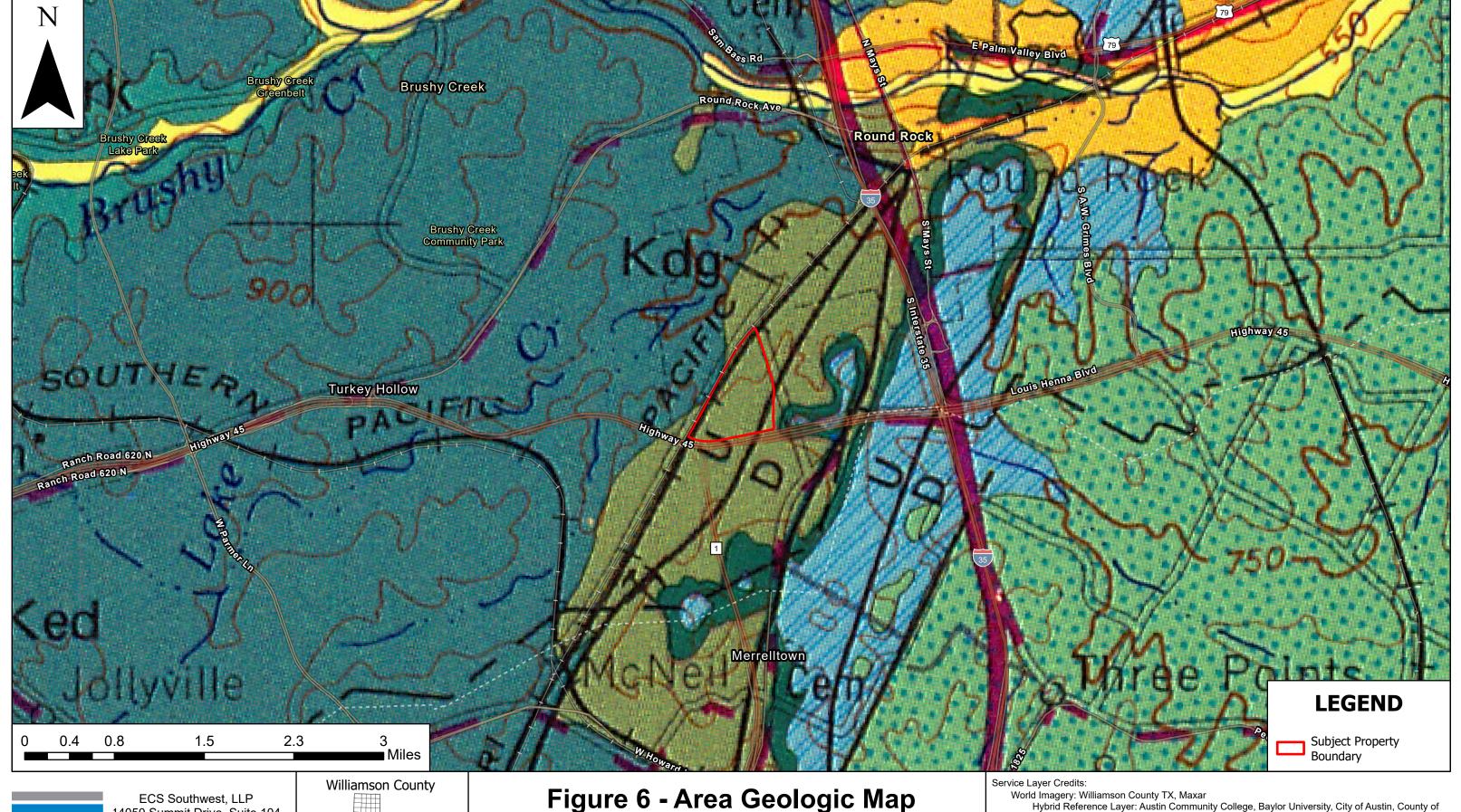
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# Figure 6 - Area Geologic Map

McNeil Road and CR 172 Geologic Assessment (GA) McNeil Road and CR 172, Round Rock, TX Williamson County, Texas (193.289 acres)

Watershed: San Gabriel USGS Quadrangle: Pflugerville West, TX 2022

March 5, 2024

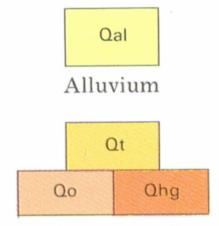
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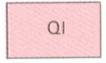


# Stratigraphic Column McNeil and CR 107 Roundrock, Williamson County, Texas





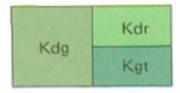
Fluviatile terrace deposits



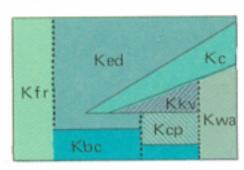
Lissie Formation undivided



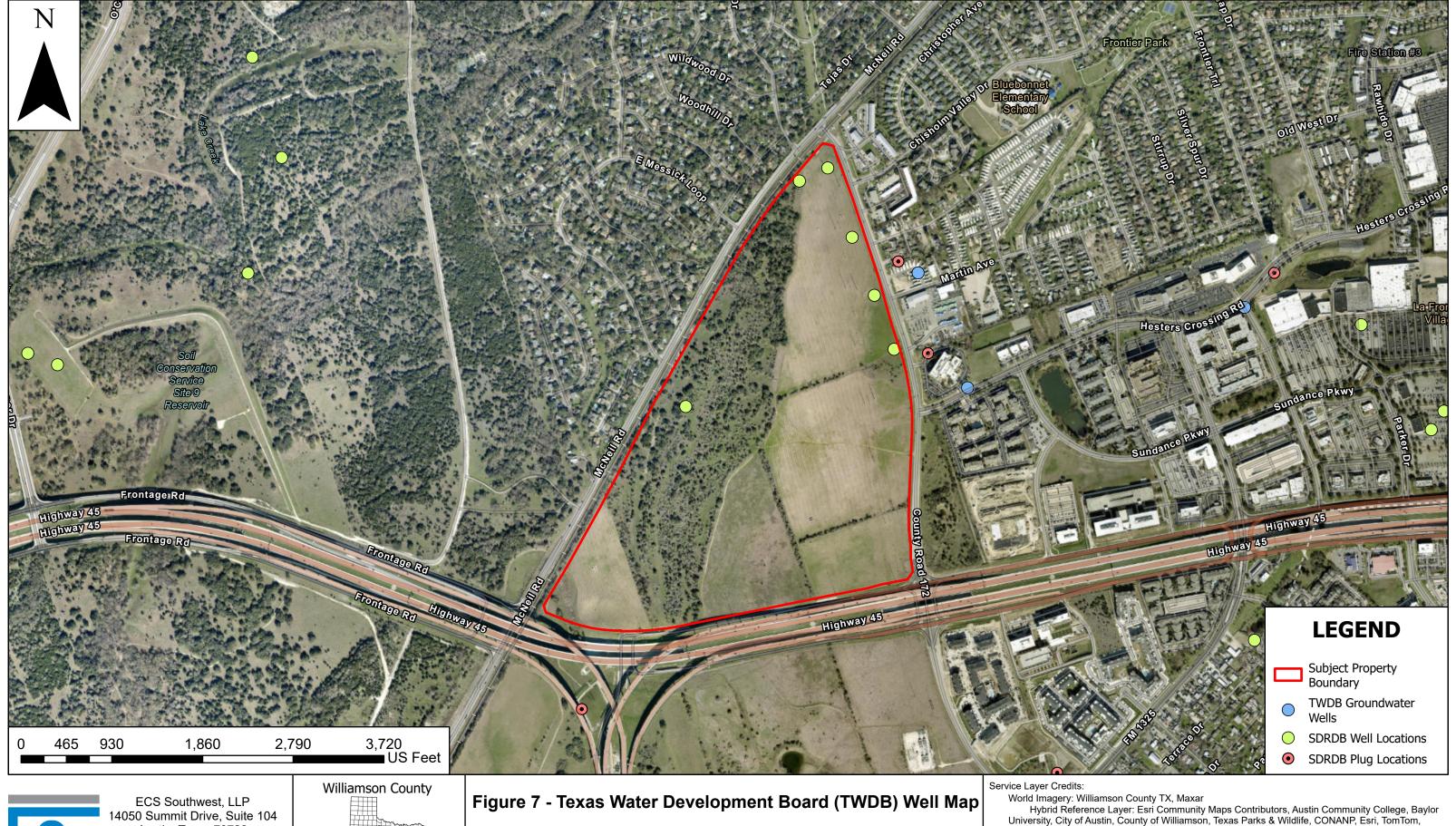
Eagle Ford Group and Buda Limestone



Del Rio Clay ("Grayson Marl") and Georgetown Formation



Fredericksburg Group





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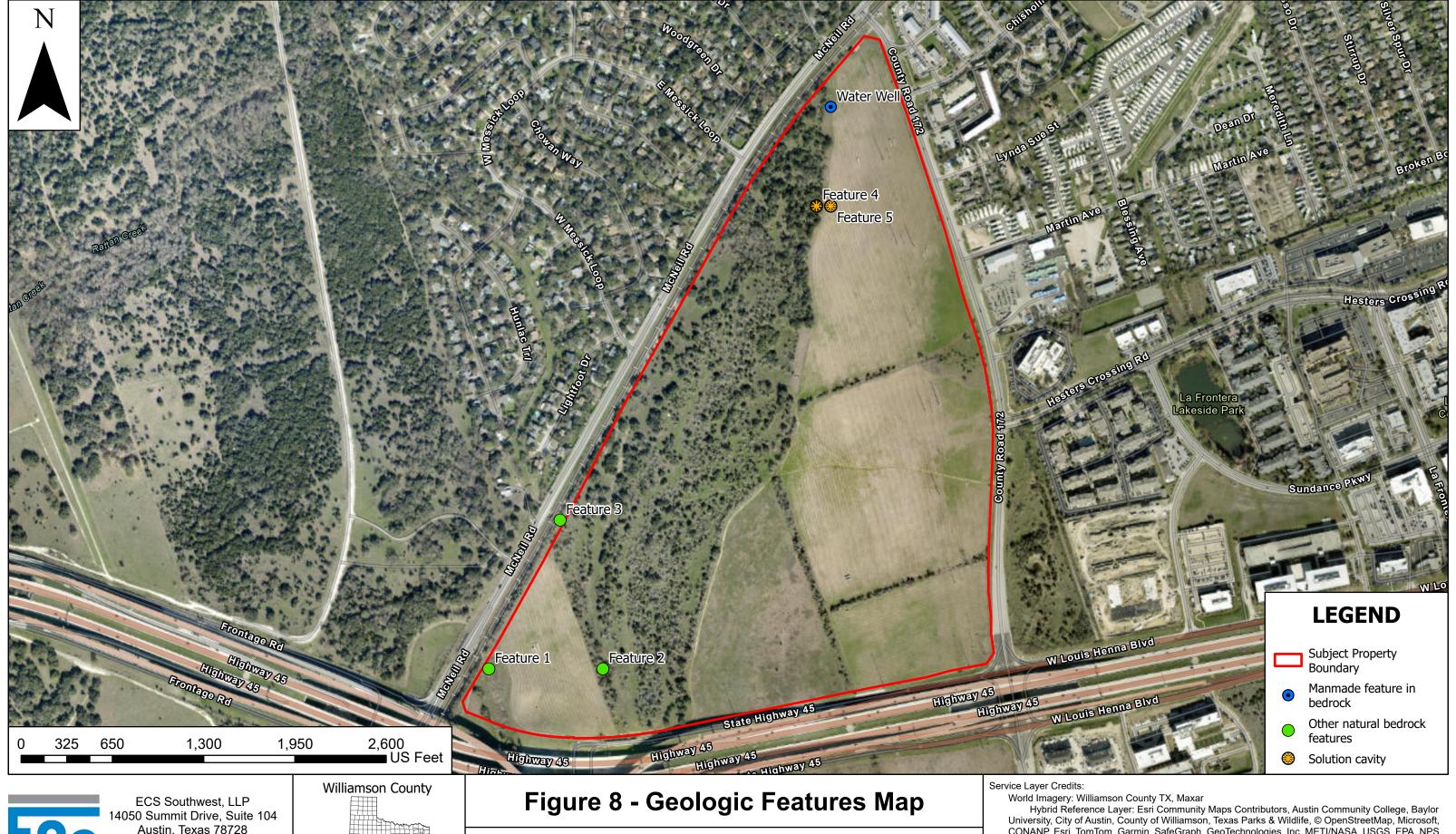
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McNeil Road and CR 172 Geologic Assessment (GA) McNeil Road and CR 172, Round Rock, TX Williamson County, Texas (193.289 acres)

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University, City of Austin, County of Williamson, Texas Parks & Wildlife, © OpenStreetMap, Microsoft, CONANP, Esri, TomTom, Garmin, SafeGraph, GeoTechnologies, Inc, METI/NASA, USGS, EPA, NPS, US Census Bureau, USDA, USFWSUSGS Topographic Map of the Pflugerville West, Texas Quadrangle (2022)

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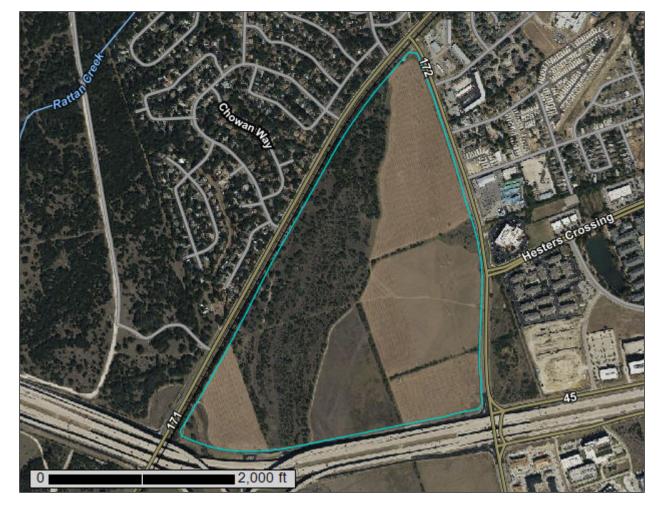




**NRCS** 

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

# Custom Soil Resource Report for Williamson County, Texas



# **Preface**

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

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# **How Soil Surveys Are Made**

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



#### MAP LEGEND

#### Area of Interest (AOI)

Area of Interest (AOI)

#### Soils

Soil Map Unit Polygons

-

Soil Map Unit Lines

Soil Map Unit Points

#### Special Point Features

(0)

Blowout

 $\boxtimes$ 

Borrow Pit

Ж

Clay Spot

 $\Diamond$ 

**Closed Depression** 

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Gravel Pit

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Gravelly Spot

0

Landfill Lava Flow

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Marsh or swamp

2

Mine or Quarry

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Miscellaneous Water

Perennial Water

0

Rock Outcrop

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Saline Spot

0.0

Sandy Spot

Severely Eroded Spot

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Sinkhole

3>

Slide or Slip

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Sodic Spot

#### **U**\_..\_

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Spoil Area Stony Spot

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Very Stony Spot

3

Wet Spot Other

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Special Line Features

#### Water Features

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Streams and Canals

#### Transportation

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Rails

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Interstate Highways

US Routes

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Major Roads Local Roads

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#### **Background**

Marie Contract

Aerial Photography

#### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Williamson County, Texas Survey Area Data: Version 24, Sep 5, 2023

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Data not available.

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
DnB	Denton silty clay, 1 to 3 percent slopes	20.5	11.2%	
DnC	Denton silty clay, 3 to 5 percent slopes	36.0	19.6%	
DoC	Doss silty clay, moist, 1 to 5 percent slopes	32.3	17.7%	
EaD	Eckrant cobbly clay, 1 to 8 percent slopes	7.5	4.1%	
EeB	Eckrant stony clay, 0 to 3 percent slopes, stony	47.9	26.2%	
FaA	Fairlie clay, 0 to 1 percent slopes	27.0	14.7%	
FaB	Fairlie clay, 1 to 2 percent slopes	0.6	0.3%	
GsB	Georgetown stony clay loam, 1 to 3 percent slopes	8.6	4.7%	
HedC2	Heiden clay, 2 to 5 percent slopes, moderately eroded	2.8	1.5%	
Totals for Area of Interest		183.1	100.0%	

# **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They

generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

# Williamson County, Texas

# DnB—Denton silty clay, 1 to 3 percent slopes

#### **Map Unit Setting**

National map unit symbol: 2t26l Elevation: 570 to 1,870 feet

Mean annual precipitation: 31 to 36 inches Mean annual air temperature: 65 to 68 degrees F

Frost-free period: 220 to 260 days

Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

Denton and similar soils: 88 percent Minor components: 12 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Denton**

#### Setting

Landform: Hillslopes

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope

Down-slope shape: Convex Across-slope shape: Linear

Parent material: Silty and clayey slope alluvium over residuum weathered from

limestone

#### Typical profile

A - 0 to 14 inches: silty clay Bw - 14 to 25 inches: silty clay Bk - 25 to 33 inches: silty clay

Ck - 33 to 36 inches: gravelly silty clay

R - 36 to 80 inches: bedrock

#### Properties and qualities

Slope: 1 to 3 percent

Depth to restrictive feature: 22 to 60 inches to lithic bedrock

Drainage class: Well drained

Runoff class: High

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.20 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 80 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Low (about 4.3 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3s

Hydrologic Soil Group: D

Ecological site: R081CY357TX - Clay Loam 29-35 PZ

Hydric soil rating: No

#### **Minor Components**

#### Krum

Percent of map unit: 6 percent Landform: Drainageways

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Concave

Ecological site: R081CY357TX - Clay Loam 29-35 PZ

Hydric soil rating: No

#### **Doss**

Percent of map unit: 4 percent

Landform: Hillslopes

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Linear

Ecological site: R081BY343TX - Shallow 23-31 PZ

Hydric soil rating: No

#### Anhalt

Percent of map unit: 2 percent

Landform: Hillslopes

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope

Down-slope shape: Linear Across-slope shape: Linear

Ecological site: R081CY358TX - Deep Redland 29-35 PZ

Hydric soil rating: No

# DnC—Denton silty clay, 3 to 5 percent slopes

#### Map Unit Setting

National map unit symbol: 2t26r Elevation: 570 to 1,870 feet

Mean annual precipitation: 31 to 36 inches
Mean annual air temperature: 65 to 68 degrees F

Frost-free period: 220 to 260 days

Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

Denton and similar soils: 88 percent Minor components: 12 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Denton**

#### Setting

Landform: Hillslopes

Landform position (two-dimensional): Backslope, footslope

Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Linear

Parent material: Silty and clayey slope alluvium over residuum weathered from

limestone

#### Typical profile

A - 0 to 14 inches: silty clay Bw - 14 to 25 inches: silty clay Bk - 25 to 33 inches: silty clay

Ck - 33 to 36 inches: gravelly silty clay

R - 36 to 80 inches: bedrock

#### **Properties and qualities**

Slope: 3 to 5 percent

Depth to restrictive feature: 22 to 60 inches to lithic bedrock

Drainage class: Well drained

Runoff class: High

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.20 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 80 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Low (about 4.3 inches)

#### Interpretive groups

Land capability classification (irrigated): 3s Land capability classification (nonirrigated): 3s

Hydrologic Soil Group: D

Ecological site: R081CY357TX - Clay Loam 29-35 PZ

Hydric soil rating: No

#### **Minor Components**

#### **Brackett**

Percent of map unit: 6 percent

Landform: Hillslopes

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R081CY355TX - Adobe 29-35 PZ

Hydric soil rating: No

#### **Doss**

Percent of map unit: 4 percent

Landform: Hillslopes

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Linear

Ecological site: R081CY574TX - Shallow 29-35 PZ

Hydric soil rating: No

#### **Purves**

Percent of map unit: 2 percent

Landform: Hillslopes

Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Linear

Ecological site: R081CY574TX - Shallow 29-35 PZ

Hydric soil rating: No

## DoC—Doss silty clay, moist, 1 to 5 percent slopes

#### **Map Unit Setting**

National map unit symbol: 2s0st Elevation: 630 to 1,840 feet

Mean annual precipitation: 30 to 36 inches Mean annual air temperature: 66 to 68 degrees F

Frost-free period: 210 to 240 days

Farmland classification: Not prime farmland

#### **Map Unit Composition**

Doss and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Doss**

#### Setting

Landform: Hillslopes

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Linear

Parent material: Residuum weathered from limestone

#### Typical profile

A - 0 to 9 inches: silty clay Bk - 9 to 17 inches: silty clay Cr - 17 to 80 inches: bedrock

#### **Properties and qualities**

Slope: 1 to 5 percent

Depth to restrictive feature: 11 to 20 inches to paralithic bedrock

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.57 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 70 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm) Available water supply, 0 to 60 inches: Very low (about 2.4 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4e

Hydrologic Soil Group: D

Ecological site: R081CY574TX - Shallow 29-35 PZ

Hydric soil rating: No

#### **Minor Components**

#### **Brackett**

Percent of map unit: 7 percent

Landform: Ridges

Landform position (two-dimensional): Shoulder, backslope, footslope

Landform position (three-dimensional): Side slope

Down-slope shape: Linear Across-slope shape: Convex

Ecological site: R081CY362TX - Steep Adobe 29-35 PZ

Hydric soil rating: No

#### Bolar

Percent of map unit: 5 percent

Landform: Ridges

Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R081CY357TX - Clay Loam 29-35 PZ

Hydric soil rating: No

#### Denton

Percent of map unit: 1 percent

Landform: Plains

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope

Down-slope shape: Linear Across-slope shape: Linear

Ecological site: R081CY357TX - Clay Loam 29-35 PZ

Hydric soil rating: No

#### **Eckrant**

Percent of map unit: 1 percent

Landform: Ridges

Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R081CY360TX - Low Stony Hill 29-35 PZ

Hydric soil rating: No

#### **Purves**

Percent of map unit: 1 percent

Landform: Plains

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Linear

Ecological site: R081CY574TX - Shallow 29-35 PZ

Hydric soil rating: No

## EaD—Eckrant cobbly clay, 1 to 8 percent slopes

#### **Map Unit Setting**

National map unit symbol: 2t0sg Elevation: 650 to 1,900 feet

Mean annual precipitation: 30 to 35 inches
Mean annual air temperature: 65 to 69 degrees F

Frost-free period: 210 to 250 days

Farmland classification: Not prime farmland

#### **Map Unit Composition**

Eckrant and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Eckrant**

#### Setting

Landform: Ridges

Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Interfluve, side slope

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Residuum weathered from limestone

#### Typical profile

A1 - 0 to 4 inches: cobbly clay
A2 - 4 to 11 inches: very cobbly clay

R - 11 to 80 inches: bedrock

#### **Properties and qualities**

Slope: 1 to 8 percent

Surface area covered with cobbles, stones or boulders: 2.3 percent

Depth to restrictive feature: 4 to 20 inches to lithic bedrock

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.57 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Calcium carbonate, maximum content: 10 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Sodium adsorption ratio, maximum: 1.0

Available water supply, 0 to 60 inches: Very low (about 1.0 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: D

Ecological site: R081CY360TX - Low Stony Hill 29-35 PZ

Hydric soil rating: No

#### **Minor Components**

#### **Brackett**

Percent of map unit: 7 percent

Landform: Ridges

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R081CY355TX - Adobe 29-35 PZ

Hydric soil rating: No

#### **Bexar**

Percent of map unit: 5 percent

Landform: Ridges

Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope

Down-slope shape: Linear Across-slope shape: Linear

Ecological site: R081CY361TX - Redland 29-35 PZ

Hydric soil rating: No

#### Krum

Percent of map unit: 3 percent

Landform: Ridges

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope

Down-slope shape: Linear Across-slope shape: Linear

Ecological site: R081CY357TX - Clay Loam 29-35 PZ

Hydric soil rating: No

# EeB—Eckrant stony clay, 0 to 3 percent slopes, stony

#### Map Unit Setting

National map unit symbol: djpv Elevation: 650 to 1,320 feet

Mean annual precipitation: 30 to 35 inches

Mean annual air temperature: 65 to 69 degrees F

Frost-free period: 210 to 250 days

Farmland classification: Not prime farmland

#### **Map Unit Composition**

Eckrant, stony, and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Eckrant, Stony**

#### Setting

Landform: Ridges

Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Residuum weathered from limestone

#### Typical profile

A1 - 0 to 4 inches: stony clay

A2 - 4 to 11 inches: extremely stony clay

R - 11 to 80 inches: bedrock

#### Properties and qualities

Slope: 0 to 3 percent

Surface area covered with cobbles, stones or boulders: 0.0 percent

Depth to restrictive feature: 4 to 20 inches to lithic bedrock

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.57 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 10 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Sodium adsorption ratio, maximum: 1.0

Available water supply, 0 to 60 inches: Very low (about 0.8 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: D

Ecological site: R081CY360TX - Low Stony Hill 29-35 PZ

Hydric soil rating: No

#### **Minor Components**

#### Georgetown

Percent of map unit: 8 percent

Landform: Ridges

Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve

Down-slope shape: Linear Across-slope shape: Linear

Ecological site: R081CY361TX - Redland 29-35 PZ

Hydric soil rating: No

#### **Doss**

Percent of map unit: 7 percent

Landform: Ridges

Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R081CY355TX - Adobe 29-35 PZ

Hydric soil rating: No

## FaA—Fairlie clay, 0 to 1 percent slopes

#### **Map Unit Setting**

National map unit symbol: djq0 Elevation: 550 to 850 feet

Mean annual precipitation: 30 to 42 inches
Mean annual air temperature: 64 to 68 degrees F

Frost-free period: 230 to 260 days

Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

Fairlie and similar soils: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Fairlie**

#### Setting

Landform: Ridges

Landform position (two-dimensional): Footslope, toeslope Landform position (three-dimensional): Base slope

Down-slope shape: Linear Across-slope shape: Convex

Parent material: Residuum weathered from austin chalk formation

#### Typical profile

H1 - 0 to 8 inches: clay H2 - 8 to 46 inches: clay H3 - 46 to 54 inches: bedrock

#### **Properties and qualities**

Slope: 0 to 1 percent

Depth to restrictive feature: 40 to 60 inches to paralithic bedrock

Drainage class: Moderately well drained

Runoff class: High

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

low (0.00 to 0.06 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 20 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm) Available water supply, 0 to 60 inches: Moderate (about 7.8 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2s

Hydrologic Soil Group: D

Ecological site: R086AY011TX - Southern Blackland

Hydric soil rating: No

## FaB—Fairlie clay, 1 to 2 percent slopes

#### **Map Unit Setting**

National map unit symbol: djq1 Elevation: 550 to 850 feet

Mean annual precipitation: 30 to 42 inches Mean annual air temperature: 64 to 68 degrees F

Frost-free period: 230 to 260 days

Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

Fairlie and similar soils: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Fairlie**

#### Setting

Landform: Ridges

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Base slope

Down-slope shape: Linear Across-slope shape: Convex

Parent material: Residuum weathered from austin chalk formation

#### **Typical profile**

H1 - 0 to 8 inches: clay
H2 - 8 to 46 inches: clay
H3 - 46 to 54 inches: bedrock

#### **Properties and qualities**

Slope: 1 to 2 percent

Depth to restrictive feature: 40 to 60 inches to paralithic bedrock

Drainage class: Moderately well drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

low (0.00 to 0.06 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 20 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Moderate (about 7.8 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: D

Ecological site: R086AY011TX - Southern Blackland

Hydric soil rating: No

# GsB—Georgetown stony clay loam, 1 to 3 percent slopes

#### **Map Unit Setting**

National map unit symbol: 2t277 Elevation: 620 to 1,250 feet

Mean annual precipitation: 32 to 36 inches
Mean annual air temperature: 65 to 68 degrees F

Frost-free period: 230 to 260 days

Farmland classification: Not prime farmland

#### **Map Unit Composition**

Georgetown and similar soils: 90 percent

Minor components: 10 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Georgetown**

#### Setting

Landform: Ridges

Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Interfluve, side slope

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Clayey residuum weathered from limestone

#### Typical profile

A - 0 to 7 inches: stony clay loam
Bt - 7 to 35 inches: cobbly clay
R - 35 to 60 inches: bedrock

#### **Properties and qualities**

Slope: 1 to 3 percent

Surface area covered with cobbles, stones or boulders: 0.1 percent

Depth to restrictive feature: 20 to 40 inches to lithic bedrock

Drainage class: Well drained Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

low (0.00 to 0.06 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 5 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Low (about 3.8 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: D

Ecological site: R081CY361TX - Redland 29-35 PZ

Hydric soil rating: No

#### **Minor Components**

#### **Tarpley**

Percent of map unit: 5 percent

Landform: Ridges

Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Interfluve, side slope

Down-slope shape: Linear Across-slope shape: Linear

Ecological site: R081CY361TX - Redland 29-35 PZ

Hydric soil rating: No

#### **Eckrant**

Percent of map unit: 3 percent

Landform: Ridges

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Interfluve, side slope

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R081CY360TX - Low Stony Hill 29-35 PZ

Hydric soil rating: No

#### **Fairlie**

Percent of map unit: 2 percent

Landform: Ridges

Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Interfluve, side slope

Down-slope shape: Linear Across-slope shape: Concave

Ecological site: R086AY011TX - Southern Blackland

Hydric soil rating: No

## HedC2—Heiden clay, 2 to 5 percent slopes, moderately eroded

#### **Map Unit Setting**

National map unit symbol: 2sshp

Elevation: 320 to 750 feet

Mean annual precipitation: 37 to 40 inches Mean annual air temperature: 67 to 68 degrees F

Frost-free period: 245 to 260 days

Farmland classification: Not prime farmland

#### **Map Unit Composition**

Heiden, moderately eroded, and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Heiden, Moderately Eroded**

#### Setting

Landform: Ridges

Landform position (two-dimensional): Shoulder, backslope Landform position (three-dimensional): Interfluve, side slope

Microfeatures of landform position: Linear gilgai

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Clayey residuum weathered from mudstone

#### **Typical profile**

Ap - 0 to 6 inches: clay Bkss1 - 6 to 18 inches: clay Bkss2 - 18 to 58 inches: clay CBdk - 58 to 80 inches: clay

#### **Properties and qualities**

Slope: 2 to 5 percent

Depth to restrictive feature: 40 to 65 inches to densic material

Drainage class: Well drained Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

low (0.00 to 0.06 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 40 percent

Gypsum, maximum content: 5 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Sodium adsorption ratio, maximum: 12.0

Available water supply, 0 to 60 inches: High (about 9.3 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hvdrologic Soil Group: D

Ecological site: R086AY009TX - Southern Eroded Blackland

Hydric soil rating: No

#### **Minor Components**

#### Ferris, moderately eroded

Percent of map unit: 8 percent

Landform: Ridges

Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Microfeatures of landform position: Linear gilgai

Down-slope shape: Linear Across-slope shape: Convex

Ecological site: R086AY009TX - Southern Eroded Blackland

Hydric soil rating: No

#### Heiden

Percent of map unit: 7 percent

Landform: Ridges

Landform position (two-dimensional): Shoulder, backslope Landform position (three-dimensional): Interfluve, side slope

Microfeatures of landform position: Linear gilgai

Down-slope shape: Convex Across-slope shape: Convex

Ecological site: R086AY011TX - Southern Blackland

Hydric soil rating: No

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McNeil and CR 107 Round Rock, Williamson County, Texas ECS Project No. 51:3992 February 22, 2024

#### NARRATIVE DESCRIPTION OF SITE-SPECIFIC GEOLOGY

Ranging from north to south, two primary physiographic provinces are present in Williamson County: the Great Plain and the Gulf Coastal Plain. The Gulf Coastal Plain is comprised mainly of Blackland prairie. The Great Plain is comprised chiefly of limestone plains, which merges with the Edwards Plateau in the vicinity of the Colorado River.

Groundwater recharge and flow are controlled by faulted Edwards Aquifer and adjacent strata. Water enters the aquifer by means of solution features controlled by faults, fractures and solution conduits. Solution features are created by the dissolution of limestone primarily from rainwater and groundwater. Deformation of the Balcones fault system controls both the large- and small-scale flow barriers and pathways present in the Edwards Aquifer.

Geological information pertaining to the area was obtained from the Geologic Atlas of Texas, Austin Sheet, published by University of Texas at Austin, Bureau of Economic Geology (BEG), 1997. The subject property is situated on Terraces along streams (Qt) (Figure 6).

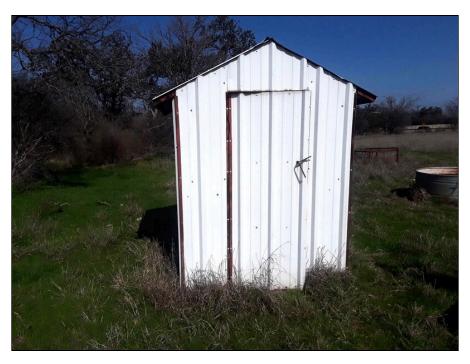
BEG describes Qt as "consist of three or more levels which may correspond to coastal Piestocene units; gravel, sand, silt, and clay in various proportions with gravel prominent in the older, higher terraces, gravel along Guadalupe River, siliceous, course, along Colorado River, mostly dolomite, limestone, chert, quartz, and various igneous and metamorphic rocks from the Llano Region and dolomite, limestone and chert from the Edwards Plateau; sand mostly quartz."

The Georgetown Limestone is inferred to lie just below the porous and permeable Qt. The Kgt is described by the BEG as "limestone and marl; mostly fine-grained, argillaceous, nodular, moderately indurated, light gray; some limestone, hard, brittle, thick-bedded, shite; some shale, marly soft, light gray to yellowish gray; marine megafossils include Kingena wacoensis and Gryphaea washitaensis; thickness 30 - 80 feet thing southward"

Other potential natural recharge features such as caves, sinkholes, closed depressions, solution cavities, fractured rock outcrops, faults or lineaments were not observed on the subject property. Additionally, seeps or springs were not observed on the subject property.

ECS

Attachment 1: NRCS Soil Map Report



1 - Wellhouse located on the northern part of the subject property



2 - View of the Inside of the well house



3 - Well house located on the subject property



4 - Outside of wellhouse



5 - Outside of wellhouse faucet



6 - Drainage feature S-3



7 - Drainage feature S-2



8 - Drainage feature S-1 along the western site boundary



9 - View of wetland area observed near the southwest corner of the subject property.



10 - View of apparent wetland area observed near the southwest portion of the subject property.



11 - Drainage feature S-3 along the western site boundary



12 - Drainage feature S-3 along the western site boundary



13 - Drainage feature S-3 along the western site boundary



14 - View south along subject property boundary from near feature S-3.



15 - View north along subject property boundary from near feature S-3.



16 - View of feature S-2.



17 - View south along drainage feature S-2



18 - View of standing water observed in feature S-2.



19 - View north across property interior from the south poriton of the site.



20 - View northeast across the interior of the subject property.



21 - View south from the northern part of the subject property



22 - View north from the southern part of the subject property



23 - View of scattered/exposed rocks near the north-center of the subject property



24 - View east from west center portion of the subject property



25 - View of railroad tracks, and McNeil towards the western property boundary



26 - Karst feature S-4



27 - Karst feature S-4



28 - Karst feature S-4



29 - Karst feature S-4



30 - Karst feature S-4



31 - Karst feature S-5



32 - Karst feature S-5



33 - Karst feature S-5



34 - Karst feature S-5

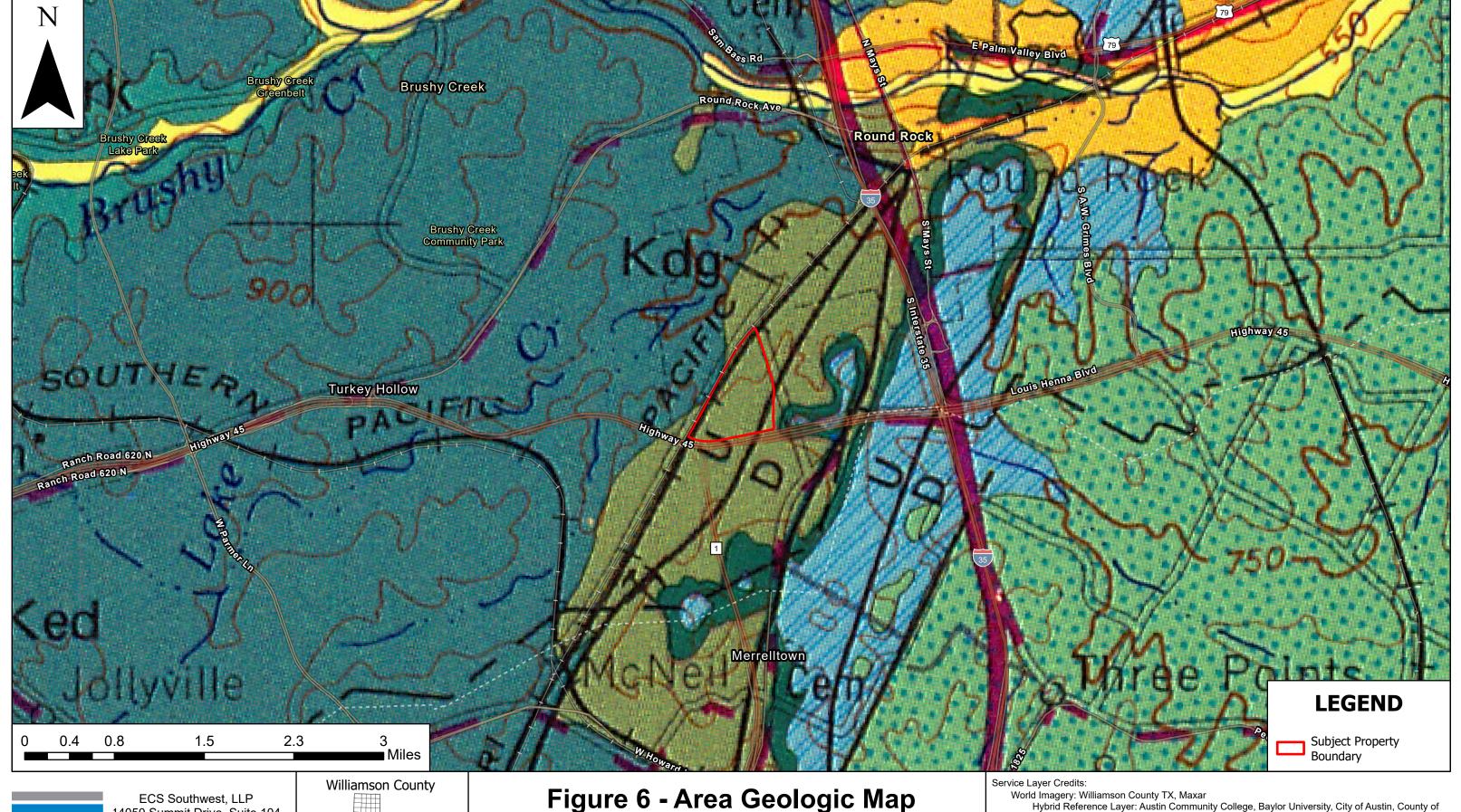


35 - Karst feature S-5



36 - Karst feature S-5

## SITE GEOLOGIC MAP(S)





14050 Summit Drive, Suite 104 Austin, Texas 78728 Phone: (512) 837-8005 www.ecslimited.com

ECS Project No. 51:3992



# Figure 6 - Area Geologic Map

McNeil Road and CR 172 Geologic Assessment (GA) McNeil Road and CR 172, Round Rock, TX Williamson County, Texas (193.289 acres)

Watershed: San Gabriel USGS Quadrangle: Pflugerville West, TX 2022

March 5, 2024

Williamson, Texas Parks & Wildlife, CONANP, Esri, TomTom, Garmin, SafeGraph, GeoTechnologies, Inc, METI/NASA, USGS, EPA, NPS, USDA, USFWSUSGS Topographic Map of the Pflugerville West, Texas Quadrangle (2022)
Soils Data: USDA NRCS Web Soil Survey

Wetlands Data: National Wetlands Inventory
Floodplain Data: FEMA National Flood Hazard Layer LIDAR Data: USGS 3D Elevation Program





# SECTION 4: MODIFICATION OF A PREVIOUSLY APPROVED PLAN

# Modification of a Previously Approved Plan

#### **Texas Commission on Environmental Quality**

for Regulated Activities on the Edwards Aquifer Recharge Zone and Transition Zone and Relating to 30 TAC 213.4(j), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

#### Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This request for a **Modification of a Previously Approved Plan** is hereby submitted for TCEQ review and executive director approval. The request was prepared by:

Print Name of Customer/Agent: Nicholas C. Brown, P.E.

Date: 05/15/2025

Signature of Customer/Agent:

#### **Project Information**

1. Current Regulated Entity Name: <u>2525 CR 172 Industrial</u> Original Regulated Entity Name: <u>2525 CR 172 Industrial</u> Regulated Entity Number(s) (RN): RN112000070

Edwards Aguifer Protection Program ID Number(s): 11004052

X The applicant has not changed and the Customer Number (CN) is: CN605619519

The applicant or Regulated Entity has changed. A new Core Data Form has been provided.

2. X Attachment A: Original Approval Letter and Approved Modification Letters. A copy of the original approval letter and copies of any modification approval letters are attached.

<ul> <li>A modification of a previously approved plan is requested for (check all that apply):         <ul> <li>Physical or operational modification of any water pollution abatement structure(s) including but not limited to ponds, dams, berms, sewage treatment plants, and diversionary structures;</li> <li>Change in the nature or character of the regulated activity from that which was originally approved or a change which would significantly impact the ability of the plan to prevent pollution of the Edwards Aquifer;</li> <li>Development of land previously identified as undeveloped in the original water pollution abatement plan;</li> <li>Physical modification of the approved organized sewage collection system;</li> <li>Physical modification of the approved underground storage tank system;</li> <li>Physical modification of the approved aboveground storage tank system.</li> </ul> </li> <li>Summary of Proposed Modifications (select plan type being modified). If the approved plan has been modified more than once, copy the appropriate table below, as</li> </ul>					
necessary, and complete the information for each additional modification.					
WPAP I	Modification	Approved Project	Proposed Modification		
Summa	ıry				
Acres		N/A	N/A		
Type of Development		N/A	N/A		
Number of Residential		N/A	N/A		
Lots					
Impervious Cover (acres)		N/A	N/A		
Impervious Cover (%		N/A	N/A		
Permanent BMPs		N/A	N/A		
Other		N/A	<u>N/A</u>		
SCS Mo	odification	Approved Project	Proposed Modification		
Summa	ıry				
Linear Feet		1,036	494		
Pipe Diameter		8"	8"		

N/A

N/A

Other

AST Modification	Approved Project	Proposed Modification			
Summary					
Number of ASTs	N/A	N/A			
Volume of ASTs	N/A	N/A			
Other	N/A	N/A			
UST Modification	Approved Project	<b>Proposed Modification</b>			
Summary					
Number of USTs	N/A	N/A			
Volume of USTs	N/A	N/A			
Other	N/A	N/A			
5. X Attachment B: Narrative of Proposed Modification. A detailed narrative description of the nature of the proposed modification is attached. It discusses what was approved, including any previous modifications, and how this proposed modification will change the approved plan.					
<ul> <li>Attachment C: Current Site Plan of the Approved Project. A current site plan showing the existing site development (i.e., current site layout) at the time this application for modification is attached. A site plan detailing the changes proposed in the submitted modification is required elsewhere.  The approved construction has not commenced. The original approval letter and any subsequent modification approval letters are included as Attachment A to document that the approval has not expired.  The approved construction has commenced and has been completed. Attachment C illustrates that the site was constructed as approved.  The approved construction has commenced and has been completed. Attachment C illustrates that the site was not constructed as approved.  X The approved construction has commenced and has not been completed.  Attachment C illustrates that, thus far, the site was constructed as approved.  The approved construction has commenced and has not been completed.  Attachment C illustrates that, thus far, the site was not constructed as approved.</li> </ul>					
<ul> <li>The acreage of the approved plan has increased. A Geologic Assessment has been provided for the new acreage.</li> <li>Acreage has not been added to or removed from the approved plan.</li> </ul>					
8. X Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.					

# ORIGINAL APPROVAL LETTER AND APPROVED MODIFICATION LETTERS

Jon Niermann, *Chairman*Bobby Janecka, *Commissioner*Catarina R. Gonzales, *Commissioner*Kelly Keel, *Executive Director* 



#### TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

Protecting Texas by Reducing and Preventing Pollution

August 16, 2024

Mr. David Barnett Amazon.com Services, LLC Amazon Tower 1, 101 Platform Way N Nashville, TN 37203

Re: Approval of an Organized Sewage Collection System (SCS) Plan

2525 CR 172 Industrial; Located West of CR 172 and Martin Ave., Round Rock ETJ,

Williamson County, Texas

Edwards Aquifer Protection Program ID: 11004052, Regulated Entity No. RN112000070

#### Dear Mr. Barnett:

The Texas Commission on Environmental Quality (TCEQ) has completed its review on the application for the above-referenced project submitted to the Edwards Aquifer Protection Program (EAPP) by Kimley-Horn and Associates, on behalf of the applicant, Amazon.com Services, LLC, on June 25, 2024. Final review of the application was completed after additional material was received on August 13, 2024.

As presented to the TCEQ, the application was prepared in general compliance with the requirements of 30 Texas Administrative Codes (TAC) Chapter §213 and Chapter §217. The engineering design report, technical specifications and final design plans were prepared by a Texas licensed professional engineer (PE). All construction plans and design information were sealed, signed, and dated by a Texas licensed PE. Therefore, the application for the construction of the proposed project and methods to protect the Edwards Aquifer are hereby **approved**, subject to applicable state rules and the conditions in this letter.

This approval expires two years from the date of this letter, unless, prior to the expiration date, more than 10 percent of the construction has commenced on the project or an extension of time has been officially requested. This approval or extension will expire, and no extension will be granted if more than 50 percent of the project has not been completed within ten years from the date of this letter.

The applicant or a person affected may file with the chief clerk a motion for reconsideration of the executive director's final action on this Edwards Aquifer protection plan. A motion for reconsideration must be filed in accordance with 30 TAC §50.139.

#### PROJECT DESCRIPTION

The proposed SCS will provide disposal service for future commercial development. The system includes a lift station, a force main, gravity lines, and other appurtenance necessary for conveying wastewater to a treatment plant. The proposed SCS will consist of 938 linear feet of 8-inch diameter PVC SDR 26 ASTM D3034 pipe gravity line, and 98 linear feet of 4-inch diameter HDPE DR-9 ASTM D3350 pipe force main, with associated appurtenance necessary for conveying wastewater to a treatment plant.

#### LIFT STATION DETAILS

The proposed lift station will consist of a 6-foot diameter wet well with an approximate depth of 35.5 feet, two submersible 3 HP pumps, and an Emergency Power Generator. Each pump will have a pumping capacity of 99 gallons per minute (gpm) at a total dynamic head (TDH) of 32.46 feet. The lift station is designed and will be constructed to ensure that bypassing of sewage does not occur.

Additional equipment will include a control panel, an audio-visual alarm, auto-dial telemetry, hoisting equipment, level pump controllers, pump supports and discharge piping with valves, and a security fence with controlled access.

#### TREATMENT FACILITY

The system will be connected to an existing City of Round Rock wastewater line for conveyance to the Brushy Creek Regional Wastewater Treatment Plant for treatment and disposal. **The proposed system shall be connected for conveyance prior to use of the development.** The project will conform to all applicable codes, ordinances, and requirements of the City of Round Rock.

#### **GEOLOGY**

According to the Geologic Assessment (GA) included with the application, the surficial unit of the site is Del Rio Clay and Georgetown Formation (Kdr). No sensitive geologic features were identified in the GA. The site assessment conducted on August 6, 2024 by TCEQ staff determined the site to be generally as described by the GA.

#### SPECIAL CONDITIONS

- I. Upon completion of any lift station excavation, a geologist **must certify** that the excavation has been inspected for the presence of sensitive features. Certification that the excavation has been inspected must be submitted to the EAPP within 30 days of the inspection.
- II. This approval does not authorize construction of any associated project requiring a Water Pollution Abatement Plan (WPAP) at the site. A WPAP is required for construction of any commercial project located on the Recharge Zone and shall be submitted at a later date.

#### STANDARD CONDITIONS

- 1. The plan holder (applicant) must comply with all provisions of 30 TAC Chapter §213 and all technical specifications in the approved plan. The plan holder should also acquire and comply with additional and separate approvals, permits, registrations or authorizations from other TCEQ Programs (i.e., Water Quality) as required based on the specifics of the plan.
- 2. In addition to the rules of the Commission, the plan holder must also comply with state and local ordinances and regulations providing for the protection of water quality as applicable.

#### Prior to Commencement of Construction:

- 3. The plan holder of any approved Edwards Aquifer protection plan must notify the EAPP and obtain approval from the executive director prior to initiating any modification to the activities described in the referenced application following the date of the approval.
- 4. The plan holder must provide written notification of intent to commence construction, replacement, or rehabilitation of the referenced project. Notification must be submitted to the EAPP no later than 48 hours prior to commencement of the regulated activity. Notification must include the date on which the regulated activity will commence, the name of the approved plan and program ID number for the regulated activity, and the name of the prime contractor with the name and telephone number of the contact person.
- 5. Temporary erosion and sedimentation (E&S) controls as described in the referenced application, must be installed prior to construction, and maintained during construction. Temporary E&S controls may be removed when vegetation is established, and the

Mr. David Barnett Page 3 August 16, 2024

construction area is stabilized. The TCEQ may monitor stormwater discharges from the site to evaluate the adequacy of temporary E&S control measures. Additional controls may be necessary if excessive solids are being discharged from the site.

#### **During Construction:**

- 6. This approval does not authorize the installation of temporary or permanent aboveground storage tanks on this project that will have a total storage capacity of 500 gallons or more of static hydrocarbons or hazardous substances without prior approval of an Aboveground Storage Tank facility application.
- 7. If any sensitive feature is encountered during construction, replacement, or rehabilitation on this project, all regulated activities must be **immediately** suspended near it and notification must be made to TCEQ EAPP staff. Temporary BMPs must be installed and maintained to protect the feature from pollution and contamination. Regulated activities near the feature may not proceed until the executive director has reviewed and approved the methods proposed to protect the feature and the aquifer from potentially adverse impacts to water quality.
- 8. If sediment escapes the construction site, the sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain). Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50 percent. Litter, construction debris, and construction chemicals shall be prevented from becoming stormwater discharge pollutants.
- 9. Intentional discharges of sediment laden water are not allowed. If dewatering becomes necessary, the discharge must be filtered through appropriately selected BMPs.
- 10. The following records shall be maintained and made available to the executive director upon request: the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 11. Stabilization measures shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, and construction activities will not resume within 21 days. When the initiation of stabilization measures by the 14th day is precluded by weather conditions, stabilization measures shall be initiated as soon as practicable.

#### After Completion of Construction:

- 12. No part of the organized sewage collection system may be used as a sewage holding tank, as defined in 30 TAC §213.3 (excluding lift stations), over the Edwards Aquifer recharge zone.
- 13. A Texas licensed PE **must certify** in writing that the new sewage collection system (including force mains) has passed all required testing. The certification shall be submitted to the EAPP within 30 days of test completion and prior to the new sewage collection system being put into service.
- 14. A Texas licensed PE **must certify** subsequent testing required every five years of the existing sewage collection system after being put into use to determine types and locations of structural damage and defects such as offsets, open joints, or cracked or crushed lines that would allow exfiltration to occur. The test results must be retained by the plan holder for five years and made available to the executive director upon request.

The holder of the approved Edwards Aquifer protection plan is responsible for compliance with Chapter §213 and any condition of the approved plan through all phases of plan implementation. Failure to comply with any condition within this approval letter is a violation of Chapter §213 and is subject to administrative rule or orders and penalties as provided under §213.10 of this title (relating to Enforcement). Such violations may also be subject to civil

Mr. David Barnett Page 4 August 16, 2024

penalties and injunction. Upon legal transfer of this property, the new owner is required to comply with all terms of the approved Edwards Aquifer protection plan.

This action is taken as delegated by the executive director of the Texas Commission on Environmental Quality. If you have any questions or require additional information, please contact Ms. Miki Chilarescu, P.E., of the Edwards Aquifer Protection Program at 512-239-6175 or the regional office at 512-339-2929.

Sincerely,

Monica Reyes

Monica Reyes, Section Manager

Edwards Aquifer Protection Program

Texas Commission on Environmental Quality

MR/mec

cc: Mr. Nicholas C. Brown, P.E. - Kimley-Horn and Associates

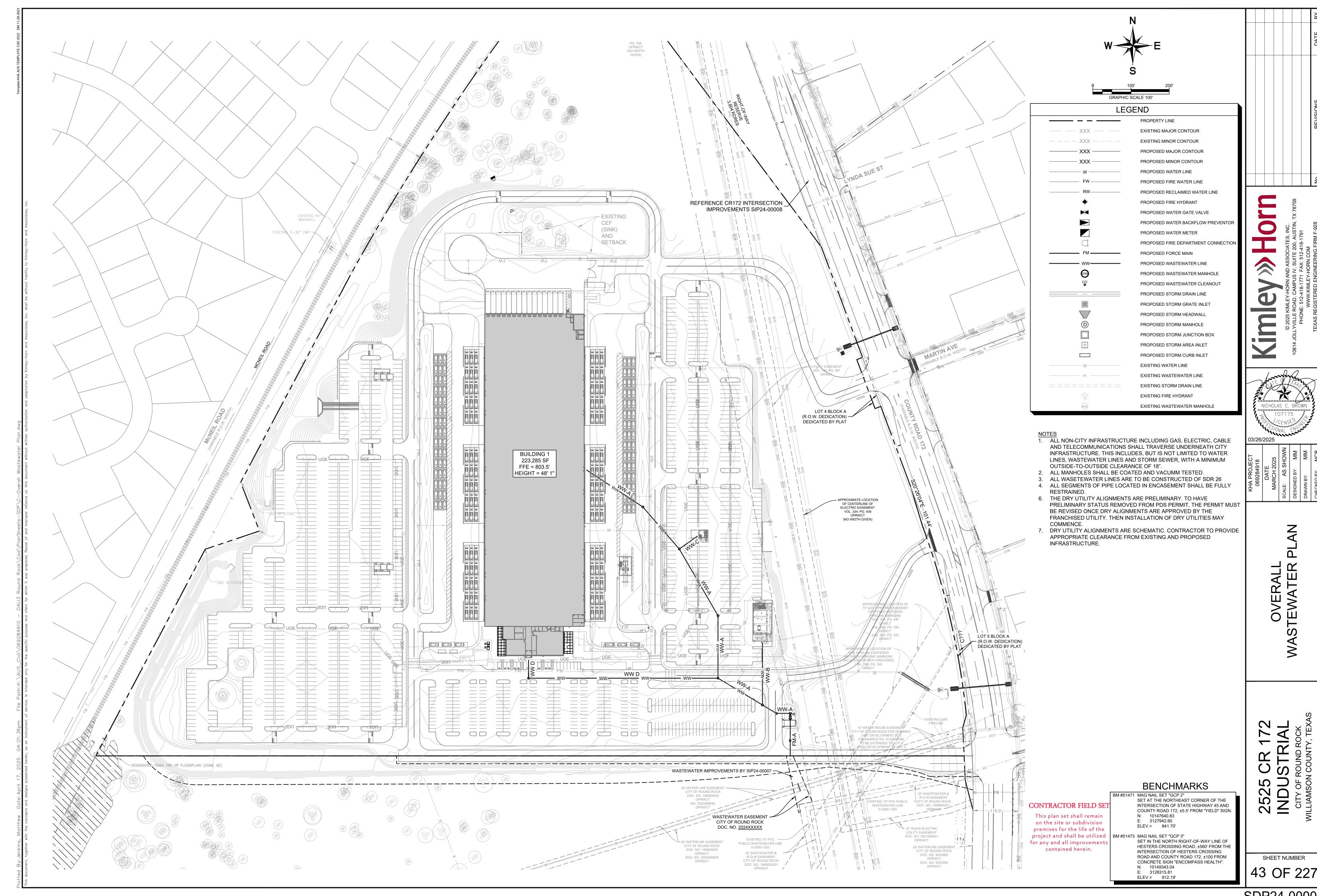
#### NARRATIVE OF PROPOSED MODIFICATION

The proposed 2525 CR 172 Industrial project located at 2525 CR 172 in Round Rock, Texas is located south of the McNeil Rd and County Road 172 intersection within the Round Rock City Limits. The property is currently zoned Planned Unit Development (PUD) 153. The current zoning supports the use of the proposed development. The proposed existing property is approximately 69.56-acres and the site is currently undeveloped. The proposed land use would be classified as Industrial. Construction has begun but has not been completed.

Two WPAP's have been submitted separately, and both have been approved (EAPP ID No. 11004138 & 11004324). The wastewater extension will serve a proposed industrial building and miscellaneous smaller buildings located on the property. The limits of construction for the wastewater improvements will consist of 1.01 acres out of the 69.56-total site acres.

The current proposed modification to the previously approved Organized Sewage Collection System includes an updated layout of the wastewater lines leading to the onsite structures. The updated layout of the wastewater lines was approved by the City of Round Rock and the client (Amazon). This layout will allow for a shallower line throughout the site. This modification does not propose any additional changes to the wastewater line.

### **CURRENT SITE PLAN OF THE APPROVED PROJECT**



Kimley » Horn

# SECTION 5: ORGANIZED SEWAGE COLLECTION SYSTEM PLAN

### Organized Sewage Collection System Application

#### **Texas Commission on Environmental Quality**

For Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(c), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Regulated Entity Name: 2525 CR 172 Industrial

1. X Attachment A – SCS Engineering Design Report. This Engineering Design Report is provided to fulfill the requirements of 30 TAC Chapter 217, including 217.10 of Subchapter A, §§217.51 – 217.70 of Subchapter C, and Subchapter D as applicable, and is required to be submitted with this SCS Application Form.

#### **Customer Information**

2. The entity and contact person responsible for providing the required engineering certification of testing for this sewage collection system upon completion (including private service connections) and every five years thereafter to the appropriate TCEQ region office pursuant to 30 TAC §213.5(c) is:

Contact Person: <u>David Barnett</u> Entity: <u>Amazon.com Services</u>, <u>LLC</u>

Mailing Address: Amazon Tower 1, 101 Platform Way N

City, State: Nashville, TN Zip:  $\underline{37203}$  Telephone:  $\underline{901-438-4156}$  Fax:  $\underline{N/A}$ 

Email Address: barnettu@amazon.com

The appropriate regional office must be informed of any changes in this information within 30 days of the change.

3. The engineer responsible for the design of this sewage collection system is:

Contact Person: Nicholas C. Brown, P.E.

Texas Licensed Professional Engineer's Number: 107175

Entity: Kimley-Horn and Associates

Mailing Address: 10814 Jollyville Rd, Avallon IV, Suite 200

 City, State: Austin, TX
 Zip: 78759

 Telephone: 512-418-1771
 Fax: N/A

Email Address: nick.brown@kimley-horn.com

# **Project Information**

4.	Anticipated type of development to be served (estimated future population to be served, plus adequate allowance for institutional and commercial flows):
	Residential: Number of single-family lots:  Multi-family: Number of residential units:  Commercial  Industrial  Off-site system (not associated with any development)  Other:
5.	The character and volume of wastewater is shown below:
	% Domesticgallons/day% Industrialgallons/day% Commingledgallons/day Total gallons/day: 126,695
6.	Existing and anticipated infiltration/inflow is $\underline{}$ gallons/day. This will be addressed by: $\underline{}$ N/A $\underline{}$ .
7.	A Water Pollution Abatement Plan (WPAP) is required for construction of any associated commercial, industrial or residential project located on the Recharge Zone.
	<ul> <li>The WPAP application for this development was approved by letter dated &amp; 3/7/2025 A copy of the approval letter is attached.</li> <li>The WPAP application for this development was submitted to the TCEQ on, but has not been approved.</li> <li>A WPAP application is required for an associated project, but it has not been submitted.</li> <li>There is no associated project requiring a WPAP application.</li> </ul>

#### 8. Pipe description:

**Table 1 - Pipe Description** 

Pipe Diameter(Inches)	Linear Feet (1)	Pipe Material (2)	Specifications (3)
8	396	PVC SDR-26	ASTM D-3034
4 (FM)	98	HDPE DR-9	ASTM D-3350

Total Linear Feet: 494

- (1) Linear feet Include stub-outs and double service connections. Do not include private service laterals.
- (2) Pipe Material If PVC, state SDR value.
- (3) Specifications ASTM / ANSI / AWWA specification and class numbers should be included.

9.	e sewage collection system will convey the wastewater to the <u>Brushy Creek Regional Wastewa</u> eatment Plant. The treatment facility is:           X         Existing           Proposed	<u>ater</u>
10.	components of this sewage collection system will comply with:    X   The City of   Round Rock   standard specifications.     Other.   Specifications are attached.	
11.	No force main(s) and/or lift station(s) are associated with this sewage collection system.	
	A force main(s) and/or lift station(s) is associated with this sewage collection system and the <b>Lift Station/Force Main System Application</b> form (TCEQ-0624) is included with this application.	
ΑI	nment	
12.	There are no deviations from uniform grade in this sewage collection system without manholes and with open cut construction.	
13.	There are no deviations from straight alignment in this sewage collection system without manholes.	
	Attachment B - Justification and Calculations for Deviation in Straight Alignment without Manholes. A justification for deviations from straight alignment in this sewage collection system without manholes with documentation from pipe manufacturer allowing pipe curvature is attached.  For curved sewer lines, all curved sewer line notes (TCEQ-0596) are included on the construction plans for the wastewater collection system.	
Ma	holes and Cleanouts	
1/1	Manholas or cloan outs exist at the end of each sower line(s). These locations are listed	

14. |X| Manholes or clean-outs exist at the end of each sewer line(s). These locations are listed below: (Please attach additional sheet if necessary)

**Table 2 - Manholes and Cleanouts** 

Line	Shown on Sheet	Station	Manhole or Clean- out?
FM-A	44 of 227	1+00.00	Manhole
FM-A	44 of 227	2+31.17	Manhole
FM-A	44 of 227	2+80.77	Manhole
WW-A	44 of 227	1+36.25	Manhole
WW-A	44 of 227	1+78.84	Manhole
WW-A	44 of 227	3+14.71	Manhole

Shown on Sheet	Station	out?

16. $\overline{X}$ The maximum spacing between	manholes on this project for each pipe diameter is no
greater than:	

Pipe Diameter (inches)	Max. Manhole Spacing (fe
6 - 15	500
16 - 30	800
36 - 48	1000
≥54	2000

Attachment C – Justification for Variance from Maximum Manhole Spacing. The
maximum spacing between manholes on this project (for each pipe diameter used) is
greater than listed in the table above. A justification for any variance from the
maximum spacing is attached, and must include a letter from the entity which will
operate and maintain the system stating that it has the capability to maintain lines with
manhole spacing greater than the allowed spacing.

- 17. All manholes will be monolithic, cast-in-place concrete.
  - X The use of pre-cast manholes is requested for this project. The manufacturer's specifications and construction drawings, showing the method of sealing the joints, are attached.

# Site Plan Requirements

#### Items 18 - 25 must be included on the Site Plan.

18.  $\overline{X}$  The Site Plan must have a minimum scale of 1" = 400'. Site Plan Scale: 1" =  $\underline{100}$ '.

19. X The Site Plan must include the sewage collection system general layout, including manholes with station numbers, and sewer pipe stub outs (if any). Site plan must be overlain by topographic contour lines, using a contour interval of not greater than ten feet and showing the area within both the five-year floodplain and the 100-year floodplain of any drainage way.

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<b>ZU.</b>	Laiti	aı วเเ	มมาน	<i>.</i>

L		The location of all lateral stub-outs are shown and labeled.
	X	No lateral stub-outs will be installed during the construction of this sewer collection
		system.

21.	Location of existing and proposed water lines:
	<ul> <li>☐ The entire water distribution system for this project is shown and labeled.</li> <li>☐ If not shown on the Site Plan, a Utility Plan is provided showing the entire water and sewer systems.</li> <li>☐ There will be no water lines associated with this project.</li> </ul>
22.	100-year floodplain:
	After construction is complete, no part of this project will be in or cross a 100-year floodplain, either naturally occurring or manmade. (Do not include streets or concrete-lined channels constructed above of sewer lines.)  After construction is complete, all sections located within the 100-year floodplain will have water-tight manholes. These locations are listed in the table below and are shown and labeled on the Site Plan. (Do not include streets or concrete-lined channels constructed above sewer lines.)

Table 3 - 100-Year Floodplain

Line	Sheet	Station
N/A	N/A	N/A
	of	to
	of	to
	of	to

#### 23. 5-year floodplain:

X	After construction is complete, no part of this project will be in or cross a 5-year			
	floodplain, either naturally occurring or man-made. (Do not include streets or concrete-			
	lined channels constructed above sewer lines.)			

After construction is complete, all sections located within the 5-year floodplain will be encased in concrete or capped with concrete. These locations are listed in the table below and are shown and labeled on the Site Plan. (Do not include streets or concrete-lined channels constructed above sewer lines.)

Table 4 - 5-Year Floodplain

Line	Sheet	Station
N/A	N/A	N/A
	of	to
	of	to
	of	to

- 24.  $\boxed{X}$  Legal boundaries of the site are shown.
- 25. X The *final plans and technical specifications* are submitted for the TCEQ's review. Each sheet of the construction plans and specifications are dated, signed, and sealed by the Texas Licensed Professional Engineer responsible for the design on each sheet.

# Items 26 - 33 must be included on the Plan and Profile sheets. 26. X All existing or proposed water line crossings and any parallel water lines within 9 feet of sewer lines are listed in the table below. These lines must have the type of pressure rated pipe to be installed shown on the plan and profile sheets. Any request for a variance from the required pressure rated piping at crossings must include a variance approval from 30 TAC Chapter 290. There will be no water line crossings. There will be no water lines within 9 feet of proposed sewer lines. **Table 5 - Water Line Crossings** Horizontal Vertical Station or Crossing or Separation Separation Line Closest Point **Parallel** Distance Distance FM-A 1+55.09 N/A 2.0 ft Crossing 27. Vented Manholes: |X| No part of this sewer line is within the 100-year floodplain and vented manholes are not required by 30 TAC Chapter 217. A portion of this sewer line is within the 100-year floodplain and vented manholes will be provided at less than 1500 foot intervals. These water-tight manholes are listed in the table below and labeled on the appropriate profile sheets. A portion of this sewer line is within the 100-year floodplain and an alternative means of venting shall be provided at less than 1500 feet intervals. A description of the alternative means is described on the following page. A portion of this sewer line is within the 100-year floodplain; however, there is no interval longer than 1500 feet located within. No vented manholes will be used. Table 6 - Vented Manholes

Line	Manhole	Station	Sheet
N/A	N/A	N/A	N/A

Line	Manhole	Station	Sheet		
N/A	N/A	N/A	N/A		
28. Drop manholes:					
o. Drop mannoics.					
There are no drop manholes associated with this project.					
X Sewer lines which	enter new or existing r	nanholes or "manhole st	ructures" higher than		

24 inches above the manhole invert are listed in the table below and labeled on the

appropriate profile sheets. These lines meet the requirements of 30 TAC

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Table	/ -	Drop	ıvıan	noies

§217.55(I)(2)(H).

Line	Manhole	Station	Sheet
FM-A	4'	1+00.00	44

29.	Sewer line stub-outs (	For proposed extension	ons):	
	= :	o-outs are to be install	r line stub-outs are show ed during the construction	
30.	Lateral stub-outs (For	proposed private serv	rice connections):	
		_	al stub-outs are shown an luring the construction of	
31.	Minimum flow velocit	y (From Appendix A)		
		e flowing full; all slope eet per second for this	es are designed to produce system/line.	e flows equal to or
32.	Maximum flow veloci	ty/slopes (From Apper	ndix A)	
	less than or equal Attachment D – Co Assuming pipes ar	to 10 feet per second alculations for Slopes e flowing full, some sl	es are designed to produce for this system/line. for Flows Greater Than 1 opes produce flows which red in the table below. Cal	<b>0.0 Feet per Second.</b> are greater than 10

Table 8 - Flows Greater Than 10 Feet per Second

Line	Profile Sheet	Station to Station	FPS	% Slope	Erosion/Shock Protection
N/A	N/A	N/A	N/A	N/A	N/A

33.	Assuming pipes are flowing full, where flows are ≥ 10 feet per second, the provisions noted below have been made to protect against pipe displacement by erosion and/or shock under 30 TAC §217.53(I)(2)(B).
	<ul> <li>□ Concrete encasement shown on appropriate Plan and Profile sheets for the locations listed in the table above.</li> <li>□ Steel-reinforced, anchored concrete baffles/retards placed every 50 feet shown on appropriate Plan and Profile sheets for the locations listed in the table above.</li> <li>□ N/A</li> </ul>

#### Administrative Information

- 34. X The final plans and technical specifications are submitted for TCEQ review. Each sheet of the construction plans and specifications are dated, signed, and sealed by the Texas Licensed Professional Engineer responsible for the design on each sheet.
- 35. X Standard details are shown on the detail sheets, which are dated, signed, and sealed by the Texas Licensed Professional Engineer, as listed in the table below:

Table 9 - Standard Details

Standard Details	Shown on Sheet
Lateral stub-out marking [Required]	N/A
Manhole, showing inverts comply with 30 TAC §217.55(I)(2) [Required]	SECTION 5, ATTACHMENT F
Alternate method of joining lateral to existing SCS line for potential future connections [Required]	N/A
Typical trench cross-sections [Required]	SECTION 5, ATTACHMENT F
Bolted manholes [Required]	N/A
Sewer Service lateral standard details [Required]	SECTION 5, ATTACHMENT F
Clean-out at end of line [Required, if used]	N/A
Baffles or concrete encasement for shock/erosion protection [Required, if flow velocity of any section of pipe >10 fps]	N/A
Detail showing Wastewater Line/Water Line Crossing [Required, if crossings are proposed]	SECTION 5, ATTACHMENT F
Mandrel detail or specifications showing compliance with 30 TAC §217.57(b) and (c) [Required, if Flexible Pipe is used]	N/A

Standard Details	Shown on Sheet
Drop manholes [Required, if a pipe entering a manhole is more than 24	SECTION 5,
inches above manhole invert]	ATTACHMENT F

- 36. X All organized sewage collection system general construction notes (TCEQ-0596) are included on the construction plans for this sewage collection system.
- 37. X All proposed sewer lines will be sufficiently surveyed/staked to allow an assessment prior to TCEQ executive director approval. If the alignments of the proposed sewer lines are not walkable on that date, the application will be deemed incomplete and returned.
  - Survey staking was completed on this date: \_\_\_\_\_
- 38. X Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
- 39. X Any modification of this SCS application will require TCEQ approval, prior to construction, and may require submission of a revised application, with appropriate fees.

## Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Organized Sewage Collection System Application** is hereby submitted for TCEQ review and executive director approval. The system was designed in accordance with the requirements of 30 TAC §213.5(c) and 30 TAC §217 and prepared by:

Print Name of Licensed Professional Engineer: Nicholas C. Brown, P.E.

Date: 05/15/2025

Place engineer's seal here:



Signature of Licensed Professional Engineer:

## Appendix A-Flow Velocity Table

**Flow Velocity (Flowing Full)** All gravity sewer lines on the Edwards Aquifer Recharge Zone shall be designed and constructed with hydraulic slopes sufficient to give a velocity when flowing full of not less than 2.0 feet per second, and not greater than 10 feet per second. The grades shown in the following table are based on Manning's formula and an n factor of 0.013 and shall be the minimum and maximum acceptable slopes unless provisions are made otherwise.

Table 10 - Slope Velocity

Pipe Diameter(Inches)	% Slope required for minimum flow velocity of 2.0 fps	% Slope which produces flow velocity of 10.0 fps
6	0.50	12.35
8	0.33	8.40
10	0.25	6.23
12	0.20	4.88
15	0.15	3.62
18	0.11	2.83
21	0.09	2.30
24	0.08	1.93
27	0.06	1.65
30	0.055	1.43
33	0.05	1.26
36	0.045	1.12
39	0.04	1.01
>39	*	*

<sup>\*</sup>For lines larger than 39 inches in diameter, the slope may be determined by Manning's formula (as shown below) to maintain a minimum velocity greater than 2.0 feet per second when flowing full and a maximum velocity less than 10 feet per second when flowing full.

$$v = \frac{1.49}{n} \times R_h^{0.67} \times \sqrt{S}$$

Figure 1 - Manning's Formula

#### Where:

v = velocity (ft/sec)
n = Manning's roughness coefficient
(0.013)
Rh = hydraulic radius (ft)
S = slope (ft/ft)

#### SCS ENGINEERING DESIGN REPORT

This Engineering Design Report has been prepared to comply with the Texas Commission on Environmental Quality Design Criteria for Domestic Wastewater Systems, 30 TAC Chapter 217, including 217.10 of Subchapter A, §§217.51 – 217.70 of Subchapter C, and Subchapter D as applicable.

### **Project Description**

#### Introduction

2525 CR 172 Industrial is an undeveloped 69.56 acre property located south of the McNeil Rd and County Road 172 intersection, within the Round Rock City Limits. The proposed 2525 CR 172 Industrial project includes the construction of one (1) story industrial building and miscellaneous smaller buildings with associated roadway, water, wastewater, and drainage improvements to support the project. This project proposes 40.28 acres (57.90%) of impervious cover.

This project is located within the Lake Creek Watershed. A portion of this site is located in the Federal Emergency Management Agency's 100-year floodplain according to FIRM 48491C0630F. The site is located within the Edwards Aquifer Recharge Zone according to the TCEQ Edward's Aquifer Maps. A Critical Environmental Feature (CEF) is located on the property, however, no wastewater or site improvements are proposed within the CEF or associated setback. The site consists of 69.56 acres of undeveloped land.

On-site infrastructure is comprised of water, electric, wastewater, and storm sewer lines. The wastewater service outlined in this report will consist of an 8" line that will convey wastewater to one lift station and eventually will travel via a 4" force main to a proposed public manhole. It will then travel via an 8" public gravity wastewater line through to an existing off-site manhole that leads to an off-site line. The offsite line will gravity flow and be deposited at the Brushy Creek Regional Wastewater Treatment Plant to be treated. A Development Agreement between Amazon.com Services, LLC and the City of Round Rock recognizes the proposed flow from 2525 CR 172 Industrial development and that the city will accept to treat it through the Brushy Creek Regional Wastewater Treatment Plant. All lines will consist of single service connections to all structures on-site that are proposed per this development. All proposed manholes shall be watertight, with watertight rings and covers. All proposed wastewater lines will be installed within the Edwards Aquifer Recharge Zone (EARZ).

# Pipe Design

## Flow Design Basis

Service for the build-out of the 69.56 ac one (1) story industrial building, located at 2525 CR 172, Round Rock, TX 78681, will be served by this wastewater system. The City of Round Rock Utility Criteria Manual (UCM) was used to determine the parameters for the design of the wastewater line system. Calculations for this wastewater line system will be shown at the end of this report.

#### Gravity Pipe and Joint Materials

The proposed pipe to be used for the 8" wastewater line will be ASTM D3034 SDR-26 PVC pipe (cell class 12454). The joints for this pipe shall meet the requirements of ASTM

D3212. The pipe joints shall have an integral bell and rubber gasket seal with the locked-in type gasket.

#### Separation Distances for Water and Wastewater

A 14.5-foot minimum horizontal separation is maintained between all proposed wastewater infrastructure and proposed water lines. There are no water line crossings that need to meet the nine-foot minimum vertical separation.

#### Service Connections

Service connections have been included for each proposed structure on-site.

#### Boring and Tunneling of Crossings

No boring or tunneling of crossings are proposed for this project.

#### Corrosion Potential

PVC pipe will be utilized for or all proposed wastewater lines. No deterioration of the proposed pipe or its associated components is anticipated in this application.

#### Odor Control

All flows contributing to the proposed wastewater lines are from industrial developments generating domestic sewage. There is a lift station proposed with development. Odor control measures are proposed for this project.

#### Active Geologic Faults

Per the Geologic Assessment, no active geologic faults were located within the area of the project.

#### Capacity Analysis

The capacity of each proposed wastewater segment is calculated below based on Manning's Equation. The calculation for each segment is based on the minimum proposed slope.

$$Q = \frac{1.49}{n} * A * R^{0.67} * S^{0.5}$$

Where:

Qfull = flow rate of fluid in pipe at full flow (ft<sup>3</sup>/s) (cfs)

Q90%= flow rate of fluid in pipe at 90% full flow (ft<sup>3</sup>/s) (cfs)

A = area of pipe (ft^2) = 
$$\frac{\pi * d^2}{4}$$

d = internal pipe diameter (ft) = Do – 2t

Do = outside diameter (in) t = pipe wall thickness (in)

n = Manning's Roughness coefficient = 0.013

Rfull = hydraulic radius of pipe (full flow) = A/P = D/4 (ft)

R90%= hydraulic radius of pipe (90% full flow) = 0.9\*A/P = 0.9\*D/4 (ft)

- P = wetted perimeter of pipe =  $\pi$  \*D (ft)
- S = slope of energy line

D:	Length	Slope	Slope	Diam	eter	Pipe	N4	Р	Α	Rfull	R90%	Qfull	Q90%	Vfull	V90%
Pipes	ft	%	ft/ft	in	ft	Material	Manning's	ft	sf	ft	ft	cfs	cfs	fps	fps
FMA-1	131	3.00	0.030	8.00	0.67	PVC	0.013	2.09	0.35	0.17	0.15	2.09	1.88	5.98	5.38
FMA-2	50	1.00	0.010	8.00	0.67	PVC	0.013	2.09	0.35	0.17	0.15	1.20	1.08	3.45	3.11
FMA-3	98	-0.41	-0.004	4.00	0.33	HDPE			SEE SI	ECTION 6 (	LIFT STATIO	ON/FORCE M	AIN)		
WWA-1	36	5.02	0.050	8.00	0.67	PVC	0.013	2.09	0.35	0.17	0.15	2.70	2.43	7.73	6.96
WWA-2	43	6.61	0.066	8.00	0.67	PVC	0.013	2.09	0.35	0.17	0.15	3.10	2.79	8.87	7.98
WWA-2	136	0.47	0.005	8.00	0.67	PVC	0.013	2.09	0.35	0.17	0.15	0.83	0.74	2.37	2.13

Pipe	Minimum Velocity	Maximum Velocity		
Туре	fps	fps		
4" HDPE	SEE SECTION 6 (LIFT S	TATION/FORCE MAIN)		
8" PVC	2.37	8.87		
ALL	2.37	8.87		

The proposed wastewater line installed at the slope specified provides capacity in excess of the calculated peak wet weather design flows at full flow and 90% full flow conditions.

#### Structural Analysis

Flexible pipe is proposed on this project. Structural calculations are provided for the flexible pipe to be installed. The proposed collection system piping is designed to have a minimum structural life of 50 years. As previously mentioned, all proposed PVC pipe shall be cell class 12454 with a tensile strength of 7,000 psi.

Live Load Calculations - no significant live loads are anticipated on any segment of this project.

Buckling Pressure - the following equations utilized for the calculation of buckling pressure are taken from the *Handbook of PVC Pipe: Design and Construction* (Uni-Bell PVC Pipe Association, 2001).

Pcr = 
$$\frac{2*E}{(1-v^2)*(DR-1)^3}$$
 (Equation 7.14)

$$Pb = 1.15*\sqrt{Pcr*E}$$
 (Equation 7.18)  
 $H = (Pb*144)/w$  (Equation 6.7)

Where:

Pcr = critical buckling pressure (psi)

E = modulus of elasticity (psi) = 400,000 psi for PVC

v = Poisson's Ratio = 0.38 for PVC

DR = dimension ratio

Pb = buckling pressure in soil (psi)

E' = modulus of soil reaction (psi) = 2,000 psi for crushed rock compacted to greater than 95% relative density

H = maximum allowable cover height of soil (ft)

W = weight of soil (lbs/ft<sup>3</sup>) = 120 lbs/ft<sup>3</sup>

#### 8" ASTM D3034 SDR-26

$$Pcr = \frac{2*400,000}{(1-0.38^2)*(26-1)^3}$$

Pcr = 59.84 psi

Pb = 
$$1.15 * \sqrt{59.84 * 2,000}$$

Pb = 397.84 psi

H = (397.84\*144) / 120

H = 477.41 ft height of soil to cause pipe buckling

*Prism Load Calculations* - the following equations utilized for the calculation of prism loads are taken from the *Handbook of PVC Pipe: Design and Construction* (Uni-Bell PVC Pipe Association, 2001).

 $P = H^*w$  (Equation 6.7)

Where:

P = prism load pressure due to soil weight (lbs/ft²)

H = depth of pipe (ft)

w = soil density (lbs/ft³) = 120 lbs/ft³

#### 8" ASTM D3034 SDR-26

P = 20 \* 120

 $P = 2,400 \text{ lbs/ft}^2 \text{ or } 16.67 \text{ psi}$ 

Long Term Deflection Calculations - the following equations utilized for the calculation of long term deflection are taken from the Handbook of PVC Pipe: Design and Construction (Uni-Bell PVC Pipe Association, 2001).

$$\Delta Y/D = \frac{DL*K*P+K*W_1}{[2E/(3(DR-1)^3)]+0.061*E'}*100$$
 (Equation 7.10)

Where:

ΔY/D = long term deflection (%)

DL = Deflection Lag Factor = 1.0 for prism load calculation

K = bedding constant = 0.096 for 90°

P = prism load pressure due to soil weight (lbs/ft²)

 $W_1 =$  live load (psi) = 0 psi

E = modulus of elasticity (psi) = 400,000 psi for PVC

DR = dimension ratio

E' = modulus of soil reaction (psi) = 2,000 psi for crushed rock bedding compacted to greater than 95% relative density

Note: Leonhardt's Zeta factor is assumed to equal 1, and thus is not required in the calculation. This is a conservative assumption that results in a more conservatively calculated value for long term deflection.

#### 8" ASTM D3034 SDR-26

$$\Delta Y/D = \frac{1.0*0.096*16.67 + 0.096*0}{[2(400,000)/(3(26-1)^3)] + 0.061*2,000} * 100$$
$$\Delta Y/D = 1.15\%$$

Wall Crushing Calculations - the following equations utilized for the calculation of wall crushing are taken from the Handbook of PVC Pipe: Design and Construction (Uni-Bell PVC Pipe Association, 2001).

$$Py = \frac{\Theta c * 2 * A}{D}$$
 (Equation 7.20)

$$H = Py / w$$
 (Equation 6.7)

Where:

Py = pressure due to soil weight (psi)

 $\Theta c$  = compressive stress (psi) = 4,000 psi for PVC pipe

A= surface area of the pipe wall (in<sup>2</sup>/in)

D = mean pipe diameter (in) = Do - t

t = pipe wall thickness (in)

H = maximum allowable height of cover (ft)

 $W = soil density (lbs/ft^3) = 120 lbs/ft^3$ 

#### 8" ASTM D3034 SDR-26

Do = 
$$8.4 - 0.323 = 8.077$$
 in, A =  $3.88$  in<sup>2</sup>/ft ( $0.323$  in \* 12 in/ft)

$$Py = \frac{4,000*2*(3.88/12)}{8.077}$$

Py = 320.25 psi

H = (320.25\*144) / 120

H = 384.30 ft height of soil to cause wall crushing

Strain Calculations - the following equations utilized for the calculation of strain are taken from the *Handbook of PVC Pipe: Design and Construction* (Uni-Bell PVC Pipe Association, 2001).

$$\epsilon h = \frac{P*D}{2*t*E}$$
(Equation 7.22)

$$\varepsilon f = \frac{t * [3 * \Delta Y / D]}{D * [1 - 2 * \Delta Y / D]}$$
 (Equation 7.24)

$$\varepsilon = \varepsilon h + \varepsilon f$$
 (Equation 7.25)

Where:

 $\varepsilon h = maximum strain in the pipe wall due to hoop stress (in/in)$ 

P = prism load pressure due to soil weight (psi)

D = mean pipe diameter (in) = Do - t

t = pipe wall thickness (in)

E = modulus of elasticity (psi) = 400,000 psi for PVC

 $\varepsilon f = maximum strain in the pipe due to ring deflection or flexure (in/in)$ 

ΔY/D = long term deflection

 $\varepsilon$  = maximum combined strain in pipe wall (in/in)

#### 8" ASTM D3034 SDR-26

$$\epsilon h = \frac{16.67 * 8.077}{2 * 0.323 * 400,000}$$

 $\varepsilon h = 0.00052 \text{ in/in}$ 

$$\epsilon f = \frac{0.323}{8.077} * \frac{[3*0.0115]}{[1-2*0.0115]}$$

 $\varepsilon f = 0.0014 in/in$ 

 $\varepsilon = 0.00035 + 0.0014$ 

 $\varepsilon = 0.00175 \text{ in/in}$ 

Per the *Handbook of PVC Pipe: Design and Construction* (Uni-Bell PVC Pipe Association, 2001), deflection test samples have experienced a pipe wall strain of up to 0.025 in/in and have not "showed any failures or cracks". The calculated strains for this project are significantly below this level, so no failure due to strain is anticipated.

*Pipe Stiffness Calculation* - the following equations utilized for the calculation of pipe stiffness are taken from the *Handbook of PVC Pipe: Design and Construction* (Uni-Bell PVC Pipe Association, 2001).

$$Ps = 4.47*\frac{E}{(DR-1)^3}$$
 (Equation 7.3)

Where:

Ps = pipe stiffness (psi)

DR = Dimensional Ration = Do / t

Do = Outside diameter (in)

t = pipe wall thickness (in)

E = modulus of elasticity (psi) = 400,000 psi for PVC

#### 8" ASTM D3034 SDR-26

DR = 26

$$Ps = 4.47 * \frac{400,000}{(26-1)^3}$$

Ps = 115 psi

## Criteria for Laying Pipe

#### Pipe Embedment

Bedding and initial backfill material selection and installation will be carried out in accordance with applicable governing procedures contained within the *City of Round Rock Standard Specifications for Underground Pipe Utilities, TCEQ Chapter 217.54(a),* and in accordance with the City of Round Rock Details WW-18 and WW-20. Bedding material shall be in accordance with City of Round Rock Standard Specification Item 510.

#### Compaction

Trench compaction will be carried out in accordance with the *City of Round Rock Standard Specifications for Underground Pipe Utilities* and *TCEQ Chapter 217.54(b)*. Proper placement of the backfill and compaction per City of Round Rock requirements will not negatively impact the structural integrity of the pipe.

#### Envelope Size

Envelope size will be in accordance with *City of Round Rock Standard Specifications for Underground Pipe Utilities* and *TCEQ Chapter 217.54(c)*. Per the City of Round Rock Details WW-18 and WW-20, a minimum of 6-inch and maximum of 12-inch space shall be allowed between the outside diameter of the pipe and the trench wall and floor. In addition, a minimum of 6-inch and maximum of 12-inch space shall be allowed between the outside diameter of the pipe bell and the trench wall. The embedment and initial backfill must be installed to a minimum depth of 12 inches above the crown of the pipe.

#### Trench Width

Trench width will be in accordance with the City of Round Rock Details WW-18 and WW-20 and TCEQ Chapter 217.54(d). Per the City of Round Rock Details WW-18, and WW-20, a minimum

of 6-inch and maximum of 12-inch space shall be allowed between the outside diameter of the pipe and the trench wall and floor. In addition, a minimum of 6-inch and maximum of 12-inch space shall be allowed between the outside diameter of the pipe bell and the trench wall. These limits shall be maintained to protect the structural integrity of the pipe and will be sufficient for the placement of materials and use of compaction equipment in the pipe zone.

#### Manholes and Related Structures

#### Manhole and Appurtenance Placement

Manholes are located at all points of change in alignment or grade and at the intersection of all pipes for this project.

#### Manhole Stub Outs

No manholes are being placed at the end of a line that may be extended in the future, so no stub outs are included on this project.

#### Cleanouts

No dead-end lines are included in this project, so no cleanouts are proposed.

#### Manhole Material

Monolithic or precast manholes are acceptable for the contractor to utilize and are included in the City of Round Rock Detail WW-01. The use of bricks is not acceptable for the manhole or for cover adjustments.

#### Manhole Spacing

Manhole spacing meets the requirements of Table C.2 in TCEQ Chapter 217.55.

#### Manholes within Waterways

No manholes will be located within flow paths of waterways or in areas where water ponding is probable.

#### Manhole Covers, Inlets, and Bases

Per the City of Round Rock Details WW-07 and WW-08, the manhole covers shall have a 30-inch diameter clear opening. Manhole covers shall be constructed of cast iron and have no openings for water to infiltrate. No proposed manholes are located within the 100-year flood plain. All manholes shall be watertight, with watertight rings and covers.

As shown in the project details, the bottom of the manhole shall have a U-shaped channel to provide smooth continuation between the inlet and outlet pipes. For the proposed pipe, the manhole channel depth shall be equal to at least half the largest pipe diameter. Manholes with different pipe sizes shall have the tops of the pipes at the same elevation and flow channels in the invert sloped evenly from pipe to pipe.

#### Manhole Steps

No steps shall be allowed in any proposed manholes.

#### Manhole Connections

Manhole-pipe connections shall be watertight per City of Round Rock pipe to manhole connector in accordance with ASTM C923. See detail WW-10.

# ORGANIZED SEWAGE COLLECTION SYSTEM ATTACHMENT A

#### Manhole Venting

The proposed manholes are spaced at less than 1,500-foot intervals and none are located within the 100-year flood plain. Therefore, no vented manholes are proposed on this project.

#### Trenchless Pipe Installation

There will be no trenchless pipe installation.

## Testing Requirements for Gravity Pipes

#### Infiltration/Exfiltration and Low Pressure Air Test

All testing will be in compliance with Texas Administrative Code title 30 Part 1 Chapter 217 Subchapter C 217.57 and 217.58.

Infiltration and exfiltration or low-pressure air testing in accordance with ASTM C828, C924 or F1417 are required for all proposed gravity wastewater pipe as specified in the project notes, Sheet 7. The requirements specified are in accordance with *TCEQ Chapter 217.57*.

#### Deflection Testing

For the proposed 8-inch wastewater line, deflection shall be measured with a rigid mandrel. The requirements specified are in accordance with *TCEQ Chapter 217.57*.

#### Owner Inspection

The Owner shall have an inspector on-site during construction of the project. A professional engineer registered in the state of Texas (Nicholas C. Brown, P.E.) shall be present to witness the testing of the wastewater lines.

# Testing Requirements for Manholes

Manhole testing in accordance with *TCEQ Chapter 217.58* is specified in the project notes. Manholes will be tested after assembly and backfilling for leakage by either a hydrostatic test and/or a vacuum test.

For the vacuum test, all lift holes and exterior joints shall be plugged with an approved nonshrink grout and no grout shall be placed in horizontal joints before testing. All pipes entering the manhole shall be plugged, taking care to securely brace the plugs from being drawn into the manhole. Stub outs, manhole boots, and pipe plugs shall be secured to prevent movement while the vacuum is drawn. A minimum 60-inch/lb torque wrench shall be used to tighten the external clamps that secure the test cover to the top of the manhole. The test head shall be placed at the inside of the top of the cone section and the seal inflated in accordance with the manufacturer's recommendation. A vacuum of 10 inches of mercury shall be drawn and the vacuum pump shut off. With the valves closed, the time shall be measured for the vacuum to drop to 9 inches of mercury. The manhole shall pass if the time is greater than 2 minutes. If the manhole fails the initial test, necessary repairs shall be made with a non-shrink grout while the vacuum is still being drawn. If the manhole fails a second time, repairs should again be made and the manhole shall be tested by means of a hydrostatic test. If any manhole fails the hydrostatic test, after failing the vacuum test twice, the contractor should consider replacing that manhole. If the contractor chooses to attempt to repair that manhole, the manhole must be retested by means of the hydrostatic test until it passes.

Inspection will be provided during critical phases of construction by a qualified inspector under the direction of a P.E. (Nicholas C. Brown, P.E.). Critical phases of construction are deemed at a minimum to include testing of pipe and manholes for leakage and testing of flexible pipe for installed deflection.

TCEQ approval letters for plans and specifications review contain the requirement that once the project is completed, a P.E. registered in the state of Texas (Nicholas C. Brown, P.E.) much certify that the construction was performed substantially in accordance with the approved plans and specifications.

#### Notification and Inspection

TCEQ Chapter 213.5(f) requires that the applicant must provide written notification to the Round Rock regional office at least 48 hours prior to commencing construction on the regulated activity. If any sensitive feature is discovered during construction, then the work shall be suspended immediately and the Round Rock regional office shall be notified to then determine the appropriate course of action. All other notification and inspection requirements identified in TCEQ Chapter 213.5(c) shall be met.

KIMLEY-HORN AND ASSOCIATES, INC.

Nick Brown, P.E. Senior Project Manager (512) 418-1771

Nick.Brown@kimley-horn.com

# JUSTIFICATION AND CALCULATIONS FOR DEVIATION IN STRAIGHT ALIGNMENT WITHOUT MANHOLES

(NOT APPLICABLE)

# JUSTIFICATION FOR VARIANCE FROM MAXIMUM MANHOLE SPACING

(NOT APPLICABLE)

# CALCULATIONS FOR SLOPES FOR FLOWS GREATER THAN 10.0 FEET PER SECOND (NOT APPLICABLE)

# SITE PLAN

# REVISIONS/CORRECTIONS **ROCK APPROVAL** DESCRIPTION SIGNATURE

# CIVIL SITE DEVELOPMENT IMPROVEMENTS FOR

# 2525 CR 172 INDUSTRIAL

2525 CR 172, ROUND ROCK, TX 78681 SDP24-0001 ZONING: PUD 153; O-2024-008 SUBDIVISION CASE NO.: FP24-000001

# **GENERAL PLAN NOTES:**

- 1. ALL RESPONSIBILITY FOR THE ADEQUACY OF THESE PLANS REMAINS WITH THE REGISTERED PROFESSIONAL ENGINEER WHO PREPARED THEM. IN REVIEWING THESE PLANS THE CITY OF ROUND ROCK MUST RELY UPON THE ADEQUACY OF THE WORK OF THE DESIGN ENGINEER.
- APPROVAL OF THESE PLANS BY THE CITY OF ROUND ROCK INDICATES COMPLIANCE WITH APPLICABLE CITY REGULATIONS ONLY. APPROVAL BY OF CONSTRUCTION. THE APPLICANT IS RESPONSIBLE FOR DETERMINING
- INCORPORATED AREAS (EFFECTIVE DATE DECEMBER 20, 2019).
- WATER AND WASTEWATER SERVICE WILL BE PROVIDED BY THE CITY OF
- 6. NO STRUCTURES CAN BE BUILT WITHIN WATER & WASTEWATER
- RELEASE OF THIS APPLICATION DOES NOT CONSTITUTE A VERIFICATION OF THE ENGINEER OF RECORD IS SOLELY RESPONSIBLE FOR THE COMPLETENESS, ACCURACY AND ADEQUACY OF HIS/HER SUBMITTAL WHETHER OR NOT THE APPLICATION IS REVIEWED FOR CODE COMPLIANCE
- 8. AS PART OF THIS SITE PLAN, THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP) IS REQUIRED TO BE ON SITE AT ALL TIMES.
- 9. THIS SITE IS LOCATED IN THE EDWARDS AQUIFER RECHARGE ZONE. WATER QUALITY MEASURES ARE PROVIDED.
- 10. THIS SITE PROPOSES ON SITE DETENTION TO THE 100 YEAR STORM EVENT.

IMPERVIOUS COVER					
EXISTING	0	SF			
BUILDING FOOTPRINT	223,285	SF			
PARKING, PRIVATE SIDEWALK & ROAD, CURB & GUTTER, ETC.	1,201,805	SF			
PUBLIC SIDEWALKS	0	SF			
TOTAL	1,425,090	SF			
DISTURBED AREA					
TOTAL DISTLIBBED AREA (LOC)	50.0044	4.0			

WPAP: CASE NUMBER 11004138; APPROVED DECEMBER 20, 2024 SCS: CASE NUMBER 11004052; APPROVED AUGUST 16, 2024

PREVIOUS RELATED SITE DEVELOPMENT CASE NO.: N/A

WATERSHED: LAKE CREEK

SUBMITTAL DATE: 03/26/2025

PRESSURE ZONE: ZONE 2; ELEV. 971

BM #51471	MAG NAIL SET "GCP 2"
	SET AT THE NORTHEAST CORNER OF THE
	INTERSECTION OF STATE HIGHWAY 45 AND
	COUNTY ROAD 172, ±5.5' FROM "YIELD" SIGN.
	N: 10147640.83
	E: 3127942.60
	FI FV = 841 70'
BM #51473	MAG NAIL SET "GCP 3"
	SET IN THE NORTH RIGHT-OF-WAY LINE OF
	HESTERS CROSSING ROAD, ±560' FROM THE
	INTERSECTION OF HESTERS CROSSING
	ROAD AND COUNTY ROAD 172, ±100 FROM
	CONCRETE SIGN "ENCOMPASS HEALTH".
	N: 10149343.04
	E: 3128315.81
	ELEV.= 812.19'

# **PROJECT** LOCATION **VICINITY MAP** MAPSCO: 406N, 406P, 406S, 406T

# MARCH 2025

# A 149.32 ACRE TRACT OF LAND SITUATED IN THE JACOB M. HARRELL SURVEY. ABSTRACT 284, WILLIAMSON COUNTY, TEXAS; AND BEING A PORTION OF A CALLED 184.435 ACRES TRACT OF LAND DESCRIBED TO AMAZON.COM SERVICES LLC, AS SHOWN ON INSTRUMENT RECORDED IN DOCUMENT NO. 2021166943 OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS (O.P.R.W.C.T)

# **LISTS OF CONTACTS:**

(817) 701-4828

SARAH VOSS, PROJECT MANAGER PDMS DESIGN GROUP 2225 E. RANDOL MILL RD., SUITE 300 ARLINGTON, TEXAS 76011

LANDSCAPE ARCHITECT MORGAN DEPINE KIMLEY-HORN 5301 SOUTHWEST PKWY, BLDG 2, STE 200 AUSTIN, TEXAS 78735 (512) 646-2237

OWNER/DEVELOPER DAVID BARNETT **AMAZON TOWER 1** 101 PLATFORM WAY N NASHVILLE, TN 37203 (901) 438-4156

MICHAEL A. MONTGOMERY II. RPLS KIMLEY-HORN 10814 JOLLYVILLE ROAD, BLDG, STE 200 AUSTIN, TEXAS 78759 (512) 418-1771

WATER & SANITARY SEWER CITY OF ROUND ROCK 3400 SUNRISE ROAD **ROUND ROCK, TEXAS 78665** (512) 218-5555

WILLIAMSON COUNTY ESD #9 402 A WEST PALM VALLEY BLVD **ROUND ROCK, TX 78664** (512) 479-9267

GAS ATMOS ENERGY MARTIN PEREZ 3110 N INTERSTATE HWY 35 ROUND ROCK, TX 78681 (512) 415-8426

ELECTRIC ONCOR ELECTRIC DELIVERY JUSTIN JACKS 350 TEXAS AVENUE **ROUND ROCK, TX 78664** (512) 244-5616 STORM SEWER CITY OF ROUND ROCK 3400 SUNRISE ROAD **ROUND ROCK, TEXAS 78665** (512) 218-5555

**DEVELOPMENT SERVICES** CITY OF ROUND ROCK 301 W. BAGDAD AVE #210 ROUND ROCK, TX 78664 (512) 218-5428

#### STATE OF TEXAS ALL RESPONSIBILITY FOR THE ADEQUACY OF THESE PLANS REMAINS **COUNTY OF WILLIAMSON** WITH THE ENGINEER WHO PREPARED THEM. IN ACCEPTING THESE PLANS, THE CITY OF ROUND ROCK MUST RELY UPON THE ADEQUACY , NICHOLAS C. BROWN, DO HEREBY CERTIFY THAT THE PUBLIC WORKS OF THE WORK OF THE DESIGN ENGINEER. AND DRAINAGE IMPROVEMENTS SHOWN HEREIN HAVE BEEN DESIGNED IN COMPLIANCE WITH THE SUBDIVISION AND BUILDING REGULATION ORDINANCES AND STORMWATER DRAINAGE POLICY ADOPTED BY THE CITY OF ROUND ROCK, TEXAS. ACCEPTED FOR CONSTRUCTION Jacob Bradley 03/26/2025 DATE CITY OF ROUND ROCK, TEXAS LICENSED PROFESSIONAL ENGINEER NO. 107175 PLANNING AND DEVELOPMENT SERVICES DEPARTMENT

CONTRACTOR FIELD SET

This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

AUSTIN, TEXAS 78759 CERTIFICATE OF REGISTRATION #928

SDP24-00001

SHE

COVER

SHEET NUMBER OF 227

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1 1 92 1 1	PRIVATESTORM PLANY \\\\\\\\	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	PHOTOMETRIC PLAND RETAINING WALL(1)	210	EV PLANCEAQ / / / / / / / /
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74	GRADING / / / /	1/3/1/	RETAINING WALL (40)	421	
					REMOVED SINCE THEY ARE NOT APPLICABLE REVIEW.
				10 1020	

CONTRACTOR FIELD SET

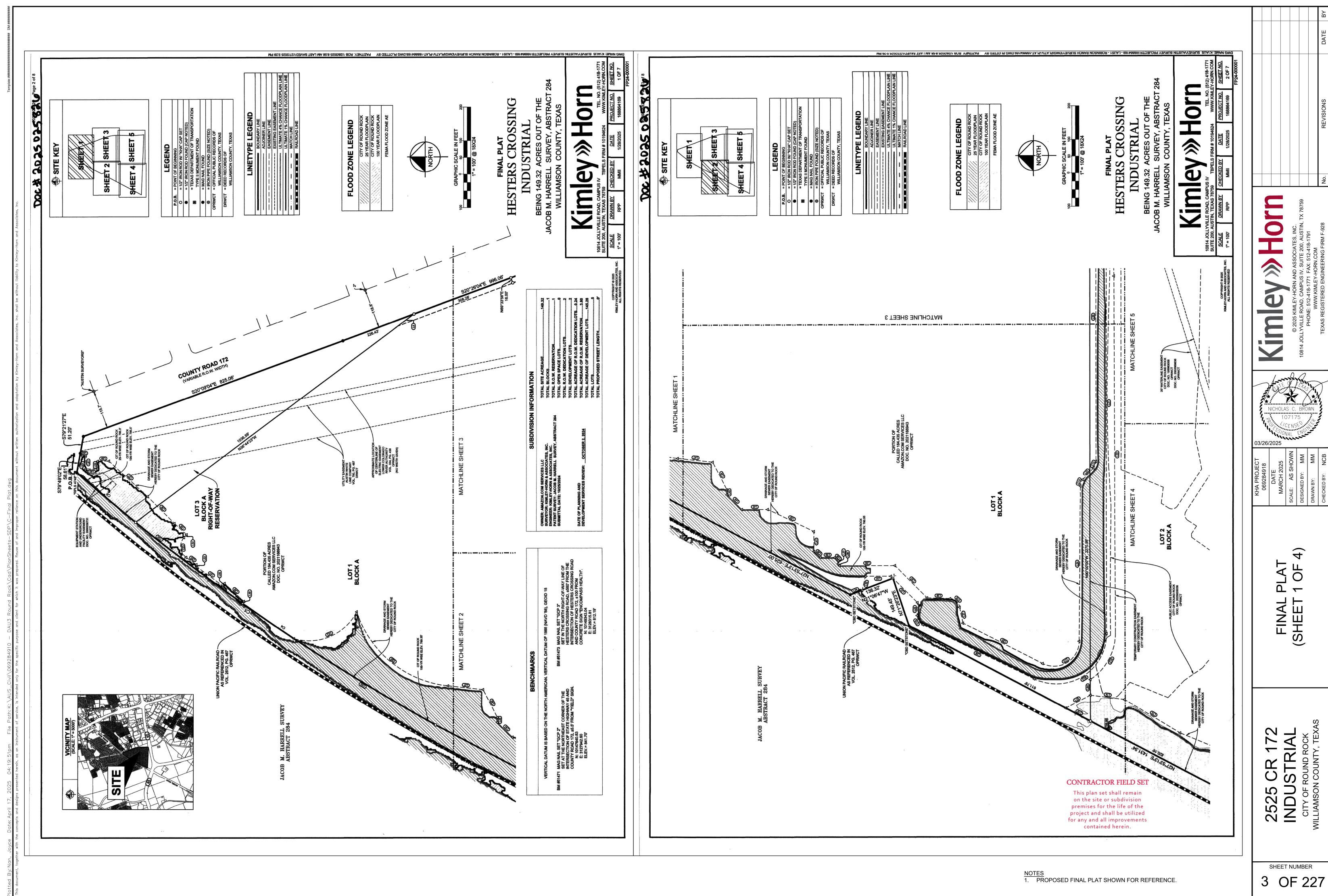
This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

SHEET NUMBER

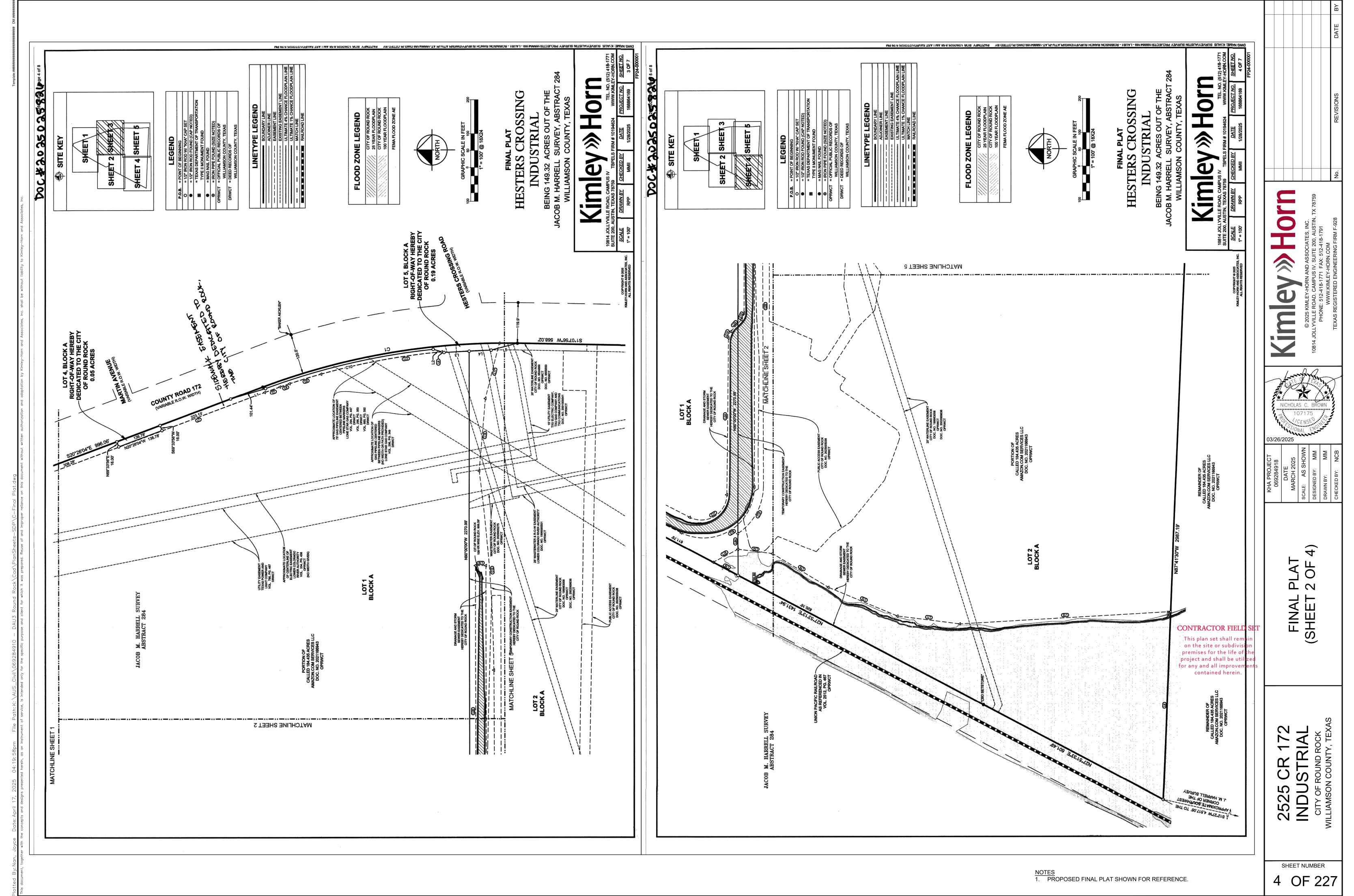
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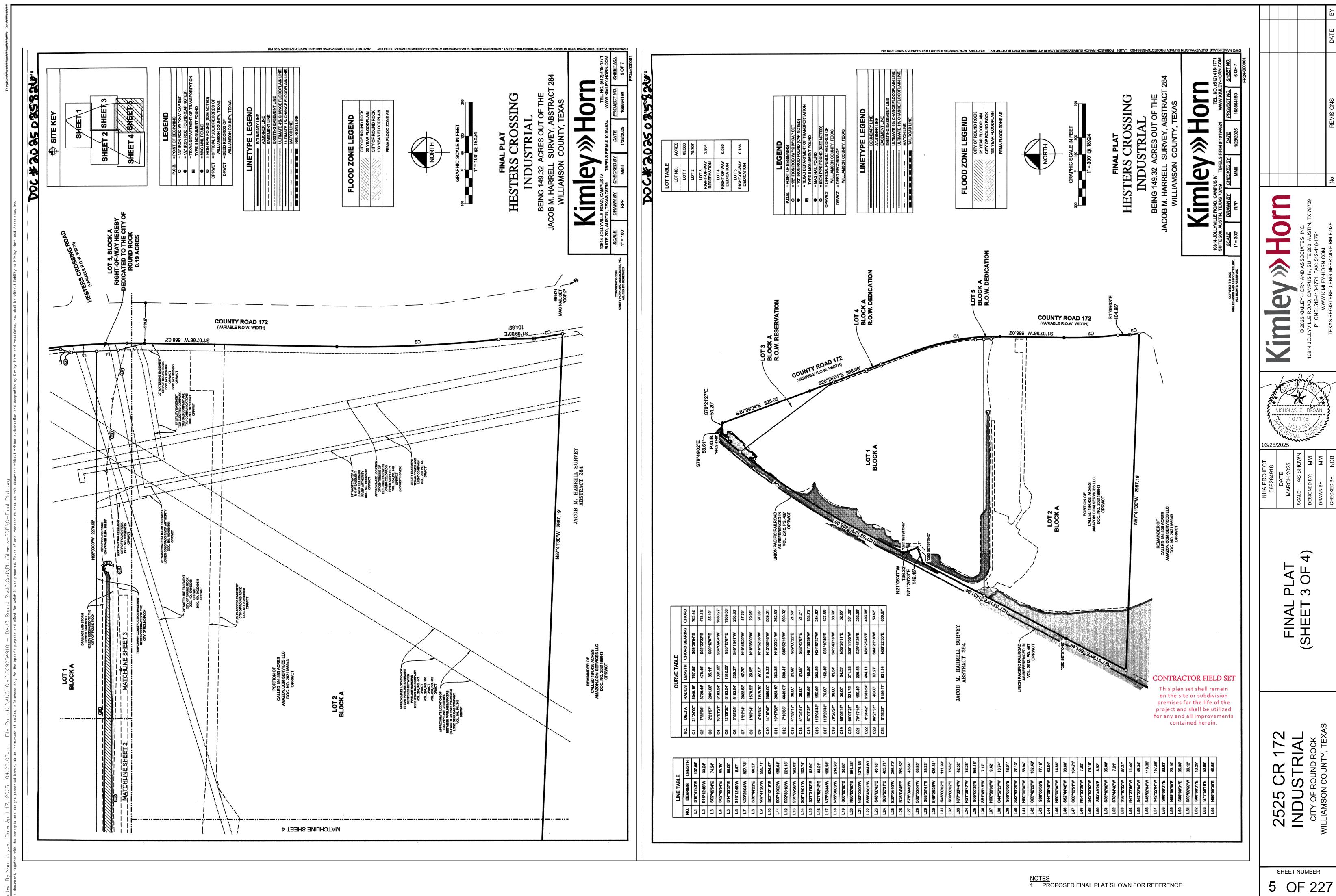
SHEET INDEX

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OF





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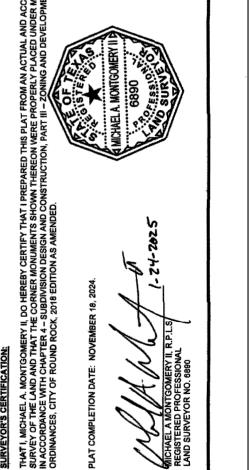
TION OF THIS TRACT IS ENCROACHED BY THE ULTIMATE 1% A LIM TWO (2) FEET ABOVE THE 1% ANNUAL CHANCE FLOODPLA NCES, STRUCTURES, STORAGE, OR FILL SHALL BE PLACED WES APPROVED BY THE CITY ENGINEER. FILL MAY ONLY BE PEI

JUNTY, TEXAS, DO HEREBY CERTIFY THAT THE PRILED FOR RECORD IN MY OFFICE ON 2007. AT 10:30 O'CLOCK MY, AND DI 2007. AT 10:30 O'CLOCK MY, IN THE PROPERTY OF WILLIAMSON C

WG NAME: K:NG\_SURVEYNUSTIN SURVEY PROJECTS/168964169 - LAUS1 - ROBINSON RANCH SURVEYNUNG/PLATV-PLAT-168864169. DWG PLOTTED BY PAZITNEY, ROB 11/18/2024 4:40 PM LAST SAVED 11/18/2024 4:35 PM

Kimley» BEING 149.32 AC ACOB M. HARRELL S WILLIAMSON

NOTES
1. PROPOSED FINAL PLAT SHOWN FOR REFERENCE.



CONTRACTOR FIELD SET This plan set shall remain

on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

2525 CR 172 INDUSTRIAL

4

PLAT 4 OF

FINAL (SHEET

SDP24-00001

SHEET NUMBER

6 OF 227

### CITY OF ROUND ROCK

- . ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE CITY OF ROUND ROCK (CORR) DESIGN AND CONSTRUCTION STANDARDS (DACS) SPECIFICATIONS MANUAL
- ANY EXISTING UTILITIES, PAVEMENT, CURBS, SIDEWALKS, STRUCTURES, TREES, ETC. (NOT PLANNED FOR DEMOLITION OR REMOVAL) THAT ARE DAMAGED OR REMOVED. SHALL BE REPAIRED, OR REPLACED, AT THE CONTRACTOR'S EXPENSE
- THE CONTRACTOR SHALL VERIFY ALL DEPTHS AND LOCATIONS OF EXISTING UTILITIES PRIOR TO ANY CONSTRUCTION ACTIVITIES. ANY DISCREPANCIES WITH THE CONSTRUCTION PLANS FOUND IN THE FIELD SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF WATER AND WASTEWATER NOTES: THE ENGINEER WHO SHALL BE RESPONSIBLE FOR REVISING THE PLANS AS APPROPRIATE. FAILURE TO COMPLETE THIS STEP PRIOR TO COMMENCEMENT OF CONSTRUCTION MAY RESULT IN SIGNIFICANT DELAYS AND/OR EXPENDITURES FOR WHICH THE CITY SHALL NOT BE HELD LIABLE.
- 4. MANHOLE FRAMES, COVERS, VALVES, CLEANOUTS, ETC. SHALL BE RAISED TO FINISHED GRADE PRIOR TO FINAL PAVING
- 5. THE CONTRACTOR SHALL PROVIDE THE CITY OF ROUND ROCK WITH A 48-HOUR NOTICE BEFORE BEGINNING EACH PHASE OF CONSTRUCTION. TELEPHONE (512) 218-5428 (PLANNING AND DEVELOPMENT SERVICES DEPARTMENT - PDS)
- ALL AREAS DISTURBED OR EXPOSED DURING CONSTRUCTION SHALL BE REVEGETATED IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. THIS INCLUDES ANY AREAS LOCATED OUTSIDE OF THE DEFINED LIMITS OF CONSTRUCTION (LOC). IN RIGHTS-OF-WAY (ROW), OR LOCATED ON ADJACENT PROPERTIES. REVEGETATION OF ALL DISTURBED OR EXPOSED AREAS SHALL CONSIST OF SODDING OR SEEDING, AT THE CONTRACTOR'S DISCREPANCY, AS OUTLINED IN THE CITY'S DESIGN AND CONSTRUCTION 4. ALL FIRE HYDRANT AND SPRINKLER LEADS SHALL BE DUCTILE IRON PIPE (AWWA C-100, MIN. CLASS 200). STANDARDS. THE TYPE OF REVEGETATION PROVIDED MUST BE EQUIVALENT TO OR EXCEED THE TYPE OF VEGETATION PRESENT PRIOR TO CONSTRUCTION.
- PRIOR TO ANY CONSTRUCTION, A PRE-CONSTRUCTION MEETING SHALL BE HELD BETWEEN THE CITY OF ROUND ROCK, THE DESIGN ENGINEER, THE CONTRACTORS, OTHER UTILITY COMPANIES, AND ANY AFFECTED PARTIES OR OTHER ENTITY THE

  6. THE CONTRACTOR SHALL CONTACT THE CITY OF ROUND ROCK INSPECTOR TO COORDINATE UTILITY TIE-INS AND NOTIFY THEM AT CITY OR DESIGN ENGINEER DEEM NECESSARY
- 8. THE CONTRACTOR AND THE DESIGN ENGINEER SHALL KEEP ACCURATE RECORDS OF ALL CONSTRUCTION THAT DEVIATES FROM THE PLANS. CHANGES TO APPROVED, CONSTRUCTION-STAMPED PLANS WILL REQUIRE A REVISION FROM THE DESIGN ENGINEER THAT IS APPROVED BY THE CITY PRIOR TO FIELD USE. THE DESIGN ENGINEER SHALL FURNISH THE CITY OF ROUND ROCK ACCURATE "AS-BUILT" RECORD DRAWINGS FOLLOWING COMPLETION OF ALL CONSTRUCTION. THESE "AS-BUILT" RECORD DRAWINGS SHALL MEET WITH THE SATISFACTION OF THE PLANNING AND DEVELOPMENT SERVICES DEPARTMENT PRIOR TO FINAL ACCEPTANCE OF THE
- THE CITY OF ROUND ROCK SHALL NOT BE PETITIONED FOR ACCEPTANCE UNTIL ALL NECESSARY EASEMENT DOCUMENTS HAVE BEEN SIGNED AND RECORDED.
- 10. WHENEVER CONSTRUCTION ACTIVITIES ARE TAKING PLACE WITHIN AN EXISTING EASEMENT, THE CONTRACTOR SHALL CONFINE THEIR WORK TO WITHIN THE BOUNDS OF SAID EASEMENT. PRIOR TO FINAL ACCEPTANCE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING ALL TRASH AND DEBRIS WITHIN ANY PERMANENT OR TEMPORARY EASEMENTS. CLEAN-UP SHALL BE TO THE SATISFACTION OF THE CITY OF ROUND ROCK CIVIL INSPECTOR AND/OR THE CITY ENGINEER.
- 11. PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR SHALL APPLY FOR AND SECURE ALL PROPER PERMITS FROM THE APPROPRIATE AUTHORITIES.
- 12. AVAILABLE PERMANENT BENCHMARKS (CITY OF ROUND ROCK DATUM) WITH VERTICAL DATUM INFORMATION THAT MAY BE UTILIZED FOR THE CONSTRUCTION OF THIS PROJECT AND ARE DESCRIBED AS FOLLOWS:

VERTICAL DATUM: NAVD88 GEOID: 18

# BM #51471 MAG NAIL SET "GCP 2"

- SET AT THE NORTHEAST CORNER OF THE INTERSECTION OF STATE HIGHWAY 45 AND COUNTRY ROAD 172, +/-5.5' FROM "YIELD"
- A: 10147640.83
- E: 3127942.60
- ELEV.= 841.70

# BM #51473 MAG NAIL SET "GCP 3"

- SET IN THE NORTH RIGHT-OF-WAY LINE OF HESTERS CROSSING RAOD, +/-560' FROM THE INTERSECTION FO HESTERS CROSSING ROAD AND COUNTY ROAD 172, +/-100' FROM CONCRETE SIGN "ENCOMPASS HEALTH".
  - N: 10149343.04
  - E: 3128315.81 ELEV.= 812.19'

### TRENCH SAFETY NOTES:

- IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) REGULATIONS, ALL TRENCHES OVER 5 FEET IN DEPTH, IN EITHER HARD AND COMPACT OR SOFT AND UNSTABLE SOIL, SHALL BE SLOPED, SHORED, SHEETED, BRACED OR OTHERWISE SUPPORTED. FURTHERMORE, ALL TRENCHES LESS THAN 5 FEET IN DEPTH SHALL ALSO BE EFFECTIVELY PROTECTED WHEN HAZARDOUS GROUND MOVEMENT MAY BE EXPECTED. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT SHALL BE PROVIDED AS PART OF A PACKAGE REQUIRED PRIOR TO THE PRE-CONSTRUCTION MEETING AND ANY CONSTRUCTION ACTIVITIES.
- IN ACCORDANCE WITH THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS, WHEN PERSONS ARE IN TRENCHES 4 FEET DEEP OR MORE, ADEQUATE MEANS OF EXIT, SUCH AS A LADDER OR STEPS, MUST BE PROVIDED AND LOCATED IN SUCH A MANNER AS TO REQUIRE NO MORE THAN 25 FEET OF LATERAL TRAVEL.
- IF TRENCH SAFETY SYSTEM DETAILS WERE NOT PROVIDED IN THE PLANS BECAUSE TRENCHES WERE ANTICIPATED TO BE LESS THAN 5 FEET IN DEPTH BUT, DURING CONSTRUCTION, IT IS FOUND THAT TRENCHES ARE IN FACT 5 FEET OR MORE IN DEPTH (OR) TRENCHES 19. THE CITY OF ROUND ROCK FIRE DEPARTMENT SHALL BE NOTIFIED (48) HOURS PRIOR TO THE TESTING OF ANY BUILDING SPRINKLER LESS THAN 5 FEET IN DEPTH ARE IN AN AREA WHERE HAZARDOUS GROUND MOVEMENT IS EXPECTED. ALL CONSTRUCTION SHALL CEASE, THE TRENCHED AREA SHALL BE BARRICADED AND THE DESIGN ENGINEER NOTIFIED IMMEDIATELY. CONSTRUCTION SHALL NOT RESUME UNTIL APPROPRIATE TRENCH SAFETY SYSTEM DETAILS. AS DESIGNED BY A PROFESSIONAL ENGINEER. ARE SUBMITTED TO THE CITY OF ROUND ROCK FOR REVIEW AND APPROVAL

# STREET AND DRAINAGE NOTES:

- ALL TESTING SHALL BE DONE BY AN INDEPENDENT LABORATORY AT THE OWNER'S EXPENSE. ANY RETESTING SHALL BE PAID FOR BY THE CONTRACTOR. A CITY INSPECTOR SHALL BE PRESENT DURING ALL TESTS. TESTING SHALL BE COORDINATED WITH THE CITY INSPECTOR, AND THEY SHALL BE GIVEN A MINIMUM 24-HOUR NOTICE PRIOR TO ANY TESTING.
- PUBLIC ROADWAYS CONSTRUCTED AS PART OF ANY DEVELOPMENT PERMIT SHALL BE FREE FROM DEFECTS, PATCHES, OR REPAIRS PRIOR TO ACCEPTANCE BY THE CITY OF ROUND ROCK. ROADWAYS SHALL HAVE A CLEAR SURFACE FREE FROM ANY GOUGES. MARRING, OR CRACKING TO BE CONSIDERED SUITABLE TO THE CITY OF ROUND ROCK TRANSPORTATION DEPT. NO NEW ROADWAYS SHALL BE ACCEPTED UNTIL ALL CONSTRUCTION TRAFFIC RELATED TO THIS OR ANY ASSOCIATED PERMIT HAS CEASED, AND THE ROADWAY IS OPEN TO AND EXCLUSIVELY USED BY THE GENERAL PUBLIC.
- BACKFILL BEHIND THE CURB SHALL BE COMPACTED TO OBTAIN A MINIMUM OF 95% MAXIMUM DENSITY TO WITHIN 3" OF TOP OF CURB. MATERIAL USED SHALL BE PRIMARILY GRANULAR WITH NO ROCKS LARGER THAN 6" IN THE GREATEST DIMENSION. THE REMAINING 3" SHALL BE CLEAN TOPSOIL FREE FROM ALL CLUMPS AND SUITABLE FOR SUSTAINING PLANT LIFE.
- 4. THE DEPTH OF COVER FOR ALL CROSSINGS UNDER PAVEMENT INCLUDING GAS, ELECTRIC, TELEPHONE, CABLE TV, WATER SERVICES, TRAFFIC MARKING NOTES: ETC. SHALL BE A MINIMUM OF 30" BELOW SUBGRADE.
- 5. STREET RIGHTS-OF-WAY SHALL BE GRADED AT A SLOPE OF 1/4" PER FOOT TOWARD THE CURB UNLESS OTHERWISE INDICATED. HOWEVER, IN NO CASE SHALL THE WIDTH OF RIGHT-OF-WAY AT 1/4" PER FOOT SLOPE BE LESS THAN 10 FEET UNLESS A SPECIFIC REQUEST FOR AN ALTERNATE GRADING SCHEME IS SUBMITTED TO AND APPROVED BY THE CITY OF ROUND ROCK PLANNING AND DEVELOPMENT SERVICES DEPARTMENT.
- BARRICADES, BUILT TO CITY OF ROUND ROCK STANDARDS, SHALL BE CONSTRUCTED ON ALL DEAD-END STREETS AND, AS NECESSARY, DURING CONSTRUCTION TO MAINTAIN JOB AND PUBLIC SAFETY.
- 7. ALL REINFORCED CONCRETE PIPE (RCP) SHALL BE MINIMUM CLASS ILL. ALL PUBLIC RCP SHALL BE A MINIMUM OF 18-INCHES IN
- THE SUBGRADE MATERIAL FOR THE STREETS SHOWN HEREIN WAS TESTED BY ECS SOUTHWEST, LLP ON THIS DATE: MARCH 22, 2024 AND THE PAVING SECTIONS DESIGNED IN ACCORDANCE WITH THE CURRENT CITY OF ROUND ROCK DESIGN CRITERIA. THE PAVING SECTIONS ARE TO BE CONSTRUCTED AS FOLLOWS:

# PAVEMENT SECTION PORTLAND CEMENT ASPHALT SURFACE ASPHALT BASE COURSE LIME STAB. THICKNESS

STANDARD DUTY (RIGID)	5 IN			4 IN
STANDARD DUTY (ASPHALT)		2 IN	3 IN	15 IN
HEAVY-DUTY (RIGID)	8 IN			6 IN
HEAVY-DUTY (ASPHALT)	•	2 IN	4 IN	19 IN
EXTRA HEAVY-DUTY	8 IN			6 IN

THE GEOTECHNICAL ENGINEER SHALL INSPECT THE SUBGRADE FOR COMPLIANCE WITH THE DESIGN ASSUMPTIONS MADE DURING PREPARATION OF THE ACCEPTED GEOTECHNICAL REPORT. ANY ADJUSTMENTS THAT ARE REQUIRED SHALL BE MADE THROUGH REVISION OF THE CONSTRUCTION PLANS AND ADDENDUM TO ANY ACCEPTED GEOTECHNICAL REPORT.

9. WHERE PLASTICITY INDEX (PI) IS OVER 20, SUBGRADES MUST BE STABILIZED UTILIZING A METHOD ACCEPTABLE TO THE PLANNING AND DEVELOPMENT SERVICES DEPARTMENT. THE GEOTECHNICAL ENGINEER SHALL RECOMMEND AN APPROPRIATE SUBGRADE STABILIZATION IF SULFATES ARE DETERMINED TO BE PRESENT. WHEN UTILIZING LIME FOR SOIL STABILIZATION, PLACEMENT SHALL BE IN THE FORM OF LIME SLURRY, NOT PELLETS.

10. FIELD DENSITY CONTROL REQUIREMENTS

TEX-115-E

- MOISTURE CONTENT SOIL DESCRIPTION DENSITY PERCENT
- TEX-115-E  $\geq$  98% D<sub>A</sub>\* AND  $\leq$  105% D<sub>A</sub> N/A  $\geq$  98% D<sub>A</sub> AND  $\leq$  102% D<sub>A</sub> 15 ≤ PI ≤ 35 ≥ W<sub>OPT</sub> + 3% PI > 35 ≥ 95% D<sub>A</sub> AND ≤100% D<sub>A</sub>

- PIPE MATERIAL FOR WATER MAINS SHALL BE PVC (AWWA C-900, MIN. CLASS 200), OR DUCTILE IRON (AWWA C-100, MIN. CLASS 200). WATER SERVICES (2" OR LESS) SHALL BE POLYETHYLENE TUBING (BLACK, 200 PSI, DR 9).
- 2. PIPE MATERIAL FOR PRESSURE WASTEWATER MAINS SHALL BE PVC (AWWA C-900, MIN. CLASS 150), SDR26 HIGHER PRESSURE RATED (160 PSI), OR DUCTILE IRON (AWWA C-100, MIN. CLASS 200). PIPE MATERIAL FOR GRAVITY WASTEWATER MAINS SHALL BE SDR26 PVC, 8. EMERGENCY RESPONDER RADIO COVERAGE: ADEQUATE EMERGENCY RESPONDER RADIO COVERAGE SHALL BE REQUIRED FOR ALL PVC (ASTM D2241 OR D3034, MAX. DR-26), DUCTILE IRON (AWWA C-100, MIN. CLASS 200).
- 3. UNLESS OTHERWISE ACCEPTED BY THE PLANNING AND DEVELOPMENT SERVICES DEPARTMENT, MINIMUM DEPTH OF COVER FOR ALL LINES OUTSIDE OF THE PAVED AREAS SHALL BE 42" BELOW FINISHED GRADE AND 30" BELOW SUBGRADE FOR ALL LINES LOCATED IN
- 5. ALL DUCTILE IRON PIPE AND FITTINGS SHALL BE WRAPPED WITH A MINIMUM OF 8-MIL POLYETHYLENE AND SEALED WITH DUCT TAPE OR EQUAL ACCEPTED BY THE CITY OF ROUND ROCK CIVIL INSPECTOR.
- LEAST 48 HOURS PRIOR TO CONNECTING TO ANY EXISTING LINES.
- ALL MANHOLES SHALL BE CONCRETE WITH CAST IRON RING AND COVER. ALL MANHOLES LOCATED OUTSIDE OF THE PAVEMENT SHALL HAVE BOLTED COVERS. CORE CONNECTIONS TO FIBERGLASS MANHOLES ARE PROHIBITED.
- THE CONTRACTOR MUST OBTAIN A BULK WATER PERMIT OR PURCHASE AND INSTALL A WATER METER FOR ALL WATER USED DURING CONSTRUCTION. A COPY OF THIS PERMIT MUST ALWAYS BE POSSESSED BY ANY PARTIES WHO UTILIZE WATER. CONTACT WATER DISTRIBUTION AT (512) 801-4435 FOR ADDITIONAL INFORMATION.
- 9. LINE FLUSHING, OR ANY ACTIVITY USING A LARGE QUANTITY OF WATER, MUST BE SCHEDULED A MINIMUM (10) DAYS IN ADVANCE WITH CONSTRUCTION SUMMARY THE CITY OF ROUND ROCK CIVIL INSPECTOR.
- 10. THE CONTRACTOR, AT HIS EXPENSE, SHALL PERFORM STERILIZATION OF ALL POTABLE WATER LINES CONSTRUCTED AND SHALL PROVIDE ALL EQUIPMENT (INCLUDING TEST GAUGES), SUPPLIES (INCLUDING CONCENTRATED CHLORINE DISINFECTING MATERIAL) AND NECESSARY LABOR REQUIRED FOR THE STERILIZATION PROCEDURE. THE STERILIZATION PROCEDURE SHALL BE MONITORED BY THE CITY OF ROUND ROCK CIVIL INSPECTOR. WATER SAMPLES WILL BE COLLECTED BY THE CITY OF ROUND ROCK TO VERIFY EACH TREATED LINE HAS ATTAINED AN INITIAL CHLORINE CONCENTRATION OF 50 PPM, WHERE MEANS OF FLUSHING IS NECESSARY, THE CONTRACTOR, AT HIS EXPENSE, SHALL PROVIDE FLUSHING DEVICES AND REMOVE SAID DEVICES PRIOR TO FINAL ACCEPTANCE BY THE CITY OF ROUND ROCK.
- 11. SAMPLING TAPS SHALL BE BROUGHT UP TO 3 FEET ABOVE GRADE AND SHALL BE EASILY ACCESSIBLE FOR CITY PERSONNEL. AT THE CONTRACTOR'S REQUEST, AND IN THEIR PRESENCE, SAMPLES FOR BACTERIOLOGICAL TESTING WILL BE COLLECTED BY THE CITY OF ROUND ROCK NOT LESS THAN (24) HOURS AFTER THE TREATED LINE HAS BEEN FLUSHED OF THE CONCENTRATED CHLORINE SOLUTION AND CHARGED WITH WATER APPROVED BY THE CITY. THE CONTRACTOR SHALL SUPPLY A CHECK OR MONEY ORDER, PAYABLE TO THE CITY OF ROUND ROCK, TO COVER THE FEE CHARGED FOR TESTING EACH WATER SAMPLE. FEE AMOUNTS MAY BE OBTAINED BY CONTACTING THE CITY OF ROUND ROCK ENVIRONMENTAL SERVICES LABORATORY AT (512) 218-5561 OR WATERLAB@ROUNDROCKTEXAS.GOV.
- 12. THE CONTRACTOR, AT THEIR EXPENSE, SHALL PERFORM QUALITY TESTING FOR ALL WASTEWATER PIPE INSTALLED AND PRESSURE PIPE HYDROSTATIC TESTING OF ALL WATERLINES CONSTRUCTED. THE CONTRACTOR SHALL PROVIDE ALL EQUIPMENT (INCLUDING PUMPS AND GAUGES), SUPPLIES, AND LABOR NECESSARY TO PERFORM THESE TESTS. QUALITY AND PRESSURE TESTING SHALL BE MONITORED BY THE CITY OF ROUND ROCK CIVIL INSPECTOR.
- 13. THE CONTRACTOR SHALL COORDINATE TESTING WITH THE CITY OF ROUND ROCK CIVIL INSPECTOR AND PROVIDE NO LESS THAN (24) HOURS OF NOTICE PRIOR TO PERFORMING STERILIZATION, QUALITY TESTING, OR PRESSURE TESTING.
- 14. THE CONTRACTOR (OR SUBCONTRACTORS) SHALL NOT OPEN OR CLOSE ANY VALVES UNLESS DIRECTED TO DO SO BY CITY OF ROUND ROCK PERSONNEL.

#### 15. ALL VALVE BOXES AND COVERS SHALL BE CAST IRON.

- 16. ALL WATER SERVICE, WASTEWATER SERVICE AND VALVE LOCATIONS SHALL BE APPROPRIATELY MARKED AS FOLLOWS:
- WATER SERVICE "W" ON TOP OF CURB (BLUE COLOR)
- WASTEWATER SERVICE "S" ON TOP OF CURB "V" ON FACE OF CURB
- 17. TOOLS FOR MARKING THE CURB SHALL BE PROVIDED BY THE CONTRACTOR. OTHER APPROPRIATE MEANS OF MARKING SERVICE AND DESIGN ENGINEER AND APPROVED BY THE CITY OF ROUND ROCK.
- 18. CONTACT THE CITY OF ROUND ROCK UTILITIES AND ENVIRONMENTAL SERVICES (UES) DEPARTMENT FOR ASSISTANCE IN DETERMINING EXISTING WATER AND WASTEWATER LOCATIONS.
- PIPING SO THAT THEY MAY BE PRESENT TO MONITOR SUCH TESTING.
- 20. SAND, AS DESCRIBED IN SPECIFICATION ITEM 510 PIPE, SHALL NOT BE USED AS BEDDING FOR WATER AND WASTEWATER LINES. ACCEPTABLE BEDDING MATERIALS ARE PIPE BEDDING STONE, PEA GRAVEL AND, IN LIEU OF SAND, A NATURALLY OCCURRING OR MANUFACTURED STONE MATERIAL CONFORMING TO ASTM C33 FOR STONE QUALITY AND MEETING THE FOLLOWING GRADATION SPECIFICATION:

# PERCENT RETAINED BY WEIGHT

3/8"	0-2
#4	40-85
#10	95-100

- 21. THE CONTRACTOR IS HEREBY NOTIFIED THAT CONNECTING TO, SHUTTING DOWN, OR TERMINATING EXISTING UTILITY LINES MAY HAVE TO OCCUR AT OFF-PEAK HOURS. SUCH HOURS ARE USUALLY OUTSIDE NORMAL WORKING HOURS (7AM - 4 PM) AND POSSIBLY BETWEEN 12 AM AND 6 AM.
- 22. ALL WASTEWATER CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ) REGULATIONS, 30 TAC CHAPTER 213 AND 217, AS APPLICABLE. ALL WATER CONSTRUCTION SHALL BE IN ACCORDANCE WITH TCEQ REGULATIONS, 30 TAC CHAPTER 290. WHENEVER TCEQ AND CITY OF ROUND ROCK SPECIFICATIONS CONFLICT, THE MORE STRINGENT

- ANY METHODS, STREET MARKINGS AND SIGNAGE NECESSARY FOR WARNING MOTORISTS, WARNING PEDESTRIANS, OR DIVERTING TRAFFIC DURING CONSTRUCTION SHALL CONFORM TO THE TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS (TMUTCD), LATEST EDITION.
- 2. ALL PAVEMENT MARKINGS, MARKERS, PAINT, TRAFFIC BUTTONS, TRAFFIC CONTROLS, AND SIGNS SHALL BE INSTALLED IN ACCORDANCE WITH THE TEXAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR CONSTRUCTION OF HIGHWAYS, STREETS AND BRIDGES AND, THE TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS, LATEST EDITIONS.

# **EROSION AND SEDIMENTATION CONTROL NOTES:**

- EROSION CONTROL MEASURES, SITE WORK, AND RESTORATION WORK SHALL BE IN ACCORDANCE WITH THE CITY OF ROUND ROCK DESIGN AND CONSTRUCTION STANDARDS (DACS) AND CODE OF ORDINANCES.
- 2. ALL SLOPES SHALL BE SODDED OR SEEDED WITH APPROVED GRASS, GRASS MIXTURES, OR GROUND COVER THAT IS SUITABLE TO THE AREA AND THE SEASON IN WHICH THEY ARE APPLIED.
- 3. SILT FENCES, ROCK BERMS, SEDIMENTATION BASINS, AND SIMILARLY RECOGNIZED TECHNIQUES AND MATERIALS SHALL BE EMPLOYED DURING CONSTRUCTION TO PREVENT POINT SOURCE SEDIMENTATION LOADING OF DOWNSTREAM FACILITIES. INSTALLATION AND CONDITION SHALL BE REGULARLY INSPECTED BY THE CITY OF ROUND ROCK FOR EFFECTIVENESS. ADDITIONAL MEASURES MAY BE REQUIRED IF, IN THE OPINION OF THE CITY ENGINEER, THEY ARE WARRANTED.
- 4. ALL TEMPORARY EROSION CONTROL MEASURES SHALL NOT BE REMOVED UNTIL REVEGETATION HAS BEEN ESTABLISHED AND APPROVAL RECEIVED FROM THE CIVIL INSPECTOR. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN ALL TEMPORARY EROSION CONTROL STRUCTURES AND TO REMOVE ALL ONCE APPROVED TO DO SO BY THE CIVIL INSPECTOR.
- 5. ALL MUD, DIRT, ROCKS, DEBRIS, ETC., SPILLED, TRACKED, OR OTHERWISE DEPOSITED ON EXISTING PAVED STREETS, DRIVES AND AREAS USED BY THE PUBLIC SHALL BE CLEANED UP IMMEDIATELY.

# ROUND ROCK FIRE DEPARTMENT NOTES:

- 1. GENERAL: ALL DEVELOPMENTS SHALL COMPLY WITH THE CURRENT FIRE CODE, APPENDICES, AND ANY LOCAL AMENDMENTS AS ADOPTED BY THE CITY OF ROUND ROCK.
- COMBUSTIBLE MATERIALS ON-SITE: ALL-WEATHER ACCESS ROADS/DRIVES (ASPHALT/CONCRETE CAPABLE OF SUPPORTING 80,000 LB. APPARATUS LOADING) SHALL BE CONSTRUCTED, AND ALL WATER LINES SHALL BE TESTED AND FIRE HYDRANTS IN-SERVICE, PRIOR TO BRINGING COMBUSTIBLE MATERIALS (WOOD, PACKAGING, PLASTICS, ETC.) ON ANY JOB SITE. BASE MATERIAL IS NOT ACCEPTABLE FOR FIRE ACCESS ROADS/DRIVES

- 3. FIRE LANES: FIRE APPARATUS ACCESS ROADS/DRIVES SHALL HAVE A MINIMUM UNOBSTRUCTED WIDTH OF (20) FEET, WHERE TRAFFIC IS TWO-WAY DIRECTIONAL, BUILDINGS EXCEED (30) FEET OR THREE STORIES IN HEIGHT, TOTAL BUILDING AREA EXCEEDS 62,000 SQUARE FEET, OR WHERE HYDRANTS ARE LOCATED ALONG THE FIRE ACCESS ROADS, THE MINIMUM WIDTH SHALL BE (26) FEET. IF RAISED CURBING OR MEDIANS COMPROMISE MINIMUM WIDTH, CURBING SHALL BE MOUNTABLE AND RAISED AREA SHALL CONTAIN NO OBSTRUCTIONS SUCH AS LANDSCAPING, SIGNAGE, GROUND-MOUNTED EQUIPMENT, ETC.
- 4. ALL-WEATHER SURFACE: THE PAVEMENT STRUCTURE FOR FIRE ACCESS ROADS/DRIVES MUST BE ALL- WEATHER SURFACE (ASPHALT/CONCRETE) DESIGNED TO SUPPORT AN 80,000 LB. APPARATUS LOADING.
- 5. GRADE: THE GRADE THROUGH THE FIRE LANE ACCESS SHALL NOT EXCEED 7% AND NO GRADE BREAKS SHALL EXCEED 3%.
- 6. TURNING RADII: TURNING RADII SHALL BE A MINIMUM OF 25-FTT INSIDE AND 50-FT OUTSIDE AS MEASURED FROM FACE-OF-CURB (WHEN PRESENT) OR ON DRIVABLE, PAVED SURFACE.
- 7. VERTICAL CLEARANCE: THE VERTICAL CLEARANCE OVER A DESIGNATED FIRE LANE SHALL NOT BE LESS THAN 13'-6".
- NEW BUILDINGS. A PRE-ENHANCEMENT RADIO SURVEY SHALL BE REQUIRED AT THE 80% CONSTRUCTION PHASE FOR CERTAIN BUILDING TYPES BASED ON THE SIZE OF THE BUILDING. PRE-ENHANCEMENT RADIO SURVEY REQUIREMENTS INCLUDE THE
- FOLLOWING BUILDING TYPES: GREATER THAN (5) STORIES
- BELOW GRADE PLANE
- WOOD FRAMED CONSTRUCTION GREATER THAN 50,000 SF
- CONCRETE OR METAL FRAMED CONSTRUCTION GREATER THAN 25,000 SF
- REQUIRED FIRE FLOWS: A PROJECT'S MINIMUM FIRE FLOW FOR THE LARGEST BUILDING SHALL BE MEASURED AT (20) PSI RESIDUAL PRESSURE THAT IS AVAILABLE FOR FIREFIGHTING PER THE FLOWS ON TABLES B105.1 OR B105.2 OF THE INTERNATIONAL FIRE CODE (IFC), APPENDIX B. DISCLAIMER: IT IS THE RESPONSIBILITY OF THE DEVELOPER AND ENGINEER TO ENSURE THESE MINIMUM FIRE FLOW REQUIREMENTS FOR THE SITE ARE MET VIA FLOW TESTING AND WATER MODELING.
- 10. SPRINKER SYSTEMS: BUILDINGS EQUIPPED WITH ANY FIRE DEPARTMENT CONNECTIONS (FDC) SHALL HAVE A FIRE HYDRANT LOCATED WITHIN 100' OF THE FDC (REMOTE FDC IS PERMISSIBLE). FDC SHALL BE IDENTIFIED ON THE SITE VIA SIGNAGE.
- 11. GATES: IF GATES ARE PROVIDED ALONG ANY FIRE ACCESS ROAD/DRIVE, MINIMUM PASSABLE WIDTH SHALL NOT BE LESS THAN (20) FEET AND SHALL COMPLY WITH IFC APPENDIX D AND ROUND ROCK CODE OF ORDINANCES REGARDING EMERGENCY ACCESS SYSTEMS. GATES WILL REQUIRE A KNOX-BOX® KEY BOX THAT SHALL CONTAIN KEYS TO GAIN NECESSARY ACCESS AS REQUIRED BY

Dino Sizo	WATER			
Pipe Size	TYPE	LENGTH (LF)		
2"	BLACK POLY DR9	60		
12"	DUCTILE IRON	324		

VALVE	S
Size	Total
12"	8

#### THESE PLAN AND GENERAL NOTES REFER TO: GEOTECHNICAL REPORT BY ECS SOUTHWEST, LLP, ECS PROJECT NUMBER 17:6399-A DATED: MARCH 22, 2024

REPORT THAT MAY HAVE BEEN RELEASED AFTER THE NOTED DATE.

INCLUDING ALL REVISIONS AND ADDENDA TO THE

# CONTRACTOR FIELD SET

This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

# BENCHMARKS

BM #51471 MAG NAIL SET "GCP 2" SET AT THE NORTHEAST CORNER OF THE INTERSECTION OF STATE HIGHWAY 45 AND COUNTY ROAD 172, ±5.5' FROM "YIELD" SIGN. I: 10147640.83 E: 3127942.60 ELEV.= 841.70'

FI FV = 812.19'

BM #51473 MAG NAIL SET "GCP 3" SET IN THE NORTH RIGHT-OF-WAY LINE OF HESTERS CROSSING ROAD, ±560' FROM THE INTERSECTION OF HESTERS CROSSING ROAD AND COUNTY ROAD 172, ±100 FROM CONCRETE SIGN "ENCOMPASS HEALTH". E: 3128315.81

SHEET NUMBER

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SPECIFICATIONS OR DETAILS, THE MORE RESTRICTIVE SPECIFICATION AND DETAIL SHALL BE FOLLOWED 2. THE CONTRACTOR SHALL COMPLY WITH CITY (OR TOWN) "GENERAL NOTES" FOR CONSTRUCTION. IF EXISTING AND REQUIRED BY THE CITY. FOR INSTANCES WHERE THEY CONFLICT WITH THESE KH GENERAL NOTES, THEN THE MORE RESTRICTIVE SHALL APPLY. 3. THE CONTRACTOR SHALL FURNISH ALL MATERIAL AND LABOR TO CONSTRUCT THE FACILITY AS SHOWN AND DESCRIBED IN THE CONSTRUCTION DOCUMENTS IN ACCORDANCE WITH THE APPROPRIATE AUTHORITIES' SPECIFICATIONS AND REQUIREMENTS.

4. THE CONTRACTOR SHALL VISIT THE SITE PRIOR TO BIDDING TO DETERMINE EXISTING CONDITIONS. 5. THE EXISTING CONDITIONS SHOWN ON THESE PLANS WERE PROVIDED BY THE TOPOGRAPHIC SURVEY PREPARED BY THE PROJECT SURVEYOR, AND ARE BASED ON THE BENCHMARKS SHOWN. THE CONTRACTOR SHALL REFERENCE THE SAME BENCHMARKS. 6. THE CONTRACTOR SHALL REVIEW AND VERIFY THE EXISTING TOPOGRAPHIC SURVEY SHOWN ON THE PLANS REPRESENTS EXISTING FIELD CONDITIONS PRIOR TO CONSTRUCTION, AND SHALL REPORT ANY DISCREPANCIES FOUND TO THE OWNER AND ENGINEER

7. IF THE CONTRACTOR DOES NOT ACCEPT THE EXISTING TOPOGRAPHIC SURVEY AS SHOWN ON THE PLANS, WITHOUT EXCEPTION, THEN THE CONTRACTOR SHALL SUPPLY AT THEIR OWN EXPENSE, A TOPOGRAPHIC SURVEY BY A REGISTERED PROFESSIONAL LAND SURVEYOR TO THE OWNER AND ENGINEER FOR REVIEW. 8. CONTRACTOR SHALL PROVIDE ALL CONSTRUCTION SURVEYING AND STAKING.

9. CONTRACTOR SHALL VERIFY HORIZONTAL AND VERTICAL CONTROL, INCLUDING BENCHMARKS PRIOR TO COMMENCING CONSTRUCTION OR STAKING OF IMPROVEMENTS. PROPERTY LINES AND CORNERS SHALL BE HELD AS THE HORIZONTAL CONTROL. 10. THE CONTRACTOR SHALL REVIEW AND VERIFY ALL DIMENSIONS, ELEVATIONS, AND FIELD CONDITIONS THAT MAY AFFECT CONSTRUCTION. ANY DISCREPANCIES ON THE DRAWINGS SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE COMMENCING WORK. NO FIELD CHANGES OR DEVIATIONS FROM DESIGN ARE TO BE MADE WITHOUT PRIOR APPROVAL OF THE ARCHITECT, ENGINEER, AND IF APPLICABLE THE CITY AND OWNER. NO CONSIDERATION WILL BE GIVEN TO CHANGE ORDERS FOR WHICH THE CITY, ENGINEER, AND OWNER WERE NOT CONTACTED PRIOR TO CONSTRUCTION OF THE AFFECTED ITEM. 11. CONTRACTOR SHALL THOROUGHLY CHECK COORDINATION OF CIVIL, LANDSCAPE, MEP, ARCHITECTURAL, AND OTHER PLANS PRIOR TO COMMENCING CONSTRUCTION. OWNER/ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY PRIOR TO COMMENCING WITH

12.IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE VARIOUS UTILITY COMPANIES WHICH MAY HAVE BURIED OR AERIAL UTILITIES WITHIN OR NEAR THE CONSTRUCTION AREA BEFORE COMMENCING WORK TO HAVE THEM LOCATE THEIR EXISTING UTILITIES PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE AN ADEQUATE MINIMUM NOTICE TO ALL UTILITY COMPANIES PRIOR TO BEGINNING CONSTRUCTION.

13. CONTRACTOR SHALL CALL TEXAS 811 AN ADEQUATE AMOUNT OF TIME PRIOR TO COMMENCING CONSTRUCTION OR ANY EXCAVATION. 14. CONTRACTOR SHALL USE EXTREME CAUTION AS THE SITE CONTAINS VARIOUS KNOWN AND UNKNOWN PUBLIC AND PRIVATE UTILITIES. 15. THE LOCATIONS, ELEVATIONS, DEPTH, AND DIMENSIONS OF EXISTING UTILITIES SHOWN ON THE PLANS WERE OBTAINED FROM AVAILABLE UTILITY COMPANY MAPS AND PLANS, AND ARE CONSIDERED APPROXIMATE AND INCOMPLETE. IT SHALL BE THE CONTRACTORS' RESPONSIBILITY TO VERIFY THE PRESENCE, LOCATION, ELEVATION, DEPTH, AND DIMENSION OF EXISTING UTILITIES SUFFICIENTLY IN ADVANCE OF CONSTRUCTION SO THAT ADJUSTMENTS CAN BE MADE TO PROVIDE ADEQUATE CLEARANCES. THE ENGINEER SHALL BE NOTIFIED WHEN A DEVELOPED IMPROVEMENT CONFLICTS WITH AN EXISTING UTILITY

16. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING ANY ADJUSTMENTS AND RELOCATIONS OF EXISTING UTILITIES THAT CONFLICT WITH THE DEVELOPED IMPROVEMENTS, INCLUDING BUT NOT LIMITED TO, ADJUSTING EXISTING MANHOLES TO MATCH DEVELOPED GRADE, RELOCATING EXISTING POLES AND GUY WIRES THAT ARE LOCATED IN DEVELOPED DRIVEWAYS, ADJUSTING THE HORIZONTAL OR VERTICAL ALIGNMENT OF EXISTING UNDERGROUND UTILITIES TO ACCOMMODATE DEVELOPED GRADE OR CROSSING WITH A DEVELOPED UTILITY, AND ANY OTHERS THAT MAY BE ENCOUNTERED THAT ARE UNKNOWN AT THIS TIME AND NOT SHOWN ON THESE PLANS 17. CONTRACTOR SHALL ARRANGE FOR OR PROVIDE, AT ITS EXPENSE, ALL GAS, TELECOMMUNICATIONS, CABLE, OVERHEAD AND

8. CONTRACTOR IS RESPONSIBLE FOR COORDINATING INSTALLATION OF FRANCHISE UTILITIES THAT ARE NECESSARY FOR ON-SITE AND OFF-SITE CONSTRUCTION, AND SERVICE TO THE DEVELOPED DEVELOPMENT. 19. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR ALL DAMAGES DUE TO THE CONTRACTORS' FAILURE TO EXACTLY LOCATE AND PRESERVE ALL UTILITIES. THE OWNER OR ENGINEER WILL ASSUME NO LIABILITY FOR ANY DAMAGES SUSTAINED OR COST INCURRED BECAUSE OF THE OPERATIONS IN THE VICINITY OF EXISTING UTILITIES OR STRUCTURES. IF IT IS NECESSARY TO SHORE, BRACE, SWING OR RELOCATE A UTILITY, THE UTILITY COMPANY OR DEPARTMENT AFFECTED SHALL BE CONTACTED BY THE CONTRACTOR AND THEIR

UNDERGROUND POWER LINE, AND UTILITY POLE ADJUSTMENTS NEEDED.

SUBSEQUENT ADDENDA

PERMISSION OBTAINED REGARDING THE METHOD TO USE FOR SUCH WORK. 20.BRACING OF UTILITY POLES MAY BE REQUIRED BY THE UTILITY COMPANIES WHEN TRENCHING OR EXCAVATING IN CLOSE PROXIMITY TO THE POLES. THE COST OF BRACING POLES WILL BE BORNE BY THE CONTRACTOR, WITH NO SEPARATE PAY ITEM FOR THIS WORK. THE COST IS INCIDENTAL TO THE PAY ITEM.

21.CONTRACTOR SHALL USE ALL NECESSARY SAFETY PRECAUTIONS TO AVOID CONTACT WITH OVERHEAD AND UNDERGROUND POWER LINES. CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE LOCAL, STATE, FEDERAL AND UTILITY OWNER REGULATIONS PERTAINING TO WORK SETBACKS FROM POWER LINES 22.THE CONTRACTOR SHALL BE RESPONSIBLE TO OBTAIN ALL REQUIRED CONSTRUCTION PERMITS, APPROVALS, AND BONDS PRIOR TO

CONSTRUCTION. 23. THE CONTRACTOR SHALL HAVE AVAILABLE AT THE JOB SITE AT ALL TIMES A COPY OF THE CONTRACT DOCUMENTS INCLUDING PLANS. GEOTECHNICAL REPORT AND ADDENDA, PROJECT AND CITY SPECIFICATIONS, AND SPECIAL CONDITIONS, COPIES OF ANY REQUIRED CONSTRUCTION PERMITS FROSION CONTROL PLANS SWPPP AND INSPECTION REPORTS 24.ALL SHOP DRAWINGS AND OTHER DOCUMENTS THAT REQUIRE ENGINEER REVIEW SHALL BE SUBMITTED BY THE CONTRACTOR

SUFFICIENTLY IN ADVANCE OF CONSTRUCTION OF THAT ITEM, SO THAT NO LESS THAN 10 BUSINESS DAYS FOR REVIEW AND RESPONSE

25.ALL NECESSARY INSPECTIONS AND/OR CERTIFICATIONS REQUIRED BY CODES, JURISDICTIONAL AGENCIES, AND/OR UTILITY SERVICE COMPANIES SHALL BE PERFORMED PRIOR TO USE OF THE FACILITY AND THE FINAL CONNECTION OF SERVICES. 26.CONTRACTOR SHALL ARRANGE FOR REQUIRED CITY INSPECTIONS.

27. CONTRACTOR'S BID PRICE SHALL INCLUDE ALL INSPECTION FEES 28.ALL SYMBOLS SHOWN ON THESE PLANS (E.G. FIRE HYDRANT, METERS, VALVES, INLETS, ETC....) ARE FOR PRESENTATION PURPOSES ONLY AND ARE NOT TO SCALE. CONTRACTOR SHALL COORDINATE FINAL SIZES AND LOCATIONS WITH APPROPRIATE CITY INSPECTOR. 29. THE SCOPE OF WORK FOR THE CIVIL IMPROVEMENTS SHOWN ON THESE PLANS TERMINATES 5-FEET FROM THE BUILDING. REFERENCE THE BUILDING PLANS (E.G. ARCHITECTURAL, STRUCTURAL, MEP) FOR AREAS WITHIN 5-FEET OF THE BUILDING AND WITHIN THE BUILDING FOOTPRINT.

30.REFER TO ARCHITECTURAL AND STRUCTURAL PLANS FOR ALL FINAL BUILDING DIMENSIONS. 31. THE DEVELOPED BUILDING FOOTPRINT(S) SHOWN IN THESE PLANS WAS PROVIDED TO KIMLEY-HORN AND ASSOCIATES, INC. (KH) BY THE PROJECT ARCHITECT AT THE TIME THESE PLANS WERE PREPARED. IT MAY NOT BE THE FINAL CORRECT VERSION BECAUSE THE BUILDING DESIGN WAS ONGOING. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONFIRMING THE FINAL CORRECT VERSION OF THE BUILDING FOOTPRINT WITH THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO LAYOUT. DIMENSIONS AND/OR COORDINATES SHOWN ON THESE PLANS WERE BASED ON THE ABOVE STATED ARCHITECTURAL FOOTPRINT, AND ARE THEREFORE A PRELIMINARY LOCATION OF THE BUILDING. THE CONTRACTOR IS SOLELY RESPONSIBLE TO VERIFY WHAT PART OF THE BUILDING THE ARCHITECT'S FOOTPRINT REPRESENTS (E.G. SLAB, OUTSIDE WALL, MASONRY LEDGE, ETC ......) AND TO CONFIRM ITS FINAL POSITION ON THE SITE

BASED ON THE FINAL ARCHITECTURAL FOOTPRINT, CIVIL DIMENSION CONTROL PLAN, SURVEY BOUNDARY AND/OR PLAT. ANY DIFFERENCES FOUND SHALL BE REPORTED TO KH IMMEDIATELY. 32.ALL CONSTRUCTION SHALL COMPLY WITH THE PROJECT'S FINAL GEOTECHNICAL REPORT (OR LATEST EDITION), INCLUDING

33.CONTRACTOR IS RESPONSIBLE FOR ALL MATERIALS TESTING AND CERTIFICATION, UNLESS SPECIFIED OTHERWISE BY OWNER. ALL MATERIALS TESTING SHALL BE COORDINATED WITH THE APPROPRIATE CITY INSPECTOR AND COMPLY WITH CITY STANDARD SPECIFICATIONS AND GEOTECHNICAL REPORT. TESTING SHALL BE PERFORMED BY AN APPROVED INDEPENDENT AGENCY FOR TESTING MATERIALS. OWNER SHALL APPROVE THE AGENCY NOMINATED BY THE CONTRACTOR FOR MATERIALS TESTING 34.ALL COPIES OF MATERIALS TEST RESULTS SHALL BE SENT TO THE OWNER, ENGINEER AND ARCHITECT DIRECTLY FROM THE TESTING

35.IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO SHOW, BY THE STANDARD TESTING PROCEDURES OF THE MATERIALS, THAT THE WORK CONSTRUCTED MEETS THE PROJECT REQUIREMENTS AND CITY SPECIFICATIONS 36.DUE TO THE POTENTIAL FOR DIFFERENTIAL SOIL MOVEMENT ADJACENT TO THE BUILDING, THE CONTRACTOR SHALL ADHERE TO

BUILDING. THE OWNER AND CONTRACTOR ARE ADVISED TO OBTAIN A GEOTECHNICAL ENGINEER RECOMMENDATION SPECIFIC TO FLATWORK ADJACENT TO THE BUILDING, IF NONE IS CURRENTLY EXISTING. 37.ALL CONTRACTORS MUST CONFINE THEIR ACTIVITIES TO THE WORK AREA. NO ENCROACHMENTS OUTSIDE OF THE WORK AREA WILL BE ALLOWED. ANY DAMAGE RESULTING THEREFROM SHALL BE CONTRACTOR'S SOLE RESPONSIBILITY TO REPAIR. 38. THE CONTRACTOR SHALL PROTECT ALL EXISTING STRUCTURES, UTILITIES, MANHOLES, POLES, GUY WIRES, VALVE COVERS, VAULT LIDS, FIRE HYDRANTS, COMMUNICATION BOXES/PEDESTALS, AND OTHER FACILITIES TO REMAIN AND SHALL REPAIR ANY DAMAGES AT

NO COST TO THE OWNER. 39.THE CONTRACTOR SHALL IMMEDIATELY REPAIR OR REPLACE ANY PHYSICAL DAMAGE TO PRIVATE PROPERTY OR PUBLIC IMPROVEMENTS, INCLUDING BUT NOT LIMITED TO: FENCES, WALLS, SIGNS, PAVEMENT, CURBS, UTILITIES, SIDEWALKS, GRASS, TREES, LANDSCAPING, AND IRRIGATION SYSTEMS, ETC .... TO ORIGINAL CONDITION OR BETTER AT NO COST TO THE OWNER. 40.ALL AREAS IN EXISTING RIGHT-OF-WAY DISTURBED BY SITE CONSTRUCTION SHALL BE REPAIRED TO ORIGINAL CONDITION OR BETTER,

INCLUDING AS NECESSARY GRADING, LANDSCAPING, CULVERTS, AND PAVEMENT. 41.THE CONTRACTOR SHALL SALVAGE ALL EXISTING POWER POLES, SIGNS, WATER VALVES, FIRE HYDRANTS, METERS, ETC... THAT ARE TO BE RELOCATED DURING CONSTRUCTION 42.CONTRACTOR SHALL MAINTAIN ADEQUATE SITE DRAINAGE DURING ALL PHASES OF CONSTRUCTION, INCLUDING MAINTAINING EXISTING

DITCHES OR CULVERTS FREE OF OBSTRUCTIONS AT ALL TIMES. 43.THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND SUBMITTING A TRENCH SAFETY PLAN, PREPARED BY A PROFESSIONAL ENGINEER IN THE STATE OF TEXAS, TO THE CITY PRIOR TO CONSTRUCTION. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING TRENCH OPEN TRENCHES SHALL BE ALLOWED OVERNIGHT WITHOUT PRIOR WRITTEN APPROVAL OF THE CITY.

44.THE CONTRACTOR SHALL KEEP TRENCHES FREE FROM WATER. 45.SITE SAFETY IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. 46.THESE PLANS DO NOT EXTEND TO OR INCLUDE DESIGNS OR SYSTEMS PERTAINING TO THE SAFETY OF THE CONTRACTOR OR ITS EMPLOYEES, AGENTS OR REPRESENTATIVES IN THE PERFORMANCE OF THE WORK. THE ENGINEER'S SEAL HEREON DOES NOT EXTEND TO ANY SUCH SAFETY SYSTEM. THE CONTRACTOR SHALL BE RESPONSIBLE FOR IMPLEMENTATION OF ALL REQUIRED SAFETY

PROCEDURES AND PROGRAMS. 47.SIGNS RELATED TO SITE OPERATION OR SAFETY ARE NOT INCLUDED IN THESE PLANS. 48.CONTRACTOR OFFICE AND STAGING AREA SHALL BE AGREED ON BY THE OWNER AND CONTRACTOR PRIOR TO BEGINNING OF CONSTRUCTION. CONTRACTOR IS RESPONSIBLE FOR ALL PERMITTING REQUIREMENTS FOR THE CONSTRUCTION OFFICE, TRAILER, STORAGE, AND STAGING OPERATIONS AND LOCATIONS

49.LIGHT POLES, SIGNS, AND OTHER OBSTRUCTIONS SHALL NOT BE PLACED IN ACCESSIBLE ROUTES. 50.ALL SIGNS, PAVEMENT MARKINGS, AND OTHER TRAFFIC CONTROL DEVICES SHALL CONFORM TO THE "TEXAS MANUAL ON UNIFORM 51.TOP RIM ELEVATIONS OF ALL EXISTING AND DEVELOPED MANHOLES SHALL BE COORDINATED WITH TOP OF PAVEMENT OR FINISHED

GRADE AND SHALL BE ADJUSTED TO BE FLUSH WITH THE ACTUAL FINISHED GRADE AT THE TIME OF PAVING. 52.CONTRACTOR SHALL ADJUST ALL EXISTING AND DEVELOPED VALVES, FIRE HYDRANTS, AND OTHER UTILITY APPURTENANCES TO MATCH ACTUAL FINISHED GRADES AT THE TIME OF PAVING.

53.THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTION SEQUENCING AND PHASING, AND SHALL CONTACT THE APPROPRIATE CITY OFFICIALS, INCLUDING BUILDING OFFICIAL, ENGINEERING INSPECTOR, AND FIRE MARSHALL TO LEARN OF ANY REQUIREMENTS. 54. CONTRACTOR IS RESPONSIBLE FOR PREPARATION, SUBMITTAL, AND APPROVAL BY THE CITY OF A TRAFFIC CONTROL PLAN PRIOR TO THE START OF CONSTRUCTION, AND THEN THE IMPLEMENTATION OF THE PLAN.

55.CONTRACTOR SHALL KEEP A NEAT AND ACCURATE RECORD OF CONSTRUCTION, INCLUDING ANY DEVIATIONS OR VARIANCES FROM

56.THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING AS-BUILT PLANS TO THE ENGINEER AND CITY IDENTIFYING ALL DEVIATIONS AND VARIATIONS FROM THESE PLANS MADE DURING CONSTRUCTION.

THE CONTRACTOR SHALL COMPLY WITH ALL LOCAL, STATE, AND FEDERAL EROSION CONTROL AND WATER QUALITY REQUIREMENTS, LAWS, AND ORDINANCES THAT APPLY TO THE CONSTRUCTION SITE LAND DISTURBANCE.

2. CONTRACTOR SHALL COMPLY WITH THE REQUIREMENTS OF THE "TCEQ GENERAL PERMIT TO DISCHARGE UNDER THE TEXAS POLLUTANT DISCHARGE ELIMINATION SYSTEM TXR 150000" 3. EROSION CONTROL DEVICES SHOWN ON THE EROSION CONTROL PLAN FOR THE PROJECT SHALL BE INSTALLED PRIOR TO THE START

4. ALL EROSION CONTROL DEVICES ARE TO BE INSTALLED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS FOR THE 5. CONTRACTOR IS SOLELY RESPONSIBLE FOR INSTALLATION, IMPLEMENTATION, MAINTENANCE, AND EFFECTIVENESS OF ALL EROSION

CONTROL DEVICES, BEST MANAGEMENT PRACTICES (BMPS), AND FOR UPDATING THE EROSION CONTROL PLAN DURING CONSTRUCTION AS FIELD CONDITIONS CHANGE. 6. CONTRACTOR SHALL DOCUMENT THE DATES OF INSTALLATION, MAINTENANCE OR MODIFICATION, AND REMOVAL FOR EACH BMP EMPLOYED IN THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP) IF APPLICABLE.

7. AS STORM SEWER INLETS ARE INSTALLED ON-SITE, TEMPORARY EROSION CONTROL DEVICES SHALL BE INSTALLED AT EACH INLET PER APPROVED DETAILS. 8. THE EROSION CONTROL DEVICES SHALL REMAIN IN PLACE UNTIL THE AREA IT PROTECTS HAS BEEN PERMANENTLY STABILIZED.

9. CONTRACTOR SHALL PROVIDE ADEQUATE EROSION CONTROL DEVICES NEEDED DUE TO PROJECT PHASING. 10. CONTRACTOR SHALL OBSERVE THE EFFECTIVENESS OF THE EROSION CONTROL DEVICES AND MAKE FIELD ADJUSTMENTS AND MODIFICATIONS AS NEEDED TO PREVENT SEDIMENT FROM LEAVING THE SITE. IF THE EROSION CONTROL DEVICES DO NOT

EFFECTIVELY CONTROL EROSION AND PREVENT SEDIMENTATION FROM WASHING OFF THE SITE, THEN THE CONTRACTOR SHALL

11 OFF-SITE SOIL BORROW SPOIL AND STORAGE AREAS (IF APPLICABLE) ARE CONSIDERED AS PART OF THE PROJECT SITE AND MUST ALSO COMPLY WITH THE EROSION CONTROL REQUIREMENTS FOR THIS PROJECT. THIS INCLUDES THE INSTALLATION OF BMP'S TO CONTROL EROSION AND SEDIMENTATION AND THE ESTABLISHMENT OF PERMANENT GROUND COVER ON DISTURBED AREAS PRIOR TO FINAL APPROVAL OF THE PROJECT. CONTRACTOR IS RESPONSIBLE FOR MODIFYING THE SWPPP AND EROSION CONTROL PLAN TO INCLUDE BMPS FOR ANY OFF-SITE THAT ARE NOT ANTICIPATED OR SHOWN ON THE EROSION CONTROL PLAN.

12. ALL STAGING, STOCKPILES, SPOIL, AND STORAGE SHALL BE LOCATED SUCH THAT THEY WILL NOT ADVERSELY AFFECT STORM WATER QUALITY. PROTECTIVE MEASURES SHALL BE PROVIDED IF NEEDED TO ACCOMPLISH THIS REQUIREMENT, SUCH AS COVERING OR ENCIRCLING THE AREA WITH AN APPROPRIATE BARRIER.

13. CONTRACTORS SHALL INSPECT ALL EROSION CONTROL DEVICES, BMPS, DISTURBED AREAS, AND VEHICLE ENTRY AND EXIT AREAS WEEKLY AND WITHIN 24 HOURS OF ALL RAINFALL EVENTS OF 0.5 INCHES OR GREATER, AND KEEP A RECORD OF THIS INSPECTION IN THE SWPPP BOOKLET IF APPLICABLE TO VERIEY THAT THE DEVICES AND EROSION CONTROL PLAN ARE FUNCTIONING PROPERLY 14. CONTRACTOR SHALL CONSTRUCT A STABILIZED CONSTRUCTION ENTRANCE AT ALL PRIMARY POINTS OF ACCESS IN ACCORDANCE WITH CITY SPECIFICATIONS. CONTRACTOR SHALL ENSURE THAT ALL CONSTRUCTION TRAFFIC USES THE STABILIZED ENTRANCE AT

ALL TIMES FOR ALL INGRESS/EGRESS 15. SITE ENTRY AND EXITS SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT THE TRACKING AND FLOWING OF SEDIMENT AND DIRT ONTO OFF-SITE ROADWAYS. ALL SEDIMENT AND DIRT FROM THE SITE THAT IS DEPOSITED ONTO AN OFF-SITE ROADWAY SHALL BE REMOVED IMMEDIATELY

16. THE CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL SILT AND DEBRIS FROM THE AFFECTED OFF-SITE ROADWAYS THAT ARE A RESULT OF THE CONSTRUCTION, AS REQUESTED BY OWNER AND CITY. AT A MINIMUM, THIS SHOULD OCCUR ONCE PER DAY FOR THE OFF-SITE ROADWAYS. 17. WHEN WASHING OF VEHICLES IS REQUIRED TO REMOVE SEDIMENT PRIOR TO EXITING THE SITE, IT SHALL BE DONE IN AN AREA

STABILIZED WITH CRUSHED STONE THAT DRAINS INTO AN APPROVED SEDIMENT TRAP BMP. 18. CONTRACTOR SHALL INSTALL A TEMPORARY SEDIMENT BASIN FOR ANY ON-SITE DRAINAGE AREAS THAT ARE GREATER THAN 10 ACRES, PER TCEQ AND CITY STANDARDS. IF NO ENGINEERING DESIGN HAS BEEN PROVIDED FOR A SEDIMENTATION BASIN ON THESE PLANS, THEN THE CONTRACTOR SHALL ARRANGE FOR AN APPROPRIATE DESIGN TO BE PROVIDED.

19 ALL FINES IMPOSED FOR SEDIMENT OR DIRT DISCHARGED FROM THE SITE SHALL BE PAID BY THE RESPONSIBLE CONTRACTOR 20. WHEN SEDIMENT OR DIRT HAS CLOGGED THE CONSTRUCTION ENTRANCE VOID SPACES BETWEEN STONES OR DIRT IS BEING TRACKED ONTO A ROADWAY, THE AGGREGATE PAD MUST BE WASHED DOWN OR REPLACED. RUNOFF FROM THE WASH-DOWN OPERATION SHALL 5. RETAINING WALL ENGINEER SHALL CONSULT THESE PLANS AND THE GEOTECHNICAL REPORT FOR POTENTIAL CONFLICTS. NOT BE ALLOWED TO DRAIN DIRECTLY OFF SITE WITHOUT FIRST FLOWING THROUGH ANOTHER BMP TO CONTROL SEDIMENTATION. PERIODIC RE-GRADING OR NEW STONE MAY BE REQUIRED TO MAINTAIN THE EFFECTIVENESS OF THE CONSTRUCTION ENTRANCE. 21.TEMPORARY SEEDING OR OTHER APPROVED STABILIZATION SHALL BE INITIATED WITHIN 14 DAYS OF THE LAST DISTURBANCE OF ANY AREA, UNLESS ADDITIONAL CONSTRUCTION IN THE AREA IS EXPECTED WITHIN 21 DAYS OF THE LAST DISTURBANCE. 22. CONTRACTOR SHALL FOLLOW GOOD HOUSEKEEPING PRACTICES DURING CONSTRUCTION, ALWAYS CLEANING UP DIRT, LOOSE

MATERIAL, AND TRASH AS CONSTRUCTION PROGRESSES. 23.UPON COMPLETION OF FINE GRADING, ALL SURFACES OF DISTURBED AREAS SHALL BE PERMANENTLY STABILIZED. STABILIZATION IS ACHIEVED WHEN THE AREA IS EITHER COVERED BY PERMANENT IMPERVIOUS STRUCTURES, SUCH AS BUILDINGS, SIDEWALK, PAVEMENT. OR A UNIFORM PERENNIAL VEGETATIVE COVER. 24.AT THE CONCLUSION OF THE PROJECT, ALL INLETS, DRAIN PIPE, CHANNELS, DRAINAGEWAYS AND BORROW DITCHES AFFECTED BY THE

ACCORDANCE WITH APPLICABLE REGULATIONS. 25 ANY SEQUENCE OF CONSTRUCTION SHOWN HEREON IS A GENERAL OVERVIEW AND IS INTENDED TO CONVEY THE GENERAL CONCEPTS. OF THE EROSION CONTROL DESIGN AND SHOULD NOT BE RELIED UPON FOR CONSTRUCTION PURPOSES. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR DETAILED PHASING AND CONSTRUCTION SEQUENCING NECESSARY TO CONSTRUCT THE DEVELOPED IMPROVEMENTS INCLUDED IN THESE PLANS. THE CONTRACTOR SHALL NOTIFY ENGINEER IN WRITING IMMEDIATELY, PRIOR TO AND/OR DURING CONSTRUCTION IF ANY ADDITIONAL INFORMATION ON THE CONSTRUCTION SEQUENCE IS NECESSARY, CONTRACTOR IS SOLELY RESPONSIBLE FOR COMPLYING WITH THE REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION AND ALL OTHER

CONSTRUCTION SHALL BE DREDGED, AND THE SEDIMENT GENERATED BY THE PROJECT SHALL BE REMOVED AND DISPOSED IN

STORM WATER DISCHARGE AUTHORIZATION

APPLICABLE LAWS.

CONTRACTOR SHALL COMPLY WITH ALL TCEQ AND EPA STORM WATER POLLUTION PREVENTION REQUIREMENTS. 2. CONTRACTOR SHALL COMPLY WITH THE REQUIREMENTS OF THE TCEQ GENERAL PERMIT TO DISCHARGE UNDER THE TEXAS POLLUTANT DISCHARGE ELIMINATION SYSTEM TXR 150000.

. THE CONTRACTOR SHALL ENSURE THAT ALL PRIMARY OPERATORS SUBMIT A NOI TO TCEQ AT LEAST SEVEN DAYS PRIOR TO COMMENCING CONSTRUCTION (IF APPLICABLE), OR IF UTILIZING ELECTRONIC SUBMITTAL, PRIOR TO COMMENCING CONSTRUCTION. ALL PRIMARY OPERATORS SHALL PROVIDE A COPY OF THE SIGNED NOI TO THE OPERATOR OF ANY MS4 (TYPICALLY THE CITY) RECEIVING DISCHARGE FROM THE SITE

4. CONTRACTOR SHALL BE RESPONSIBLE FOR THE IMPLEMENTATION OF THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP) IF APPLICABLE, INCLUDING POSTING SITE NOTICE, INSPECTIONS, DOCUMENTATION, AND SUBMISSION OF ANY INFORMATION REQUIRED

ALL CONTRACTORS AND SUBCONTRACTORS PROVIDING SERVICES RELATED TO THE SWPPP SHALL SIGN THE REQUIRED CONTRACTOR CERTIFICATION STATEMENT ACKNOWLEDGING THEIR RESPONSIBILITIES AS SPECIFIED IN THE SWPPP 6. A COPY OF THE SWPPP, INCLUDING NOI, SITE NOTICE, CONTRACTOR CERTIFICATIONS, AND ANY REVISIONS, SHALL BE SUBMITTED TO THE CITY BY THE CONTRACTOR AND SHALL BE RETAINED ON-SITE DURING CONSTRUCTION.

. A NOTICE OF TERMINATION (NOT) SHALL BE SUBMITTED TO TCEQ BY ANY PRIMARY OPERATOR WITHIN 30 DAYS AFTER ALL SOIL DISTURBING ACTIVITIES AT THE SITE HAVE BEEN COMPLETED AND A UNIFORM VEGETATIVE COVER HAS BEEN ESTABLISHED ON ALL UNPAVED AREAS AND AREAS NOT COVERED BY STRUCTURES, A TRANSFER OF OPERATIONAL CONTROL HAS OCCURRED, OR THE OPERATOR HAS OBTAINED ALTERNATIVE AUTHORIZATION UNDER A DIFFERENT PERMIT. A COPY OF THE NOT SHALL BE PROVIDED TO THE OPERATOR OF ANY MS4 RECEIVING DISCHARGE FROM THE SITE.

SUBSEQUENT ADDENDA

. KH IS NOT RESPONSIBLE FOR THE MEANS AND METHODS EMPLOYED BY THE CONTRACTOR TO IMPLEMENT THIS DEMOLITION PLAN. THIS PRELIMINARY DEMOLITION PLAN SIMPLY INDICATES THE KNOWN OBJECTS ON THE SUBJECT TRACT THAT ARE TO BE DEMOLISHED AND REMOVED FROM THE SITE.

2. KH DOES NOT WARRANT OR REPRESENT THAT THE PLAN, WHICH WAS PREPARED BASED ON SURVEY AND UTILITY INFORMATION PROVIDED BY OTHERS, SHOWS ALL IMPROVEMENTS AND UTILITIES, THAT THE IMPROVEMENTS AND UTILITIES ARE SHOWN ACCURATELY, OR THAT THE UTILITIES SHOWN CAN BE REMOVED. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING ITS OWN SITE RECONNAISSANCE TO SCOPE ITS WORK AND TO CONFIRM WITH THE OWNERS OF IMPROVEMENTS AND UTILITIES THE ABILITY AND PROCESS FOR THE REMOVAL OF THEIR FACILITIES.

3. THIS PLAN IS INTENDED TO GIVE A GENERAL GUIDE TO THE CONTRACTOR, NOTHING MORE, THE GOAL OF THE DEMOLITION IS TO LEAVE THE SITE IN A STATE SUITABLE FOR THE CONSTRUCTION OF THE DEVELOPED DEVELOPMENT. REMOVAL OR PRESERVATION OF IMPROVEMENTS, UTILITIES, ETC. TO ACCOMPLISH THIS GOAL ARE THE RESPONSIBILITY OF THE CONTRACTOR. 4. CONTRACTOR IS STRONGLY CAUTIONED TO REVIEW THE FOLLOWING REPORTS DESCRIBING SITE CONDITIONS PRIOR TO BIDDING AND IMPLEMENTING THE DEMOLITION PLAN:

a. ENVIRONMENTAL SITE ASSESSMENT PROVIDED BY THE OWNER, b. ASBESTOS BUILDING INSPECTION REPORT(S) PROVIDED BY THE OWNER,

c. GEOTECHNICAL REPORT PROVIDED BY THE OWNER. OTHER REPORTS THAT ARE APPLICABLE AND AVAILABLE

5. CONTRACTOR SHALL CONTACT THE OWNER TO VERIFY WHETHER ADDITIONAL REPORTS OR AMENDMENTS TO THE ABOVE CITED REPORTS HAVE BEEN PREPARED AND TO OBTAIN/REVIEW/AND COMPLY WITH THE RECOMMENDATION OF SUCH STUDIES PRIOR TO STARTING ANY WORK ON THE SITE

6. CONTRACTOR SHALL COMPLY WITH ALL LOCAL, STATE, AND FEDERAL REGULATIONS REGARDING THE DEMOLITION OF OBJECTS ON THE SITE AND THE DISPOSAL OF THE DEMOLISHED MATERIALS OFF-SITE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO REVIEW THE SITE, DETERMINE THE APPLICABLE REGULATIONS, RECEIVE THE REQUIRED PERMITS AND AUTHORIZATIONS, AND COMPLY. 7. KH DOES NOT REPRESENT THAT THE REPORTS AND SURVEYS REFERENCED ABOVE ARE ACCURATE, COMPLETE, OR COMPREHENSIVE SHOWING ALL ITEMS THAT WILL NEED TO BE DEMOLISHED AND REMOVED.

8. SURFACE PAVEMENT INDICATED MAY OVERLAY OTHER HIDDEN STRUCTURES, SUCH AS ADDITIONAL LAYERS OF PAVEMENT, FOUNDATIONS OR WALLS, THAT ARE ALSO TO BE REMOVED.

GEOTECHNICAL REPORT'S RECOMMENDATION FOR SUBGRADE PREPARATION SPECIFIC TO FLATWORK ADJACENT TO THE DEVELOPED 1. THE CONTRACTOR AND GRADING SUBCONTRACTOR SHALL VERIFY THE SUITABILITY OF EXISTING AND DEVELOPED SITE CONDITIONS INCLUDING GRADES AND DIMENSIONS BEFORE START OF CONSTRUCTION. THE CIVIL ENGINEER SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCIES. CONTRACTOR SHALL OBTAIN ANY REQUIRED GRADING PERMITS FROM THE CITY

3. UNLESS OTHERWISE NOTED, DEVELOPED CONTOURS AND SPOT ELEVATIONS SHOWN IN PAVED AREA REFLECT TOP OF PAVEMENT SURFACE. IN LOCATIONS ALONG A CURB LINE, ADD 6-INCHES (OR THE HEIGHT OF THE CURB) TO THE PAVING GRADE FOR TOP OF CURB 10. WHERE COVER EXCEEDS 20-FEET OR IS LESS THAN 2-FEET, CLASS IV RCP SHALL BE USED.

4. DEVELOPED SPOT ELEVATIONS AND CONTOURS OUTSIDE THE PAVEMENT ARE TO TOP OF FINISHED GRADE. 5. DEVELOPED CONTOURS ARE APPROXIMATE. DEVELOPED SPOT ELEVATIONS AND DESIGNATED GRADIENT ARE TO BE USED IN CASE OF

6 ALL FINISHED GRADES SHALL TRANSITION UNIFORMLY BETWEEN THE FINISHED ELEVATIONS SHOWN 7. CONTOURS AND SPOT GRADES SHOWN ARE ELEVATIONS OF TOP OF THE FINISHED SURFACE. WHEN PERFORMING THE GRADING OPERATIONS. THE CONTRACTOR SHALL PROVIDE AN APPROPRIATE ELEVATION HOLD-DOWN ALLOWANCE FOR THE THICKNESS OF PAVEMENT, SIDEWALK, TOPSOIL, MULCH, STONE, LANDSCAPING, RIP-RAP AND ALL OTHER SURFACE MATERIALS THAT WILL CONTRIBUTE TO THE TOP OF FINISHED GRADE. FOR EXAMPLE, THE LIMITS OF EARTHWORK IN PAVED AREAS IS THE BOTTOM OF THE

8. NO REPRESENTATIONS OF EARTHWORK QUANTITIES OR SITE BALANCE ARE MADE BY THESE PLANS. THE CONTRACTOR SHALL PROVIDE THEIR OWN EARTHWORK CALCULATION TO DETERMINE THEIR CONTRACT QUANTITIES AND COST. ANY SIGNIFICANT VARIANCE FROM A BALANCED SITE SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE CIVIL ENGINEER. SAFETY REQUIREMENTS IN ACCORDANCE WITH CITY, STATE, AND FEDERAL REQUIREMENTS, INCLUDING OSHA FOR ALL TRENCHES. NO 9. ALL GRADING AND EARTHWORK SHALL COMPLY WITH THE PROJECT'S FINAL GEOTECHNICAL REPORT (OR LATEST EDITION), INCLUDING

> 10. ALL EXCAVATION IS UNCLASSIFIED AND SHALL INCLUDE ALL MATERIALS ENCOUNTERED. UNUSABLE EXCAVATED MATERIAL AND ALL WASTE RESULTING FROM SITE CLEARING AND GRUBBING SHALL BE REMOVED FROM THE SITE AND APPROPRIATELY DISPOSED BY THE 11.EROSION CONTROL DEVICES SHOWN ON THE EROSION CONTROL PLAN FOR THE PROJECT SHALL BE INSTALLED PRIOR TO THE START OF GRADING. REFERENCE EROSION CONTROL PLAN, DETAILS, GENERAL NOTES, AND SWPPP FOR ADDITIONAL INFORMATION AND

12.BEFORE ANY EARTHWORK IS PERFORMED, THE CONTRACTOR SHALL STAKE OUT AND MARK THE LIMITS OF THE PROJECT'S PROPERTY LINE AND SITE IMPROVEMENTS. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY ENGINEERING AND SURVEYING FOR LINE AND GRADE CONTROL POINTS RELATED TO EARTHWORK

13. CONTRACTOR TO DISPOSE OF ALL EXCESS EXCAVATION MATERIALS IN A MANNER THAT ADHERES TO LOCAL, STATE AND FEDERAL LAWS AND REGULATIONS. THE CONTRACTOR SHALL KEEP A RECORD OF WHERE EXCESS EXCAVATION WAS DISPOSED, ALONG WITH THE RECEIVING LANDOWNER'S APPROVAL TO DO SO.

14. CONTRACTOR IS RESPONSIBLE FOR REMOVAL AND REPLACEMENT OF TOPSOIL AT THE COMPLETION OF FINE GRADING. CONTRACTOR SHALL REFER TO LANDSCAPE ARCHITECTURE PLANS FOR SPECIFICATIONS AND REQUIREMENTS FOR TOPSOIL. 15. CONTRACTOR SHALL MAINTAIN ADEQUATE SITE DRAINAGE DURING ALL PHASES OF CONSTRUCTION, INCLUDING MAINTAINING EXISTING DITCHES OR CULVERTS FREE OF OBSTRUCTIONS AT ALL TIMES. 16.NO EARTHWORK FILL SHALL BE PLACED IN ANY EXISTING DRAINAGE WAY, SWALE, CHANNEL, DITCH, CREEK, OR FLOODPLAIN FOR ANY REASON OR ANY LENGTH OF TIME, UNLESS THESE PLANS SPECIFICALLY INDICATE THIS IS REQUIRED.

17. TEMPORARY CULVERTS MAY BE REQUIRED IN SOME LOCATIONS TO CONVEY RUN-OFF. 18. REFER TO DIMENSION CONTROL PLAN, AND PLAT FOR HORIZONTAL DIMENSIONS. 19. THE CONTRACTOR SHALL CLEAR AND GRUB THE SITE AND PLACE, COMPACT, AND CONDITION FILL PER THE PROJECT GEOTECHNICAL ENGINEER'S SPECIFICATIONS. THE FILL MATERIAL TO BE USED SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO

20.CONTRACTOR IS RESPONSIBLE FOR ALL SOILS TESTING AND CERTIFICATION, UNLESS SPECIFIED OTHERWISE BY OWNER. ALL SOILS TESTING SHALL BE COORDINATED WITH THE APPROPRIATE CITY INSPECTOR AND SHALL COMPLY WITH CITY STANDARD SPECIFICATIONS AND THE GEOTECHNICAL REPORT. SOILS TESTING SHALL BE PERFORMED BY AN APPROVED INDEPENDENT AGENCY FOR TESTING SOILS. THE OWNER SHALL APPROVE THE AGENCY NOMINATED BY THE CONTRACTOR FOR SOILS TESTING. 21.ALL COPIES OF SOILS TEST RESULTS SHALL BE SENT TO THE OWNER, ENGINEER AND ARCHITECT DIRECTLY FROM THE TESTING

22.IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO SHOW, BY THE STANDARD TESTING PROCEDURES OF THE SOILS, THAT THE WORK CONSTRUCTED MEETS THE PROJECT REQUIREMENTS AND CITY SPECIFICATIONS. 23.THE SCOPE OF WORK FOR CIVIL IMPROVEMENT SHOWN ON THESE PLANS TERMINATES 5-FEET FROM THE BUILDING. CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT AND STRUCTURAL PLANS AND SPECIFICATIONS FILL, CONDITIONING, AND PREPARATION

GEOTECHNICAL REPORT'S RECOMMENDATION FOR SUBGRADE PREPARATION SPECIFIC TO FLATWORK ADJACENT TO THE DEVELOPED BUILDING. THE OWNER AND CONTRACTOR ARE ADVISED TO OBTAIN A GEOTECHNICAL ENGINEER RECOMMENDATION SPECIFIC TO FLATWORK ADJACENT TO THE BUILDING. IF NONE IS CURRENTLY EXISTING 25.CONTRACTOR SHALL ENSURE THAT SUFFICIENT POSITIVE SLOPE AWAY FROM THE BUILDING PAD IS ACHIEVED FOR ENTIRE PERIMETER OF THE DEVELOPED BUILDING(S) DURING GRADING OPERATIONS AND IN THE FINAL CONDITION. IF THE CONTRACTOR OBSERVES THAT 13. CONTRACTOR SHALL COMPLY WITH CITY REQUIREMENTS FOR WATER AND WASTEWATER SERVICE DISRUPTIONS AND THE AMOUNT OF THIS WILL NOT BE ACHIEVED, THE CONTRACTOR SHALL CONTACT THE ENGINEER TO REVIEW THE LOCATION.

24.DUE TO THE POTENTIAL FOR DIFFERENTIAL SOIL MOVEMENT ADJACENT TO THE BUILDING, THE CONTRACTOR SHALL ADHERE TO

26.THE CONTRACTOR SHALL TAKE ALL AVAILABLE PRECAUTIONS TO CONTROL DUST. CONTRACTOR SHALL CONTROL DUST BY SPRINKLING WATER, OR BY OTHER MEANS APPROVED BY THE CITY, AT NO ADDITIONAL COST TO THE OWNER. 27. CONTRACTOR SHALL COORDINATE WITH THE UTILITY COMPANIES FOR ANY REQUIRED UTILITY ADJUSTMENTS AND/OR RELOCATIONS NEEDED FOR GRADING OPERATIONS AND TO ACCOMMODATE DEVELOPED GRADE. INCLUDING THE UNKNOWN UTILITIES NOT SHOWN ON THESE PLANS. CONTRACTOR SHALL REFER TO THE GENERAL NOTES "OVERALL" SECTION THESE PLANS FOR ADDITIONAL

AT THE TOP AND BOTTOM OF THE WALL

28.EXISTING TREE LOCATIONS SHOWN ON THESE PLANS ARE APPROXIMATE. CONTRACTOR SHALL REPORT ANY DISCREPANCIES FOUND IN THE FIELD THAT AFFECT THE GRADING PLAN TO THE CIVIL ENGINEER 29. CONTRACTOR SHALL FIELD VERIFY ALL PROTECTED TREE LOCATIONS, INDIVIDUAL PROTECTED TREE CRITICAL ROOT ZONES, AND DEVELOPED SITE GRADING, AND NOTIFY THE CIVIL ENGINEER AND LANDSCAPE ARCHITECT OF ANY CONFLICTS WITH THE TREE PRESERVATION PLAN BY THE LANDSCAPE ARCHITECT PRIOR TO COMMENCING THE WORK.

APPROVED TREE PRESERVATION PLANS BY THE LANDSCAPE ARCHITECT. 31.CONTRACTOR SHALL REFER TO THE LANDSCAPING AND TREE PRESERVATIONS PLANS FOR ALL INFORMATION AND DETAILS REGARDING EXISTING TREES TO BE REMOVED AND PRESERVED.

30.TREE PROTECTION MEASURES SHALL BE INSTALLED IN ACCORDANCE WITH THE CITY STANDARD TREE PROTECTION DETAILS AND THE

32.NO TREE SHALL BE REMOVED UNLESS A TREE REMOVAL PERMIT HAS BEEN ISSUED BY THE CITY, OR CITY HAS OTHERWISE CONFIRMED IN WRITING THAT ONE IS NOT NEEDED FOR THE TREE(S). 33 NO TREE SHALL BE REMOVED OR DAMAGED WITHOUT PRIOR AUTHORIZATION OF THE OWNER OR OWNER'S REPRESENTATIVE. EXISTING TREES SHALL BE PRESERVED WHENEVER POSSIBLE AND GRADING IMPACT TO THEM HELD TO A MINIMUM 34.AFTER PLACEMENT OF SUBGRADE AND PRIOR TO PLACEMENT OF PAVEMENT, CONTRACTOR SHALL TEST AND OBSERVE PAVEMENT

AREAS FOR EVIDENCE OF PONDING AND INADEQUATE SLOPE FOR DRAINAGE. ALL AREAS SHALL ADEQUATELY DRAIN TOWARDS THE INTENDED STRUCTURE TO CONVEY STORMWATER RUNOFF. CONTRACTOR SHALL IMMEDIATELY NOTIFY OWNER AND ENGINEER IF ANY AREAS OF POOR DRAINAGE ARE DISCOVERED 35.CONTRACTOR FIELD ADJUSTMENT OF DEVELOPED SPOT GRADES IS ALLOWED, IF THE APPROVAL OF THE CIVIL ENGINEER IS OBTAINED.

RETAINING WALLS SHOWN ARE FOR SITE GRADING PURPOSES ONLY, AND INCLUDE ONLY LOCATION AND SURFACE SPOT ELEVATIONS

2. RETAINING WALL TYPE OR SYSTEM SHALL BE SELECTED BY THE OWNER. 3. RETAINING WALL DESIGN SHALL BE PROVIDED BY OTHERS AND SHALL FIT IN THE WALL ZONE OR LOCATION SHOWN ON THESE PLANS. STRUCTURAL DESIGN AND PERMITTING OF RETAINING WALLS, RAILINGS, AND OTHER WALL SAFETY DEVICES SHALL BE PERFORMED BY A LICENSED ENGINEER AND ARE NOT PART OF THIS PLAN SET.

4. RETAINING WALL DESIGN SHALL MEET THE INTENT OF THE GRADING PLAN AND SHALL ACCOUNT FOR ANY INFLUENCE ON ADJACENT BUILDING FOUNDATIONS, UTILITIES, PROPERTY LINES AND OTHER CONSTRUCTABILITY NOTES.

1. ALL PAVING MATERIALS AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THESE PLANS. THE CITY STANDARD DETAILS AND SPECIFICATIONS, THE FINAL GEOTECHNICAL REPORT AND ALL ISSUED ADDENDA, AND COMMONLY ACCEPTED CONSTRUCTION STANDARDS. THE CITY SPECIFICATIONS SHALL GOVERN WHERE OTHER SPECIFICATIONS DO NOT EXIST. IN CASE OF CONFLICTING SPECIFICATIONS OR DETAILS, THE MORE RESTRICTIVE SPECIFICATION/DETAIL SHALL BE FOLLOWED. 2. ALL PRIVATE ON-SITE PAVING AND PAVING SUBGRADE SHALL COMPLY WITH THE PROJECT'S FINAL GEOTECHNICAL REPORT (OR LATEST EDITION) INCLUDING ALL ADDENDA

3. ALL FIRELANE PAVING AND PAVING SUBGRADE SHALL COMPLY WITH CITY STANDARDS AND DETAILS. IF THESE ARE DIFFERENT THAN THOSE IN THE GEOTECHNICAL REPORT, THEN THE MORE RESTRICTIVE SHALL BE FOLLOWED. 4. ALL PUBLIC PAVING AND PAVING SUBGRADE SHALL COMPLY WITH CITY STANDARD CONSTRUCTION DETAILS AND SPECIFICATIONS. 5. CONTRACTOR IS RESPONSIBLE FOR ALL PAVING AND PAVING SUBGRADE TESTING AND CERTIFICATION, UNLESS SPECIFIED OTHERWISE BY OWNER. ALL PAVING AND PAVING SUBGRADE TESTING SHALL BE COORDINATED WITH THE APPROPRIATE CITY INSPECTOR. TESTING SHALL BE PERFORMED BY AN APPROVED INDEPENDENT AGENCY FOR TESTING PAVING AND SUBGRADE. OWNER SHALL APPROVE THE AGENCY NOMINATED BY THE CONTRACTOR FOR PAVING AND PAVING SUBGRADE TESTING 6. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO SHOW, BY THE STANDARD TESTING PROCEDURES OF THE PAVING AND PAVING

SUBGRADE, THAT THE WORK CONSTRUCTED MEETS THE PROJECT REQUIREMENTS AND CITY SPECIFICATIONS. 7 DUE TO THE POTENTIAL FOR DIFFERENTIAL SOIL MOVEMENT ADJACENT TO THE BUILDING. THE CONTRACTOR SHALL ADHERE TO GEOTECHNICAL REPORT'S RECOMMENDATION FOR SUBGRADE PREPARATION SPECIFIC TO FLATWORK ADJACENT TO THE DEVELOPED BUILDING. THE OWNER AND CONTRACTOR ARE ADVISED TO OBTAIN A GEOTECHNICAL ENGINEER RECOMMENDATION SPECIFIC TO FLATWORK ADJACENT TO THE BUILDING. IF NONE IS CURRENTLY EXISTING. 8. CURB RAMPS ALONG PUBLIC STREETS AND IN THE PUBLIC RIGHT-OF-WAY SHALL BE CONSTRUCTED BASED ON THE CITY STANDARD CONSTRUCTION DETAIL AND SPECIFICATIONS

SHALL HAVE A DETECTABLE WARNING SURFACE THAT IS FULL WIDTH AND FULL DEPTH OF THE CURB RAMP, NOT INCLUDING FLARES. 10. ALL ACCESSIBLE RAMPS, CURB RAMPS, STRIPING, AND PAVEMENT MARKINGS SHALL CONFORM TO ADA AND TAS STANDARDS, LATEST 11. ANY COMPONENTS OF THE PROJECT SUBJECT TO RESIDENTIAL USE SHALL ALSO CONFORM TO THE FAIR HOUSING ACT, AND COMPLY WITH THE FAIR HOUSING ACT DESIGN MANUAL BY THE US DEPARTMENT OF HOUSING AND URBAN DEVELOPMENT. 12. CONTRACTOR SHALL CONSTRUCT DEVELOPED PAVEMENT TO MATCH EXISTING PAVEMENT WITH A SMOOTH, FLUSH, CONNECTION.

13. CONTRACTOR SHALL FURNISH AND INSTALL ALL PAVEMENT MARKINGS FOR FIRE LANES, PARKING STALLS, HANDICAPPED PARKING SYMBOLS, AND MISCELLANEOUS STRIPING WITHIN PARKING LOT AND AROUND BUILDING AS SHOWN ON THE PLANS. ALL PAINT AND PAVEMENT MARKINGS SHALL ADHERE TO CITY AND OWNER STANDARDS. 14 REFER TO GEOTECHNICAL REPORT FOR PAVING JOINT LAYOUT PLAN REQUIREMENTS FOR PRIVATE PAVEMENT. 15. REFER TO CITY STANDARD DETAILS AND SPECIFICATIONS FOR JOINT LAYOUT PLAN REQUIREMENTS FOR PUBLIC PAVEMENT.

16. ALL REINFORCING STEEL SHALL CONFORM TO THE GEOTECHNICAL REPORT, CITY STANDARDS, AND ASTM A-615, GRADE 60, AND SHALL EA BE SUPPORTED BY BAR CHAIRS. CONTRACTOR SHALL USE THE MORE STRINGENT OF THE CITY AND GEOTECHNICAL STANDARDS. 17 ALL JOINTS SHALL EXTEND THROUGH THE CURB 18. THE MINIMUM LENGTH OF OFFSET JOINTS AT RADIUS POINTS SHALL BE 2 FEET.

20.ALL SAWCUTS SHALL BE FULL DEPTH FOR PAVEMENT REMOVAL AND CONNECTION TO EXISTING PAVEMENT. 21.FIRE LANES SHALL BE MARKED AND LABELED AS A FIRELANE PER CITY STANDARDS. 22.UNLESS THE PLANS SPECIFICALLY DICTATE TO THE CONTRARY, ON-SITE AND OTHER DIRECTIONAL SIGNS SHALL BE ORIENTED SO THEY ARE READILY VISIBLE TO THE ONCOMING TRAFFIC FOR WHICH THEY ARE INTENDED.

19. CONTRACTOR SHALL SUBMIT A JOINTING PLAN TO THE ENGINEER AND OWNER PRIOR TO BEGINNING ANY OF THE PAVING WORK.

23.CONTRACTOR IS RESPONSIBLE FOR INSTALLING NECESSARY CONDUIT FOR LIGHTING, IRRIGATION, ETC. PRIOR TO PLACEMENT OF PAVEMENT. ALL CONSTRUCTION DOCUMENTS (CIVIL, MEP, LANDSCAPE, IRRIGATION, AND ARCHITECT) SHALL BE CONSULTED. 24.BEFORE PLACING PAVEMENT, CONTRACTOR SHALL VERIFY THAT SUITABLE ACCESSIBLE PEDESTRIAN ROUTES (PER ADA, TAS, AND FHA) EXIST TO AND FROM EVERY DOOR AND ALONG SIDEWALKS. ACCESSIBLE PARKING SPACES. ACCESS AISLES. AND ACCESSIBLE ROUTES. IN NO CASE SHALL AN ACCESSIBLE RAMP SLOPE EXCEED 1 VERTICAL TO 12 HORIZONTAL. IN NO CASE SHALL SIDEWALK CROSS SLOPE EXCEED 2.0 PERCENT. IN NO CASE SHALL LONGITUDINAL SIDEWALK SLOPE EXCEED 5.0 PERCENT. ACCESSIBLE PARKING SPACES AND ACCESS AISLES SHALL NOT EXCEED 2.0 PERCENT SLOPE IN ANY DIRECTION 25.CONTRACTOR SHALL TAKE FIELD SLOPE MEASUREMENTS ON FINISHED SUBGRADE AND FORM BOARDS PRIOR TO PLACING PAVEMENT TO VERIFY THAT ADA/TAS SLOPE REQUIREMENTS ARE PROVIDED. CONTRACTOR SHALL CONTACT ENGINEER PRIOR TO PAVING IF ANY

EXCESSIVE SLOPES ARE ENCOUNTERED. NO CONTRACTOR CHANGE ORDERS WILL BE ACCEPTED FOR ADA AND TAS SLOPE COMPLIANCE ISSUES. ALL STORM SEWER MATERIALS AND CONSTRUCTION SHALL COMPLY WITH CITY STANDARD CONSTRUCTION DETAILS AND

2. THE SITE UTILITY CONTRACTOR SHALL PROVIDE ALL MATERIALS AND APPURTENANCES NECESSARY FOR COMPLETE INSTALLATION OF 3. THE CONTRACTOR SHALL FIELD VERIFY THE SIZE, CONDITION, HORIZONTAL, AND VERTICAL LOCATIONS OF ALL EXISTING STORM SEWER FACILITIES THAT ARE TO BE CONNECTED TO, PRIOR TO START OF CONSTRUCTION OF ANY STORM SEWER, AND SHALL NOTIFY THE ENGINEER OF ANY CONFLICTS DISCOVERED.

4. THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS SHOWN, INCLUDING THE HORIZONTAL AND VERTICAL LOCATION OF CURB INLETS AND GRATE INLETS AND ALL UTILITIES CROSSING THE STORM SEWER. 5. FLOW LINE, TOP-OF-CURB, RIM, THROAT, AND GRATE ELEVATIONS OF DEVELOPED INLETS SHALL BE VERIFIED WITH THE GRADING PLAN AND FIELD CONDITIONS PRIOR TO THEIR INSTALLATION. 6. ALL PUBLIC STORM SEWER CONSTRUCTION, PIPE, STRUCTURES, AND FITTINGS SHALL ADHERE TO CITY PUBLIC WORKS STANDARD DETAILS AND SPECIFICATIONS. CONTRACTOR SHALL ARRANGE FOR REQUIRED CITY INSPECTIONS 7. ALL PRIVATE STORM SEWER CONSTRUCTION, PIPE, STRUCTURES, AND FITTINGS SHALL ADHERE TO THE APPLICABLE PLUMBING CODE. OC

CONTRACTOR SHALL ARRANGE FOR REQUIRED CITY INSPECTIONS 8. ALL PVC TO RCP CONNECTIONS AND ALL STORM PIPE CONNECTIONS ENTERING STRUCTURES OR OTHER STORM PIPES SHALL HAVE A OSHA CONCRETE COLLAR AND BE GROUTED TO ASSURE THE CONNECTION IS WATERTIGHT. 9. ALL PUBLIC STORM SEWER LINES SHALL BE MINIMUM CLASS III RCP. PRIVATE STORM SEWER LINES 18-INCHES AND GREATER SHALL BE CLASS III RCP OR OTHER APPROVED MATERIAL

11.IF CONTRACTOR PROPOSES TO USE HDPE OR PVC IN LIEU OF RCP FOR PRIVATE STORM SEWER, CONTRACTOR SHALL SUBMIT TECHNICAL DATA TO THE OWNER, ENGINEER AND CITY ENGINEER/INSPECTOR FOR APPROVAL PRIOR TO ORDERING THE MATERIAL. ANY DEVELOPED HDPE AND PVC SHALL BE WATERTIGHT 12. THE CONTRACTOR SHALL PROVIDE CONSTRUCTION SURVEYING FOR ALL STORM SEWER LINES.

13. EMBEDMENT FOR ALL STORM SEWER LINES, PUBLIC OR PRIVATE, SHALL BE PER CITY STANDARD DETAILS. 14. ALL WYE CONNECTIONS AND PIPE BENDS ARE TO BE PREFABRICATED AND INSTALLED PER MANUFACTURERS SPECIFICATIONS. 15.USE 4 FOOT JOINTS WITH BEVELED ENDS IF RADIUS OF STORM SEWER IS LESS THAN 100 FEET.

16. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND SUBMITTING A TRENCH SAFETY PLAN, PREPARED BY A PROFESSIONAL ENGINEER IN THE STATE OF TEXAS, TO THE CITY PRIOR TO CONSTRUCTION. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING TRENCH RCP SAFETY REQUIREMENTS IN ACCORDANCE WITH CITY, STATE, AND FEDERAL REQUIREMENTS, INCLUDING OSHA FOR ALL TRENCHES. NO ROW OPEN TRENCHES SHALL BE ALLOWED OVERNIGHT WITHOUT PRIOR WRITTEN APPROVAL OF THE CITY. 17. THE CONTRACTOR SHALL KEEP TRENCHES FREE FROM WATER.

ANY PONDS THAT ARE INTENDED TO HOLD WATER INDEFINITELY SHALL BE CONSTRUCTED WATERTIGHT

CONSTRUCTION, AND SHALL NOTIFY THE ENGINEER OF ANY CONFLICTS DISCOVERED.

2. FOR ANY PONDS INTENDED TO HOLD WATER INDEFINITELY: THE CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT FOR POND LINER SPECIFICATIONS 3. A GEOTECHNICAL ENGINEER SHALL REVIEW AND APPROVE ALL POND LINER MATERIAL, PLACEMENT PROCEDURES, AND PROVIDE TESTING TO ENSURE THE POND LINER MATERIAL PLACED IS WATERTIGHT. 4. STORM SEWER PIPES AND HEADWALLS THAT CONNECT TO A POND INTENDED TO HOLD WATER INDEFINITELY SHALL BE INSTALLED

WITH WATERTIGHT JOINTS TO AT LEAST 1-FOOT ABOVE THE NORMAL POOL WATER SURFACE ELEVATION. 5. ANY GRAVEL OR OTHER PERVIOUS EMBEDMENT AROUND PIPES OR OUTFALL STRUCTURES NEAR THE POND SHALL BE ELIMINATED FOR AT LEAST 20-FEET FROM THE POND SO NO ROUTE FOR WATER TO LEAK THROUGH THE EMBEDMENT MATERIAL IS PROVIDED. BACKFILL IN THESE AREAS SHALL BE OF IMPERVIOUS MATERIAL 6. FOR ANY PONDS INTENDED TO HOLD WATER INDEFINITELY: THE WATER LEVEL FOLLOWING COMPLETION AND FILLING OF THE POND

SHALL BE MONITORED BY THE CONTRACTOR FOR AT LEAST 60 DAYS TO OBSERVE WATER INFLOW, OUTFLOW, AND CALCULATE EVAPORATION TO VERIFY THAT THE POND IS WATERTIGHT. 7. FOR ANY PONDS INTENDED TO HOLD WATER INDEFINITELY: THE POND WATER LEVEL SHALL ALSO BE MAINTAINED BY THE

CONTRACTOR FOR THE DURATION OF CONSTRUCTION SO THAT IT REMAINS FULL TO ITS DESIGN WATER LEVEL, AND IS NOT LOWERED, AS THIS MAY DRY-OUT THE POND LINER AND RISK ITS WATERTIGHT PROPERTIES. WATER AND WASTEWATER

ALL WATER AND WASTEWATER MATERIALS AND CONSTRUCTION SHALL COMPLY WITH CITY STANDARD CONSTRUCTION DETAILS AND 2. CONTRACTOR SHALL FIELD VERIFY THE SIZE, CONDITION, HORIZONTAL, AND VERTICAL LOCATIONS OF ALL EXISTING WATER AND WASTEWATER FACILITIES THAT ARE TO BE CONNECTED TO, PRIOR TO START OF CONSTRUCTION OF ANY WATER OR WASTEWATER

ALL UTILITY SERVICES ENTERING THE BUILDING. 4. THE CONTRACTOR SHALL FIELD VERIFY THE ELEVATION OF ALL UTILITY CROSSINGS PRIOR TO THE INSTALLATION OF ANY PIPE. 5. THE SITE UTILITY CONTRACTOR SHALL PROVIDE ALL MATERIALS AND APPURTENANCES NECESSARY FOR COMPLETE INSTALLATION OF THE WATER AND WASTEWATER IMPROVEMENTS. 6. ALL PUBLIC WATER AND WASTEWATER CONSTRUCTION, PIPE, STRUCTURES, AND FITTINGS SHALL ADHERE TO CITY PUBLIC WORKS STANDARD DETAILS AND SPECIFICATIONS. CONTRACTOR SHALL ARRANGE FOR REQUIRED CITY INSPECTIONS 7. ALL PRIVATE WATER AND WASTEWATER CONSTRUCTION, PIPE, STRUCTURES, AND FITTINGS SHALL ADHERE TO THE APPLICABLE

3. CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS SHOWN, INCLUDING THE HORIZONTAL AND VERTICAL LOCATION OF

PLUMBING CODE. CONTRACTOR SHALL ARRANGE FOR REQUIRED CITY INSPECTIONS. 8. FIRE SPRINKLER LINES SHALL BE DESIGNED AND INSTALLED BY A LICENSED FIRE SPRINKLER CONTRACTOR, AND COMPLY TO THE APPLICABLE CODES AND INSPECTIONS REQUIRED. THESE PLANS WERE PREPARED WITHOUT THE BENEFIT OF THE FIRE SPRINKLER DESIGN. CONTRACTOR SHALL NOTIFY THE ENGINEER IF ANY DISCREPANCIES. 9. EMBEDMENT FOR ALL WATER AND WASTEWATER LINES, PUBLIC OR PRIVATE, SHALL BE PER CITY STANDARD DETAILS. 10. CONTRACTOR SHALL TAKE REQUIRED SANITARY PRECAUTIONS, FOLLOWING ANY CITY, TCEQ, AND AWWA STANDARDS, TO KEEP

WATER PIPE AND FITTINGS CLEAN AND CAPPED AT TIMES WHEN INSTALLATION IS NOT IN PROGRESS.

SUBSIDIARY TO THE PROJECT AND NO ADDITIONAL COMPENSATION SHALL BE ALLOWED.

11.CONTRACTOR SHALL PROVIDE CONSTRUCTION SURVEYING FOR ALL WATER AND WASTEWATER LINES 12. ALL WATER AND WASTEWATER SERVICES SHALL TERMINATE 5-FEET OUTSIDE THE BUILDING, UNLESS NOTED OTHERWISE PRIOR NOTICE THAT IS REQUIRED, AND SHALL COORDINATE DIRECTLY WITH THE APPROPRIATE CITY DEPARTMENT. 14. CONTRACTOR SHALL SEQUENCE WATER AND WASTEWATER CONSTRUCTION TO AVOID INTERRUPTION OF SERVICE TO SURROUNDING 15. CONTRACTOR SHALL MAINTAIN WATER SERVICE AND WASTEWATER SERVICE TO ALL CUSTOMERS THROUGHOUT CONSTRUCTION (IF

NECESSARY, BY USE OF TEMPORARY METHODS APPROVED BY THE CITY AND OWNER). THIS WORK SHALL BE CONSIDERED

16. THE CONTRACTOR IS RESPONSIBLE TO PROTECT ALL WATER AND WASTEWATER LINES CROSSING THE PROJECT. THE CONTRACTOR SHALL REPAIR ALL DAMAGED LINES IMMEDIATELY. ALL REPAIRS OF EXISTING WATER MAINS, WATER SERVICES, SEWER MAINS, AND SANITARY SEWER SERVICES ARE SUBSIDIARY TO THE WORK, AND NO ADDITIONAL COMPENSATION SHALL BE ALLOWED. 17. VALVE ADJUSTMENTS SHALL BE CONSTRUCTED SUCH THAT THE COVERS ARE AT FINISHED SURFACE GRADE OF THE DEVELOPED

PAVEMENT. 18 THE ENDS OF ALL EXISTING WATER MAINS THAT ARE CUT BUT NOT REMOVED. SHALL BE PLUGGED AND ABANDONED IN PLACE. THIS WORK SHALL BE CONSIDERED AS A SUBSIDIARY COST TO THE PROJECT AND NO ADDITIONAL COMPENSATION SHALL BE ALLOWED. 19. ALL FIRE HYDRANTS, VALVES, TEES, BENDS, WYES, REDUCERS, FITTINGS, AND ENDS SHALL BE MECHANICALLY RESTRAINED AND/OR

THRUST BLOCKED TO CITY STANDARDS. 20.CONTRACTOR SHALL INSTALL A FULL SEGMENT OF WATER OR WASTEWATER PIPE CENTERED AT ALL UTILITY CROSSINGS SO THAT THE JOINTS ARE GREATER THAN 9-FEET FROM THE CROSSING 21.ALL CROSSINGS AND LOCATIONS WHERE WASTEWATER IS LESS THAN 9-FEET FROM WATER, WASTEWATER CONSTRUCTION AND

MATERIALS SHALL COMPLY WITH TCFO CHAPTER 217 53 22.ALL CROSSING AND LOCATIONS WHERE WATER IS LESS THAN 9-FEET FROM WASTEWATER, WATER CONSTRUCTION AND MATERIALS SHALL COMPLY WITH TCEQ CHAPTER 290.44. 23.ALL WATER AND WASTEWATER SHALL BE TESTED IN ACCORDANCE WITH THE CITY, AWWA, AND TCEQ STANDARDS AND SPECIFICATIONS. AT A MINIMUM, THIS SHALL CONSIST OF THE FOLLOWING:

a. ALL WATERLINES SHALL BE HYDROSTATICALLY TESTED AND CHLORINATED BEFORE BEING PLACED INTO SERVICE. CONTRACTOR SHALL COORDINATE WITH THE CITY FOR THEIR REQUIRED PROCEDURES AND SHALL ALSO COMPLY WITH TCEQ REGULATIONS. b. WASTEWATER LINES AND MANHOLES SHALL BE PRESSURE TESTED. CONTRACTOR SHALL COORDINATE WITH THE CITY FOR THEIR REQUIRED PROCEDURES AND SHALL ALSO COMPLY WITH TCEQ REGULATIONS. AFTER COMPLETION OF THESE TESTS, A TELEVISION INSPECTION SHALL BE PERFORMED AND PROVIDED TO THE CITY AND OWNER ON A DVD.

24. CONTRACTOR SHALL INSTALL DETECTABLE WIRING OR MARKING TAPE A MINIMUM OF 12" ABOVE WATER AND WASTEWATER LINES. MARKER DECALS SHALL BE LABELED "CAUTION - WATER LINE", OR "CAUTION - SEWER LINE". DETECTABLE WIRING AND MARKING TAPE SHALL COMPLY WITH CITY STANDARDS, AND SHALL BE INCLUDED IN THE COST OF THE WATER AND WASTEWATER PIPE. 25.DUCTILE IRON PIPE SHALL BE PROTECTED FROM CORROSION BY A LOW-DENSITY POLYETHYLENE LINER WRAP THAT IS AT LEAST A SINGLE LAYER OF 8-MIL. ALL DUCTILE IRON JOINTS SHALL BE BONDED.

26 WATERLINES SHALL BE INSTALLED AT NO LESS THAN THE MINIMUM COVER REQUIRED BY THE CITY

27.CONTRACTOR SHALL PROVIDE CLEAN-OUTS FOR PRIVATE SANITARY SEWER LINES AT ALL CHANGES IN DIRECTION AND 100-FOOT INTERVALS, OR AS REQUIRED BY THE APPLICABLE PLUMBING CODE. CLEAN-OUTS REQUIRED IN PAVEMENT OR SIDEWALKS SHALL HAVE CAST IRON COVERS FLUSH WITH FINISHED GRADE. 28.CONTRACTOR SHALL PROVIDE BACKWATER VALVES FOR PLUMBING FIXTURES AS REQUIRED BY THE APPLICABLE PLUMBING CODE (E.C. FLOOR ELEVATION OF FIXTURE UNIT IS BELOW THE ELEVATION OF THE MANHOLE COVER OF THE NEXT UPSTREAM MANHOLE IN THE PUBLIC SEWER). CONTRACTOR SHALL REVIEW BOTH MEP AND CIVIL PLANS TO CONFIRM WHERE THESE ARE REQUIRED

29. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND SUBMITTING A TRENCH SAFETY PLAN, PREPARED BY A PROFESSIONAL ENGINEER IN THE STATE OF TEXAS, TO THE CITY PRIOR TO CONSTRUCTION. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING TRENCH SAFETY REQUIREMENTS IN ACCORDANCE WITH CITY, STATE, AND FEDERAL REQUIREMENTS, INCLUDING OSHA FOR ALL TRENCHES. NO OPEN TRENCHES SHALL BE ALLOWED OVERNIGHT WITHOUT PRIOR WRITTEN APPROVAL OF THE CITY.

30. THE CONTRACTOR SHALL KEEP TRENCHES FREE FROM WATER. **ABBREVIATIONS AND DEFINITIONS** AMERICANS WITH DISABILITIES ACT ADA AMERICAN WATER WORKS ASSOCIATION B-B BACK TO BACK **BEGIN CURVE** BACK OF CURB BEGIN CURB RETURN BEST MANAGEMENT PRACTICE BOC BACK OF CURB BEGIN VERTICAL CURVE ELEVATION 9. PRIVATE CURB RAMPS ON THE SITE (I.E. OUTSIDE PUBLIC STREET RIGHT-OF-WAY) SHALL CONFORM TO ADA AND TAS STANDARDS AND BVCS BEGIN VERTICAL CURVE STATION BW **BOTTOM OF WALL** CFS CUBIC FEET PER SECOND CITY, TOWN, OR OTHER APPLICABLE LOCAL GOVERNMENT JURISDICTION CENTERLINE CENTERI INF CONCRETE CUBIC YARD DEMOLITION DECOMPOSED GRANITE DG DTL DETAIL EACH END CURVE **ECR** END CURB RETURN EXISTING GROUND FI EVATION ELECTRICAL / ELECTRICITY ELEV FI EVATION UNITES STATES ENVIRONMENTAL PROTECTION AGENCY EASEMENT END VERTICAL CURVE ELEVATION END VERTICAL CURVE STATION EXISTING

ESMT **EVCE EVCS** FACE TO FACE FINISHED GROUND FIRE HYDRANT FLOW LINE FACE OF CURB HYDRAULIC GRADE LINE

HGL KIMLEY-HORN AND ASSOCIATES, INC KIMLEY-HORN AND ASSOCIATES, INC LINEAR FEET MAXIMUM

MATCH EXISTING ELEVATION MANHOLE MINUTE / MINIMUM NOTICE OF INTENT, REF. TCEQ GENERAL PERMIT

NOTICE OF TERMINATION, REF. TCEQ GENERAL PERMIT NOT TO SCALE ON CENTER

OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION POINT OF CURVATURE PORTLAND CEMENT CONCRETE / POINT OF COMPOUND CURVATURE DEVELOPED GRADE LINE

POINT OF INFLECTION PROP PROPOSED POINT OF REVERSE CURVATURE POUNDS PER SQUARE INCH POINT OF TANGENCY

POLYVINYL CHLORIDE POINT OF VERTICAL INFLECTION PAVEMENT REINFORCED CONCRETE PIPE

RIGHT OF WAY SQUARE FEET SANITARY SEWER SANITARY SEWER MANHOLE STATION

STANDARD SQUARE YARD ARCHITECTURAL BARRIERS TEXAS ACCESSIBILITY STANDARDS TOP OF CURB

TEXAS COMMISSION OF ENVIRONMENTAL QUALITY TEMPORAR' TXDOT TEXAS DEPARTMENT OF TRANSPORTATION TXMUTCD TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES

VERTICAL CURVE WTR WATER WASTEWATER

TOP OF WALL

**TYPICAL** 

UTILITY CONTACTS: 1. <u>TELECOM COMPANY</u>, **REFERENCE COVER** 

2. <u>CABLE COMPANY</u>, **REFERENCE COVER** 

3. <u>ELECTRIC COMPANY</u>, **REFERENCE COVER** 

4. GAS COMPANY, REFERENCE COVER 5. <u>UTILITY DISTRICT</u>, **REFERENCE COVER** 

6. <u>CITY WATER/UTILITIES DEPARTMENT</u>, **REFERENCE COVER** 

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THESE PLAN AND GENERAL NOTES REFER TO: GEOTECHNICAL ENGINEERING REPORT (FIRM) ECS SOUTHWEST, LLP ECS PROJECT NUMBER 17:6399-A (DATE) MARCH 22, 2024 INCLUDING ALL REVISIONS AND ADDENDA TO THIS REPORT THAT MAY HAVE BEEN RELEASED AFTER CONTRACTOR FIELD SET This plan set shall remain

on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

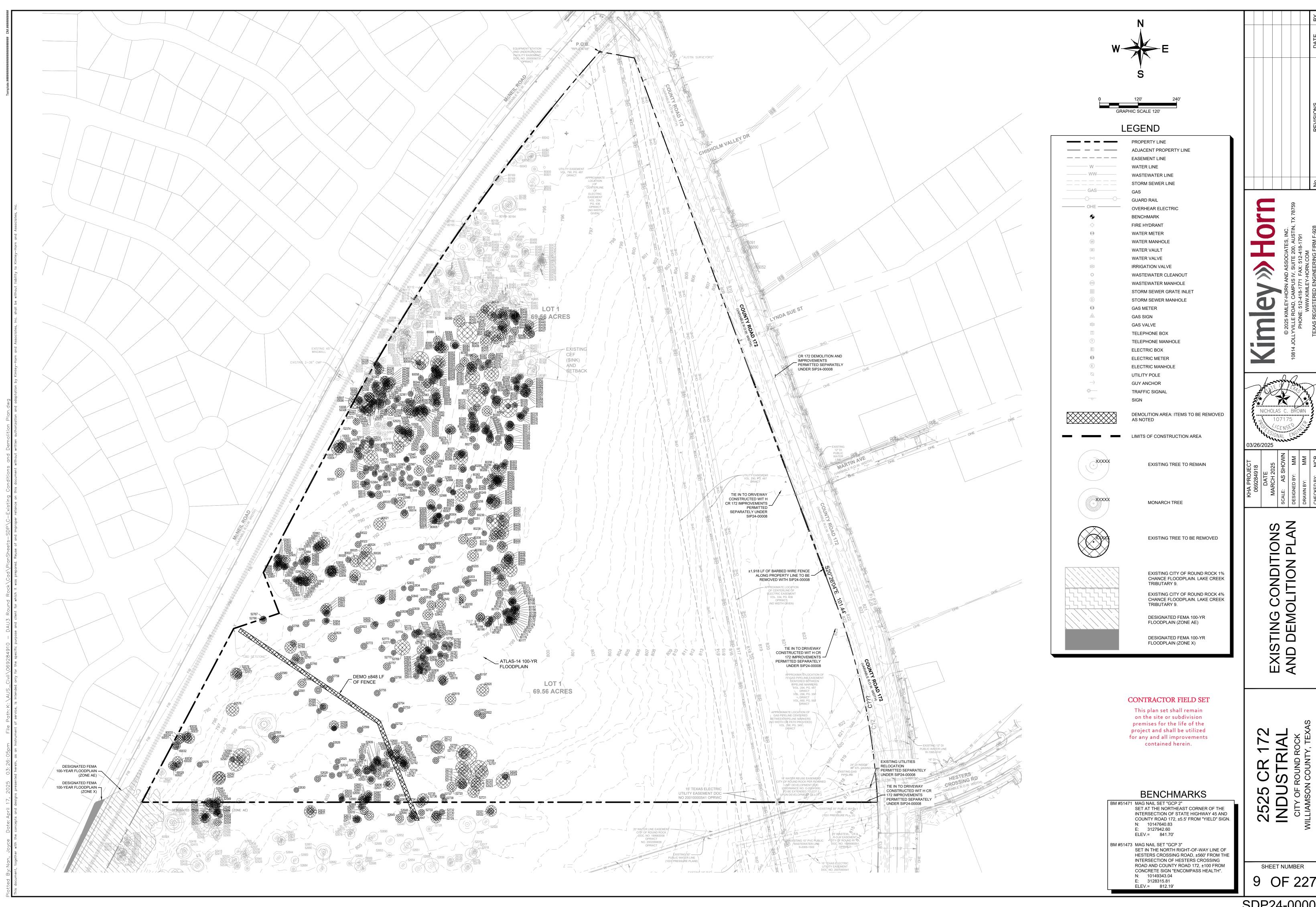
N THE EVENT OF CONFLICT, CITY OF ROUND ROCK NOTES SHALL SUPERCEDE.

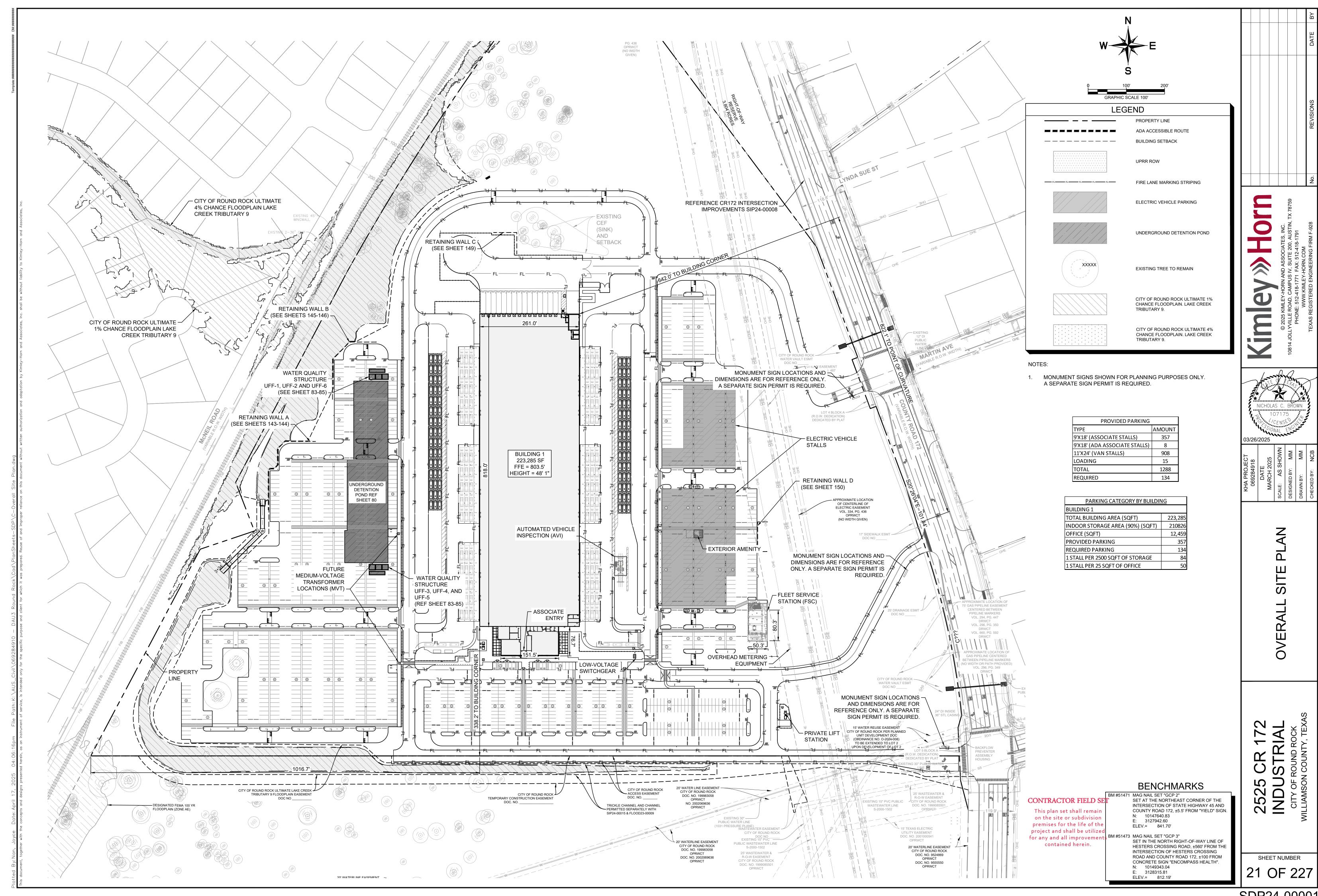
**BENCHMARKS** SET AT THE NORTHEAST CORNER OF THE INTERSECTION OF STATE HIGHWAY 45 AND

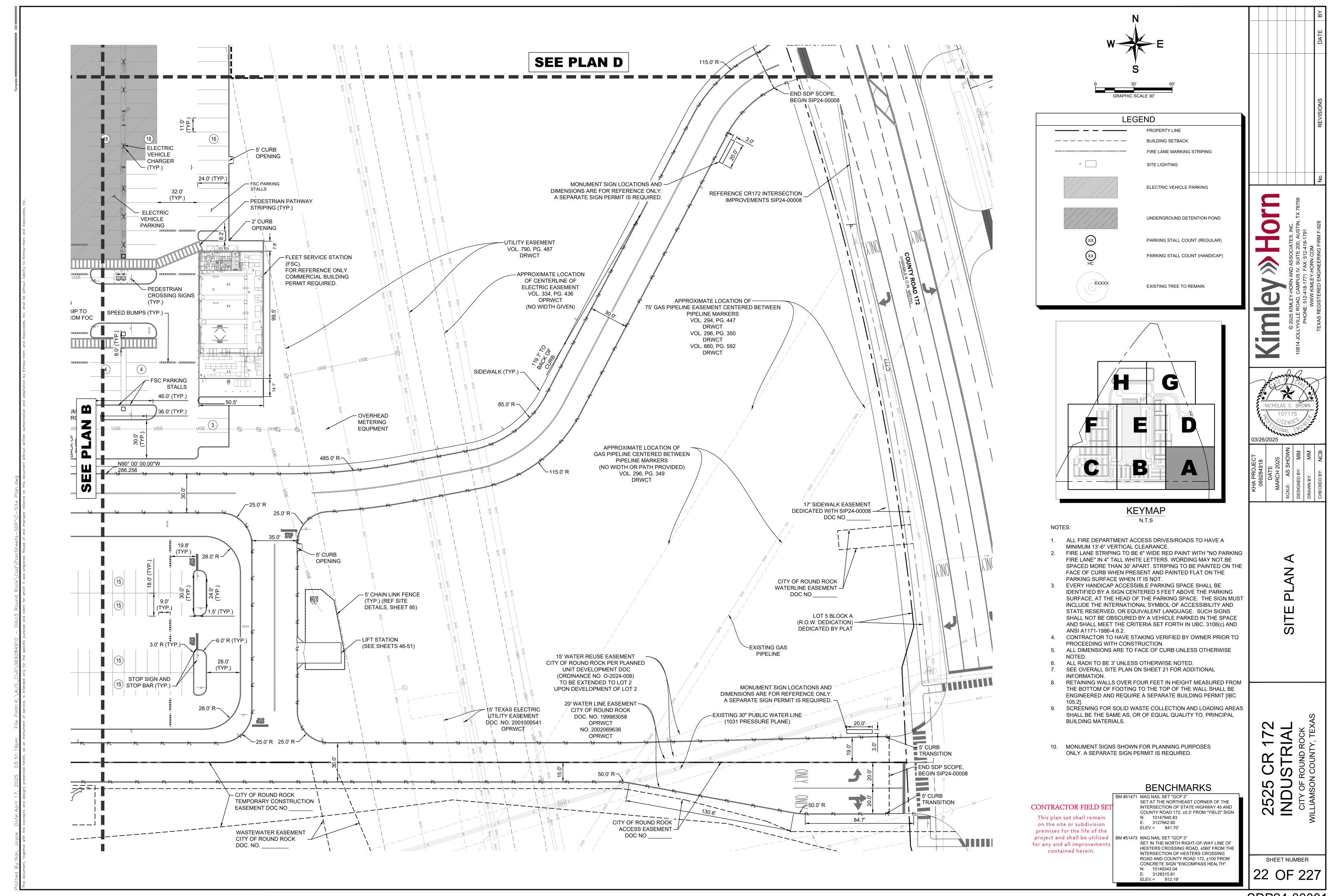
COUNTY ROAD 172, ±5.5' FROM "YIELD" SIGN. 3127942.60 ELEV.= 841.70' BM #51473 MAG NAIL SET "GCP 3"

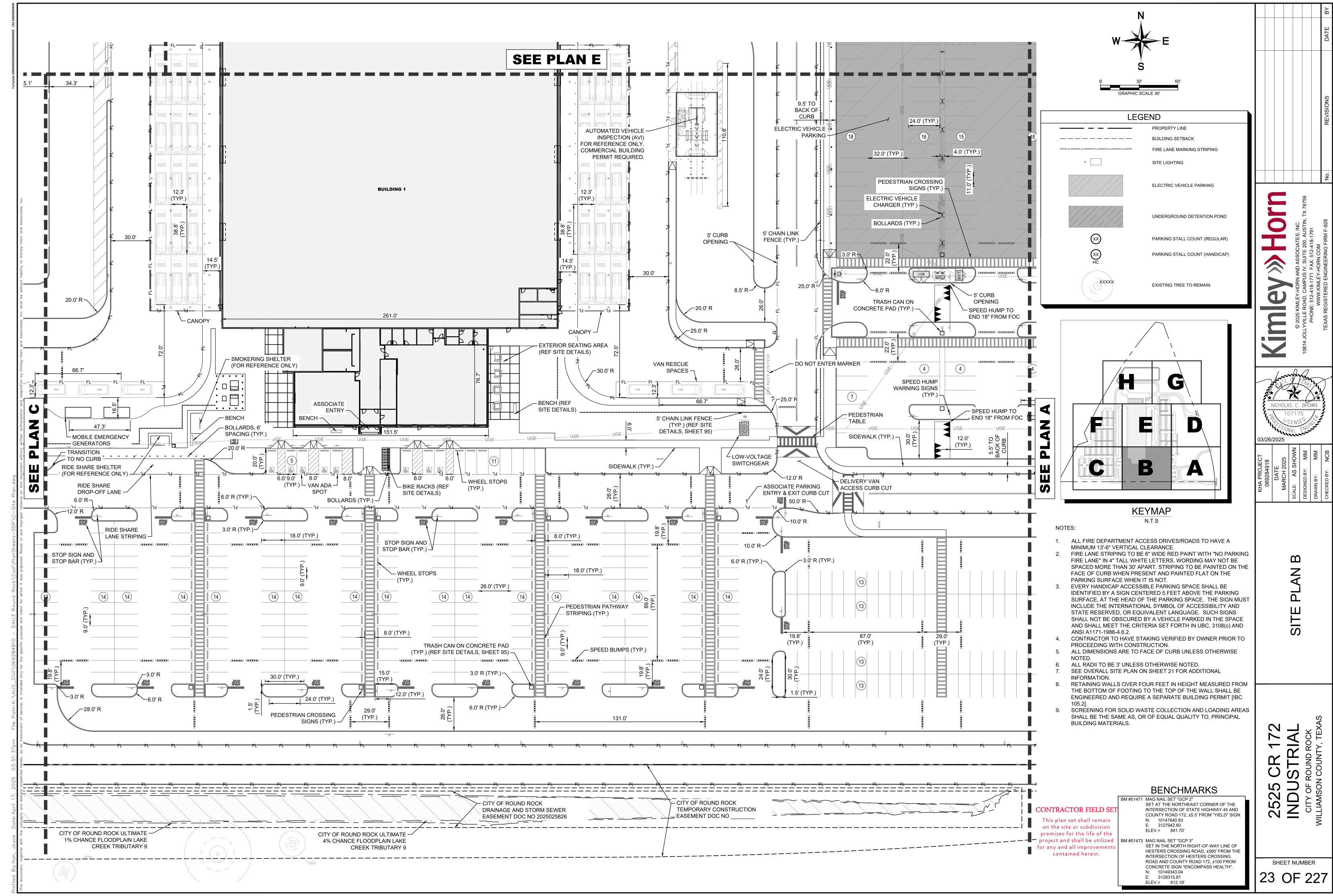
SET IN THE NORTH RIGHT-OF-WAY LINE OF HESTERS CROSSING ROAD, ±560' FROM THE INTERSECTION OF HESTERS CROSSING ROAD AND COUNTY ROAD 172, ±100 FROM CONCRETE SIGN "ENCOMPASS HEALTH". E: 3128315.81 ELEV.= 812.19'

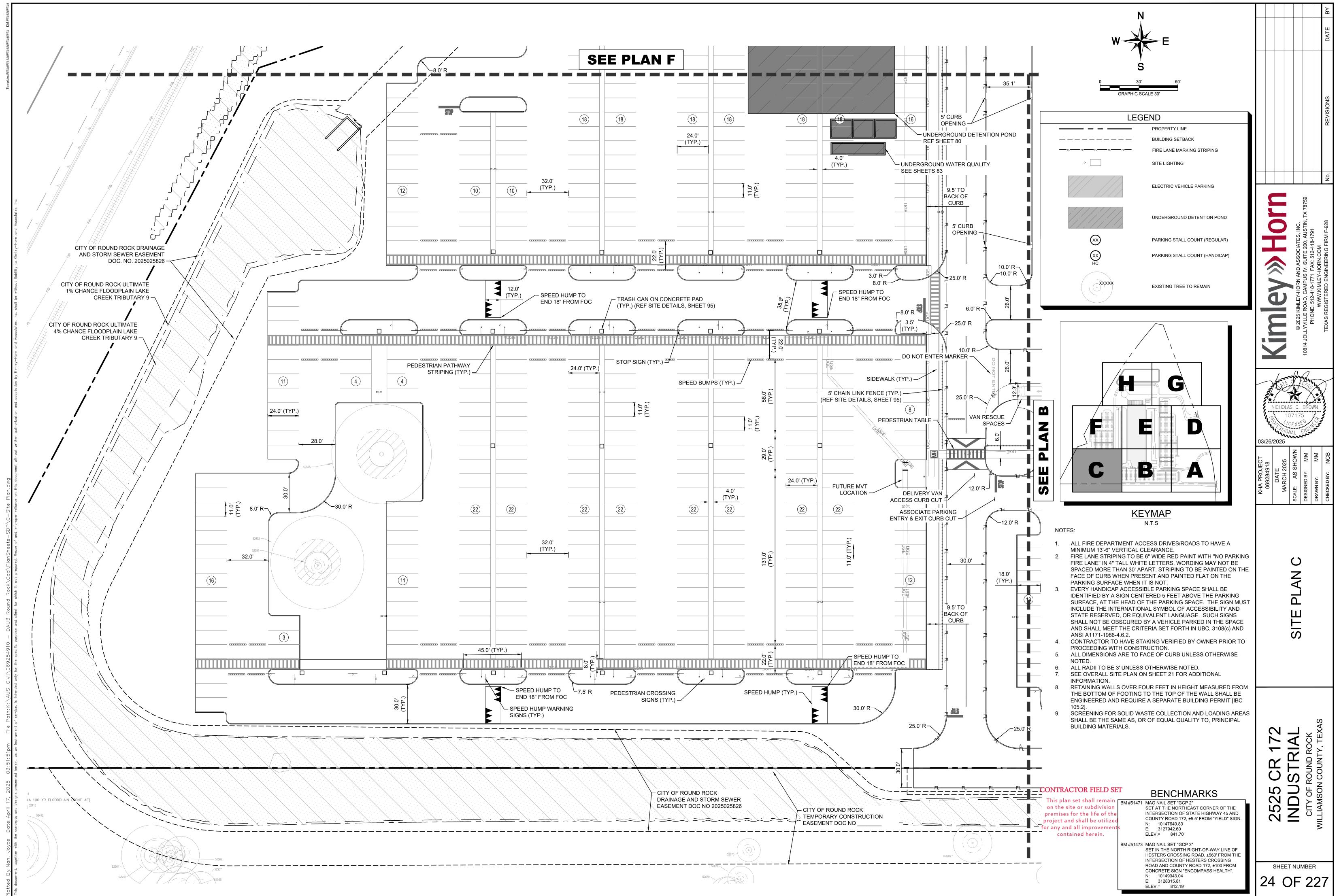
SHEET NUMBER

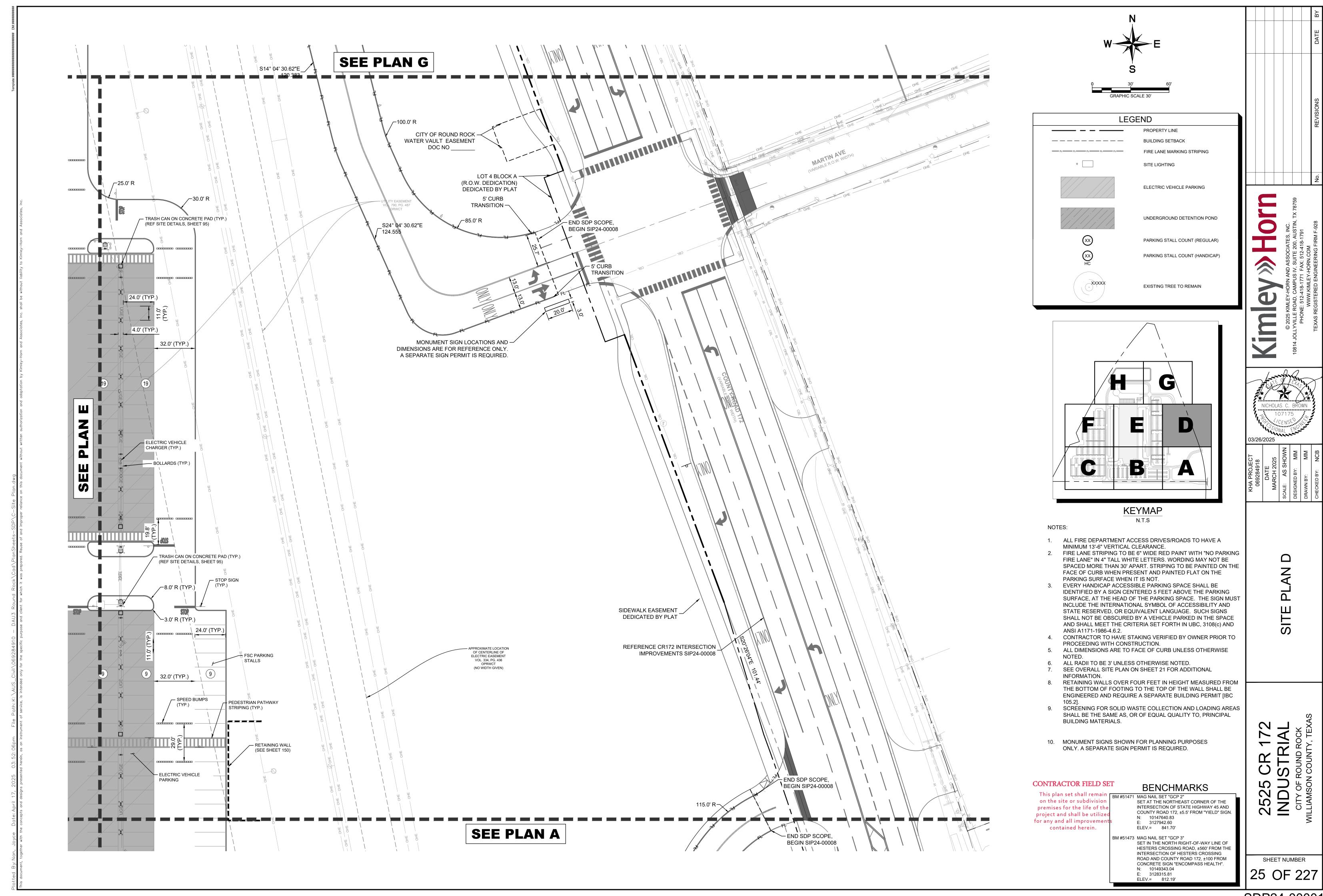


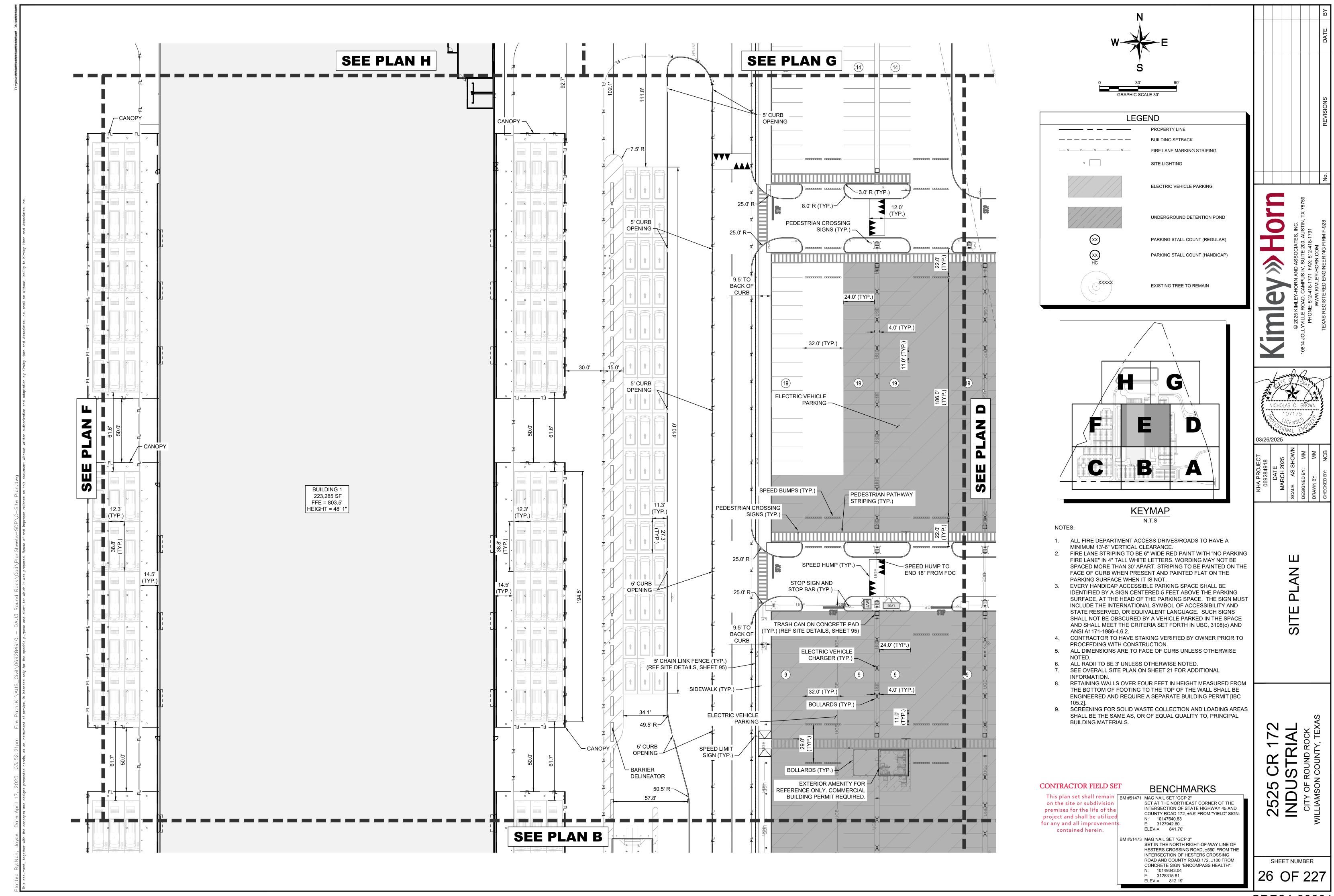


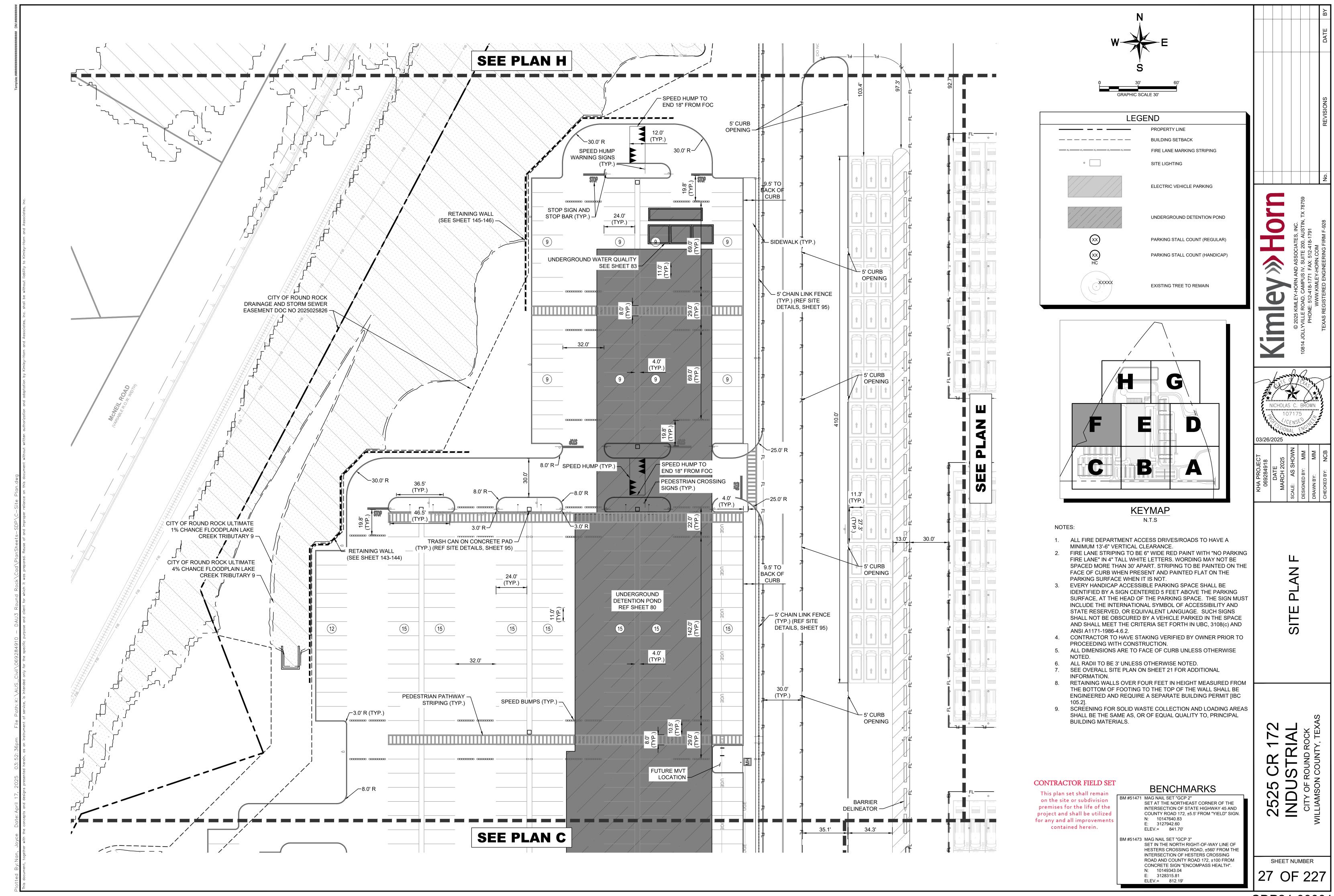


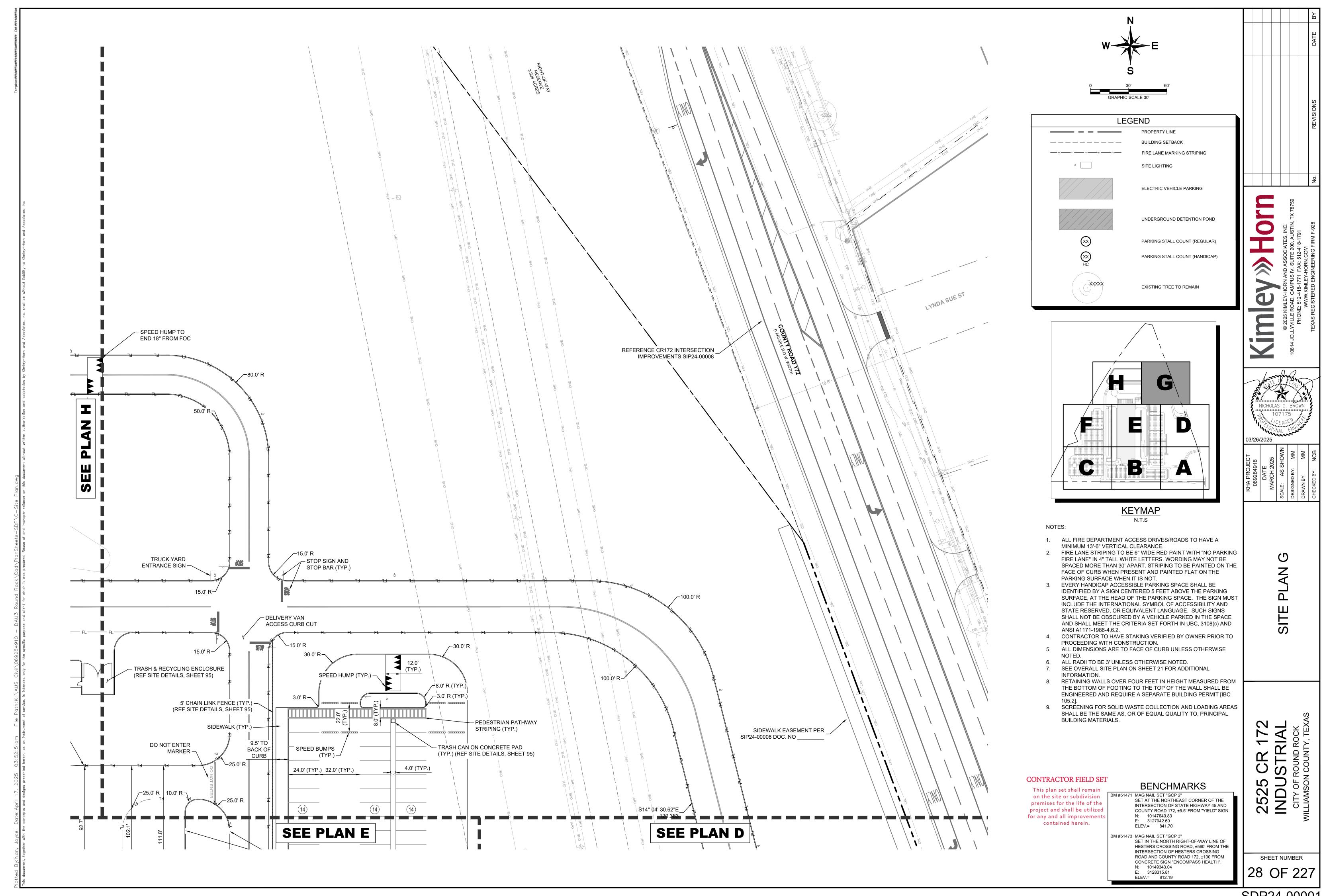


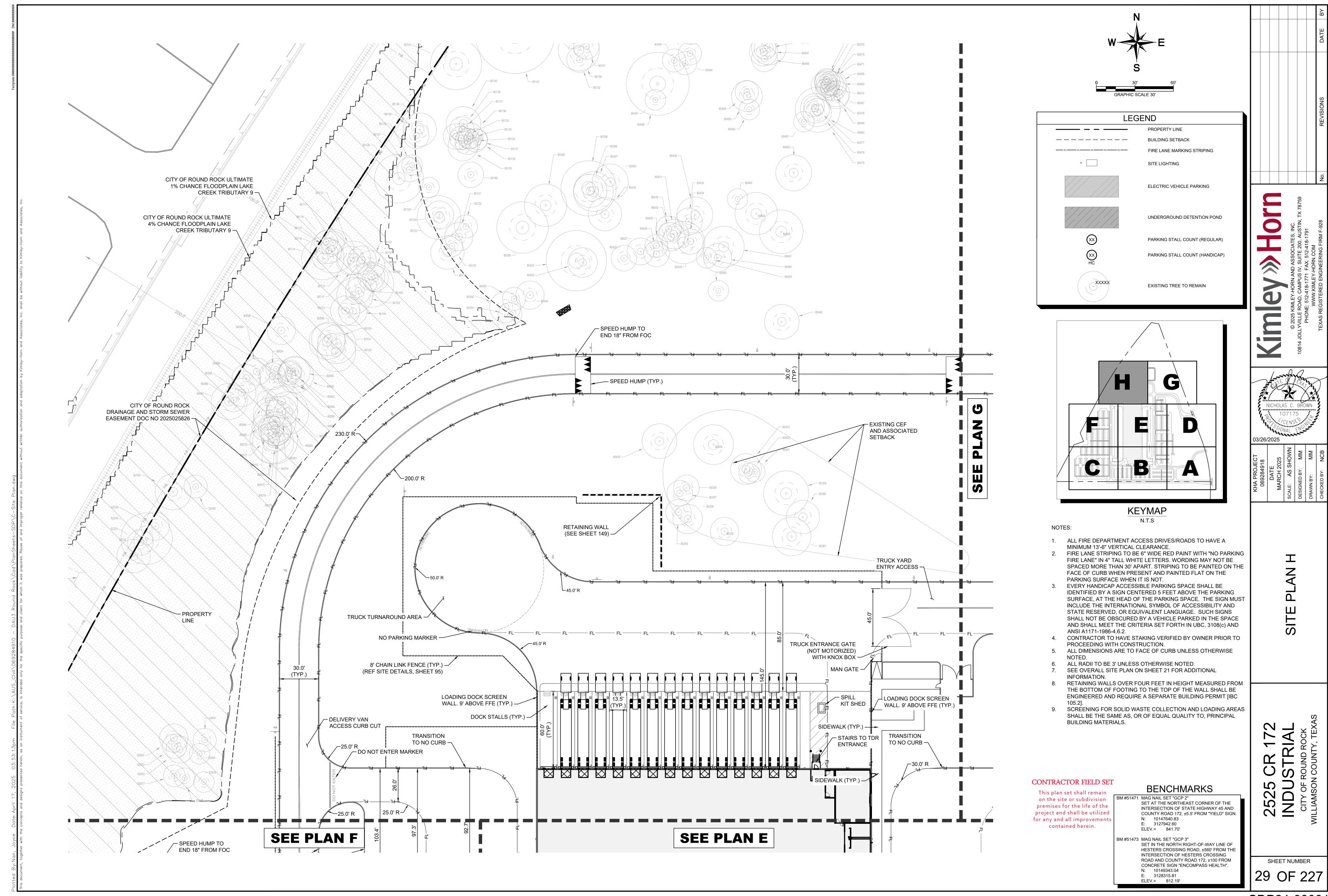


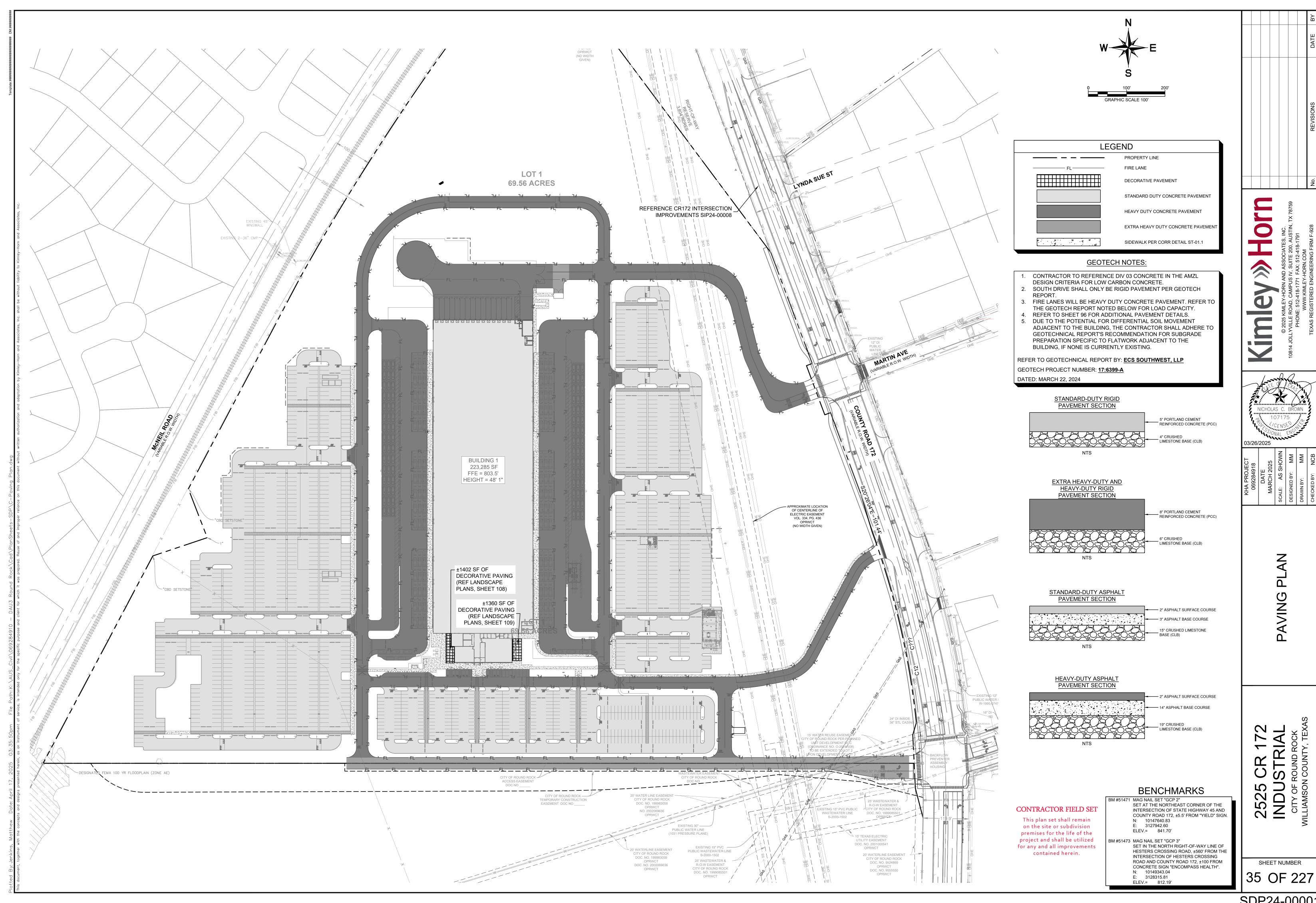


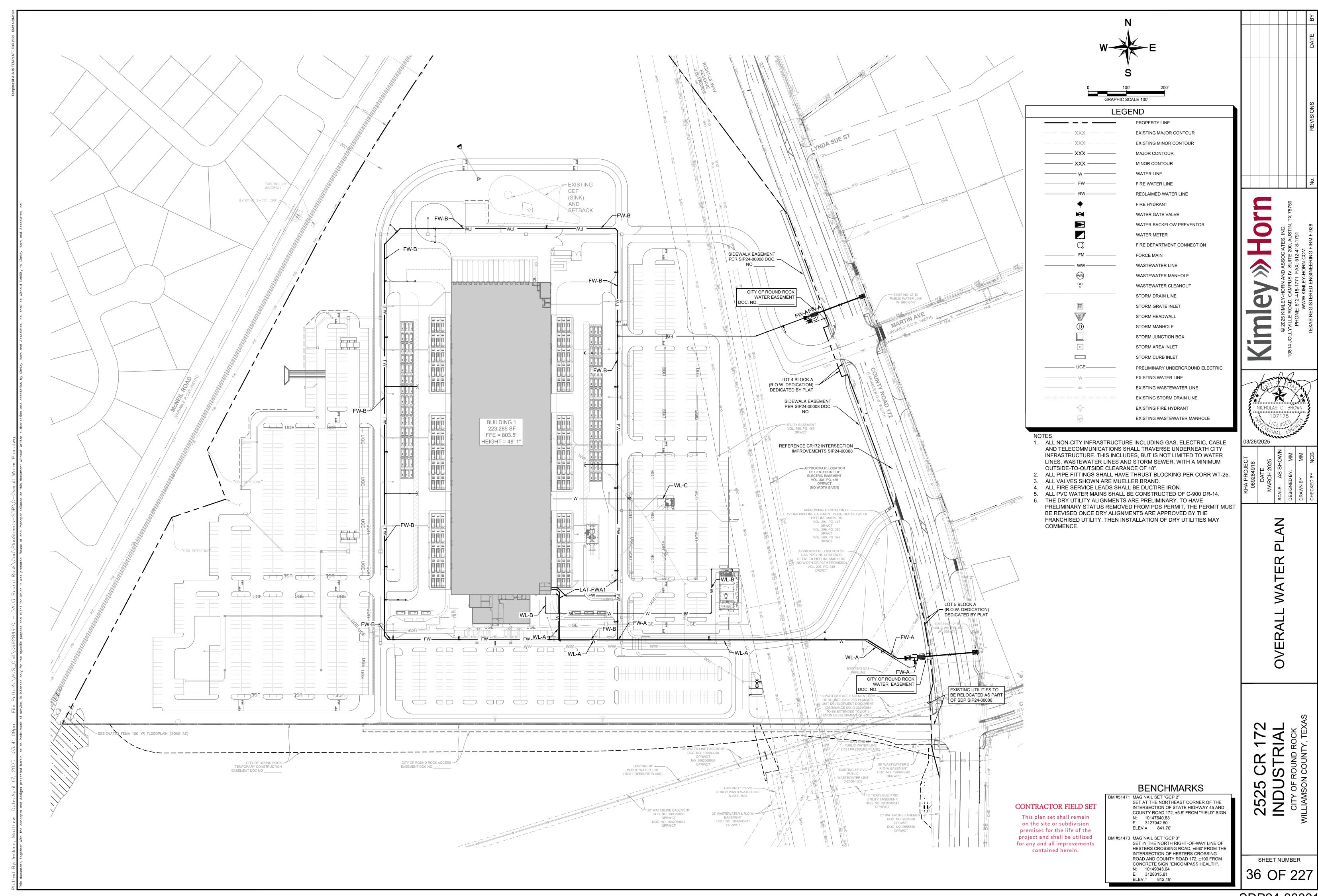


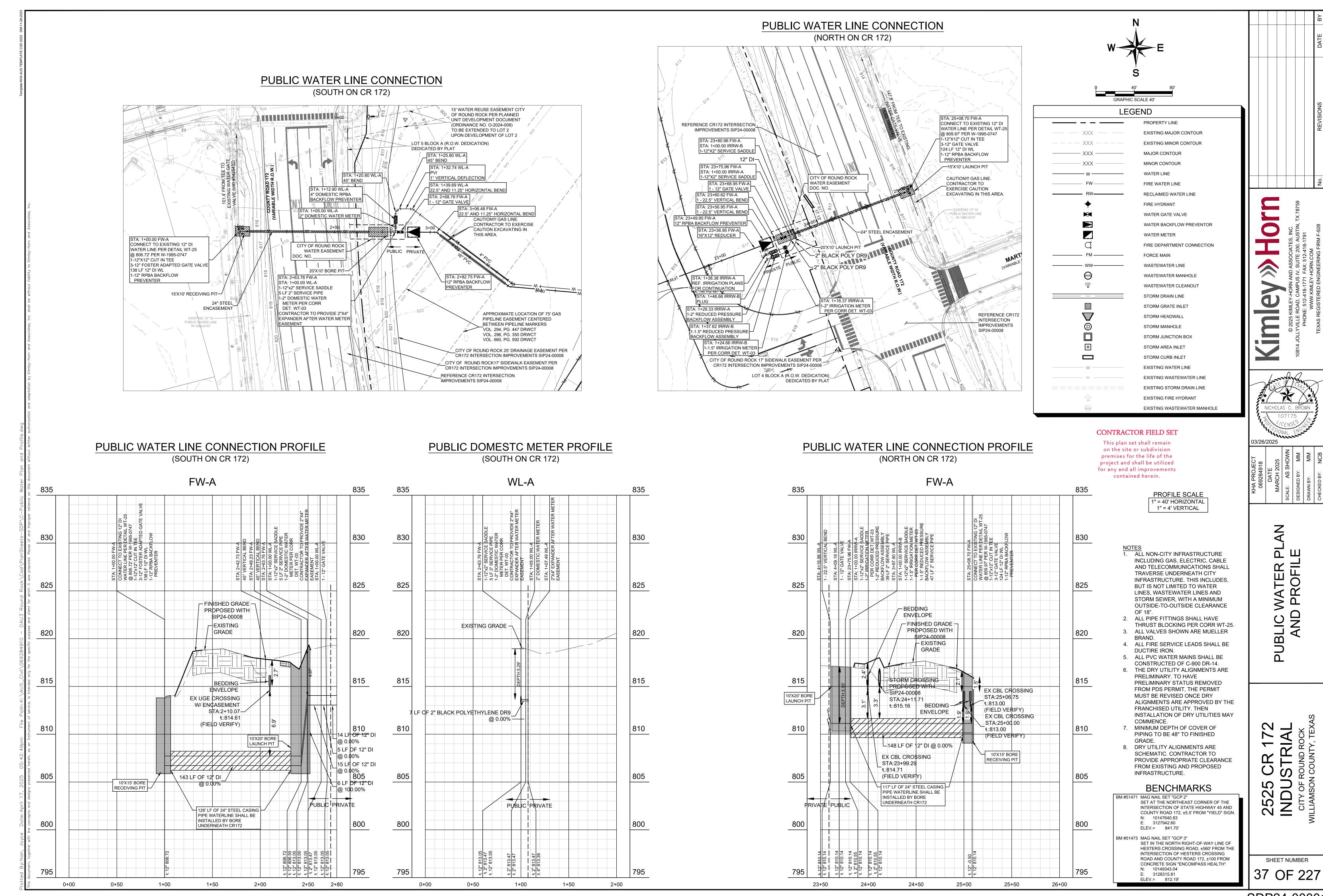


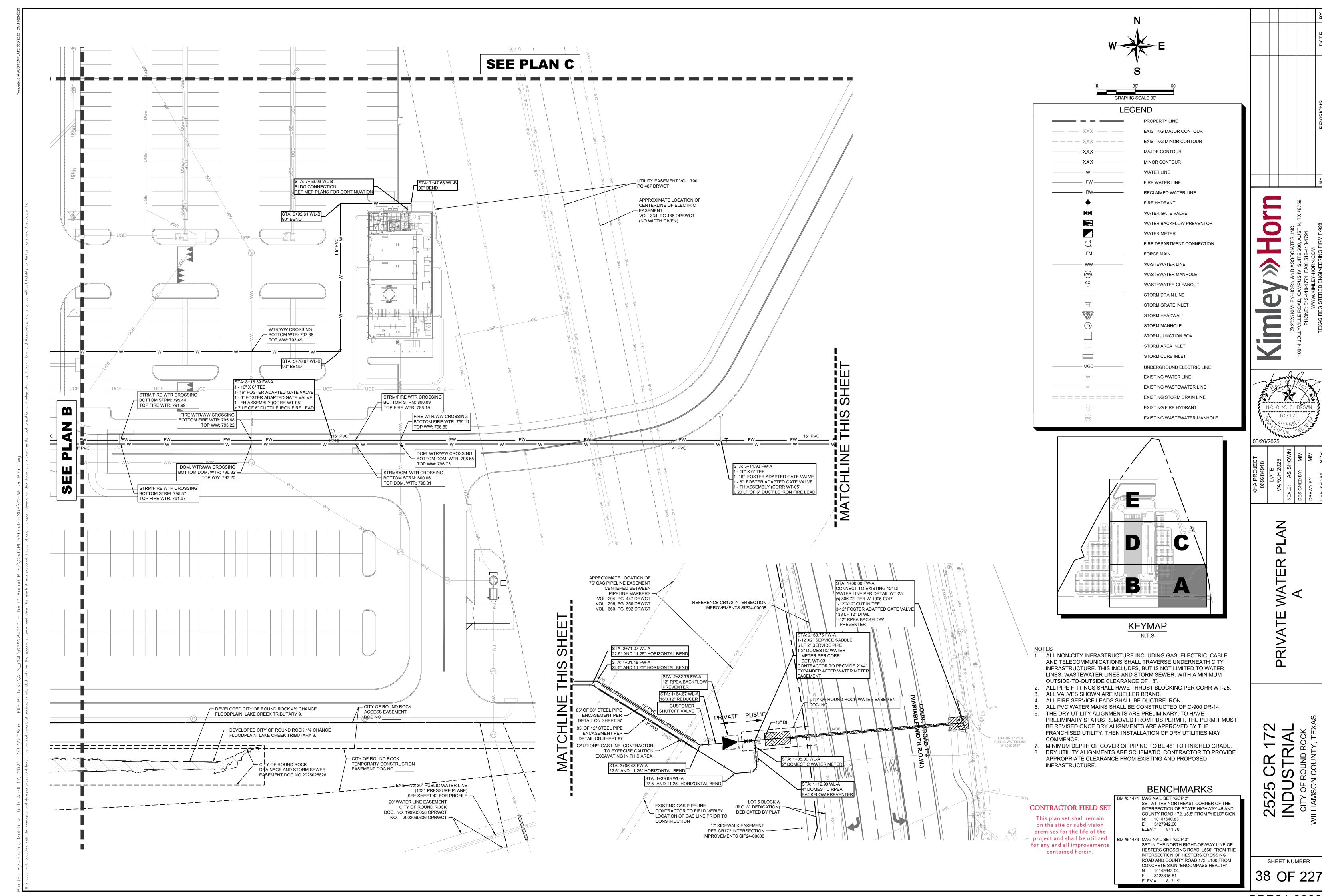


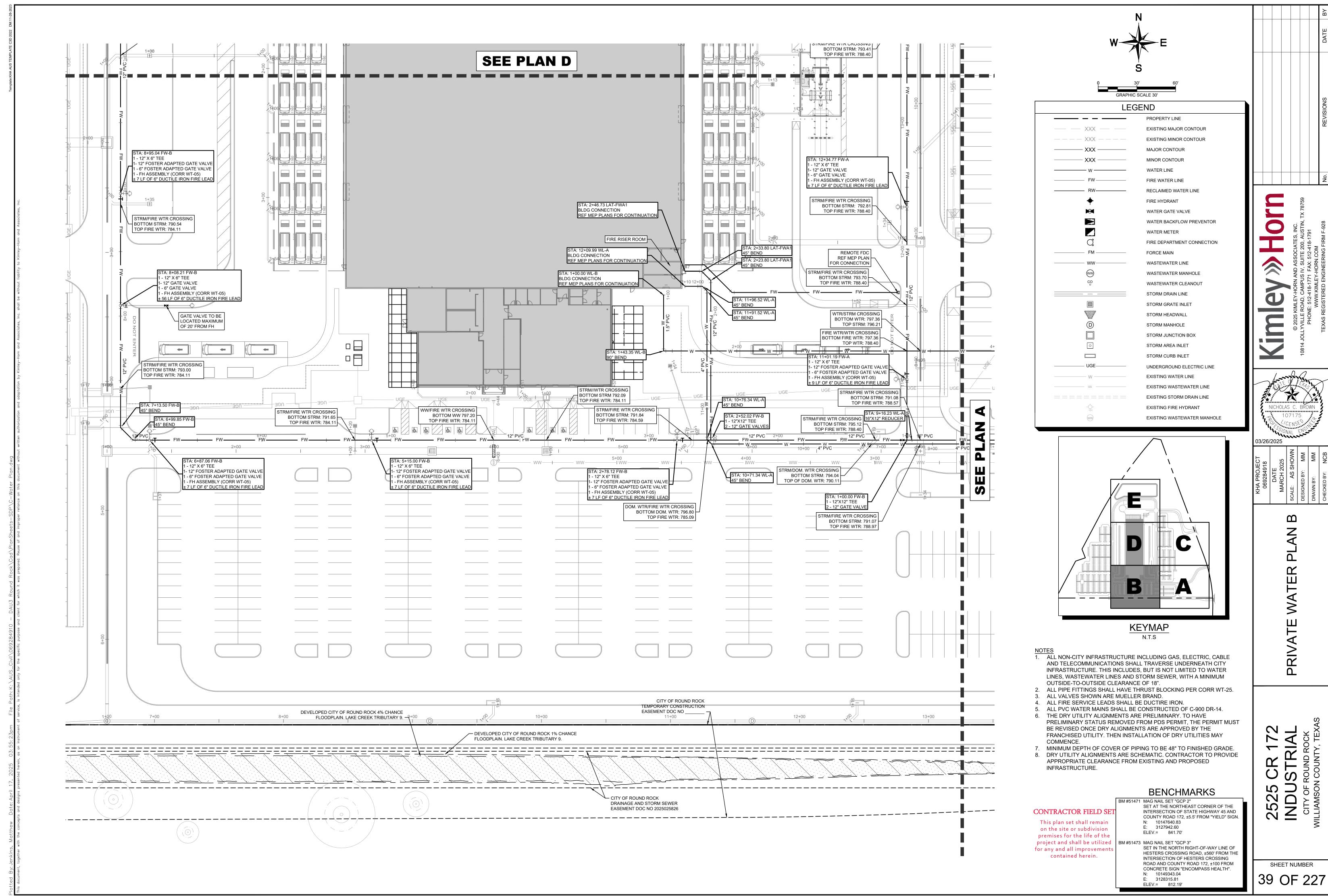


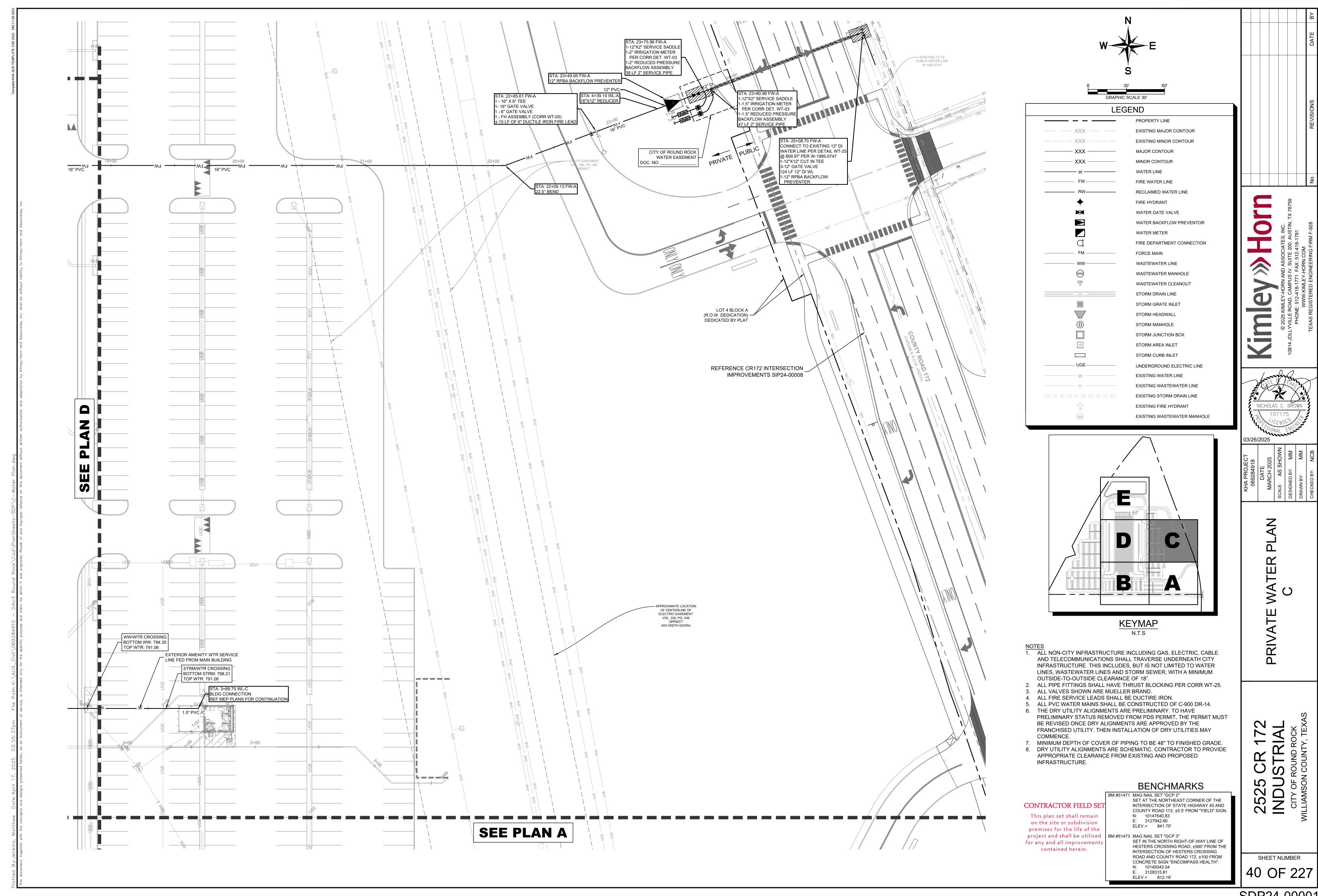


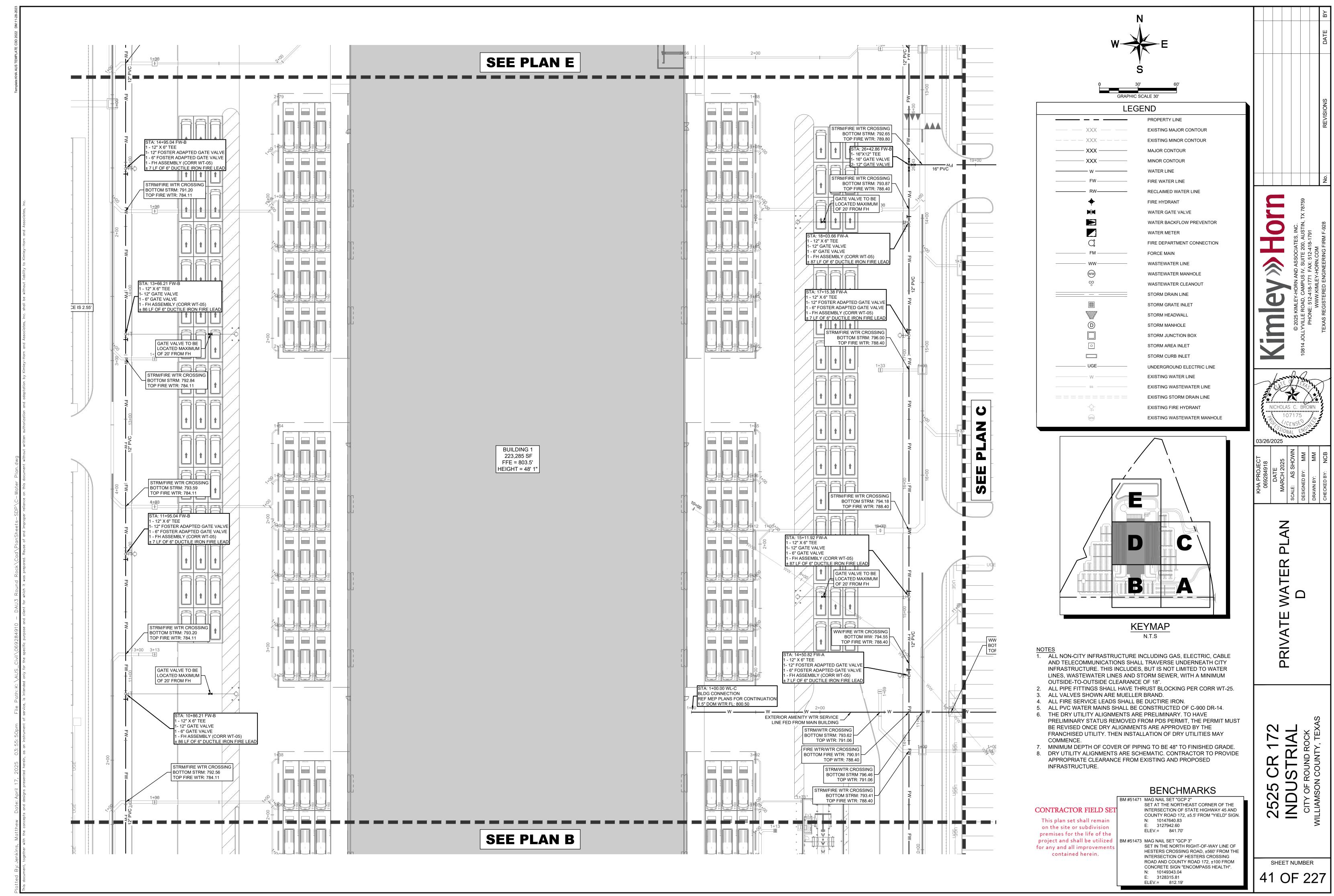


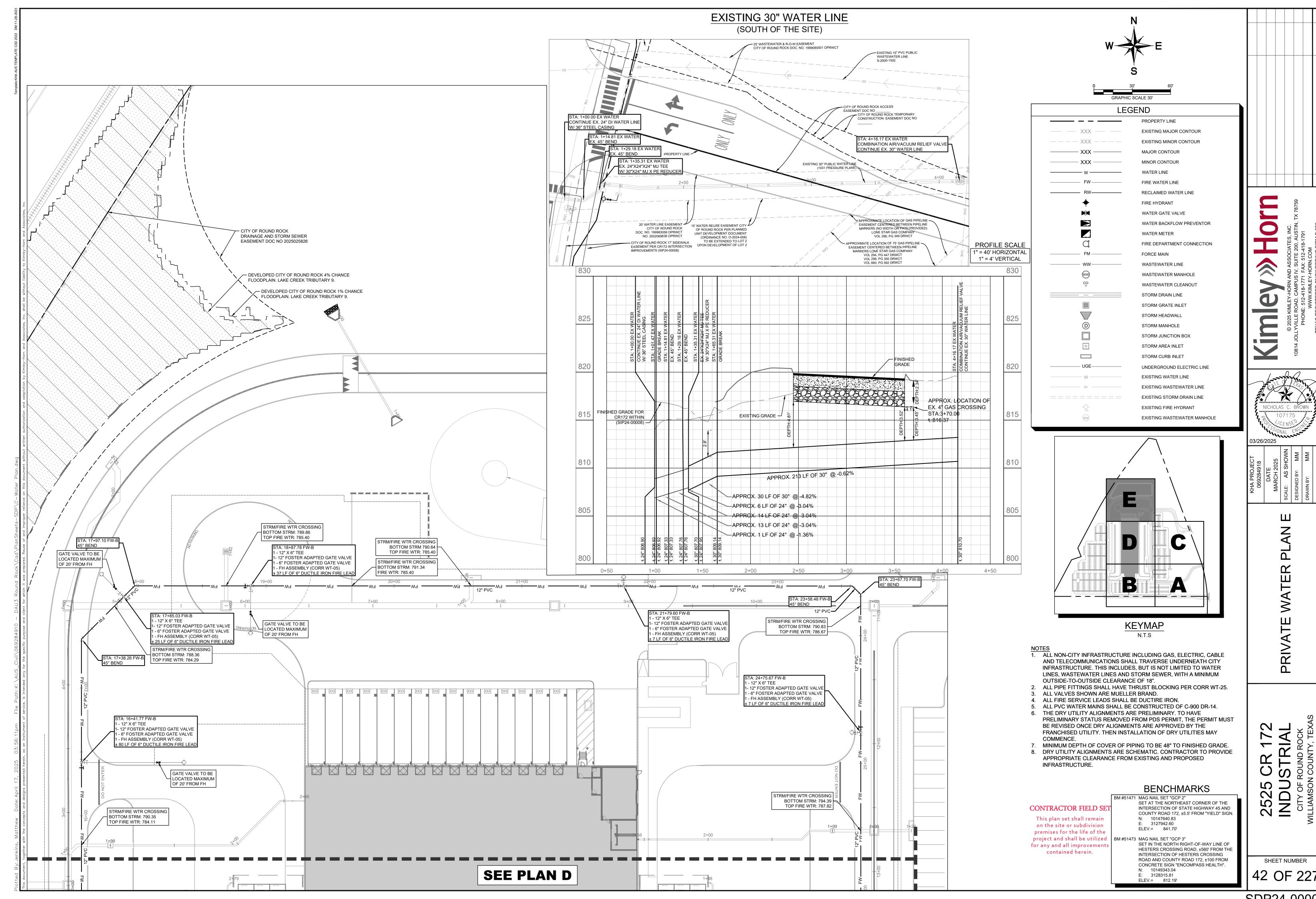


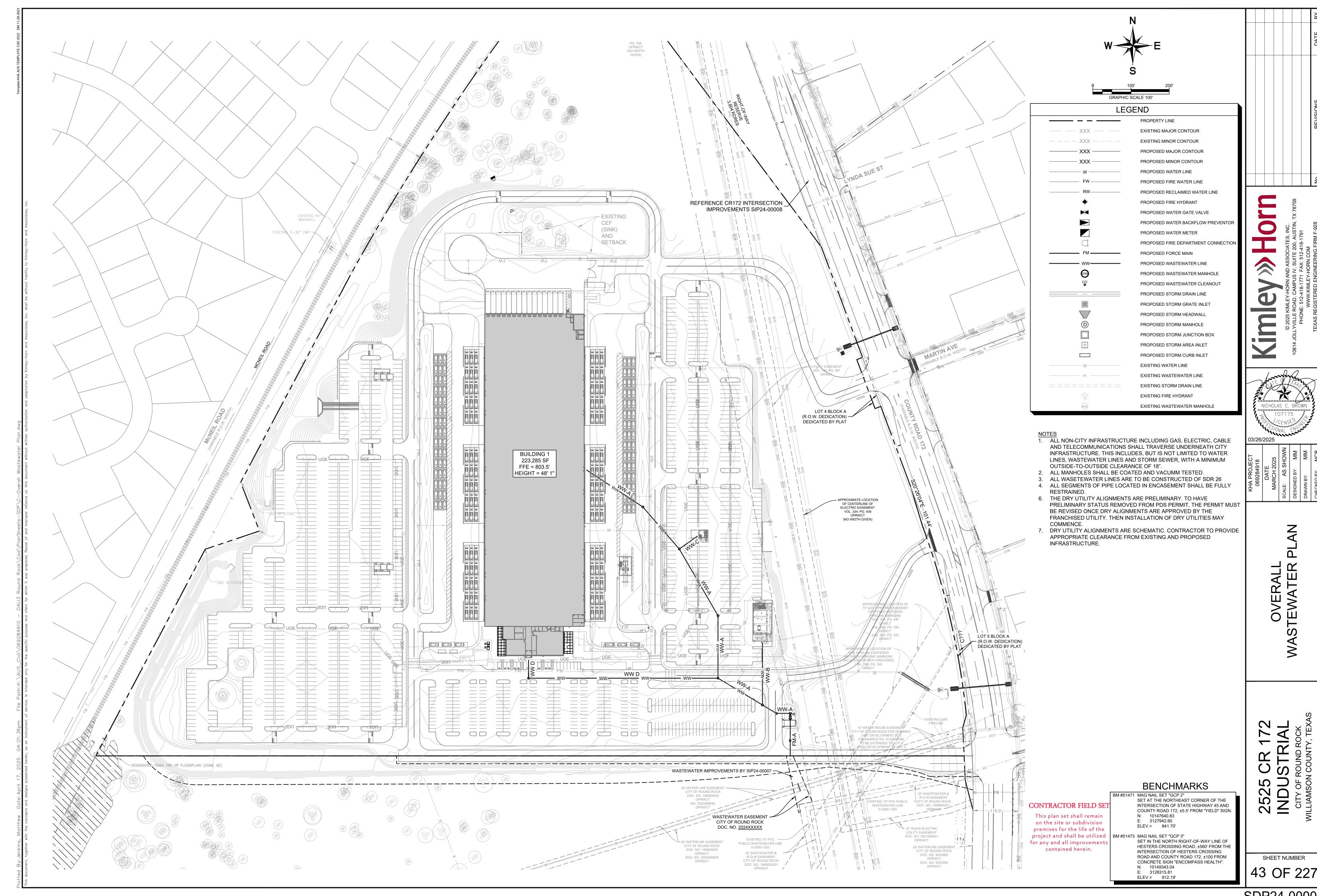


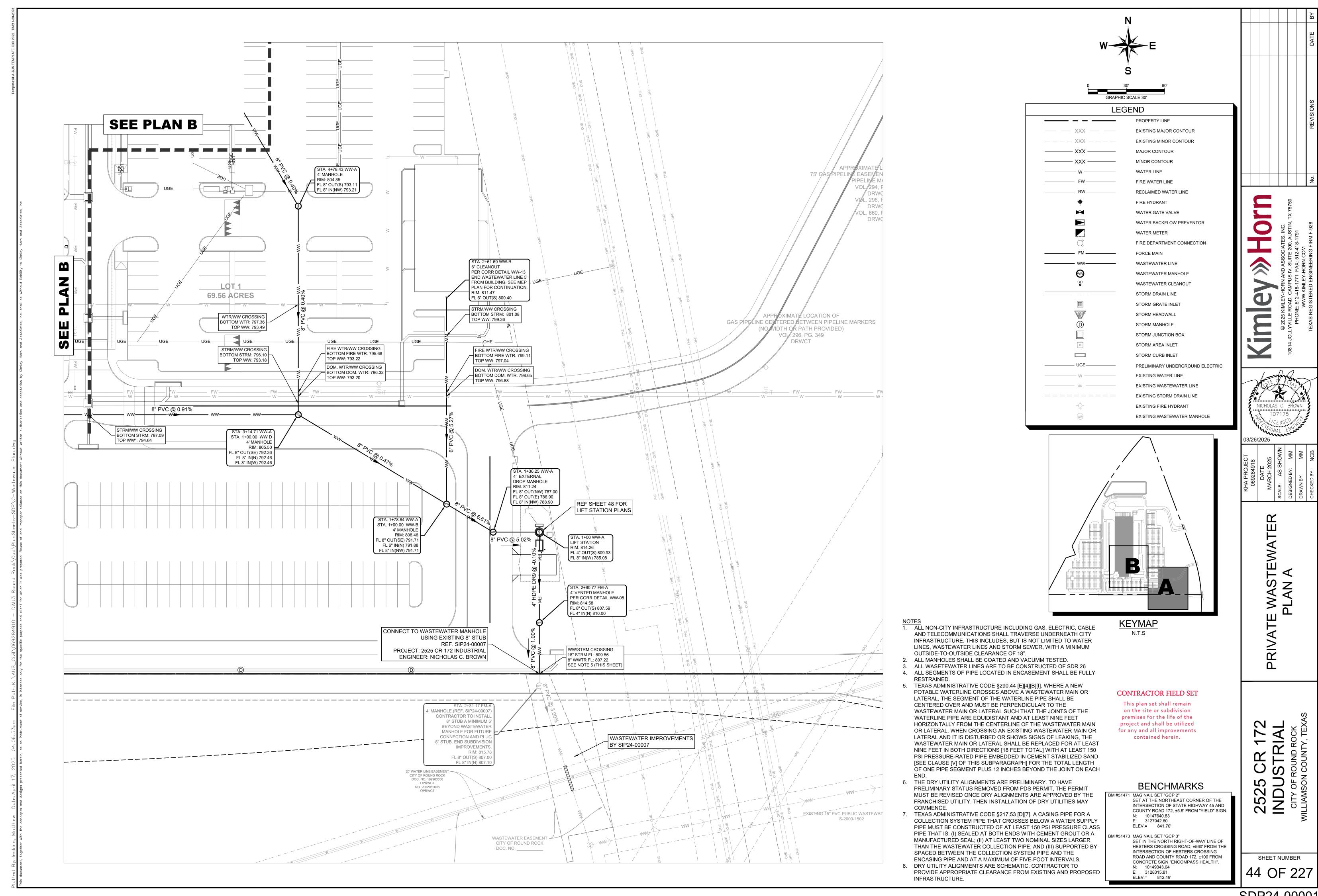


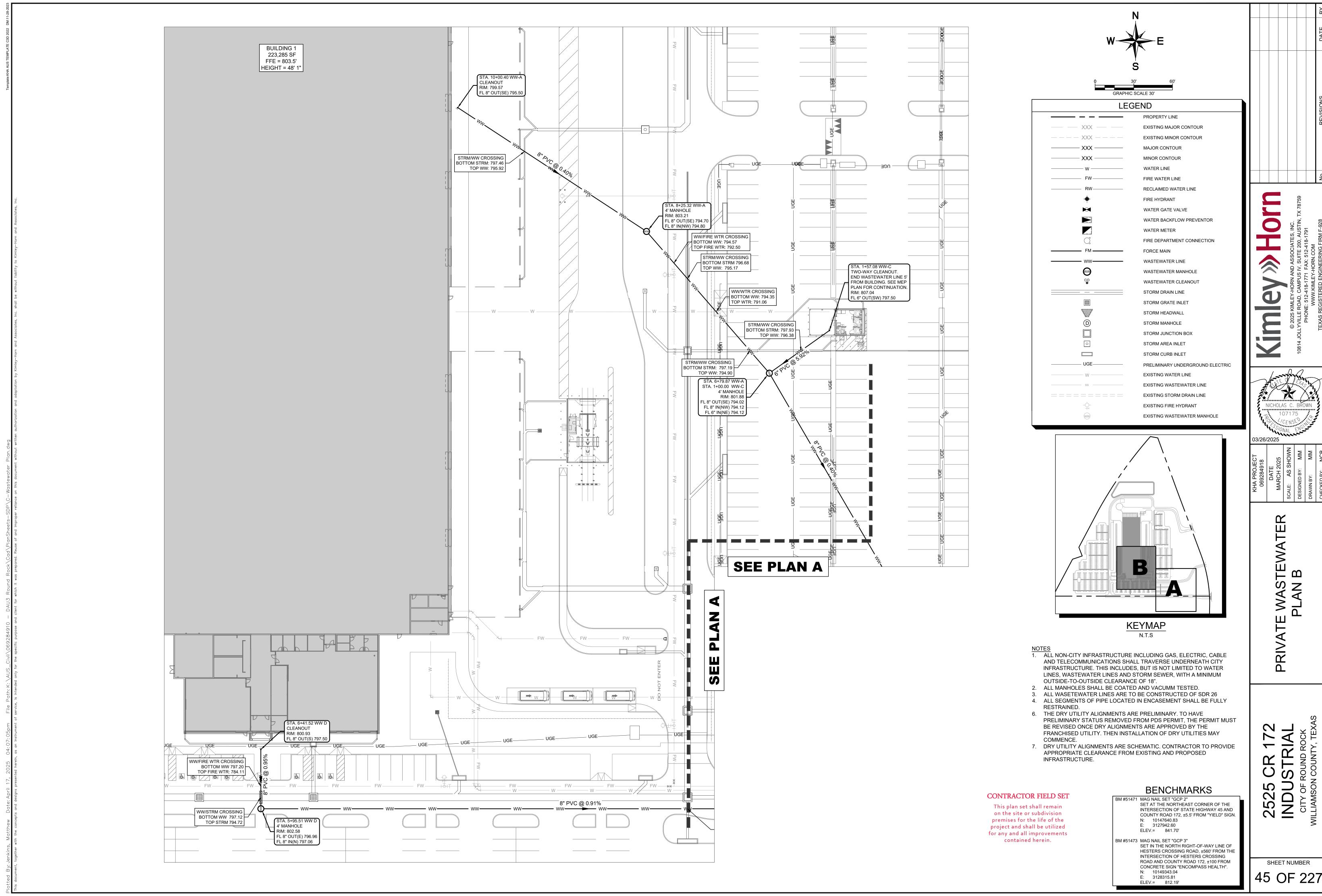












#### **GENERAL NOTES**

- 1. CONFIGURATIONS AND DIMENSIONS SHOWN ARE BASED ON THE EQUIPMENTS SPECIFIED. THE CONTRACTOR SHALL VERIFY THE LAYOUT AND ALL DIMENSIONS PRIOR TO FABRICATION.
- 2. ALL COUPLINGS SHALL BE EPOXY COATED STEEL AND SHALL BE DRESSER, SMITH-BLAIR, OR EQUAL. COUPLINGS SHALL BE RESTRAINED WITH A THRUST HARNESS DESIGNED IN ACCORDANCE WITH AWWA
- 3. ALL PIPING WITHIN WET WELL AND VALVE VAULT SHALL BE SPECIAL THICKNESS CLASS 53 EPOXY LINED (PROTECTO 401) DUCTILE IRON PIPE.
- 4. ALL BURIED PIPE SHALL BE POLYWRAPPED DUCTILE IRON PRESSURE CLASS 350 PUSH JOINT PIPE WITH JOINT RESTRAINT GASKETS.
- 5. ALL METALS WITHIN THE WET WELL, INCLUDING FLANGE BOLTS, SHALL BE STAINLESS STEEL, UNLESS OTHERWISE INDICATED.
- 6. THE EDGE OF EXPOSED CONCRETE SLABS SHALL RECEIVE A 3/4" CHAMFER.
- 7. REINFORCED STEEL SHALL BE GRADE 60.

DAMAGE INCURRED DURING CONSTRUCTION.

- 8. ALL VENTS SHALL HAVE SCREENS INSTALLED OVER OPENINGS AND SHALL BE 304 STAINLESS STEEL 16 MESH. OPENINGS SHALL BE A MINIMUM OF 12" ABOVE THE SLAB.
- 9. THE HIGH LEVEL ALARM SHALL ACTIVATE A FLASHING RED LIGHT TO BE MOUNTED ON THE ROOF OF THE ELECTRICAL SHELTER. THE CONTRACTOR SHALL INSTALL METAL INFORMATION SIGN AT OR NEAR THE LIFT STATION ENTRY GATE, VISIBLE TO THE PUBLIC.

COMPLETE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY REPAIRS TO EXISTING UTILITIES DUE TO

- 10. ALL PENETRATIONS SHALL BE SEALED WATER AND GAS TIGHT PER APPROVED METHODS.
- 11. FORCEMAIN TO CONTINUE AT THE SHOWN FLOWLINE UNTIL IT CONNECTS WITH THE FORCEMAIN SHOWN ON SHEET 44.
- 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL OF THE NECESSARY WASTEWATER CONNECTIONS TO THE SITE. CONTRACTOR SHALL NOTIFY THE UTILITY AUTHORITIES INSPECTORS 72 HOURS PRIOR TO CONNECTING TO ANY EXISTING LINE. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR
- 13. CONTRACTOR SHALL USE KENNEDY VALVE CO. KS-RW GATYE VALVES OR APPROVED EQUAL IN ACCORDANCE WITH AWWA C517.
- 14. CONTRACTOR SHALL USE VAL-MATIC SWING-FLEX CHECK VALVE OR APPROVED EQUAL IN ACCORDANCE WITH AWWA C508. CHECK VALVES SHOULD BE SUITABLE FOR DIRECT BURY.
- 15. A CHECK VALVE MUST BE A SWING TYPE VALVE WITH AN EXTERNAL LEVER OR EXTERNAL POSITION INDICATOR TO SHOW ITS OPEN AND CLOSED POSITIONS.
- 16. AN ISOLATION VALVE MUST INCLUDE AN EXTERNAL POSITION INDICATOR TO SHOW ITS OPEN AND CLOSED POSITIONS, UNLESS A FULL-CLOSING VALVE IS A RISING-STEM GATE VALVE.
- 17. CONTRACTOR SHALL USE ARI D-020 COMBINATION AIR VALVE OR APPROVED EQUAL.
- 18. CONFIGURATIONS AND DIMENSIONS SHOWN ARE BASED ON THE EQUIPMENT SPECIFIED. THE CONTRACTOR SHALL VERIFY THE LAYOUT AND ALL DIMENSIONS PRIOR TO CONSTRUCTION.
- 19. CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING THE STRUCTURAL DESIGN FOR BRACING THE J-VENT SYSTEM.
- 20. INSTALL ISOLATION KITS BETWEEN DISSIMILAR METAL PIPING.
- 21. CONTRACTOR SHALL FOLLOW THE INSTALLATION INSTRUCTIONS PROVIDED BY THE SUPPLIER/MANUFACTURER.
- 22. UPON RECEIPT OF THE LIFT STATION SHIPMENT, INSPECT THE ENTIRE SHIPMENT FOR DAMAGE BEFORE THE LIFT STATION IS TAKEN OFF THE TRUCK. IF THERE IS DAMAGE, NOTE AS SPECIFICALLY AS POSSIBLE INCLUDING CLEAR PHOTOS OF DAMAGE ON THE BILL OF LADING AS TO ANY DAMAGE, THEN OFFLOAD. CONTACT THE SHIPPER AT ONCE AND HAVE THE BILL OF LADING WITH YOU.
- 23. CHECK VALVE, COUPLING, GATE VALVE AND AIR RELEASE VALVES (BALL VALVES MAY BE USED DEPENDING ON TYPE OF PUMPS).
- 24. PUMPS TO BE SELECTED BASED ON APPLICATION, FLOW, AND HEAD CONDITIONS OF FORCE MAIN.
- 25. SELECT BACKFILL MATERIAL FREE OF VOIDS, SHARP OBJECTS, OR OTHER DEBRIS.

### PIPE AND FITTINGS

- 1.1. 4" HDPE DR9 FORCE MAIN WITH PRESSURE RATING OF 200 PSI SHALL BE USED.
- 2. LIFT STATION PIPING
- DIP PIPE SHALL BE IN ACCORDANCE WITH AWWA C111 AND AWWA C150 2.2. FITTINGS AND PIPE SHALL BE FLANGED.
- FITTINGS IN ACCORDANCE WITH AWWA C153 AND AWWA C110. FLANGES: AWWA/ANSI C115/A21.15, ASME B16.1, CLASS 125
- 2.2.3. BOLTS AND NUTS SHALL BE 316 STAINLESS.
- 2.3. PIPE AND FITTINGS SHALL BE LINED WITH PROTECTO 401. 2.4. PROVIDE POLYETHYLENE ENCASEMENT IN ACCORDANCE WITH AWWA FOR BURIED DIP.

### SHOP DRAWINGS, PRODUCT DATA, AND SAMPLES

- 1. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND PRODUCT DATA FOR LIFT STATION EQUIPMENT, PIPING AND APPURTENANCES. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE OWNER FOR REVIEW PRIOR TO INSTALLATION, ALLOWING SUFFICIENT TIME FOR THE OWNER'S REVIEW AND RESPONSE.
- 2. CONTRACTOR SHALL SUBMIT DIMENSIONAL LAYOUT DRAWINGS AND PRODUCT DATA, CERTIFIED CORRECT FOR CONSTRUCTION, FOR REVIEW BY THE OWNER.
- 3. THE CONTRACTOR WILL MAKE SPECIFIC MENTION OF THOSE ITEMS THAT VARY FROM THE REQUIREMENTS OF THE CONTRACT DOCUMENTS IN THE LETTER OF TRANSMITTAL.
- 4. THE CONTRACTOR WILL ASSIGN A SEQUENTIAL NUMBER TO EACH SUBMITTAL (1, 2, 3, ETC.). RE-SUBMITTALS WILL BE IDENTIFIED WITH THEIR ORIGINAL NUMBER FOLLOWED BY A SEQUENTIAL LETTER (A, B, C, ETC.). FOR EXAMPLE, SUBMITTAL 12-C IS THE THIRD RE-SUBMITTAL OF THE OF THE TWELFTH ITEM
- 5. THE CONTRACTOR WILL NOT DELIVER TO THE SITE, STORE, OR INCORPORATE INTO THE WORK, ANY MATERIALS OR EQUIPMENT FOR WHICH APPROVED SUBMITTALS HAVE NOT BEEN OBTAINED.
- 6. OWNER'S REVIEW, APPROVAL, OR OTHER APPROPRIATE ACTION REGARDING CONTRACTOR'S SUBMISSIONS WILL BE ONLY TO CHECK CONFORMITY WITH THE DESIGN CONCEPT OF THE PROJECT AND FOR COMPLIANCE WITH THE INFORMATION CONTAINED IN THE CONTRACT DOCUMENTS AND SHALL NOT EXTEND TO MEANS, METHODS, TECHNIQUES, DEQUENCES OR PROCEDURES OF CONSTRUCTION (EXCEPT WHERE A SPECIFIC MEANS, METHOD, TECHNIQUE, SEQUENCE OR PROCEDURE OF CONSTRUCTION IS INDICATED IN OR REQUIRED BY THE CONTRACT DOCUMENTS) OR TO SAFETY PRECAUTIONS OR PROGRAMS INCIDENT THERETO. THE REVIEW AND APPROVAL OF A SEPARATE COMPONENT ITEM WILL NOT INDICATE APPROVAL OF THE ASSEMBLY INTO WHICH THE ITEM IS FUNCTIONALLY INTEGRATED. CONTRACTOR SHALL MAKE CORRECTIONS REQUIRED BY OWNER. AND SHALL RETURN THE REQUIRED NUMBER OF CORRECTED COPIES OF SHOP DRAWINGS TO THE OWNER. CONTRACTOR MAY BE REQUIRED TO RESUBMIT AS REQUIRED REVISED SHOP DRAWINGS OR SAMPLES FOR FURTHER REVIEW AND APPROVAL. CONTRACTOR SHALL DIRECT SPECIFIC ATTENTION IN WRITING TO ANY NEW REVISIONS NOT SPECIFIED BY CONTRACTOR ON PREVIEW CONTRACTOR SUBMISSIONS.

### LIFT STATION AND SUBMERSIBLE SEWAGE PUMPS

- 1. CONTRACTOR SHALL FURNISH ALL LABOR, MATERIALS, ELECTRICAL EQUIPMENT AND INCIDENTALS REQUIRED TO PROVIDE A LIFT STATION AS PROVIDED BY MANSFIELD PUMP COMPANY. (JAMES MANSFIELD, 512-745-7647) THE CONTRACTOR'S RESPONSIBILITIES ARE AS FOLLOWS: RECEIVING THE LIFT STATION AND OFF LOADING, EXCAVATION AND SETTING, ALL PLUMBING CONNECTIONS, ALL ELECTRICAL CONDUITS FROM THE WET WELL TO CONTROL PANEL. PROVIDE PROPER BACKFILL AND COMPACTION PROCEDURES. THE CONTRACTOR WILL BE REQUIRED TO LOWER THE PUMPS INTO PLACE AND CHECK FOR PROPER ROTATION. A STARTUP PROCEDURE FORM WILL BE PROVIDED AND IT MUST BE FILLED OUT AND RETURNED TO THE LIFT STATION SUPPLIER PRIOR TO THE OWNER'S POSSESSION DATE.
- 2. THE PUMP SHALL HAVE A FLANGED DISCHARGE ADAPTABLE TO 3" VERTICAL DISCHARGE AND BE CAPABLE OF HANDLING SANITARY SEWAGE AND ENABLING IT TO BE PUMPED OVER LONG DISTANCES OR HIGH VERTICAL LIFTS IN PIPELINES AS SMALL AS 3.00" IN DIAMETER.
- 3. THE CENTRIFUGAL PUMP SHALL BE EQUAL TO THE SERIES EBG-33. A PUMP SUBMERSIBLE TYPE AS MANUFACTURED BY GRUNDFOS WITH 4" IMPELLER OR APPROVED EQUAL. THE PUMP SHALL BE EXPLOSION-PROOF. THE CASTINGS SHALL BE CONSTRUCTED OF CAST IRON.
- 4. SUBMERSIBLE SEWAGE PUMP MATERIALS:
- 4.1. PUMP CASE: CAST IRON, ASTM A48, CLASS 30 4.2. MOTOR HOUSING: CAST IRON, ASTM A48, CLASS 30
- 4.3. IMPELLER: CAST BRASS
- 4.4. INTERMEDIATE HOUSING (BACKPLATE): CAST IRON, ASTM A48, CLASS 35B 4.5. DISCHARGE BASE ELBOW: CAST IRON, ASTM A48, CLASS 35B
- 4.6. PUMP/MOTOR SHAFT: ENTIRE SHAFT IS TO BE ASTM A276 TYPE 420 STAINLESS STEEL
- O-RINGS: NITRILE RUBBER (NBR) 4.8. FASTENERS (INCLUDING IMPELLER FASTENER): 304 STAINLESS STEEL
- 4.9. LOWER SEAL FACES: SILICON CARBIDE/SILICON CARBIDE
- 4.10. UPPER SEAL FACES: SILICON CARBIDE STATIONARY/CARBON ROTATING 4.11. GUIDE RAILS/CABLES AND MOUNTING BRACKETS: STAINLESS STEEL, ASTM A276 TYPE 316 (CABLES SHALL BE NYLON COATED)
- 4.12. LIFTING CHAIN OR CABLE: STAINLESS STEEL, ASTM A276 TYPE 316
- 4.13. OIL-ALL USES (SEAL LUBRICATION, ETC): ECOLOGICALLY SAFE, PARIFIN OR MINERAL BASE 4.14. POWER/CONTROL CABLE JACKET: CHLOROPRENE WITH NON-WICKING FILLERS
- 5. THE MOTOR HOUSING SHALL BE FINNED AND AIR-FILLED TO DISSIPATE HEAT AND ENABLE THE UNIT TO OPERATE UNSUBMERGED WITHOUT DAMAGE TO THE MOTOR. ALL EXTERNAL-MATING PARTS SHALL BE MACHINED AND SEALED WITH A bUNA-n SQUARE RING. ALL FASTENERS EXPOSED TO THE LIQUID SHALL BE 300 SERIES STAINLESS STEEL. THE MOTOR SHALL BE PROTECTED ON THE TOP SIDE WITH AN ATTACHED SEALED JUNCTION BOX CHAMBER WHICH IN THE EVENT OF CORD DAMAGE WILL PREVENT MOISTURE WICKING INTO THE MOTOR HOUSING. THE MOTOR SHALL BE PROTECTED ON THE LOWER SIDE WITH A TANDEM MECHANICAL SEAL ARRANGEMENT WITH EACH SEAL HAVING A SEPARATE SPRING ASSEMBLY THE UPPER AND LOWER BALL BEARINGS SHALL BE CAPABLE OF HANDLING ALL THRUST LOADS. THE PUMP HOUSING HAVING A SEPARATE SPRING ASSEMBLY. THE UPPER AND LOWER BALL BEARINGS SHALL BE CAPABLE OF HANDLING ALL THRUST LOADS. THE PUMP HOUSING SHALL BE OF THE CONCENTRIC DESIGN THEREBY EQUALIZING THE PRESSURE FORCES INSIDE THE HOUSING, WHICH WILL EXTEND THE SERVICE LIFE OF THE SEALS AND BEARINGS. THE TOP CAP SHALL HAVE CAST IN LIFTING LUGS. THE MOTOR SHALL BE PROTECTED BY A MOISTURE DETECTION SYSTEM, WHICH WILL ACTIVATE AN ALARM CIRCUIT IF LIQUID IS EVER PRESENT IN THE UPPER JUNCTION CHAMBER OR LOWER SEAL CAVITY. THERMAL SENSORS LOCATED IN THE OIL-FILLED MOTOR HOUSING SHALL PROVIDE TEMPERATURE PROTECTION. PROVIDE THE AUTOMATIC REVERSING FEATURE, AVAILABLE FOR 3 PHASE MODELS, THAT ENABLE THE PUMP TO ROTATE IN BOTH CW AND CCW DIRECTION.
- 6. THE PUMP SHALL BE SUPPLIED WITH 49.2' MULTI-CONDUCTOR POWER CORD. IT SHALL BE SO TYPE CORD CAPABLE OF CONTINUED EXPOSURE TO THE PUMPED LIQUID. POWER CORD SHALL BE SIZED FOR THE RATED FULL LOAD AMP RATING OF THE PUMP IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE.
- 7. THE MOTOR SHALL BE FM LISTED FOR CLASS 1 DIVISION 1 GROUPS C&D, HAVE A CLASS F OR BETTER RATED DESIGN RATED FOR CONTINUOUS DUTY. THERMAL SENSORS LOCATED IN THE MOTOR HOUSING SHALL PROVIDE TEMPERATURE PROTECTION.
- 8. FURNISH UPPER AND LOWER BALL BEARINGS TO PROVIDE B10 LIFE OF, A MINIMUM 100,000 HOURS AT ALL ANTICIPATED AXIAL AND RADIAL LOADINGS.
- 9. PUMP SHALL HAVE A DUAL MECHANICAL SEAL CONFIGURATION WITH THE SEALS MOUNTED IN TANDEM. EACH SEAL ASSEMBLY WITH ITS OWN SPRING. PROVIDE SEALS THAT DO NOT REQUIRE ROUTINE MAINTENANCE OR ADJUSTMENT, BUT CAPABLE OF BEING REPLACED.
- 10. THE IMPELLER SHALL BE A NON-CLOG TYPE IMPELLER, CAPABLE OF PASSING 3" SPHERICAL SOLID. STATICALLY AND DYNAMICALLY BALANCED IMPELLERS ARE REQUIRED.
- 11. COMPONENTS REQUIRED FOR THE REPAIR OF THE PUMP SHALL BE READILY AVAILABLE WITHIN 24 HOURS. COMPONENTS SUCH AS MECHANICAL SEALS AND BEARINGS SHALL NOT BE OF A PROPRIETARY DESIGN AND BE AVAILABLE FROM LOCAL INDUSTRIAL SUPPLY HOUSES. SPECIAL TOOLS SHALL NOT BE REQUIRED TO SERVICE THE PUMP. A NETWORK OF SERVICE STATIONS SHALL BE AVAILABLE NATIONWIDE IN THOSE CASES WHERE SERVICE REQUIREMENTS ARE BEYOND THE COPE OF IN-HOUSE SERVICE MECHANICS.
- 12. THE TWO YEAR PRO-RATED WARRANTY FOR PERMANENT WASTEWATER LIFT STATIONS SHALL BE IN WRITING IN A PUBLISHED SERVICE BULLETIN. LABOR CHARGES FROM AN AUTHORIZED SERVICE STATION FOR REPAIRS WILL BE INCLUDED IN THIS WARRANTY AGREEMENT DURING THE FIRST 12 MONTHS OF OPERATION.





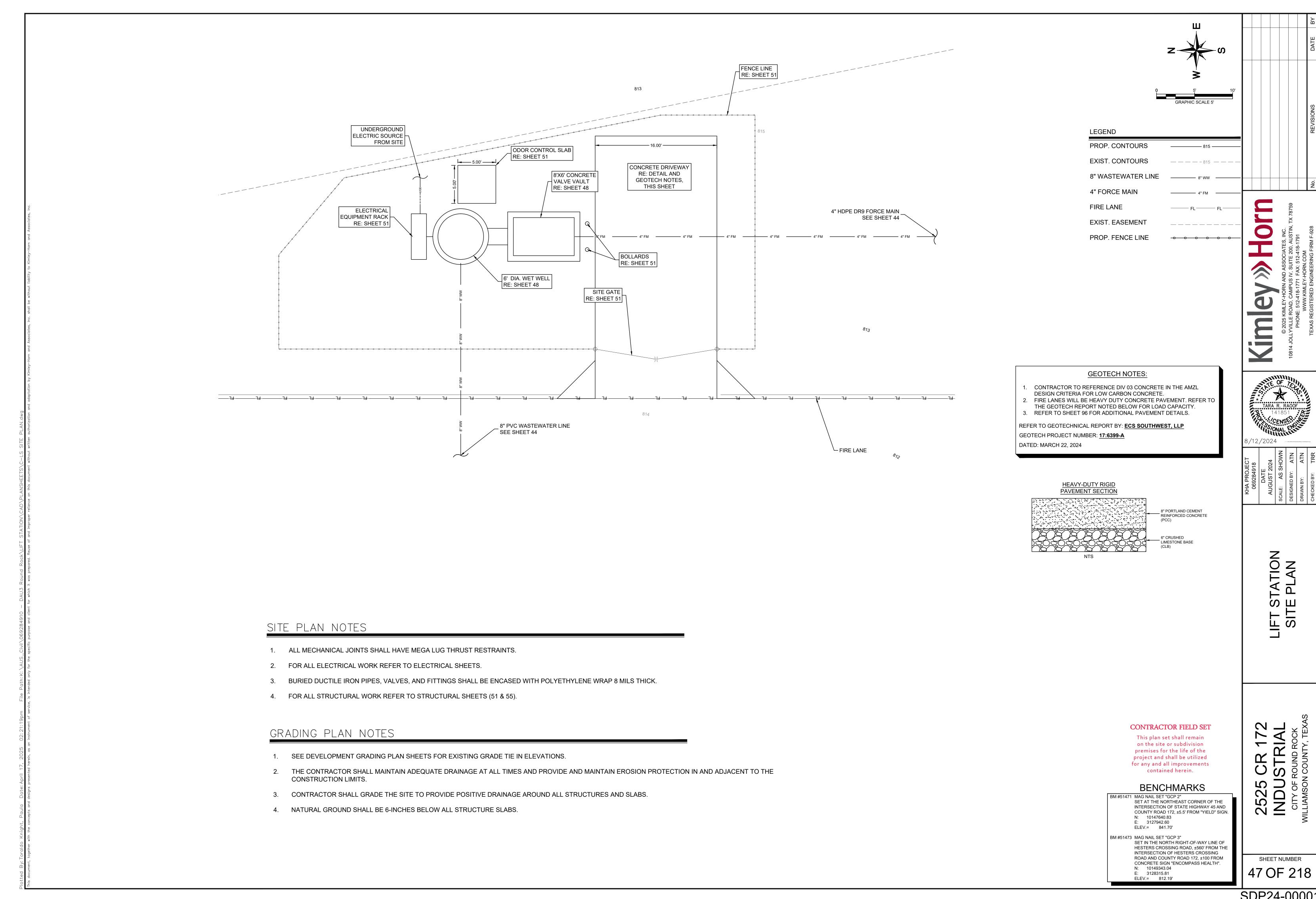
### CONTRACTOR FIELD SET

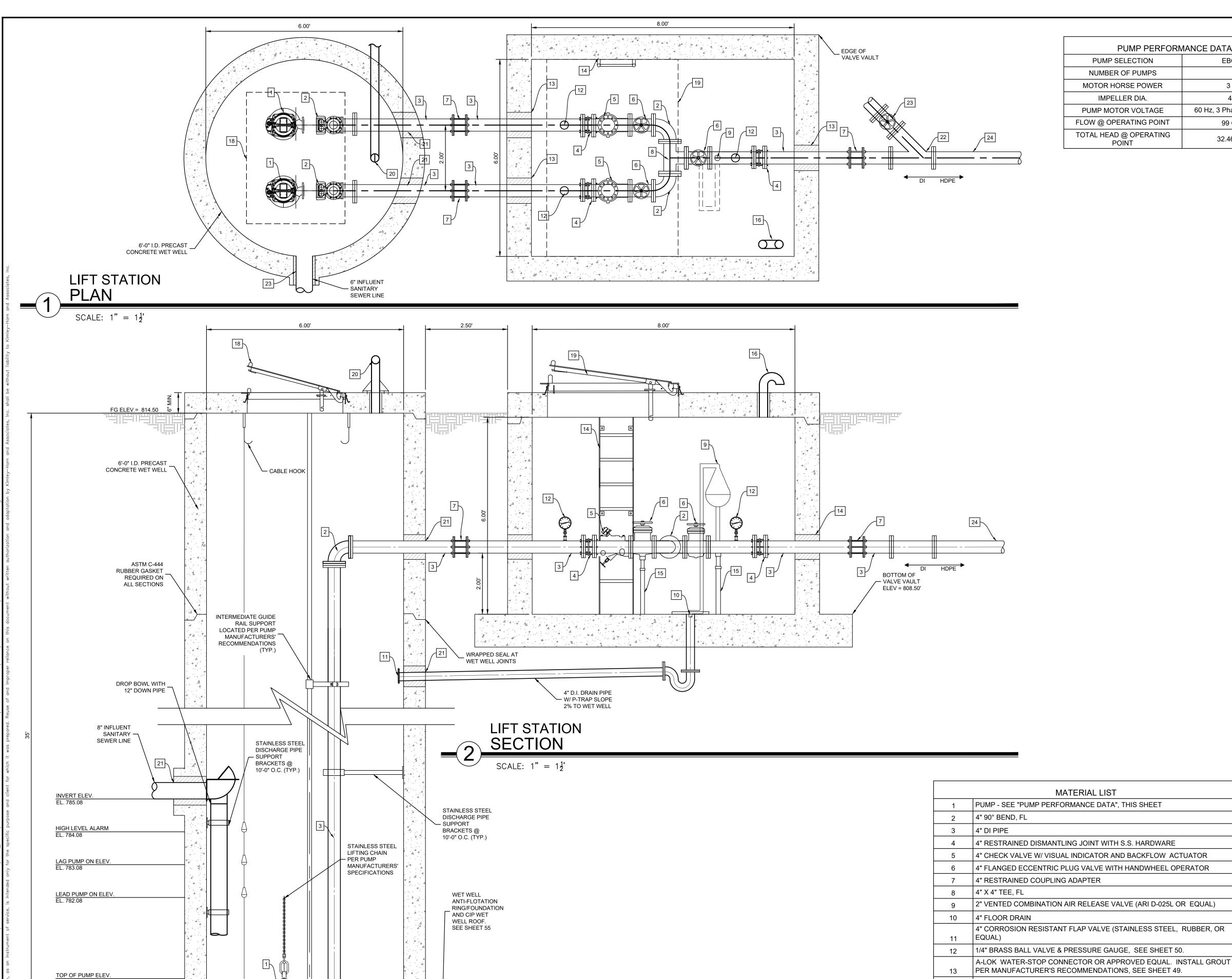
on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

This plan set shall remain

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SHEET NUMBER





ADJUSTED WET WELL ELEV.

PUMP PERFORN	MANCE DATA
PUMP SELECTION	EBG-33
NUMBER OF PUMPS	2
MOTOR HORSE POWER	3 HP
IMPELLER DIA.	4 IN
PUMP MOTOR VOLTAGE	60 Hz, 3 Phase, 460 Volts
FLOW @ OPERATING POINT	99 GPM
TOTAL HEAD @ OPERATING	32.46' TDH

POINT

ALUMINUM LADDER WITH RETRACTABLE HAND RAIL.

4" STAINLESS STEEL GOOSE NECK VENT WITH 304 STAINLESS STEEL

3'X4' WET WELL ALUMINUM ACCESS COVER WITH THREE DOORS

4'X6' VALVE VAULT ALUMINUM ACCESS COVER WITH THREE DOORS

INCLUDING SAFE HATCH, H-20 LOAD RATING (FLYGT OR EJIW)

INCLUDING SAFE HATCH, H-20 LOAD RATING (FLYGT OR EJIW)

PIPE SUPPORT, SEE SHEET 49.

ODOR CONTROL AERATOR, SEE SHEET 51

WET WELL PENETRATION, SEE SHEET 49

4" ECCENTRIC PLUG VALVE, MJ

SCREENS

| 4" X 4" WYE, MJ

24 4" HDPE DR9 PIPE

LIFT STATION INFORMATION	
SERVICE AREA TOTAL (ACRES)	69
SERVICE AREA SIZE (LUEs)	
PEAK DRY WEATHER FLOW (GPM)	
PEAK WET WEATHER FLOW (DESIGN	
FLOW) (GPM)	84.22
WET WELL DIAMETER (FT)	
ACTIVE VOLUME (GAL)	516
MINIMUM PUMP CYCLE TIME (MIN)	46
FORCE MAIN SIZE (IN)	
FORCE MAIN LENGTH (LF)	
FLOW VELOCITY (FPS)	

### KEYNOTES:

- 1. PUMP SUPPLIER SHALL PROVIDE DIMENSIONS OF THE GUIDE RAILS TO ENSURE COMPATIBILITY WITH SUPPLIED EQUIPMENT. THE PUMP SHALL BE EASILY REMOVED FOR INSPECTION OR SERVICE. PERSONNEL SHALL HAVE NO REASON TO ENTER THE WET WELL. GUIDE RAILS SHALL BE SUPPORTED EVERY 10 FEET WITH STAINLESS STEEL SUPPORTS.
- THE GUIDE BRACKETS SHALL BE CONSTURCTED OF 316 STAINLESS STEEL. GUIDE BRACKETS FOR EACH PUMP MUST BE SUPPLIED BY THE PUMP MANUFACTURER TO ENSURE COMPATIBILITY WITH SUPPLIED EQUIPMENT.
- 3. EACH PUMPING UNIT SHALL BE PROVIDED WITH A STAINLESS STEEL LIFTING CHAIN OR CABLE. LIFTING CHAIN SHALL EXTEND AT LEAST 3-4 FEET ABOVE WET WELL.
- 4. ALL HARDWARE IN THE WET WELL SHALL BE 316 STAINLESS STEEL.
- 5. PUMP DISCHARGE LINES SHALL HAVE 1/4 INCH TAPS WITH STAINLESS STEEL OR BRONZE BALL VALVES.
- 6. ALL DISCHARGE LINES SHALL HAVE ADEQUATE THRUST SUPPORT MEMBERS AT EACH FITTING. WHERE POSSIBLE, LONG RADIUS 90 DEGREE BENDS SHALL BE
- 7. THE DISCHARGE LINE FROM EACH PUMP SHALL BE FITTED WITH A CHECK VALVE AND A ECCENTRIC PLUG VALVE, WITH THE CHECK VALVE ON THE PUMP SIDE OF THE ECCENTRIC PLUG VALVE. WHEN NECESSARY, AIR RELEASE VALVE(S) SHALL BE INSTALLED DOWNSTREAM OF THE ECCENTRIC PLUG VALVES.
- 8. THE VALVE VAULT SHALL BE SIZED LARGE ENOUGH TO PROVIDE AT LEAST 1 FOOT OF CLEARANCE AROUND ALL VALVES AND ALL FLANGES. THE LID OF THE VAULT SHALL BE A MINIMUM OF 42 INCHES BY 42 INCHES AND SHALL BE ABLE TO BE PADLOCKED. THE VAULT SHALL BE INTERNALLY COATED WITH SPL WW-511 COATING, AND EXTERNALLY COATED WITH A COAL TAR EPOXY.
- 9. THE VALVE VAULT SHALL HAVE A DRAIN TO THE WET WELL. THE DRAIN SHALL HAVE A 4 INCH MINIMUM DIAMETER AND BE FITTED WITH A FLAP VALVE OR BACK-FLOW PREVENTER AND A TRAP TO PREVENT GASES OR WATER FROM ENTERING THE VALVE VAULT. THE OPENING TO THE DRAIN SHALL BE COVERED WITH A STAINLESS STEEL SCREEN.
- 10. ALL VENTS SHALL BE COATED INSIDE AND OUT PER CITY OF ROUND ROCK STANDARD SPECIFICATIONS. THE WET WELL VENT SHALL BE A TEE FITTED WITH STAINLESS STEEL SCREENS 10 FEET ABOVE WET WELL COVER.

### CONTRACTOR FIELD SET

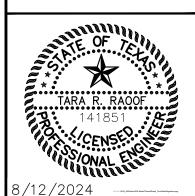
This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

### **GENERAL NOTES**

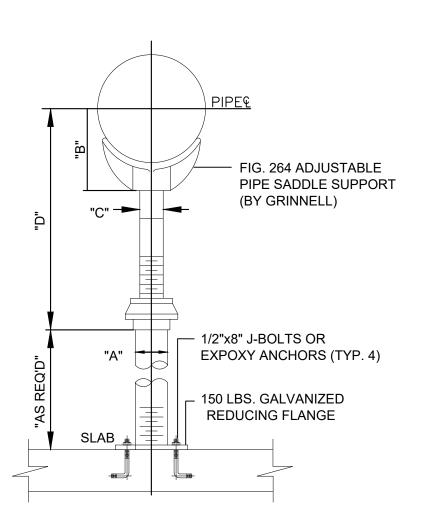
- 1. THE WORK PERFORMED UNDER THIS CONTRACT SHALL COMPLY WITH THE CITY OF ROUND ROCK SUBMERSIBLE LIFT STATION SPECIFICATIONS.
- 2. CONFIGURATIONS AND DIMENSIONS SHOWN ARE BASED ON THE EQUIPMENTS SPECIFIED. THE CONTRACTOR SHALL VERIFY THE LAYOUT AND ALL DIMENSIONS PRIOR TO FABRICATION.
- 3. ALL COUPLINGS SHALL BE EPOXY COATED STEEL AND SHALL BE DRESSER, SMITH-BLAIR, OR EQUAL. COUPLINGS SHALL BE RESTRAINED WITH A THRUST HARNESS DESIGNED IN ACCORDANCE WITH AWWA M-11.
- 4. CONTRACTOR SHALL COORDINATE VALVE VAULT ACCESS HATCH LOCATION WITH LADDER MANUFACTURER PRIOR TO FABRICATION.
- 5. CONTRACTOR SHALL VERIFY WET WELL ACCESS HATCH LOCATION AND DIMENSION WITH PUMP MANUFACTURER PRIOR TO FABRICATION.
- 6. PROVIDE 316 STAINLESS STEEL ANCHOR BOLTS FOR PUMP BASE MOUNTING TO SLAB.
- 7. INSTALL ISOLATION KITS BETWEEN DISSIMILAR METAL PIPING.
- 8. ALL PIPING WITHIN WET WELL AND VALVE VAULT SHALL BE SPECIAL THICKNESS CLASS 53 EPOXY LINED (PROTECTO 401) DUCTILE IRON PIPE.
- JOINT PIPE WITH JOINT RESTRAINT GASKETS.

9. ALL BURIED PIPE SHALL BE POLYWRAPPED DUCTILE IRON PRESSURE CLASS 350 PUSH

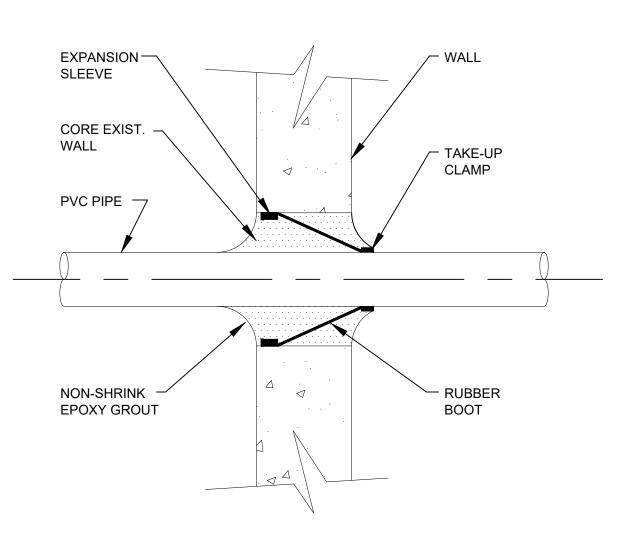
- 10. ALL METALS WITHIN THE WET WELL, INCLUDING FLANGE BOLTS, SHALL BE STAINLESS STEEL, UNLESS OTHERWISE INDICATED.
- 11. CONCRETE FOR FOUNDATION SHALL BE CLASS S. CONCRETE SHALL HAVE A MIN
- COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS.
- 12. THE EDGE OF EXPOSED CONCRETE SLABS SHALL RECEIVE A 3/4" CHAMFER.
- 13. REINFORCED STEEL SHALL BE GRADE 60.
- 14. ALL VENTS SHALL HAVE SCREENS INSTALLED OVER OPENINGS AND SHALL BE 304 STAINLESS STEEL - 16 MESH. OPENINGS SHALL BE A MINIMUM OF 12" ABOVE THE SLAB
- 15. THE HIGH LEVEL ALARM SHALL ACTIVATE A FLASHING RED LIGHT TO BE MOUNTED ON THE ROOF OF THE ELECTRICAL SHELTER. THE CONTRACTOR SHALL INSTALL METAL INFORMATION SIGN AT OR NEAR THE LIFT STATION ENTRY GATE, VISIBLE TO THE
- 16. ALL PENETRATIONS SHALL BE SEALED WATER AND GAS TIGHT PER APPROVED
- 17. BACK FILLING OF THE WET WELL SHALL COMPLY WITH CITY OF ROUND ROCK STANDARD SPECIFICATIONS.
- 18. ALL INTERIOR SURFACES OF CONCRETE WET WELL SHALL BE COATED WITH AN 80 MIL (MIN.) COAT OF RAVEN 400 SERIES HIGH BUILD EPOXY LINER OR APPROVED EQUAL.



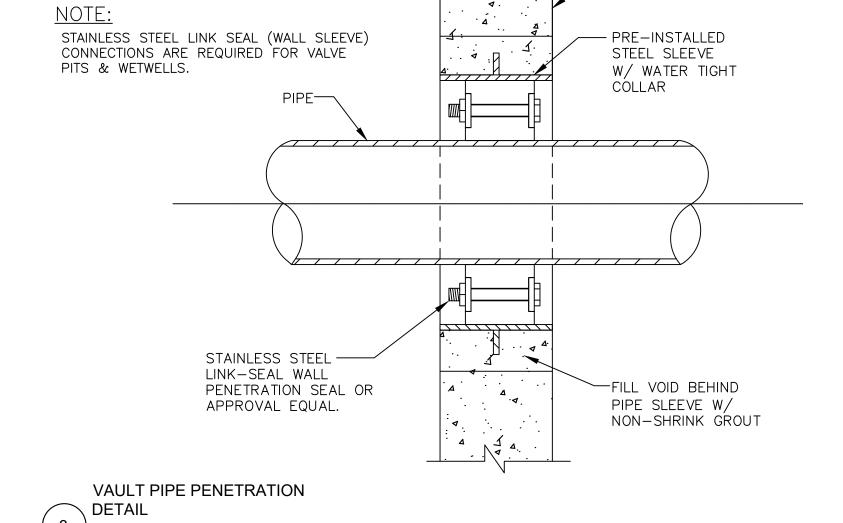
SHEET NUMBER 48 OF 218



WG	T APPROX. LBS EAC					ı	D
PIPE SIZE	COMPLETE	SADDLE ONLY	Α	А	А	MINIMUM	MAXIMUM
2 1/2	9.0	4.8	2 1/2	3 1/2	1 1/2	8	13
3	9.2	5.0	2 1/2	3 3/4	1 1/2	8 1/4	13 1/4
3 1/2	9.4	5.2	2 1/2	4	1 1/2	8 1/2	13 1/2
4	15.0	7.6	3	4 1/4	2 1/2	9 1/4	14
5	16.7	8.3	3	4 7/8	2 1/2	10	14 3/4
6	17.7	10.3	3	5 1/2	2 1/2	10 1/2	15 1/4
8	20.2	12.8	3	6 7/8	2 1/2	11 3/4	16 1/2
10	25.2	17.8	3	5 1/2	2 1/2	13 1/2	18 1/4
12	29.0	21.6	3	9 15/16	2 1/2	15	19 3/4
14	49.2	38.0	4	10 15/16	3	16 1/4	20 3/4
16	53.2	42.0	4	12 3/8	3	17 3/4	22 1/4
18	70.8	51.0	6	13 7/8	3 1/2	19 1/2	24
20	104.8	85.0	6	15 3/8	3 1/2	21	25 1/2
24	137.0	110.0	6	17 15/16	4	23 3/4	28 1/2
30	170.0	150.0	6	21 5/16	4	27	31 1/2
32	181.0	161.1	6	22 1/2	4	28 1/8	32 3/4
36	249.0	229.0	6	24 1/4	4	30 1/4	34 3/4

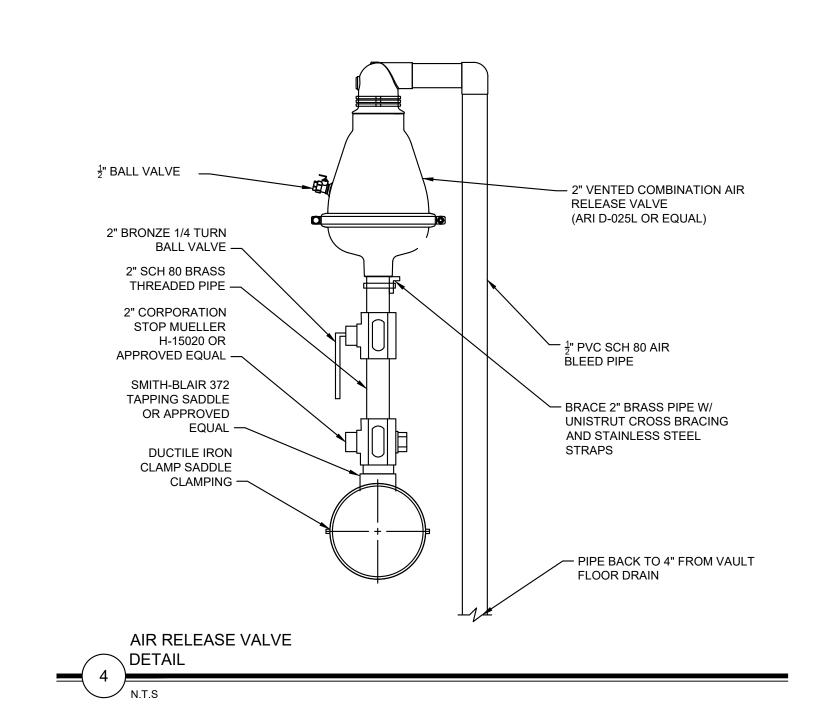


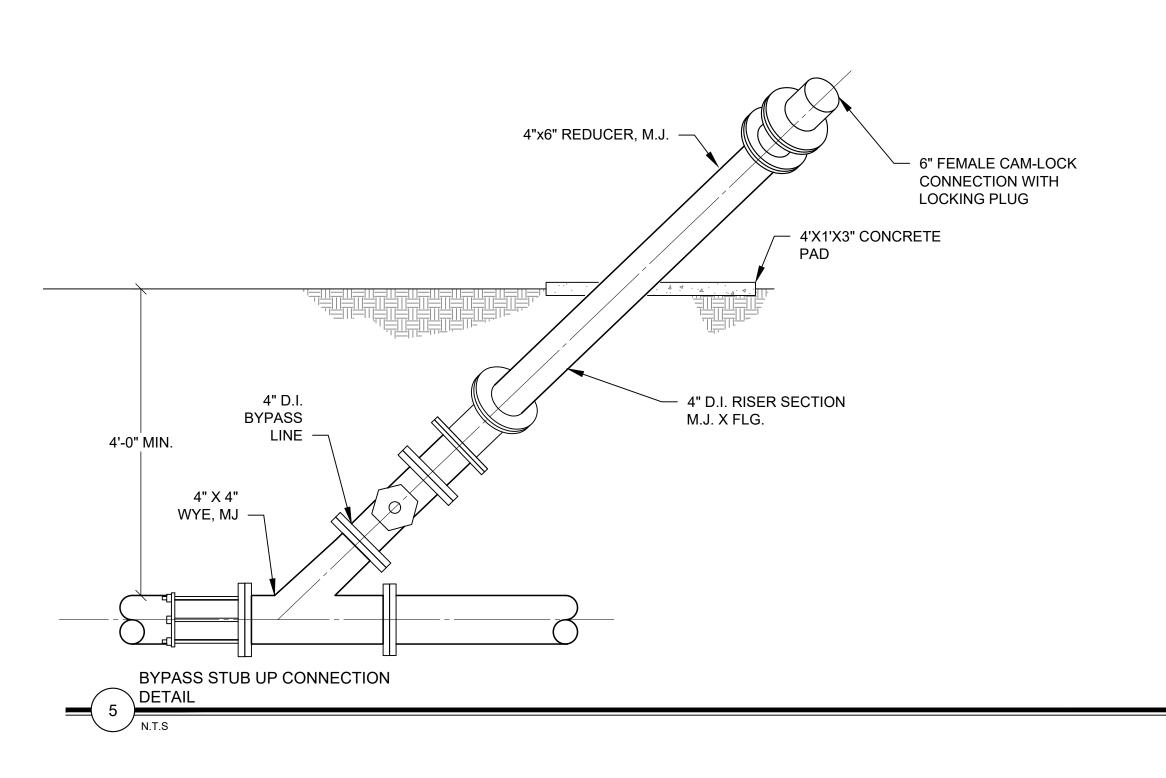
COA WW-146-D WET WELL PENETRATION



ADJUSTABLE PIPE SUPPORT DETAIL

1 N.T.S



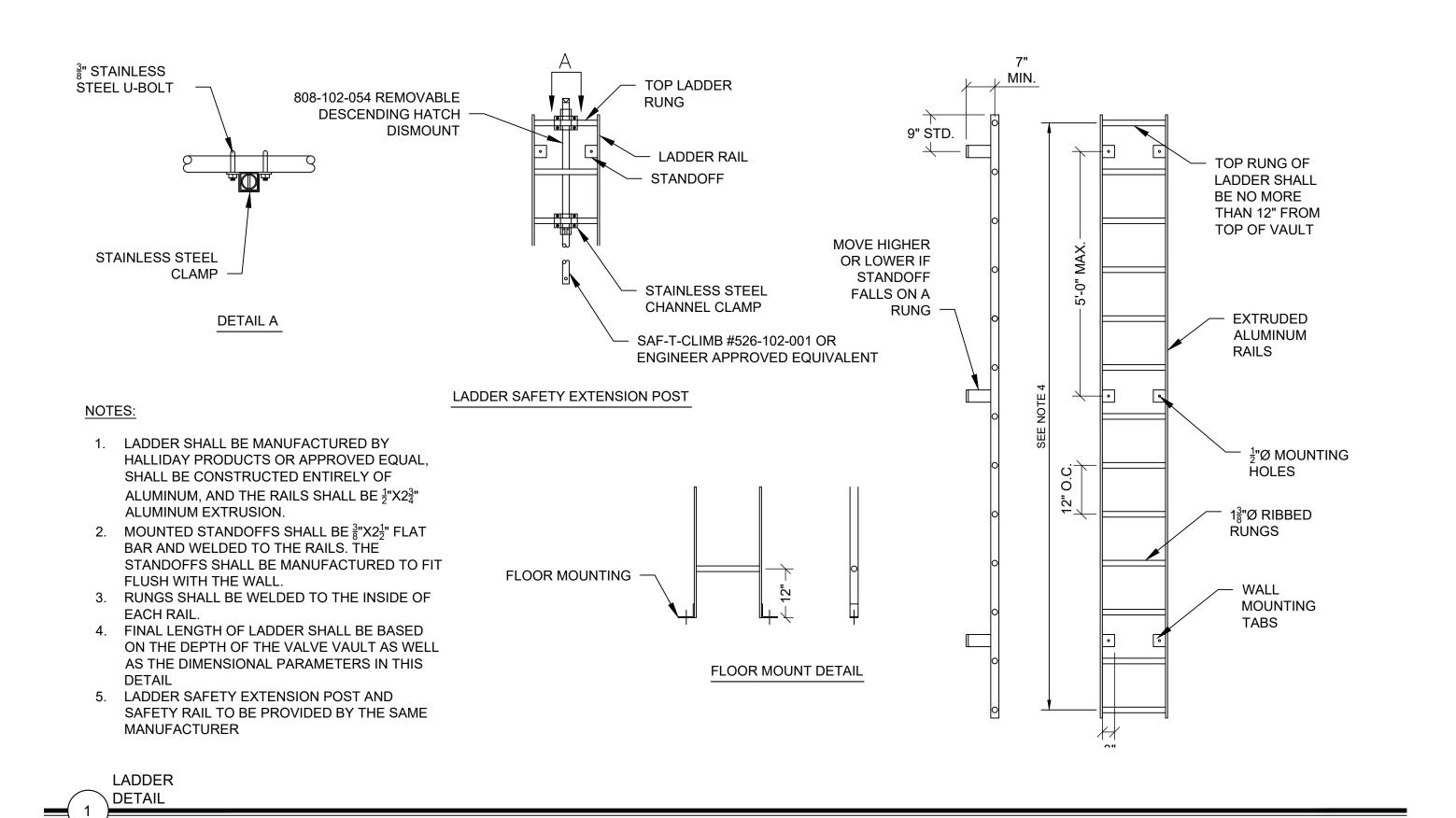


CONTRACTOR FIELD SET

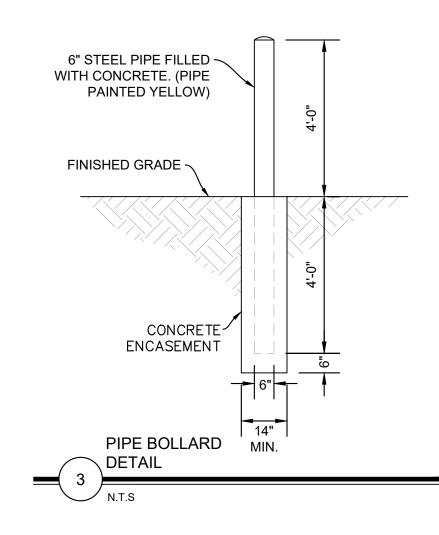
This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

SHEET NUMBER 49 OF 218

2525 CR INDUSTI



1. PRESSURE GAUGES SHALL HAVE 4-INCH DIAMETER FACES AND BE CALIBRATED FROM 0 TO 150 PSI. PRESSURE GAUGE~ 2. GAUGE SHALL BE FILLED WITH GLYCOL AND INSTALLED WITH A BRASS PRESSURE PRESSURE SNUBBER AND SHALL SNUBBER BE OF STAINLESS STEEL S.S. PROTECTIVE CONSTRUCTION. DIAPHRAGM 3. INSTALL GAUGE WITH A 0.25-INCH NPT CONNECTION AND %" THREADED INCLUDE A CORPORATION STOP BRONZE VALVE AND AIR BLEED. 4. VALVES TO BE RATED AT 200 1"X½" 300 LB. BRONZĒ BUSHING 5. ALL WELDED COUPLINGS SHALL 3000 LB. COUPLING / WELDED TO PIPE BE RATED FOR 3000 LBS. 6. GAUAGE SHALL BE TRERICE MANUFACTURED OR APPROVED PRESSURE GAUGE



2525 INDU

CONTRACTOR FIELD SET

This plan set shall remain

on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements

contained herein.

SHEET NUMBER

EBG-31 EBG-33 (3HP) Synchronous Speed: 3450 RPM 2<sup>1</sup>/<sub>2</sub> / 3 inch Discharge

Head / [Ft] 4.25" DIA 120 80 200 40 Capacity [USGPM]

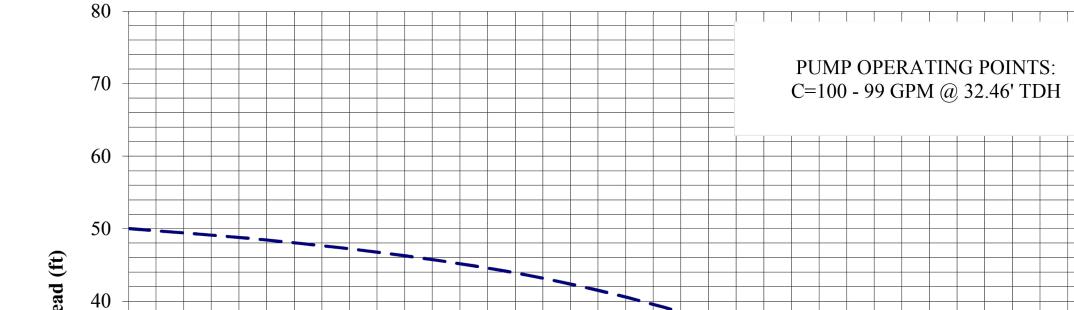
Note: Full diameter impeller included in price of pump. Consult factory for reduced diameter impeller and pricing.



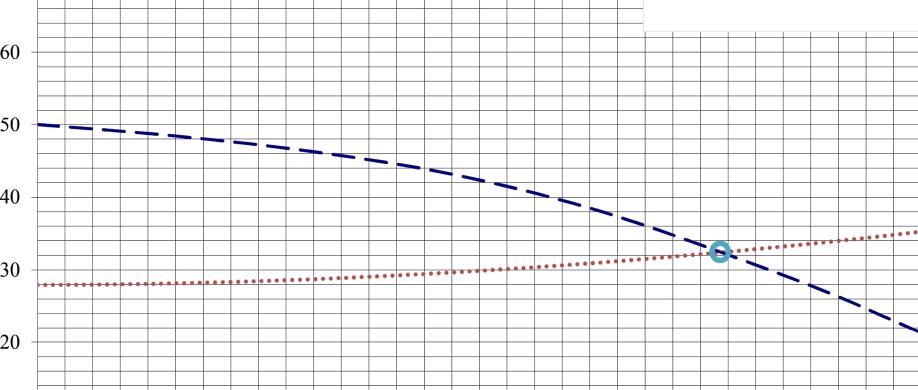
1-197

rev. 05/15

PUMP INFORMATION



**System and Pump Curves** 

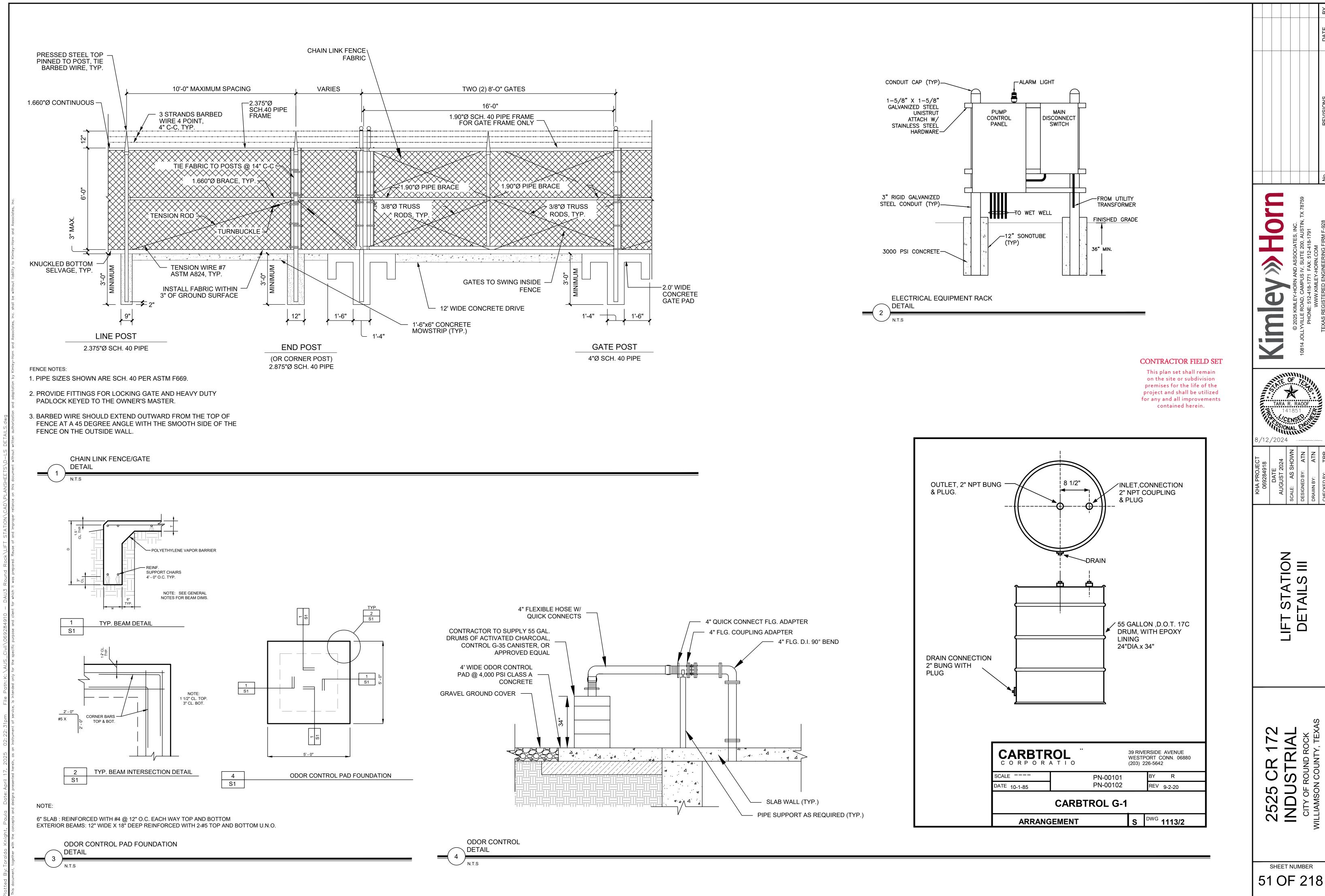


Flow (gpm)

•••• C=100

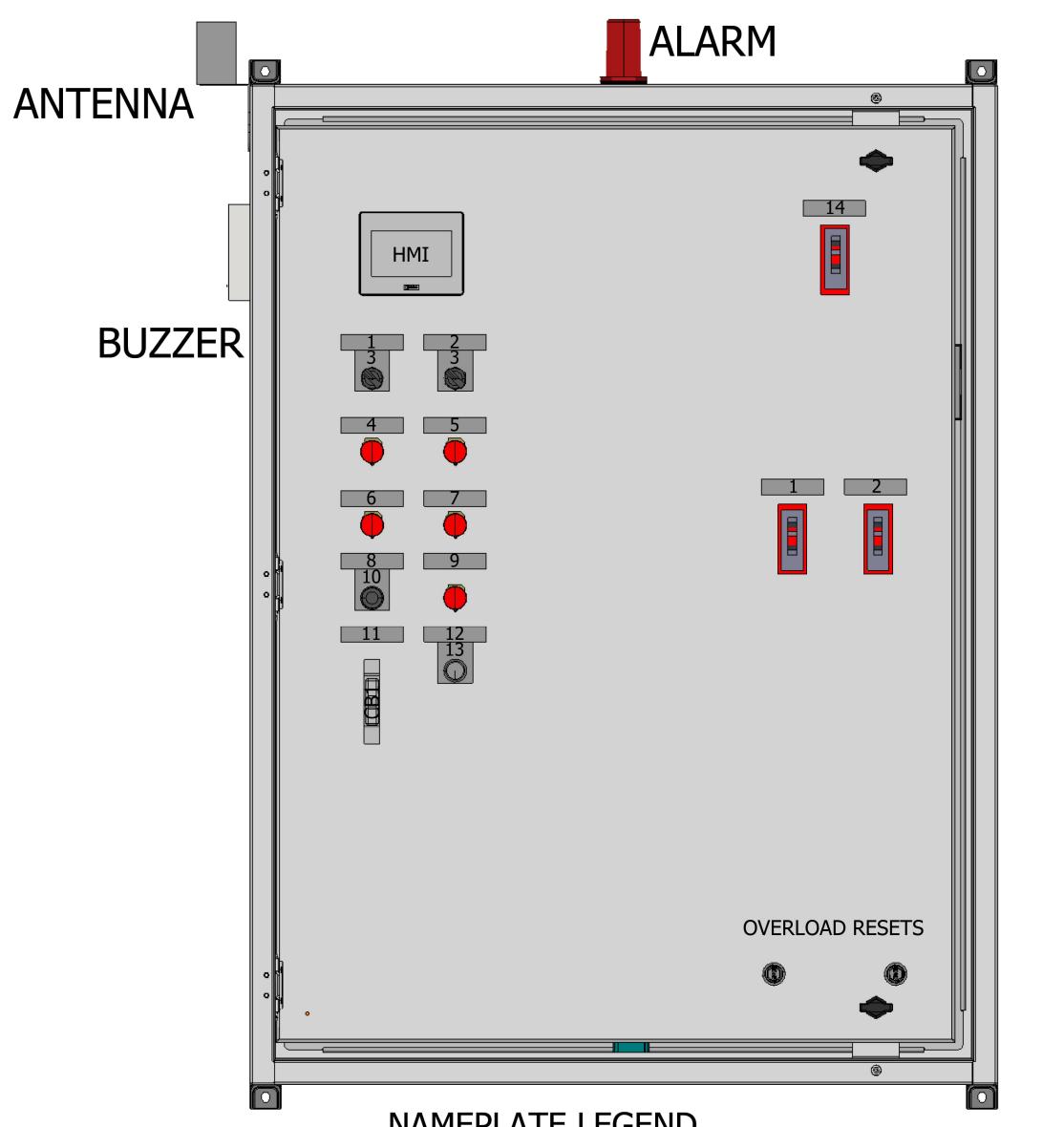
— Pump Model Information

50 OF 218



# NEMA 4X STAINLESS STEEL ENCLOSURE 48"H x 36"W x 12"DP OUTER DOOR NOT SHOWN

# **BACK PANEL**



**FRONT** 

NAMEPLATE LEGEND 1. PUMP 1

2. PUMP 2

11. CONTROL

3. HAND OFF AUTO

12. BACKUP

13. RESET 14. MAIN

4. PUMP 1 OVERTEMP

5. PUMP 2 OVERTEMP

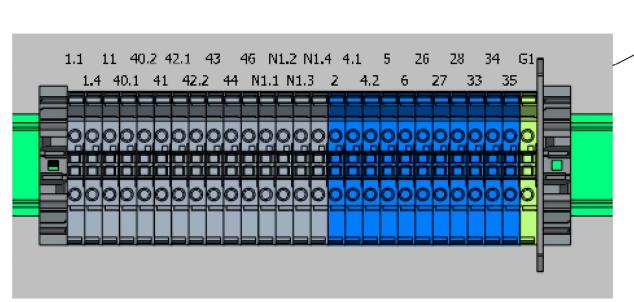
6. PUMP 1 SEAL FAIL

7. PUMP 2 SEAL FAIL

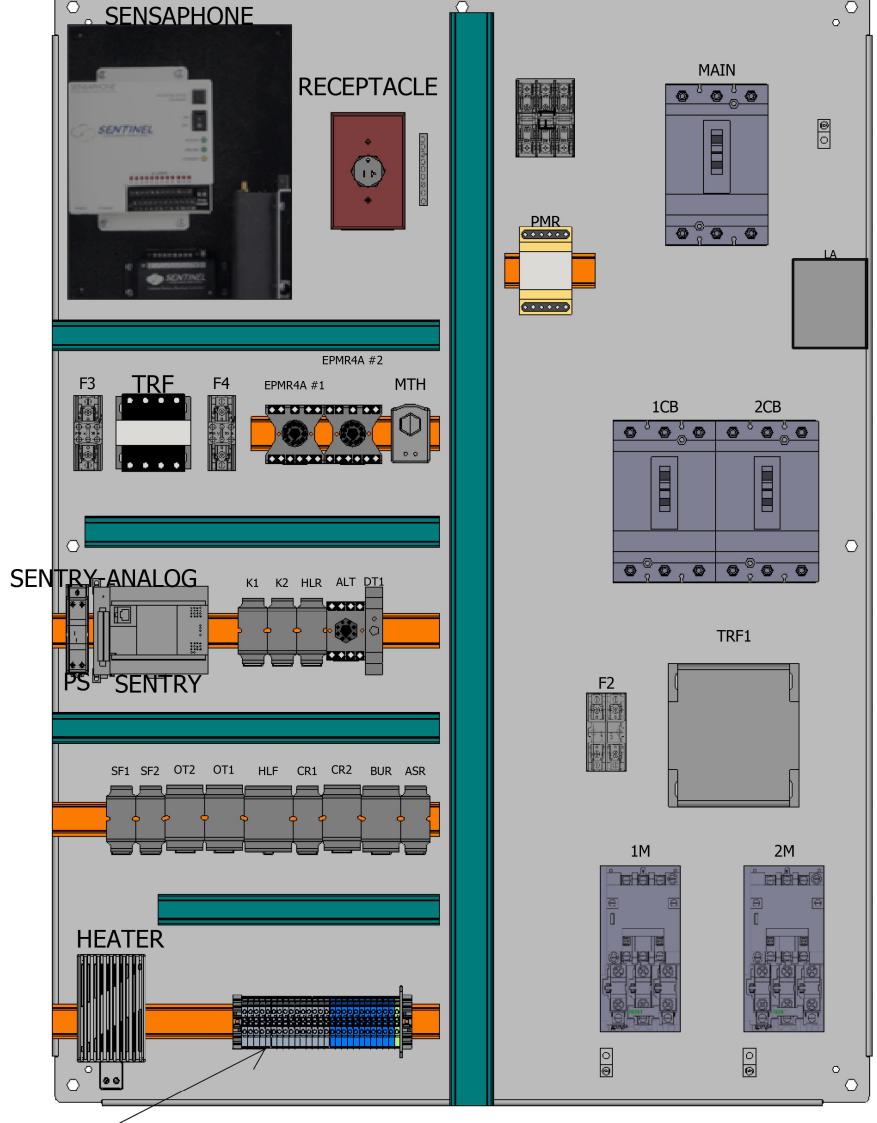
8. ALARM

9. BACKUP ON

10. SILENCE



LAMINATED DRAWING REQUIRED



CONTRACTOR FIELD SET This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

**USEMCO INCORPORATED** 1602 REZIN RD, TOMAH, WI 54660 **SUSEMCO**® PHONE: (608) 372-5911 FAX: (608) 372-5016 WWW.USEMCO.COM

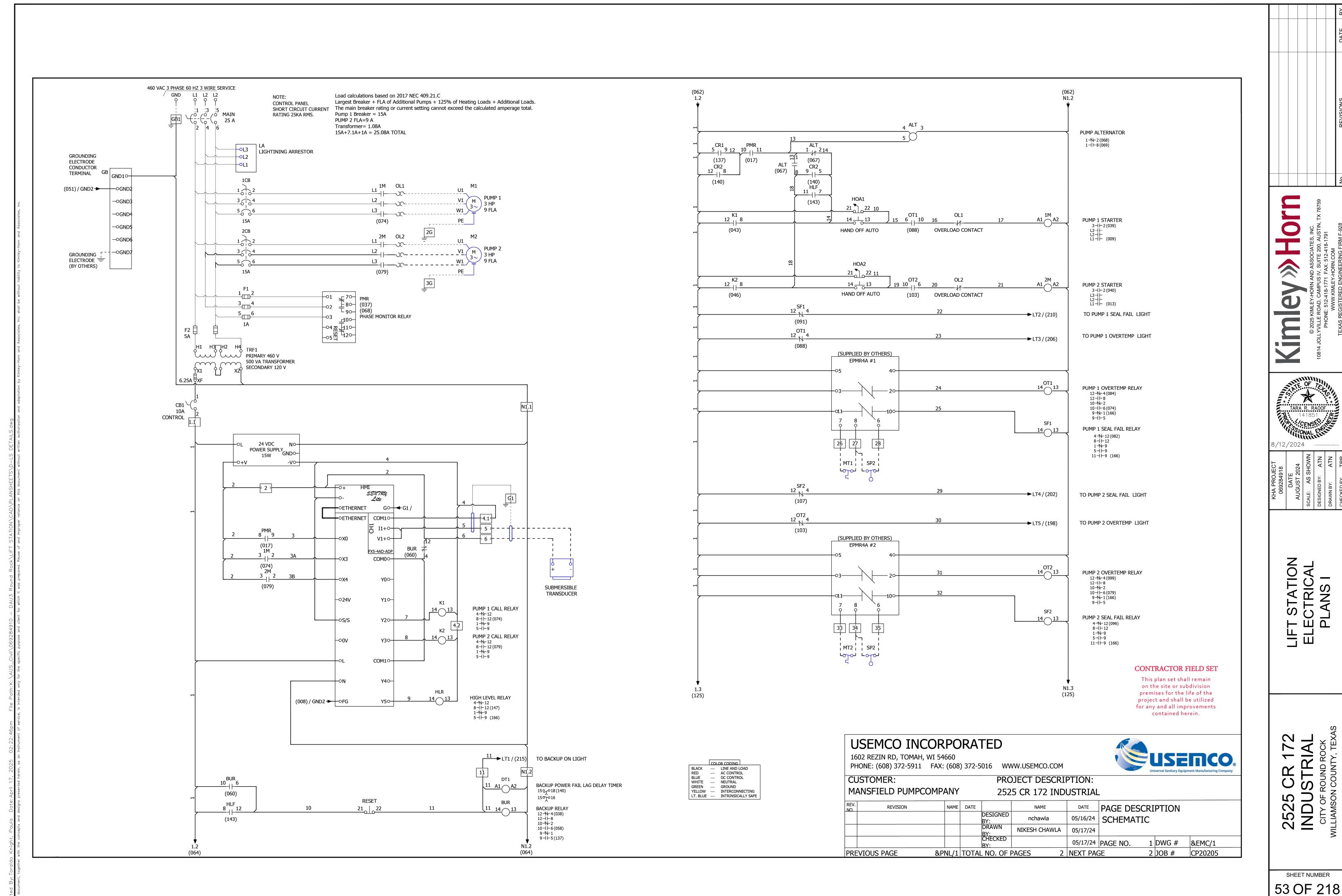
CUSTOMER: PROJECT DESCRIPTION: 2525 CR 172 INDUSTRIAL MANSFIELD PUMPCOMPANY

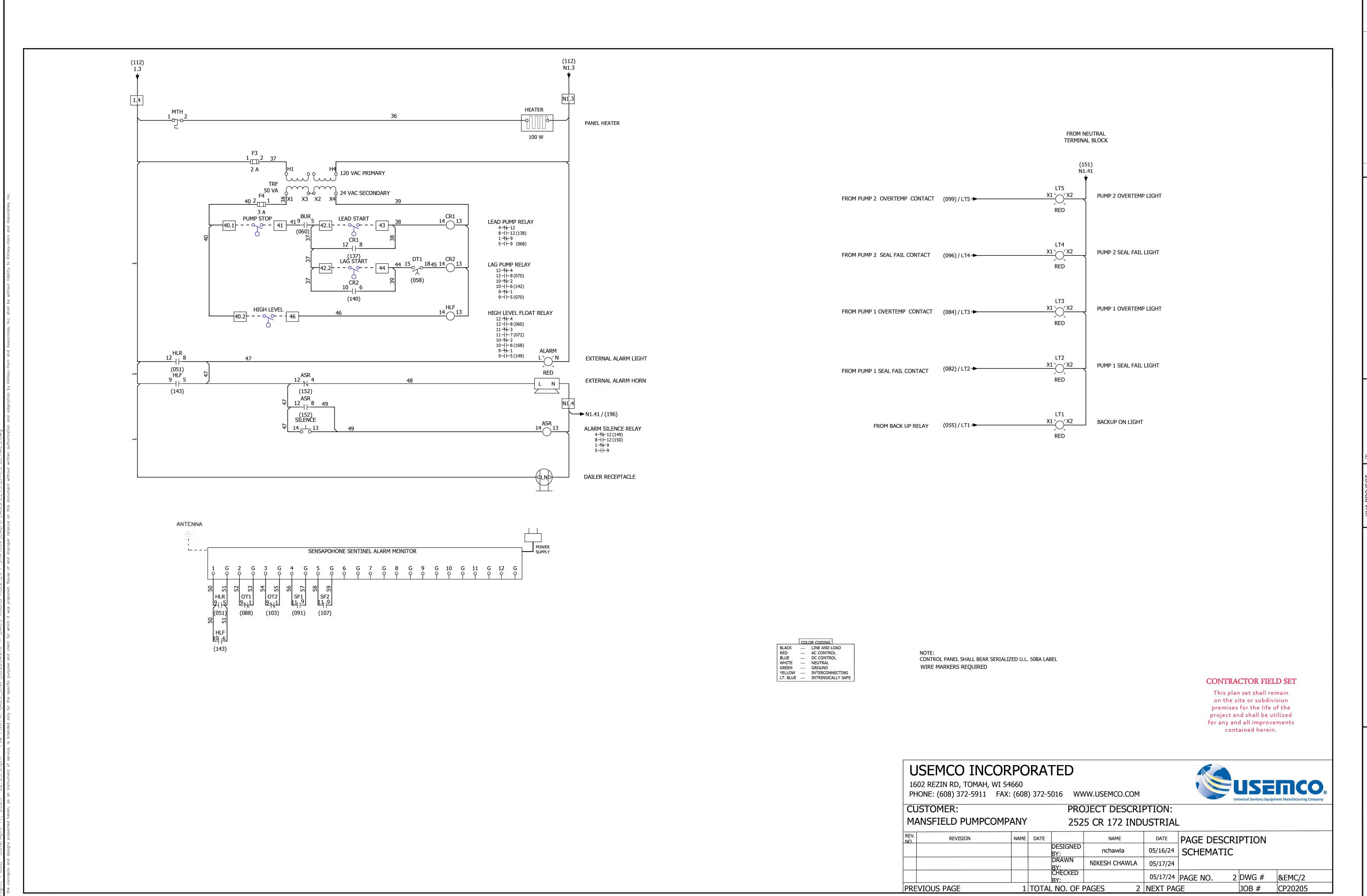
REV. NO.	REVISION	NAME	DATE		NAME	DATE	F
				DESIGNED BY:	nchawla	05/16/24	İ
				DRAWN BY:	NIKESH CHAWLA	05/17/24	
				CHECKED BY:		05/17/24	P
PR	EVIOUS PAGE		&/3	TOTAL NO. OF	PAGES	1	N

DATE	PAGE DESCRIPT	ION		
5/16/24	PANEL LAYOUT			
5/17/24				
5/17/24	PAGE NO.	1	DWG NO.	&PNL/1
1	NEXT PAGE	&EMC/1	JOB NO.	CP20205

SHEET NUMBER 52 OF 218

**ELECTRICAL PANEL** 





78759 No. REVISIONS D.

© 2025 KIMLEY-HORN AND ASSOCIATES, INC.
10814 JOLLYVILLE ROAD, CAMPUS IV, SUITE 200, AUSTIN, TX 787
PHONE: 512-418-1771 FAX: 512-418-1791
www.KIMLEY-HORN.COM

TARA R. RAOOF

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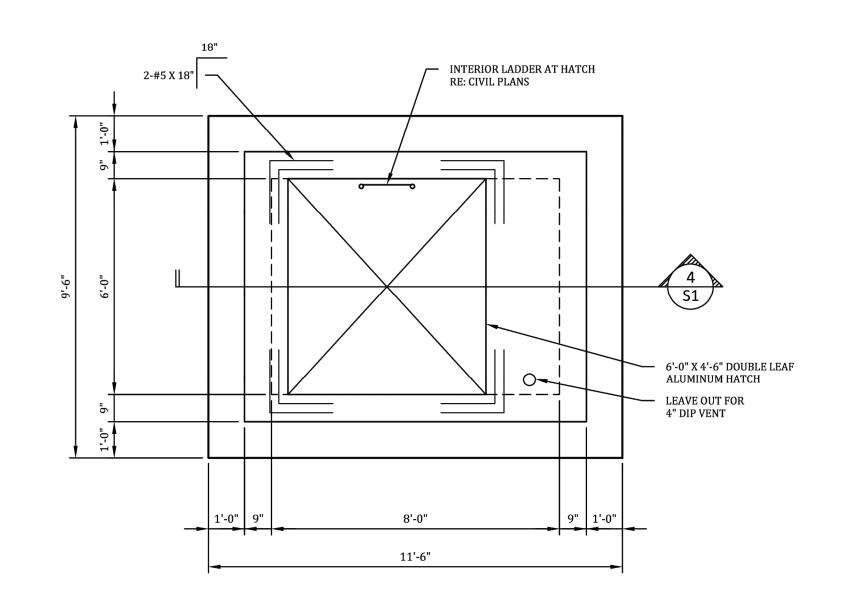
8/12/2024

DATE
AUGUST 2024
SCALE: AS SHOWN
DESIGNED BY: ATN

LIFT STATION ELECTRICAL PLANS II

2525 CR 172
INDUSTRIAL
CITY OF ROUND ROCK
WILLIAMSON COUNTY, TEXAS

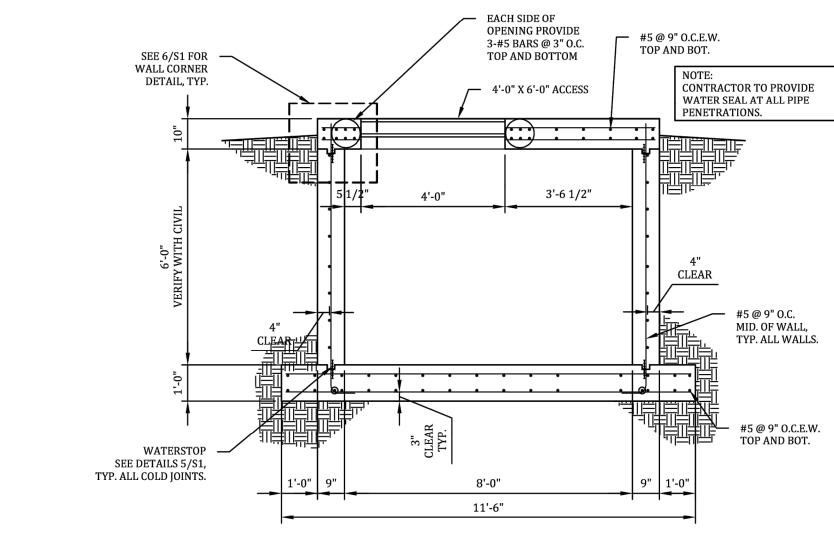
SHEET NUMBER 54 OF 218



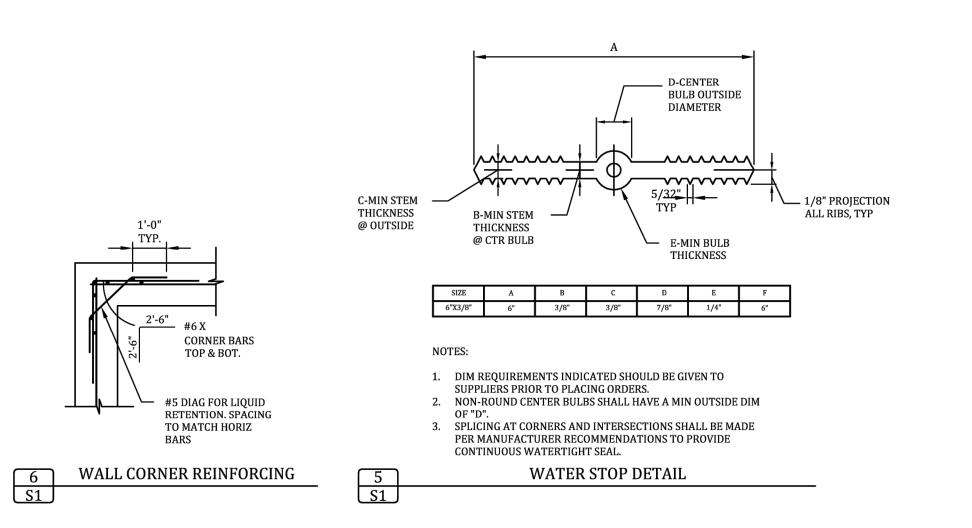
VALVE VAULT PLAN VIEW - DIM CONTROL

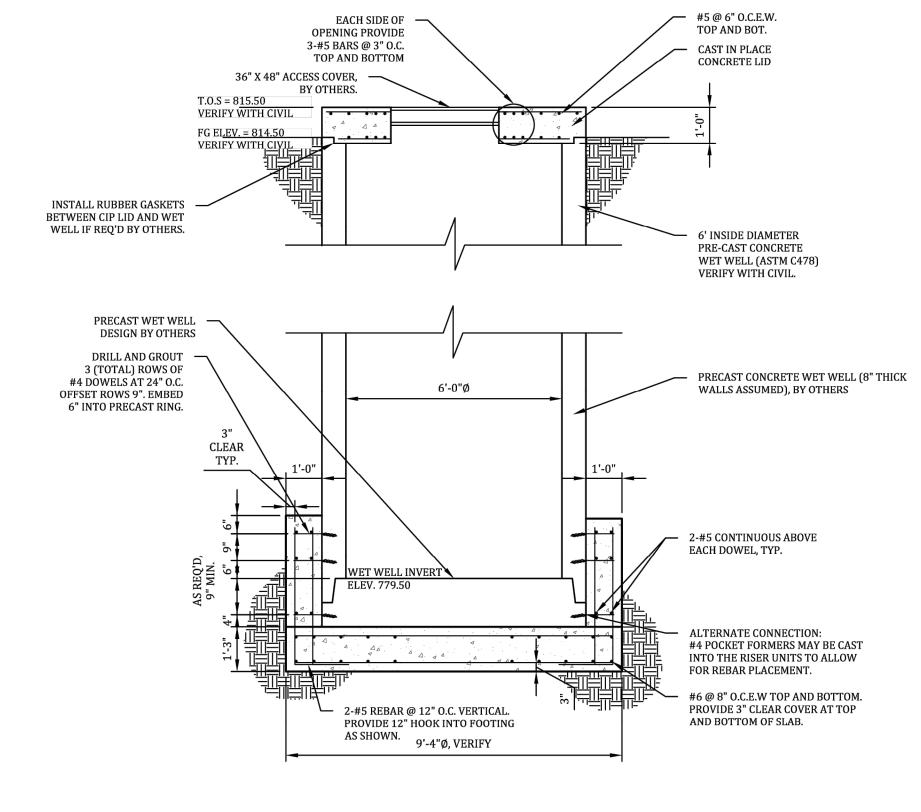
EDGE OF FOUNDATION EDGE OF TOP SLAB 36" X 48" ACCESS COVER, 2-#5 X 3'-0" DIAGONAL. CAST IN PLACE CONCRETE LID 2-#5 X 3'-0" DIAGONAL AT CORNER OF HATCH OPENINGS EACH SIDE OF OPENING PROVIDE #5 @ 6" O.C. 3-#6 BARS @ 3" O.C. #5 @ 6" O.C. TOP AND BOTTOM #5 @ 3" O.C. 6'-0" VERIFY - WET WELL 9'-4" VERIFY

WET WELL PLAN VIEW AND CAP REINF. AND DIM CONTROI



VALVE VAULT CROSS SECTION





COORDINATE ALL PENETRATIONS WET WELL ANTI-FLOTATION RING AND ANCHORING WITH THE CIVIL, HAS BEEN DESIGNED FOR 35'-0" MECHANICAL, AND ELECTRICAL DRAWINGS WET WELL DEPTH PER KIMLEY HORN PLANS DATE ALL GROUTING IS TO BE DONE USING HILTI HIT-RE 500 V3 EPOXY MORTAR. ALL REBAR TO BE EPOXY-COATED. CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY ENGINEER OF DISCREPANCIES BEFORE PROCEEDING WITH CONSTRUCTION. AN 8" WET WELL WALL THICKNESS IS ASSUMED IN GEOMETRY OF ANTI-FLOTATION RING/FOUNDATION AND CIP

MAY 22, 2024.

WET WELL ROOF.

WET WELL ANTI-FLOTATION RING/FOUNDATION AND CIP WET WELL ROOF CROSS SECTION

GENERAL NOTES

CONCRETE MIX DESIGN:

1. ALL CONCRETE SHALL HAVE MINIMUM COMPRESSIVE STRENGTH f'c=3600 psi, TxDOT CLASS C CONCETE, AT 28 DAYS. PROVIDE 5" SLUMP. ADDING WATER AT SITE IS NOT PERMITTED.

2. THE CONCRETE MIX DESIGNS SHALL BE PROPORTIONED TO MINIMIZE THE ADVERSE EFFECTS OF CLIMATE AT THE TIME OF YEAR THE CONCRETE IS PLACED. USE OF WORKABILITY ADMIXTURES AND

3. USE OF CALCIUM CHLORIDE ADMIXTURES IS NOT PERMITTED.

4. THE BUILDER/CONTRACTOR IS RESPONSIBLE FOR MAKING CONCRETE COMPRESSIVE STRENGTH CYLINDER TESTS AS REQUIRED BY A.C.I. 318-14.

1. ALL REINFORCING STEEL SHALL BE NEW BILLET STEEL CONFORMING TO A.S.T.M. A-615 GRADE 60. REINFORCEMENT SHALL BE FREE OF RUST AND DELETERIOUS MATERIALS.

1. SUFFICIENT SUPPORT BARS AND CHAIRS SHALL BE PROVIDED TO MAINTAIN PROPER

3. TRENCHES FOR DEEP PLUMBING LINES SHOULD NOT BE LOCATED DIRECTLY UNDER (PARALLEL TO)

4. ALL SLABS AND BEAMS SHALL BE PROPERLY GRADED AND LEVELED. A THIN (1"±) LAYER OF SAND

5. A POLYETHYLENE VAPOR BARRIER SHALL BE PLACED BETWEEN THE CONCRETE AND SOIL. A THIN

6. CONTRACTOR NOTE: BEFORE PROCEEDING WITH ANY WORK OR ORDERING OF MATERIALS, THE CONTRACTOR AND/OR SUBCONTRACTORS SHALL VERIFY ALL MEASUREMENTS AND LOCATION OF BUILDING COMPONENTS AND THEIR INTERRELATIONSHIP AT THE BUILDING SITE AND FOR THEIR

1. THE REINFORCED CONCRETE DESIGN OF THIS FOUNDATION STRUCTURE HAS BEEN ANALYZED USING STANDARD ENGINEERING PRACTICES.

2. THE REINFORCED CONCRETE SLAB-ON-GRADE DESIGN SHOWN REFLECTS THE MINIMUM REQUIREMENTS NEEDED TO ALLOW THIS STRUCTURE SYSTEM TO ADEQUATELY PERFORM IN CONJUNCTION WITH THE SOIL PARAMETERS NOTED BELOW. THE INTENT IS NOT TO LIMIT SOIL MOVEMENT, BUT INSTEAD TO LIMIT EXCESSIVE FOUNDATION FLEXURE SO THAT DIFFERENTIAL

3. FOUNDATION MAINTENANCE: DRAINAGE AWAY FROM THE FOUNDATION SHALL BE MAINTAINED AT ALL TIMES SO THAT GROUND WATER WILL NOT COLLECT ADJACENT TO OR UNDER THE SLAB. THE INTENT IS TO MAINTAIN AN EVEN SOIL MOISTURE CONTENT AROUND THE BUILDING SO THAT

4. HEAVY VEGETATION SUCH AS TREES WITHIN 10-15 FEET OF THE PERIMETER OF THE FOUNDATION MAY CAUSE EXCESSIVE WATER REMOVAL FROM THE SOIL NEAR OR UNDER THE FOUNDATION. SUCH UNANTICIPATED PARAMETERS ARE NOT CONSIDERED UNLESS REPORTED BY THE CONTRACTOR TO FALKOFSKE ENGINEERING, INC. IN WRITING PRIOR TO DESIGN.

5. GEO-TECHNICAL INVESTIGATION PROVIDED BY: ROCK ENGINEERING & TESTING LABORATORY, INC. REPORT NO. G221847

ALLOWABLE SOIL BEARING PRESSURE (BEARING IN CLAY) ALLOWABLE SOIL BEARING PRESSURE (BEARING IN BEDROCK) MINIMUM DEPTH OF FOUNDATION INTO BEARING SOIL

Qa = 15,000 psf

7. BUILDING CODE: 2015 INTERNATIONAL BUILDING CODE

1. INSPECTIONS ARE TO BE AS REQUIRED BY LOCAL BUILDING DEPARTMENT.

2. FALKOFSKE ENGINEERING INC. SHALL BE CALLED FOR CONSTRUCTION REVIEW OF FOUNDATION

The use of precast concrete bar supports (commonly called "Dobies") to support slab reinforcement or beam reinforcement for concrete slab on grade foundations is acceptable. It is important that the precast concrete bar supports are in fact made of concrete with an allowable f c of 3000 psi. If the pads purchased are from the Aztec Concrete Accessories, Inc. which produces them, then they are pre-approved by Falkofske Engineering, Inc.

As referenced by Aztec Concrete Accessories, Inc., the Concrete Reinforcing Steel Institute (CRSI) references the use of concrete bar supports in the Manual of Standard Practice. The Manual of Standard Practice shows for dowel bar support). The ACI also references the use of dobies as a standard practice, and the CRSI Manual of Standard Practice as a source for placing layouts.

It is our opinion that precast concrete bar supports do a much better job of supporting slab reinforcement

AIR ENTRAINMENT IS PERMITTED AND MUST BE NOTED ON MIX DESIGNS.

CONCRETE REINFORCEMENT:

REINFORCEMENT PROFILE THROUGHOUT CONCRETE PLACEMENT. BARS SHALL BE SUPPORTED ON CHAIRS AND TIED AT ALL INTERSECTIONS. USE 4'-0" MAXIMUM SPACING FOR SUPPORT CHAIRS.

2. INSURE PROPER VIBRATION OF CONCRETE AROUND ALL REINFORCEMENT.

MAY BE USED UNDER THE SLAB.

LAYER OF LEVELING SAND MAY BE PLACED ON TOP OF THE POLYETHYLENE.

MOVEMENT WILL NOT CAUSE UNREASONABLE DISTRESS.

DIFFERENTIAL SOIL MOVEMENT IS LIMITED.

DATED: 12/23/21

6. ALL FILL AREAS AND MATERIALS SHALL BE PLACED, COMPACTED, AND TESTED PER THE GEO-TECHNICAL ENGINEERS RECOMMENDATIONS.

BEAMS AND REINFORCEMENT PRIOR TO PLACING CONCRETE. CONTRACTORS OPTION:

the use of wire dobies, plain dobies, combination (those with grooves), and dowel dobies (those with a hole

than plastic chairs which is typical in the today's Slab on Grade foundation construction.

Brick veneer of either clay brick or concrete brick shall not be used in any Falkofske Engineering, Inc. foundation design.

ERIK GARCIA

147076

**CONTRACTOR FIELD SET** 

This plan set shall remain

on the site or subdivision

premises for the life of the

project and shall be utilized

for any and all improvements

contained herein.

CONTRACTOR TO VERIFY ALL DIMENSIONS

WITH EQUIPMENT SIZES PRIOR TO

CONSTRUCTION

SHEET NUMBER 55 OF 218



	Project Name:	SDP - 2525 CR 172 INDUSTRIAL		
	Date Prepared:	10/16/2024		
1. The Required Lo	ad Reduction for t	he Total Project.		
		Calculations from RG-348, Pages 3-27 to 3-30		
		Page 3-29 Equation 3.3:		
Where:		$L_{M} = 27.2(A_{N} \times P)$		
vviiere.	L <sub>M TOTAL PROJECT</sub> =	Required TSS removal resulting from the proposed develo	pment = 80% of i	increased lo
		Net increase in impervious area for the project		
	P =	Average annual precipitation, inches		
Sito Doto:	Dotormino Boquiro	ad Lond Pomoval Based on the Entire Project		
Sile Dala.	Determine Require	ed Load Removal Based on the Entire Project  County =	Williamson	
		Total project area included in plan =	49.10	acres
		evelopment impervious area within the limits of the plan =	0.00	acres
	Total post-de	evelopment impervious area within the limits of the plan =	32.72	acres
		Total post-development impervious cover fraction = P =	0.67 32	inches
		L <sub>M TOTAL PROJECT</sub> =	28479	lb —
	Number	of drainage basins / outfalls areas leaving the plan area =	2	
2. Drainage Basin F	Parameters (This in	nformation should be provided for each basin).		
		Drainage Basin/Outfall Area No. =	2	
		Total drainage basin/outfall area =	26.98	acres
	•	ment impervious area within drainage basin/outfall area =	0.00	acres
		ment impervious area within drainage basin/outfall area =	17.97	acres
	Post-developme	nt impervious fraction within drainage basin/outfall area =	0.67	11-
		L <sub>M THIS BASIN</sub> =	15,641	lb
3. Indicate the Prop	osed BMP Code fo	or this Basin.		
4. Calculate Maxim	um TSS Load Rem	Removal efficiency =	78	percent
4. Calculate Maxim	um TSS Load Rem	noved (L <sub>R</sub> ) for this Drainage Basin by the Selected BMP  RG-348 Page 3-33 Equation 3.7:		percent
4. Calculate Maxim	um TSS Load Rem	oved (L <sub>R</sub> ) for this Drainage Basin by the Selected BMP		percent
	A <sub>C</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area		регости
	A <sub>C</sub> = A <sub>I</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area		percent
	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area	Type.	percent
	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area	Type.	percent
	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area	Type.	acres
	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> =	RG-348 Page 3-33 Equation 3.7:  L <sub>R</sub> = (BMP efficiency) x P x (A <sub>I</sub> x 34.6 + A <sub>P</sub> x 0.54)  Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed	Type.	
	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area area	Type.  psed BMP  26.98	acres
	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{l} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area area.	Type.  sed BMP  26.98  17.97	acres acres
Where:	$A_{C} = A_{I} = A_{P} = L_{R} =$	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area area	Type.  26.98 17.97 9.01 15,641	acres acres acres
Where:	$A_{C} = A_{I} = A_{P} = L_{R} =$	RG-348 Page 3-33 Equation 3.7: $L_R = (BMP \text{ efficiency}) \times P \times (A_1 \times 34.6 + A_P \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area LR = $A_R = A_R $	Type.  26.98 17.97 9.01 15,641	acres acres acres lb
Where:	$A_{C} = A_{I} = A_{P} = L_{R} =$	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP c	Type.  26.98 17.97 9.01 15,641	acres acres acres lb
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{l} \times 34.6 + A_{p} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP c	Type.  26.98 17.97 9.01 15,641 No. 15,641 1.00	acres acres acres lb
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{l} \times 34.6 + A_{p} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area by the BMP catchment area by the proposed in the BMP catchment area by	Type.  26.98 17.97 9.01 15,641 No. 15,641 1.00	acres acres acres lb
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{l} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP c	Type.  26.98 17.97 9.01 15,641 No. 15,641 1.00 a.	acres acres lb ote
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area by the BMP c	Type.  26.98 17.97 9.01 15,641 1.00  a.  4.00 0.47	acres acres lb ote lb
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area by the proposed in the BMP catchment area by the BMP catch	Type.  26.98 17.97 9.01 15,641 1.00 a. 4.00	acres acres lb ote
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area by the BMP c	Type.  26.98 17.97 9.01 15,641 1.00  a.  4.00 0.47	acres acres lb ote lb
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{1} \times 34.6 + A_{p} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area by the BMP catchment ar	Type.  Dised BMP  26.98 17.97 9.01 15,641 No.  15,641 1.00 a.  4.00 0.47 185,502	acres acres acres lb ote lb inches cubic fee
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area  TSS Load removed from this catchment area by the proposed in the BMP catchment area by the proposed in the BMP catchment area Pervious area remaining in the BMP catchment area by the proposed in the BMP catchment area by the BMP catchment are	Type.  26.98 17.97 9.01 15,641 1.00  a.  4.00 0.47	acres acres lb ote lb
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area  TSS Load removed from this catchment area by the proposed by the proposed in the BMP catchment area by the BMP catchment a	Type.  26.98 17.97 9.01 15,641 1.00  a.  4.00 0.47 185,502	acres acres lb ote lb inches cubic fee
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7:  L <sub>R</sub> = (BMP efficiency) x P x (A <sub>I</sub> x 34.6 + A <sub>P</sub> x 0.54)  Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP and the BMP catchment area by the proposed in the BMP and the B	Type.  26.98 17.97 9.01 15,641 1.00  15,641 1.00  0.47 185,502	acres acres acres lb ote lb inches cubic fee
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area  TSS Load removed from this catchment area by the proposed by the proposed in the BMP catchment area by the proposed in the BMP and the BMP catchment area by the proposed in the BMP and the BMP a	Type.  Dised BMP  26.98  17.97  9.01  15,641  No.  15,641  1.00  a.  4.00  0.47  185,502	acres acres lb ote lb inches cubic fee
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area Pervious area remaining in the BMP catchment area Pervious area remaining in the BMP catchment area Desired Load removed from this catchment area by the proposed in the BMP catchment area by the proposed in the BMP catchment area Pervious Acceptable Impervious Proposed Impervious Catchment area by the proposed Impervious Catchment area by the proposed Impervious Catchment area by the proposed Impervious Catchment area Desired Load Impervious Catchment Impervious C	Type.  26.98 17.97 9.01 15,641 1.00  a.  4.00 0.47 185,502  0.00 0 0.00 0 37,100	acres acres acres lb ote lb inches cubic fee
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area Pervious area remaining in the BMP catchment area Pervious area remaining in the BMP catchment area by the proposed in the BMP and the proposed in the BMP and	Type.  Dised BMP  26.98  17.97  9.01  15,641  1.00  15,641  1.00  a.  4.00  0.47  185,502	acres acres acres lb ote lb inches cubic fee
Where:	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the BMP catchment area Desired Lambers	Type.  26.98 17.97 9.01 15,641 1.00  a.  4.00 0.47 185,502  0.00 0 0.00 0 37,100	acres acres acres lb ote lb inches cubic fee
Where:  5. Calculate Fraction  6. Calculate Captur	A <sub>C</sub> = A <sub>I</sub> = A <sub>P</sub> = L <sub>R</sub> =	RG-348 Page 3-33 Equation 3.7: $L_{R} = (BMP \text{ efficiency}) \times P \times (A_{I} \times 34.6 + A_{P} \times 0.54)$ Total On-Site drainage area in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the proposed in the BMP catchment area Desired Lambers and the BMP catchment area Desired Lambers	Type.  26.98 17.97 9.01 15,641 1.00  a.  4.00 0.47 185,502  0.00 0 0.00 0 37,100	acres acres acres lb ote lb inches cubic fee



CONTRACTOR FIELD SET

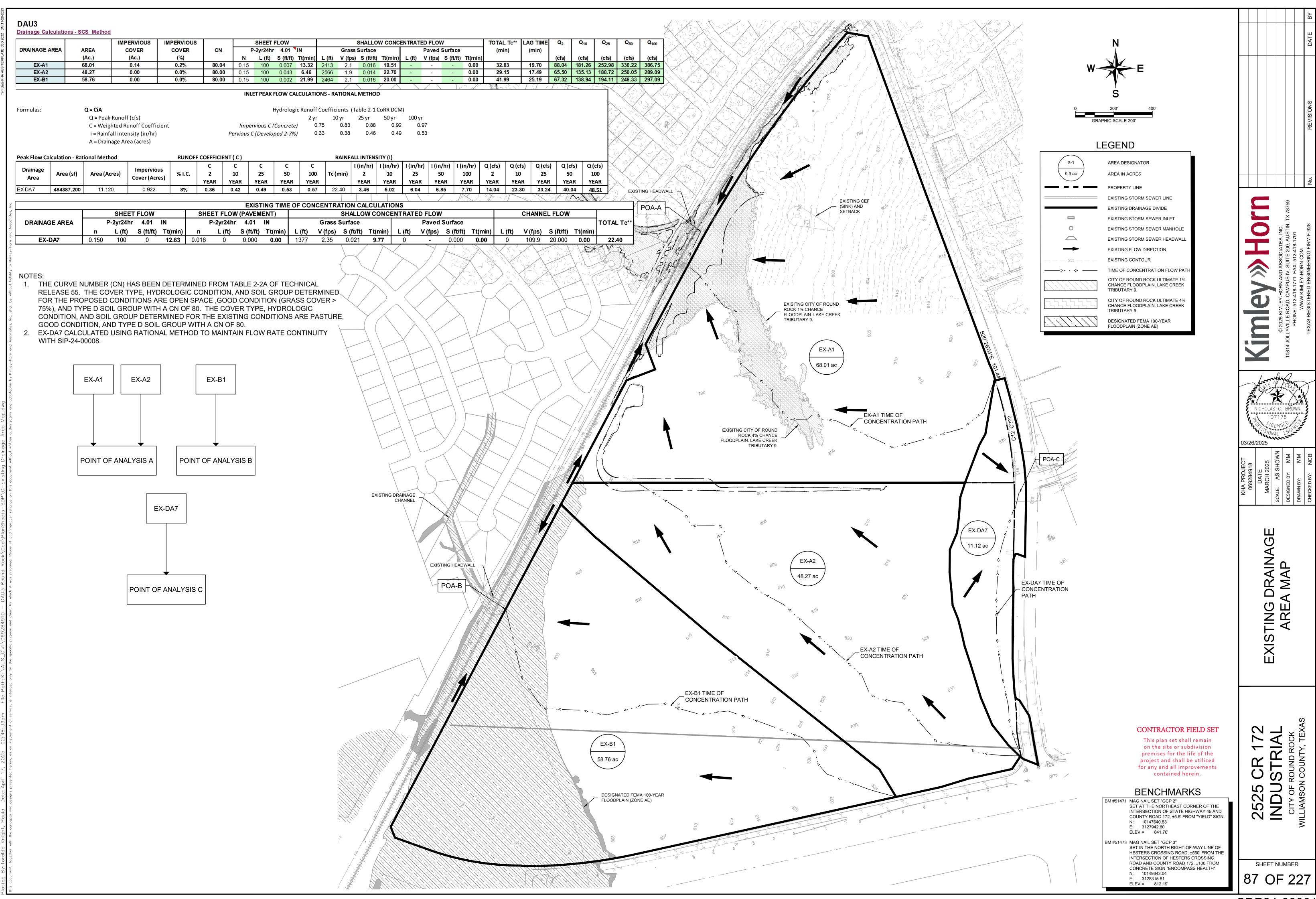
This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements

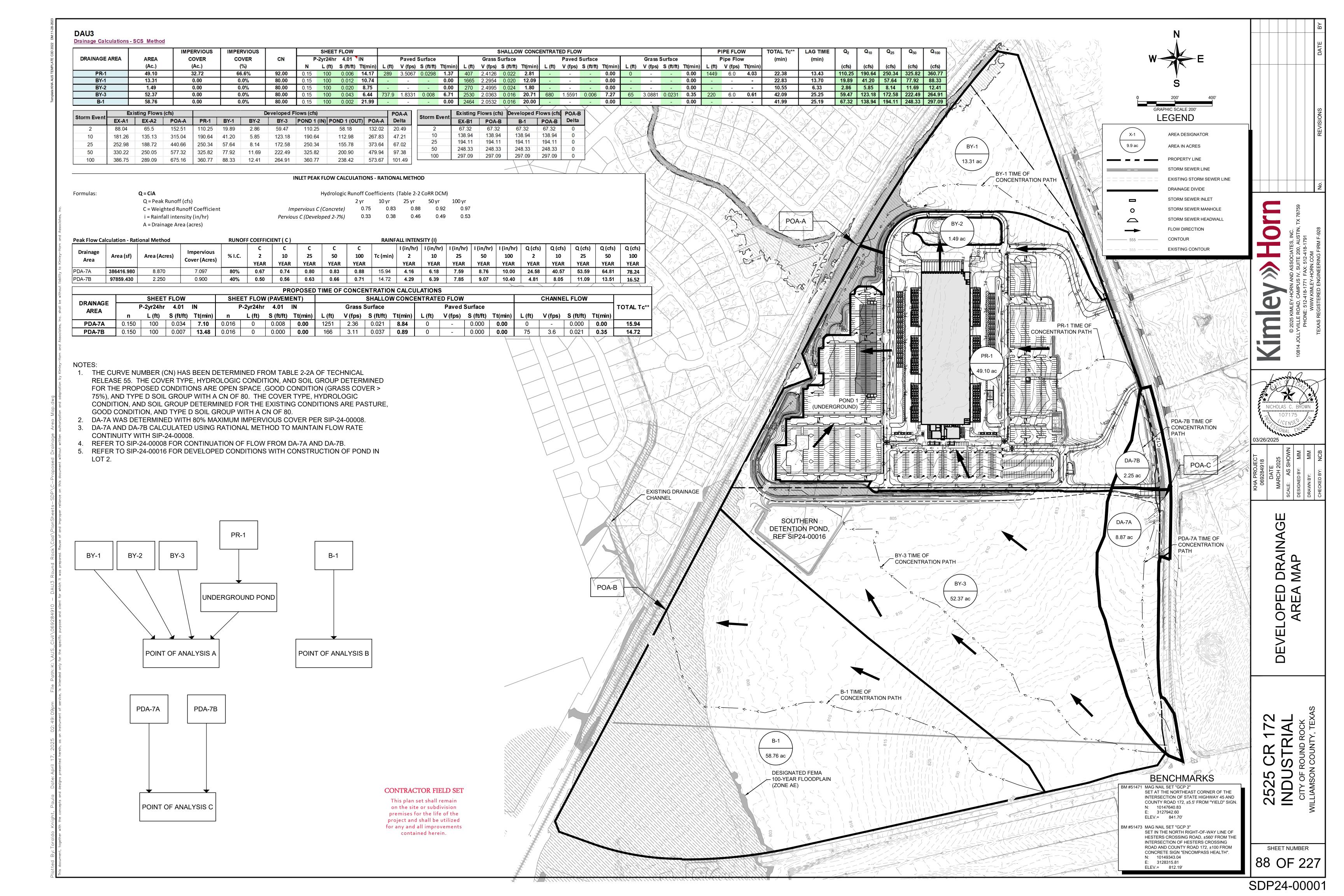
SHEET NUMBER

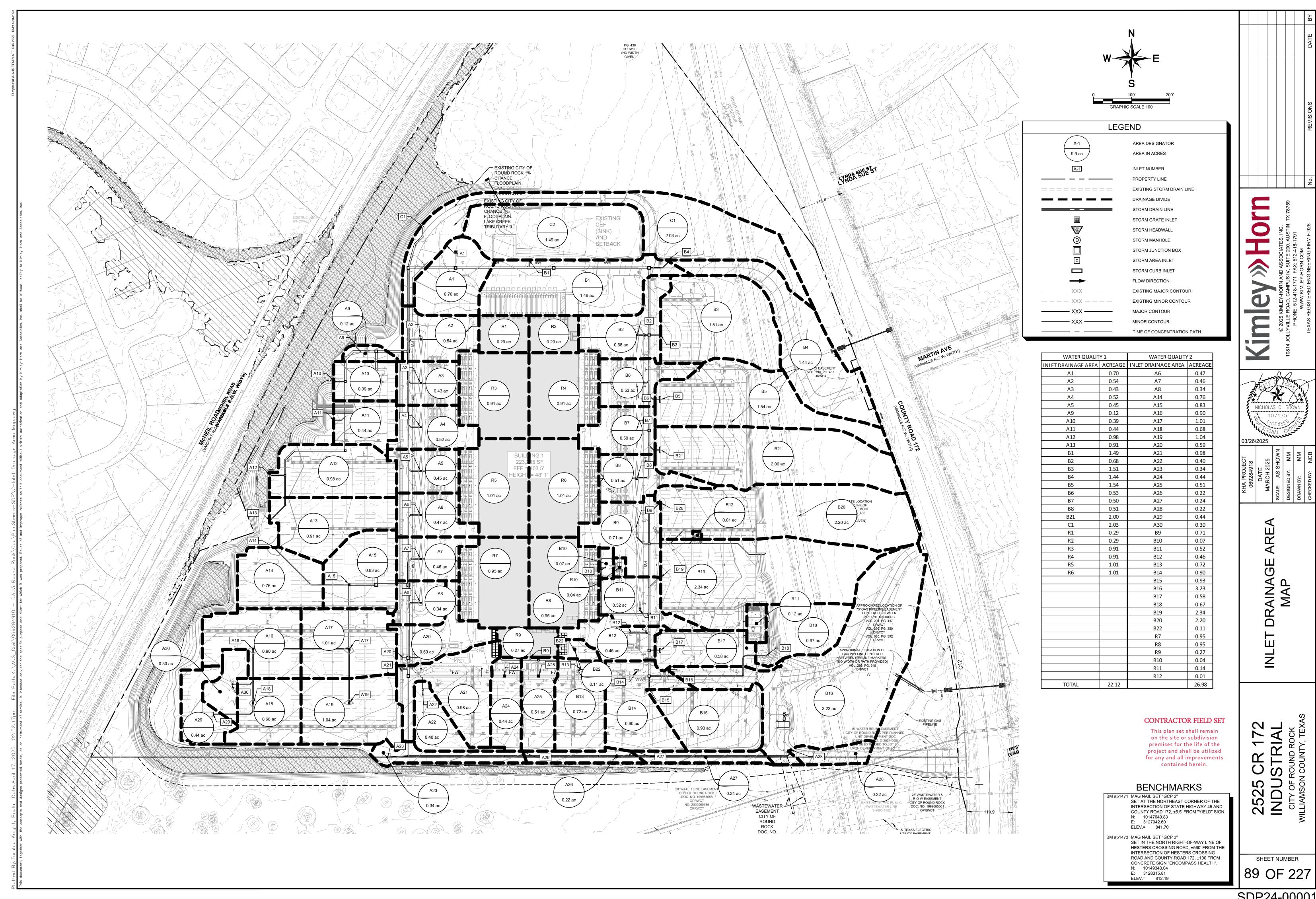
2525

CALCULATIONS

TCEQ







#### Kimley» Horn **INLET PEAK FACTOR CALCULATIONS - RATIONAL METHOD** Formulas: Q=CiA Hydrologic Runoff Coefficients (Table 2-2 DCM) Q=Peak Factor Runoff 2yr 10yr 25yr 100yr Rainfall Intensities from Table 6 Round C=Weighted Runoff Coefficient Impervious C (Concrete) 0.75 0.83 0.88 0.97 Rock RAIn (NOAA Atlas 14 Lake Creek i=Rainfall Intensity Pervious C (Good, Average) 0.29 0.35 0.39 0.46 Watershed Centroid Rainfall Intensity). A=Drainage Area (acres) RUNOFF COEFFICIENT (C) RAINFALL INTENSITY (I) PEAK RUNOFF (Q)

AREA   NEEA   Pervious impervious   C			T	T		IXX	JIVOIT COL	FFICIENT (				AIN ALL IIV	TENSITY ( I	1		PEAK RUN	1011 ( Q )	
MAME   ACC   Cover (ACC   Cover (ACC   Cover (ACC   Cover (ACC   Cover (ACC   Cover (ACC   ACC   ACC	ΔRFΔ	ΔRFΔ	Pervious	Impervious	Impervious	C	c	C	C	Tc	1		1	1	0	0	Q	Q
All				-	-					10 10 10	2-VFΔR	10-VFAR	25-VFAR	100-VEAR			25-YEAR	100-YEAR
A2         0.54         0.11         0.43         89%         0.06         0.72         0.78         0.87         5         6.23         9.39         11.00         15.10         1.22         3.69           AA         0.52         0.11         0.41         79%         0.06         0.73         0.78         0.86         5         6.23         9.29         11.00         15.10         2.10         3.50           AS         0.45         0.11         0.31         6.89         0.07         0.01         5         6.22         9.29         11.00         15.10         1.210         1.120         1.210         1.210         1.210         1.210         1.210         1.210         1.210         1.210         1.210	IVAIVIE	(AC.)	Cover (Ac.)	Cover (AC.)	Cover 78	Z-TEAN	10-TEAR	25-1 LAN	100-1 LAK	(111111)	Z-TEAN	10-1 LAK	23-1 LAN	100-TEAK	Z-1 LAN	10-1 LAK	23-TEAR	100-TEAK
A3	A1	0.70	0.45	0.25	36%	0.45	0.52	0.56	0.64	5	6.23	9.29	11.40	15.10	1.98	3.40	4.52	6.80
A4         0.42         0.11         0.12         7.79         0.04         0.71         0.76         0.73         0.78         0.65         5         6.23         9.39         11.60         15.10         2.17         2.55           A5         0.45         0.14         0.31         68%         0.05         0.07         0.81         5         6.23         9.39         11.60         15.10         1.68         2.87           A6         0.47         0.11         0.37         7.79         0.07         0.87         5         6.23         9.39         11.60         15.10         1.18         2.17           A8         0.14         0.13         0.33         7.79         0.06         0.07         0.73         5         6.23         9.39         11.60         15.10         1.79         2.93           A9         0.12         0.02         0.09         8.06         0.07         0.73         5         6.23         9.39         11.60         15.10         0.48         0.79           A11         0.48         0.02         0.02         0.03         0.88         0.97         5         6.23         9.39         11.60         15.10         0.48	A2	0.54	0.11	0.43	80%	0.66	0.73	0.78	0.87	5	6.23	9.29	11.40	15.10	2.22	3.69	4.82	7.09
Math	-							100000			10 M. M. M. C.		The section of the section of				3.73	5.49
A6         0.45         0.14         0.31         6.68         0.00         0.68         0.72         0.81         5         6.22         9.29         11.00         15.10         1.88         2.82           A7         0.46         0.13         0.33         7.79         0.85         0.77         0.83         5         6.23         9.29         11.00         15.10         1.79         2.98           A8         0.34         0.13         0.22         638         0.65         0.70         0.78         5         6.23         9.29         11.00         15.10         1.79         2.98           A9         0.12         0.02         0.09         80%         0.66         0.74         0.78         5         6.23         9.29         11.00         15.10         1.48         3.00           A11         0.44         0.02         0.02         998         0.73         0.81         0.88         0.97         5         6.23         9.29         11.00         15.10         4.08         7.31           A12         0.88         0.04         9.44         9.07         0.73         0.81         0.86         0.95         5         6.23         9.29			<del> </del>	<del> </del>													4.57	6.73
A6			<u> </u>	1							N 0						3.70	5.46
A7         0.46         0.13         0.33         72%         0.02         0.75         0.83         5         6.23         9.29         11.40         15.10         1.79         2.99           A9         0.12         0.02         0.09         80%         0.66         0.74         0.78         5         6.23         9.29         11.40         15.10         1.02         2.00           A11         0.39         0.00         0.39         0.00         0.73         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.82         3.00           A11         0.08         0.04         0.99         0.73         0.81         0.86         0.95         5         6.23         9.29         11.40         15.10         4.68         7.37           A13         0.91         0.00         0.91         100%         0.73         0.81         0.86         0.95         5         6.23         9.29         11.40         15.10         4.86         7.37           A13         0.76         0.00         0.34         0.34         0.72         0.81         0.89         0.95         5         6.23         9.29		100 100 10 100		1														
A8         0.34         0.21         0.22         65%         0.89         0.65         0.70         0.78         5         6.22         9.29         11.40         15.10         0.48         0.79           A10         0.39         0.00         0.39         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.20         3.33           A11         0.44         0.02         0.24         98%         0.73         0.81         0.85         5         5         6.23         9.29         11.40         15.10         2.02         3.33           A12         0.98         0.04         0.99         0.00         0.99         0.00         0.99         0.00         0.99         0.00         0.99         0.00         0.99         0.00         0.99         0.00         0.99         0.00         0.99         0.00 <th< td=""><td></td><td></td><td><u> </u></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>4.16</td><td>6.13</td></th<>			<u> </u>														4.16	6.13
AD   0.12	-																3.92	5.78
A10												+	11.40	15.10			2.75	4.07
A11	A9	0.12	0.02	0.09	80%	0.66	0.74	0.78	0.87	5	6.23	9.29	11.40	15.10	0.48	0.79	1.03	1.52
A12	A10	0.39	0.00	0.39	100%	0.75	0.83	0.88	0.97	5	6.23	9.29	11.40	15.10	1.82	3.00	3.91	5.71
A13	A11	0.44	0.02	0.42	96%	0.73	0.81	0.86	0.95	5	6.23	9.29	11.40	15.10	2.02	3.33	4.34	6.35
A13 0.91 0.00 0.91 100% 0.75 0.83 0.88 0.97 5 6.23 9.99 11.40 15.10 3.48 5.95 A15 0.83 0.88 0.97 5 6.23 9.99 11.40 15.10 3.48 5.95 A15 0.83 0.03 0.06 0.08 0.97 0.73 0.81 0.86 0.95 5 6.23 9.99 11.40 15.10 3.48 6.67 A15 0.83 0.03 0.06 0.84 99% 0.72 0.80 0.80 0.95 5 6.23 9.99 11.40 15.10 4.03 6.67 A17 1.01 0.07 0.94 93% 0.72 0.80 0.80 0.95 5 6.23 9.99 11.40 15.10 4.03 6.67 A17 1.01 0.07 0.94 93% 0.72 0.80 0.84 0.93 5 6.23 9.99 11.40 15.10 4.03 6.67 A17 1.01 0.07 0.94 0.95 0.05 0.05 0.05 0.05 0.05 0.05 0.05	A12	0.98	0.04	0.94	96%	0.73	0.81	0.86	0.95	5	6.23	9.29	11.40	15.10	4.46	7.37	9.60	14.03
A14				1						5							9.10	13.28
A15	-																7.48	10.94
A16			<b>+</b>						1								8.13	11.88
A17				-														_
A18										120							8.70	12.72
A19	-	a_200000 000		l		2000		200700000	0.01 0.01 0.00			A	VI				9.76	14.28
A20	A18	0.68	0.03	0.66	96%	0.73	0.81	0.86	0.95	5	6.23	9.29	11.40	15.10	3.11	5.14	6.70	9.79
A21	A19	1.04	0.08	0.96	92%	0.71	0.79	0.84	0.93	5	6.23	9.29	11.40	15.10	4.64	7.68	10.01	14.65
A22	A20	0.59	0.22	0.37	63%	0.58	0.65	0.70	0.78	5	6.23	9.29	11.40	15.10	2.11	3.55	4.66	6.90
A22	A21	0.98	0.28	0.70	72%	0.62	0.69	0.74	0.83	5	6.23	9.29	11.40	15.10	3.79	6.33	8.30	12.25
A23				1													2.49	3.76
A24																	3.00	4.40
A25         0.51         0.05         0.46         91%         0.71         0.79         0.84         0.92         5         6.23         9.29         11.40         15.10         2.24         3.71           A26         0.22         0.01         0.21         96%         0.73         0.81         0.86         0.97         5         6.23         9.29         11.40         15.10         1.00         1.66           A27         0.24         0.00         0.24         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         0.77         1.29           A28         0.44         0.12         0.32         7.3%         0.63         0.70         0.75         0.83         6.23         9.29         11.40         15.10         0.77         1.29           A30         0.30         0.30         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         0.54         0.98           B1         1.49         0.35         1.15         7.7%         0.65         0.72         0.77         0.85         5 <td></td> <td></td> <td></td> <td>1</td> <td></td> <td>4.17</td> <td>6.10</td>				1													4.17	6.10
A26         0.22         0.01         0.21         96%         0.73         0.81         0.96         0.97         5         6.23         9.29         11.40         15.10         1.00         1.66           A77         0.24         0.00         0.24         100%         0.75         0.83         0.98         0.97         5         6.23         9.29         11.40         15.10         1.14         18.8           A28         0.22         0.09         0.13         61%         0.57         0.64         0.69         0.77         5         6.23         9.29         11.40         15.10         170         2.83           A30         0.30         0.30         0.00         0.96         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         0.54         0.98           B1         1.49         0.35         1.15         77%         0.65         0.72         0.77         0.85         5         6.23         9.29         11.40         15.10         0.59         9.79         15.20         15.00         11.40         15.10         0.77         0.65         0.72         0.77         0.85			<u> </u>	<del> </del>													4.83	7.08
A27         0.24         0.00         0.24         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         0.14         1.88           A28         0.22         0.09         0.13         61%         0.57         0.64         0.69         0.77         5         6.23         9.29         11.40         15.10         0.77         1.29           A30         0.30         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         0.77         2.83           B1         1.49         0.35         1.15         77%         0.64         0.72         0.77         0.85         5         6.23         9.29         11.40         15.10         0.54         0.98           B2         0.68         0.15         0.53         77%         0.66         0.72         0.77         0.85         5         6.23         9.29         11.40         15.10         2.74         4.56           B3         1.51         0.87         0.64         42%         0.49         0.55         0.60         0.68         7	-		<del> </del>	i														
A28																	2.16	3.15
A29         0.44         0.12         0.32         73%         0.63         0.70         0.75         0.83         5         6.23         9.29         11.40         15.10         1.70         2.83           A30         0.30         0.30         0.00         0%         0.29         0.35         0.09         0.46         5         6.23         9.29         11.40         15.10         5.98         9.97           B2         0.68         0.15         0.53         7.7%         0.65         0.72         0.77         0.85         5         6.23         9.29         11.40         15.10         5.98         9.97           B2         0.68         0.15         0.53         7.7%         0.65         0.72         0.77         0.85         5         6.23         9.29         11.40         15.10         2.74         4.56           B3         1.51         0.87         0.64         42%         0.49         0.55         0.60         0.68         7         5.62         8.06         10.30         13.70         4.10         6.72           B4         1.44         0.80         0.66         0.57         0.61         0.69         5.623         9.29			<b>†</b>														2.44	3.56
A30         0.30         0.30         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         0.54         0.98           B1         1.49         0.35         1.15         77%         0.64         0.72         0.77         0.85         5         6.23         9.29         11.40         15.10         2.74         4.56           B3         1.51         0.87         0.64         42%         0.49         0.55         0.60         0.68         7         5.62         8.06         10.30         13.70         4.10         6.72           B4         1.44         0.78         0.66         46%         0.50         0.57         0.61         0.69         5         6.23         9.29         11.40         15.10         4.10         6.72           B5         1.54         1.00         0.54         35%         0.45         0.52         0.56         0.64         13         4.44         6.61         8.13         10.70         3.09         5         6.23         9.29         11.40         15.10         0.00         1.30         1.30         0.87         5         6.23			<u> </u>														1.70	2.52
B1         1.49         0.35         1.15         77%         0.64         0.72         0.77         0.85         5         6.23         9.29         11.40         15.10         5.98         9.97           B2         0.68         0.15         0.53         77%         0.65         0.77         0.85         5         6.23         9.29         11.40         15.10         2.74         4.56           B3         1.51         0.87         0.64         42%         0.49         0.55         0.60         0.68         7         5.62         8.06         10.30         13.70         4.10         6.72           B4         1.44         0.78         0.66         46%         0.50         0.57         0.61         0.69         5         6.23         9.29         11.40         15.10         4.51         7.65           B5         1.54         1.00         0.54         35%         0.45         0.52         0.56         0.64         13         4.44         6.61         8.13         10.70         3.09         5.23           B6         0.53         0.12         0.41         78%         0.65         0.72         0.77         0.83         0.87	A29	0.44	0.12	0.32	73%	0.63	0.70	0.75	0.83	5	6.23	9.29	11.40	15.10	1.70	2.83	3.71	5.47
B2         0.68         0.15         0.53         77%         0.65         0.72         0.77         0.85         5         6.23         9.29         11.40         15.10         2.74         4.56           B3         1.51         0.87         0.64         42%         0.49         0.55         0.60         0.68         7         5.62         8.06         10.30         13.70         4.10         6.72           B4         1.44         0.78         0.66         46%         0.50         0.57         0.61         0.69         5         6.23         9.29         11.40         15.10         4.51         7.75         8.6         6.6         1.54         1.00         0.54         35%         0.45         0.52         0.56         0.64         13         4.44         6.61         8.13         10.70         3.09         5.28           B6         0.53         0.12         0.41         7.8%         0.65         0.72         0.77         0.86         5         6.23         9.29         11.40         15.10         2.04         3.40           B8         0.51         0.10         0.41         80%         0.66         0.73         0.78         0.87	A30	0.30	0.30	0.00	0%	0.29	0.35	0.39	0.46	5	6.23	9.29	11.40	15.10	0.54	0.98	1.33	2.08
B3         1.51         0.87         0.64         42%         0.49         0.55         0.60         0.68         7         5.62         8.06         10.30         13.70         4.10         6.72           B4         1.44         0.78         0.66         46%         0.50         0.57         0.61         0.69         5         6.23         9.29         11.40         15.10         4.51         7.65           B5         1.54         1.00         0.54         35%         0.45         0.52         0.56         0.64         13         4.44         6.61         8.13         10.70         3.09         5.28           B6         0.53         0.12         0.41         78%         0.65         0.72         0.77         0.86         5         6.23         9.29         11.40         15.10         2.04         3.45           B7         0.50         0.10         0.41         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.04         3.46           B9         0.71         0.13         0.58         81%         0.66         0.74         0.79         0.87	B1	1.49	0.35	1.15	77%	0.64	0.72	0.77	0.85	5	6.23	9.29	11.40	15.10	5.98	9.97	13.04	19.20
B3         1.51         0.87         0.64         42%         0.49         0.55         0.60         0.68         7         5.62         8.06         10.30         13.70         4.10         6.72           B4         1.44         0.78         0.66         46%         0.50         0.57         0.61         0.69         5         6.23         9.29         11.40         15.10         4.51         7.65           B5         1.54         1.00         0.54         35%         0.45         0.52         0.56         0.64         13         4.44         6.61         8.13         10.70         3.09         5.28           B6         0.53         0.12         0.41         78%         0.65         0.72         0.77         0.86         5         6.23         9.29         11.40         15.10         2.04         3.46           B8         0.51         0.10         0.41         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.04         3.46           B9         0.71         0.13         0.58         81%         0.66         0.74         0.79         0.87	B2	0.68	0.15	0.53	77%	0.65	0.72	0.77	0.85	5	6.23	9.29	11.40	15.10	2.74	4.56	5.96	8.78
B4         1.44         0.78         0.66         46%         0.50         0.57         0.61         0.69         5         6.23         9.29         11.40         15.10         4.51         7.65           B5         1.54         1.00         0.54         35%         0.45         0.52         0.56         0.64         13         4.44         6.61         8.13         10.70         3.09         5.28           B6         0.53         0.12         0.41         78%         0.65         0.72         0.77         0.86         5         6.23         9.29         11.40         15.10         2.04         3.40           B8         0.51         0.10         0.41         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.04         3.40           B9         0.71         0.13         0.58         81%         0.66         0.74         0.79         0.87         5         6.23         9.29         11.40         15.10         2.08         3.46           B10         0.07         0.00         0.07         100%         0.75         0.83         0.88         0.97	B3	1.51	0.87	0.64	42%	0.49	0.55	0.60	0.68	7	5.62	8.06	10.30	13.70	4.10	6.72	9.27	13.95
B5         1.54         1.00         0.54         35%         0.45         0.52         0.56         0.64         13         4.44         6.61         8.13         10.70         3.09         5.28           B6         0.53         0.12         0.41         78%         0.65         0.72         0.77         0.86         5         6.23         9.29         11.40         15.10         2.13         3.55           B7         0.50         0.10         0.40         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.04         3.40           B8         0.51         0.10         0.41         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.04         3.46           B9         0.71         0.13         0.58         81%         0.66         0.74         0.79         0.87         5         6.23         9.29         11.40         15.10         2.25         4.90           B10         0.07         1.00         0.07         1.00%         0.75         0.83         0.88         0.97				1						5							10.12	15.13
B6         0.53         0.12         0.41         78%         0.65         0.72         0.77         0.86         5         6.23         9.29         11.40         15.10         2.13         3.55           B7         0.50         0.10         0.40         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.04         3.40           B8         0.51         0.10         0.41         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.04         3.46           B9         0.71         0.13         0.58         81%         0.66         0.74         0.79         0.87         5         6.23         9.29         11.40         15.10         2.08         3.46           B10         0.07         0.00         0.07         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         0.33         0.54           B11         0.52         0.11         0.41         79%         0.63         0.71         0.76         0.84			1	1					1			<del> </del>					7.05	10.54
B7         0.50         0.10         0.40         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.04         3.40           B8         0.51         0.10         0.41         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.08         3.46           B9         0.71         0.13         0.58         81%         0.66         0.74         0.79         0.87         5         6.23         9.29         11.40         15.10         2.08         3.46           B10         0.07         0.00         0.07         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         0.33         0.54           B11         0.52         0.11         0.41         79%         0.65         0.73         0.78         0.86         5         6.23         9.29         11.40         15.10         0.33         0.54           B12         0.46         0.12         0.34         75%         0.63         0.71         0.76         0.84			<u> </u>	+								<u> </u>					4.64	6.83
B8         0.51         0.10         0.41         80%         0.66         0.73         0.78         0.87         5         6.23         9.29         11.40         15.10         2.08         3.46           B9         0.71         0.13         0.58         81%         0.66         0.74         0.79         0.87         5         6.23         9.29         11.40         15.10         2.95         4.90           B10         0.07         0.00         0.07         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         0.33         0.54           B11         0.52         0.11         0.41         79%         0.65         0.73         0.78         0.86         5         6.23         9.29         11.40         15.10         0.15         1.12         3.53           B12         0.46         0.12         0.34         75%         0.63         0.71         0.76         0.84         5         6.23         9.29         11.40         15.10         3.07         15.82         3.03           B13         0.72         0.10         0.62         87%         0.69         0.77															F 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			
B9         0.71         0.13         0.58         81%         0.66         0.74         0.79         0.87         5         6.23         9.29         11.40         15.10         2.95         4.90           B10         0.07         0.00         0.07         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         0.33         0.54           B11         0.52         0.11         0.41         79%         0.65         0.73         0.78         0.86         5         6.23         9.29         11.40         15.10         2.12         3.53           B12         0.46         0.12         0.34         75%         0.63         0.71         0.76         0.84         5         6.23         9.29         11.40         15.10         1.18         3.03           B13         0.72         0.10         0.62         87%         0.69         0.77         0.81         0.90         5         6.23         9.29         11.40         15.10         3.07         5.08           B14         0.90         0.18         0.73         80%         0.66         0.74         0.78         0.88			1	1											-		4.44	6.53
B10         0.07         0.00         0.07         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         0.33         0.54           B11         0.52         0.11         0.41         79%         0.65         0.73         0.78         0.86         5         6.23         9.29         11.40         15.10         2.12         3.53           B12         0.46         0.12         0.34         75%         0.63         0.71         0.76         0.84         5         6.23         9.29         11.40         15.10         1.82         3.03           B13         0.72         0.10         0.62         87%         0.69         0.77         0.81         0.90         5         6.23         9.29         11.40         15.10         3.07         5.08           B14         0.90         0.18         0.73         80%         0.66         0.74         0.78         0.87         5         6.23         9.29         11.40         15.10         3.86         6.42           B15         0.93         0.17         0.76         81%         0.52         0.60         16         4.04																	4.53	6.66
B11         0.52         0.11         0.41         79%         0.65         0.73         0.78         0.86         5         6.23         9.29         11.40         15.10         2.12         3.53           B12         0.46         0.12         0.34         75%         0.63         0.71         0.76         0.84         5         6.23         9.29         11.40         15.10         1.82         3.03           B13         0.72         0.10         0.62         87%         0.69         0.77         0.81         0.90         5         6.23         9.29         11.40         15.10         3.07         5.08           B14         0.90         0.18         0.73         80%         0.66         0.74         0.78         0.87         5         6.23         9.29         11.40         15.10         3.07         5.08           B15         0.93         0.17         0.76         81%         0.66         0.74         0.79         0.88         5         6.23         9.29         11.40         15.10         3.71         6.18           B16         3.23         2.35         0.88         2.7%         0.42         0.48         0.52         0.60 <td>1</td> <td></td> <td>0.13</td> <td></td> <td></td> <td></td> <td>0.74</td> <td>0.79</td> <td></td> <td>5</td> <td>6.23</td> <td></td> <td>11.40</td> <td>15.10</td> <td>2.95</td> <td>4.90</td> <td>6.40</td> <td>9.41</td>	1		0.13				0.74	0.79		5	6.23		11.40	15.10	2.95	4.90	6.40	9.41
B12         0.46         0.12         0.34         75%         0.63         0.71         0.76         0.84         5         6.23         9.29         11.40         15.10         1.82         3.03           B13         0.72         0.10         0.62         87%         0.69         0.77         0.81         0.90         5         6.23         9.29         11.40         15.10         3.07         5.08           B14         0.90         0.18         0.73         80%         0.66         0.74         0.78         0.87         5         6.23         9.29         11.40         15.10         3.71         6.18           B15         0.93         0.17         0.76         81%         0.66         0.74         0.79         0.88         5         6.23         9.29         11.40         15.10         3.86         6.42           B16         3.23         2.35         0.88         27%         0.42         0.48         0.52         0.60         16         4.04         5.99         7.35         9.68         5.42         9.30           B17         0.58         0.10         0.48         82%         0.67         0.75         0.79         0.88	B10	0.07	0.00	0.07	100%	0.75	0.83	0.88	0.97	5	6.23	9.29	11.40	15.10	0.33	0.54	0.70	1.03
B13         0.72         0.10         0.62         87%         0.69         0.77         0.81         0.90         5         6.23         9.29         11.40         15.10         3.07         5.08           B14         0.90         0.18         0.73         80%         0.66         0.74         0.78         0.87         5         6.23         9.29         11.40         15.10         3.71         6.18           B15         0.93         0.17         0.76         81%         0.66         0.74         0.79         0.88         5         6.23         9.29         11.40         15.10         3.86         6.42           B16         3.23         2.35         0.88         27%         0.42         0.48         0.52         0.60         16         4.04         5.99         7.35         9.68         5.42         9.30           B17         0.58         0.10         0.48         82%         0.67         0.75         0.79         0.88         5         6.23         9.29         11.40         15.10         2.43         4.03           B18         0.67         0.67         0.00         0%         0.29         0.35         0.39         0.46	B11	0.52	0.11	0.41	79%	0.65	0.73	0.78	0.86	5	6.23	9.29	11.40	15.10	2.12	3.53	4.61	6.79
B14         0.90         0.18         0.73         80%         0.66         0.74         0.78         0.87         5         6.23         9.29         11.40         15.10         3.71         6.18           B15         0.93         0.17         0.76         81%         0.66         0.74         0.79         0.88         5         6.23         9.29         11.40         15.10         3.86         6.42           B16         3.23         2.35         0.88         2.7%         0.42         0.48         0.52         0.60         16         4.04         5.99         7.35         9.68         5.42         9.30           B17         0.58         0.10         0.48         82%         0.67         0.75         0.79         0.88         5         6.23         9.29         11.40         15.10         2.43         4.03           B18         0.67         0.67         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         1.22         2.19           B19         2.34         1.41         0.93         40%         0.47         0.54         0.58         0.66	B12	0.46	0.12	0.34	75%	0.63	0.71	0.76	0.84	5	6.23	9.29	11.40	15.10	1.82	3.03	3.97	5.85
B15         0.93         0.17         0.76         81%         0.66         0.74         0.79         0.88         5         6.23         9.29         11.40         15.10         3.86         6.42           B16         3.23         2.35         0.88         27%         0.42         0.48         0.52         0.60         16         4.04         5.99         7.35         9.68         5.42         9.30           B17         0.58         0.10         0.48         82%         0.67         0.75         0.79         0.88         5         6.23         9.29         11.40         15.10         2.43         4.03           B18         0.67         0.67         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         1.22         2.19           B19         2.34         1.41         0.93         40%         0.47         0.54         0.58         0.66         13         4.44         6.61         8.13         10.70         4.91         8.36           B20         2.20         1.51         0.70         32%         0.44         0.50         0.55         0.62	B13	0.72	0.10	0.62	87%	0.69	0.77	0.81	0.90	5	6.23	9.29	11.40	15.10	3.07	5.08	6.64	9.73
B15         0.93         0.17         0.76         81%         0.66         0.74         0.79         0.88         5         6.23         9.29         11.40         15.10         3.86         6.42           B16         3.23         2.35         0.88         27%         0.42         0.48         0.52         0.60         16         4.04         5.99         7.35         9.68         5.42         9.30           B17         0.58         0.10         0.48         82%         0.67         0.75         0.79         0.88         5         6.23         9.29         11.40         15.10         2.43         4.03           B18         0.67         0.67         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         1.22         2.19           B19         2.34         1.41         0.93         40%         0.47         0.54         0.58         0.66         13         4.44         6.61         8.13         10.70         4.91         8.36           B20         2.20         1.51         0.70         32%         0.44         0.50         0.55         0.62				1								<b>+</b>					8.07	11.87
B16         3.23         2.35         0.88         27%         0.42         0.48         0.52         0.60         16         4.04         5.99         7.35         9.68         5.42         9.30           B17         0.58         0.10         0.48         82%         0.67         0.75         0.79         0.88         5         6.23         9.29         11.40         15.10         2.43         4.03           B18         0.67         0.67         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         1.22         2.19           B19         2.34         1.41         0.93         40%         0.47         0.54         0.58         0.66         13         4.44         6.61         8.13         10.70         4.91         8.36           B20         2.20         1.51         0.70         32%         0.44         0.50         0.55         0.62         16         4.04         5.99         7.35         9.68         3.88         6.63           B21         2.00         1.23         0.77         39%         0.47         0.54         0.58         0.66	- 1		<u> </u>	<u> </u>								+					8.39	12.32
B17         0.58         0.10         0.48         82%         0.67         0.75         0.79         0.88         5         6.23         9.29         11.40         15.10         2.43         4.03           B18         0.67         0.67         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         1.22         2.19           B19         2.34         1.41         0.93         40%         0.47         0.54         0.58         0.66         13         4.44         6.61         8.13         10.70         4.91         8.36           B20         2.20         1.51         0.70         32%         0.44         0.50         0.55         0.62         16         4.04         5.99         7.35         9.68         3.88         6.63           B21         2.00         1.23         0.77         39%         0.47         0.54         0.58         0.66         17         3.92         5.82         7.14         9.39         3.67         6.23           B22         0.11         0.08         0.03         31%         0.43         0.50         0.54         0.62			1														12.43	18.72
B18         0.67         0.67         0.00         0%         0.29         0.35         0.39         0.46         5         6.23         9.29         11.40         15.10         1.22         2.19           B19         2.34         1.41         0.93         40%         0.47         0.54         0.58         0.66         13         4.44         6.61         8.13         10.70         4.91         8.36           B20         2.20         1.51         0.70         32%         0.44         0.50         0.55         0.62         16         4.04         5.99         7.35         9.68         3.88         6.63           B21         2.00         1.23         0.77         39%         0.47         0.54         0.58         0.66         17         3.92         5.82         7.14         9.39         3.67         6.23           B22         0.11         0.08         0.03         31%         0.43         0.50         0.54         0.62         5         6.23         9.29         11.40         15.10         0.30         0.52           C1         2.03         1.67         0.36         18%         0.37         0.44         0.48         0.55	t t																5.27	7.74
B19         2.34         1.41         0.93         40%         0.47         0.54         0.58         0.66         13         4.44         6.61         8.13         10.70         4.91         8.36           B20         2.20         1.51         0.70         32%         0.44         0.50         0.55         0.62         16         4.04         5.99         7.35         9.68         3.88         6.63           B21         2.00         1.23         0.77         39%         0.47         0.54         0.58         0.66         17         3.92         5.82         7.14         9.39         3.67         6.23           B22         0.11         0.08         0.03         31%         0.43         0.50         0.54         0.62         5         6.23         9.29         11.40         15.10         0.30         0.52           C1         2.03         1.67         0.36         18%         0.37         0.44         0.48         0.55         5         6.23         9.29         11.40         15.10         4.71         8.22           C2         1.49         1.49         0.00         0%         0.29         0.35         0.39         0.46																		
B20         2.20         1.51         0.70         32%         0.44         0.50         0.55         0.62         16         4.04         5.99         7.35         9.68         3.88         6.63           B21         2.00         1.23         0.77         39%         0.47         0.54         0.58         0.66         17         3.92         5.82         7.14         9.39         3.67         6.23           B22         0.11         0.08         0.03         31%         0.43         0.50         0.54         0.62         5         6.23         9.29         11.40         15.10         0.30         0.52           C1         2.03         1.67         0.36         18%         0.37         0.44         0.48         0.55         5         6.23         9.29         11.40         15.10         4.71         8.22           C2         1.49         1.49         0.00         0%         0.29         0.35         0.39         0.46         6         5.90         8.81         10.80         14.40         2.54         4.58           R1         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97	-																2.99	4.67
B21         2.00         1.23         0.77         39%         0.47         0.54         0.58         0.66         17         3.92         5.82         7.14         9.39         3.67         6.23           B22         0.11         0.08         0.03         31%         0.43         0.50         0.54         0.62         5         6.23         9.29         11.40         15.10         0.30         0.52           C1         2.03         1.67         0.36         18%         0.37         0.44         0.48         0.55         5         6.23         9.29         11.40         15.10         4.71         8.22           C2         1.49         1.49         0.00         0%         0.29         0.35         0.39         0.46         6         5.90         8.81         10.80         14.40         2.54         4.58           R1         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.36         2.24           R2         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97											10 AL 10	<u> </u>					11.12	16.58
B22         0.11         0.08         0.03         31%         0.43         0.50         0.54         0.62         5         6.23         9.29         11.40         15.10         0.30         0.52           C1         2.03         1.67         0.36         18%         0.37         0.44         0.48         0.55         5         6.23         9.29         11.40         15.10         4.71         8.22           C2         1.49         1.49         0.00         0%         0.29         0.35         0.39         0.46         6         5.90         8.81         10.80         14.40         2.54         4.58           R1         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.37         2.26           R2         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.36         2.24           R3         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97			<del>                                     </del>	1								1					8.83	13.26
C1         2.03         1.67         0.36         18%         0.37         0.44         0.48         0.55         5         6.23         9.29         11.40         15.10         4.71         8.22           C2         1.49         1.49         0.00         0%         0.29         0.35         0.39         0.46         6         5.90         8.81         10.80         14.40         2.54         4.58           R1         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.37         2.26           R2         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.36         2.24           R3         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.25         7.01           R4         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97	-		<del>                                     </del>	+						17		<del>                                     </del>					8.27	12.33
C2         1.49         1.49         0.00         0%         0.29         0.35         0.39         0.46         6         5.90         8.81         10.80         14.40         2.54         4.58           R1         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.37         2.26           R2         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.36         2.24           R3         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.25         7.01           R4         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.24         7.00	B22	0.11	0.08		31%	0.43	0.50	0.54	0.62	5	6.23	9.29	11.40	15.10	0.30	0.52	0.69	1.04
R1         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.37         2.26           R2         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.36         2.24           R3         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.25         7.01           R4         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.25         7.01           R4         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.24         7.00	C1	2.03	1.67	0.36	18%	0.37	0.44	0.48	0.55	5	6.23	9.29	11.40	15.10	4.71	8.22	11.05	16.89
R1         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.37         2.26           R2         0.29         0.00         0.29         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         1.36         2.24           R3         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.25         7.01           R4         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.25         7.01           R4         0.91         0.00         0.91         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.24         7.00	C2	1.49	1.49	0.00	0%	0.29	0.35	0.39	0.46	6	5.90	8.81	10.80	14.40	2.54	4.58	6.26	9.85
R2     0.29     0.00     0.29     100%     0.75     0.83     0.88     0.97     5     6.23     9.29     11.40     15.10     1.36     2.24       R3     0.91     0.00     0.91     100%     0.75     0.83     0.88     0.97     5     6.23     9.29     11.40     15.10     4.25     7.01       R4     0.91     0.00     0.91     100%     0.75     0.83     0.88     0.97     5     6.23     9.29     11.40     15.10     4.24     7.00			<u> </u>							5							2.94	4.29
R3 0.91 0.00 0.91 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 4.25 7.01 R4 0.91 0.00 0.91 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 4.24 7.00			<u> </u>	1								-					2.92	4.26
R4 0.91 0.00 0.91 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 4.24 7.00	-		1	<b>+</b>													9.12	13.31
			<u> </u>	1								<u> </u>						1
I DE 1 101   0.00   1.01   1.000   0.70   0.00   0.00   0.07   E 1 0.00   0.00   4440   4640   4.74   3.77			<del> </del>	1					1			<del> </del>					9.11	13.30
	R5	1.01	0.00	1.01	100%	0.75	0.83	0.88	0.97	5	6.23	9.29	11.40	15.10	4.71	7.77	10.11	14.76
R6         1.01         0.00         1.01         100%         0.75         0.83         0.88         0.97         5         6.23         9.29         11.40         15.10         4.70         7.76	-						0.83	0.88		5			11.40	15.10	4.70	7.76	10.09	14.74
R7 0.95 0.00 0.95 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 4.42 7.30	R7	0.95	0.00	0.95	100%	0.75	0.83	0.88	0.97	5	6.23	9.29	11.40	15.10	4.42	7.30	9.50	13.86
R8 0.95 0.00 0.95 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 4.42 7.29	R8	0.95	0.00	0.95	100%	0.75	0.83	0.88	0.97	5	6.23	9.29	11.40	15.10	4.42	7.29	9.49	13.85
R9 0.27 0.00 0.27 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 1.28 2.11	R9	0.27	0.00	0.27	100%	0.75	0.83	0.88	0.97	5	6.23	9.29	11.40	15.10	1.28	2.11	2.74	4.00
R10 0.04 0.00 0.04 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 0.17 0.28			1	1													0.36	0.53
R11 0.14 0.00 0.14 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 0.67 1.10	1											+					1.44	2.10
				1														0.17
R12 0.01 0.00 0.01 100% 0.75 0.83 0.88 0.97 5 6.23 9.29 11.40 15.10 0.05 0.09	UTZ	U.UI	U.00	U.U1	100%	U./3	0.83	0.00	0.97	Э	0.25	3.23	11.40	13.10	0.05	0.09	0.12	0.1/

						Kimle	ey»}	lorn							
			Vari	able Definit	tions						Constant Va	alues			
W	depressed	width			$E_{O}$	ratio of flow	v (Q <sub>w</sub> /Q)				C <sub>w</sub> (constan	t-Weir)		2.3	
а	gutter depr	ession			$L_{T}$	inlet length	(100% flow	intercept)			K <sub>u</sub> (constant	t)		0.56	
S <sub>e</sub>	equivalent	cross slope			d <sub>o</sub>	effective he	ead, orifice				K <sub>t</sub> (constant	:)		0.6	
S <sub>W</sub> cross slope of depressed width d <sub>i</sub>							of curb ope	ning			C <sub>o</sub> (constant	t-Orifice)		0.67	
Т	spread or po	onded width	1		d	depth at no	rmal cross-s	lope			S <sub>x</sub> (cross-slo	pe)		2.00%	
$S_L$	longitudina	l slope									Manning's (	Coefficient (	n)	0.013	
											Maximum S	pread T (ft)		12	
					0	N-GRADE	<b>CURB INI</b>	ETS (25-Y	R)						
	Inle	t Characteri	stics			St	treet Draina	ge			l	nlet Drainag	ge		
Q-25	Q <sub>pass</sub>	Q <sub>total</sub>	Inlet	W	а	S.	d	Т	S	F.	S	L <sub>T</sub>	Efficiency	Q <sub>intercept</sub>	$Q_{pass}$
(cfs)	(cfs)	(cfs)	Length	(ft)	(in)	٦,	(in)	(ft)	3 <sub>W</sub>	-0	Je	(ft)	Littleffley	(cfs)	(cfs)
2.16	0.06	2.21	5	1.5	7	1.00%	2.16	8.98	40.89%	0.79	34.3%	5.42	99.0%	2.19	0.02
2.44	0.00	2.44	5	1.5	7	1.00%	2.24	9.32	40.89%	0.77	33.6%	5.71	97.6%	2.38	0.06
1.70	0.00	1.70	5	1.5	7	1.00%	1.95	8.14	40.89%	0.83	35.9%	4.71	100.0%	1.70	0.00
10.12	0.00	10.12	10	1.5	7	1.00%	3.81	15.88	40.89%	0.50	22.3%	13.29	91.9%	9.30	0.82
8.39	0.00	8.39	15	1.5	7	1.00%	3.55	14.80	40.89%	0.53	23.8%	11.80	100.0%	8.39	0.00
-	Q-25 (cfs) 2.16 2.44 1.70 10.12	a gutter depression of the series of the ser	a gutter depression  Se equivalent cross slope  Sw cross slope of depresse  T spread or ponded width  SL longitudinal slope  Inlet Characteri  Q-25 Q <sub>pass</sub> Q <sub>total</sub> (cfs) (cfs) (cfs)  2.16 0.06 2.21  2.44 0.00 2.44  1.70 0.00 1.70  10.12 0.00 10.12	W   depressed width	W         depressed width           a         gutter depression           Se         equivalent cross slope           SW         cross slope of depressed width           T         spread or ponded width           SL         longitudinal slope           Inlet Characteristics           Q-25         Qpass         Qtotal         Inlet         W           (cfs)         (cfs)         Length         (ft)           2.16         0.06         2.21         5         1.5           2.44         0.00         2.44         5         1.5           1.70         0.00         1.70         5         1.5           10.12         0.00         10.12         10         1.5	Variable Definitions   W   depressed width   E <sub>O</sub>   a   gutter depression   L <sub>T</sub>   S <sub>e</sub>   equivalent cross slope   d <sub>o</sub>   S <sub>W</sub>   cross slope of depressed width   d <sub>i</sub>   T   spread or ponded width   d   S <sub>L</sub>   longitudinal slope	Variable Definitions           W         depressed width         E <sub>O</sub> ratio of flow inlet length           S <sub>e</sub> equivalent cross slope         d <sub>o</sub> effective he depressed width           S <sub>W</sub> cross slope of depressed width         d         depth at lip depth at lip depth at not depth de	Variable Definitions           W         depressed width         E <sub>O</sub> ratio of flow (Q <sub>w</sub> /Q)           a         gutter depression         L <sub>T</sub> inlet length (100% flow           S <sub>e</sub> equivalent cross slope         d <sub>o</sub> effective head, orifice           S <sub>W</sub> cross slope of depressed width         d <sub>i</sub> depth at lip of curb ope           T         spread or ponded width         d depth at normal cross-s           S <sub>L</sub> longitudinal slope           ON-GRADE CURB INI           Inlet Characteristics         Street Drainal           Q-25         Q <sub>pass</sub> Q <sub>total</sub> Inlet W a S <sub>L</sub> (in)         S <sub>L</sub> (in)           (cfs)         (cfs)         (cfs)         Length (ft)         (in)           2.16         0.06         2.21         5         1.5         7         1.00%         2.16           2.44         0.00         2.44         5         1.5         7         1.00%         2.24           1.70         0.00         1.70         5         1.5         7         1.00%         1.95           10.12         0.00         10.12         10         1.5         7         1.00%         3.81	W         depressed width a gutter depression         E <sub>O</sub> ratio of flow (Q <sub>w</sub> /Q)           Se         equivalent cross slope         do         effective head, orifice           S <sub>w</sub> cross slope of depressed width         di         depth at lip of curb opening           T         spread or ponded width         d         depth at normal cross-slope           SL         Inlet Characteristics         Street Drainage           Q-25         Q <sub>pass</sub> Q <sub>total</sub> Inlet         W         a         S <sub>L</sub> d         T           (cfs)         (cfs)         (cfs)         Length         (ft)         (in)         (in)         (ft)           2.16         0.06         2.21         5         1.5         7         1.00%         2.16         8.98           2.44         0.00         2.44         5         1.5         7         1.00%         2.24         9.32           1.70         0.00         1.70         5         1.5         7         1.00%         3.81         15.88	Variable Definitions           W         depressed width a gutter depression         E <sub>O</sub> ratio of flow (Q <sub>w</sub> /Q) inlet length (100% flow intercept)           Se equivalent cross slope         do effective head, orifice           Sw cross slope of depressed width         di depth at lip of curb opening           T spread or ponded width         d depth at normal cross-slope           ON-GRADE CURB INLETS (25-YR)           Inlet Characteristics         Street Drainage           Q-25 (cfs) (cfs	Variable Definitions           W         depressed width         E <sub>O</sub> ratio of flow (Q <sub>w</sub> /Q)           a         gutter depression         L <sub>T</sub> inlet length (100% flow intercept)           S <sub>e</sub> equivalent cross slope         d <sub>o</sub> effective head, orifice           S <sub>W</sub> cross slope of depressed width         d <sub>i</sub> depth at lip of curb opening           T         spread or ponded width         d         depth at normal cross-slope           S <sub>L</sub> longitudinal slope         longitudinal slope    **ON-GRADE CURB INLETS (25-YR)  **Street Drainage**    Q-25   Q <sub>pass</sub>   Q <sub>total</sub>   Inlet   W   a   S <sub>L</sub>   d   T   S <sub>W</sub>   E <sub>O</sub>	Variable Definitions   Constant Variable Definition   Constant Variable Definition	Variable Definitions   Constant Values	Variable Definitions   Constant Values	Variable Definitions   Constant Values

B16 12.43 0.00 12.43 20 1.5 7 1.00% 4.12 17.15 40.89% 0.46 20.6% 15.16 100.0% 12.43 0.00

		Site Curl	b Inlet Ca	alculation <sup>-</sup>	Table			
Weir Equation:	$Q=C_WLh^{1.5}(1-C_f)$	Weir Coef C <sub>w</sub> :	3.0	h (ft):	0.50	Cloggir	ng Factor C <sub>f</sub> :	10%
Drainage Area	Q <sub>25</sub> (cfs)	Q <sub>100</sub> (cfs)	h (ft)	L <sub>Req.</sub> (ft)	Inlet Selected	Weir Length (ft.)	Inlet Capacity	Q25 Pass? (Y/N)
A9	1.03 cfs	1.52 cfs	0.50'	1.1'	5' CI	5'	4.77 cfs	YES
A10	3.91 cfs	5.71 cfs	0.50'	4.1'	5' CI	5'	4.77 cfs	YES
A11	4.34 cfs	6.35 cfs	0.50'	4.5'	5' CI	5'	4.77 cfs	YES
A12	9.60 cfs	14.03 cfs	0.50'	10.1'	15' CI	15'	14.32 cfs	YES
A13	9.10 cfs	13.28 cfs	0.50'	9.5'	10' CI	10'	9.55 cfs	YES
A14	7.48 cfs	10.94 cfs	0.50'	7.8'	10' CI	10'	9.55 cfs	YES
A20	4.66 cfs	6.90 cfs	0.50'	4.9'	10' CI	10'	9.55 cfs	YES
A21	8.30 cfs	12.25 cfs	0.50'	8.7'	10' CI	10'	9.55 cfs	YES
A22	2.49 cfs	3.76 cfs	0.50'	2.6'	5' CI	5'	4.77 cfs	YES
A29	3.71 cfs	5.47 cfs	0.50'	3.9'	10' CI	10'	9.55 cfs	YES
B1	13.04 cfs	19.20 cfs	0.50'	13.7'	15' CI	15'	14.32 cfs	YES
В3	10.09 cfs	17.12 cfs	0.50'	10.6'	15' CI	15'	14.32 cfs	YES
B5	7.05 cfs	10.54 cfs	0.50'	7.4'	15' CI	15'	14.32 cfs	YES
B12	3.97 cfs	5.85 cfs	0.50'	4.2'	5' CI	5'	4.77 cfs	YES
B13	6.64 cfs	9.73 cfs	0.50'	7.0'	10' CI	10'	9.55 cfs	YES
B14	8.07 cfs	11.87 cfs	0.50'	8.5'	10' CI	10'	9.55 cfs	YES
B17	5.27 cfs	7.74 cfs	0.50'	5.5'	10' CI	10'	9.55 cfs	YES
B19	11.12 cfs	16.58 cfs	0.50'	11.6'	15' CI	15'	14.32 cfs	YES
B20	8.83 cfs	13.26 cfs	0.50'	9.3'	15' CI	15'	14.32 cfs	YES

			Area	a Inlet Cald	culation T	able				
Weir Equation:	$Q=C_WLh^{1.5}(1-C_f)$	Weir Coef C <sub>w</sub> :	3.0	h (ft):	Varies	Cloggi	ng Factor C <sub>f</sub> :	50%	Note: Both	Weirand
Orifice Equation:	$Q=C_0h_TL(2gd_0)^{0.5}$	(1-C <sub>f</sub> )	Orif	ice Coef C <sub>0</sub> :	0.67	Inlet T	hroat h <sub>⊤</sub> (ft):	0.50	Orifice Flow	
	onstant g (ft/s²):	32.174	Effective H	Effective Head d <sub>0</sub> : Varies [Orifice Head only applicable where h/h <sub>T</sub> >1.						ed h creates condition.
Drainage Area	Q <sub>25</sub> (cfs)	Q <sub>100</sub> (cfs)	h (ft)	L <sub>Req.</sub> (ft) [Weir]	L <sub>Req.</sub> (ft) [Orifice]	L <sub>Req.</sub> (ft)	Inlet Selected	Weir Length (ft.)	Inlet Capacity	
A2	4.82 cfs	7.09 cfs	1.50'	1.7'	3.2'	3.2'	2' X 2'	8'	12.02 cfs	YES
А3	3.73 cfs	5.49 cfs	1.50'	1.4'	2.5'	2.5'	2' X 2'	8'	12.02 cfs	YES
A4	4.57 cfs	6.73 cfs	1.50'	1.7'	3.0'	3.0'	2' X 2'	8'	12.02 cfs	YES
A5	3.70 cfs	5.46 cfs	1.50'	1.3'	2.5'	2.5'	2' X 2'	8'	12.02 cfs	YES
A6	4.16 cfs	6.13 cfs	1.50'	1.5'	2.8'	2.8'	2' X 2'	8'	12.02 cfs	YES
A7	3.92 cfs	5.78 cfs	1.50'	1.4'	2.6'	2.6'	2' X 2'	8'	12.02 cfs	YES
A8	2.75 cfs	4.07 cfs	1.50'	1.0'	1.8'	1.8'	2' X 2'	8'	12.02 cfs	YES
A23	3.02 cfs	4.78 cfs	1.50'	1.1'	2.0'	2.0'	2' X 2'	8'	12.02 cfs	YES
A30	1.33 cfs	2.08 cfs	1.50'	0.5'	0.9'	0.9'	2' X 2'	8'	12.02 cfs	YES
B2	5.96 cfs	8.78 cfs	1.50'	2.2'	4.0'	4.0'	2' X 2'	8'	12.02 cfs	YES
В6	4.64 cfs	6.83 cfs	1.50'	1.7'	3.1'	3.1'	2' X 2'	8'	12.02 cfs	YES
В7	4.44 cfs	6.53 cfs	1.50'	1.6'	3.0'	3.0'	2' X 2'	8'	12.02 cfs	YES
B8	4.53 cfs	6.66 cfs	1.50'	1.6'	3.0'	3.0'	2' X 2'	8'	12.02 cfs	YES
В9	6.40 cfs	9.41 cfs	1.50'	2.3'	4.3'	4.3'	3' X 3'	12'	18.03 cfs	YES
B11	4.61 cfs	6.79 cfs	1.50'	1.7'	3.1'	3.1'	3' X 3'	12'	18.03 cfs	YES
B18	2.99 cfs	4.67 cfs	1.50'	1.1'	2.0'	2.0'	2' X 2'	8'	12.02 cfs	YES

	Grate Inlet Calculation Table												
rifice Equation:	Q=C <sub>O</sub> A(2gh) <sup>0.5</sup> (1	-C <sub>f</sub> )	Ori	fice Coef Co:	0.67	Cloggii	ng Factor C <sub>f</sub> :	50%					
Gravitational Co	onstant g (ft/s²):	32.174	h (ft):	Varies - Parl	king Areas T	yp. 0.5', Load	ing/Landsca	pe >0.5'					
Drainage Area	Q <sub>25</sub> (cfs)	Q <sub>100</sub> (cfs)	h (ft)	A <sub>Req.</sub> (S.F.)	Inlet Selected	Open Area (S.F.)	Inlet Capacity	Q25 Pass? (Y/N)					
A1	4.52 cfs	6.80 cfs	0.50'	2.38	3' X 3'	4.52	8.59 cfs	YES					
A15	8.13 cfs	11.88 cfs	0.50'	4.28	4' X 4'	5.69	10.82 cfs	YES					
A16	8.70 cfs	12.72 cfs	0.50'	4.58	4' X 4'	5.69	10.82 cfs	YES					
A17	9.76 cfs	14.28 cfs	0.50'	5.14	4' X 4'	5.69	10.82 cfs	YES					
A18	6.70 cfs	9.79 cfs	0.50'	3.53	3' X 3'	4.52	8.59 cfs	YES					
A19	10.01 cfs	14.65 cfs	0.50'	5.27	4' X 4'	5.69	10.82 cfs	YES					
A24	4.17 cfs	6.10 cfs	0.50'	2.19	4' X 4'	5.69	10.82 cfs	YES					
A25	4.83 cfs	7.08 cfs	0.50'	2.54	4' X 4'	5.69	10.82 cfs	YES					
B10	0.70 cfs	1.03 cfs	0.50'	0.37	2' X 2'	2.17	4.13 cfs	YES					

### CONTRACTOR FIELD SET

This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

### **BENCHMARKS**

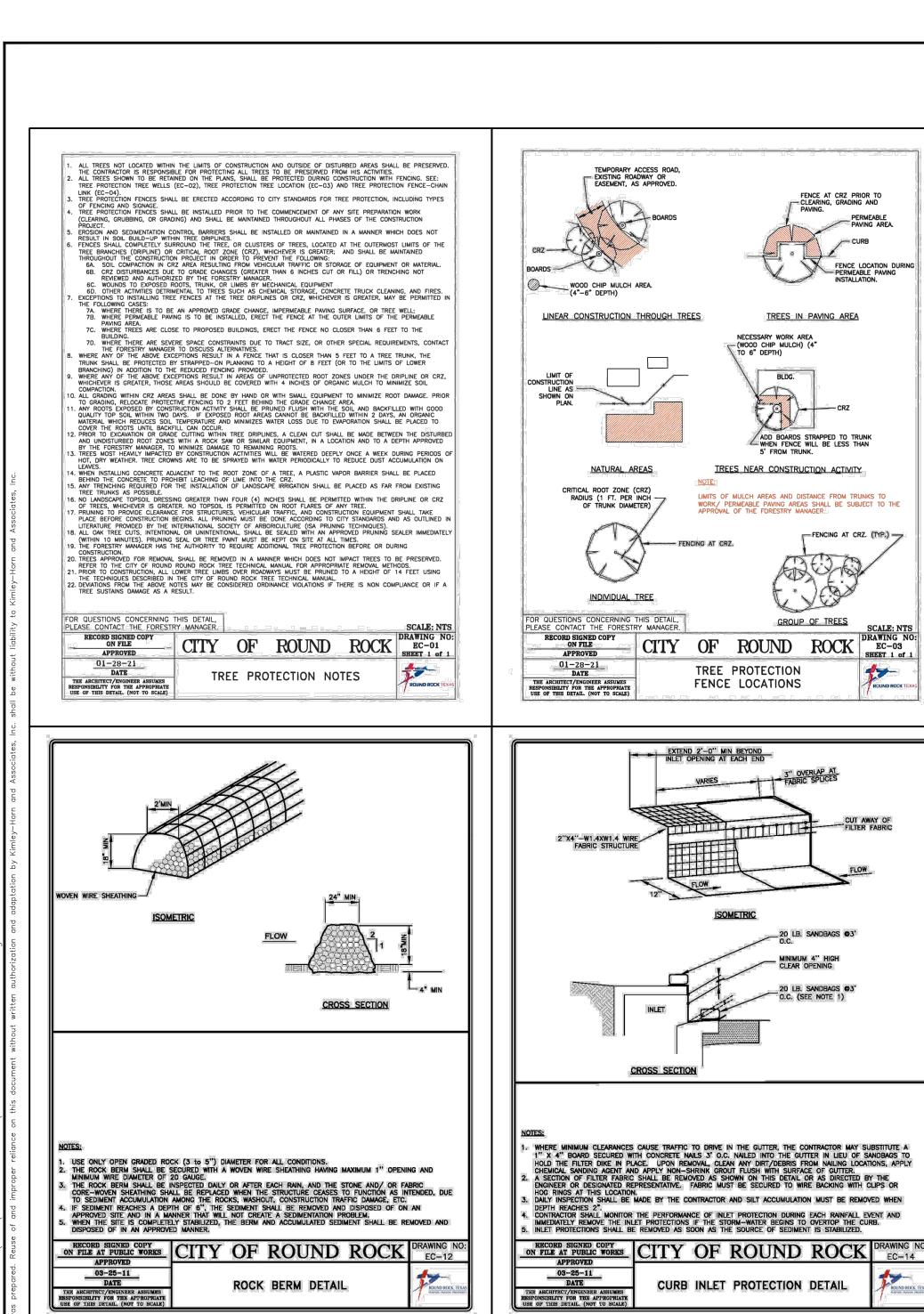
BM #51471 MAG NAIL SET "GCP 2" SET AT THE NORTHEAST CORNER OF THE INTERSECTION OF STATE HIGHWAY 45 AND COUNTY ROAD 172, ±5.5' FROM "YIELD" SIGN. N: 10147640.83 E: 3127942.60 ELEV.= 841.70'

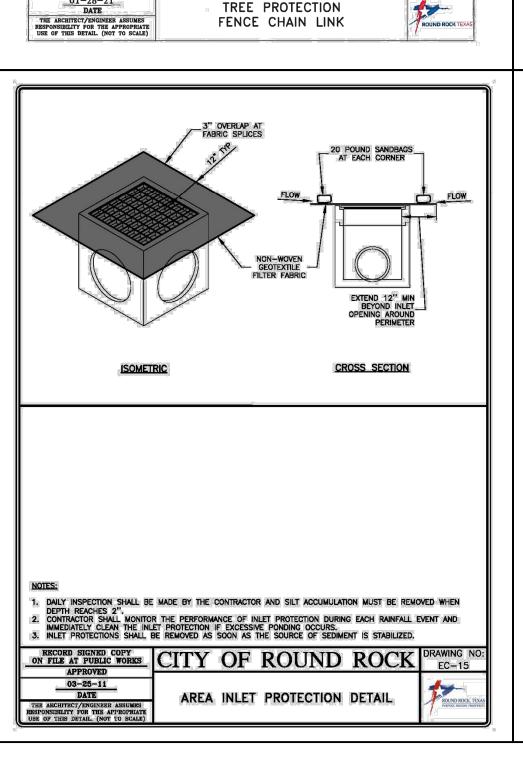
BM #51473 MAG NAIL SET "GCP 3" SET IN THE NORTH RIGHT-OF-WAY LINE OF HESTERS CROSSING ROAD, ±560' FROM THE INTERSECTION OF HESTERS CROSSING
ROAD AND COUNTY ROAD 172, ±100 FROM
CONCRETE SIGN "ENCOMPASS HEALTH". N: 10149343.04 E: 3128315.81 ELEV.= 812.19'

SHEET NUMBER

SDP24-00001

2525 INDU





TREE PROTECTION FENCE - CHAIN LINK

TREE PROTECTION FENCE (MODIFIED) - CHAIN LINK

WOOD CHIP MULCH AREA (4"-6" DEPTH)

SCALE: NTS

SCALE: NTS

OF ROUND ROCK

DRAWING NO:
EC-04
SHEET 1 of 1

10' MAX

(LIMITS OF CRITICAL ROOT ZONE)
RADIUS=1 FOOT PER INCH
OF TRUNK DIAMETER

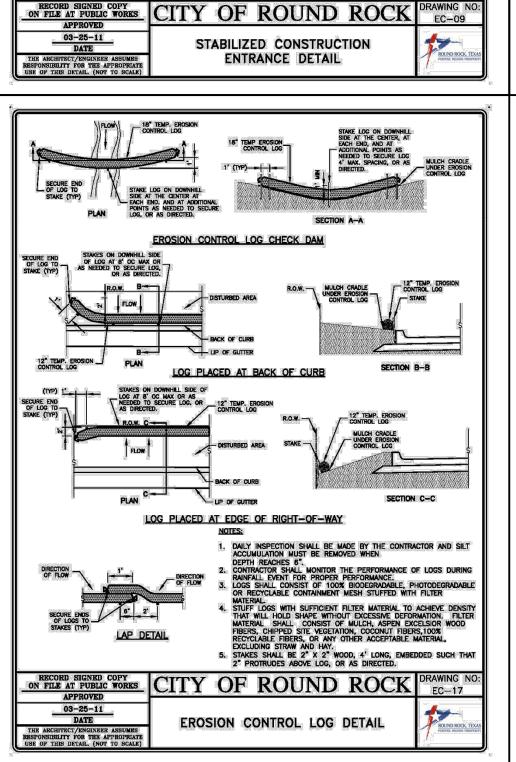
tensons from

\*AS NEEDED TO PROVIDE NECESSARY WORK SPACE. IF LESS THAN 5', THEN ADD BOARDS STRAPPED TO TRUNK.

FOR QUESTIONS CONCERNING THIS DETAIL, PLEASE CONTACT THE FORESTRY MANAGER.

APPROVED

CURB INLET PROTECTION DETAIL

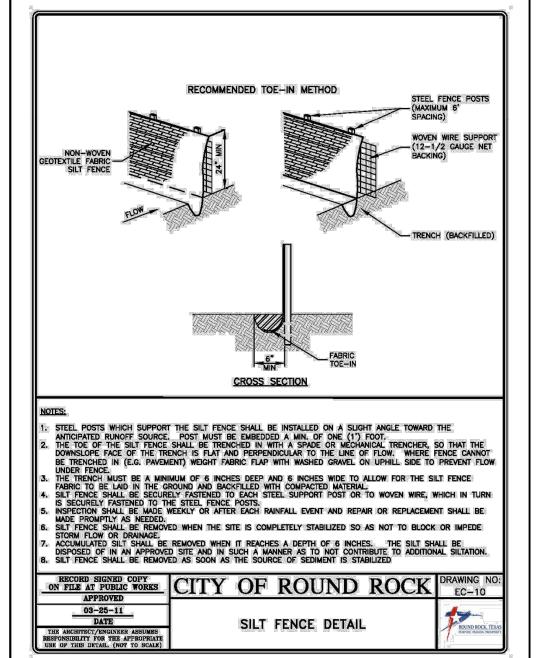


GRADE TO PREVENT

RUNOFF FROM LEAVING
SITE

CROSS SECTION

1. STONE SIZE SHALL BE 3" — 8" DPEN GRADED ROCK.
2. THICKNESS OF CRUSHED STONE PAD TO BE NOT LESS THAN 8".
3. LENGTH SHALL BE A MINIMUM OF 50' FROM ACTUAL ROADWAY, AND WIDTH NOT LESS THAN FULL WIDTH OF INGRESS/EGRESS.
4. ENTRANCE SHALL BE PROPERLY GRADED TO PREVENT RUNOFF FROM LEAVING THE CONSTRUCTION SITE.
5. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OF FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS OF WAY. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS OF WAY MUST BE REMOVED IMMEDIATELY BY CONTRACTOR.
6. AS NECESSARY, WHEELS MUST BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO PUBLIC RIGHT OF WAY, WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN, ALL SEDIMENT SHALL BE PREVENTED FROM ENTERING ANY STORM DRAIN, DITCH OR WATERCOURSE USING APPROVED METHODS.



CONTRACTOR FIELD SET

This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

## **BENCHMARKS**

BM #51471 MAG NAIL SET "GCP 2" SET AT THE NORTHEAST CORNER OF THE INTERSECTION OF STATE HIGHWAY 45 AND COUNTY ROAD 172, ±5.5' FROM "YIELD" SIGN. N: 10147640.83 E: 3127942.60 ELEV.= 841.70'

BM #51473 MAG NAIL SET "GCP 3" SET IN THE NORTH RIGHT-OF-WAY LINE OF HESTERS CROSSING ROAD, ±560' FROM THE INTERSECTION OF HESTERS CROSSING ROAD AND COUNTY ROAD 172, ±100 FROM CONCRETE SIGN "ENCOMPASS HEALTH". N: 10149343.04 E: 3128315.81 ELEV.= 812.19'

SHEET NUMBER 91 OF 227

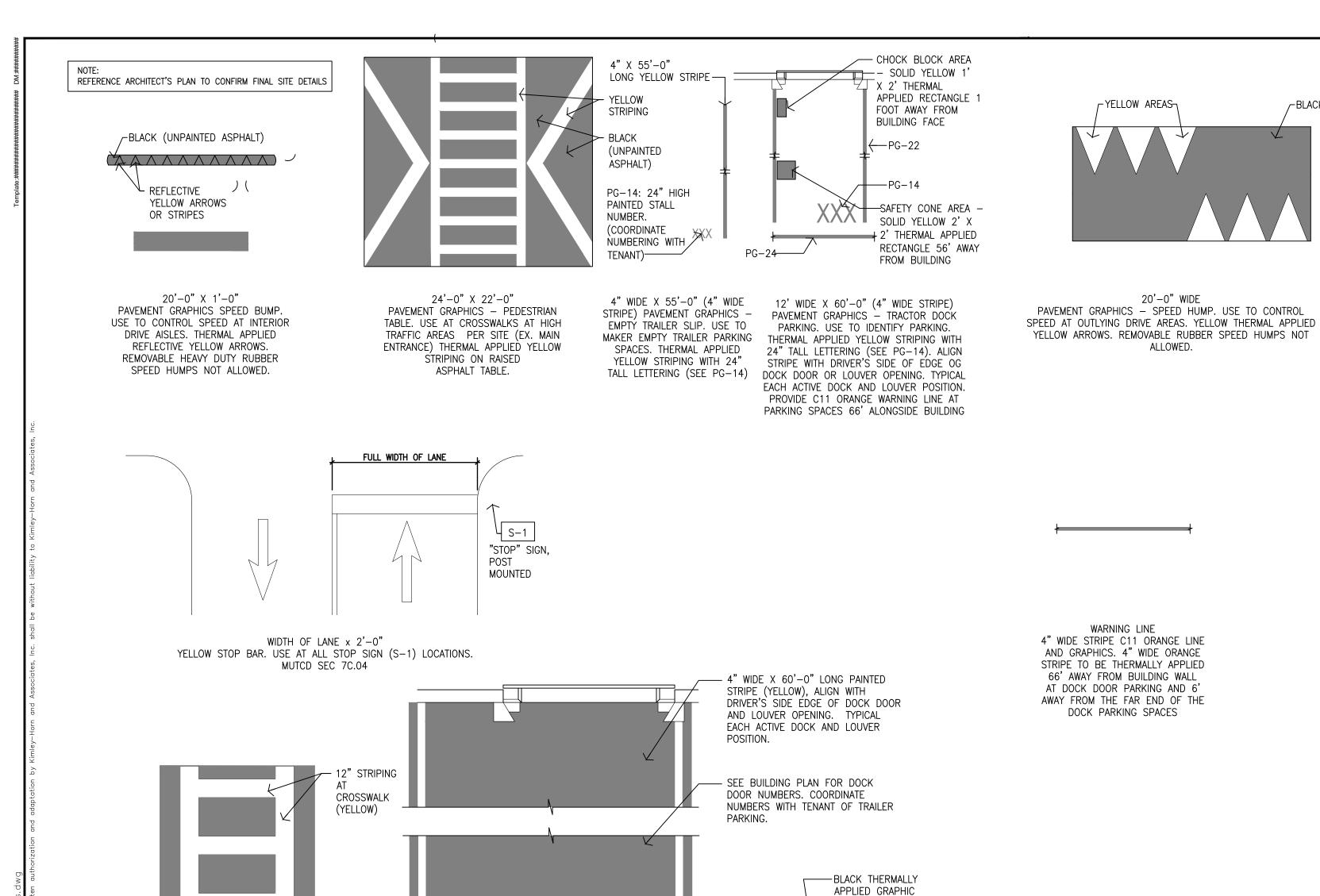
NDO

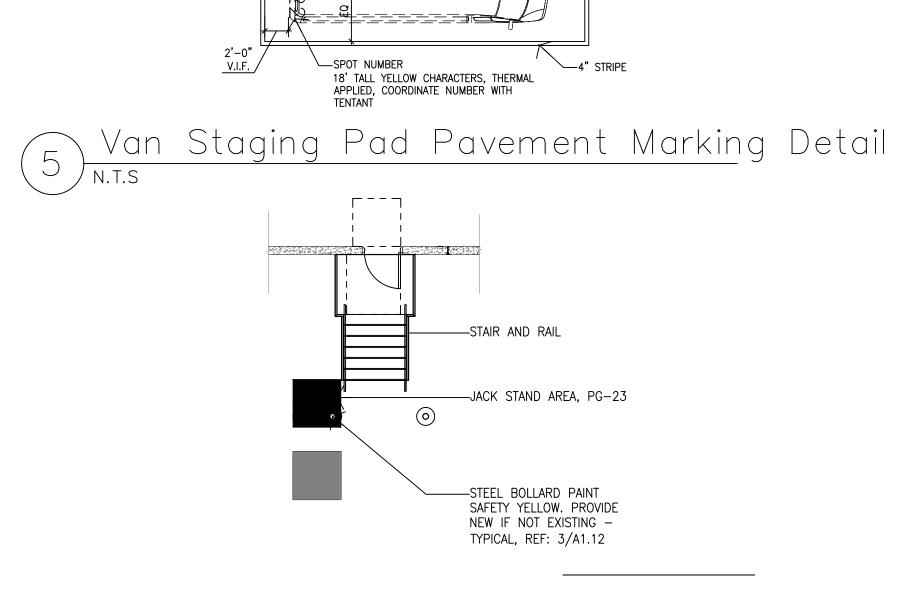
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03/26/2025





—LAUNCH PAD ID

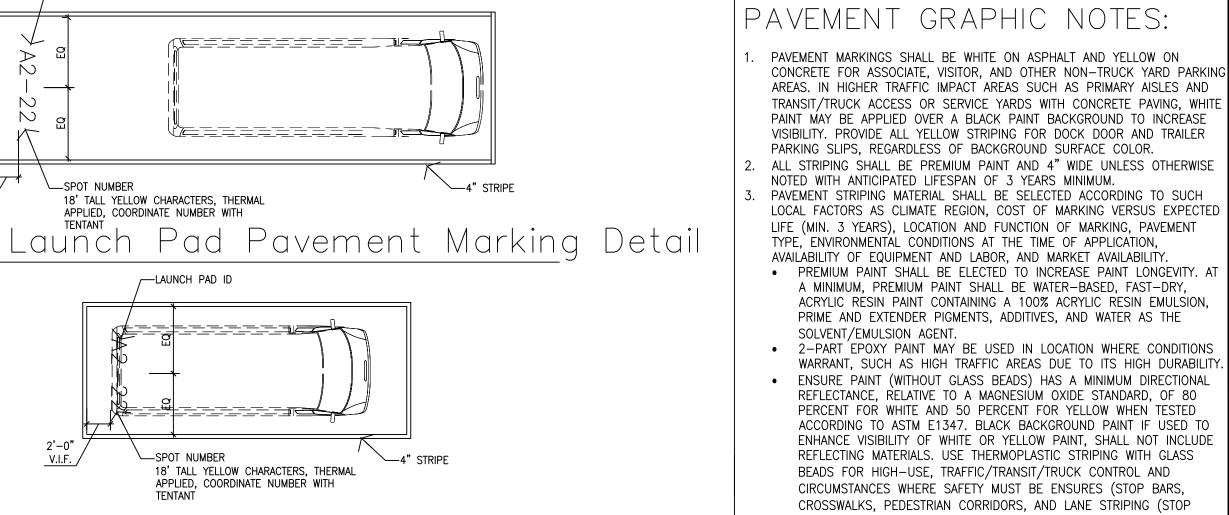
-SPOT NUMBER

18' TALL YELLOW CHARACTERS, THERMAL

—LAUNCH PAD ID

APPLIED, COORDINATE NUMBER WITH

/-BLACK



A MINIMUM, PREMIUM PAINT SHALL BE WATER-BASED, FAST-DRY, ACRYLIC RESIN PAINT CONTAINING A 100% ACRYLIC RESIN EMULSION, PRIME AND EXTENDER PIGMENTS, ADDITIVES, AND WATER AS THE SOLVENT/EMULSION AGENT. 2-PART EPOXY PAINT MAY BE USED IN LOCATION WHERE CONDITIONS WARRANT, SUCH AS HIGH TRAFFIC AREAS DUE TO ITS HIGH DURABILITY ENSURE PAINT (WITHOUT GLASS BEADS) HAS A MINIMUM DIRECTIONAL REFLECTANCE, RELATIVE TO A MAGNESIÚM OXIDE STANDARD, OF 80 PERCENT FOR WHITE AND 50 PERCENT FOR YELLOW WHEN TESTED ACCORDING TO ASTM E1347. BLACK BACKGROUND PAINT IF USED TO ENHANCE VISIBILITY OF WHITE OR YELLOW PAINT, SHALL NOT INCLUDE REFLECTING MATERIALS. USE THERMOPLASTIC STRIPING WITH GLASS BEADS FOR HIGH-USE, TRAFFIC/TRANSIT/TRUCK CONTROL AND CIRCUMSTANCES WHERE SAFETY MUST BE ENSURES (STOP BARS, CROSSWALKS. PEDESTRIAN CORRIDORS. AND LANE STRIPING (STOP BARS, CROSSWALKS, PEDESTRIAN CORRIDORS, AND LANE STRIPING

(CENTER LINES, SHOULDERS, CORE AREAS, ETC.) ACCEPTABLE ALTERNATIVES TO PREMIUM PAINT INCLUDE: THERMOPLASTICS, POLYUREA, PREFORMED FLAT TAPE, AND METHYL METHACRYLATE (MMA) FOR APPLICATION IN COLDER CLIMATES. SOLVENT-BASED PAINT SHALL NOT BE USED. ALL PAINT SHALL BE APPLIED FOLLOWING MANUFACTURER

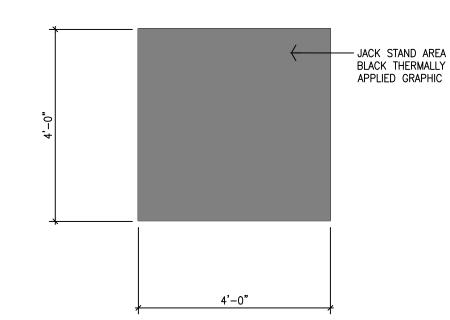
RECOMMENDATIONS TO PRESERVE AVAILABLE WARRANTIES. ALL PAVEMENT GRAPHICS SHALL BE HOT-APPLIED THERMOPLASTIC (REFLECTIVE). APPLY IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS TO PRESERVE AVAILABLE WARRANTIES.

## GRAPHICS / PAVEMENT PAINT

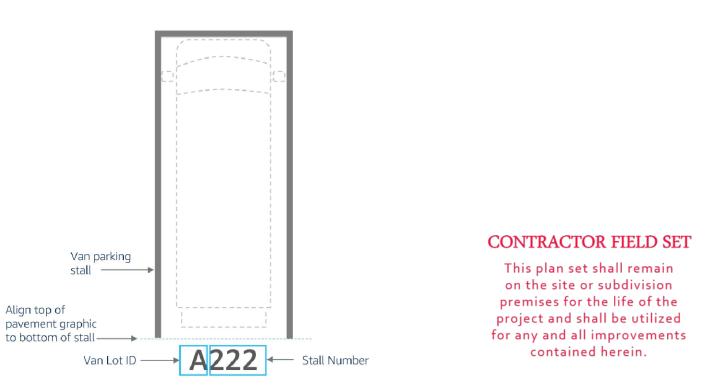
1/PP1 WHITE 2/PP2 RED (PANTONE 485C) BLUE (PANTONE 294)

4/PP4 GREEN (PANTONE 342) YELLOW (PANTONE 116)

NOTE: TWO COAT APPLICATION REQUIRED FOR ALL PAVEMENT GRAPHICS.

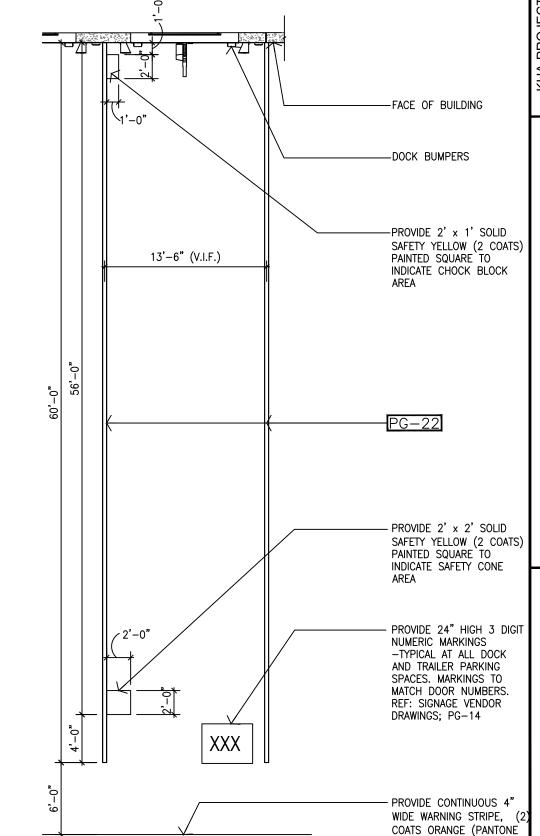






Van Stall Numbering Pavement Graphic

Figure 2.P: Van Stall Numbering Pavement Graphic



INDU POSITION 66' AWAY FROM FACE OF BUILDING WALL,

021C) PAVEMENT PAINT.

SHEET NUMBER 92 OF 227

SDP24-00001

(BLACK INDICATES ASPHALT) 8'-0" X 10'-0"

5'-0" X 3'-6" (24" TALL LETTERING)

PAVEMENT GRAPHICS - TRAILER SPACE NUMBERING.

USE TO LABEL TRAILER SPACES. TYPICAL ALL TRUCK

SPACES. YELLOW GRAPHICS (BLACK INDICATES

SEE PLANS FOR DOCK DOOR NUMBERS. COORDINATE

NUMBERS WITH TENANT OF TRAILER PARKING.

8'-0" X 8'-0" PAVEMENT GRAPHICS - LEFT TURN ARROW. USE TO INDICATE TURNING LANES. YELLOW THERMAL GRAPHIC. (BLACK INDICATES ASPHALT)

18" STRIPING OUTLINE AND FILL AT 45 DEGREES. PAVEMENT GRAPHICS - TRAFFIC ISLAND. USE TO PROHIBIT VEHICLE TRAVEL AND PARKING. C14 YELLOW AT NO TRAFFIC, C8 BLUE AT ACCESSIBLE PARKING SPACES, YELLOW AT VENDOR PARKING (BLACK INDICATES ASPHALT). INCLUDE AT ALL EMERGENCY EXITS INTO THE PARKING LOT AT ALL PARKING SPACES THAT IMPEDE DIRECT ACCESS TO THE THOROUGHFARE.

4' X 4' SOLID BLACK SQUARE.

PAVEMENT GRAPHICS - JACK

STAND AREAS. USE TO IDENTIFY

JACK STAND LOCATION. THERMAL

APPLIED GRAPHICS

CROSSWALK (YELLOW) - 4" STRIPING ACCESSIBLE PARKING (BLUE)

12" STRIPING OUTLINE AND HATCH FILL AT 36" O.C. PAVEMENT GRAPHICS - PEDESTRIAN CROSSWALK. USE TO INDICATE PEDESTRIAN CROSSING. YELLOW STRIPES, SEE PLAN FOR WIDTH OF CROSSWALK (BLACK INDICATES ASPHALT)

Pavement Graphic Details

PAVEMENT GRAPHICS - STRAIGHT

ARROW. USE TO DIRECT TRAFFIC AT

ONE-WAY DRIVE

AISLES. YELLOW THERMAL GRAPHIC.

(BLACK INDICATES ASPHALT)

12" STRIPING OUTLINE AND HATCH FILL AT 36" O.C.

PAVEMENT GRAPHICS - DRIVER ENTRANCE.

USE TO DIRECT DRIVER ENTRANCE IN DOCK FIELD.

POSITION IN TRUCK COURT TO CORRESPOND WITH THE LOCATION OF

THE TRUCKER'S LOUNGE AND TRUCKER'S ENTRY POINTS. FC

(TOM)/SC (LOGISTICS) PARTNER WITH SAFETY TO FIND CLOSEST

APPROVED EXIT DOOR TO GUARD SHACK. DESIGNATE A PARKING

AREA FOR THE HOSTLER VEHICLE(S). PROVIDE WALKING SPACE

AROUND THE PARKING AREA FOR THE PRE-OPERATIONAL INSPECTION PARTNER WITH BASE BUILDING MANAGER TO MARK A CROSSWALK FROM THE BUILDING EXIT POINT TO THE GUARD SHACK AND HOSTLER PARKING AREA USING THE SHORTEST LOGICAL PATH.

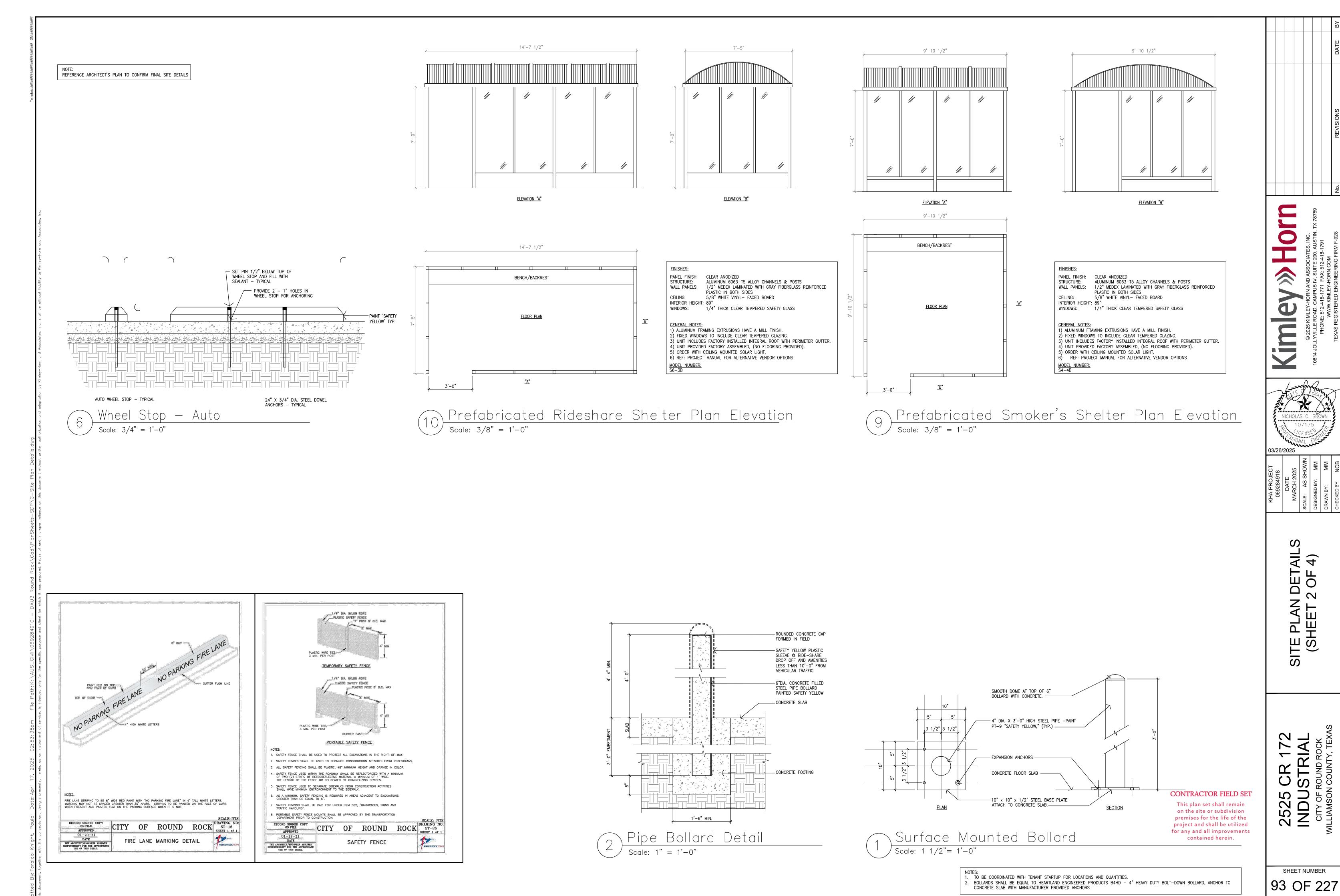
ASSOCIATES WILL NEED TO BE INSIDE THIS PATH TO TRANSIT THE YARD WITHOUT A SPOTTER. THERMAL APPLIED C14 YELLOW STRIPES.

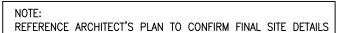
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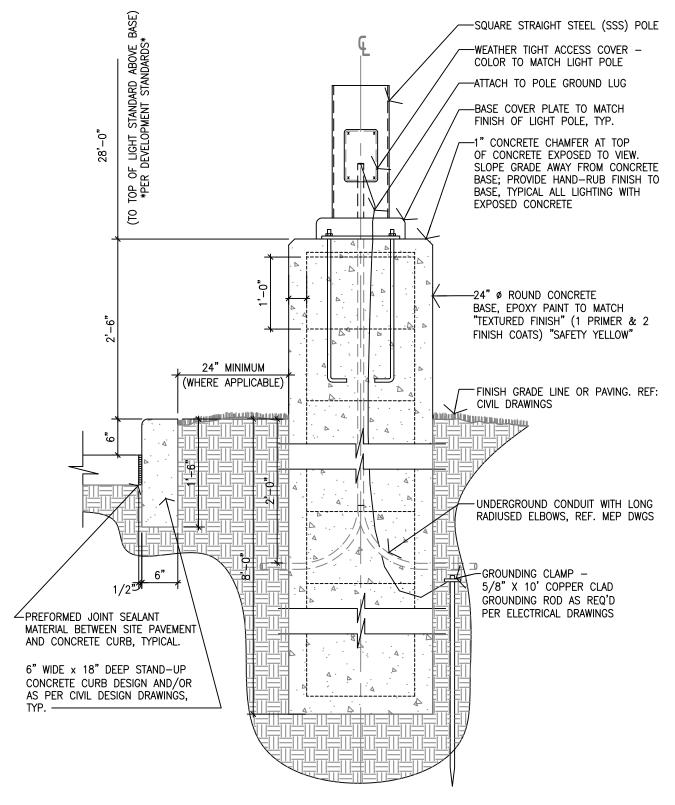
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03/26/2025

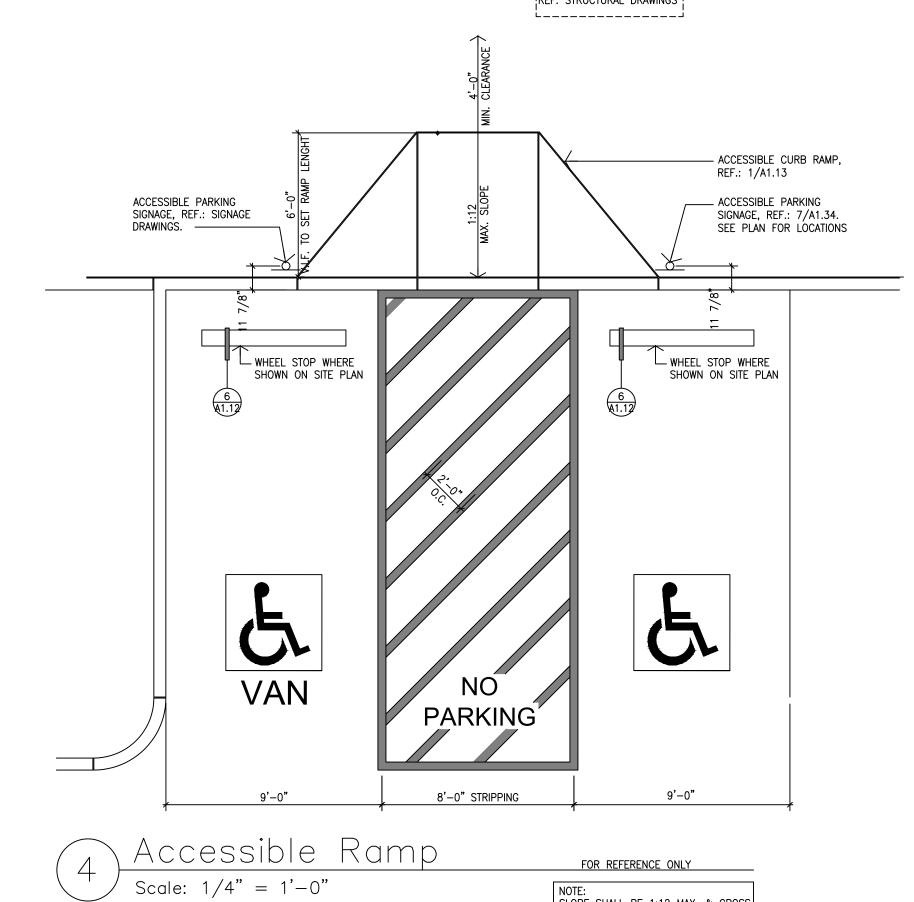
SITE PL (SHE







Development Site Lighting Pole Base REF. STRUCTURAL DRAWINGS



FOR REFERENCE ONLY

SLOPE SHALL BE 1:12 MAX. & CROSS SLOPE SHALL BE 1:48 OR 2% MAX., PER IBC SECTION 1012

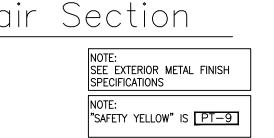
FOR REFERENCE ONLY. SEE CIVIL SHEETS AND 7/A1.11

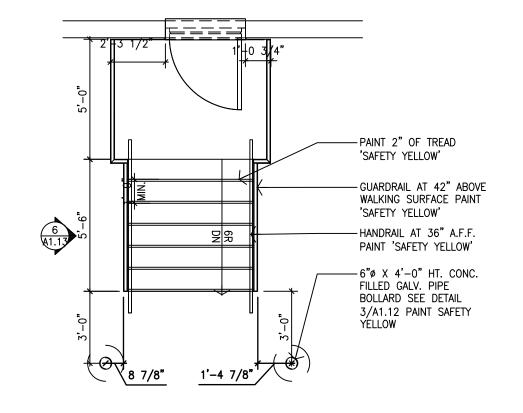
LANDING SUPPORT ANGLES -STEEL CLIP ANGLE STEEL STRINGER HANDRAIL EXTENSION; RETURN TO GUARD POST. - OPEN STEEL WITH "GRIP STRUT" TREADS AND LANDING 1/4" X 12" AND 1/4" CLOSED" STEEL -1-1/2"ø ROUND STEEL HANDRAIL WELD & GRIND - 2"ø STEEL PIPE COLUMN @ LANDING CORNERS —— 6"ø X 4'-0" HT. CONC. FILLED GALV. PIPE BOLLARD, SEE DETAIL 2/A1.12 PAINTED SAFETY YELLOW — MAX. LEADING EDGE RADIUS ⊚ STAIR: 1/2" PER IBC 1011.5.5 ELEVATION

Pre-Eng. MTL. Stair Section

Scale: 1/4"= 1'-0"

NOTE:

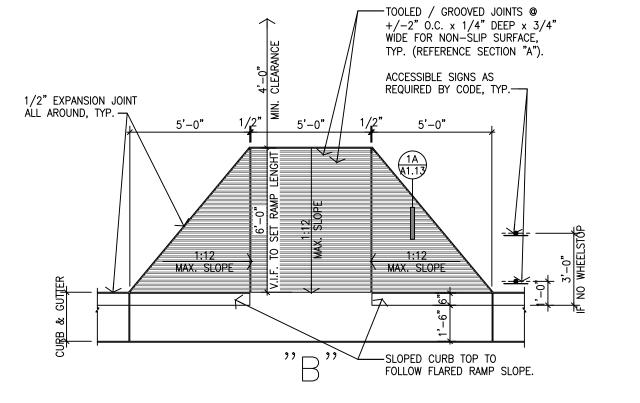




Pre-Eng. MTL. Stair Plan

Scale: 1/4" = 1'-0"

NOTE: "SAFETY VELLOW" IS TOTAL NOTE:
"SAFETY YELLOW" IS PT-9



Accessible Curb Ramp

Scale: 1/4" = 1'-0"

NOTES:
PROVIDE INTEGRALLY COLORED CONCRETE
FOR ALL CURB RAMP SURFACE. COLOR
TO BE APPROVED BY ARCHITECT. HANDRAILS SHOULD BE PROVIDED IF WALK OR RAMP SURFACE IS GREATER THAN 6'-0" IN LENGTH FOR MAX. 1:12 SLOPE, TYPICAL FOR ALL THIS PROJECT. FOR FLARED SIDES TO BE CONSTRUCTED TO A 1:10 SLOPE, 48" OF CLEARANCE MUST BE MAINTAINED BEYOND RAMP TOP, OTHERWISE FLARED SIDES MUST BE SLOPED 1:12 MAXIMUM. MEDIUM BROOM FINISH PERPENDICULAR TO DIRECTION OF TRAVEL ON ALL RAMP SURFACES, TYP. CENTER CROWN RAMP SLOPE TO SIDES, (1/8"/12") FOR GROOVE DRAINAGE.

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CONTRACTOR FIELD SET

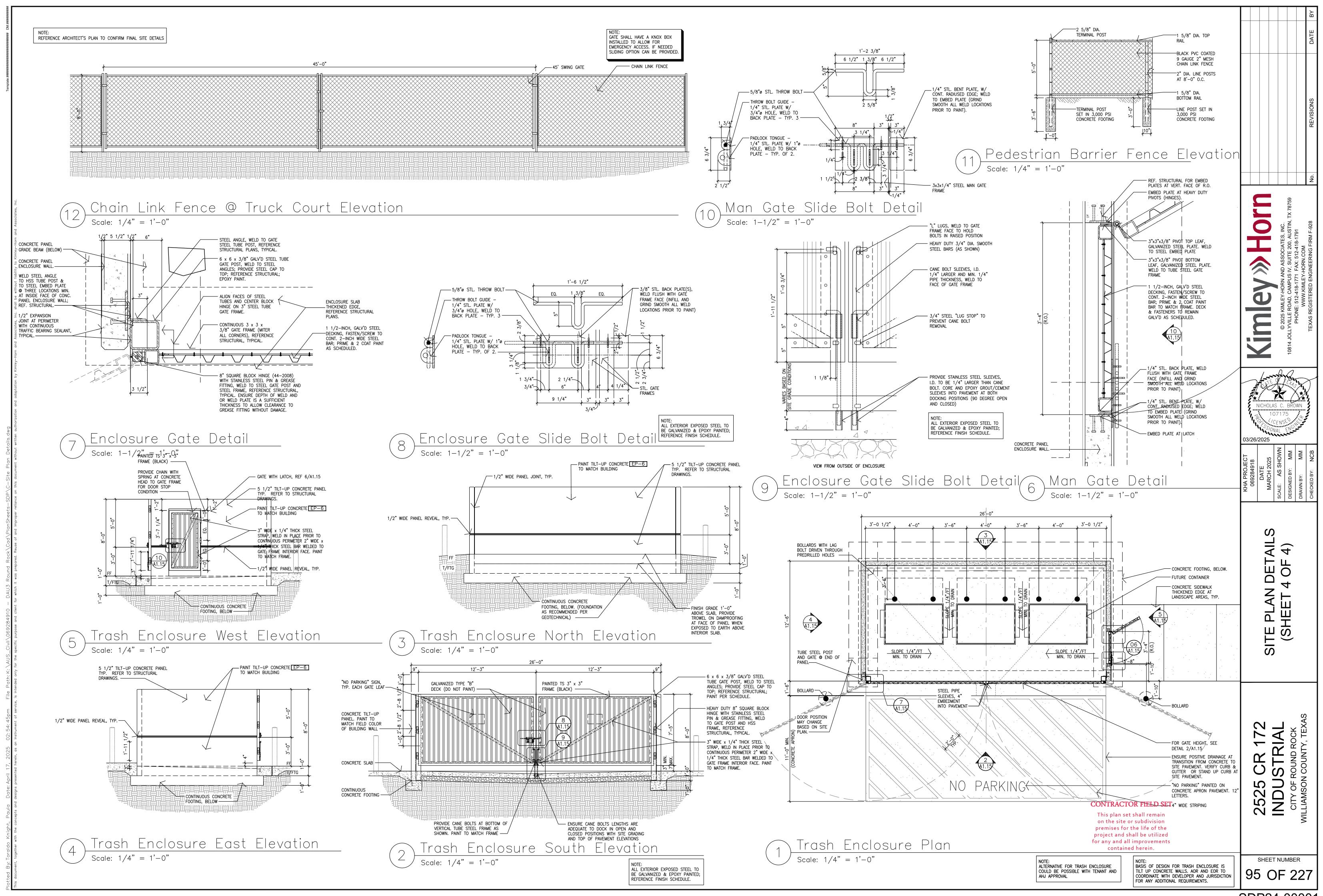
SHEET NUMBER 94 OF 227

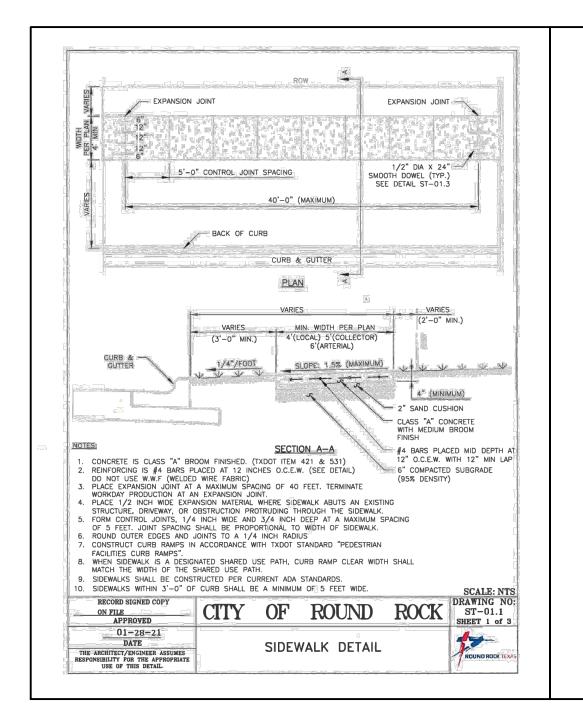
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INDUSTR
CITY OF ROUND

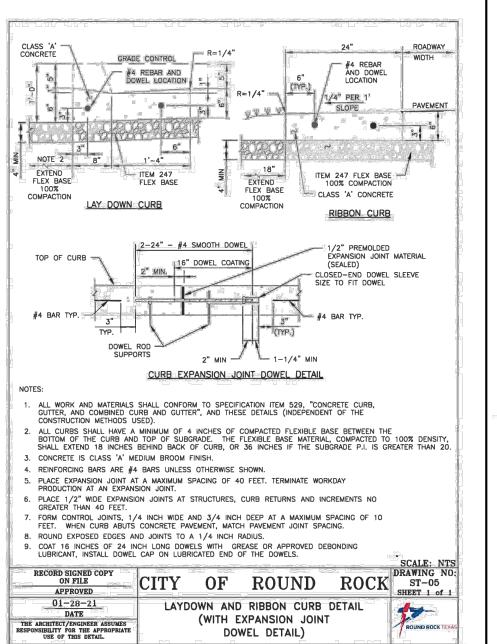
03/26/2025

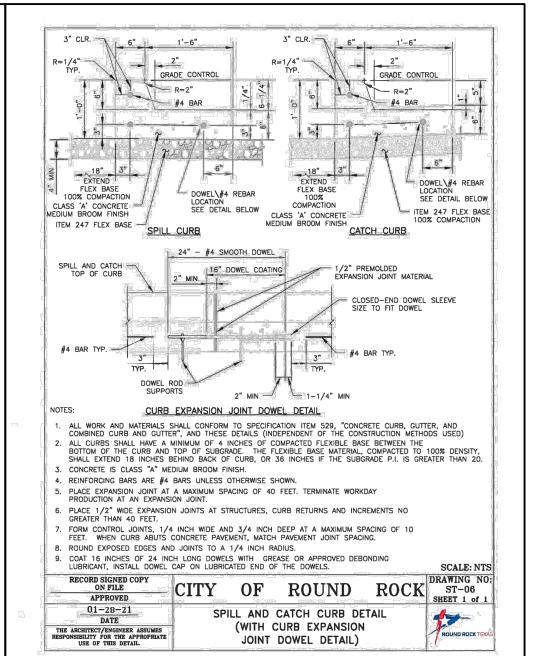
DETAIL:

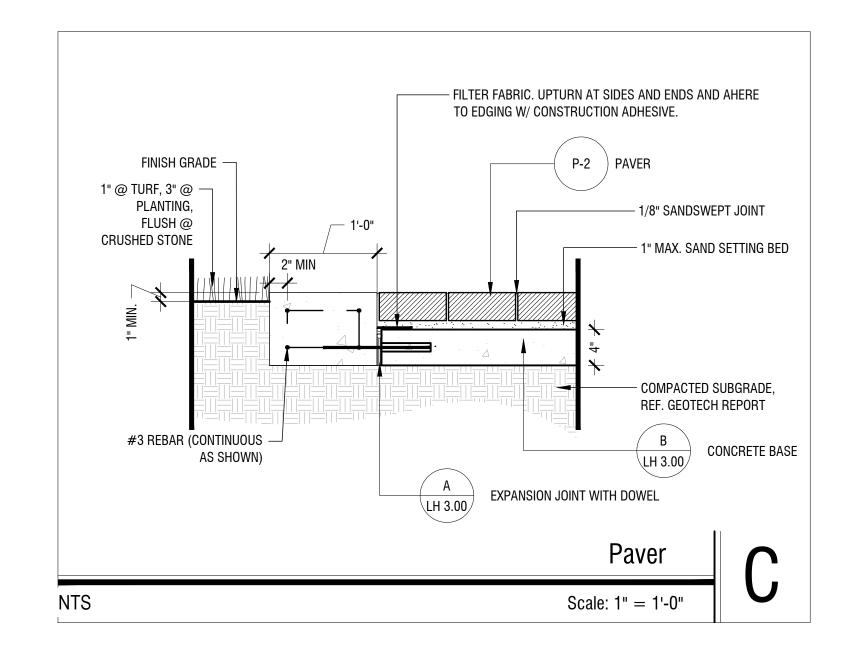
SITE PLAN I (SHEET 3











# CONTRACTOR FIELD SET

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# **BENCHMARKS**

BM #51471 MAG NAIL SET "GCP 2" SET AT THE NORTHEAST CORNER OF THE INTERSECTION OF STATE HIGHWAY 45 AND COUNTY ROAD 172, ±5.5' FROM "YIELD" SIGN. N: 10147640.83 E: 3127942.60 ELEV.= 841.70' BM #51473 MAG NAIL SET "GCP 3"

SET IN THE NORTH RIGHT-OF-WAY LINE OF HESTERS CROSSING ROAD, ±560' FROM THE INTERSECTION OF HESTERS CROSSING ROAD AND COUNTY ROAD 172, ±100 FROM CONCRETE SIGN "ENCOMPASS HEALTH". N: 10149343.04 E: 3128315.81 ELEV.= 812.19'

SHEET NUMBER

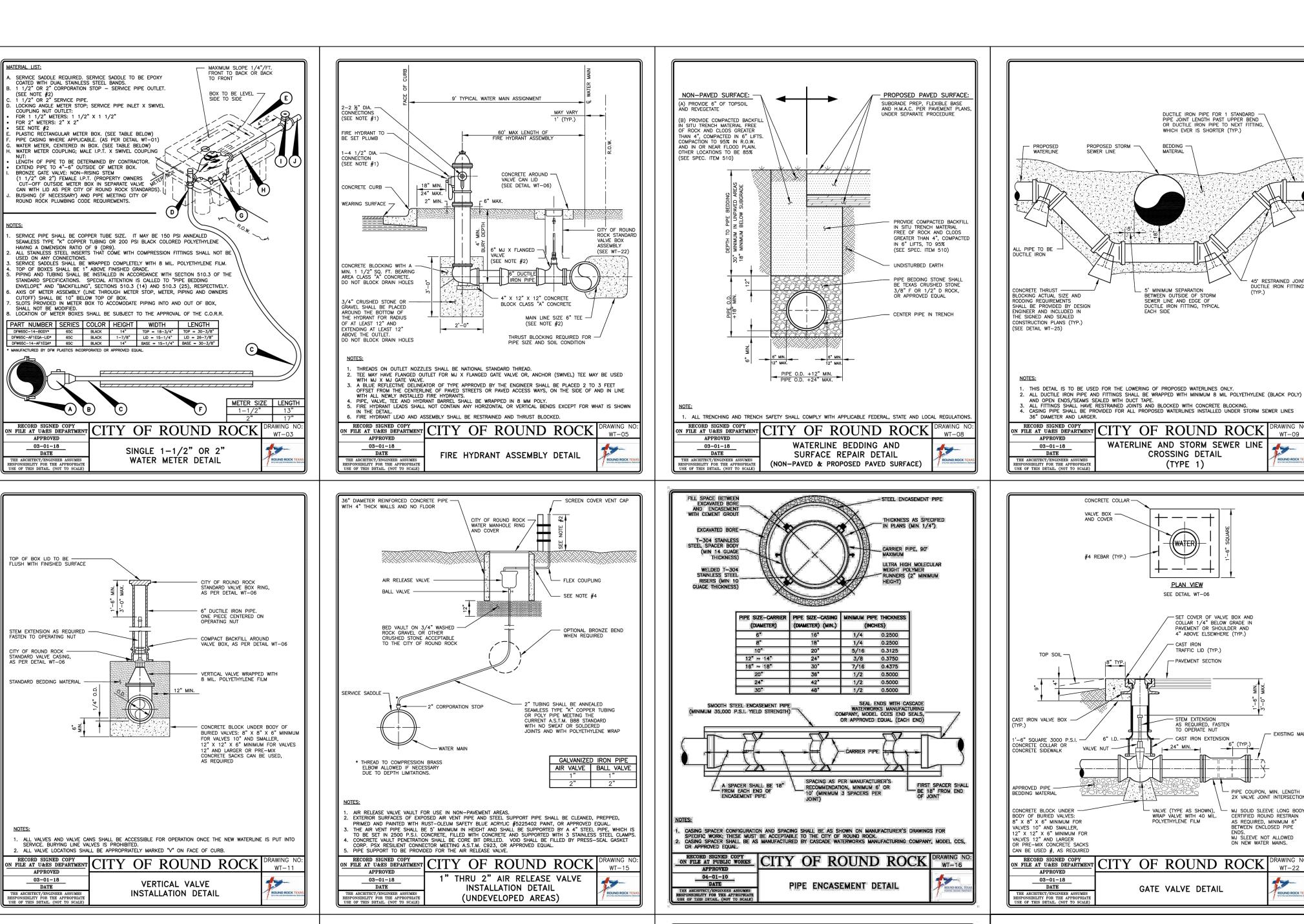
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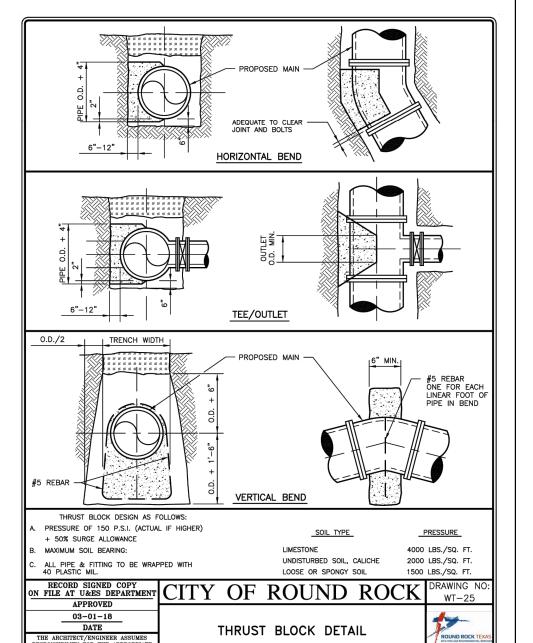
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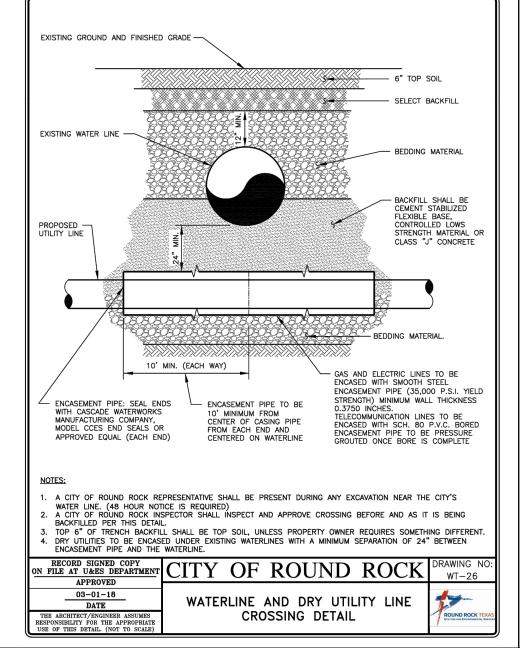
SDP24-00001

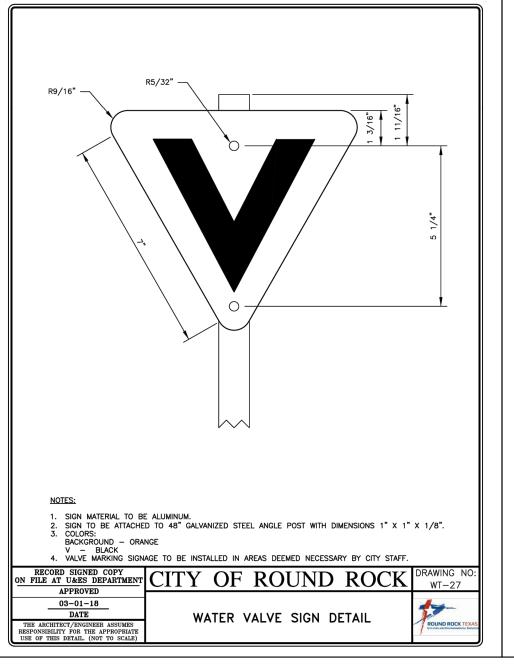
DETAIL **PAVING** 

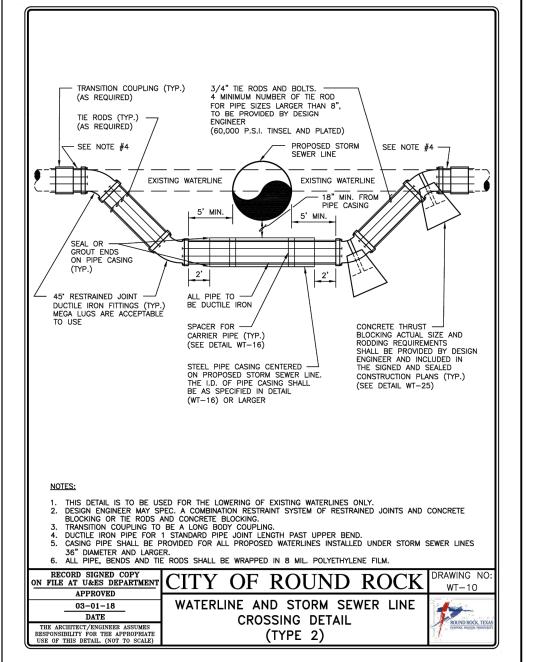
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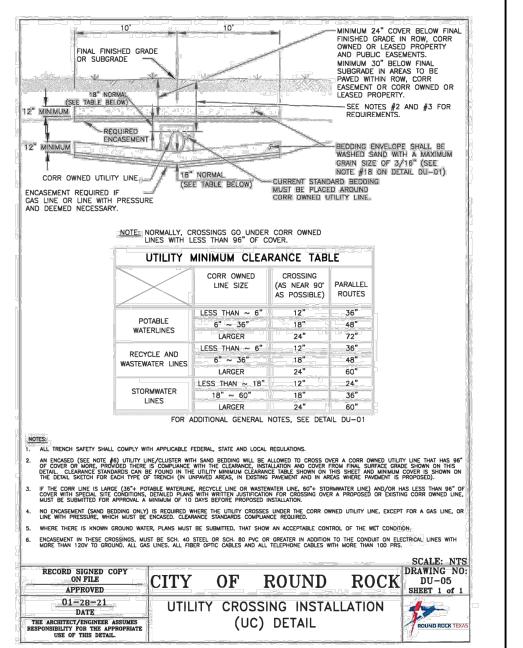








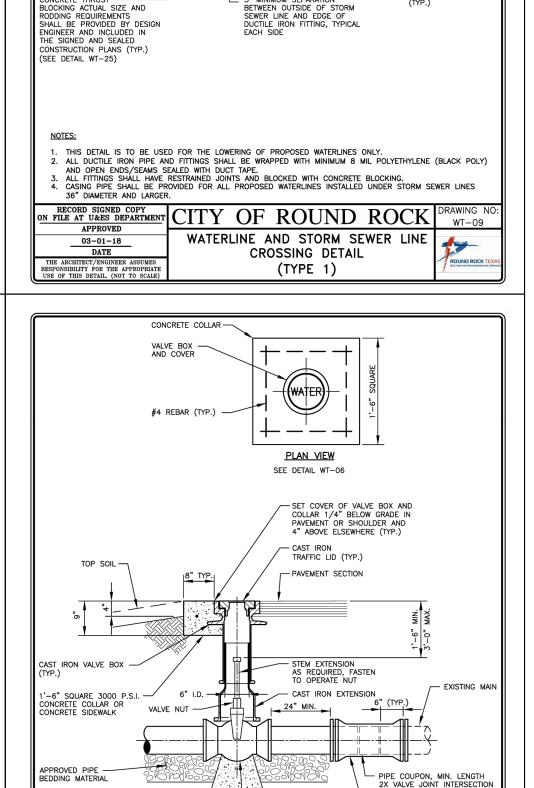




GATE VALVE DETAIL

MJ SOLID SLEEVE LONG BE CERTIFIED ROUND RESTRAIN AS REQUIRED, MINIMUM 6" BETWEEN ENCLOSED PIPE

ENDS.
MJ SLEEVE NOT ALLOWED
ON NEW WATER MAINS.



DUCTILE IRON PIPE FOR 1 STANDARD
PIPE JOINT LENGTH PAST UPPER BEND
OR DUCTILE IRON PIPE TO NEXT FITTING,

WHICH EVER IS SHORTER (TYP.)

PROPOSED STORM — SEWER LINE

# CONTRACTOR FIELD SET

This plan set shall remain on the site or subdivision premises for the life of the project and shall be utilized for any and all improvements contained herein.

# **BENCHMARKS**

SET AT THE NORTHEAST CORNER OF THE INTERSECTION OF STATE HIGHWAY 45 AND COUNTY ROAD 172, ±5.5' FROM "YIELD" SIGN. N: 10147640.83 E: 3127942.60 ELEV.= 841.70' BM #51473 MAG NAIL SET "GCP 3"

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SHEET NUMBER

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NDO

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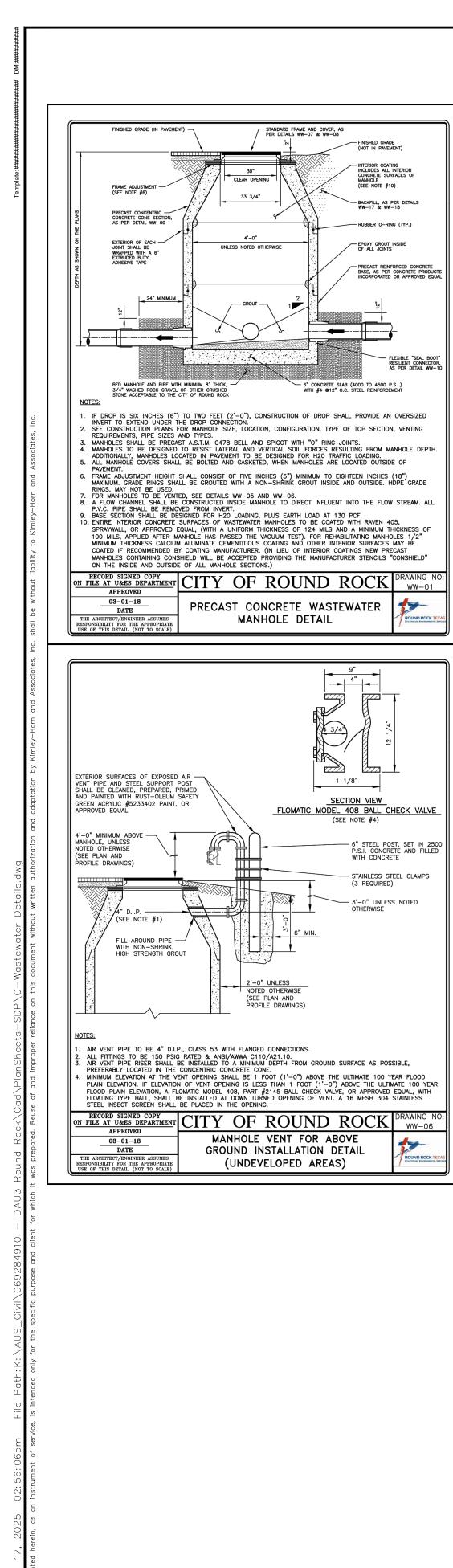
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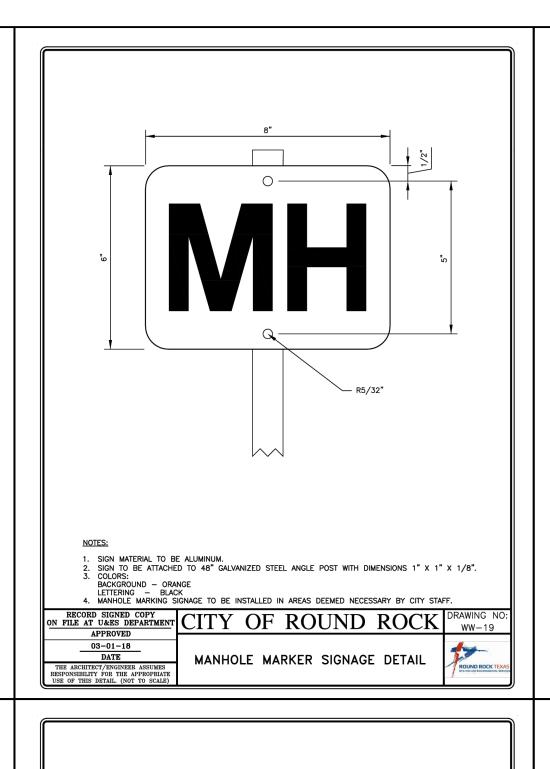
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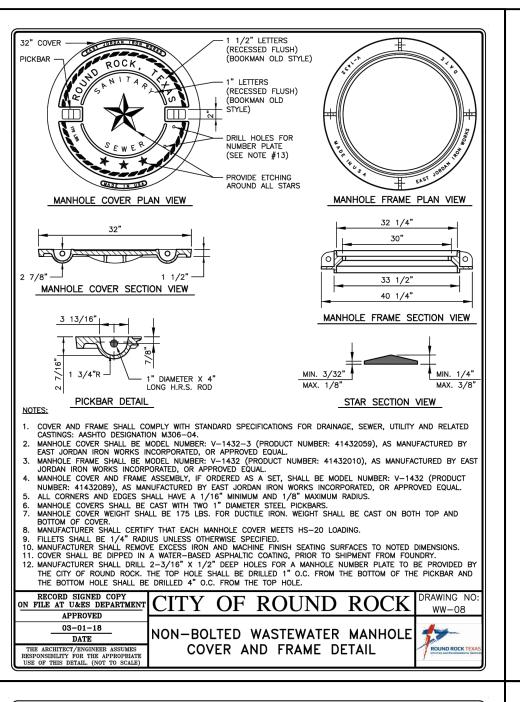
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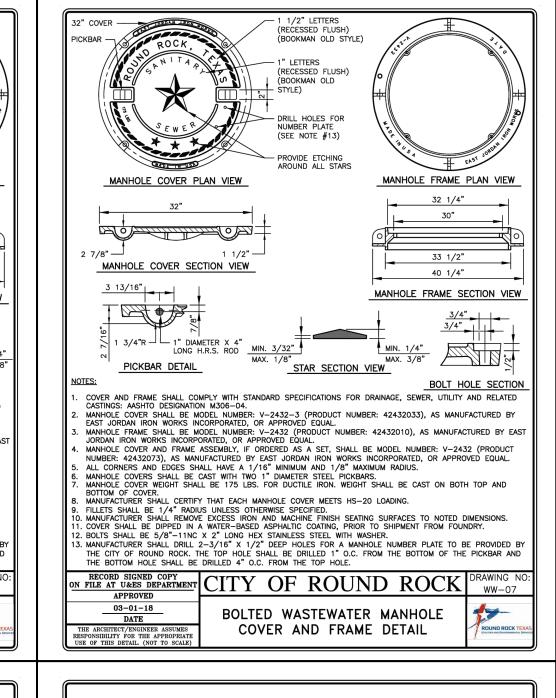
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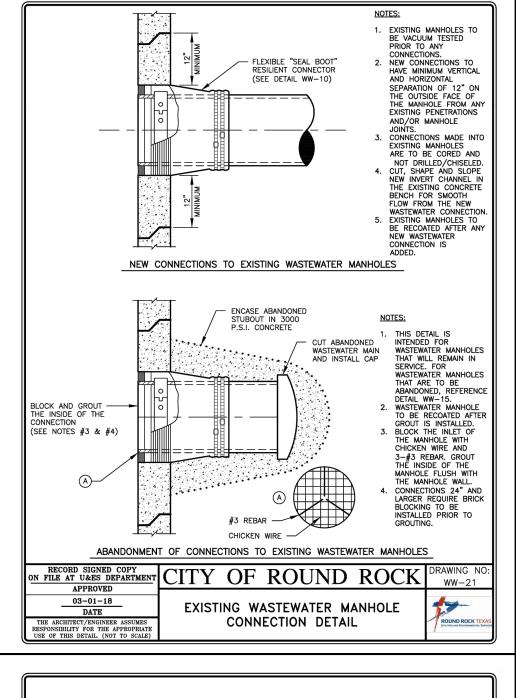
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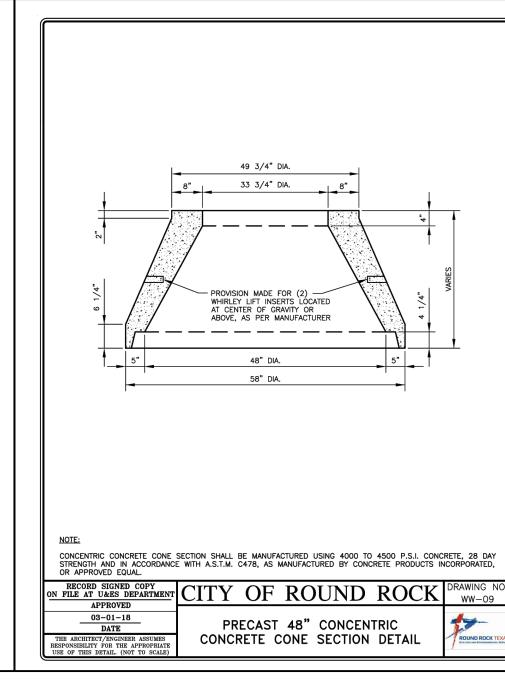


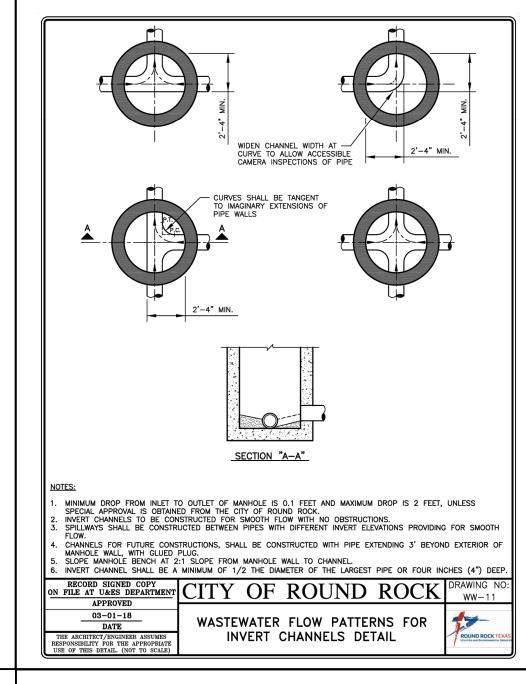


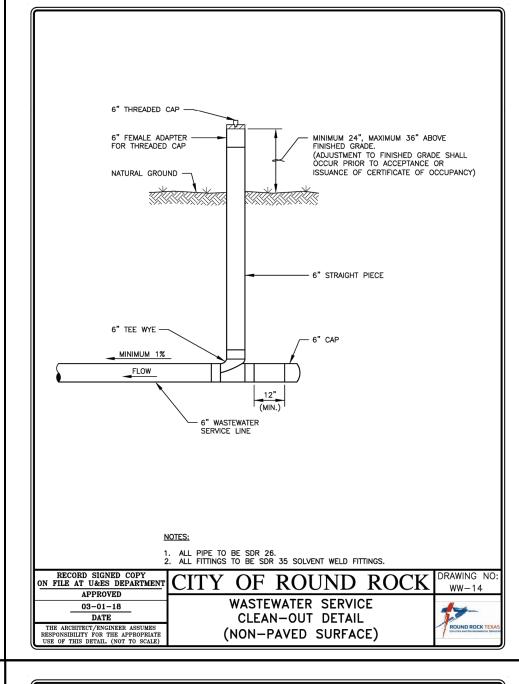


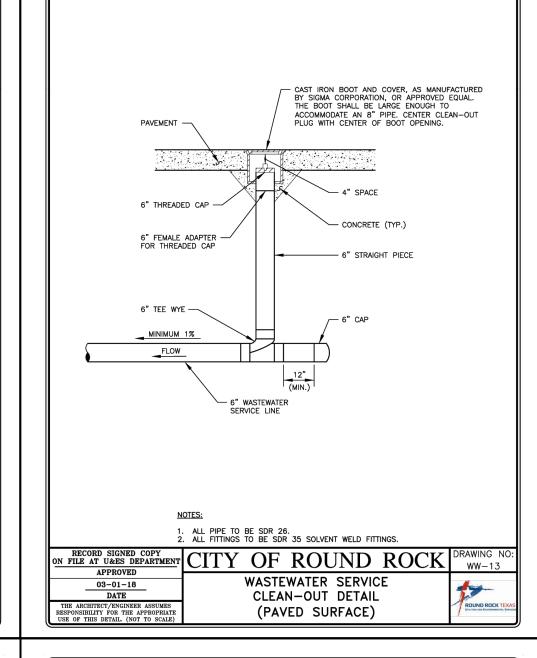


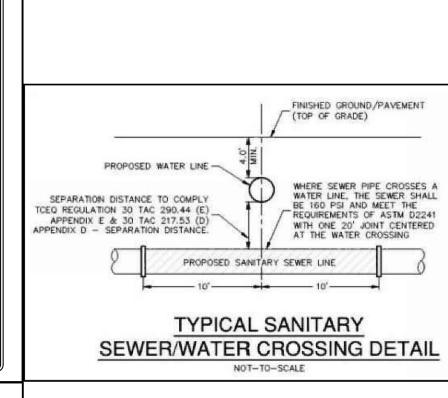


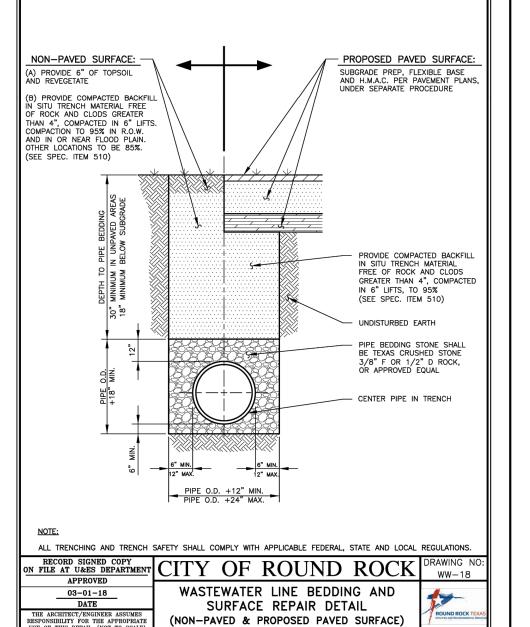


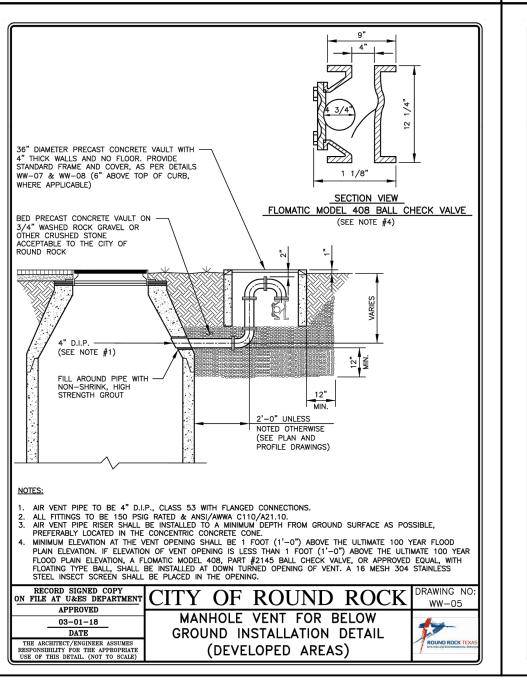


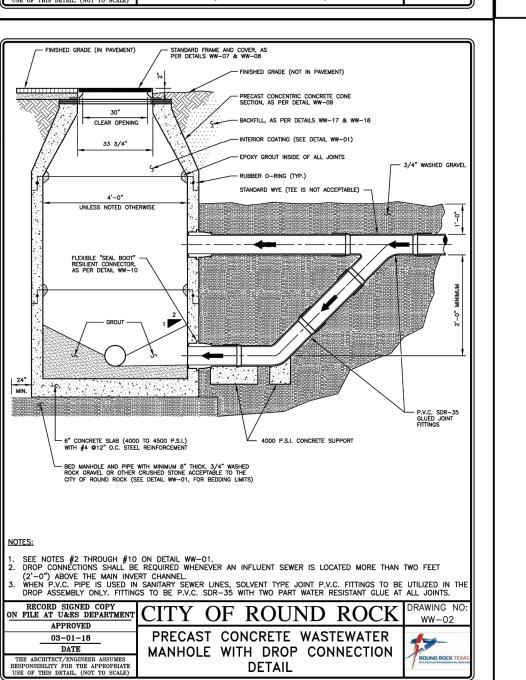














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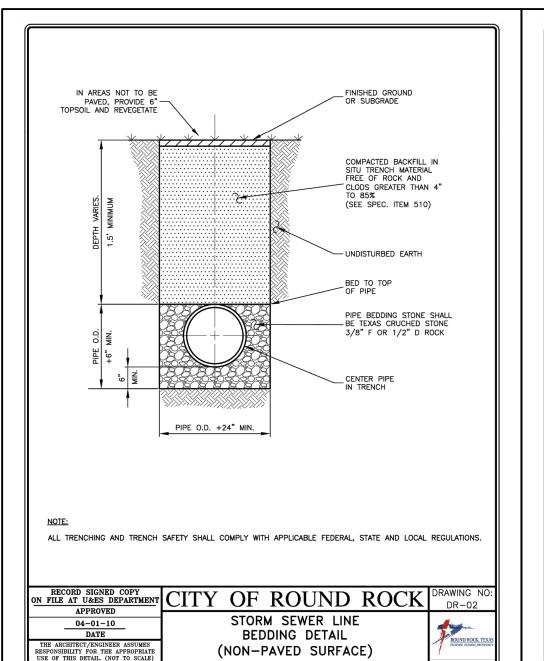
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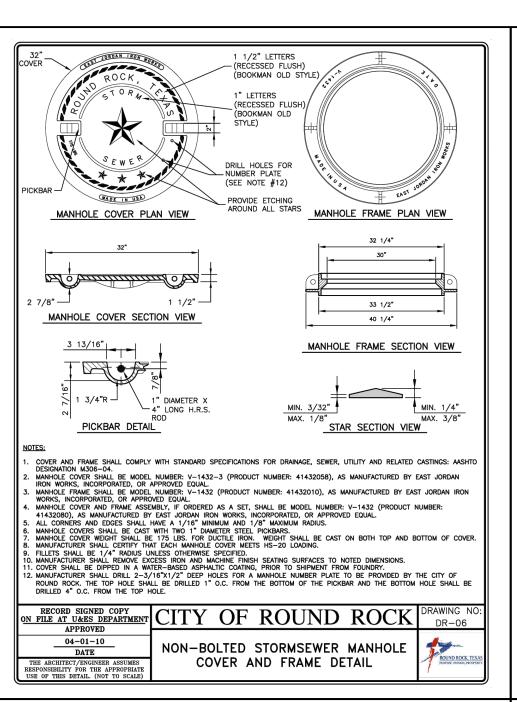
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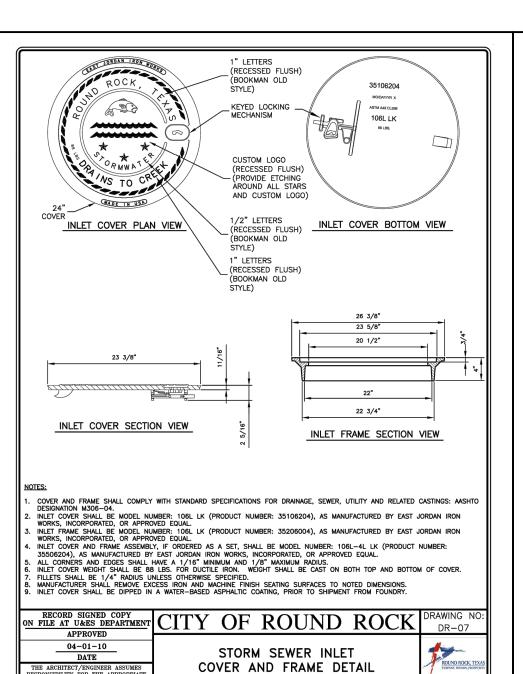
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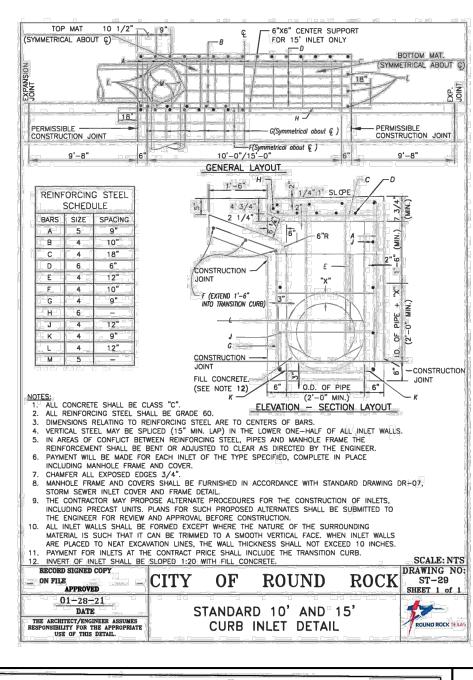
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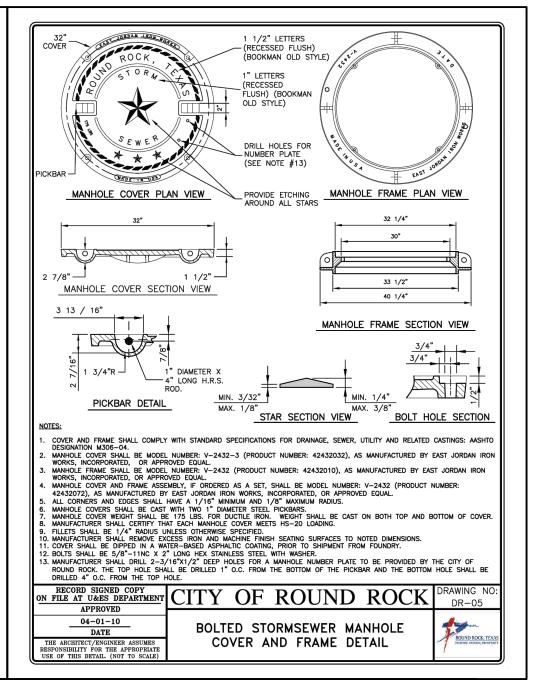
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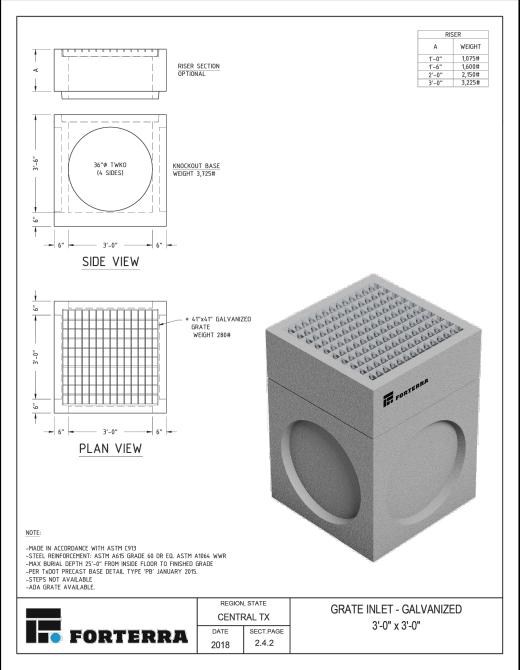


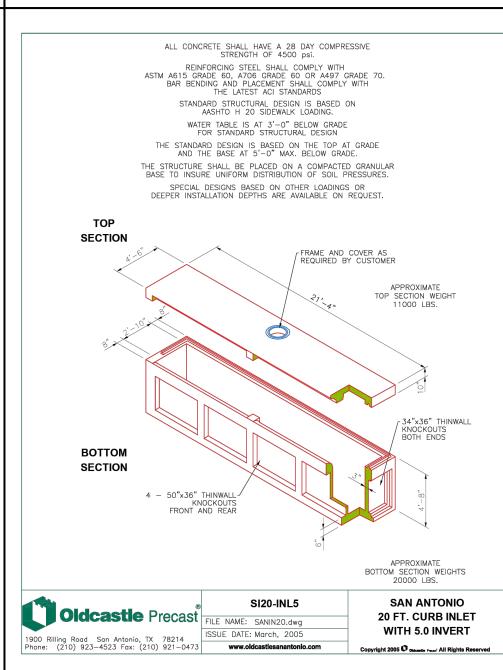


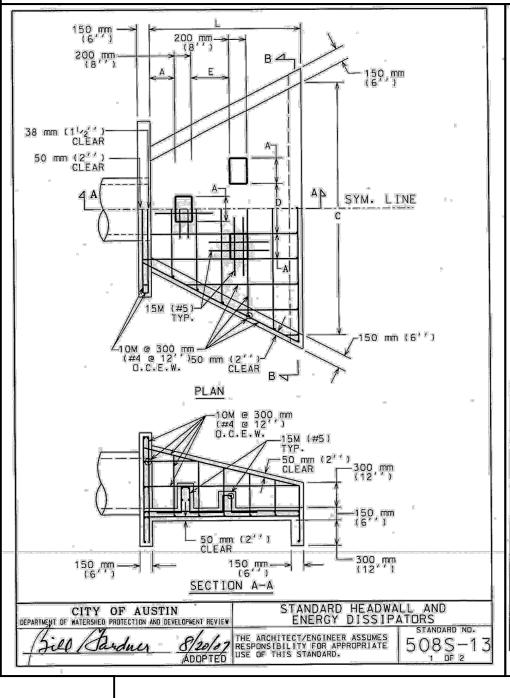


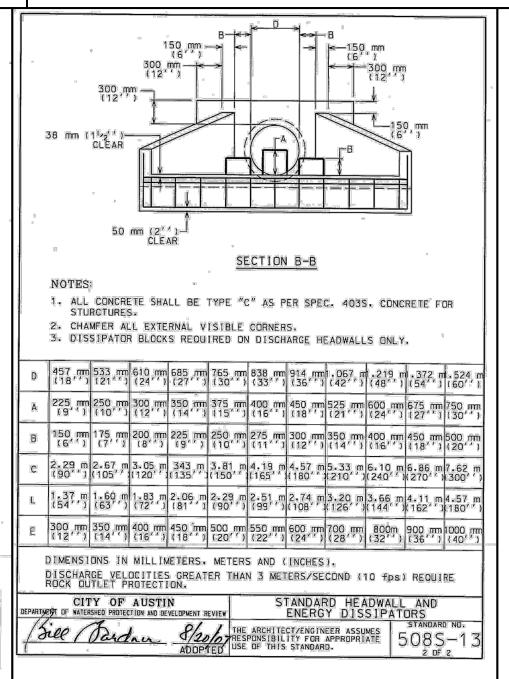


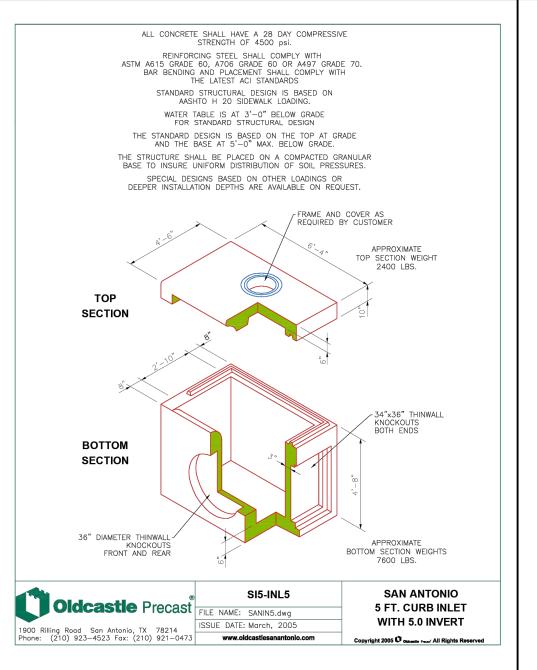


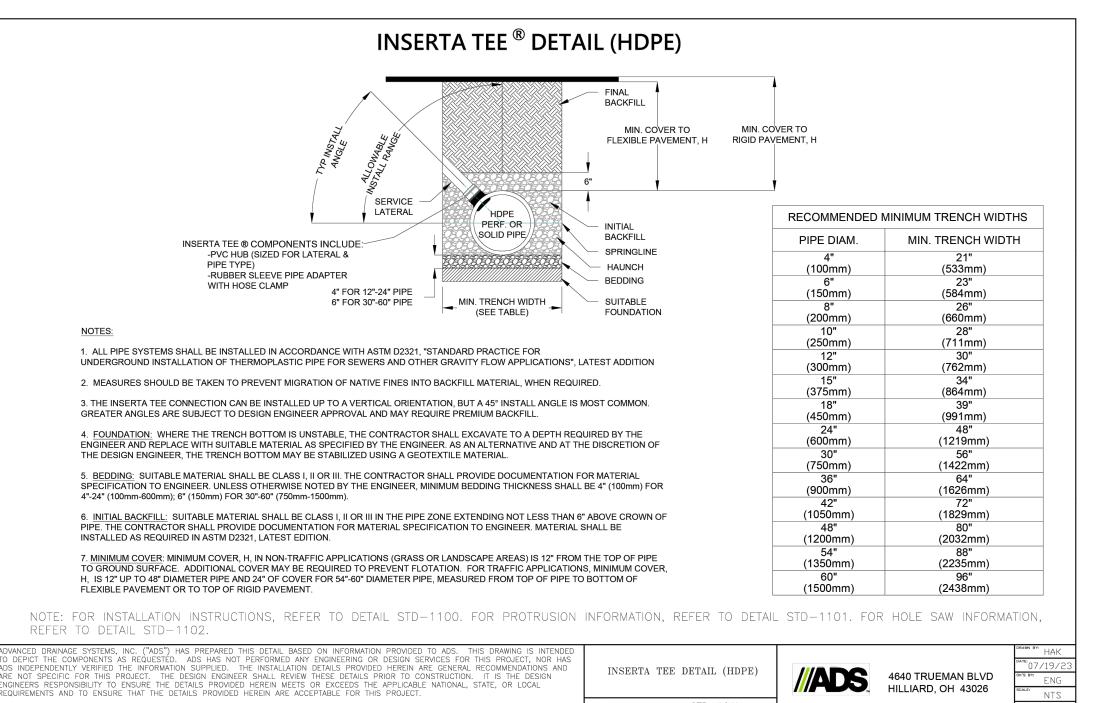




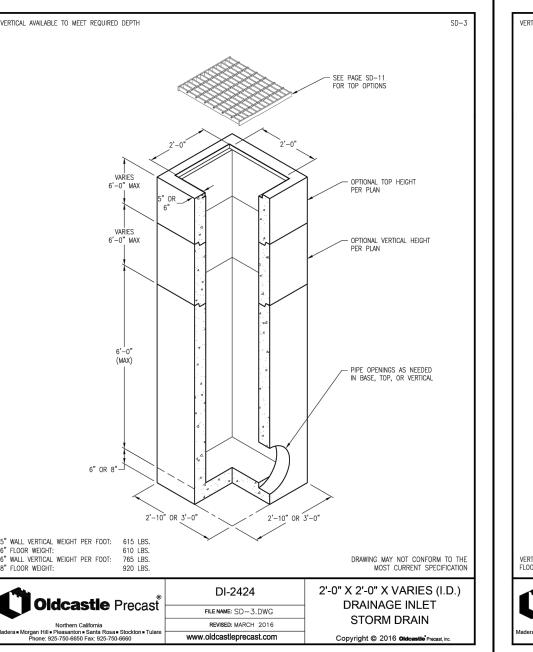


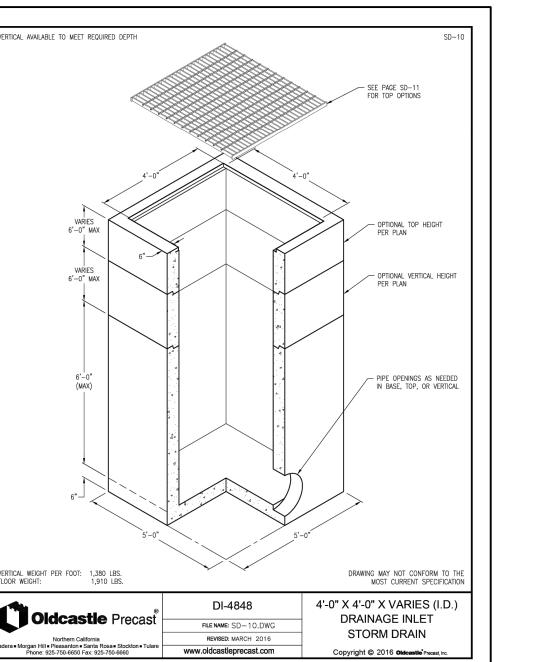






RAWING NUMBER STD-





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NICHOLAS C. BROWN

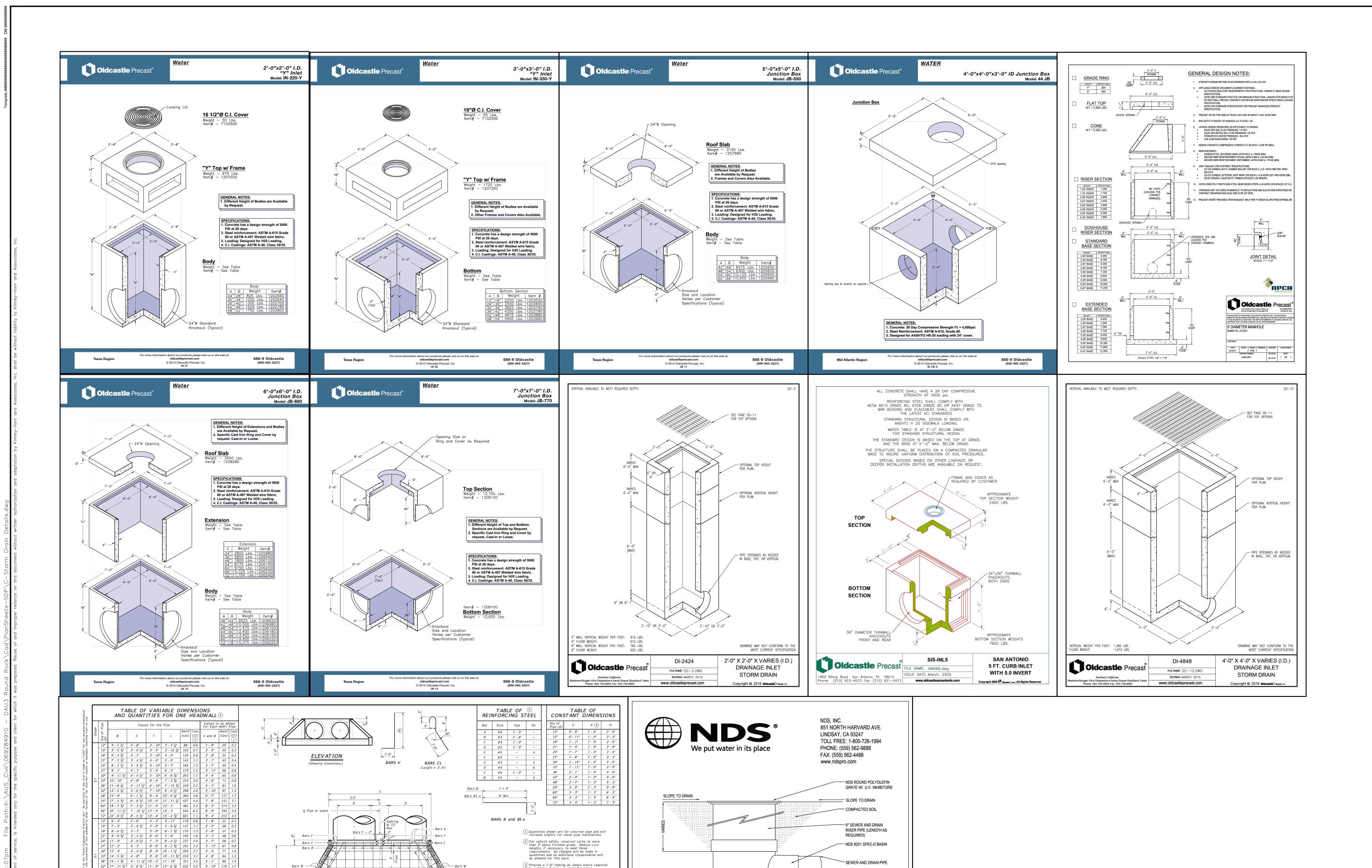
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ST(S)

03/26/2025

SDP24-00001



- INVERT ELEVATION

REVISION DATE 8-24-2015

- CRUSHED STONE

SECTION

<u>IOTES:</u>
INSTALLATION TO BE COMPLETED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.

PROFESSIONALS FOR PLANNING PURPOSES ONLY.

ROUND SPEE-D BASINS

ROUND GRATE WITH SPEE-D BASIN

DO NOT SCALE DRAWING.
 THIS DRAWING IS INTENDED FOR USE BY ARCHITECTS, ENGINEERS, CONTRACTORS, CONSULTANTS AND DESIGN

ALL INFORMATION CONTAINED HEREIN WAS CURRENT AT THE TIME OF DEVELOPMENT BUT MUST BE REVIEWED AND APPROVED BY THE PRODUCT MANUFACTURER TO BE CONSIDERED ACCURATE.

Dimensions shown are usual and maximum.
 Quantities shown are for one structure end only (one headwall).

Max Length = 12 x H - 3"  $\times \left(\frac{12 \times H - 7}{12 \times L}\right)$  - 1"

7 Lengths of wings based on SL:1 slope along this

6 Min Length = 6" + 3"  $\times \left( \frac{12 \times H - 7}{12 \times L} \right)$ 

MATERIAL NOTES:

Provide Grade 60 reinforcing steel.

Provide Class C concrete (f'c = 3,600 psi).

GENERAL NOTES:
Designed according to AASHTO LRFD Bridge Design Specifications.
Do not mount bridge rails of any type directly to these culvert headwalls.
This standard may not be used for wall heights, H, exceeding the values shown.

Texas Department of Transportation

Bridge
Division
Standard

CONCRETE HEADWALLS

WITH FLARED WINGS FOR O° SKEW PIPE CULVERTS

TYPICAL WING ELEVATION

SECTION A-A

CH-FW-0

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# BENCHMARKS

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N: 10149343.04
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ELEV.= 812.19'

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AD 172, ±100 FROM
MPASS HEALTH".

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NICHOLAS C. BROWN

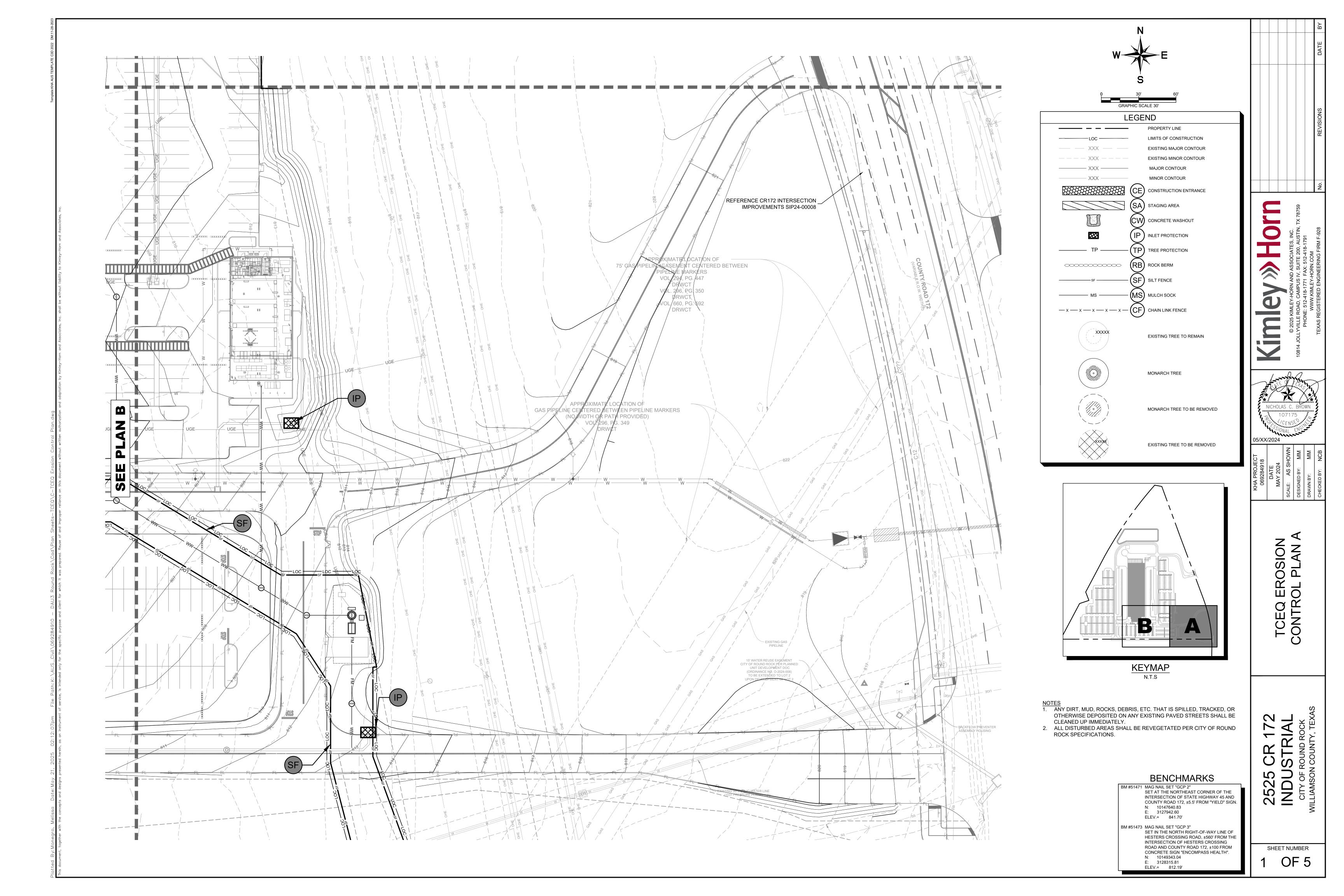
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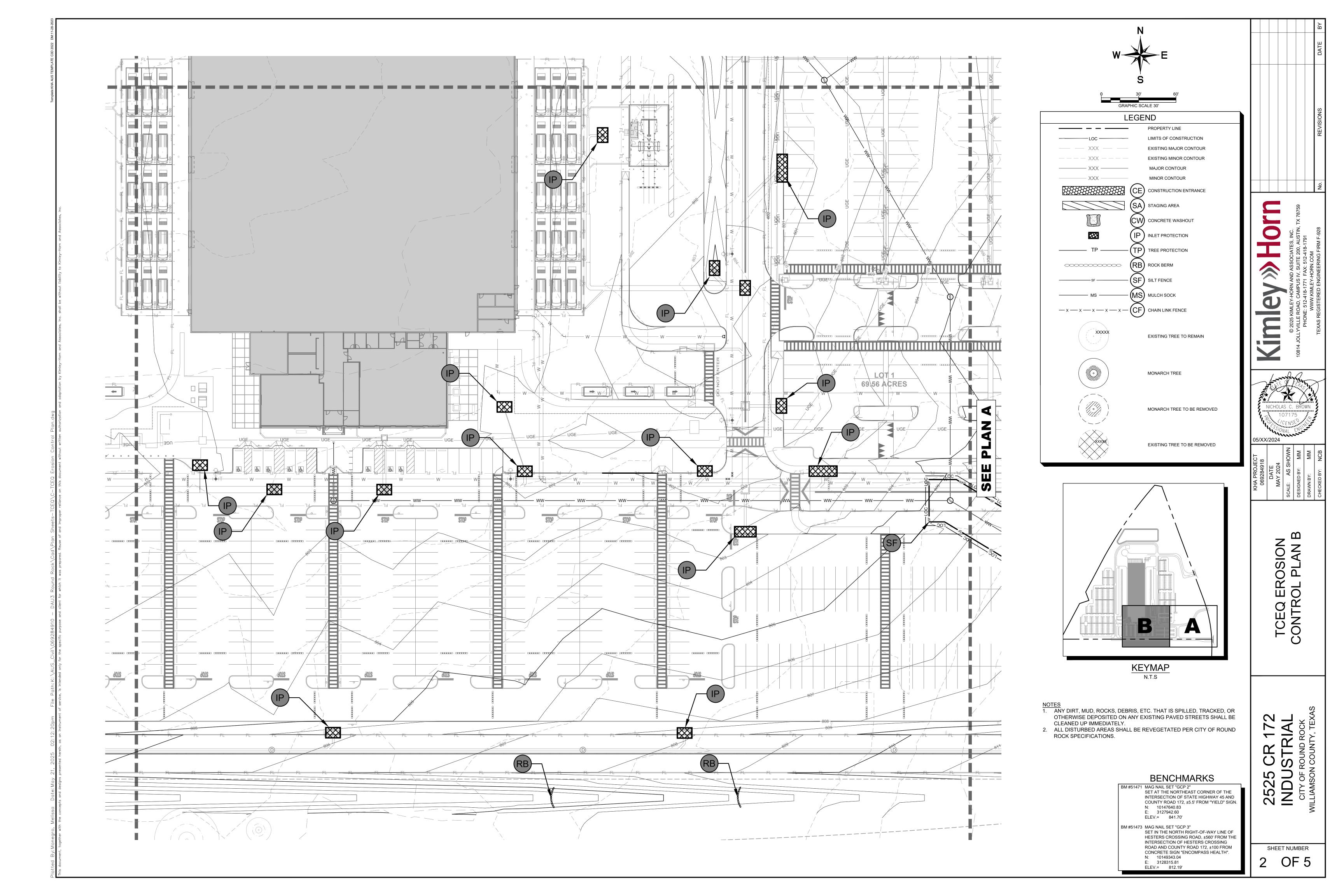
STORM [ SHEET

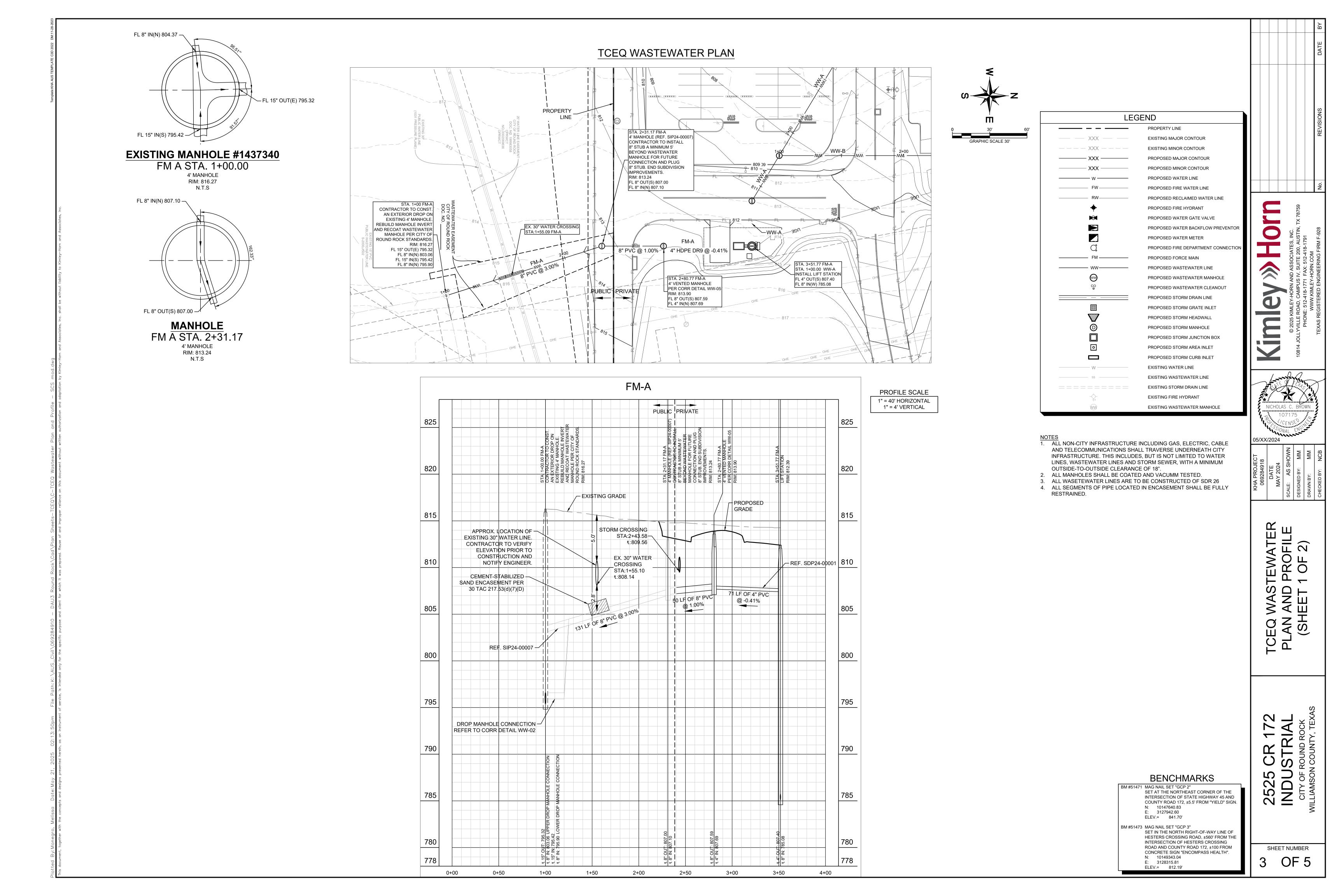
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03/26/2025

## **FINAL PLAN AND PROFILE SHEETS**







# TCEQ WASTEWATER PLAN /\_8" PVC @ 5.02% STA. 1+78.84 WW-A STA. 1+00.00 WW-B INSTALL 4' MANHOLE

STA. 3+51.77 FM-A STA. 1+00.00 WW-A

INSTALL LIFT STATION

FL 8" IN(W) 785.08

FL 4" OUT(S) 807.40 STA. 1+36.25 WW-A

4' EXTERNAL DROP MANHOLE

L 8" OUT(NW) 787.00 FL 8" OÙT(E) 786.90

FL 8" IN(NW) 788.90

RIM: 811.24

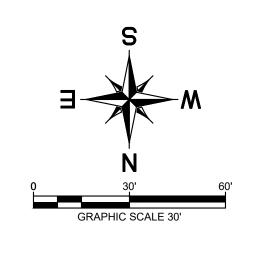
FL 8" OUT(SE) 791.71 FL 6" IN(N) 791.88 FL 8" IN(NW) 791.71

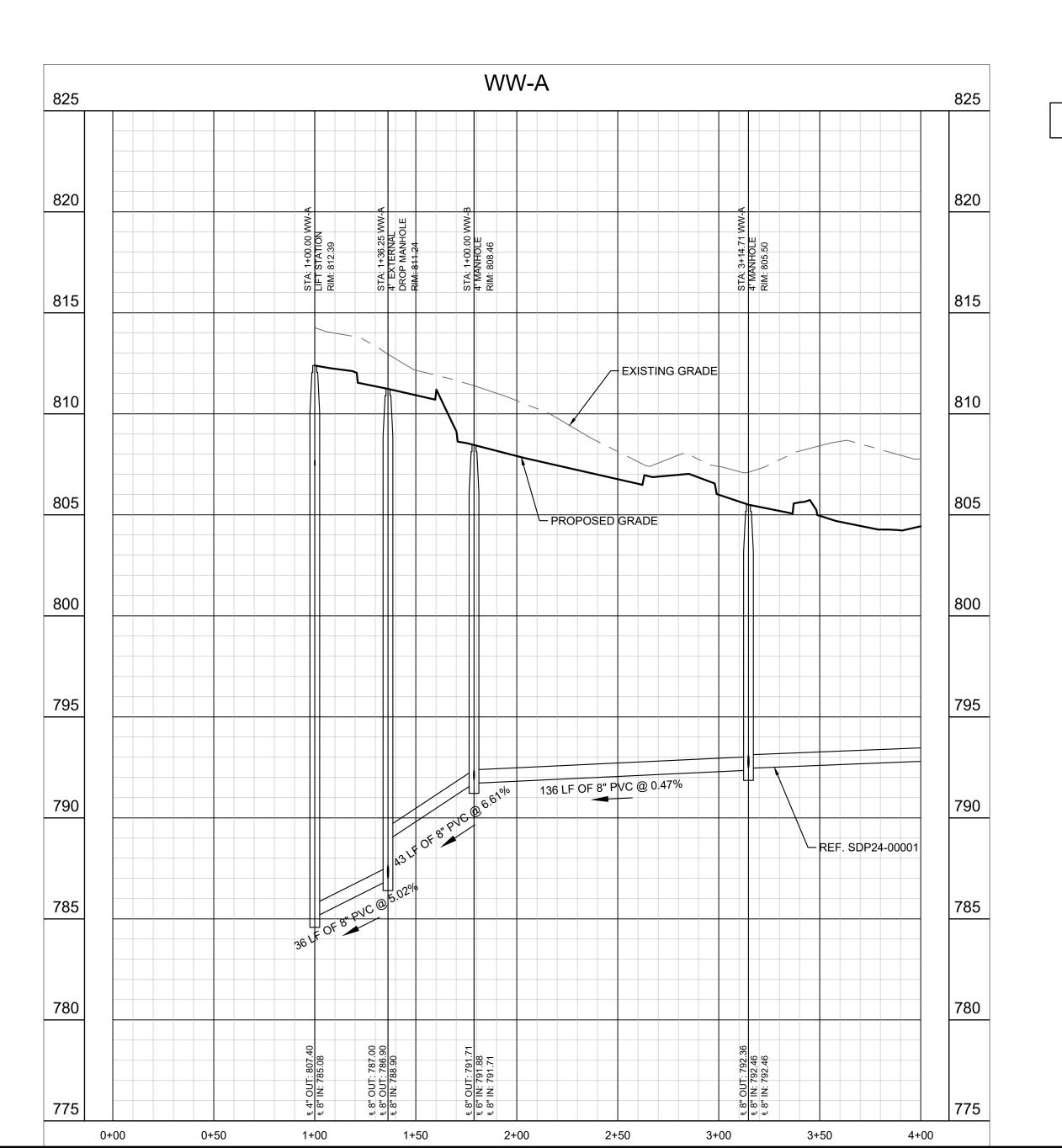
STA. 3+14.71 WW-A STA. 1+00.00 WW D INSTALL 4' MANHOLE

WW D

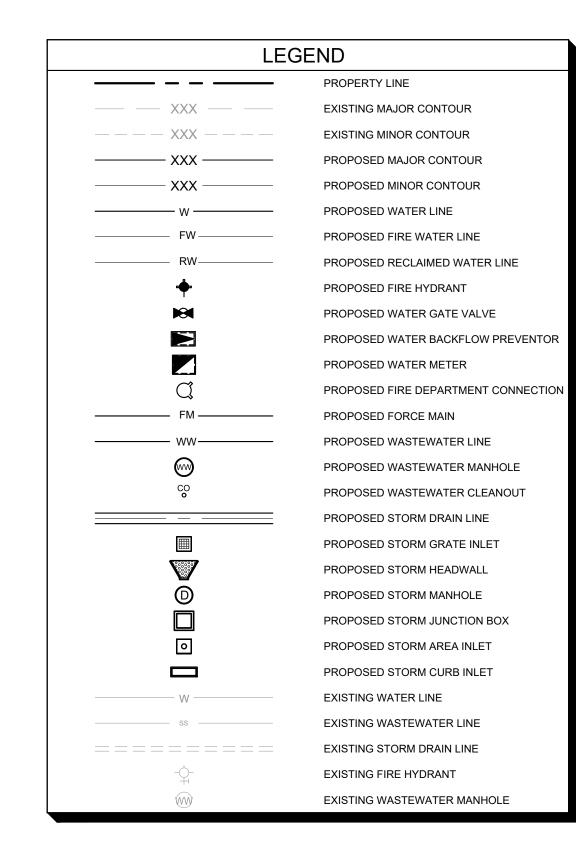
□FL 8" OUT(SE) 792.36 📗

FL 8" IN(N) 792.46 FL 8" IN(W) 792.46





PROFILE SCALE 1" = 40' HORIZONTAL 1" = 4' VERTICAL



1. ALL NON-CITY INFRASTRUCTURE INCLUDING GAS, ELECTRIC, CABLE AND TELECOMMUNICATIONS SHALL TRAVERSE UNDERNEATH CITY INFRASTRUCTURE. THIS INCLUDES, BUT IS NOT LIMITED TO WATER LINES, WASTEWATER LINES AND STORM SEWER, WITH A MINIMUM

- OUTSIDE-TO-OUTSIDE CLEARANCE OF 18". 2. ALL MANHOLES SHALL BE COATED AND VACUMM TESTED.
- 3. ALL WASETEWATER LINES ARE TO BE CONSTRUCTED OF SDR 26 4. ALL SEGMENTS OF PIPE LOCATED IN ENCASEMENT SHALL BE FULLY RESTRAINED.

05/XX/2024

2525 CR 172
INDUSTRIAL
CITY OF ROUND ROCK
WILLIAMSON COUNTY, TEXAS

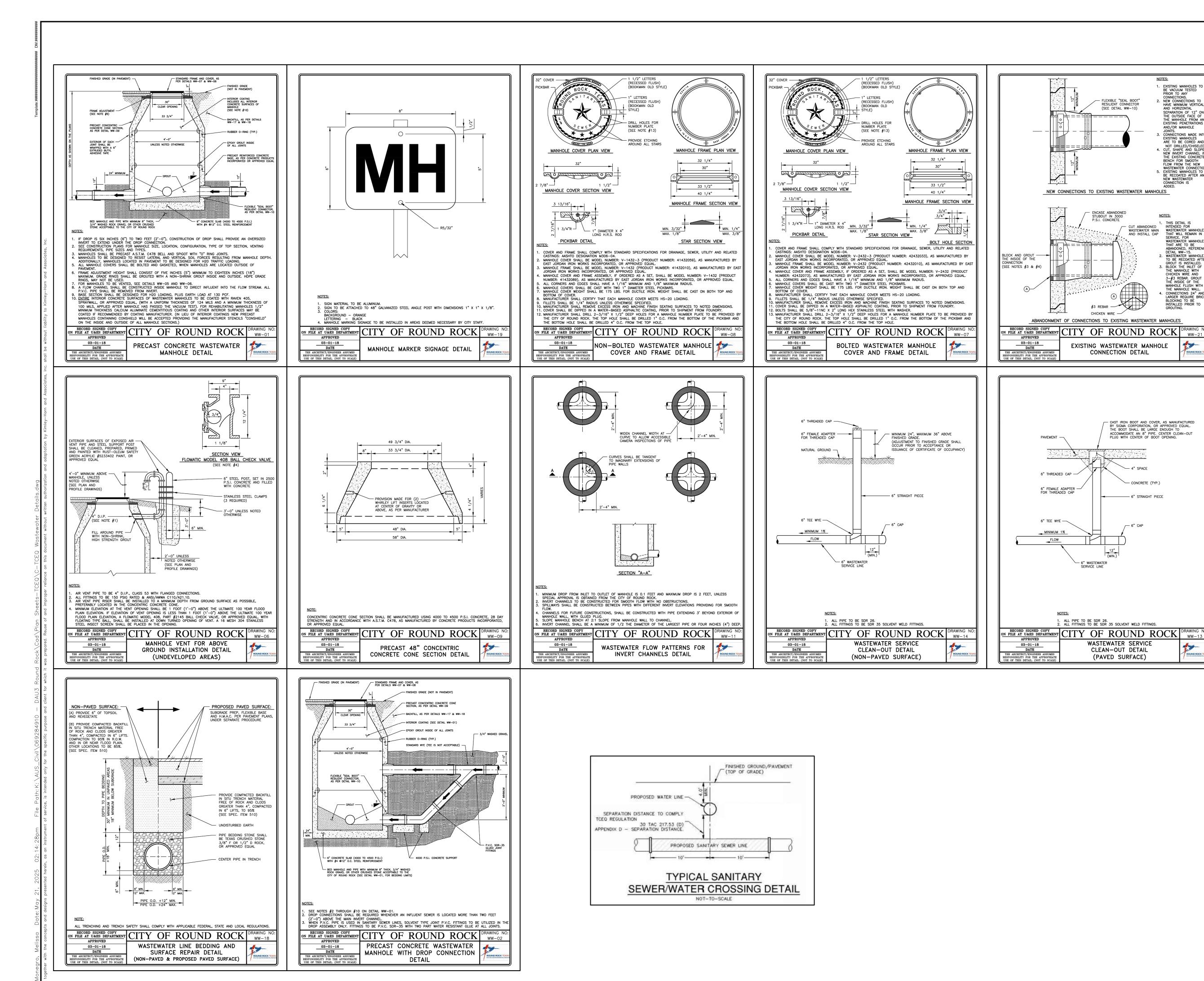
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# **BENCHMARKS**

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OF 5



NICHOLAS C. BROWN 05/XX/2024

**BENCHMARKS** 

NOTES:

NOTES:

CONNECTIONS.

2. NEW CONNECTIONS TO HAVE MINIMUM VERTICAL AND HORIZONTAL

AND FORECUTIAL
SEPARATION OF 12" ON
THE OUTSIDE FACE OF
THE MANHOLE FROM AND
EXISTING PENETRATIONS
AND/OR MANHOLE
JOINTS.
CONNECTIONS MADE INTO
EXISTING MANHOLES
ARE TO BE CORED AND
AND THE INTO CHIESE ED.

NOT DRILLED/CHISELED.
CUT, SHAPE AND SLOPE
NEW INVERT CHANNEL IN
THE EXISTING CONCRETE
BENCH FOR SMOOTH
FLOW FROM THE NEW

1. THIS DETAIL IS
INTENDED FOR
WASTEWATER MANHOLES
P THAT WILL REMAIN IN
SERVICE. FOR
WASTEWATER MANHOLES
THAT ARE TO BE
ABANDONED, REFERENCE
DETAIL WW-15.
WASTEWATER MANHOLES

DETAIL WW—15.

WASTEWANTER MANHOLE
TO BE RECOATED AFTEI
GROUT IS INSTALLED.
BLOCK THE INLET OF
THE MANHOLE WITH
CHICKEN WIRE AND

CHICKEN WIRE AND
3-#3 REBAR. GROUT
THE INSIDE OF THE
MANHOLE FLUSH WITH
THE MANHOLE WALL.
CONNECTIONS 24" AND
LARGER REQUIRE BRICK
BLOCKING TO BE
INSTALLED PRIOR TO
GROUTING.

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INDU

SHEET NUMBER OF 5



# SECTION 6: LIFT STATION / FORCE MAIN SYSTEM APPLICATION

# Lift Station/Force Main System Application

#### **Texas Commission on Environmental Quality**

for Regulated Activities On the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(c)(3)(B)and(c), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Regulated Entity Name: 2525 CR 172 Industrial

#### **Customer Information**

(If different than customer information provided on core data form)

1. The person(s) responsible for providing the engineering certification to the TCEQ pursuant to 30 TAC §213.5(f)(2)(C) during construction and 30 TAC §213.5 (c)(3)(D) upon completion of construction is:

Contact Person: <u>David Barnett</u>
Entity: Amazon.com Services, LLC

Mailing Address: Amazon Tower 1, 101 Platform Way N

City, State: Nashville, TN Zip:  $\underline{37203}$  Telephone:  $\underline{901-438-4156}$  Fax:  $\underline{N/A}$ 

Email Address: barnettu@amazon.com

2. The engineer responsible for the design of this lift station and force main:

Contact Person: <u>Tara Raoof, P.E.</u> Entity: Kimley-Horn and Associates

Mailing Address:  $\underline{10814}$  Jollyville Rd, Avallon IV, Suite  $\underline{200}$  City, State:  $\underline{\text{Austin, TX}}$  Zip:  $\underline{78759}$  Telephone:  $\underline{512-418-1771}$  Fax:  $\underline{\text{N/A}}$ 

Email Address: <a href="mailto:tara.raoof@kimley-horn.com">tara.raoof@kimley-horn.com</a>

Texas Licensed Professional Engineer's Serial Number: 141851

#### Project Information

3.	This project is for the construction or replacement of:
	Lift Station only.

**Table 1 - Geologic or Manmade Features** 

Line	Station to Station	Type of Feature
N/A	N/A	N/A
	to	

9. $\overline{X}$ Existing topographic contours are shown and labeled. The contour interval is $\underline{1}$ feet. (Contour interval must not be greater than 5 feet).
10. X Finished topographic contours are shown and labeled. The contour interval is1 feet. (Contour interval must not be greater than 5 feet).
Finished topographic contours will not differ from the existing topographic configuration and are not shown.
11. 100-year floodplain boundaries
<ul> <li>Some part(s) of the project site is located within the 100-year floodplain. The floodplain is shown and labeled.</li> <li>No part of the project site is located within the 100-year floodplain.</li> </ul>
The 100-year floodplain boundaries are based on the following specific (including date of material) sources(s): FEMA Flood Insurance Rate Map (FIRM) dated December 20, 2019
12. 5-year floodplain:
<ul> <li>After construction is complete, no part of this project will be in or cross a 5-year floodplain, either naturally occurring or manmade. (Do not include streets or concrete-lined channels constructed above sewer lines.)</li> <li>After construction is complete, all sections of the force main located within the 5-year floodplain will be encased in concrete or capped with concrete. These locations are listed in the table below and are shown and labeled on the Site Plan. (Do not include streets or concrete-lined channels constructed above sewer lines.)</li> </ul>
Table 2 - 5-Year Floodplain

Line	Sheet	Station to Station
N/A	N/A	N/A
	of	to
	of	to
	of	to

13. All known wells (oil, water, unplugged, capped and/or abandoned, test holes, etc.):

13. All known wens (on, water, unplugged, capped and/or abandoned, test noies, etc.).
If applicable, this must agree with Item No. 15 on the Geologic Assessment Form.  There are (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply)
<ul> <li>The wells are not in use and have been properly plugged.</li> <li>The wells are not in use and will be properly plugged.</li> <li>The wells are in use and comply with 16 TAC Chapter 76.</li> </ul>
X There are no wells or test holes of any kind known to exist on the project site.
14. X Legal boundaries of the site are shown.

#### Plan and Profile Sheets

The construction drawings and technical specifications will not be considered for review unless they are the **final plans and technical specifications** which will be used by the contractor for bidding and construction.

#### Items 15 – 18 must be included on the Plan and Profile sheets.

- 15.  $\boxed{X}$  The equipment installation construction plans must have a minimum scale of 1" = 10'. Plan sheet scale: 1" = 5 '.
- 16. X Locations, descriptions and elevations of all required equipment and piping for the lift station and force main are shown and labeled.
- 17. X Air Release/Vacuum Valves will be provided at all peaks in elevation of the proposed force main. These locations are listed in the table below and labeled on the appropriate plan and profile sheets.

Table 3 - Air Release/Vacuum Valves

Line	Station	Sheet
4" FM in Valves Vault	N/A	47
		of

- 18. X The **final plans and technical specifications** are submitted for the TCEQ's review. Each sheet of the construction plans and specifications are dated, signed, and sealed by the Texas Licensed Professional Engineer responsible for the design on each sheet.
- 19. X Attachment A Engineering Design Report. An engineering design report with the following required items is attached:
  - X The report is dated, signed, and sealed by a Texas Licensed Professional Engineer.
  - $\overline{X}$  Calculations for sizing system.
  - X Pump head calculations, including, but not limited to, system head and pump capacity curves, head loss calculations, and minimum and maximum static head C values for normal and peak operational conditions.
  - X 100-year and 25-year flood considerations.
  - X Total lift station pumping capacity with the largest pump out of service.
  - X Type of pumps, including standby units.
  - X Type of pump controllers, including standby air supply for bubbler controllers, as applicable.

X	ump	cycle	time
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- X Type of wet well ventilation; include number of air changes for mechanical ventilation.
- X Minimum and maximum flow velocities for the force main.
- X Lift station security.
- X Lift station emergency provisions and reliability.

#### Administrative Information

- 20. X Upon completion of the wet well excavation, a geologist must certify that the excavation was inspected for the presence of sensitive features and submit the signed, sealed, and dated certification to the appropriate regional office.
- 21. X The TCEQ Lift Stations and Force Mains General Construction Notes (TCEQ-0591) are included on the General Notes Sheet of the Final Construction Plans for this lift station and/or force main system.
- 22. X Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
- 23. X Any modification of this lift station/force main system application will require TCEQ approval, prior to construction, and may require submission of a revised application, with appropriate fees.

### Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Lift Station/Force Main System Application** is hereby submitted for TCEQ review and executive director approval. The system was designed in accordance with the requirements of 30 TAC §213.5(c)(3)(C) and 30 TAC Chapter 217, and prepared by:

Print Name of Licensed Professional Engineer: Tara Raoof, P.E.

Place engineer's seal here:

Date: <u>05/16/2024</u>

Signature of Licensed Professional Engineer:



# **ENGINEERING DESIGN REPORT**

# Lift Station Report

# 2525 CR 172 Industrial Lift Station Williamson County, Texas

May 15, 2024

Prepared for:

Amazon.com Services, LLC Amazon Tower 1 101 Platform Way N Nashville, TN 37203 (901) 438-4156



Prepared by:



(Texas Firm Registration No. F-928) 10814 Jollyville Road, Campus IV, Suite 200 Austin, Texas 78759

KHA Project No. 069284918

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l.	PROJECT BACKGROUND	2
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	2. Total Dynamic Head	4
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	4. Net Positive Suction Head (NPSH)	
	5. Vent Sizing	
	6. Force Main Velocity and System Flush Time	6
D.	Surge Protection and Water Hammer	6

#### **APPENDIX**

**Appendix A – Lift Station Design Calculations** 

Appendix B – Pump Data

Appendix C - Control Panel Data

#### I. PROJECT BACKGROUND

Kimley-Horn and Associates, Inc. (KHA) was retained by Amazon.com Services, LLC to design an industrial building and miscellaneous smaller buildings. The site is located south of the McNeil Rd and County Road 172 intersection within the Round Rock City Limits in Williamson County, Texas. This report addresses the design of the proposed preliminary lift station to serve the 2525 CR 172 Industrial (Amazon) site.

The Amazon Lift Station will be constructed during the development of the site and will be in the southeast quadrant of the Amazon site development, shown below in Figure 1. The lift station is at a high point on the site. Wastewater flows from the site will gravity flow to the lift station, and then will be pumped via force main to a public manhole. The design of the lift station will serve the Amazon development only, which contributes 84.23 gallons per minute (GPM) (0.12 million gallons per day (MGD)).

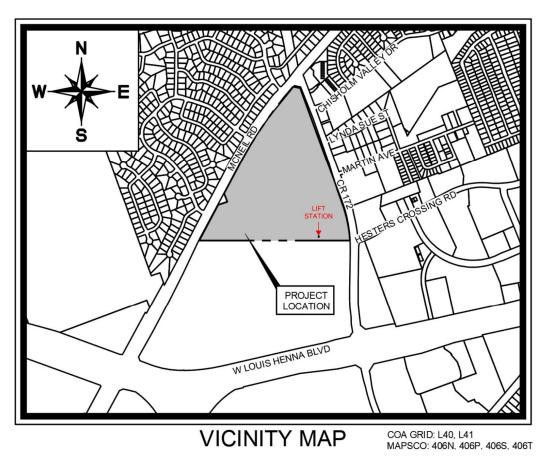


Figure 1 – Project Location Map

#### II. INTRODUCTION

The Amazon Lift Station is defined as a private lift station necessary to serve the Amazon site only. The Amazon Lift Station will have an ultimate capacity design. The design will have a pumping capacity of 100.42 GPM (0.14 MGD) to provide capacity for wastewater flows on-site. The purpose of the design is to serve the entire Amazon site which has a total area of approximately 69.56 acres with 60 living unit equivalents (LUEs).

The design construction will include the private Amazon Lift Station and one (1) 4-inch HDPE force main that will connect to a proposed 4-foot public manhole. This force main will meet the TCEQ velocity requirements and will have flows of 3.04 feet per second (fps) which will be maintained over time. The force main will run for 98 linear feet (this includes the length of the force main inside the wet well) before out falling to the public 4-foot wastewater manhole that leads into a public 8-inch gravity sewer main.

The Peak Wet Weather Flow (PWWF) for the proposed development has been calculated to be 84.23 GPM. The PWWF has been rounded up to 88 GPM to allow for a more conservative design for the Lift Station. The Total Dynamic Head (TDH) calculated for the design flow was approximately 31.51 ft. The lift station design will consist of two (2) pumps with a flow at operating point of 99 GPM to provide the required pumping capacity for the design flow of 88 GPM at the required TDH (operating TDH is approximately 32.46 ft). Rail and piping will be installed for ease of installation for the system. The wet well has been designed to accommodate the detention volume required by the design flow of 88 GPM.

#### III. DESIGN

#### A. Lift Station Site Layout

Refer to site layout in lift station plans. The lift station site footprint is approximately 30.76 feet X 53.83 feet (1634.49 square feet). A 12-foot-wide access concrete driveway will be constructed within the lift station and will tie-in to the Amazon site. A pedestrian barrier and truck yard perimeter safety chain-link fence will be installed through-out the site. The pedestrian safety chain-link security fence will stand five feet in height with iron bars spaced on four-inch centers. The site will have dedicated slab for the odor control unit.

#### B. Planning Flows

The lift station is designed for 60 LUEs within the proposed project site. The planning flows were calculated using City of Round Rock's Design Criteria. The Average Dry Weather Flow (ADWF) of 11.67 GPM (0.02 MGD) is determined based on the number of LUEs and a capacity of 280 gallons per day (GPD) per LUE. Based on the calculated ADWF, the Peaking Factor (PF) was determined to be 4.11. The Amazon lift station will accommodate the flow for 69.56 total acres, so the Inflow and Infiltration (I&I) was determined to be based 36.23 GPM (0.05 MGD) at 750 gallons per acre. The Peak Dry Weather Flow (PDWF) was determined by applying a peaking factor of 4.11 to the ADWF resulting in a PDWF of 48 GPM (0.07 MGD). The Peak Wet Weather Flow (PWWF) was determined by the sum of the PDWF and the I&I leading to a PWWF of 84.23 GPM (0.12 MGD) for the Amazon development. The PWWF was rounded up to a design flow of 88

GPM to remain conservative. See Exhibit 1 for the properties and flows planned for the preliminary lift station.

The design of the lift station will serve up to 88 GPM, which includes 60 LUEs and 69.56 acres of proposed development in the sewer shed contributing to the lift station.

#### C. Lift Station & Force Main Design Calculations

All the calculations performed for the proposed lift station and force main are in accordance with the *City of Round Rock Water and Wastewater Systems Design Criteria*, as well as *TCEQ Chapter 217*. All components of this lift station and force main system also comply with City of Round Rock standard specifications. The Amazon lift station will have one (1) 4-inch discharge line coming from the pumps. A valve to the force main will remain closed until the lift station design is installed. The force main is approximately 98 linear feet and made of 4-inch HDPE DR9. The detailed lift station and force main calculations can be found in Appendix A.

#### 1. Wet Well Sizing

A 6-foot diameter precast concrete wet well was proposed for the Amazon lift station. The depth of the wet well was determined with the design flow of 88 GPM with two (2) 3 HP pumps. Minimum 6 minutes cycle time and the design flow of 88 GPM was used to determine the detention volume. With the selected 6-foot diameter wet well, there is 3.44 feet of storage (between the lag pump on and all pumps off) which resulted in the total depth of the wet well to be approximately 36 feet.

#### 2. Total Dynamic Head

The Total Dynamic Head (TDH) was derived by summing up static head, friction loss and the minor losses. The static head of 27.94 feet was calculated by subtracting the anticipated "All Pumps Off Elevation" at 780.50 feet from the highest point along the force main system which is approximately 808.44 feet. Hazen-Williams Coefficient (C) of 100 and 120 were utilized in the design to calculate the friction losses in the system. Minor losses were incorporated based on anticipated valves and fittings for the proposed lift station discharge piping and the force main system. HDPE DR-9 force main was proposed for this development with an inner pipe diameter of 3.44 inches. The velocity in the force main will be 3.04 fps, which meets TCEQ requirements of wastewater flows between 3 fps and 7 fps.

The force main system includes one 4-inch HDPE DR9 force main that runs for approximately 98 feet before discharging into a downstream public manhole that connects to a public gravity wastewater line. The lift station operator can turn on both pumps periodically to ensure there is no blockage in the force main after installation.

TDH for the 4-inch force main was calculated to be 31.51feet (C=100) at the design flow of 88 GPM. System curves were generated and are shown in Figure 2.

# 

#### Figure 2 -Phase I Lift Station Pump and System Curves

- Pump Model Information

----- C=100

#### 3. Pump Selection

A duplex system was adopted in the design phase. The engineer's intention was to select a pump that can serve the design. Two (2) 3 HP pumps with 4-inch impeller were selected to accommodate the design flow of 88 GPM. Pump information and operating conditions are summaries in Table 1 below. Additional pump data can be found in Appendix B.

Table 1 – EBARA Submersible Pump Design

EBARA Submersible Pump Model	Force Main Diameter (in)	Force Main Length (ft)	Operating Flow (GPM)	Total Dynamic Head (ft)	Velocity (ft/s)	Impeller Size (in)
EBG-33	4	98	99	32.46	3.04	4

#### 4. Net Positive Suction Head (NPSH)

The Available Net Positive Suction Head (NPSHa) was calculated by using the equation from the *City of Round Rock Water and Wastewater System Design Criteria*. Barometric pressure (PB) of 33.4 feet and vapor pressure (PV) of 1.4 feet were given and used in the calculation. Minimum static suction head (Hs) was estimated to be 1.4 feet from All Pumps Off elevation to the eye of the impeller. Since the selected pumps are submersible without suction pipe, friction loss (Hfs) is expected to be negligible. Therefore, NPSHa was determined to be 32.50 feet. Which is greater than Required Net Positive Suction Head (NPSHr) of the pump.

#### 5. Vent Sizing

Per the TCEQ design criteria, the maximum passive ventilation system is to not exceed air velocity no greater than 600 feet per minute. Based on the pumping capacity during the initial design phase, the air velocity has been determined to be 539.18 feet per minute with a diameter of 2-inch diameter vent pipe. One 4-inch diameter vent pipes will be used. The vent pipe will be connected to a passive Carbtrol Activated Carbon odor control unit as shown on the plans.

#### 6. Force Main Velocity and System Flush Time

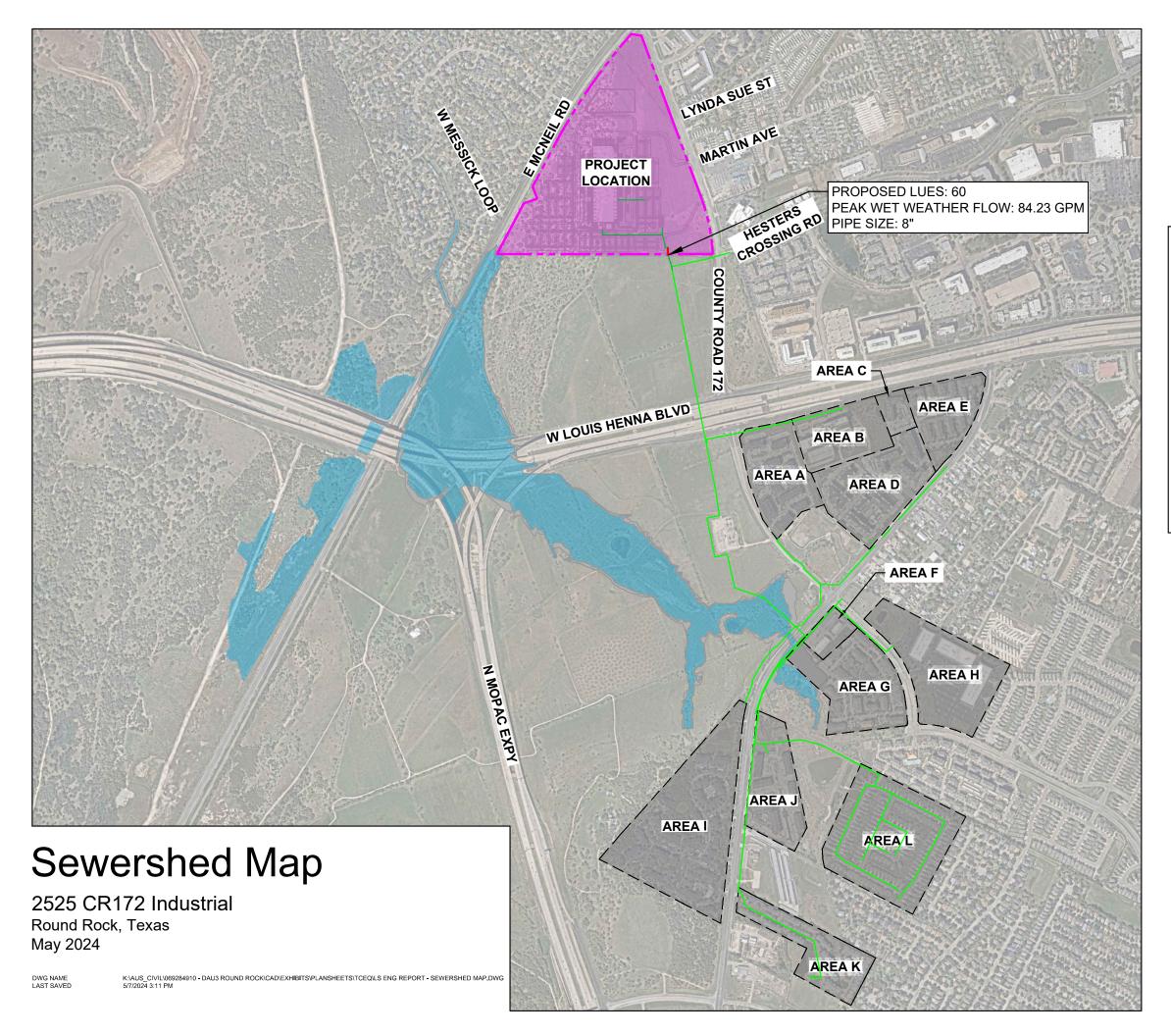
The proposed 4-inch force main shall be HDPE. With the design flow of 88 GPM, the velocity inside the 4-inch diameter force main was calculated to be approximately 3.04 fps. The total flush time for the 4-inch diameter force main based on the Peak Wet Weather Flow during the design is 51.07 minutes. Detailed detention time and flush time calculations can be found in Appendix A.

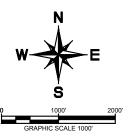
#### D. Surge Protection and Water Hammer

Surge can occur in the pipes from the pumps starting and stopping and valves opening and closing. The surge pressure in the pipe shall be 63.61 psi based on the design flow of 88 GPM. The recommended pipe is DR9, which is rated for 250psi, which is greater than the surge pressure in the pipe. Therefore, the pipe will not be damaged due to a water hammer. Refer to Surge Pressure Calculations in Appendix A.

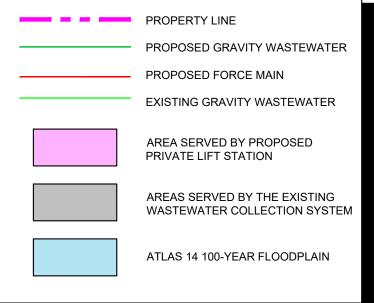


# **EXHIBIT 1 – WASTEWATER FLOW PROJECTIONS**





### LEGEND





10814 Jollyville Road Campus IV, Suite 200 Austin, TX 78759 512-418-1771 State of Texas Registration No. F-928

OTE: THIS PLAN IS CONCEPTUAL IN NATURE AND HAS BEEN PRODUCED WITHOUT



# APPENDIX A – LIFT STATION CALCULATIONS

#### Flow Calculations

### Maximum Build Out

LUEs (total)	60	
Area (total)	69.56	acres

### Flow Properties

Q/LUE	280	GPD/LUE
1&1	750	GPD/Ac
Peaking Factor	4.11	

#### Flow Values

Avg Dry Weather Flow (F)	11.66667	gpm
Peak Dry Weather Flow	48	gpm
I & I Flow	36.22914	gpm
Peak Wet Weather Flow	84.22914	gpm
Minimum Dry Weather Flow	2	gpm

#### **Design Flow**

Design Flow	84.22914	gpm
	0.19	cfs

### **Operating Information**

One Pump Operating Flow	88	gpm
One Fullip Operating Flow	0.2	cfs
Two Pump Operating Flow	88	gpm
Two Fullip Operating Flow	0.2	cfs

#### **Wet Well Sizing Calculations**

#### **City of Round Rock**

Natural Ground Elev. =	814.50					
Lowest Top of MH or Service=	815.00	I	11		11	34.50'
Should be 6" above the natural	l ground	1	11		11	
elevation			11		11	
High Water Alarm =	784.94	<u> </u>	11			
			1.1		1 1	
			11		11	
			11		11	
			11		11	
			11		1.1	
Invert In Elev. =	785.94	i	ii		ii	
mivelt in Elevi =		l i	1.1		1.1	2.00
Lag Pump On Elev. =	783.94	()	11		11	
Suggested: 784.94		ĺ	11		11	
		I	1.1		1.1	
		1	1.1		11	
		I	1.1		11	
Lead Pump On Elev. = _	782.94	()	11		11	
		I	1.1		11	
		I	11		11	
Top of Pump(s) Elev. = _	782.83	l l	11		11	2.44'
All Division - Off Flori	700 50					
All Pumps Off Elev. = _	780.50	()	Pump 1		Pump 2	
			-		-	0.17'
Bottom of Pump(s) Elev. =	780.33					0.17
Wet Well Invert Elev. =			/ \	J .	/ \	
VOC VVS.II III VOIT EIGV. =	780.00		,		,	0.33'
Bottom of Concrete Base =	779.00					
Invert In to WW Invert	5.94	-				

Generator? (yes or no)
------------------------

### Needed Input

Longest Power Outage	20
Wetwell Diameter (feet) =	6
Elev. of Deepest Invert In =	808.50
Dual Pump Capacity (gpm) =	88
Single Pump Capacity (gpm) =	88
Pump Height (ft) =	2.33
Minimum Submersed Depth (ft) =	0.50
Natural Ground Elev. =	814.50
Bottom of Pump to Invert Dimension (ft) =	0.33

#### **Wet Well Depth Calculations**

Storage Depth (ft) =	0.00
Total Depth (ft) =	36.00

#### **Volume Calculations**

Volume Galculations	
Gallons/Foot of Wet Well =	211.51
Minimum Cycle Time (TCEQ) =	6
Required Active Volume (gal) =	132.00
Depth Required Active Volume (feet) =	0.62
Active Volume Provided (gal) =	516.71
Active Volume Depth (feet) =	2.44
Total Emergency Storage Needed (gal) =	1684.58
Required Emergency Storage Depth (feet) =	7.96
Available Emergency Storage Depth (feet) =	30.06
Available Emergency Storage Volume (gal) =	6357.26

#### **TDH CALCULATIONS**

Minor Loss	K-Value	Qty.	Totals					
Discharge Into Manhole	1.00	1	1.00		C =	100.00		
90 Degree Bend	0.70	6	4.20		C =	120.00		
45 Degree Bend	0.50	C	0.00					
22.5 Degree Bend	0.30	C	0.00					
11.25 Degree Bend	0.20	C	0.00					
5.625 Degree Bend	0.10	C	0.00					
Reducer	0.50	1	0.50					
Gate Valve	2.50	2	2. 5.00					
Check Valve	0.20	2	<mark>?</mark> 0.40					
	Ī	<(total) =	11.1 Minor loss	s coefficient for valv	es, ben	ıds, tees, ar	nd other fitt	ings
_								
Minor Losses	1.59	t	Headloss due to a fitt	ing				
Friction Head(c=120)	1.98							
Friction Head(c=140)	1.41	't	Calculated using Haz	en Williams equation	on; C va	lue based o	on pipe ma	terial
•								
"All Off" Elev.	780.50							
FM High-Point Elevation	808.44	This is the ulti	mate elevation that the	pumps will have to	pump	against.		
Static Head	27.94	Only a functio	n of elevation	Q	D (in)	V	length	Pipe Area
'		-		88	3.44	3.04	98	0.065
				Nominal	4			

Total Head (C=100) 31.51

Total Head (C=120) 30.95

This is the total amount of pressure that the pumps will overcome to see that the wastewater is sufficiently lifted beyond the highest point to wherever the FM is discharging

Pipe Thickness 0.5

Diameter is based on values from North American Pipe Co or JM Eagle

4.44

Outer Diameter

Detention Times Calculation
One Pump Flow (gpm) 88
Two Pump Flow (gpm) 88

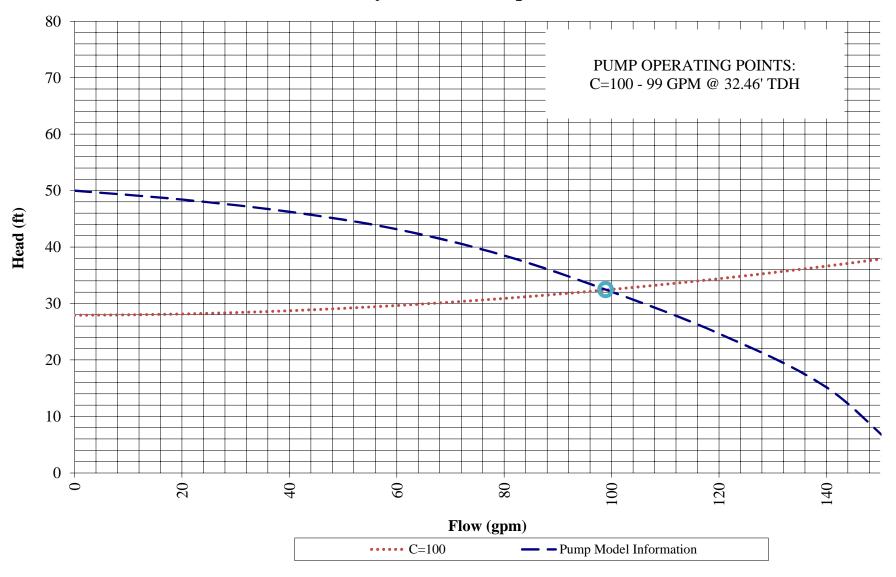
Active	Volume	(gal)
--------	--------	-------

	Primary	gpd	i (gpm)	Q (gpm)	Tf	Te	Td	Tc	
Average Daily Flow	517	16,800	11.66667	88	44.29	6.77	51.06	102.12	
Peak Dry Weather Flow	517	69,120	48	88	10.76	12.92	23.68	47.37	
Peak Wet Weather Flow	517	121,290	84.22914	88	6.13	5.78	11.91	23.83	
Minimum Dry Weather Flow	517	2,880	2	88	258.36	6.01	264.36	528.73	
						Ī	Max detent	ion time	
Minimum Cycle Time	517	63,360	44	88	11.74	11.74	23.49	46.97	<== always occurs at 1/2 outflow (normally PWWF)

Tf = time to fill the wet well in minutes

To entime to the time west well in limitates
To entime to empty the west well in minutes. This considers wastewater flowing in at the same time the west well is emptying.
To entention time (sum of Tf and Te)
To encycle time is equal to twice the detention time

# **System and Pump Curves**



#### Force Main Flush Time

Wetwell dia (ft)	6
$\Delta$ (on - off) (ft)	2.44

i (gpm) 11.66666667 \*USE AVERAGE DAILY FLOW!

Pumped Q (gpm) 88
FM Length (ft) 98
FM Velocity (fps) 3.04

Wet Well Filling Time 44.292 mins

Pump Run Time 6.77 mins

Wet Well Det. Time 51.06 mins

Flushing Cycles 0.08 cycles

Flush Cycle Whole # 0.00 <== whole cycles
Flush Cycle Decimal# 0.08 <== decimal cycles

Flush Time 0.54 mins

Flush Time (COA - using PWW 0.415951313

#### COA Tflush - average dry weather flow

Tf [min]	44.29
Te [min]	6.77
Foce Main Length [ft]	98
θ (pump cycle time) [min]	6
Vfm (force main velocty) [fps]	0.40
Tflush	51.06938368

SUBCE DRESSURE CALCULA	TIONS						
SURGE PRESSURE CALCULA	IONS_						
INSTRUCTIONS: REFER TO UNI-BELL	IANDROOK (	) OF DVC DIDE	LIDDATE E				
BASED ON PIPE MATERIAL.							
BASED ON PIPE WATERIAL.	VERIFT VVIII	7					
Critical Time, tc (s)							
	2 L / a						
10-	Z L / u						
	Length of P	ine (ft)					
L =		ft		Approximate M	laximum Pine I	Length from pla	ns
		466					
a	$=\frac{1}{\sqrt{1}}$	7/-	<u>- 7</u>				
	_ / 1	<b>_</b>	$^{r}a_{i}$				
	$\sqrt{}$	E	'*t _				
			,				
	Bulk Moduli		ty of the liqu				
K =	3.00E+05	lb/in2		@ 70oF			
	Madulus of	Floaticity of	nina matari	al (lb/in2)			
	Modulus of <b>1.53E+05</b>		pipe materia	ai (ib/in2)			
	1.53E+05	ID/IIIZ					
D-	Inside Pipe	Diamotor (ir	)				
D =		in	1)				
	3.44						
e =	Pipe Wall T	hickness (ft)					
e =	0.50	in					1
a =	1,223	ft/s		see Table 6-2	for typical rang	e	
tc =	0.16	S		using a =	1,223	ft/s	
	- D /						
ΔΗ =	-a Dv/g						
a =	Elastic Wav	e Speed (ft/s	s)	Calculated Pre	viously		
				- 3.53.3104 1 10			
g =	Gravitationa	al Constant (	ft/s2)				
g =	32.2	ft/s2	-		-		
			(6) ( )				
$\Delta \varpi =$		iquid velocit	y (ft/s)				
	v2 - v1	Λ		Du Oant in 1	Daine alia In		
$\Delta \overline{\omega} = Q2$	(Q2 - Q1) / <b>0.00</b>	gpm (min. f	low)	By Continuity F	rincipie		
Q2 = Q1 =		gpm (min. i					
Q1 = A =		ft2	purip 110W)				
		ft/s					
$\Delta \varpi =$	-3.0	11/5					

		ΔH =	115.42	ft		uoina a –	1,223	ft/s (calculated	oroviouoly)
		Δη –	113.42	11	_	using a =	1,223	It/S (Calculated	previously)
		-			ssociated with				
	the rapid ve	elocity chang	je across a v	vaterhamme	er wave (Psu	rge) is:			
		Pressure	49.97	psi		using a =	1,223	ft/s (calculated	previously)
Total Pressure i	n Pipe Due	to Waterha	mmer/Surg	e and TDH	:				
The Total Dynamic Head (TDH) @ peak flow was calculated as:				31.51	ft (See system curve for Hazen-Williams)				
,									•
Therefore, the pressure in the pipe at this TDH (P TDH) is:			H) is:		13.64	psi			
			,						
Therefore, the to	tal pressure	that the PVC	C pipe (P tot	)will be sub	jected to dur	ing water h	ammer cond	ditions @ pea	ζ
	flow is:		, , ,						
		Ptot = Psurge + PTDH							
			J						
		Pressure	63.61	psi		using a =	1,223	ft/s (calculated	oreviously)
		1.555310	33.33	1- 2.		usg u =	.,3	5 (00.00.0100	



# APPENDIX B - PUMP DATA

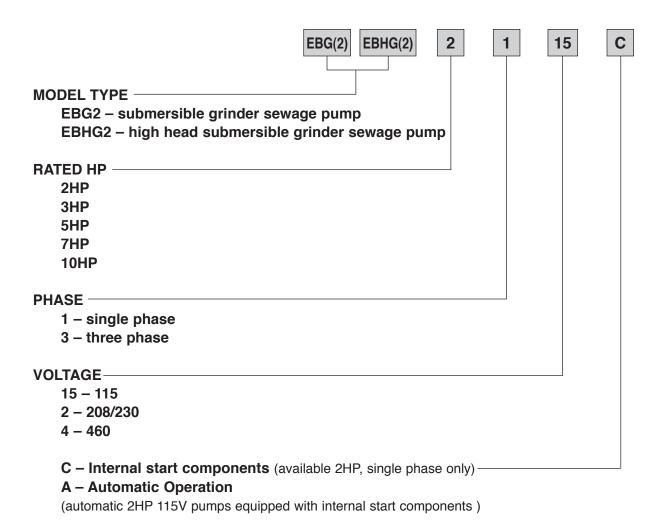
# **Contents**

# Model

EBG2-2115(A)	EBG-31	EBHG2-212C
EBG2-212C	EBG-33	EBHG2-21
EBG2-21	EBG-51	EBHG2-23
EBG2-23	EBG-53	EBHG-31
	EBG-103	EBHG-33
		EBHG-51
		EBHG-53
		EBHG-71
		EBHG-73

Section	Page
Specifications	1-191
Selection Chart	1-192
Performance Curves	1-195
Dimensions	1-201
QDC Information	1-204
Motor Data wiring diagrams motor electrical data	1-207

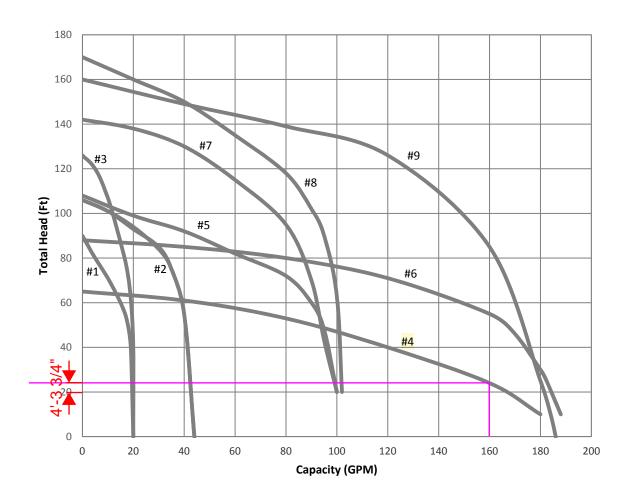
# **Model Designation**



# **Specifications**

Discharge Size/Connection	1 <sup>1</sup> / <sub>4</sub> " NPT, 2 <sup>1</sup> / <sub>2</sub> ", 3" ANSI (3, 5, 7 <sup>1</sup> / <sub>2</sub> 10 HP)
Range of HP	2, 3, 5, 7½ 10 HP
Range of Performance	Capacity 20 to 190 GPM
	Head 8 to 175 feet
Limitation	
Maximum Water Temperature	120°F (49°C) [140°F (60°C) intermittent]
Synchronous Speed	3450 RPM
Materials	
Casing	Cast Iron ASTM A-48, Class 30
Impeller	Ductile Iron (2HP models)
	Cast Brass (3-10HP models)
Shredding Ring	Hardened 440C Stainless Steel, 56-60 Rockwell C
Grinder Impeller	Hardened 440C Stainless Steel, 56-60 Rockwell C
Shaft	416 Stainless Steel
Motor Housing	Cast Iron ASTM A-48, Class 30
Fastener	304 Stainless Steel
Mechanical Seal	Double Mechanical Seal, Type 21
Material – Upper Side	Silicon Carbide/Silicon Carbide
Material – Lower Side	Silicon Carbide/Silicon Carbide
Impeller Type	Recessed Vortex
Bearing – Upper/Lower	Single Row, Ball, Oil Lubricated
<ul><li>Sleeve Bearing</li></ul>	Bronze with Oil Groove (3, 5, 71/2 10 HP)
Motor	Oil-filled, Insulation Class F
Single Phase	115 V, 208/230V for 2 HP
	208/230V only for 3, 5, 7 <sup>1</sup> / <sub>2</sub> HP
Three Phase	208/230V, 460V
Motor Protection	Internal moisture detection
Single Phase	Built-in Automatic Overload Protection
Three Phase	Non-overloading on-winding temperature sensor
	Submersible Cable 30 ft.
	Submersible Cable 40 ft. (3, 5, 7½ 10 HP)
	Consult factory for additional cable lengths.
Accessories	QDC System

# **Selection Chart**



- 1 EBG2-2115 2HP
- 2 EBG2-2 2HP
- 3 EBHG2-2 2HP
- 4 EBG-3 3HP
- 5 EBHG-3 3HP
- 6 EBG-5 5HP
- 7 EBHG-5 5HP
- 8 EBHG-7 7.5HP
- 9 EBG-103 10HP

#### Specifications - 2 HP

#### Pump Model:

Pump shall be of the centrifugal type with an integrally built-in grinder unit and submersible type motor. The grinder unit shall be capable of macerating all material in normal domestic and commercial sewage, including reasonable amounts of foreign objects such as sanitary napkins, disposable diapers, thin rubber, small wood, plastic and the like to a fine slurry that will easily pass through the pump and 1-1/4" NPT discharge.

0	perating	<b>Conditions:</b>

The pump shall have a capacity of \_\_\_\_\_ GPM at a total head of \_\_\_\_\_ feet, and shall use a motor rated at 2 HP and 3450 RPM.

#### Pump Impeller:

Ductile Iron threaded on a stainless steel shaft. The impeller shall be of the recessed vortex type to provide an unobstructed passage through the volute for the ground solids.

#### **Grinder Construction:**

Both grinder impellers and shredding ring shall be of 440C stainless steel hardened to 56-60 Rockwell C. The grinder assembly shall consist of a grinder impeller and shredding ring mounted directly below the volute passage. The grinder impeller is threaded to a stainless steel shaft, locked with a screw and washer. The shredding ring shall be pressed into the cast iron volute for easy removal. All grinding of solids shall be from the action of the grinder impeller against the shredding ring.

#### Seals:

Type 21, dual mechanical seal construction mounted in tandem, shall protect the motor. Primary seal shall be silicon / carbide. Secondary seal shall be silicon / carbide. The seal face shall be lapped to a flatness of one light band. An electrode shall be mounted in the seal chamber to detect water entering the chamber through the lower seal. Water in the chamber shall cause a red light to turn on at the control box. This signal shall not stop the motor, but shall act as a warning only, indicating service is required. Single and three phase pumps shall have an internal seal leak probe that signals an alarm in the control panel in the case of water intrusion into the seal chamber [EBG2-2115(A), EB(H)G2-21C excluded].

#### Motor

The pump motor shall be of the submersible type, rated 2 HP, 3450 RPM. The motor shall be for 60 Hz, either 115, 208, 230, 460 volt, single or three phase operation. Single-phase motors shall be capacitor start, capacitor run type for high starting torque. For the 115V motor, run capacitor ratings shall not exceed 300 volts. The motor will utilize mechanical starting switch. Start and run capacitors, and electronic relay for operating the motor will be found in the control box. Major motor operating temperature must not exceed Class B ratings.

The stator winding shall be of the open type with Class F insulation. Winding housing shall be filled with clean, high dielectric oil that lubricates bearings and seals, transferring heat from windings and rotor to the outer cast housing.

The motor shall have two heavy-duty ball bearings to support the pump shaft, taking radial and thrust loadings. Ball bearings shall be designed for a minimum 50,000 hours B-10 life. The stator shall be pressed into the motor housing. The common motor pump and grinder shaft shall be of 416 SST, threaded to take the pump and grinder impeller.

Single-phase motors shall have automatic reset overload protection attached to the top end of the motor windings to stop the motor if the motor winding temperature reaches 130 degrees C. The high temperature shut-off will cause the pump to cease operation, should a control failure cause the pump to run in a dry wet well. The overload shall automatically reset when the motor cools to a safe operating temperature. Three phase motors contain temperature sensors with (2) wires for attachment to the control panel.

#### **Power Cord:**

The motor power cord shall be 12 Ga. SOW/SJOWA or SOOW. The cable jacket shall be sealed at the motor entrance by means of a rubber compression washer and compression nut. An epoxy filled tube shall seal the outer cable jacket and individual leads to prevent water from entering the motor housing. Individual conductor strands shall be soldered within.

#### Specifications – 3, 5, 7.5 10 HP

#### Pump Model:

Pump shall be of the centrifugal type with an integrally built-in grinder unit and submersible type motor. The grinder unit shall be capable of macerating all material in normal domestic and commercial sewage, including reasonable amounts of foreign objects such as sanitary napkins, disposable diapers, thin rubber, small wood, plastic and the like to a fine slurry that easily pass through the pump and 2" discharge pipe. Discharge shall be standard with slotted bolt pattern to accommodate either a 2.5" or 3" 150 lb. ANSI flange.

Operating Conditions:			
The pump shall have a capacity of	GPM at a total head of	feet, and shall use a motor rated at	HF
and 3450 RPM			

#### Pump Impeller:

Cast brass and threaded on a stainless steel shaft. The impeller shall be of the recessed vortex type to provide an unobstructed passage through the volute for the ground solids.

#### **Grinder Construction:**

Both grinder impellers and shredding ring shall be of 440C stainless steel hardened to 56-60 Rockwell C. The grinder assembly shall consist of a grinder impeller and shredding ring mounted directly below the volute passage. The grinder impeller is threaded to a stainless steel shaft, locked with a screw and washer. The shredding ring shall be pressed into an iron holding flange for easy removal. The flange shall be provided with tapped back-off holes so screws can be used to push the shredding ring from the housing. All grinding of solids shall be from the action of the grinder impeller against the shredding ring.

#### Seals:

Type 21, dual mechanical seal construction mounted in tandem, shall protect the motor. Primary seal shall be silicon / carbide. Secondary seal shall be silicon / carbide. The seal face shall be lapped to a flatness of one light band. A double electrode shall be mounted in the seal chamber to detect water entering the chamber through the lower seal. Water in the chamber shall cause a red light to turn on at the control box. This signal shall not stop the motor, but shall act as a warning only, indicating service is required.

#### Motor:

The pump motor shall be of the submersible type, rated \_\_\_\_\_ HP, 3450 RPM. The motor shall be for 60 Hz, either 208, 230 or 460 volt, single or three-phase operation. Major operating temperature must not exceed Class B ratings.

The stator winding shall be of the open type with Class F insulation. Winding housing shall be filled with clean, high dielectric oil that lubricates bearings and seals, transferring heat from windings and rotor to the outer cast housing.

An upper motor bearing cap shall be a separate casting for easy mounting and replacement. The motor shall have two heavy-duty ball bearings to support the pump shaft, taking radial and thrust loadings. A sleeve guide bushing is mounted directly above the lower seal to take radial load and act as a flame path for the seal chamber. Ball bearings shall be designed for a minimum 50,000 hours B-10 life. The stator shall be pressed into the motor housing. The common motor pump and grinder shaft shall be of 416 SST, threaded to take the pump and grinder impeller.

Motors shall have a heat sensor thermostat attached to the top end of the motor windings to stop the motor if the motor winding temperature reaches 200 degrees F. The high temperature shut-off will cause the pump to cease operation, should a control failure cause the pump to run in a dry wet well. The thermostat shall automatically reset when the motor cools to a safe operating temperature.

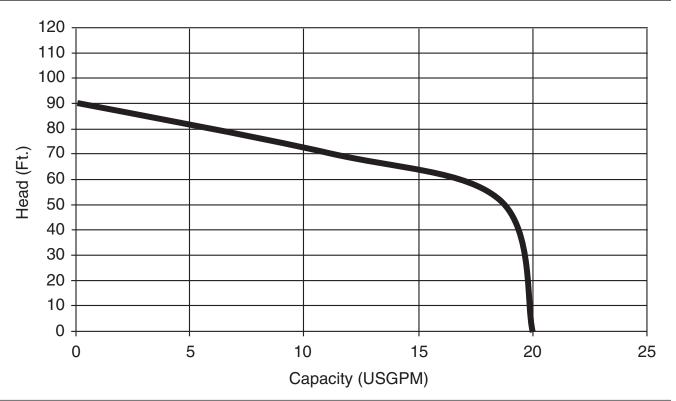
#### **Power Cord:**

The motor power cord shall be 10 Ga. SOW/SOWA or SOOW. The cable jacket shall be sealed at the motor entrance by means of a rubber compression washer and compression nut. An epoxy filled tube shall seal the outer cable jacket and individual leads to prevent water from entering the motor housing. Individual conductor strands shall be soldered within the epoxy seal. Cords shall withstand a pull of 300 pounds.

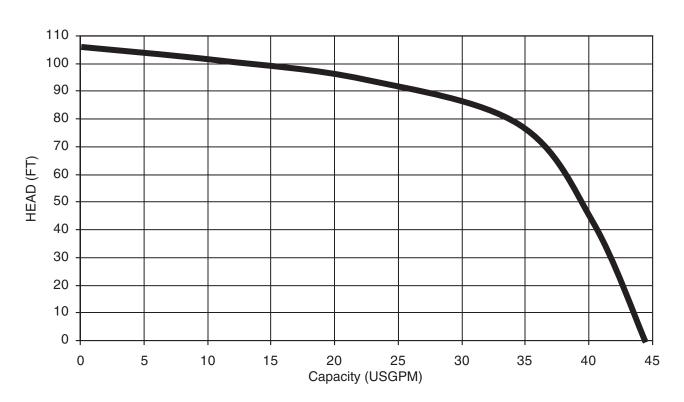
**Performance Curves** 



EBG2-2115(A) (2HP) Synchronous Speed: 3450 RPM 1<sup>1</sup>/<sub>4</sub> inch Discharge



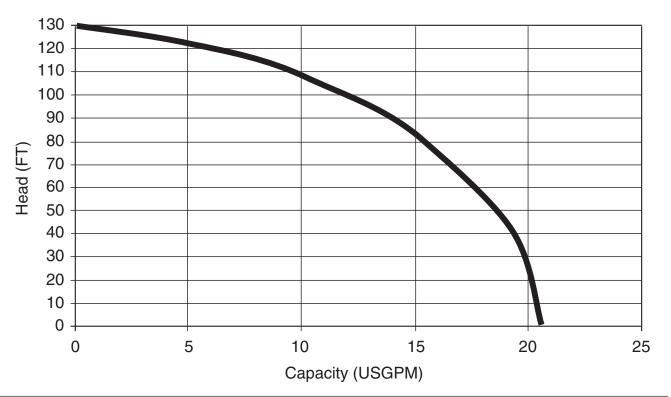
EBG2-212C (2HP) Synchronous Speed: 3450 RPM 11/4 inch Discharge



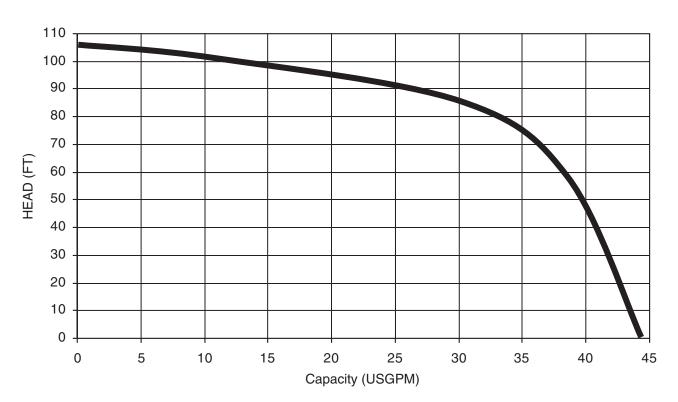
**Performance Curves** 



EBHG2-212C (2HP) Synchronous Speed: 3450 RPM 11/4 inch Discharge



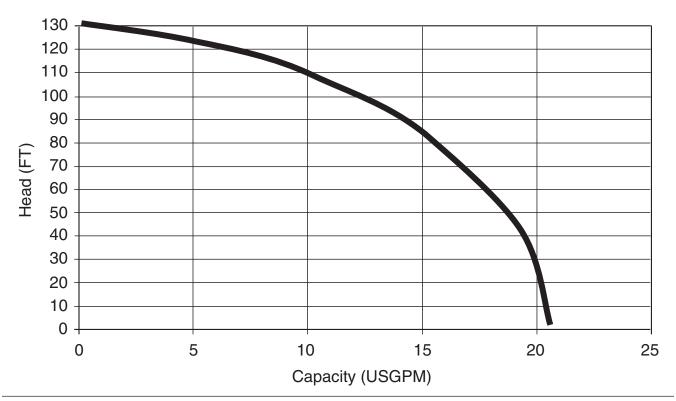
EBG2-21 EBG2-23 (2HP) Synchronous Speed: 3450 RPM 11/4 inch Discharge



Performance (	Curves
---------------	--------

D : .	0014	TDU		LID	01111	ъ.
Project:	GPM:	TDH:	EFF:	HP:	Chk'd:	Date:

EBHG2-21 EBHG2-23 (2HP) Synchronous Speed: 3450 RPM 11/4 inch Discharge

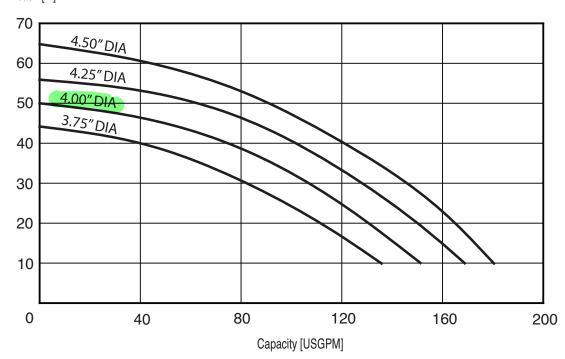


EBG-31 EBG-33 (3HP)

Synchronous Speed: 3450 RPM

 $2^{1}/_{2}$  / 3 inch Discharge

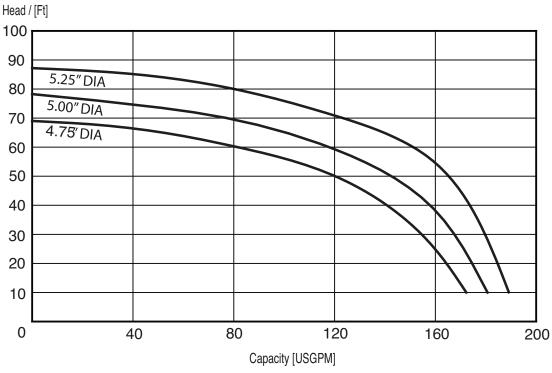




**Performance Curves** 

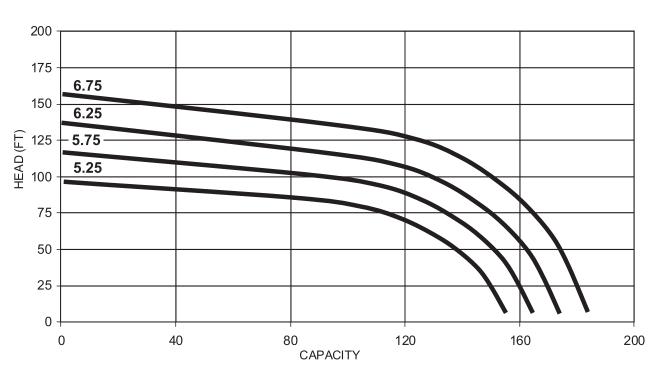


EBG-51 EBG 53 (5HP) Synchronous Speed: 3450 RPM 21/2 / 3 inch Discharge



Note: Full diameter impeller included in price of pump. Consult factory for reduced diameter impeller and pricing.

EBG-103 (10HP) Synchronous Speed: 3450 RPM 2<sup>1</sup>/<sub>2</sub> / 3 inch Discharge

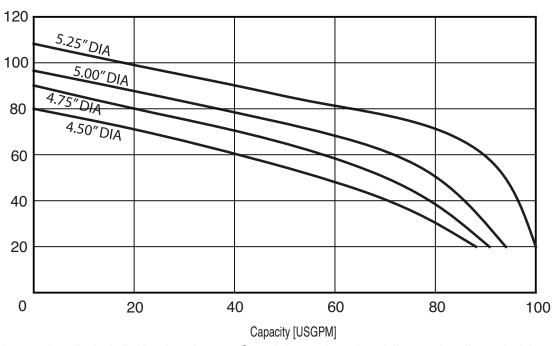


**Performance Curves** 

Project: GPM: TDH: EFF: HP: Chk'd: Date:

EBHG-31 EBHG-33 (3HP) Synchronous Speed: 3450 RPM 21/2 / 3 inch Discharge

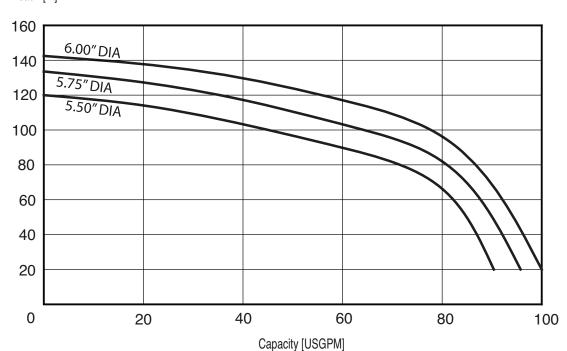
Head / [Ft]



Note: Full diameter impeller included in price of pump. Consult factory for reduced diameter impeller and pricing.

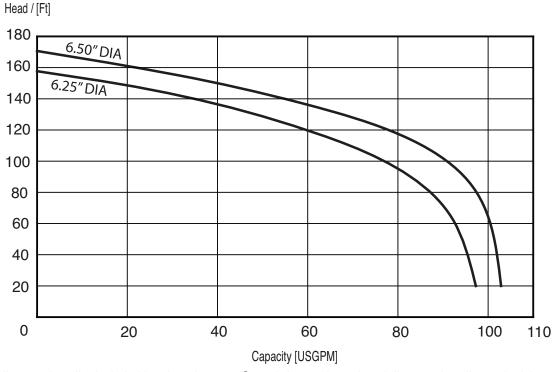
EBHG-51 EBHG-53 (5HP) Synchronous Speed: 3450 RPM 21/2 / 3 inch Discharge

Head / [Ft]



Performance Curves						
Proiect:	GPM:	TDH:	EFF:	HP:	Chk'd:	Date:

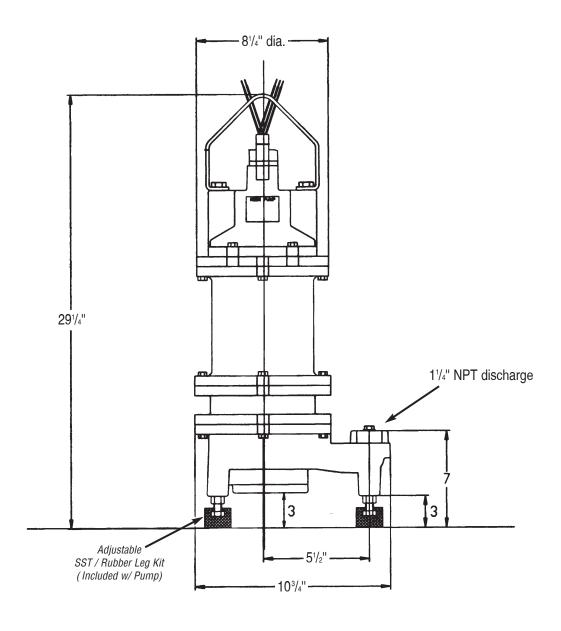
EBHG-71 EBHG-73 (7 $\frac{1}{2}$  HP) Synchronous Speed: 3450 RPM  $2\frac{1}{2}$  / 3 inch Discharge



Project: Model: Chk'd: Date:

# Model EB(H)G

2 HP Unit: inch

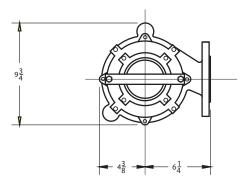


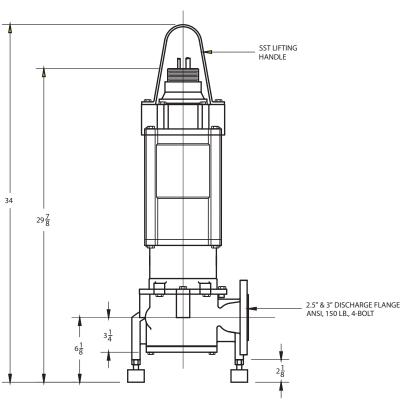
MODEL	WEIGHT – Lb
EBG2-2115(A)	83
EBG2-212C	88
EBHG2-212C	88
EBG2-21	88
EBG2-23	88
EBHG2-21	88
EBHG2-23	88

Project: Model: Chk'd: Date:

# **Model EB(H)G** 3, 5, 7.5, 10HP

Unit: inch





MODEL	WEIGHT – Lb (kg)
EBG-31	190
EBG-33	190
EBG-51	190
EBG-53	130
EBG-103	210
EBHG-31	190
EBHG-33	190
EBHG-51	190
EBHG-53	190
EBHG-71	190
EBHG-73	.30

# **Dimensions - Lift Out Dimensions**

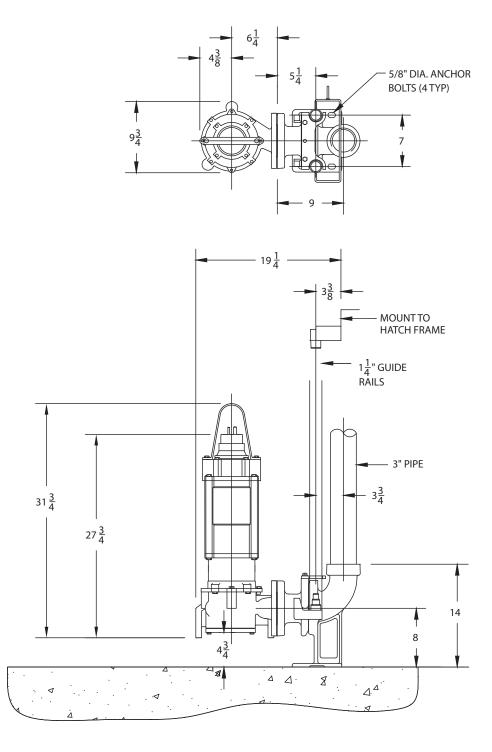
Project: Model: Chk'd: Date:

# **Lift Out Dimensions**

# Model EB(H)G

3, 5, 7.5, 10HP

Unit: inch



Model: Chk'd: Project: Date:

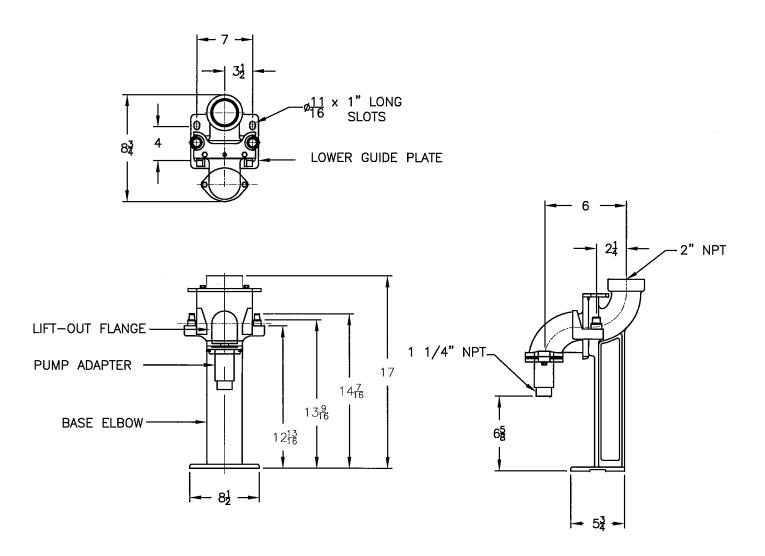
# **Quick Discharge Connector Model RS1** Model EB(H)G

EBG, 2HP EBHG, 2HP RS1 weight = 52 lbs

NOTE: ALL DIMENSIONS ARE IN INCHES.

MATERIALS OF CONSTRUCTION: BASE ELBOW: CAST IRON LIFT-OUT FLANGE: CAST IRON LOWER GUIDE BRACKET: 304 SST ALL FASTENERS ARE 304 SERIES SST

USEABLE RAIL SIZES: ¾" & 1" MAXIMUM WEIGHT ALLOWANCE: 2001bs.



Project: Model: Chk'd: Date:

# Quick Discharge Connector Model RS3-H Model EB(H)G

EBG, 3, 5, 10HP EBHG, 3, 5, 7.5HP RS3-H weight = 70 lbs

NOTE: ALL DIMENSIONS ARE IN INCHES.

MATERIALS OF CONSTRUCTION:

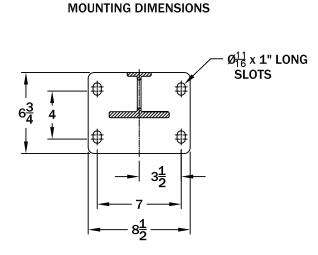
BASE ELBOW: CAST DUCTILE IRON

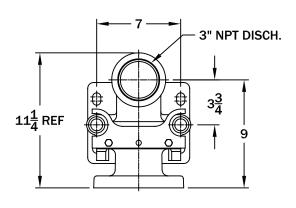
LIFT-OUT FLANGE: CAST DUCTILE IRON

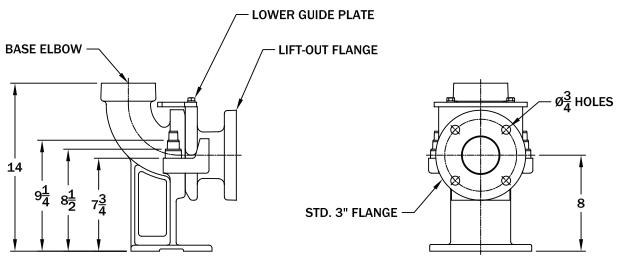
LOWER GUIDE BRACKET: 304 SST

ALL FASTENERS ARE 304 SERIES SST

USEABLE GUIDE RAIL SIZES:  $\frac{3}{4}$ ", 1",  $1\frac{1}{4}$ "



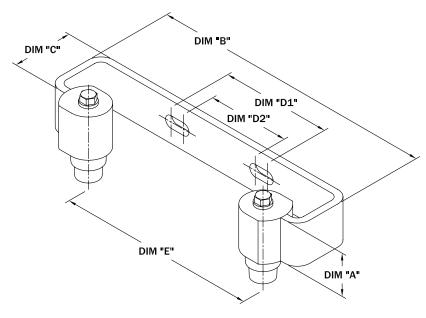




Project: Model: Chk'd: Date:

# **Upper Guide Brackets**

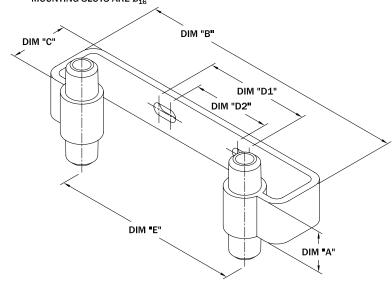
#### NOTE: MOUNTING SLOTS ARE $\emptyset_{16}^{7}$ ACCOMMODATES % , 1 AND 1 % INCH PIPE



MODEL NO.	DIA. "A"	DIM "B"	DIM "C"	DIM "D1"	DIM "D2"	DIM "E"
UGB-SS	1 1/2"	10"	2 1/8"	3 7/8"	2 7/8"	7"

#### **Intermediate Guide Brackets**

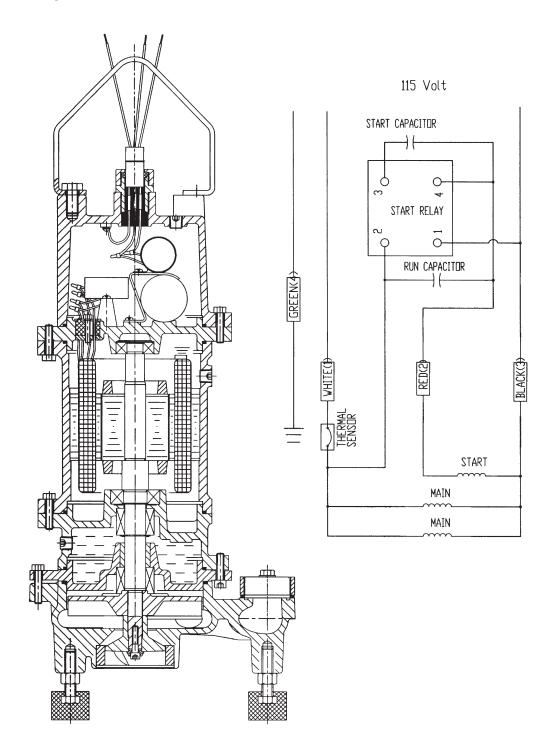
NOTE: IGB-075-SS USED FOR  $\frac{3}{4}$  RAILS IGB-100-SS USED FOR 1" RAILS IGB-125-SS USED FOR  $1\frac{1}{4}$ " RAILS MOUNTING SLOTS ARE  $\emptyset_{16}^{7}$ 



MODEL NO.	DIA. "A"	DIM "B"	DIM "C"	DIM "D1"	DIM "D2"	DIM "E"
IGB-075-SS	1 1/2"	10"	2 1/8"	3 7/8"	2 7/8"	7"
IGB-100-SS	1 1/2"	10"	2 1/8"	3 7/8"	2 7/8"	7"
IGB-125-SS	1 1/2"	10"	2 1/8"	3 7/8"	N/A	7"

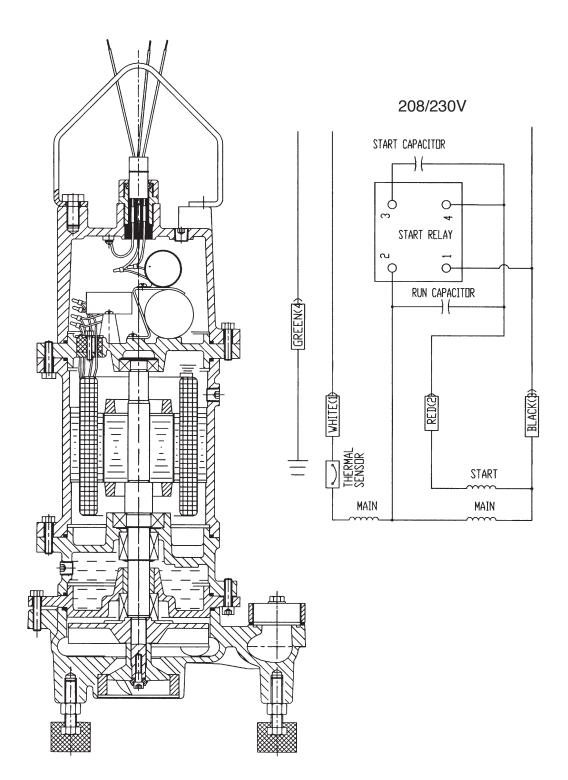
Project: Model: Chk'd: Date:

EBG2-115 2HP, 115V, Single Phase



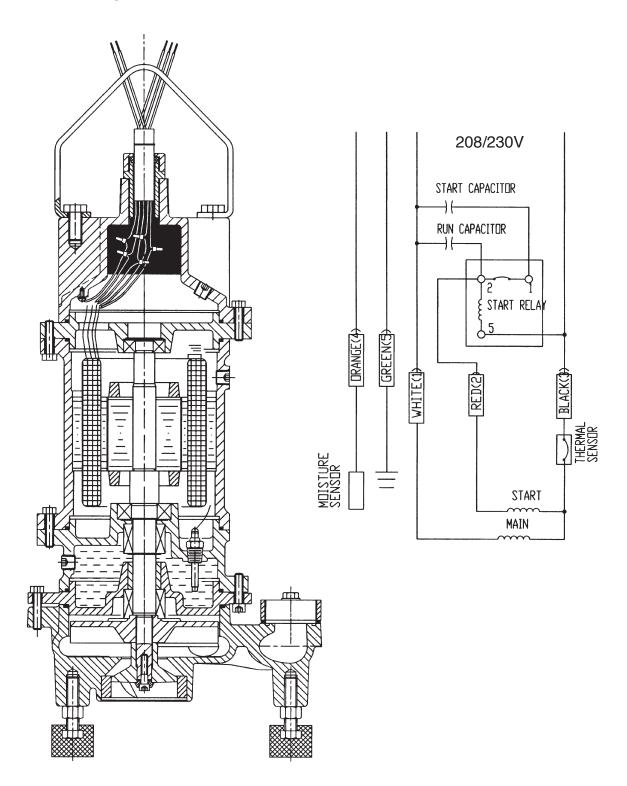
Project: Model: Chk'd: Date:

EB(H)G2-212C 2HP, 208/230V, Single Phase



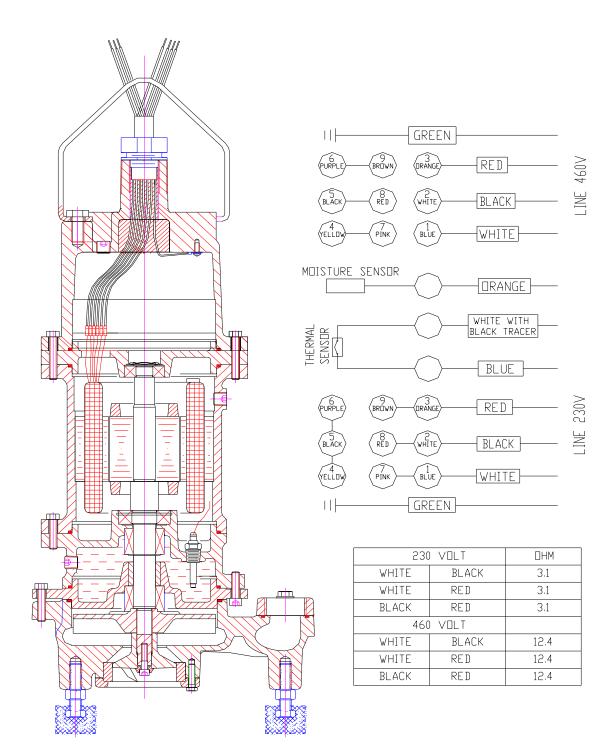
Project: Model: Chk'd: Date:

EB(H)G2-21 2HP, 208/230V, Single Phase



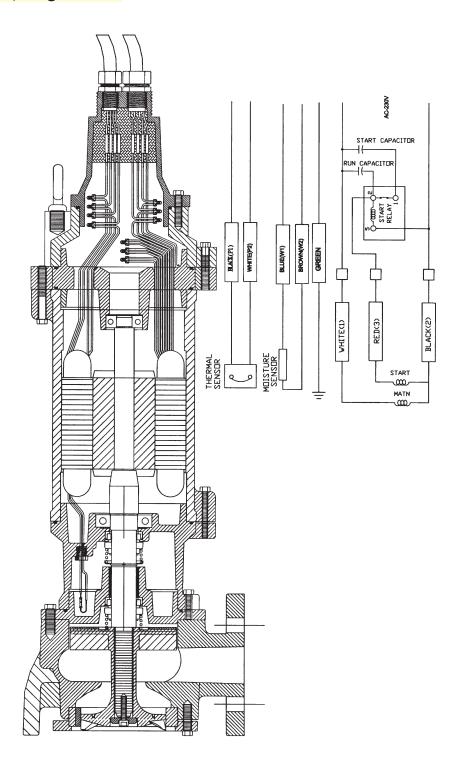
Project: Model: Chk'd: Date:

# EB(H)G2-23 2HP, 208/230/460V, Three Phase



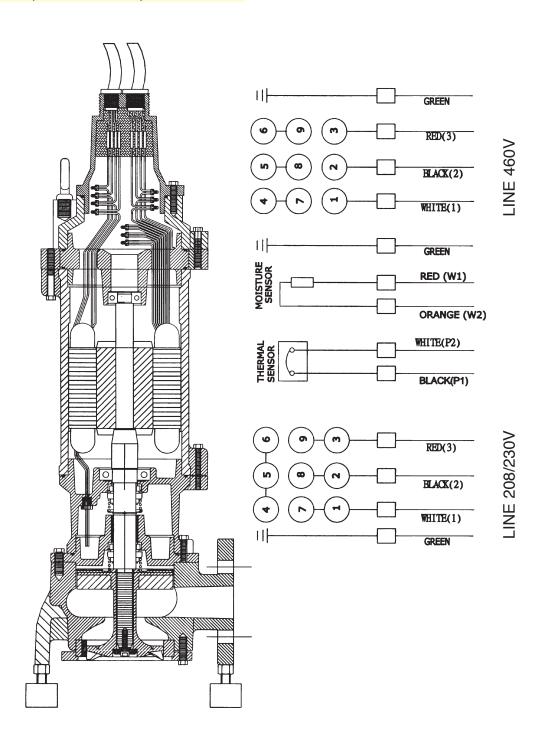
Project: Model: Chk'd: Date:

EB(H)G-31, 51 3, 5HP, 208/230, Single Phase



Project: Model: Chk'd: Date:

EB(H)G-33, 53, 73, 103 3, 5, 7.5, 10HP, 208/230/460V, Three Phase



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_	-		vui		ulu

Project: Model: Chk'd: Date:

# **Model EBG**

# 2 HP, 60Hz, Single Phase, 115V

		Item No.			
	Οι	utput (HP)	2		
	Ph	iase	1		
Name-	Poles		2		
Plate	Vo	lts	115		
Rating	Amperes		16.9		
	Speed		3450		
	Insulation Class		F		
Capacitor	и F	Start	200		
Capacitor	μ.	Run	70		
Resistance at					
20°C OHMS		Main Coil	1.2		
Start Current A		48			
Service Factor		1.59			

			ata

Project: Model: Chk'd: Date:

# **Model EBG**, EBHG

# 2, 3, 5, 7.5 HP, 60Hz, Single Phase, 208/230V

		Item No.					
	Οι	utput (HP)	2	3	5	71/2	
	Ph	nase	1	1	1	1	
Name-	Po	oles	2	2	2	2	
Plate	Vo	olts	208 / 230	208 / 230	208 / 230	230	
Rating	Amperes		15.5 / 12.8	37 / 34	44 / 40	47	
	Speed		3450	3450	3450	3450	
	Insulation Class		F	F	F	F	
Capacitor	υE	Start	150	274 - 324	274 - 324	274 - 324	
Capacitor	μι	Run	30	30	30	30	
Resistance	e at	Main Coil	1.8	.3	.3	.3	
20°C OHMS Aux. Coil		Aux. Coil	5.1	.8	.8	.8	
Start Current A		28 / 25	185 / 170	220 / 200	235		
		Service Factor	1.59	3.96	2.37	1.4	

	Data

Project: Model: Chk'd: Date:

# **Model EBG, EBHG**

# 2, 3, 5, 7.5 10 HP, 60Hz, Three Phase, 208/230/460V

	Item No.					
	Output (HP)	2	3	5	<b>7</b> <sup>1</sup> / <sub>2</sub>	10
NI	Phase	3	3	3	3	3
Name-	Poles	2	2	2	2	2
Plate	Volts	208 / 230 / 460	208 / 230 / 460	208 / 230 / 460	208 / 230 / 460	208 / 230 / 460
Rating	Amperes	10 / 9 / 4.5	21 / 18 / 9	29 / 25 / 13	37 / 32 / 16	42 / 40 / 20
	Speed	3450	3450	3450	3450	3450
	Insulation Class	F	F	F	F	F
Resistance	e at					
20°C OHM	1S Main Coil	3.1 / 12.4	.6 / 2.0	.6 / 2.0	.6 / 2.0	.6 / 2.0
	Start Current A		105 / 90 / 45	145 / 125 / 65	185 / 160 / 80	148 / 135 / 75
	Service Factor	1.92	3.5	2.1	1.25	1.25



# APPENDIX C – CONTROL PANEL DATA

# Control Panel Submittal Bluebonnet – Johnny Morris USEMCO #20205

# **URGENT**

The following information should be confirmed or supplied to USEMCO before this control panel will be entered into the manufacturing schedule.

This control panel is designed to operate on:

Voltage (V): 460

Phase: 3

Frequency (Hz): 60

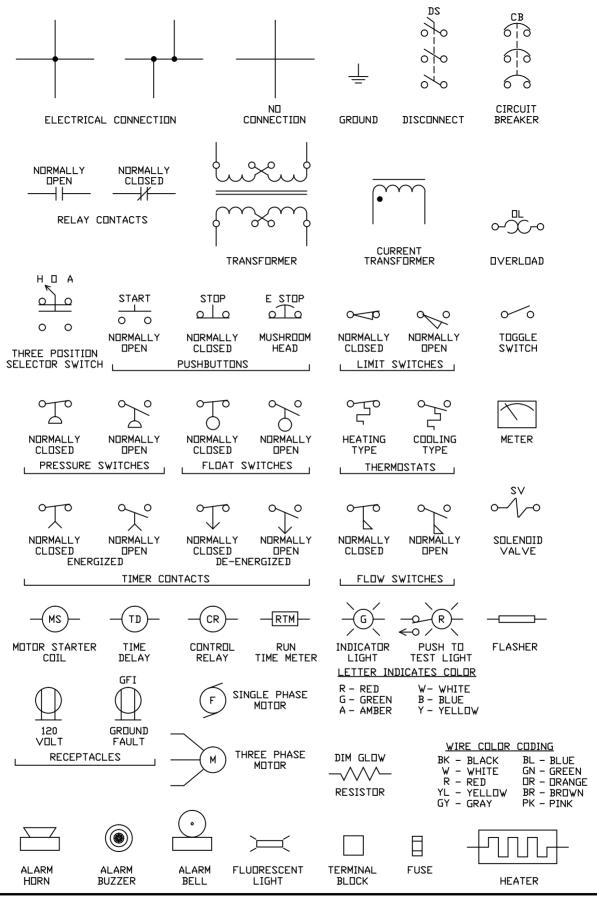
Wire electrical service: 3

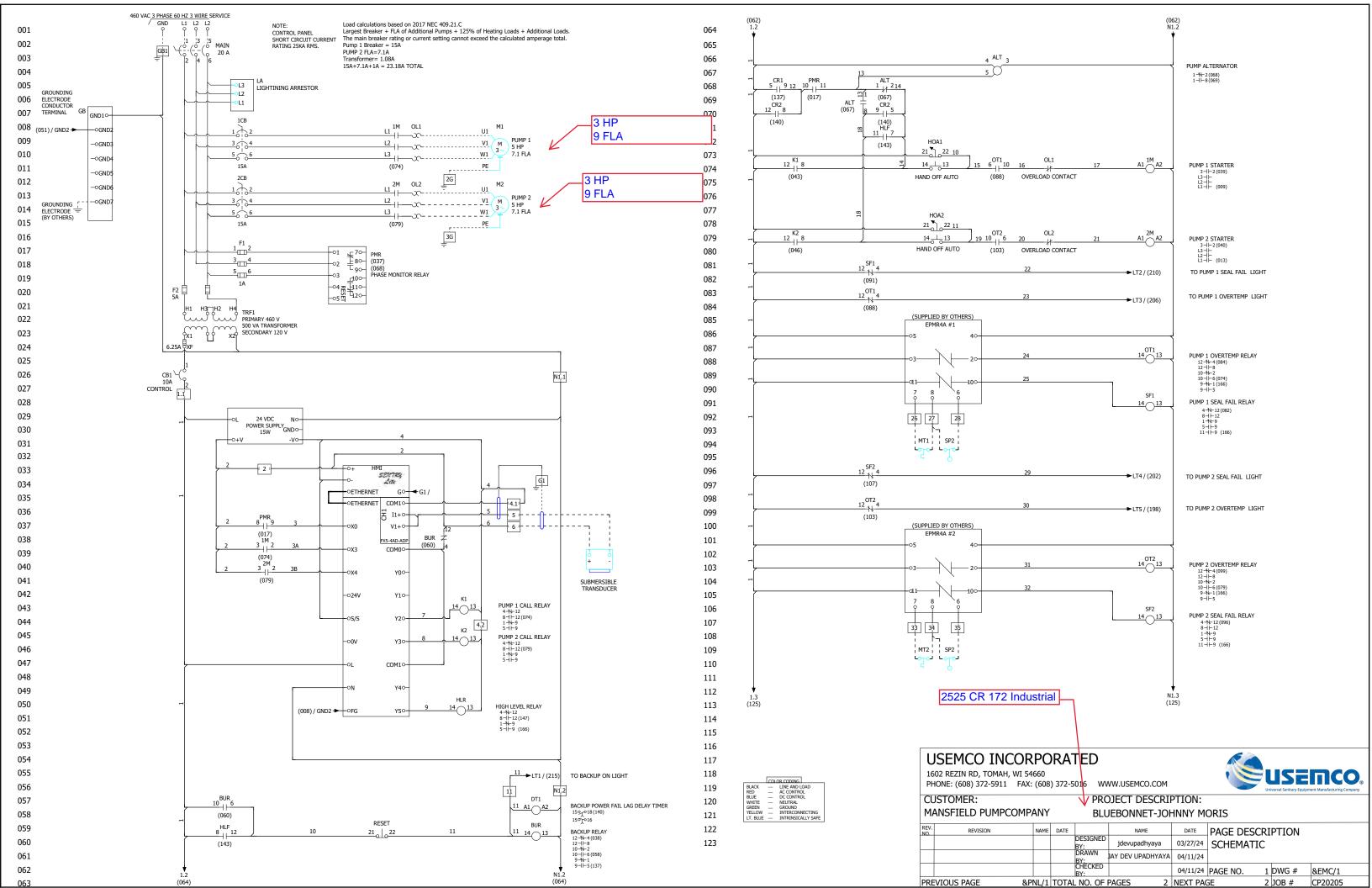
Motor pump count: 2

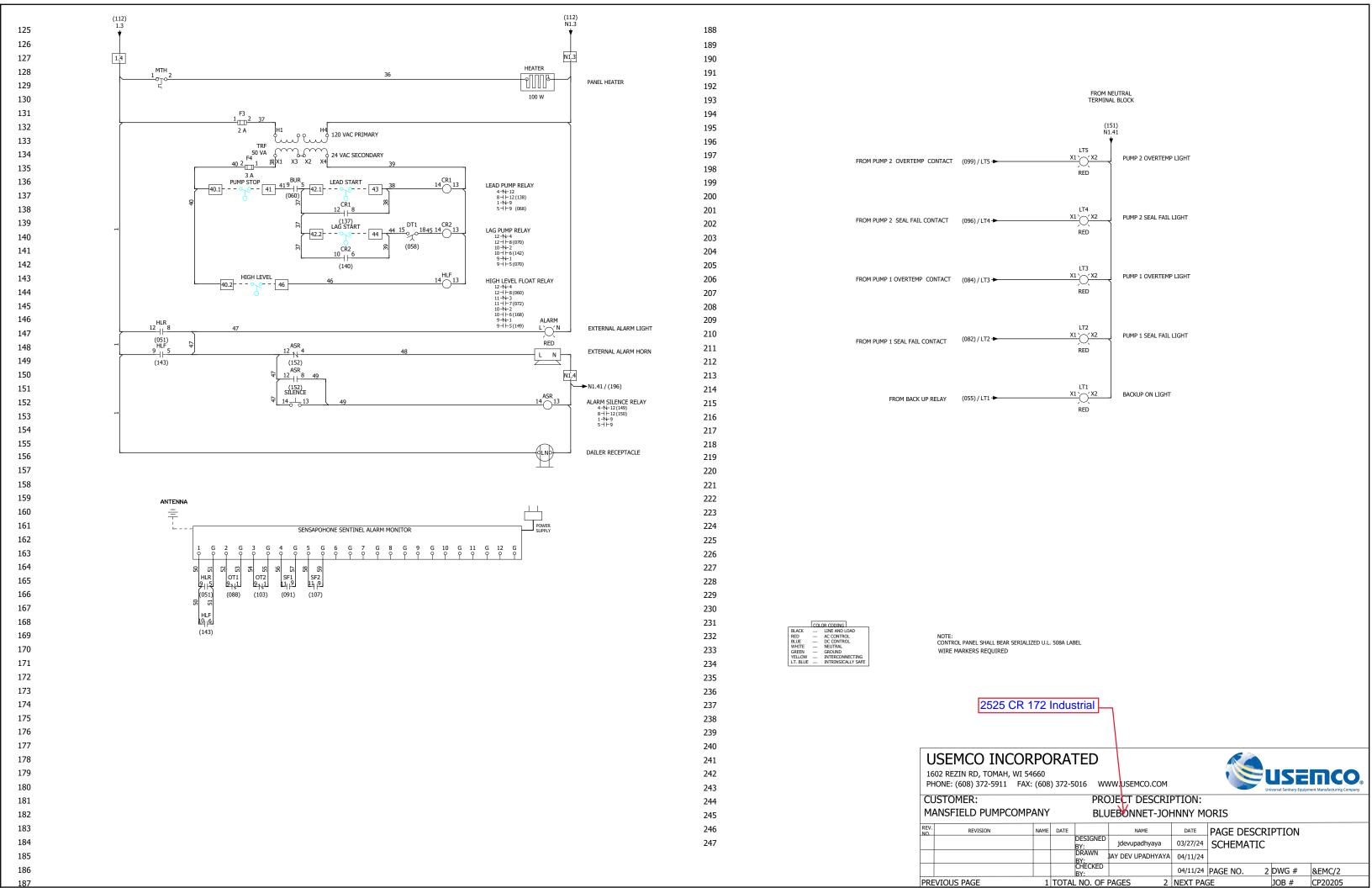
Motor rating (hp): 5

Full load amperage per motor (A): 7.1

# ELECTRIC CONTROL SYMBOLS







# NEMA 4X STAINLESS STEEL ENCLOSURE 48"H x 36"W x 12"DP OUTER DOOR NOT SHOWN

**FRONT** 

ALARM **ANTENNA** 14 HMI **BUZZER OVERLOAD RESETS** 

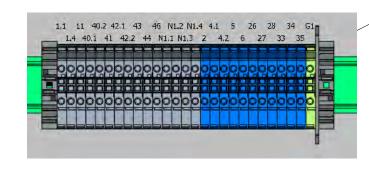
NAMEPLATE LEGEND

1. PUMP 1

11. CONTROL

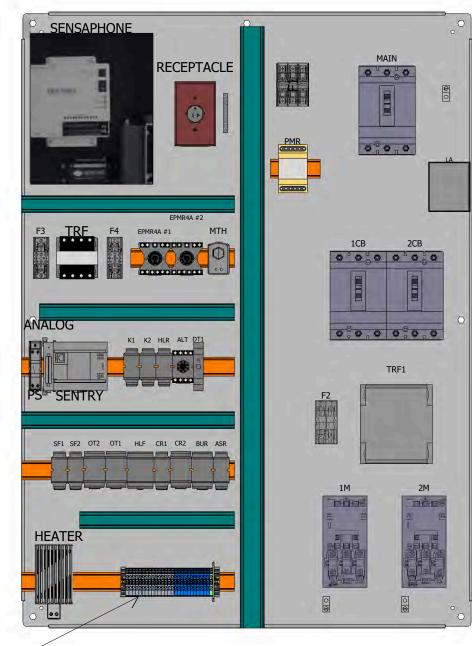
2. PUMP 2

- 12. BACKUP
- 3. HAND OFF AUTO
- 13. RESET
- 4. PUMP 1 OVERTEMP
- 14. MAIN
- 5. PUMP 2 OVERTEMP
- 6. PUMP 1 SEAL FAIL
- 7. PUMP 2 SEAL FAIL
- 8. ALARM
- 9. BACKUP ON
- 10. SILENCE



LAMINATED DRAWING REQUIRED

**BACK PANEL** 



2525 CR 172 Industrial



1602 REZIN RD, TOMAH, WI 54660 PHONE: (608) 372-5911 FAX: (608) 372-5016 WWW.USEMCO.COM

CUSTOMER: PROJECT DESCRIPTION:
MANSFIELD PUMPCOMPANY BLUEBONNET-JOHNNY MORIS

**USEMCO** 

# USEMCO, Inc.

Bill of Material M 20205-EMC

#### 4/11/2024 01:00 PM Page 1

# M 20205-EMC - 20205-EMC - BLUEBONNET-JOHNNY MORRIS BLUEBONNET-JOHNNY MORRIS

Detail	Quantity	
10 PARTS - PART ASSEMBLY NONFABRICATED PART ASSEMBLY NONFABRICATED		
322265 HORN 120VAC EDWARDS #870P-N5	1.00	IN
NEMA 4X RATED	1 00	TNI
322276.2 LIGHT ALARM AC-DC #BL5R-120VAC 420316 STARTER SOD 8536 SB02V02S	1.00 2.00	IN IN
3 POLE NEMA SIZE 0	2.00	TIN
420410 ROD RESET SINGLE SOD 9066 RA-1	2.00	IN
421494.07 BAR GROUND KIT SQD PK7GTA	1.00	IN
421500 BREAKER DOOR MOUNT SQD QOU-110	1.00	IN
421721 BREAKER 3 PL SQD HDL 36015	2.00	IN
25 KA @ 240 VAC 18 KA @ 480 VAC 14 KA @ 600 VAC		
421722 BREAKER 3 PL SQD HDL 36020	1.00	IN
25 KA @ 240 VAC 18 KA @ 480 VAC 14 KA @ 600 VAC		
422304.0535 RELAY/BASE PUMP MONITOR EBARA EPMR4A	2.00	IN
SUPPLIED BY MANSFIELD PUMP COMPANY		
422339.10 RELAY 10A IDEC RH2B-U-AC120V	1.00	IN
422339.11 RELAY 10A IDEC RH2B-U-DC24V	3.00	IN
422339.12 RELAY 10A IDEC RH2B-U-AC24V 422339.20 RELAY 10A IDEC RH3B-U-AC120V	1.00 3.00	IN IN
422339.20 RELAY 10A IDEC RH3B-U-AC24V	2.00	IN
422345 RELAY ALT 10A DIV. ARA-120-ABA	1.00	IN
422351.20 RELAY PH MNTR MACROMATIC PMDU	1.00	IN
190 - 500 VAC		
2 FORM "C" CONTACTS		
423371 POWER SUPPLY IDEC 30W 24VDC #PS5R-VC24	1.00	IN
CLASS II	1 00	
423384.3 MOUNTING KIT SQD #QOSAMK	1.00	IN
423385.19 ARRESTER SURGE 3PH SQD 480V D #SDSA4040D (3P3W)	1.00	IN
423418 BLOCK TERMINAL ABB-GRAY #1SNK506010R0000  ZS6	15.00	IN
423418.0 BLOCK TERMINAL ABB-BLUE #1SNK506020R0000  ZS6-BL	11.00	IN
423418.3 BLOCK TERMINAL ABB-GN/YL #1SNK506150R0000  GROUNDING ZS6-PE	1.00	IN
423418.4 END SECTION COVER ABB SNK #1SNK505910R0000 ES4	1.00	IN
423418.5 END STOP ABB-GRAY SNK #1SNK900001R0000  BAM3	2.00	IN
423418.7 JUMPER 2 POLE ABB SNK #1SNK906302R0000 JB6-2	4.00	IN
423418.71 JUMPER 4 POLE ABB SNK #1SNK906304R0000 JB6-4	2.00	IN
423480.2 SOCKET RD 8 PIN IDEC SR2P-06	1.00	IN
423481.2 SOCKET RD 11 PIN IDEC SR3P-06	2.00	IN
423484 SOCKET MIDGET 2P IDEC SH2B-05	5.00	IN

**USEMCO, Inc.**Bill of Material

4/11/2024

Page

2

## M 20205-EMC - 20205-EMC - BLUEBONNET-JOHNNY MORRIS BLUEBONNET-JOHNNY MORRIS

M 20205-EMC

Detail	Quantity	
423485 SOCKET MIDGET 3P IDEC SH3B-05	5.00	IN
423627 OVERLOAD SQD #B10.2	3.00	IN
425013.00 TIMER 120VAC DIVERSIFIED ON DELAY #175SO-110	1.00	IN
425360.3 TRNSFMR 50VA SQD 9070 T50D13	1.00	IN
425378 TRNSFMR 500VA HEVI-DUTY #E500	1.00	IN
W/FB2X FUSE HOLDER		
SBE ENCAPSULATED SERIES		
426103 FUSE 250V TIME-DELAY 2 AMP	1.00	IN
CLASS RK5 200,000 A.I.R.		
2" LONG x 9/16" DIAMETER		
(BUSSMANN FRN-R-2)		
(LITTELFUSE FLN-R-2)		
(MERSEN TR-2R)		
426104 FUSE 250V TIME-DELAY 3 AMP	1.00	IN
CLASS RK5 200,000 A.I.R.		
2" LONG x 9/16" DIAMETER		
(BUSSMANN FRN-R-3)		
(LITTELFUSE FLN-R-3)		
(MERSEN TR-3R)		
426206 FUSE 600V FAST-ACTING 5 AMP	2.00	IN
1-1/2" LONG x 13/32" DIAMETER		
(BUSSMANN KTK-5)		
(LITTELFUSE KLK-5)		
(MERSEN ATM5)		
100,000 A.I.R.		
426211.0001 FUSE 600V FAST-ACTING 1 AMP	3.00	IN
CLASS CC 200,000 A.I.R.		
1-1/2" LONG x 13/32" DIAMETER		
(BUSSMANN KTK-R-1)		
(LITTELFUSE KLKR-001)		
(MERSEN ATMR1)		
426211.0062 FUSE 600V TIME-DELAY 6-1/4 AMP	1.00	IN
CLASS CC 200,000 A.I.R.		
1-1/2" LONG x 13/32" DIAMETER		
(BUSSMANN FNQR-6-1/4)		
(LITTELFUSE KLDR-6-1/4)		
(MERSEN ATQR6-1/4)		
426520 BLOCK FUSE MARATHON 6M30A2SQ	1.00	IN
13/32" X 1 1/2" FUSE SIZE		
426520.02 BLOCK FUSE MARATHON 6M30A3SQ	1.00	IN
13/32" X 1 1/2" FUSE SIZE		
426521 BLOCK FUSE MARATHON F30A1S	2.00	IN
FOR 9/16" x 2" FUSES		
427712.21 DIALER WIRELESS SENSAPHONE SENTINEL SCD-1200-4GVZCD	1.00	IN
4G LTE CELLULAR MODEM ON VERIZON	2.00	
CLEAR DOOR		
12 INPUTS		
8 HOUR INTERNAL BATTERY BACKUP		
INCLUDES ANTENNA		
427712.STD-2 SERVICE WIRELESS SENSAPHONE SENTINEL SCD-1200-1YCELL	1.00	IN
VERIZON 1 YEAR	2.00	
428905.1 WIRE DUCT BASE 1.5" X 3"	3.00	IN
PANDUIT F1.5 x 3 LG6 OR TYTON SL-1.5 x 3G	3.00	T 1/
428905.2 WIRE DUCT BASE 1" X 2"	2.00	IN
	2.00	ΤIN
PANDUIT F1X2 LG6		

**USEMCO, Inc.**Bill of Material

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## M 20205-EMC - 20205-EMC - BLUEBONNET-JOHNNY MORRIS BLUEBONNET-JOHNNY MORRIS

M 20205-EMC

Detail428905.25 WIRE DUCT BASE 1" X 3"	Quantity 5.00	IN
PANDUIT F1X3 LG6	3.00	TIV
428908.1 WIRE DUCT COVER 1.5"	3.00	IN
PANDUIT C1.5LG6 OR TYTON TC1.5G		
428908.2 WIRE DUCT COVER 1"	7.00	IN
PANDUIT C1LG6		
429071.3 DECAL PANEL RATING LABEL	1.00	IN
VOLTAGETOTAL FLA		
PHASEFREQ		
AMP HP		
HPS/N SHORT CIRCUIT CURRENT RATING		
kA RMS SYMMETRICAL	1 00	T.1.T
429094.02 DECAL USEMCO 2 7/16" X 7 1/8"	1.00	IN
.020 MILL ALUMINUM BLK PRINT W/468 ADHESIVE429094.04 DECAL USEMCO 2" X 3" MADE IN USA	1.00	IN
429094.04 DECAL USEMCO 2 X 3 MADE IN USA 429105 DECAL USEMCO LIMITED WARRANTY 1 YEAR	1.00	IN
3" X 4.75" WHITE BACKGROUND BLACK LETTERS	1.00	ΤIN
Any part claimed defective will be either repaired		
or replaced, our option, when returned to our		
factory transportation charges paid. This		
guarantee only covers defect which have developed		
in the service for which the controls have been		
designed.		
The liability of the company shall not in any case		
exceed the cost of correcting defects in USEMCO^S		
controls or parts, upon the above mentioned one year, all such liability shall terminate.		
USEMCO shall not in any event be liable for		
indirect or consequential damages.		
USEMCO, Inc.		
Tomah, WI 54660		
WORDING AS FOLLOWS:		
All control panels manufactured by USEMCO, Inc.		
are guaranteed to be free from defect in		
workmanship and material for a period of one (1)		
year from date of shipment429118.43 LABELS "WARNING" BRADY B30-25-595-ANSIWA	1.00	IN
4" X 6" DIECUT	1.00	ΤIN
ORDER 175 PER ROLL		
ORANGE "WARNING" HEADER ON WHITE LABEL		
432080.8822 BRACKET ANTENNA PANORAMA #SP5-0470	1.00	IN
432160 HMI IDEC 4.3" COLOR TOUCH #HG1G-4VT22TF-B	1.00	IN
TFT LCD Touch Screen		
95.04mm x 53.856mm		
480 x 272 Pixels		
12 - 24VDC		
432359.102 PLC, MITSU, FX5 AC BASE UNIT #FX5S-30MR/ES	1.00	IN
16 DC IN / 14 RELAY OUT		
BUILT IN ETHERNET PORT	1 00	
432359.500 PLC, MITSU, 4CH ANALOG INPUT FX5-4AD-ADP	1.00	IN
LEFT SIDE ANALOG INPUT ADAPTOR MODULE	1 00	T N T
460480 RING SEAL T&B 5262 1/2"	1.00	IN
OR EQUAL460501.5 SPLIT CORD GRIP ICOTEK KVT-ER 25	1 00	TNT
460501.5 SPLIT CORD GRIP ICOTEK KVT-ER 25	1.00	IN IN
400002.002 GROWMET ICOIEN ZWM NII #3994I	1.00	T 1/

**USEMCO, Inc.**Bill of Material

4/11/2024

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91:00 PM

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4

## M 20205-EMC - 20205-EMC - BLUEBONNET-JOHNNY MORRIS BLUEBONNET-JOHNNY MORRIS

M 20205-EMC

Detail	Quantity	
460690 BRKT CAP NEWARK #14F412 VR6A	1.00	IN
1.750-1.8125" DIA.		
460695 BRKT CAP NEWARK #13M2545	1.00	IN
UNIVERSAL WRAP-AROUND		
2.00" DIA.		
(REPLACES THE OLD 81F3217 & 72V8354) 462319.0 COVER SGL GANG BROWN P&S #P-7	1.00	IN
462320 RECEPT SGL 20A 125V A-H 5361B NEMA CONFIG. NO. 5-20R	1.00	IN
462850.03 ENCL N4X SAGINAW SCE-48EL3612SSLPPL	1.00	IN
TYPE 304 STAINLESS STEEL WITH 3-POINT LATCH	1.00	ΙN
AND KEYLOCKING/PADLOCKING HANDLE	1 00	T.).T
463180.14 ENCL DEAD FRONT SAGINAW SCE-DF48EL36LP	1.00	IN
463223.12 ENCL BP SAGINAW SCE-48P36	1.00	IN
463442.72 THERMOSTAT NORMALLY CLOSED SCE-TEMNC	1.00	IN
NORMALLY OPEN & NORMALLY CLOSED		
DUAL HEATING & COOLING THERMOSTAT INDIVIDUAL ADJUSTMENTS		
14-176 DEGREES HEATING & COOLING		
14-170 DEGREES HEATING & COOLING		
463502.100 ENCL MTG KIT SAGINAW SCE-ELMFK4SS6-OS	1.00	IN
INCLUDES (4) 304 SS MUNTING FEET	1.00	
AND STAINLESS STEEL FASTENERS		
469004 LUG ALUM SOLDERLESS MAX. #2	3.00	IN
ILSCO TA-2, BLACKBURN ADR-2, BURNDY KA2U		
OR EQUAL		
469005.0018 LUG POWER KIT SQD #PDC6HD6	1.00	IN
(6) 14-6 AWG		
(3) LUGS PER KIT		
469068.12 HEATER TOUCH SAFE SAGINAW 100W SCE-TSH100	1.00	IN
470501.44 SWITCH PB BLACK TELE ZB4 BA2	2.00	IN
470620.44 SWITCH SEL 3 POS TELE ZB4 BD3	2.00	IN
472078.44 MOUNTING COLLAR TELE ZB4 BZ009	9.00	IN
FOR XB4 22mm SWITCHES		
472081.66 BLOCK CONTACT TELE ZBE101	5.00	IN
ONE NORMALLY OPEN CONTACT		
FOR XB4 22mm SWITCHES		
472082.66 BLOCK CONTACT TELE ZBE102	1.00	IN
ONE NORMALLY CLOSED CONTACT		
FOR XB4 22mm SWITCHES		
480141.2 LIGHT HEAD RED TELE ZB4 BV043	5.00	IN
RED LENS		
FOR XB4 22mm LIGHTS		
480143.0 LIGHT MODULE TELE ZBVG1	5.00	IN
WHITE 120VAC LED		
FOR XB4 22mm LIGHTS	4 00	T.1.
500586.X02 STANDOFF BOTTOM LEG SQD H SERIES	4.00	IN
ADJUSTABLE 4 1/4 - 6 3/8"	1 00	T > 7
500586.Z09 STANDOFF TOP PLATE SQD 1-HXL	1.00	IN
500586.Z10 STANDOFF TOP PLATE SQD 2-HXL 3P	1.00	IN
20 PARTS - PART ASSEMBLY NONFABRICATED SHIP LOOSE ITEMS		
OUTE DOODE ITEMS		
470017.421 TRANSDUCER BLUE RIBBON BIRDCAGE #BC001-15-70	1.00	IN
15PSI, 4-20mA, 70FT CABLE		



Saginaw Control and Engineering 95 Midland Road Saginaw, MI 48638-5770 (800) 234-6871 - Fax: (989) 799-4524 SCE@SaginawControl.com

## SCE-48EL3612SSLPPL



## **Product Specifications:**

Part Number: SCE-48EL3612SSLPPL Description: S.S. LPPL Enclosure Height: 48.00" Width: 36.00" Depth: 12.00"

## Construction

- 0.075 In. stainless steel Type 304.
- Seams continuously welded and ground smooth.
- Flange trough collar around all sides of door opening.
- Collar studs provided for mounting optional panels.
- Mounting holes in back of the enclosure for wall mounting.
- Mounting hardware, sealing washers and hole plugs included.
- Stainless steel concealed hinges.
- Removable and interchangeable doors.
- Black zinc die cast keylocking/padlocking handles.
- 3-point latching mechanism.
- Removable print pocket.
- Oil and water resistant gasket.
- Ground stud on door and body.

## Application

Designed to house electrical and electronic controls, instrumentation and components in indoor or outdoor locations. For outdoor applications a drip shield is recommended.

## Finish

#4 Brushed finish on all exterior surfaces. Optional sub-panels are powder coated white.

- Industry Standards (IS6)

  NEMA Type 3R, 4, 4X, 12 and Type 13

  UL Listed Type 3R, 4, 4X and 12
- CSA Type 4, 4X and 12
- **□** IEC 60529
- IP 66

Special Instructions apply for IS3, IS4 and IS6 to maintain the environmental rating of Type 3R for these parts. Instructions are located on the enclosure door. Drip shield is required on IS3, drip shield is recommended on IS4 and IS6. Drain holes are required on

Provision for Lifting Lugs on enclosures when Height >48" and Depth >16". The Lifting Lug assembly will be included with the enclosure bolt

## Optional Accessories

SCE-48P36 Subpanel, Bent SCE-BVK Breather Vent SCE-DF48EL36LP Panel, Dead Front (Wall Mount) SCE-DS36SS Shield, S.S. Drip SCE-ELMFK4SS Foot Kit, S.S. EL Mounting (4pc.) SCE-ELSP3 KIT, Swing-Out Panel (20 High & Up)

## Similar Part Numbers

SCE-36EL2412SSLPPLS.S. LPPL Enclosure SCE-36EL3012SSLPPLS.S. LPPL Enclosure SCE-36EL3612SSLPPLS.S. EPL Enclosure SCE-36EL3612SSLPPLS.S. EL Enclosure SCE-42EL3612SSLPPLS.S. LPPL Enclosure SCE-48EL3616SSLPPLS.S. LPPL Enclosure SCE-60EL3612SSLPPLS.S. LPPL Enclosure SCE-60EL3616SSLPPLS.S. LPPL Enclosure SCE-60EL3624SSLPPLS.S. LPPL Enclosure

## **Installation Information**

- Drain/Vents
- Sealing Washer Specifications
- Dead Front Wall Mount With 3 Point Latching Hardware
- EL Flush Mount Frame
- Dead Front Wall Mount Installation Instructions
- Swing Panel ELSP for Encl. Height > 16
- Drip Shield Kit Assembly

## Full Voltage Contactors and Starters — NEMA Application Data – Class 8502, 8536



Type SCO2 Size 1, 3-Pole Contactor



Type SCO3
Size 1. 3-Pole Starter



Starter with MOTOR LOGIC



Starter with Melting Alloy

## AC MAGNETIC CONTACTORS CLASS 8502

## **General Information**

Class 8502 Type S magnetic contactors are used to switch heating loads, capacitors, transformers, and electric motors where overload protection is separately provided. Class 8502 contactors are available in NEMA Sizes 00-7. Type S contactors are designed for operation at 600 Volts, AC 50-60 Hertz.

## **Holding Circuit Contact**

A normally open holding circuit contact for three wire control is provided on all contactors as standard. Sizes 00-2 contactors use a Class 9999 SX11 auxiliary contact as the holding circuit contact. Sizes 3-7 contactors use a Class 9999 SX6 auxiliary contact as the holding circuit contact. See Class 9999 for the holding circuit contact electrical ratings. On Size 00-1 single phase contactors, a power pole is used as the holding circuit contact and therefore has the same rating as the power contacts.

## **Enclosures**

Class 8502 magnetic contactors are available in the following enclosures:

- NEMA Type 1 General Purpose
- NEMA Type 4 & 4X Watertight and Dusttight Stainless Steel
- NEMA Type 4X Watertight, Dusttight, and Corrosion Resistant Glass — Polyester
- NEMA Type 7 & 9 Bolted and Spin-Top for Hazardous Locations
- NEMA Type 12 Dusttight and Driptight for Industrial Use

The NEMA Type 4 & 4X stainless steel enclosure (Sizes 0-5) has a brushed finish. For an electropolished finish, specify Form G16 and add 15% to the price of the standard device.

Also, NEMA Type 12 devices are available UL Listed for use in Class II, Division 2, Group G and Class III, Divisions 1 and 2 locations. Request Form G21, no additional charge.

Separate enclosures are available, see Class 9991.

## AC MAGNETIC STARTERS CLASS 8536

## **General Information**

Class 8536 Type S magnetic starters are used for full voltage starting and stopping AC squirrel cage motors. Motor overload protection is provided by melting alloy type thermal overload relays. Class 8536 starters are available in NEMA Sizes 00-7. Type S starters are designed for operation at 600 Volts AC, 50-60 Hertz.

## **Holding Circuit Contact**

A normally open holding circuit contact for three wire control is provided on all contactors as standard. Sizes 00-2 contactors use a Class 9999 SX11 auxiliary contact as the holding circuit contact. Sizes 3-7 contactors use a Class 9999 SX6 auxiliary contact as the holding circuit contact. See Class 9999 for the holding circuit contact electrical ratings.

## Overload Relays with Melting Alloys

Class 8536 Type S Sizes 00-6 starters are provided with a melting alloy thermal overload relay as standard. Interchangeable thermal units are available in standard trip (Class 20) Sizes 00-6, quick trip (Class 10) Sizes 00-4, and slow trip (Class 30) Sizes 00-3. Single-phase starters use one thermal unit, 3-phase starters use three thermal units.

Class 8536 Size 7 starters are provided with solid state Motor Logic which has selectable trip Class10/20, Ground fault detection, and Communication capabilities for future enhancement. The solid state overload relay is ambient insensitive and features phase loss, phase unbalance and over-current protection.

## MOTOR LOGIC™ Solid State Overload Relay (SSOLR)

Solid state overload relays are available for Sizes 00-7 starters. These ambient insensitive overload relays provide phase loss protection, phase unbalance protection and a LED power indicator. For additional information, see the Class 9065 catalog section. To order Type S starters with solid state overload relays, see Factory Modification (FORMS).

Bimetallic overload relays are also available for Sizes 0-6. Ambient Compensated and Noncompensated versions are supplied with manual



## Full Voltage Contactors and Starters — NEMA Application Data – Class 8502, 8536

and automatic reset, trip current adjustment, and an alarm contact on Sizes 0-2. For additional information, see the Class 9065 catalog section. To order Type S starters with bimetallic overload relays, see Factory Modifications (FORMS).

## **Enclosures**

Class 8536 magnetic starters are available in the following enclosures.

- NEMA Type 1 General Purpose Enclosure
- NEMA Type 3R Rainproof, Sleet Resistant for Outdoor Use
- NEMA Type 4 & 4X Watertight and Dusttight
- NEMA Type 4X Watertight, Dusttight, and Corrosion Resistant Glass – Polyester
- NEMA Type 7 & 9 Bolted and Spin-Top for Hazardous Locations
- · NEMA Type 9 Bolted for Hazardous Locations
- NEMA Type 12 Dusttight and Driptight for Industrial Use

The NEMA Type 4 & 4X stainless steel enclosure (Sizes 0-5) has a brushed finish. For an electropolished finish, specify Form G16 and add 15% to the price of the standard device. Sizes 6 & 7 are painted sheet steel and are rated NEMA 4 ONLY.

Also NEMA Type 12 devices are available UL Listed for use in Class II, Division 2, Group G and Class III, Divisions 1 and 2 locations. Specify Form G21, no additional charge.

Separate enclosures are available, see Class 9991.

## **Coil Voltages**

AC coils are available for application on 50-60 Hertz. NEMA Sizes 00-5 are supplied with coils that are designed to operate satisfactorily on line voltages of 85% – 110% of rated voltage. NEMA Size 6 and 7 contactors are supplied with a DC coil operated by a solid state rectifier circuit that is powered by an AC source.

Please note that **Voltage Codes** have been added to the Type designations in order to improve customer service. It is necessary to include the Voltage Code when ordering contactors and starters. Also, 120 Volt Polyphase contactors and starters will be wired for separate control.

## **Auxiliary Contacts**

Additional auxiliary contacts may be added to Type S contactors. See Page 15 for maximum number of auxiliary units and Form designations for factory installed auxiliary contacts.

## Type S Accessories

Additional accessories such as power poles, pneumatic timer attachments, and cover mounted control stations are available as factory or field modifications.

## 8502SBO2V02S

Full Voltage Magnetic Contactor , Non-Reversing, NEMA Size: 0, 18A



## **Technical Characteristics**

Control Source	Separate Control Circuit
Catalog Reference Number	8502CT9701
Depth	4.22 Inches
Width	3.22 Inches
Height	4.34 Inches
Ampere Rating	18A
Application	Used to switch heating loads, capacitors, transformer and electric motors where overload protection is provided separately
Approvals	UL Listed File E78351 CCN NLDX - CSA Certified File LR60905 Class 3211 04
Enclosure Type	Open
Horsepower Rating (3-Phase)	3HP@200/230VAC - 5HP@460/575VAC
Maximum Voltage Rating	600VAC
Mounting Type	Surface
NEMA Size	0
Number of Poles	3-Pole
Operating Voltage	120VAC@60Hz - 110VAC@50Hz
Phase	3-Phase
Action	Non-Reversing
Terminal Type	Screw Clamp
Туре	S
Weight	4 Pounds

## **Shipping and Ordering**

Category	21191 - Contactors/Starters, NEMA, Open/Enclosed (NEMA 1), Size 00 & 0
Discount Schedule	CP1
Article Number	785901862833
Package Quantity	1
Weight	3.25 lbs.
Availability Code	Stock Item: This item is normally stocked in our distribution facility.
Returnability	Υ

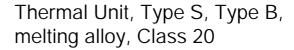
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# Disclaimer: This documentation is not intended as a substitute for and is not to be used for determining suitability or reliability of these products for specific user applications







R10 2

## Main

Range Of Product	В
Thermal Overload Type	Melting alloy
Motor Tripping Class	20

## Ordering and shipping details

Category	US10CP121781
Discount Schedule	0CP1
Gtin	785901587477
Returnability	Yes
Country Of Origin	MX

## **Packing Units**

Unit Type Of Package 1	PCE
Number Of Units In Package 1	1
Package 1 Height	0.80 in (2.032 cm)
Package 1 Width	1.20 in (3.048 cm)
Package 1 Length	2.50 in (6.35 cm)
Package 1 Weight	0.80 oz (22.68 g)

## **Contractual warranty**

Warranty	18 months

Price is "List Price" and may be subject to a trade discount – check with your local distributor or retailer for actual price.

## **HDL36020**

Molded Case Circuit Breaker , 600VAC/250VDC, 20A





by Schneider Electric

## **Technical Characteristics**

Approvals	UL Listed - CSA Certified - IEC Rated
Catalog Reference Number	0611CT0401
Circuit Breaker Type	Standard
For Use With	Industrial Enclosures and Switchboards
General Application	Provides overload and short circuit protection
AC Magnetic Trip Setting	350A - 750A Adjustable Trip
Ampere Rating	20A
Frame Type	H-Frame
HACR Rated	Yes
Marketing Trade Name	Powerpact
Voltage Rating	600VAC/250VDC
Mounting Type	Unit Mount
Weight	5 Pounds
Number of Poles	3-Pole
Short Circuit Current Rating	25kA@240VAC - 18kA@480VAC - 14kA@600VAC
Туре	HD
Terminal Type	Line: Lug - Load: Lug
Wire Size	#14-3/0 AWG(Al/Cu)
Height	6.40 Inches
Width	4.12 Inches
Depth	4.36 Inches

## **Shipping and Ordering**

Category	01110 -
Discount Schedule	DE2
Article Number	785901706694
Package Quantity	1
Weight	4.15 lbs.
Availability Code	Non-Stock Item: This item is not normally stocked in our distribution facility.
Returnability	Υ

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## SDSA 3-Phase SPDs Features

## Superior Performance

Square D brand Surgelogic SDSA 3-Phase SPDs utilize high-energy suppression circuitry that can be located at any point in the electrical system. They have the flexibility to be used with or without an Overcurrent Protective Device (OCPD).

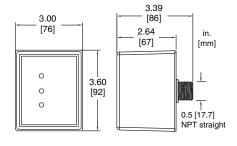
SDSA 3-Phase SPDs provide surge suppression for equipment from severe transient activity. Each metal oxide varistor (MOV) is individually fused and the products carry a NEMA Type 4X rating suitable for installing indoors, outdoors, or in other harsh environments.

## Easy Installation

Surgelogic SDSA 3-Phase SPDs are some of the most versatile, yet compact devices available on the market today. This compact package can be mounted on an electrical panel, meter socket, or inside electrical control cabinets.

## Warranty

The SDSA 3-Phase SPD warranty is 2 years.



## SDSA 3-Phase SPDs



## Performance

Surge Current Rating per Phase 40kA
Short Circuit Current Rating 200kA
Modes of Protection 6
Fusing Individually fused MOVs
Thermal Fusing Yes
Overcurrent Fusing Yes
Operating Frequency 50/60 Hz

## Mechanical Description

Enclosure Non-Metallic
NEMA Rating NEMA Type 4X
Connection Method #12 AWG
Weight 1.63 lbs (0.738 kg)
Mounting Method Close Nippled, Back Mounted
Operating Altitude Sea Level-12,000' (3,658 m)
Storage Temperature -31° F to +176° F
(-35° C to +80° C)
Operating Temperature -31° F to +176° F

(-35° C to +80° C)

Operating Humidity 0 to 95% non-condensing

Diagnostics Green status LED

Listings and Performance cULus Listed per UL 1449 4th Edition Type 1 SPD, CSA C22.2 No. 269.1-14

The SDSA2040, SDSA4040, and SDSA3650 is a four-wire surge suppressor designed for use on solidly grounded systems. The SDSA2040D, SDSA4040D, and SDSA3650D is a three-wire surge suppressor designed for delta applications.

	Surge Current	Modes of	I	I	I				VF	PR	
Voltage	per Phase	Protection	Configuration	Model Number	MCOV	SCCR	<b> </b>	L-N	L-G	L-L	N-G
208Y/120V <sup>1</sup>	40kA	6	3 Ø, 4-wire	SDSA2040	180V L-N 360V L-L	200kA	10kA	700V	N/A	1200V	N/A
240V Delta <sup>2</sup>	40kA	3	3 Ø, 3-wire	SDSA2040D	360V L-L	200kA	10kA	N/A	N/A	1200V	N/A
480Y/277V <sup>3</sup>	40kA	6	3 Ø, 4-wire	SDSA4040	420V L-N 840V L-L	200kA	10kA	1500V	N/A	2500V	N/A
480V Delta	40kA	3	3 Ø, 3-wire	SDSA4040D	840V L-L	200kA	10kA	N/A	N/A	2500V	N/A
600Y/347V	40kA	6	3 Ø, 4-wire	SDSA3650	510V L-N 1020V L-L	200kA	10kA	1800V	N/A	3000V	N/A
600V Delta	40kA	3	3 Ø, 3-wire	SDSA3650D	1020V L-L	200kA	10kA	N/A	N/A	3000V	N/A

- 1 Applicable voltages: 220Y/127V, 208Y/120V
- 2 Applicable voltages: 240V Delta, 240/120V High-Leg Delta
- 3 Applicable voltages: 480Y/277V, 415Y/240V, 400Y/230V, 380Y/220V

Schneider Electric USA, Inc. 800 Federal Street Andover, MA 01810, USA Telephone: (888) 778-2733 TAG: (800) 577-7353 www.schneider-electric.com/us Schneider Electric México, S.A. de C.V. AV. Ejercito Nacional No. 904, Col. Palmas, Polanco 11560 México, D.F. Tel. 55-5804-5000 www.schneider-electric.com.mx Schneider Electric Canada, Inc. 5985 McLaughlin Road, Mississauga, ON L5R 1B8 Canada Tel:1-800-565-6699 www.schneider-electric.ca

## **HDL36015**

Molded Case Circuit Breaker , 600VAC/250VDC, 15A





by Schneider Electric

## **Technical Characteristics**

Approvals	UL Listed - CSA Certified - IEC Rated
Catalog Reference Number	0611CT0401
Circuit Breaker Type	Standard
For Use With	Industrial Enclosures and Switchboards
General Application	Provides overload and short circuit protection
AC Magnetic Trip Setting	350A - 750A Adjustable Trip
Ampere Rating	15A
Frame Type	H-Frame
HACR Rated	Yes
Marketing Trade Name	Powerpact
Voltage Rating	600VAC/250VDC
Mounting Type	Unit Mount
Weight	5 Pounds
Number of Poles	3-Pole
Short Circuit Current Rating	25kA@240VAC - 18kA@480VAC - 14kA@600VAC
Туре	HD
Terminal Type	Line: Lug - Load: Lug
Wire Size	#14-3/0 AWG(Al/Cu)
Height	6.40 Inches
Width	4.12 Inches
Depth	4.36 Inches

## **Shipping and Ordering**

Category	01110 -
Discount Schedule	DE2
Article Number	785901577867
Package Quantity	1
Weight	4.06 lbs.
Availability Code	Non-Stock Item: This item is not normally stocked in our distribution facility.
Returnability	Υ

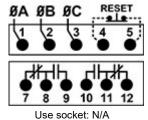
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Better. By Design.





Family: 3-PHASE MONITOR RELAY

Function: PHASE LOSS, PHASE REVERSAL, PHASE UNBALANCE, UNDERVOLTAGE

& OVERVOLTAGE

Line-Line Voltage: 190-500VAC

Output: 10A DPDT

Image may not be exact product—for reference only.

## **Description of function:**

When the proper 3-phase line voltage is applied to the unit and the phase sequence (rotation) is correct, the relay is energized after the Restart Delay is completed. Any one of five fault conditions will de-energize the relay after a delay. As standard, re-energization is automatic upon correction of the fault condition. Manual reset is available if an external momentary N.C. switch is connected to terminals 4 and 5. A bi-color status LED indicates normal condition and also provides specific fault indication to simplify troubleshooting (see right).

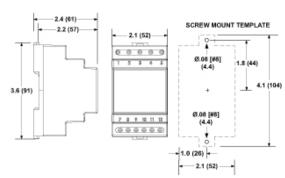
	LED STATUS	STATUS
GRE		NORMAL (RELAY ON)
EN	M	RESTART (DELAY)
		REVERSAL
RED		LOSS/UB (UNBALANCE)
D		LOW VOLT (UNDERVOLTAGE)
		HIGH VOLT (OVERVOLTAGE)

## **Application Information**

- Load (Burden): Less than 40 VA
- Response Times:
- Minimum/Maximum Voltage: 156-550V (PMDU); 77-152V (PMD120): 345-660V (PMD575), all 50/60Hz. ±5%
- Reset: As standard, the PMD Series relays are in the Automatic Reset mode. However, they can be set in the Manual Reset mode by connecting an external N.C. switch across terminals 4 and 5. Upon application of line voltage, the PMD Series will go into Manual Reset mode if it recognizes a closure across terminals 4 and 5. After a fault clears, the relay will not reset until the N.C. switch is opened.
- Output Contacts: 10A @ 277V AC/30V DC; 1/2HP @ 120/240V AC (N.O.),1/3HP @ 120/240V AC (N.C.), B300 Pilot Duty, R300 (N.O.)
- Life: Mechanical: 10,000,000 operations, Full Load: 100,000 operations
- Temperature: -28° to 65° C (-18° to 149° F)



## Dimensions Inches (Millimeters)



## **Industrial Control Transformers**



## The SBE - Encapsulated Series

The SBE Encapsulated industrial control transformers are epoxy encapsulated to seal the transformer windings against moisture, dirt and industrial contaminants. Extra deep, molded terminal barriers reduce the chance of electrical failure as the result of arcing or frayed lead wires. The rugged construction and proven reliability of the SBE design is uniquely suited for all industrial environments.

## **Features**

- 50 1000 VA, 50/60 Hz suitable for worldwide applications.
- Interleaved copper windings reduce I<sup>2</sup>R losses and maximize efficiency.
- 55°C Rise, 105°C insulation system to minimize heat
- Epoxy encapsulated to protect cores and coils against moisture, dirt, and other contaminants.
- Meets or Exceeds NEMA Standard ST 1 and ANSI C89.1 for load inrush capability.
- Integrally molded, flame retardant (IEC 707 / ISO Class 1210) Terminal Blocks provide greater terminal contact area and improved conductivity.
- · Heavy gauge steel mounting plate
- Mounting dimensions are compatible with similar control transformers.
- Secondary fuse holders (FB2X) included for 13/32 x 1- 1/2 cartridges (fuses not included).
- Factory-installed fuse holders are available (See W, WA & WB options).
- 10 + 2 year warranty

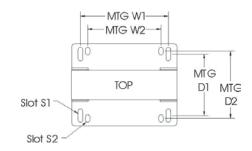




## Related Products

- · Linear Power Supplies
- DIN Rail DC Power Supplies
- · Constant Voltage Transformers
- Line Reactors

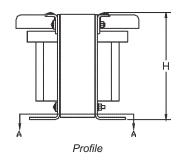
## **SBE Mounting Profiles**

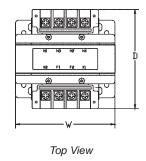


Mounting Dimensions

## **Accessories**

Catalog Number	Description
FBP	Primary "CC" Rejection Type Fuse Holder (Finger Safe covers not available)
FB2	Secondary Fuse Holder only (Glass or Ceramic, ¼" x 1¼" fuse)
FB2X	Secondary Fuse Holder only included where applicable. Not sold separately. (Midget Cartridge Type, 13/32" x 11/2" fuse)
FBPC1	Primary "CC" Rejection Type Fuse Holder and Finger Safe Cover Kit
IP20	IEC Touchproof Cover Kit
SBEDIN	IEC Fuse Holder Adaptor Kit
W	Factory installed Primary Fuse Holder with Midget Type (no covers)
WA	Factory installed Fuse Holder with Glass/Ceramic Type and Covers
WB	Factory installed Fuse Holder with Midget Type and Covers





## **Industrial Control Transformers**

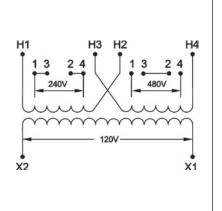


## **SBE Encapsulated Series Selection Tables**

Group 2 – 220 x 440 Volt Primary, 110 Volt Secondary, 50/60 Hz 230 x 460 Volt Primary, 115 Volt Secondary, 50/60 Hz 240 x 480 Volt Primary, 120 Volt Secondary, 60 Hz



VA	Catalog Number	Height (inch)	Width (inch)	Depth (inch)	Mtg Width W1 / W2	Mtg Depth D1 / D2	Slot Size S1 / S2	Approx. Ship Weight (lbs)
50	E050	2.72	3.01	3.99	2.51 / NA	2.02 / NA	.20 x .33 / .20 x .33	3
75	E075	2.96	3.39	4.36	2.81 / 2.50	2.10 / NA	.20 x .50 / .20 x .50	4
100	E100	2.96	3.39	4.61	2.81 / 2.50	2.37 / NA	.20 x .50 / .20 x .50	5
150	E150	3.89	4.5	4.48	3.74 / 3.12	2.56 / 2.87	.20 x .65 / .20 x .33	8
200	E200	3.89	4.5	4.79	3.74 / 3.12	2.87 / 3.18	.20 x .65 / .20 x .33	10
250	E250	3.89	4.5	5.21	3.74 / 3.12	3.29 / 3.61	.20 x .65 / .20 x .33	11
300	E300	4.53	5.25	5.09	4.38 / 3.75	3.10 / NA	.31 x .71 / .31 x .71	12
350	E350	4.53	5.25	5.53	4.38 / 3.75	3.54 / NA	.31 x .71 / .31 x .71	15
500	E500	4.53	5.25	6.31	4.38 / 3.75	4.33 / NA	.31 x .85 / .31 x .85	19
750	E750	5.56	6.38	6.93	5.32 / 4.37	4.25 / 5.75	.31 x .85 / .31 x .85	31
1000	E1000	5.56	6.38	7.36	5.32 / 4.37	4.68 / 6.18	.31 x .85 / .31 x .85	36



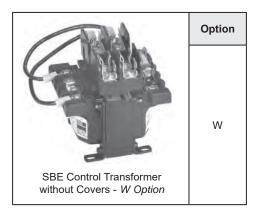
Note: Includes FB2X Secondary fuse holder.

## Group 2A - Factory Installed Primary Fuse Holder Class "CC" and:

- W Secondary Fuse Holder (Midget Cartridge, 13/32" x 11/2" fuse) supplied, no covers
- WA Secondary Fuse Holder (Glass or Ceramic Type 3AG, 1/4" x 11/4" fuse type)
- WB Secondary Fuse Holder (Midget Cartridge, 13/32" x 11/2" fuse)

	Primary Fuse Holder Class "CC"							Dimensions		
VA	W Option - Midget Type Catalog Number	WA Option - Type 3AG w/ Covers Catalog Number	WB Option - Midget Type w/ Covers Catalog Number	Height (inch)	Width (inch)	Depth (inch)	Mtg Width W1 / W2	Mtg Depth D1 / D2	Slot Size S1 / S2	Approx. Ship Weight (lbs)
50	E050W	E050WA	E050WB	4.18	3.01	3.99	2.51 / NA	2.02 / NA	.20 x .33 / .20 x .33	3
75	E075W	E075WA	E0750WB	4.41	3.39	4.36	2.81 / 2.50	2.10 / NA	.20 x .50 / .20 x .50	4
100	E100W	E100WA	E100WB	4.41	3.39	4.61	2.81 / 2.50	2.37 / NA	.20 x .50 / .20 x .50	5
150	E150W	E150WA	E150WB	5.36	4.5	4.48	3.74 / 3.12	2.56 / 2.87	.20 x .65 / .20 x .33	8
200	E200W	E200WA	E200WB	5.36	4.5	4.79	3.74 / 3.12	2.87 / 3.18	.20 x .65 / .20 x .33	10
250	E250W	E250WA	E250WB	5.36	4.5	5.21	3.74 / 3.12	3.29 / 3.61	.20 x .65 / .20 x .33	11
300	E300W	E300WA	E300WB	5.99	5.25	5.09	4.38 / 3.75	3.10 / NA	.31 x .71 / .31 x .71	12
350	E350W	E350WA	E350WB	5.99	5.25	5.53	4.38 / 3.75	3.54 / NA	.31 x .71 / .31 x .71	15
500	E500W	E500WA	E500WB	5.99	5.25	6.31	4.38 / 3.75	4.33 / NA	.31 x .85 / .31 x .85	19
750	E750W	E750WA	E750WB	7.01	6.38	6.93	5.32 / 4.37	4.25 / 5.75	.31 x .85 / .31 x .85	31
1000	E1000W	E1000WA	E1000WB	7.01	6.38	7.36	5.32 / 4.37	4.68 / 6.18	.31 x .85 / .31 x .85	36

Notes: WA and WB suffix include Finger Safe covers. Fuses not included. W option for secondary fusing requires assembly (FB2 sold separately).



	Option	Secondary Fusing	
	WA	Glass/ Ceramic - Type 3AG (FB2)	
SBE Control Transformer with Covers - WA & WB Option	WB	Midget Type (FB2X)	C. S.





## Mini circuit breaker, QOU, 10A, 1 pole, 120/240VAC, 10kA

QOU110

Main

Product or Component Type	Miniature circuit-breaker
Range of Product	QOU
Circuit breaker type	Standard
Circuit breaker application	HACR rated

Complementary

Line Rated Current	10 A
Electrical connection	Slotted box lugs, line side Slotted box lugs, load side
[Ue] rated operational voltage	120/240 V AC 120 V AC 48 V DC
Mounting Mode	Unit mount
AWG gauge	AWG 14AWG 2 aluminium/copper
Height	4.05 in (102.87 mm)
Width	0.75 in (19.05 mm)
Depth	2.95 in (74.93 mm)
Tightening torque	45 lbf.in (5.08 N.m) (AWG 14AWG 2)

## **Environment**

Product Certifications	IEC	
i roddot Goramodiono		
	CSA	
	UL Listed	

Ordering and shipping details

	<u> </u>	
Category 00900-QOU BREAKERS & SWITCH		
Discount Schedule	DE2	
GTIN	785901205678	
Returnability	Yes	
Country of origin	MX	

<sup>\*</sup> Price is "List Price" and may be subject to a trade discount – check with your local distributor or retailer for actual price.

## **Packing Units**

Unit Type of Package 1	PCE
Number of Units in Package 1	1
Package 1 Height	1.00 in (2.54 cm)
Package 1 Width	3.10 in (7.874 cm)
Package 1 Length	4.50 in (11.43 cm)
Package 1 Weight	6.40 oz (181.437 g)
Unit Type of Package 2	CAR
Number of Units in Package 2	66
Package 2 Height	3.30 in (8.382 cm)
Package 2 Width	14.00 in (35.56 cm)
Package 2 Length	20.90 in (53.086 cm)
Package 2 Weight	24.75 lb(US) (11.226 kg)
Unit Type of Package 3	PAL
Number of Units in Package 3	2772
Package 3 Height	40.00 in (101.6 cm)
Package 3 Width	40.00 in (101.6 cm)
Package 3 Length	48.00 in (121.92 cm)
Package 3 Weight	1014.00 lb(US) (459.942 kg)

## Offer Sustainability

Sustainable offer status	Green Premium product
REACh Regulation	REACh Declaration
EU RoHS Directive	Compliant EU RoHS Declaration
Toxic heavy metal free	Yes
Mercury free	Yes
China RoHS Regulation	China RoHS declaration Product out of China RoHS scope. Substance declaration for your information.
RoHS exemption information	Yes
Halogen content performance	Halogen free product

## **Contractual warranty**

Warranty 18 months

## Switching Power Supplies **PS5R-V Series**





## STANDARDS COMPLIANCE

Applicable Standards	Mark	File No. or Organization
UL508 UL1310 <sup>1</sup> ANSI/ISA 12.12.01 CSA C22.2 No.107.1 CSA C22.2 No.213 CSA C22.2 No.223 <sup>1</sup>	CUL US	UL/c-UL Listed File No. E467154, E177168
EN60950-1 EN50178		TÜV SÜD <sup>2</sup>
EN50176 EN61204-3 EN50581	( (	EU Low Voltage Directive EMC Directive RoHS Directive
SEMI F47	-	EPRI

Note 1: PS5R-VA/VB/VC/VD/VE only Note 2: EN60950-1, EN50178 only

## **POWER SUPPLY PART NUMBERS**

Output Capacity	Part Number	Input Voltage	Output Voltage	Output Current
	PS5R-VA05		5V	1.5A
7.5W	PS5R-VA12		12V	0.6A
	PS5R-VA24		24V	0.3A
10W	PS5R-VB05		5V	2.0A
15W	PS5R-VB12	100 to 240V AC (Voltage range: 85 to 264V AC / 100 to 370V DC)	12V	1.3A
1344	PS5R-VB24		24V	0.65A
30W	PS5R-VC12		12V	2.5A
3000	PS5R-VC24		24V	1.3A
60W	PS5R-VD24		24V	2.5A
90W	PS5R-VE24		24V	3.75A
120W	PS5R-VF24		24V	5.0A
240W	PS5R-VG24		24V	10.0A

## **Part Number Structure**

PS5R - V □ □ **Output Voltage** 05: 5V<sup>3</sup> 12: 12V<sup>4</sup> Note 3: PS5R-VA/VB only Note 4: PS5R-VA/VB/VC only Use only for interpreting part numbers. Do not use for developing part numbers. D: 60W

**Output Capacity** B: 10W/15W C: 30W

F: 120W G: 240W

## PRODUCT DESCRIPTION

DIN-rail mount switching power supplies with global approvals for both industrial and hazardous locations

## **KEY FEATURES**

- Compact size preserves panel space
- Slim size (width): 22.5mm (10W/15W/30W) 36mm (60W/90W) 45mm (7.5W) 46mm (120W) 60mm (240W)
- Universal Voltage Input: 85-264V AC/100-370V DC
- Wide operating temperature range
- Spring-up terminals accept ring & fork terminals
- Approved for use in Class I Division 2 hazardous locations
- Can be installed in 6 directions
- 7.5W ~ 90W meet NEC Class 2 output ratings
- Overcurrent protection with auto-reset
- Meets SEMI F47 Sag Immunity (208V AC input)
- RoHS compliant
- Five-year factory warranty







## **SPECIFICATIONS**

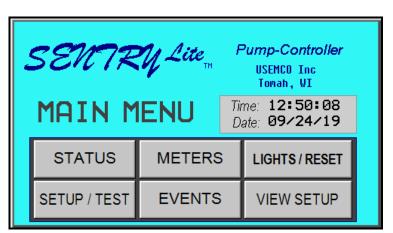
Mode	el	12V E	C output OC output	PS5R-VA05 PS5R-VA12	PS5R-VB05 PS5R-VB12	PS5R-VC12	-	-	-	-
			OC output	PS5R-VA24	PS5R-VB24	PS5R-VC24	PS5R-VD24	PS5R-VE24	PS5R-VF24	PS5R-VG24
ıtpu	t Cap	-		7.5W	15W (5V Model is 10W)	30W	60W	90W	120W	240W
	(Sing	d Input Voltage gle-phase two-wire) <sup>1</sup>			(Voltage range: 8	35 to 264V AC/100		d ≤ 80% at 100	-105V DC)	
	Freq	uency	1007.40	5V: 0.20A	5V: 0.25A		0/60 Hz	1.1.4	1.4.4	2.74
	Inpu	t Current (Typ.)	100V AC 230V AC	12V, 24V: 0.18A 5V: 0.12A	12V, 24V: 0.35A 5V: 0.14A	0.7A 0.3A	1.3A 0.8A	1.1A 0.6A	1.4A 0.7A	2.7A 1.2A
	Inrus	sh Current (Typ.)	100V AC	12V, 24V: 0.10A 15A	12V, 24V: 0.19A	0.5A	18A	0.0A	0.7A	1.2A
Input		25°C, cold start)	230V AC	36A		45A	1071		41A	30A
_			120V AC				5mA max.			
	Leak	age Current	230V AC			1.0	DmA max.			
	Effic	iency (Typ.)	100V AC	5V: 74%, 12V: 79%, 24V: 80%	5V: 77%, 12V: 82%, 24V: 84%	12V: 83%, 24V: 85%	86%		000/	89%
		ated output) <sup>2</sup>	230V AC	5V: 73%, 12V: 77%, 24V: 76%	5V: 73%, 12V: 80%, 24V: 81%	12V: 85%, 24V: 87%	86%		88% 89%	90%
			100V AC	Z4V. 7070 —	24V. 01/0 —	24 V. 07 /0 —	_		0.99	
	Pow	er Factor (Typ.)	230V AC	_	_	_	_	0.86	0.92	0.96
	Rate	d Voltage/Current		5V/1/5A, 12V/0.6A, 24V/0.3A	5V/2.0A <sup>3</sup> , 12V/1.3A,	12V/2.5A,	24V/2.5A	24V/3.75A	24V/5A	24V/10A
		stable Voltage Range		0.1, 1, 0.1, 12 1, 0.0A, 27 1, 0.0A	24V/0.65A ±10%	24V/1.3A	Z 1 1/ Z.OA	±5%	±10%	277/107
	Auju	Stable voltage hallge		5V: 45ms, 12V: 45ms,	5V: 53ms, 12V: 34ms,	12V: 13ms,	40			
	Outp (Typ.	ut Holding Time .)	100V AC	24V: 47ms 5V: 289ms	24V: 36ms 5V: 330ms	24V: 15ms 12V: 110ms	13ms	20ms	30ms	
	· ·	ated output)	230V AC	12V: 294ms 24V: 282ms	12V: 215ms 24V: 230ms	24V: 110ms	105ms	30ms	33ms	40ms
	Star	t Time (at rated input a	nd output)	450ms max.	500ms max.	600ms max.	800m	s max.	700ms max.	800ms max.
	Rise	Time (at rated input an	nd output)	220ms max	5V, 12V: 200ms max. 24V: 250ms max.			200	lms max.	
		Input Fluctuation				0.4	4% max.			
		Load Fluctuation		5V: 2.5% max. 12	V, 24V: 1.0% max.			1.0	1% max.	
Output		Temperature Chang	е	0.04%/°C max. (-10 to +65°C)	0.05%/°C max. (-10 to +65°C)	12V: 0.05%/°C max. (-10 to +50°C) 24V: 0.05%/°C max. (-10 to +55°C)		0.05%/°C max (-10 to +50°C)		0.05%/°C max. (-25 to +50°C)
	Regulation			5V: 8% p-p max. (-25 to -10°C) 12V: 6% p-p max. (-25 to -10°C) 24V: 4% p-p max. (-25 to -10°C)	5V: 8% p-p max. (-25 to -10°C) 12V: 6% p-p max. (-25 to -10°C) 24V: 4% p-p max. (-25 to -10°C)	12V: 6% p-p max. (-25 to -10°C) 24V: 4% p-p max. (-25 to -10°C)		4	1% p-p max. (-25 to -10°C)	
	Rei	Ripple (including noise)		5V: 5% p-p max. (-10 to +0°C) 12V: 2.5% p-p max. (-10 to +0°C) 24V: 1.5% p-p max. (-10 to +0°C)	5V: 5% p-p max. (-10 to +0°C) 12V: 2.5% p-p max. (-10 to +0°C) 24V: 1.5% p-p max. (-10 to +0°C)	12V: 2.5% p-p max. (-10 to +0°C) 24V: 1.5% p-p max. (-10 to +0°C)		1.	5% p-p max. (-10 to +0°C)	
				5V: 2.5% p-p max. (0 to +65°C) 12V: 1.5% p-p max. (0 to +65°C) 24V: 1% p-p max. (0 to +65°C)	5V: 2.5% p-p max. (0 to +65°C) 12V: 1.5% p-p max. (0 to +65°C) 24V: 1% p-p max. (0 to +65°C)	12V: 1.5% p-p max. (0 to +50°C) 24V: 1% p-p max. (0 to +55°C)	1% p-p max. (0 to +55°C)	1% p-p max. (0 to +50°C)	1% p-p max. (0 to +55°C)	1% p-p max. (0 t +50°C)
verc	urrent	Protection			105% min. (auto reset)			101% min.	105% min. (auto	reset)
		ndicator				I E	D (green)	(auto reset)	100 /0 (dd.)	,
		reen input and output te	erminals				AC, 1 minute			
Strength		een input and ground to					AC, 1 minute			
Str		een output and ground					AC, 1 minute			
	ation F	Resistance				out and output term out and ground term				
pera	ating 1	emperature <sup>4</sup> (No fre	ezing)	-25 to	+75°C	-25 to +		,	-25 to +65°C	
)pera	ating H	lumidity				20 to 90% RF	H (no condensati	on)		
Stora	ge Tei	mperature (No freezir	ng)			-25	to +75°C			
tora	ge Hu	midity				20 to 90% RF	H (no condensation		40. 5511 15. 1.004	40 : 5511
		esistance			tude 0.375mm, 2 hours each in 3 used with BNL6 end clips)		10 to 55Hz, ampli hours each in 3 a with BNL6 end c 10 to 55Hz, ampl 2 hours each in 3 used with BNL8	xes (when used lips) litude 0.375mm, 3 axes (when end clips)	10 to 55Hz, amplitude 0.21mm, 2 hours each in 3 axes (when used with BNL6 end clips) 10 to 55Hz, amplitude 0.375mm, 2 hours each in 3 axes (when used with BNL8 end clips)	10 to 55Hz, amplitu 0.375mm, 2 hours each in 3 axes (when use with part no. BNL6 mounting clips)
		stance				300 m/s <sup>2</sup> (30G), 3 t			dord mounting disasting)	
xpe	cted L	EMI			B years minimum (at the rated in		erating temperat 04-3 (Class B)	ure +40°6, stan	uaru mounting direction)	
MC		EMS					4-3 (class B) 4-3 (industrial)			
afet	y Star	ndards		U	L508 (Listing), UL1310 Class SA C22.2 No. 107.1, 213, 223	2, ANSI/ISA-12.12.	01		UL508 (Listing) ANSI/ISA-12.1 107.1, 213 EN60950-1, EN5	
ther	Stan	dard			, ,		208V AC input or	nly)	,	
egre	ee of P	rotection				IP20	(EN60529)			
)ime	nsions	s (mm)		$75H \times 45W \times 70D$	90H × 22.5W ×	95D	95H × 36	W × 108D	115H × 46W × 121D	125H × 60W × 12
	nt (app			130g	140g	150g	260g	310g	470g	960g
ermi	inal So	crew					M3.5			

<sup>\*</sup>At normal temperature and humidity unless otherwise specified. Notes: 1: DC input voltage is not subject to safety standards. When using on DC input, connect a fuse to the input terminal for DC input protection. 2: Under stable state. 3: PS5R-VB05 (5V DC/2.0A) is 10W (Up to 3.0A at Ta = 0 to 40°C. Not subject to safety standards above 2.0A.) 4: See the output derating curves on page 3. 5: Calculation of the expected life is based on the actual life of the aluminum electrolytic capacitor. The expected life depends on operating conditions.



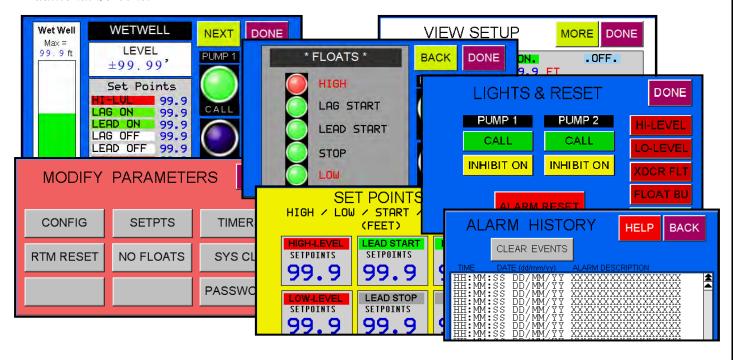
## SEMTZY Lite IM

## Lift-Station Pump Controller



- Bright Color Touch-screen
- Easy screen navigation
- Accurate systemmonitoring
- Adjustable setting to fit your application and system-requirements.

## Additional Screens:



The SENTRY Lite m pump controller by USEMCO Inc. is designed with simplicity in-mind. This controller uses state-of-the-art components and incorporates control feature envied by others in the industry. All this is packaged into one unit. Up front is a bright LCD color touch-screen that can produce over 32,000 different colors. Each screen is individually designed to provide the user with the maximum amount of feedback and information. Navigation through the different screens is simple and easy to understand.

Alarm monitoring is critical and done with ease by the **SENTRY** Lite TM pump controller. Note that every alarm event is time-and-date stamped and placed in one area for easy access and viewing.

Setup is simple and requires little to no training. Field adjustable parameters like transducer rating & offset, set points and timer-delays allow the user to custom fit this controller to the pumping system.

## **Hardware Specifications:**

Power: 24VDC +/- 10%, 8W max Ports: 2 Serial: RS232/RS485 (DB9)
Bezel: IP 66 / (NEMA 4) USB: Prog/SCADA/device
Screen: 4.3" 480 x 272 pixel Ethernet (opt.): Modbus TCP/IP, Prog.

LCD Type: TFT Color Touch screen Operating Temperature: 0C to 50C Colors: 32,000 Approvals: CE, UL (Class 1 Div 2),

Backlight: LED RoHS compliant

Mounting: Front Panel Mounting, IP66, N4/4X rated

Digital Inputs: 12 DC inputs, Bi-directional (2 high-speed)

24 VDC (Max 30 VDC)

H/S Max input frequency 200 KHz

Digital Outputs: 8 Relay outputs

2 PNP

Analog Inputs: 2 (0-5V / 0-10V / 4-20mA / 0-20mA)

16-bit resolution

Analog Outputs: 1 (4-20mA / 0-20mA / 0-5V / 0-10V)

12- bit resolution



## BLUE RIBBON CORP.

2770 Long Road, Grand Island, NY 14072 USA Tel. (716) 773-9300 • Fax (716) 773-5019 brsales@blueribboncorp.com • www.blueribboncorp.com

## SUBMERSIBLE / LEVEL TRANSMITTER MODEL BC001 - BIRDCAGE® SERIES

LIFE-TIME SURGE WARRANTY REPLACEMENT PROGRAM

(Consult factory for details)



- **Surge Protector**
- Enhanced Internal Surge & Lightning Protection
- 2.5" Sensing Diaphragm with Dual Internal Diaphragm
- · Corrosion Resistant, 316 SS Rugged, Welded Construction
- 4-20 mA, (0-5 VDC optional upon request)
- · Glycerine-filled, other fluids available
- \*\* FM, CSA, CE, and/or ATEX/IEC \*\* Approval Available Upon Request
- Optional 4-20 mA Temperature Output

Blue Ribbon reserves the right to make product improvements and amendments to the product specifications stated throughout this brochure without prior notification.

Please contact the factory on all critical dimensions and specifications for verification.

Ø3.8REF.

DIAPHRAGM

SEAL



## **BLUE RIBBON CORP.**

2770 Long Road, Grand Island, NY 14072 USA Tel. (716) 773-9300 • Fax (716) 773-5019 brsales@blueribboncorp.com • www.blueribboncorp.com

Standard Specifications listed below. Consult factory for further options, which may change the overall dimensions.

Output (at 70°F)	<b>Model BC001</b> 4-20mA 9.0 – 36 VDC Ex 0-5 VDC 9.0 – 40 VDC Ex		
Accuracy	Static Accuracy 0.5% FSO BFSL (0.2% optional)	1 Year Stability 0.20% FSO	Error Band (with temp.) 1.0% BFSL FSO
Pressure Ranges	0-5 PSI to 0-100 PSI		
Zero Balance (at 70°F)	± 1% FSO		
Range Calibration Signal	Internal Calibration Resis	tor set to 100 ±0.5% FSO unles	ss otherwise specified
Insulation Resistance	>10 megohms @ 50 VDC	& @ 70°F	
Temperature Limits	Compensated +35°F to +125°F	<b>Operating</b> 0°F to +140°F	Storage -40°F to +250°F
Temperature Compensation (Standard)	<b>Zero / Span</b> Less than ±2% FSO/100°F	at FSPR and/or URL	
Electricals	Load Impedance 1350 ohms maximum at 36 RFI Protection Voltage Surge Suppression Lightning Protection	VDC and 750 ohms maximum a	t 24 VDC
Electrical Connection		forced Cable with Integral vent to gths available upon request)	ube and Hydrophobic filter
Mechanicals	Proof Pressure 3X FSPR (or URL)	Burst Pressure 5X FSPR (or URL	)
Pressure Response	≤ 5 ms to 90%		
Weight	5.5 lbs.		
Wetted Material	Case 316 Stainless Steel 2.75" diameter Sensing Diaphragm - 316 SS	vent tube and Hyd	efzel Reinforced Cable with Integral Irophobic filter - other lengths available upon request)
Accessories	BCP3000 Surge Protector LIFE-TIME SURGE WARRANT BCP3005 Pluggable Surge BCH2000 Cable Hanger		

## **ORDERING GUIDE FOR MODEL 01:**

PART #	PRESSURE RANGE	CABLE LENGTH
•	•	●/●/●

PART #: 01

## **PRESSURE RANGE:**

<u>PSI</u> or	FT. WC
5	11.53
10	23.07
15	36.60
17.5	40.37
20	46.14
25	57.67
30	69.21
100	230.70
	5 10 15 17.5 20 25 30

## **WIRING CODE**

WIRE COLOR	BC001
RED	+ EXC./SIGNAL
BLACK	- EXC./SIGNAL
SHIELD	OPEN

## **CABLE LENGTH:**

40' Polyurethane Cable (Standard) AA Alternate Length of Cable (in feet optional) 70 ft. AA-CZ

AA/CT 40' Tefzel Cable AA/VDC 0-5 VDC

AA/FM Intrinsically Safe - FM

(Custom lengths of cable available upon request)

EXAMPLE: 01001AA

5 PSI with 40' CABLE

Signaling Lights

## **RH Series Compact Power Relays**

## **Key features**

- SPDT through 4PDT, 10A contacts
- Compact power type relays
- Miniature power relays with a large capacity
- 10A contact capacity
- Compact size saves space











## **Part Number Selection**

		Part	Number	
Contact	Model	Blade Terminal	PCB Terminal	Coil Voltage Code (Standard Stock in bold)
	Standard	RH1B-U □	RH1V2-U □	
SPDT	With Indicator	RH1B-UL □	_	AC6V, AC12V, <b>AC24V</b> , AC110V, <b>AC120V</b> ,
4.6	With Check Button	RH1B-UC □	_	AC220V, <b>AC240V</b> DC6V, <b>DC12V</b> , <b>DC24V</b> ,
	With Indicator and Check Button	RH1B-ULC □	_	DC48V, DC110V
	Top Bracket Mounting	RH1B-UT □	_	
Thu .	With Diode (DC coil only)	RH1B-UD □	RH1V2-UD □	DC6V, <b>DC12V</b> , <b>DC24V</b> , DC48V, DC110V
	With Indicator and Diode (DC coil only)	RH1B-ULD □		DC12V, DC24V, DC48V, DC110V
DPDT	Standard	RH2B-U □	RH2V2-U □	
וטייטו	With Indicator	RH2B-UL □	RH2V2-UL □	AC6V, AC12V, <b>AC24V</b> , <b>AC110-120V</b> ,
WATER TO	With Check Button	RH2B-UC □	_	AC220-240V
	With Indicator and Check Button	RH2B-ULC □	_	DC6V, <b>DC12V</b> , <b>DC24V</b> , DC48V, DC100-110V
THE RESERVE TO A	Top Bracket Mounting	RH2B-UT □	_	
Man Colla	With Diode (DC coil only)	RH2B-UD □	RH2V2-UD □	DC6V, <b>DC12V</b> , <b>DC24V</b> , DC48V, DC100-110V
	With Indicator and Diode (DC coil only)	RH2B-ULD	RH2V2-ULD □	DC6V, DC12V, DC24V, DC48V, DC100-110V
3PDT	Standard	RH3B-U	RH3V2-U □	
3501	With Indicator	RH3B-UL □	RH3V2-UL □	AC6V, AC12V, <b>AC24V</b> , AC110V, <b>AC120V</b> ,
THE PARTY NAMED IN	With Check Button	RH3B-UC □	_	AC220V, <b>AC240V</b> DC6V, <b>DC12V</b> , <b>DC24V</b> ,
	With Indicator and Check Button	RH3B-ULC □	_	DC48V, DC110V
The state of the s	Top Bracket Mounting	RH3B-UT □	_	
W. Color	With Diode (DC coil only)	RH3B-UD □	_	DC6V, DC12V, DC24V, DC48V, DC110V
	With Indicator and Diode (DC coil only)	RH3B-ULD □	_	DC0V, DC12V, DC24V, DC46V, DC110V
4PDT	Standard	RH4B-U □	RH4V2-U □	
4701	With Indicator	RH4B-UL □	RH4V2-UL □	AC6V, AC12V, <b>AC24V</b> , AC110V, <b>AC120V</b> ,
	With Check Button	RH4B-UC □	_	AC220V, <b>AC240V</b> DC6V, <b>DC12V</b> , <b>DC24V</b> , DC48V,
	With Indicator and Check Button	RH4B-ULC □	_	DC110V
The state of the s	Top Bracket Mounting	RH4B-UT □	_	
	With Diode (DC coil only)	RH4B-UD □	RH4V2-UD □	DCSV DC12V DC2AV DC40V DC110V
	With Indicator and Diode (DC coil only)	RH4B-ULD □	_	DC6V, DC12V, DC24V, DC48V, DC110V



PCB terminal relays are designed to mount directly to a circuit board without any socket.

## **Ordering Information**

When ordering, specify the Part No. and coil voltage code:

(example) RH3B-U

RH3B-U AC120V

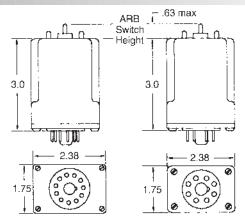
Part No. Coil Voltage Code

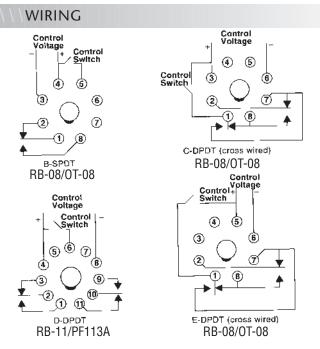




## Duplexor

## **DIMENSIONS** (INCHES)



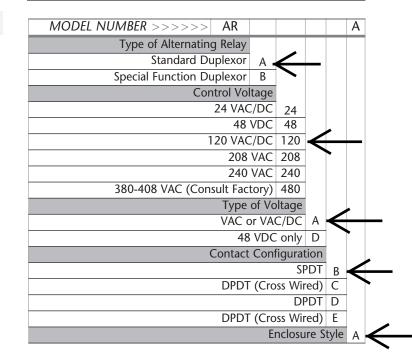


The **Duplexor** is used in control panels where **two loads** are required to alternate to provide equal run time on the loads. The alternating action is initiated by a control switch, which is common with one side of the control voltage. The output contacts will change states each time the control switch is opened, thus alternating the two loads. The LED indicators show the position of the output relay.

The ARA series is the standard Duplexor providing automatic alternating sequence. The ARB has the automatic sequencing feature plus the option of locking it into one sequence. A three position switch permits the field selection of normal duplexing action, locking in the A-B sequence, or B-A sequence.

\\\SPECIFICATIONS					
CONTROL VOLTAGE		24, 120 VAC/DC, 208, 240, 480 VAC, 50/60Hz, 48 VDC, ±10%			
CONTROL SWITCH CURRENT	1 mA	l mA			
POWER REQUIRED	3 VA (Approxi	mately)			
DUTY CYCLE	Continuous				
LIFE EXPECTANCY	Mechanical	10,000,000 Operations (Minimum)			
	Electrical	100,000 Operations @ Rated Load			
INDICATORS	LED Shows Ou	utput Position			
TEMPERATURES	Operate	-4° to 131°F (-20° to +55°C)			
rating	Storage	-40° to 185°F (-40° to +85°C)			
CONTACT RATING	10 Amps @ 240 VAC, Resistive 1/8 hp @ 120 VAC 1/4 hp @ 240 VAC Inductive 360 VA @ 240 VAC, Inductive				
ENCLOSURE	"A" Lexan® Dust Cover				
TERMINATIONS	Industrial Plug-in				
WEIGHT	4.5 oz.				
NOTE: For Analog signal inputs, ATC offers a duplexing pump					

NOTE: For Analog signal inputs, ATC offers a duplexing pump control — the ATC-Digitec 3800 Panel Meter 480 VAC is not available in the D-DPDT 11-Pin configuration



## Product data sheet Characteristics

## ZB4BA2

Harmony, 22mm Push Button, flush push button head, spring return, black, unmarked



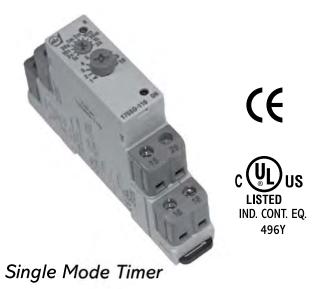


## Main

Range of Product	Harmony XB4	
Product or Component Type	Head for non-illuminated push-button	
Device short name	ZB4	
Bezel material	Chromium plated metal	
Mounting diameter	0.87 ln (22 mm)	
Sale per indivisible quantity	1	
Head type	Standard	
Shape of signaling unit head	Round	
Type of operator	Spring return	
Operator profile	Black flush, unmarked	

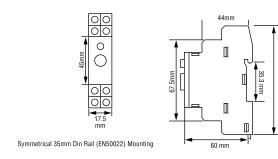
## Complementary

Device presentation	Basic element	
	C15 1 single front mounting	
	C2 9 single and double front mounting C11 3 single front mounting	
Electrical composition code	C1 9 single front mounting	
Mechanical durability	10000000 Cycles	
CAD overall depth	1.10 ln (28 mm)	
CAD overall height	1.14 ln (29 mm)	
CAD overall width	1.14 ln (29 mm)	

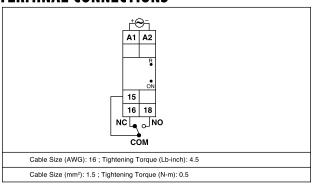


- On Delay
- 10 Time Ranges
- Front knobs for Time Range & Time Scale
- Slim, Space Saving Design
- DIN Rail Mount

## **DIMENSIONS** (MILLIMETERS)



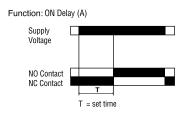
## **TERMINAL CONNECTIONS**



## **SPECIFICATIONS**

ACCURACY	Setting: ±5% of F.S.  Repeat: ±0.5% (F.S. = Full Scale)
RESET	Reset time < 100 msec
OUTPUT CONTACT	SPDT (1 C/O)
CONTACT RATING	NO/5A, NC/3A @ 250V AC
MODES	On delay (A)
TIME RANGES	0.1 - 1 sec, 0.3 - 3 sec, 1-10 sec, 3-30 sec 0.1-1 min, 0.3-3 min, 1-10 min, 3-30 min 0.1-1 hr, 0.3-3 hr
SUPPLY	110V AC : (50 or 60 Hz)
POWER CONSUMPTION	4.0 VA
TEMPERATURE	Operating: 0 to 50°C (32 to 122°F) Storage: -20 to 75°C (-4 to 167°F)
HUMIDITY (NON-CONDENS)	95% RH I <b>NG</b> )
WEIGHT	2.151 oz.
PROTECTION LEVEL	NEMA 12

## **TIMING DIAGRAM**



## ORDERING INFORMATION

**PART NO. SUPPLY VOLTAGE** 175SO-110 110V AC

## ZB4BD3

## black selector switch head Ø22 3-position stay put



## Main

Commercial Status	Commercialised
Range of product	Harmony XB4
Product or component type	Head for selector switch
Device short name	ZB4
Bezel material	Chromium plated metal
Mounting diameter	0.87 in (22 mm)
Sale per indivisible quantity	1
Shape of signaling unit head	Round
Type of operator	Stay put
Operator profile	Black standard handle
Operator position information	3 positions +/- 45°

## Complementary

CAD overall width	1.14 in (29 mm)
CAD overall height	1.14 in (29 mm)
CAD overall depth	1.73 in (44 mm)
Product weight	0.09 lb(US) (0.04 kg)
Resistance to high pressure washer	1015.26 psi (7000000 Pa) at 131 °F (55 °C),distance: 0.1 m
Mechanical durability	1000000 cycles
Electrical composition code	C11 for <= 3 contacts using single blocks in front mounting C8 for <= 4 contacts using single and double blocks in front mounting C7 for <= 4 contacts using single blocks in front mounting C6 for <= 5 contacts using single and double blocks in front mounting C5 for <= 5 contacts using single blocks in front mounting C4 for <= 6 contacts using single and double blocks in front mounting C3 for <= 6 contacts using single blocks in front mounting

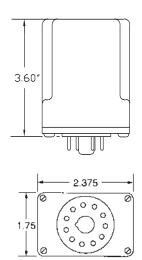
## Environment

Protective treatment	TH
Ambient air temperature for storage	-40158 °F (-4070 °C)
Ambient air temperature for operation	-13158 °F (-2570 °C)
Class of protection against electric shock	Class I conforming to IEC 60536
IP degree of protection	IP69K conforming to IEC 60529
NEMA degree of protection	NEMA 4X NEMA 13
IK degree of protection	IK06 conforming to IEC 50102
Standards	EN/IEC 60947-1 EN/IEC 60947-5-1 EN/IEC 60947-5-4 EN/IEC 60947-5-5 JIS C 4520 UL 508 CSA C22.2 No 14

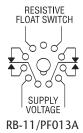
## **OPERATION**

The ATC Diversified SPM Series Single channel seal failure module is a specialized control for monitoring the **shaft seal** of a **submersible pump motor.** A leak is detected by sensing the position of a resistive float switch installed in the seal cavity. When the resistance drops below the sensitivity rating, the output relay energizes and the LED illuminates. When the fault condition clears, the output relay resets automatically.

## **DIMENSIONS (INCHES)**



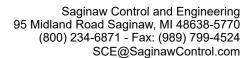
## WIRING



MODEL NUMBER >>>>> SPM   120   AAA	
Sensitivity	
470 ½ ±10% Fixed	470
470 ½ to 10K ½±10% Adjustable	10K
4.7K ½ to 100K ½ ±10% Adjustable	100K •



\\\SPECIFICA	TIONS			
CONTROL VOLTAGE	120 VAC, 50/60 Hz			
SWITCH VOLTAGE	9 VDC			
ISOLATION	2500 Volts			
POWER REQUIRED	2 VA			
DUTY CYCLE	Continuous			
	470 ½ ±10% F	ixed		
SENSITIVITY	470 ½ to 10K ½ ±10% Adjustable			
	4.7K ½ to 100K ½ ±10% Adjustable			
CONTACT RATING	DPDT, 10 A @ 250 VAC Resistive, 360 VA Inductive			
RESPONSE TIMES	Operate	15 ms (approximately)		
REST OTASE THATES	Release	8 ms (approximately)		
LIFE	Mechanical	10,000,000 Operations (Minimum)		
EXPECTANCY	Electrical	50,000 Operations @ Rated Load		
INDICATORS	Red LED illumi	nates when leak is detected		
TEMPERATURE	Operate	-4° to 131°F (-20° to +55°C)		
RATING	Storage	-40° to 185°F (-40° to +85°C)		
• ENCLOSURE	11-Pin plug-in	"A" style enclosure		
WEIGHT	8 oz.			





Your Enclosure Source ®

## **SCE-TEMNC**



## **Product Specifications:**

Part Number: SCE-TEMNC

Description: Thermostat (Normally Closed)
Height: 2.40"
Width: 1.26"

Depth: 1.42"

**Application**Designed to regulate air temperature in enclosures that operate with heaters or fans. This mechanical bi-metallic thermostat has a set point range of 30° to 140° F and is easily installed on 35mm mounting rail. (NC) contact normally closed, or (NO) contact normally open, switch capacity 10 amp 120-250 VAC Resistive load and 1 amp 120-250 VAC Inductive load, 1.25 amp 24VDC.

## Industry Standards - (IS24) UL Component Recognized

## Notes

UL File # E358385



Saginaw Control and Engineering 95 Midland Road Saginaw, MI 48638-5770 (800) 234-6871 - Fax: (989) 799-4524 SCE@SaginawControl.com

## SCE-TSH100

## **Product Specifications:**



**Watts**: 100

Max Current: 5.0 A UL File Model Number: 501100

Similar Part Numbers SCE-TSH150Heater - 150W SCE-TSH25Heater - 25W SCE-TSH50Heater - 50W SCE-TSH75Heater - 75W

## **Installation Information**

Touch Safe Heaters



**Application**PTC (Positive Temperature Coefficient)
Din rail clip design for 35mm Din Rail. Designed for protection from low temperatures, condensation and corrosion. Touch Safe with 2 screw terminal for standard AWG 14 wire. 110-240V AC or DC.

## Industry Standards - (IS24) UL Component Recognized

## Notes

UL File #E358386

## Product data sheet Characteristics

## 9070T50D13 TRANSFORMER CONTROL 50VA 120V-12/24V





Product availability: Stock - Normally stocked in distribution facility

	$\boldsymbol{\sim}$	

Commercial Status	Commercialised
Product or component type	Industrial Control Transformer
Product certifications	CE CSA UL listed
NEMA degree of protection	Not rated (open device)
Insulation temperature	221 °F (105 °C)
Phase	1 phase
Rated power in VA	50 VA
Primary voltage	120 V
Secondary voltage	12/24 V
Temperature rise	55 °C
Electrical connection	Screw clamp terminals
Range of product	Т

## Ordering and shipping details

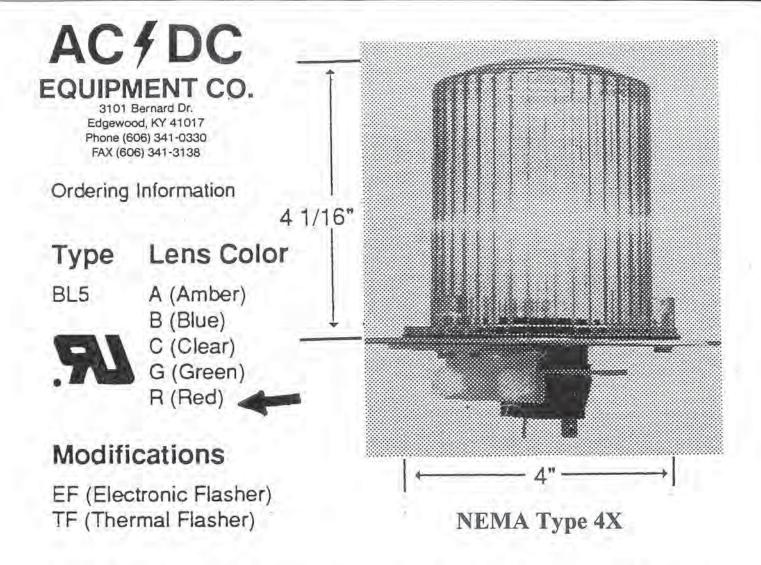
Category	16200 - 9070 T (NOT TF) 50-200VA	_
Discount Schedule	CP8	
GTIN	00785901905578	
Nbr. of units in pkg.	1	
Package weight(Lbs)	2.56	
Product availability	Stock - Normally stocked in distribution facility	
Returnability	Υ	
Country of origin	MX	

## Offer Sustainability

Sustainable offer status	Green Premium product
RoHS (date code: YYWW)	Compliant - since 1227 - Schneider Electric declaration of conformity
REACh	Reference not containing SVHC above the threshold
Product environmental profile	Available
Product end of life instructions	Need no specific recycling operations

## Contractual warranty

Warranty period	10 Years	



The BL5 with a 40 watt lamp and fluted lens will emit approximately the same amount of light as a vapor-tight with a 100 watt lamp and globe.

These lights use a 40A15 clear lamp. When specified, lamps can be furnished for 12VDC, 24VDC or 24VAC.

For durability the lens is made of shatter resistant UV stabilized Lexan.

Mounting gasket and screws are included.

The socket has 1/4" push on (quick connect) terminals and ground wire.

The lens is mounted from inside the enclosure by #8 self-tapping screws which are inserted through three 3/16" dia. holes on a 3 5/8" dia. With the screws inside, this minimizes the possibility of theft by vandals.

Suggested hole size for the lamp is 1 15/16" (punch for 1 1/2" rigid).

Relamping is done from the bottom of the light by removing two thumb screws from the bracket holding the socket.

## **Horns Vibrating** 870 Series

The 870 Series panel mount vibrating horns are low current, high decibel for heavy-duty use. They come complete with gasket and are UL listed to NEMA 4X, NEMA 12 and NEMA 12K enclosure requirements.

Designed for semi-flush panel mounting, using supplied mounting template, or installation on a 4" square box. For NEMA 4X installation, mount to a NEMA 4X enclosure using the supplied gasket.

## **Features and Specifications**

- · Corrosion resistant finish
- · Volume adjustable
- · Completely assembled
- · Heavy duty die-cast housing
- NEMA Type 4X, NEMA Type 12 and Type 12K Rated
- Operating range: -20% to +10% of nominal voltage

## 870P AC

· Adjustable output: 88 to 113dB @ 1m (78 to 103dB @ 10ft.)

## 871P DC

· Adjustable output: 88 to 101dB @ 1m (78 to 91dB @ 10ft.)



Ordering	IIIIOI	IIIal	ЮП
Description			

Description	Cat. No.	Operating Voltage <sup>2</sup>	Current	VA	dB at 1m/10ft.1	DC Coil Res (Ohms)
	870P-E5	12V AC	1.25 A	15.0	113/103	1.5
D 114 1 10	870P-G5	24V AC	0.63 A	15.1	113/103	5.2
Panel Mount, AC	870P-N5	120V AC	0.13 A	15.6	113/103	150
	870P-R5	240V AC	0.07 A	16.8	113/103	580
Panel Mount. DC	871P-C1	6V DC	0.7 A	4.2	101/91	1.4
	871P-E1	12V DC	0.27 A	3.2	101/91	6
	871P-G1	24V DC	0.16 A	3.8	101/91	24
Farier Mount, DC	871P-J1	32V DC	0.11 A	3.52	101/91	40
	871P-P1	125V DC	0.025 A	3.1	101/91	600
	871P-S1	250V DC	0.014 A	3.5	101/91	2640

<sup>&</sup>lt;sup>1</sup>dB rating measured in anechoic chamber.

<sup>&</sup>lt;sup>2</sup>AC voltage frequency is 50/60 Hz.

Accessories	
Description	Cat. No.
Plastic Projector	872-PO

## **Signal Input Load Characteristics**

These devices may be operated by PLCs with output characteristics that match the input load requirements of the signal.

Cat. No.	Operating Voltage	Max. Off State Leakage Current (A)	Continuous On Current (A)	Surge (Inrush/Duration) Amps/Seconds
870P-N5	120V AC	0.025	0.120	1.02/.000026
871P-G1	24V DC	0.025	0.150	1.7/.000042









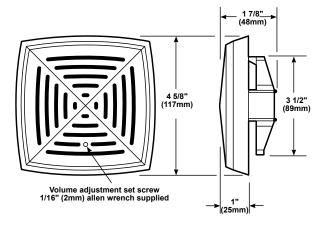




## Horns **Vibrating** 870 Series

Weig	hts and	l Dimensi	ions

Cat. No.         Approx. Net Weight (lb.)         Approx. Shipping Weight (lb.)           870P-E5         1.80         2.05           870P-G5         1.80         2.05           870P-N5         1.80         2.05           870P-R5         1.80         2.05           871P-C1         1.80         2.05           871P-E1         1.80         2.05           871P-G1         1.80         2.05           871P-J1         1.80         2.05           871P-P1         1.80         2.05           871P-S1         1.80         2.05           872-PO         0.34         0.58			
870P-G5     1.80     2.05       870P-N5     1.80     2.05       870P-R5     1.80     2.05       871P-C1     1.80     2.05       871P-E1     1.80     2.05       871P-G1     1.80     2.05       871P-J1     1.80     2.05       871P-P1     1.80     2.05       871P-S1     1.80     2.05	Cat. No.		
870P-N5       1.80       2.05         870P-R5       1.80       2.05         871P-C1       1.80       2.05         871P-E1       1.80       2.05         871P-G1       1.80       2.05         871P-J1       1.80       2.05         871P-P1       1.80       2.05         871P-S1       1.80       2.05	870P-E5	1.80	2.05
870P-R5     1.80     2.05       871P-C1     1.80     2.05       871P-E1     1.80     2.05       871P-G1     1.80     2.05       871P-J1     1.80     2.05       871P-P1     1.80     2.05       871P-S1     1.80     2.05	870P-G5	1.80	2.05
871P-C1     1.80     2.05       871P-E1     1.80     2.05       871P-G1     1.80     2.05       871P-J1     1.80     2.05       871P-P1     1.80     2.05       871P-S1     1.80     2.05	870P-N5	1.80	2.05
871P-E1     1.80     2.05       871P-G1     1.80     2.05       871P-J1     1.80     2.05       871P-P1     1.80     2.05       871P-S1     1.80     2.05	870P-R5	1.80	2.05
871P-G1     1.80     2.05       871P-J1     1.80     2.05       871P-P1     1.80     2.05       871P-S1     1.80     2.05	871P-C1	1.80	2.05
871P-J1     1.80     2.05       871P-P1     1.80     2.05       871P-S1     1.80     2.05	871P-E1	1.80	2.05
871P-P1     1.80     2.05       871P-S1     1.80     2.05	871P-G1	1.80	2.05
871P-S1 1.80 2.05	871P-J1	1.80	2.05
	871P-P1	1.80	2.05
<b>872-PO</b> 0.34 0.58	871P-S1	1.80	2.05
	872-PO	0.34	0.58



## Premium industrial grade straight blade receptacles

Project Name:	Prepared By:
Project Number:	Date:
Catalog Number:	Type:





AH5362



5461



## Description

2-Pole, 3-Wire Grounding 15A 125V; 15A 250V 20A 125V; 20A 250V NEMA 5-15, 5-20, 6-15, 6-20









6-20R

Design features

- Exclusive screw terminal guards provide fast, easy insulation from conductive surfaces (duplex only)
- Exclusive five-leaf solid brass line contacts provides consistent performance and superior plug retention (duplex only)
- Rigid, glass reinforced nylon base lends ultimate strength and superior high heat resistance in contact carrier (duplex only)
- 0.05" (1.27mm) solid brass mounting strap with non-riveted, integral one-piece grounding system for highest durability – assures superior ground integrity under heaviest use (duplex only)
- · High impact and chemical-resistant nylon face
- Tapered lead-in slots on back wiring holes assist in gathering stranded wires for faster, more secure connections
- All terminal and mounting screws triple combination (slot/Phillips/ Robertson) head for maximum installer flexibility
- Long, broad mounting strap helps reach large panel cutouts for code compliance
- Automatic grounding system eliminates need for bonding jumper in grounded metal enclosure, provides redundant measure of ground continuity where jumper used
- Screw-Catch feature on duplex devices speeds mounting of wallplates

Table 1. Premium Industrial Grade Duplex Receptacles, Back & Side Wire

Da	CK & Side Wile			
Catalog No.	Description	Amps	Volts	Color Suffix
☐ AH5262	NEMA 5-15R Premium Duplex Receptacle, Back & Side Wire	15	125	B, BK, GY, RD, V, W
□ 5261 <u></u>	NEMA 5-15R Premium Single Receptacle, Back & Side Wire	15	125	B, GY, RD, V, W
☐ AH5362	NEMA 5-20R Premium Duplex Receptacle, Back & Side Wire	20	125	B, BK, GY, LA, RD, V, W
<b>□</b> 5361	NEMA 5-20R Premium Single Receptacle, Back & Side Wire	20	125	B, BK, GY, RD, V, W
☐ AH5662	NEMA 6-15R Premium Duplex Receptacle, Back & Side Wire	15	250	B, V, W
□ 5661	NEMA 6-15R Premium Single Receptacle, Back & Side Wire	15	250	B, V, W
□ AH5462	NEMA 6-20R Premium Duplex Receptacle, Back & Side Wire	20	250	B, GY, V, W
□ 5461 <u></u>	NEMA 6-20R Premium Single Receptacle, Back & Side Wire	20	250	B, GY, V, W

Project Name:	Prepared By:
Project Number:	Date:
Catalog Number:	Type:

## **Applications**

Industrial grade receptacles are challenged on a daily basis to provide consistent, reliable means of connectivity in a wide variety of environments and need to stand up to a high degree of abuse. The applications very greatly by environment and provide challenges from impact and chemical resistance to environmental extremes such as heat and moisture. The breadth of Arrow Hart line of premium industrial grade straight blade receptacles provides solutions to all environments and applications. High impact and chemical resistant nylon faces withstand abuse to maintain structural integrity. The durable contact designs provide long life blade retention. When the job calls for receptacles that can stand up to the test of an industrial environment, turn to Arrow Hart's line of premium industrial grade receptacles.

Table 2. Specifications

Catalog No.	AH5262, AH5362, AH5462, AH5662 Series	5261, 5361, 5461, 5661 Series
Device Type	Industrial Grade Premium Duplex Receptacles	Industrial Grade Premium Single Receptacles
Wiring Type	Back & side wire	Back & side wire
Testing & Code Compliance	<ul> <li>cULus Listed to UL 498, file no. E15058</li> <li>UL verified to Federal Spec. WC-596G</li> <li>CSA certified to C22.2, no. 42</li> <li>NOM certified</li> </ul>	<ul> <li>Listed to UL 498, file no. E15058</li> <li>UL verified to Federal Spec. WC-596G</li> <li>CSA certified to C22.2, no. 42, file no. 6914 (6233-01)</li> <li>NOM certified</li> </ul>
<b>Environmental Specifications</b>	Flammability: Meets UL 94 requirements; V2 rated Temperature Rating: -20°C to 70°C (-4°F to 158°F)	Flammability: Meets UL 94 requirements; V2 rated Temperature Rating: -20°C to 60°C (-4°F to 140°F)
Electrical Specifications	Dielectric Voltage: Withstands 2000V per UL 498 Current Interrupting: Yes, at full-rated current Temperature Rise: Max. 30°C (86°F) after 250 cycles of overload @ 200% of rated current (DC)	Dielectric Voltage: Withstands 2000V per UL 498 Current Interrupting: Yes, at full-rated current Temperature Rise: Max. 30°C (86°F) after 250 cycles of overload @ 200% of rated current (DC)
Mechanical Specifications	Terminal Accommodation: #14 - 10 AWG Voltage Ratings: Permanently marked on device	Terminal Accommodation: #14 - 10 AWG Voltage Ratings: Permanently marked on device

## Table 3. Materials

Catalog No.	AH5262, AH5362, AH5462, AH5662 Series	5261, 5361, 5461, 5661 Series
Face	Nylon	Nylon
Base	Glass-filled nylon	Nylon
Strap	0.05" thick brass	0.05" thick steel, zinc plated
Auto Ground	Brass clip	Phosphor bronze staple
Line Contacts	0.031" thick 5-leaf 688 grade high-performance brass	0.037" thick 3-leaf brass
Ground Contact	Integral	Riveted
Terminal Screws	#8-32 brass, neutral screw nickel plated	#8-32 brass, neutral screw nickel plated
Ground Screw	#8-32 steel, zince plated (green)	#8-32 steel (green)
Screw Terminal Guards	PVC hinged doors	N/A

## **Table 4. Color Ordering Information**

For ordering devices, include Cat. No. followed by the color code: B (Brown), BK (Black), GY (Gray) LA (Light Almond), RD (Red), V (Ivory), W (White)





















# CLOUD-BASED MONITORING... CONVENIENT WEB-BASED MANAGEMENT

- Supervised internet connection assures
   the Sentinel is online and monitoring at
   all times
- Manage an unlimited number of devices from one account
- Monitor up to 12 external sensors or equipment status

- Receive an e-mail, phone call, or text message when an alarm is detected
- Real time status updates available
- Included battery backup
- Cellular coverage available from AT&T and Verizon



# ENVIRONMENTAL MONITORING FOR THE MOBILE AGE

The cloud-based Sentinel allows you to monitor remote facilities and environments and check critical conditions of your sensitive commodities with the same degree of certainty you've come to expect from Sensaphone.

It takes the burden out of managing your system by giving you access to your readings from anywhere using a simple, powerful web-based interface and mobile app. If there's a disruption, you'll be the first to know. Alerts can be sent straight to your mobile device—keeping you updated and giving you peace-of-mind wherever you are.

Mobile app available for Android and iphone







# IN THE CLOUD

The Sentinel system stores all sensor readings in the cloud, which provides unlimited information storage. Multiple devices can be managed from one account using intuitive web-based management tools. Enhanced data logging capabilities allow users to print, graph or export accurate historical records. The Sentinel system also can produce event reports and deliver them daily via e-mail, as well as generate an audit trail of all user data activities, edits or deletions. The device is Ethernet based, but it is available with a cellular option for locations that do not have Internet access.



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### **Distributed By:**

# SENSAPHONE® REMOTE MONITORING SOLUTIONS

Sensaphone Sentinel with Cellular Modem
Technical Specifications



### **ALARM NOTIFICATION METHODS:**

E-Mail, Text Messages, Voice Phone Calls

Programmable alarm escalation levels

Comprehensive scheduling per input, profile, and alarm destination

Unlimited number of User Profiles

Multiple contact types per user

### **INPUTS:**

12 Universal Inputs

- Normally Open / Normally Closed Dry Contact
- 2.8K / 10K Thermistor
- 4-20mA Current Loop
- Pulse Count
- 12 Bit Resolution

### **TEMPERATURE SENSING RANGE:**

-109° to 168°F | -85° to 76°C

### **RELAY OUTPUT:**

Programmable. Rated for 1A 30VAC/ 1A 30VDC

### **DATA LOGGING:**

Unlimited samples securely stored on the Sentinel servers

Programmable sampling Interval - 1 min to 24 hrs User programmable channel selection

### **COMMUNICATION PORTS:**

Ethernet 10/100Base-T

### **BATTERY BACKUP:**

4.8V 2000mAHr NiMh Battery pack (included) Provides 8 hours of backup

### **LOCAL INDICATORS:**

12 Alarm Status LEDs

- Power LED Online LED
- Standby LED Ethernet link and Activity LEDs

### **POWER REQUIREMENTS:**

Comes with 12VDC plug-in power supply. (International power options available).

### **ENVIRONMENTAL:**

**Operating Humidity:** 

0-90% RH, non-condensing

**Operating Temperature:** 

 $32^{\rm o}$  to  $122^{\rm o}$ F  $\mid$  0° to  $50^{\rm o}$ C

### **PHYSICAL:**

**Dimensions:** 

12.5 x 12.2 x 7.0" | 318 x 310 x 178mm

**Weight:** 10.5 lbs. | 4.7 kg

### STANDARDS:

FCC Part 15 - Class A Compliant

### **ENCLOSURE:**

NEMA-4 rated plastic housing when ordered with the cellular modem.

# Product data sheet Characteristics

# ZB4BV043 PILOT LIGHT HEAD RED



### Main

Range of product	Harmony XB4
Product or component type	Head for pilot light
Product compatibility	Integral LED
Device short name	ZB4
Bezel material	Chromium plated metal
Mounting diameter	22 mm
Sale per indivisible quantity	1
Shape of signaling unit head	Round
Cap/Operator or lens colour	Red
Operator additional information	With plain lens

### Complementary

CAD overall width	29 mm			
CAD overall height	29 mm			
CAD overall depth	30 mm			
Product weight	0.026 kg			
Resistance to high pressure washer	7000000 Pa at 55 °C, distance: 0.1 m			
Electrical composition code	P2 in front mounting with integral LED and transformer P1 in front mounting with integral LED			

### Environment

Environment	
Protective treatment	TH
Ambient air temperature for storage	-4070 °C
Ambient air temperature for operation	-2570 °C
Class of protection against electric shock	Class I conforming to IEC 60536
IP degree of protection	IP66 conforming to IEC 60529
NEMA degree of protection	NEMA 4X NEMA 13
IK degree of protection	IK05 conforming to IEC 50102
Standards	EN/IEC 60947-1 EN/IEC 60947-5-1 EN/IEC 60947-5-4 EN/IEC 60947-5-5 JIS C 4520 UL 508 CSA C22.2 No 14
Vibration resistance	5 gn (f = 2500 Hz) conforming to IEC 60068-2-6
Shock resistance	50 gn (duration = 11 ms) for half sine wave acceleration conforming to IEC 60068-2-27 30 gn (duration = 18 ms) for half sine wave acceleration conforming to IEC 60068-2-27

# ZBVG1

# white light block for head Ø22 integral LED 110...120V screw clamp terminals



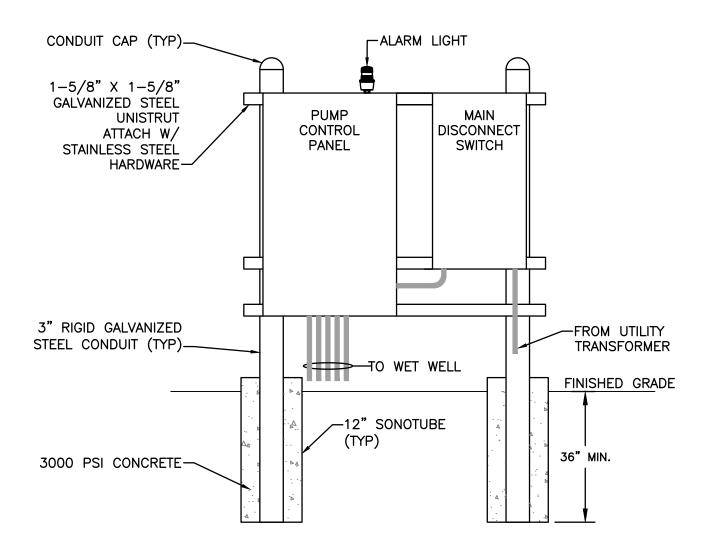
Main	
Range of product	Harmony XB4 Harmony XB5
Product or component type	Light block
Device short name	ZBV
Sale per indivisible quantity	5
Connections - terminals	Screw clamp terminals: <= 2 x 1.5 mm <sup>2</sup> with cable end conforming to EN 60947-1 Screw clamp terminals: >= 1 x 0.22 mm <sup>2</sup> without cable end conforming to EN 60947-1
Signalling type	Steady
Light source	Protected LED
Bulb base	Integral LED
Light block supply	Direct
Light source colour	White
[Us] rated supply voltage	110120 V AC, 50/60 Hz

### Complementary

Product weight	0.017 kg			
Tightening torque	0.81.2 N.m conforming to EN 60947-1			
Shape of screw head	Cross head compatible with Philips no 1 screwdriver Cross head compatible with pozidriv No 1 screwdriver Slotted head compatible with flat Ø 4 mm screwdriver Slotted head compatible with flat Ø 5.5 mm screwdriver			
Current consumption	14 mA			
Service life	100000 h at rated voltage and 25 °C			
Surge withstand	1 kV conforming to IEC 61000-4-5			
Mounting of block	Front mounting			

#### Environment

Protective treatment	TH
Ambient air temperature for storage	-4070 °C
Ambient air temperature for operation	-2570 °C
IP degree of protection	IP20 conforming to IEC 60529
Standards	CSA C22-2 No 14 EN/IEC 60947-1 EN/IEC 60947-5-1 EN/IEC 60947-5-4 EN/IEC 60947-5-5 JIS C 4520 UL 508
Product certifications	CSA UL listed
Resistance to fast transients	2 kV conforming to IEC 61000-4-4
Resistance to electromagnetic fields	10 V/m conforming to IEC 61000-4-3
Resistance to electrostatic discharge	6 kV on contact (on metal parts) conforming to IEC 61000-4-2 8 kV in free air (in insulating parts) conforming to IEC 61000-4-2
Electromagnetic emission	Class B conforming to IEC 55011
RoHS EUR status	Compliant
RoHS EUR conformity date	0720



# **ELECTRICAL EQUIPMENT RACK DETAIL**

SCALE: N.T.S.

# SITE PLAN

# **FINAL PLAN AND PROFILE SHEETS**

### **GENERAL NOTES**

- 1. CONFIGURATIONS AND DIMENSIONS SHOWN ARE BASED ON THE EQUIPMENTS SPECIFIED. THE CONTRACTOR SHALL VERIFY THE LAYOUT AND ALL DIMENSIONS PRIOR TO FABRICATION.
- 2. ALL COUPLINGS SHALL BE EPOXY COATED STEEL AND SHALL BE DRESSER, SMITH-BLAIR, OR EQUAL. COUPLINGS SHALL BE RESTRAINED WITH A THRUST HARNESS DESIGNED IN ACCORDANCE WITH AWWA
- 3. ALL PIPING WITHIN WET WELL AND VALVE VAULT SHALL BE SPECIAL THICKNESS CLASS 53 EPOXY LINED (PROTECTO 401) DUCTILE IRON PIPE.
- 4. ALL BURIED PIPE SHALL BE POLYWRAPPED DUCTILE IRON PRESSURE CLASS 350 PUSH JOINT PIPE WITH JOINT RESTRAINT GASKETS.
- 5. ALL METALS WITHIN THE WET WELL, INCLUDING FLANGE BOLTS, SHALL BE STAINLESS STEEL, UNLESS OTHERWISE INDICATED.
- 6. THE EDGE OF EXPOSED CONCRETE SLABS SHALL RECEIVE A 3/4" CHAMFER.
- 7. REINFORCED STEEL SHALL BE GRADE 60.

DAMAGE INCURRED DURING CONSTRUCTION.

- 8. ALL VENTS SHALL HAVE SCREENS INSTALLED OVER OPENINGS AND SHALL BE 304 STAINLESS STEEL 16 MESH. OPENINGS SHALL BE A MINIMUM OF 12" ABOVE THE SLAB.
- 9. THE HIGH LEVEL ALARM SHALL ACTIVATE A FLASHING RED LIGHT TO BE MOUNTED ON THE ROOF OF THE ELECTRICAL SHELTER. THE CONTRACTOR SHALL INSTALL METAL INFORMATION SIGN AT OR NEAR THE LIFT STATION ENTRY GATE, VISIBLE TO THE PUBLIC.
- 10. ALL PENETRATIONS SHALL BE SEALED WATER AND GAS TIGHT PER APPROVED METHODS.
- 11. FORCEMAIN TO CONTINUE AT THE SHOWN FLOWLINE UNTIL IT CONNECTS WITH THE FORCEMAIN SHOWN ON SHEET 44.
- 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL OF THE NECESSARY WASTEWATER CONNECTIONS TO THE SITE. CONTRACTOR SHALL NOTIFY THE UTILITY AUTHORITIES INSPECTORS 72 HOURS PRIOR TO CONNECTING TO ANY EXISTING LINE. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR
- 13. CONTRACTOR SHALL USE KENNEDY VALVE CO. KS-RW GATYE VALVES OR APPROVED EQUAL IN ACCORDANCE WITH AWWA C517.
- 14. CONTRACTOR SHALL USE VAL-MATIC SWING-FLEX CHECK VALVE OR APPROVED EQUAL IN ACCORDANCE WITH AWWA C508. CHECK VALVES SHOULD BE SUITABLE FOR DIRECT BURY.

COMPLETE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY REPAIRS TO EXISTING UTILITIES DUE TO

- 15. A CHECK VALVE MUST BE A SWING TYPE VALVE WITH AN EXTERNAL LEVER OR EXTERNAL POSITION INDICATOR TO SHOW ITS OPEN AND CLOSED POSITIONS.
- 16. AN ISOLATION VALVE MUST INCLUDE AN EXTERNAL POSITION INDICATOR TO SHOW ITS OPEN AND CLOSED POSITIONS, UNLESS A FULL-CLOSING VALVE IS A RISING-STEM GATE VALVE.
- 17. CONTRACTOR SHALL USE ARI D-020 COMBINATION AIR VALVE OR APPROVED EQUAL.
- 18. CONFIGURATIONS AND DIMENSIONS SHOWN ARE BASED ON THE EQUIPMENT SPECIFIED. THE CONTRACTOR SHALL VERIFY THE LAYOUT AND ALL DIMENSIONS PRIOR TO CONSTRUCTION.
- 19. CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING THE STRUCTURAL DESIGN FOR BRACING THE J-VENT SYSTEM.
- 20. INSTALL ISOLATION KITS BETWEEN DISSIMILAR METAL PIPING.
- 21. CONTRACTOR SHALL FOLLOW THE INSTALLATION INSTRUCTIONS PROVIDED BY THE SUPPLIER/MANUFACTURER.
- 22. UPON RECEIPT OF THE LIFT STATION SHIPMENT, INSPECT THE ENTIRE SHIPMENT FOR DAMAGE BEFORE THE LIFT STATION IS TAKEN OFF THE TRUCK. IF THERE IS DAMAGE, NOTE AS SPECIFICALLY AS POSSIBLE INCLUDING CLEAR PHOTOS OF DAMAGE ON THE BILL OF LADING AS TO ANY DAMAGE, THEN OFFLOAD. CONTACT THE SHIPPER AT ONCE AND HAVE THE BILL OF LADING WITH YOU.
- 23. CHECK VALVE, COUPLING, GATE VALVE AND AIR RELEASE VALVES (BALL VALVES MAY BE USED DEPENDING ON TYPE OF PUMPS).
- 24. PUMPS TO BE SELECTED BASED ON APPLICATION, FLOW, AND HEAD CONDITIONS OF FORCE MAIN.
- 25. SELECT BACKFILL MATERIAL FREE OF VOIDS, SHARP OBJECTS, OR OTHER DEBRIS.

# PIPE AND FITTINGS

- 1.1. 4" HDPE DR9 FORCE MAIN WITH PRESSURE RATING OF 200 PSI SHALL BE USED.
- 2. LIFT STATION PIPING
- DIP PIPE SHALL BE IN ACCORDANCE WITH AWWA C111 AND AWWA C150 2.2. FITTINGS AND PIPE SHALL BE FLANGED.
- FITTINGS IN ACCORDANCE WITH AWWA C153 AND AWWA C110. FLANGES: AWWA/ANSI C115/A21.15, ASME B16.1, CLASS 125
- 2.2.3. BOLTS AND NUTS SHALL BE 316 STAINLESS.
- 2.3. PIPE AND FITTINGS SHALL BE LINED WITH PROTECTO 401. 2.4. PROVIDE POLYETHYLENE ENCASEMENT IN ACCORDANCE WITH AWWA FOR BURIED DIP.

# SHOP DRAWINGS, PRODUCT DATA, AND SAMPLES

- 1. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND PRODUCT DATA FOR LIFT STATION EQUIPMENT, PIPING AND APPURTENANCES. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE OWNER FOR REVIEW PRIOR TO INSTALLATION, ALLOWING SUFFICIENT TIME FOR THE OWNER'S REVIEW AND RESPONSE.
- 2. CONTRACTOR SHALL SUBMIT DIMENSIONAL LAYOUT DRAWINGS AND PRODUCT DATA, CERTIFIED CORRECT FOR CONSTRUCTION, FOR REVIEW BY THE OWNER.
- 3. THE CONTRACTOR WILL MAKE SPECIFIC MENTION OF THOSE ITEMS THAT VARY FROM THE REQUIREMENTS OF THE CONTRACT DOCUMENTS IN THE LETTER OF TRANSMITTAL.
- 4. THE CONTRACTOR WILL ASSIGN A SEQUENTIAL NUMBER TO EACH SUBMITTAL (1, 2, 3, ETC.). RE-SUBMITTALS WILL BE IDENTIFIED WITH THEIR ORIGINAL NUMBER FOLLOWED BY A SEQUENTIAL LETTER (A, B, C, ETC.). FOR EXAMPLE, SUBMITTAL 12-C IS THE THIRD RE-SUBMITTAL OF THE OF THE TWELFTH ITEM
- 5. THE CONTRACTOR WILL NOT DELIVER TO THE SITE, STORE, OR INCORPORATE INTO THE WORK, ANY MATERIALS OR EQUIPMENT FOR WHICH APPROVED SUBMITTALS HAVE NOT BEEN OBTAINED.
- 6. OWNER'S REVIEW, APPROVAL, OR OTHER APPROPRIATE ACTION REGARDING CONTRACTOR'S SUBMISSIONS WILL BE ONLY TO CHECK CONFORMITY WITH THE DESIGN CONCEPT OF THE PROJECT AND FOR COMPLIANCE WITH THE INFORMATION CONTAINED IN THE CONTRACT DOCUMENTS AND SHALL NOT EXTEND TO MEANS, METHODS, TECHNIQUES, DEQUENCES OR PROCEDURES OF CONSTRUCTION (EXCEPT WHERE A SPECIFIC MEANS, METHOD, TECHNIQUE, SEQUENCE OR PROCEDURE OF CONSTRUCTION IS INDICATED IN OR REQUIRED BY THE CONTRACT DOCUMENTS) OR TO SAFETY PRECAUTIONS OR PROGRAMS INCIDENT THERETO. THE REVIEW AND APPROVAL OF A SEPARATE COMPONENT ITEM WILL NOT INDICATE APPROVAL OF THE ASSEMBLY INTO WHICH THE ITEM IS FUNCTIONALLY INTEGRATED. CONTRACTOR SHALL MAKE CORRECTIONS REQUIRED BY OWNER. AND SHALL RETURN THE REQUIRED NUMBER OF CORRECTED COPIES OF SHOP DRAWINGS TO THE OWNER. CONTRACTOR MAY BE REQUIRED TO RESUBMIT AS REQUIRED REVISED SHOP DRAWINGS OR SAMPLES FOR FURTHER REVIEW AND APPROVAL. CONTRACTOR SHALL DIRECT SPECIFIC ATTENTION IN WRITING TO ANY NEW REVISIONS NOT SPECIFIED BY CONTRACTOR ON PREVIEW CONTRACTOR SUBMISSIONS.

# LIFT STATION AND SUBMERSIBLE SEWAGE PUMPS

- 1. CONTRACTOR SHALL FURNISH ALL LABOR, MATERIALS, ELECTRICAL EQUIPMENT AND INCIDENTALS REQUIRED TO PROVIDE A LIFT STATION AS PROVIDED BY MANSFIELD PUMP COMPANY. (JAMES MANSFIELD, 512-745-7647) THE CONTRACTOR'S RESPONSIBILITIES ARE AS FOLLOWS: RECEIVING THE LIFT STATION AND OFF LOADING, EXCAVATION AND SETTING, ALL PLUMBING CONNECTIONS, ALL ELECTRICAL CONDUITS FROM THE WET WELL TO CONTROL PANEL, PROVIDE PROPER BACKFILL AND COMPACTION PROCEDURES. THE CONTRACTOR WILL BE REQUIRED TO LOWER THE PUMPS INTO PLACE AND CHECK FOR PROPER ROTATION. A STARTUP PROCEDURE FORM WILL BE PROVIDED AND IT MUST BE FILLED OUT AND RETURNED TO THE LIFT STATION SUPPLIER PRIOR TO THE OWNER'S POSSESSION DATE.
- 2. THE PUMP SHALL HAVE A FLANGED DISCHARGE ADAPTABLE TO 3" VERTICAL DISCHARGE AND BE CAPABLE OF HANDLING SANITARY SEWAGE AND ENABLING IT TO BE PUMPED OVER LONG DISTANCES OR HIGH VERTICAL LIFTS IN PIPELINES AS SMALL AS 3.00" IN DIAMETER.
- 3. THE CENTRIFUGAL PUMP SHALL BE EQUAL TO THE SERIES EBG-33. A PUMP SUBMERSIBLE TYPE AS MANUFACTURED BY GRUNDFOS WITH 4" IMPELLER OR APPROVED EQUAL. THE PUMP SHALL BE EXPLOSION-PROOF. THE CASTINGS SHALL BE CONSTRUCTED OF CAST IRON.
- 4. SUBMERSIBLE SEWAGE PUMP MATERIALS:
- 4.1. PUMP CASE: CAST IRON, ASTM A48, CLASS 30 4.2. MOTOR HOUSING: CAST IRON, ASTM A48, CLASS 30
- 4.3. IMPELLER: CAST BRASS
- 4.4. INTERMEDIATE HOUSING (BACKPLATE): CAST IRON, ASTM A48, CLASS 35B 4.5. DISCHARGE BASE ELBOW: CAST IRON, ASTM A48, CLASS 35B
- 4.6. PUMP/MOTOR SHAFT: ENTIRE SHAFT IS TO BE ASTM A276 TYPE 420 STAINLESS STEEL
- O-RINGS: NITRILE RUBBER (NBR)
- 4.8. FASTENERS (INCLUDING IMPELLER FASTENER): 304 STAINLESS STEEL
- 4.9. LOWER SEAL FACES: SILICON CARBIDE/SILICON CARBIDE 4.10. UPPER SEAL FACES: SILICON CARBIDE STATIONARY/CARBON ROTATING 4.11. GUIDE RAILS/CABLES AND MOUNTING BRACKETS: STAINLESS STEEL, ASTM A276 TYPE 316 (CABLES
- SHALL BE NYLON COATED)
- 4.12. LIFTING CHAIN OR CABLE: STAINLESS STEEL, ASTM A276 TYPE 316 4.13. OIL-ALL USES (SEAL LUBRICATION, ETC): ECOLOGICALLY SAFE, PARIFIN OR MINERAL BASE 4.14. POWER/CONTROL CABLE JACKET: CHLOROPRENE WITH NON-WICKING FILLERS
- 5. THE MOTOR HOUSING SHALL BE FINNED AND AIR-FILLED TO DISSIPATE HEAT AND ENABLE THE UNIT TO OPERATE UNSUBMERGED WITHOUT DAMAGE TO THE MOTOR. ALL EXTERNAL-MATING PARTS SHALL BE MACHINED AND SEALED WITH A bUNA-n SQUARE RING. ALL FASTENERS EXPOSED TO THE LIQUID SHALL BE 300 SERIES STAINLESS STEEL. THE MOTOR SHALL BE PROTECTED ON THE TOP SIDE WITH AN ATTACHED SEALED JUNCTION BOX CHAMBER WHICH IN THE EVENT OF CORD DAMAGE WILL PREVENT MOISTURE WICKING INTO THE MOTOR HOUSING. THE MOTOR SHALL BE PROTECTED ON THE LOWER SIDE WITH A TANDEM MECHANICAL SEAL ARRANGEMENT WITH EACH SEAL HAVING A SEPARATE SPRING ASSEMBLY THE UPPER AND LOWER BALL BEARINGS SHALL BE CAPABLE OF HANDLING ALL THRUST LOADS. THE PUMP HOUSING HAVING A SEPARATE SPRING ASSEMBLY. THE UPPER AND LOWER BALL BEARINGS SHALL BE CAPABLE OF HANDLING ALL THRUST LOADS. THE PUMP HOUSING SHALL BE OF THE CONCENTRIC DESIGN THEREBY EQUALIZING THE PRESSURE FORCES INSIDE THE HOUSING, WHICH WILL EXTEND THE SERVICE LIFE OF THE SEALS AND BEARINGS. THE TOP CAP SHALL HAVE CAST IN LIFTING LUGS. THE MOTOR SHALL BE PROTECTED BY A MOISTURE DETECTION SYSTEM, WHICH WILL ACTIVATE AN ALARM CIRCUIT IF LIQUID IS EVER PRESENT IN THE UPPER JUNCTION CHAMBER OR LOWER SEAL CAVITY. THERMAL SENSORS LOCATED IN THE OIL-FILLED MOTOR HOUSING SHALL PROVIDE TEMPERATURE PROTECTION. PROVIDE THE AUTOMATIC REVERSING FEATURE, AVAILABLE FOR 3 PHASE MODELS, THAT ENABLE THE PUMP TO ROTATE IN BOTH CW AND CCW DIRECTION.
- 6. THE PUMP SHALL BE SUPPLIED WITH 49.2' MULTI-CONDUCTOR POWER CORD. IT SHALL BE SO TYPE CORD CAPABLE OF CONTINUED EXPOSURE TO THE PUMPED LIQUID. POWER CORD SHALL BE SIZED FOR THE RATED FULL LOAD AMP RATING OF THE PUMP IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE.
- 7. THE MOTOR SHALL BE FM LISTED FOR CLASS 1 DIVISION 1 GROUPS C&D, HAVE A CLASS F OR BETTER RATED DESIGN RATED FOR CONTINUOUS DUTY. THERMAL SENSORS LOCATED IN THE MOTOR HOUSING SHALL PROVIDE TEMPERATURE PROTECTION.
- 8. FURNISH UPPER AND LOWER BALL BEARINGS TO PROVIDE B10 LIFE OF, A MINIMUM 100,000 HOURS AT ALL ANTICIPATED AXIAL AND RADIAL LOADINGS.
- 9. PUMP SHALL HAVE A DUAL MECHANICAL SEAL CONFIGURATION WITH THE SEALS MOUNTED IN TANDEM. EACH SEAL ASSEMBLY WITH ITS OWN SPRING. PROVIDE SEALS THAT DO NOT REQUIRE ROUTINE
- MAINTENANCE OR ADJUSTMENT, BUT CAPABLE OF BEING REPLACED. 10. THE IMPELLER SHALL BE A NON-CLOG TYPE IMPELLER, CAPABLE OF PASSING 3" SPHERICAL SOLID.

STATICALLY AND DYNAMICALLY BALANCED IMPELLERS ARE REQUIRED.

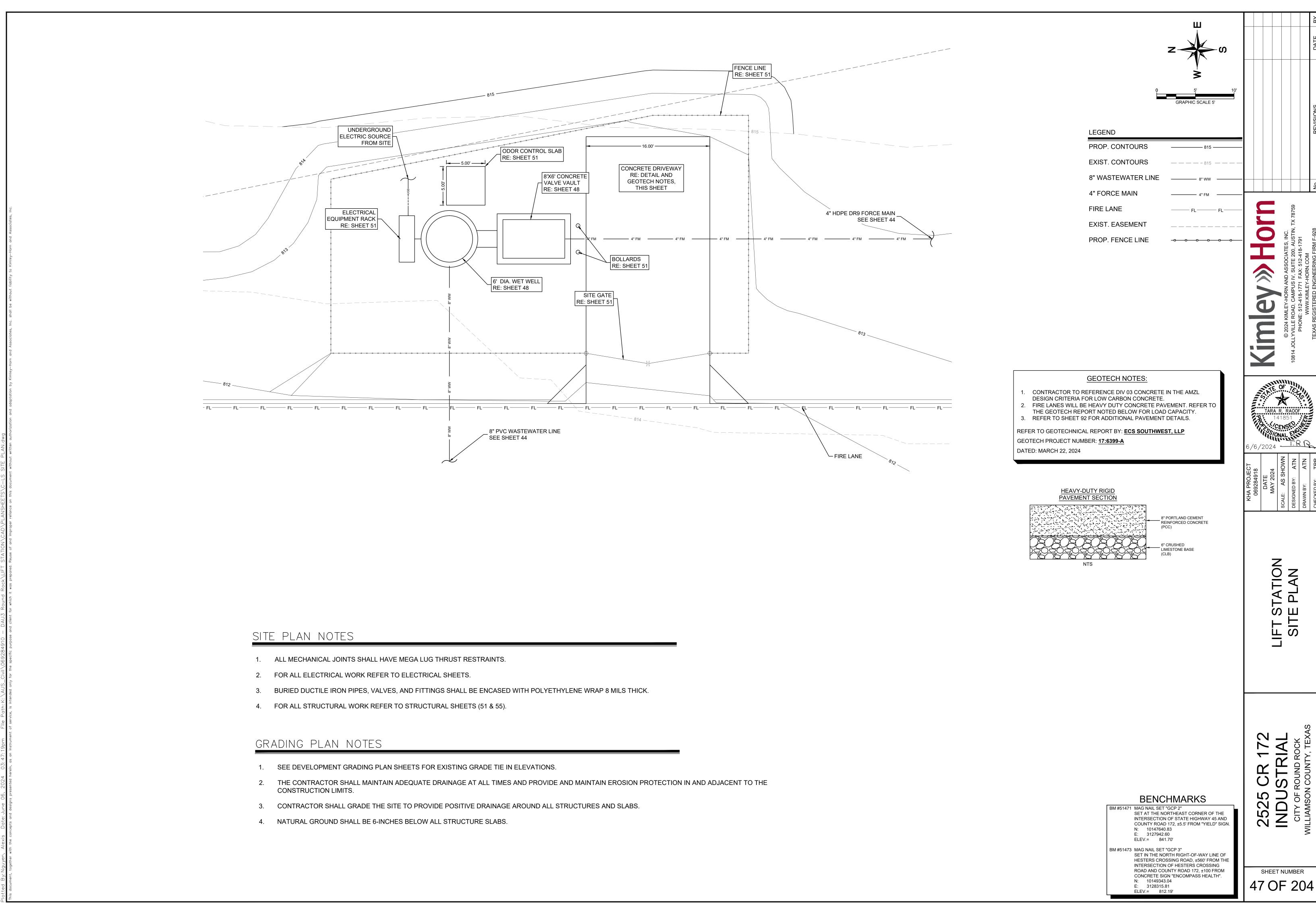
- 11. COMPONENTS REQUIRED FOR THE REPAIR OF THE PUMP SHALL BE READILY AVAILABLE WITHIN 24 HOURS. COMPONENTS SUCH AS MECHANICAL SEALS AND BEARINGS SHALL NOT BE OF A PROPRIETARY DESIGN AND BE AVAILABLE FROM LOCAL INDUSTRIAL SUPPLY HOUSES. SPECIAL TOOLS SHALL NOT BE REQUIRED TO SERVICE THE PUMP. A NETWORK OF SERVICE STATIONS SHALL BE AVAILABLE NATIONWIDE IN THOSE CASES WHERE SERVICE REQUIREMENTS ARE BEYOND THE COPE OF IN-HOUSE SERVICE MECHANICS.
- 12. THE TWO YEAR PRO-RATED WARRANTY FOR PERMANENT WASTEWATER LIFT STATIONS SHALL BE IN WRITING IN A PUBLISHED SERVICE BULLETIN. LABOR CHARGES FROM AN AUTHORIZED SERVICE STATION FOR REPAIRS WILL BE INCLUDED IN THIS WARRANTY AGREEMENT DURING THE FIRST 12 MONTHS OF OPERATION.

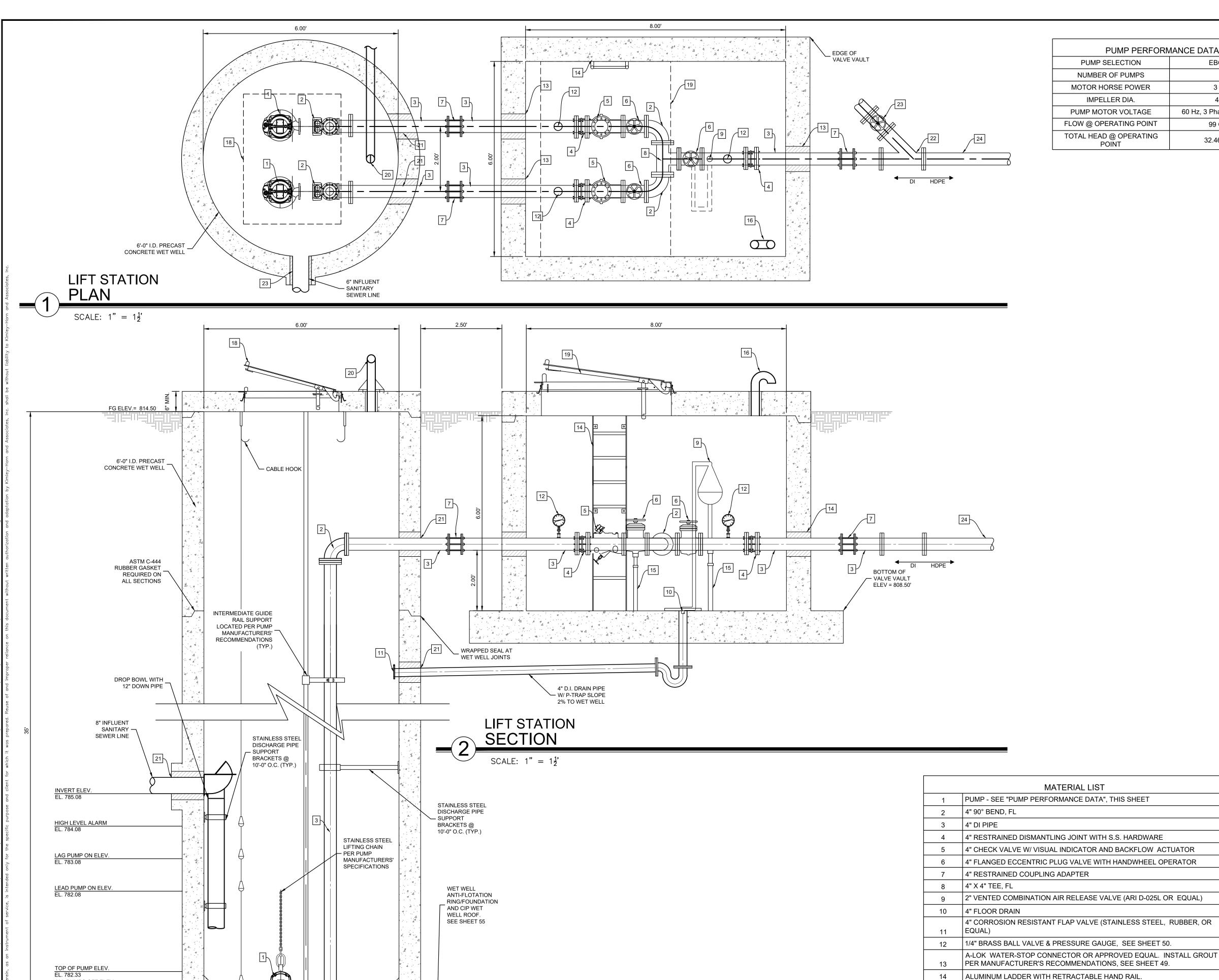




NDO

SHEET NUMBER 46 OF 204





ADJUSTED WET WELL ELEV.

PUMP PERFORMANCE DATA							
PUMP SELECTION	EBG-33						
NUMBER OF PUMPS	2						
MOTOR HORSE POWER	3 HP						
IMPELLER DIA.	4 IN						
PUMP MOTOR VOLTAGE	60 Hz, 3 Phase, 460 Volts						
FLOW @ OPERATING POINT	99 GPM						
TOTAL HEAD @ OPERATING	32.46' TDH						

PIPE SUPPORT, SEE SHEET 49.

ODOR CONTROL AERATOR, SEE SHEET 51

WET WELL PENETRATION, SEE SHEET 49

4" ECCENTRIC PLUG VALVE, MJ

SCREENS

22 4" X 4" WYE, MJ

24 4" HDPE DR9 PIPE

4" STAINLESS STEEL GOOSE NECK VENT WITH 304 STAINLESS STEEL

4'X6' VALVE VAULT ALUMINUM ACCESS COVER WITH THREE DOORS

3'X4' WET WELL ALUMINUM ACCESS COVER WITH THREE DOORS

INCLUDING SAFE HATCH, H-20 LOAD RATING (FLYGT OR EJIW)

INCLUDING SAFE HATCH, H-20 LOAD RATING (FLYGT OR EJIW)

LIFT STATION INFORMATION	
SERVICE AREA TOTAL (ACRES)	69.
SERVICE AREA SIZE (LUEs)	
PEAK DRY WEATHER FLOW (GPM)	
PEAK WET WEATHER FLOW (DESIGN	
FLOW) (GPM)	84.229
WET WELL DIAMETER (FT)	
ACTIVE VOLUME (GAL)	516.
MINIMUM PUMP CYCLE TIME (MIN)	46.
FORCE MAIN SIZE (IN)	
FORCE MAIN LENGTH (LF)	·
FLOW VELOCITY (FPS)	3.

# KEYNOTES:

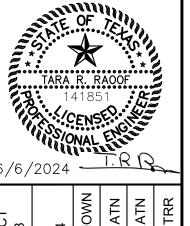
- 1. PUMP SUPPLIER SHALL PROVIDE DIMENSIONS OF THE GUIDE RAILS TO ENSURE COMPATIBILITY WITH SUPPLIED EQUIPMENT. THE PUMP SHALL BE EASILY REMOVED FOR INSPECTION OR SERVICE. PERSONNEL SHALL HAVE NO REASON TO ENTER THE WET WELL. GUIDE RAILS SHALL BE SUPPORTED EVERY 10 FEET WITH STAINLESS STEEL SUPPORTS.
- THE GUIDE BRACKETS SHALL BE CONSTURCTED OF 316 STAINLESS STEEL. GUIDE BRACKETS FOR EACH PUMP MUST BE SUPPLIED BY THE PUMP MANUFACTURER TO ENSURE COMPATIBILITY WITH SUPPLIED EQUIPMENT.
- 3. EACH PUMPING UNIT SHALL BE PROVIDED WITH A STAINLESS STEEL LIFTING CHAIN OR CABLE. LIFTING CHAIN SHALL EXTEND AT LEAST 3-4 FEET ABOVE WET WELL.
- 4. ALL HARDWARE IN THE WET WELL SHALL BE 316 STAINLESS STEEL.
- 5. PUMP DISCHARGE LINES SHALL HAVE 1/4 INCH TAPS WITH STAINLESS STEEL OR BRONZE BALL VALVES.
- 6. ALL DISCHARGE LINES SHALL HAVE ADEQUATE THRUST SUPPORT MEMBERS AT EACH FITTING. WHERE POSSIBLE, LONG RADIUS 90 DEGREE BENDS SHALL BE
- 7. THE DISCHARGE LINE FROM EACH PUMP SHALL BE FITTED WITH A CHECK VALVE AND A ECCENTRIC PLUG VALVE, WITH THE CHECK VALVE ON THE PUMP SIDE OF THE ECCENTRIC PLUG VALVE. WHEN NECESSARY, AIR RELEASE VALVE(S) SHALL BE INSTALLED DOWNSTREAM OF THE ECCENTRIC PLUG VALVES.
- 8. THE VALVE VAULT SHALL BE SIZED LARGE ENOUGH TO PROVIDE AT LEAST 1 FOOT OF CLEARANCE AROUND ALL VALVES AND ALL FLANGES. THE LID OF THE VAULT SHALL BE A MINIMUM OF 42 INCHES BY 42 INCHES AND SHALL BE ABLE TO BE PADLOCKED. THE VAULT SHALL BE INTERNALLY COATED WITH SPL WW-511 COATING, AND EXTERNALLY COATED WITH A COAL TAR EPOXY.
- 9. THE VALVE VAULT SHALL HAVE A DRAIN TO THE WET WELL. THE DRAIN SHALL HAVE A 4 INCH MINIMUM DIAMETER AND BE FITTED WITH A FLAP VALVE OR BACK-FLOW PREVENTER AND A TRAP TO PREVENT GASES OR WATER FROM ENTERING THE VALVE VAULT. THE OPENING TO THE DRAIN SHALL BE COVERED WITH A STAINLESS STEEL SCREEN.
- 10. ALL VENTS SHALL BE COATED INSIDE AND OUT PER CITY OF ROUND ROCK STANDARD SPECIFICATIONS. THE WET WELL VENT SHALL BE A TEE FITTED WITH STAINLESS STEEL SCREENS 10 FEET ABOVE WET WELL COVER.

# **GENERAL NOTES**

- 1. THE WORK PERFORMED UNDER THIS CONTRACT SHALL COMPLY WITH THE CITY OF ROUND ROCK SUBMERSIBLE LIFT STATION SPECIFICATIONS.
- 2. CONFIGURATIONS AND DIMENSIONS SHOWN ARE BASED ON THE EQUIPMENTS SPECIFIED. THE CONTRACTOR SHALL VERIFY THE LAYOUT AND ALL DIMENSIONS PRIOR TO FABRICATION.
- 3. ALL COUPLINGS SHALL BE EPOXY COATED STEEL AND SHALL BE DRESSER, SMITH-BLAIR, OR EQUAL. COUPLINGS SHALL BE RESTRAINED WITH A THRUST HARNESS DESIGNED IN ACCORDANCE WITH AWWA M-11.
- 4. CONTRACTOR SHALL COORDINATE VALVE VAULT ACCESS HATCH LOCATION WITH LADDER MANUFACTURER PRIOR TO FABRICATION.
- 5. CONTRACTOR SHALL VERIFY WET WELL ACCESS HATCH LOCATION AND DIMENSION WITH PUMP MANUFACTURER PRIOR TO FABRICATION.
- 6. PROVIDE 316 STAINLESS STEEL ANCHOR BOLTS FOR PUMP BASE MOUNTING TO SLAB.
- 7. INSTALL ISOLATION KITS BETWEEN DISSIMILAR METAL PIPING.
- 8. ALL PIPING WITHIN WET WELL AND VALVE VAULT SHALL BE SPECIAL THICKNESS CLASS 53 EPOXY LINED (PROTECTO 401) DUCTILE IRON PIPE.
- 9. ALL BURIED PIPE SHALL BE POLYWRAPPED DUCTILE IRON PRESSURE CLASS 350 PUSH JOINT PIPE WITH JOINT RESTRAINT GASKETS.
- 10. ALL METALS WITHIN THE WET WELL, INCLUDING FLANGE BOLTS, SHALL BE STAINLESS STEEL, UNLESS OTHERWISE INDICATED.
- 11. CONCRETE FOR FOUNDATION SHALL BE CLASS S. CONCRETE SHALL HAVE A MIN
- COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS.
- 12. THE EDGE OF EXPOSED CONCRETE SLABS SHALL RECEIVE A 3/4" CHAMFER.
- 13. REINFORCED STEEL SHALL BE GRADE 60.
- 14. ALL VENTS SHALL HAVE SCREENS INSTALLED OVER OPENINGS AND SHALL BE 304 STAINLESS STEEL - 16 MESH. OPENINGS SHALL BE A MINIMUM OF 12" ABOVE THE SLAB
- 15. THE HIGH LEVEL ALARM SHALL ACTIVATE A FLASHING RED LIGHT TO BE MOUNTED ON THE ROOF OF THE ELECTRICAL SHELTER. THE CONTRACTOR SHALL INSTALL METAL INFORMATION SIGN AT OR NEAR THE LIFT STATION ENTRY GATE, VISIBLE TO THE

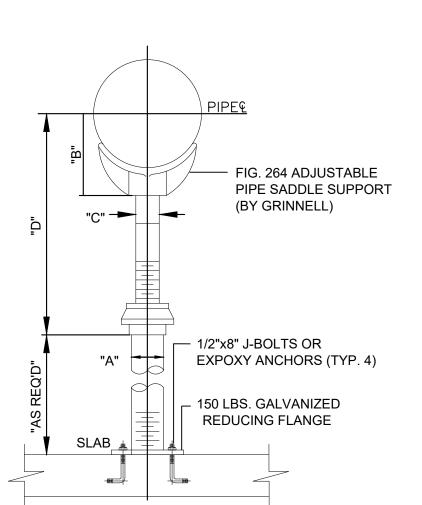
(MIN.) COAT OF RAVEN 400 SERIES HIGH BUILD EPOXY LINER OR APPROVED EQUAL.

- 16. ALL PENETRATIONS SHALL BE SEALED WATER AND GAS TIGHT PER APPROVED
- 17. BACK FILLING OF THE WET WELL SHALL COMPLY WITH CITY OF ROUND ROCK STANDARD SPECIFICATIONS.
- 18. ALL INTERIOR SURFACES OF CONCRETE WET WELL SHALL BE COATED WITH AN 80 MIL



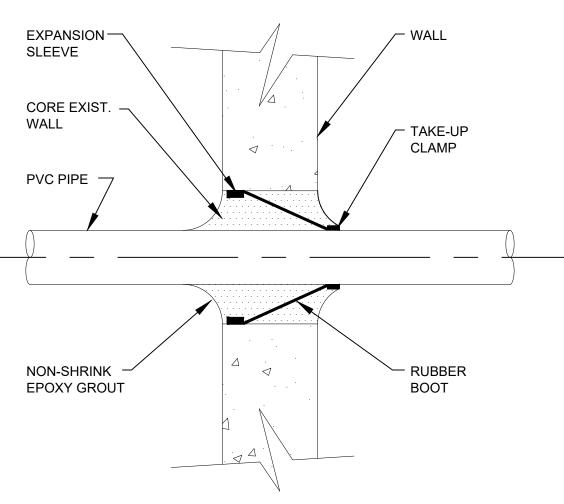
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SHEET NUMBER 48 OF 204

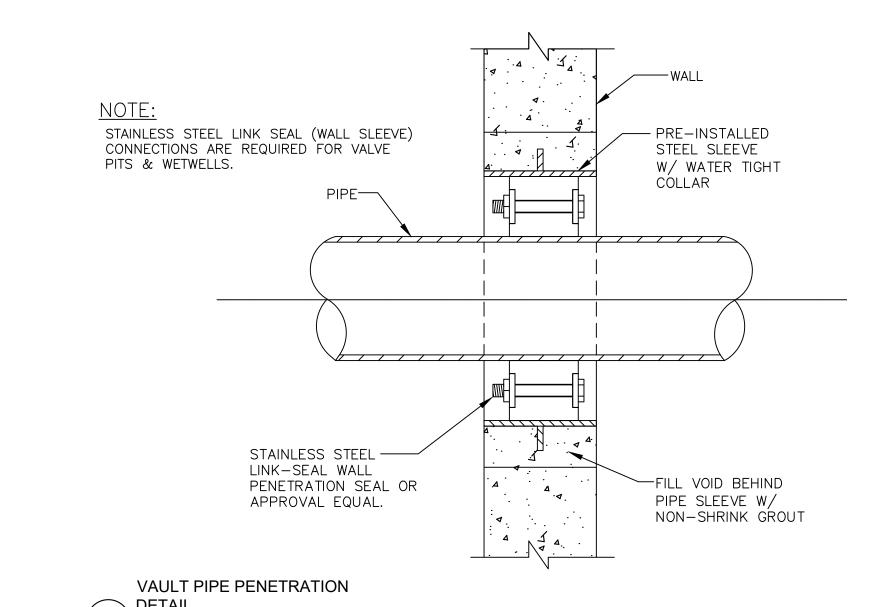


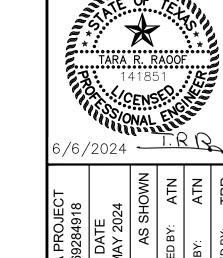
ADJUSTABLE PIPE SUPPORT

WG <sup>-</sup>	T APPROX. LBS EAC	_					D		
PIPE SIZE	COMPLETE	SADDLE ONLY	Α	А	Α	MINIMUM	MAXIMUM		
2 1/2	9.0	4.8	2 1/2	3 1/2	1 1/2	8	13		
3	9.2	5.0	2 1/2	3 3/4	1 1/2	8 1/4	13 1/4		
3 1/2	9.4	5.2	2 1/2	4	1 1/2	8 1/2	13 1/2		
4	15.0	7.6	3	4 1/4	2 1/2	9 1/4	14		
5	16.7 8.3	8.3	3	4 7/8	2 1/2	10	14 3/4		
6	17.7	10.3	3	5 1/2	2 1/2	10 1/2	15 1/4		
8	20.2	12.8	3	6 7/8	2 1/2	11 3/4	16 1/2		
10	25.2	17.8	3	5 1/2	2 1/2	13 1/2	18 1/4		
12	29.0	21.6	3	9 15/16	2 1/2	15	19 3/4		
14	49.2	38.0	4	10 15/16	3	16 1/4	20 3/4		
16	53.2	42.0	4	12 3/8	3	17 3/4	22 1/4		
18	70.8	51.0	6	13 7/8	3 1/2	19 1/2	24		
20	104.8	85.0	6	15 3/8	3 1/2	21	25 1/2		
24	137.0	110.0	6	17 15/16	4	23 3/4	28 1/2		
30	170.0	150.0	6	21 5/16	4	27	31 1/2		
32	181.0	161.1	6	22 1/2	4	28 1/8	32 3/4		
36	249.0	229.0	6	24 1/4	4	30 1/4	34 3/4		









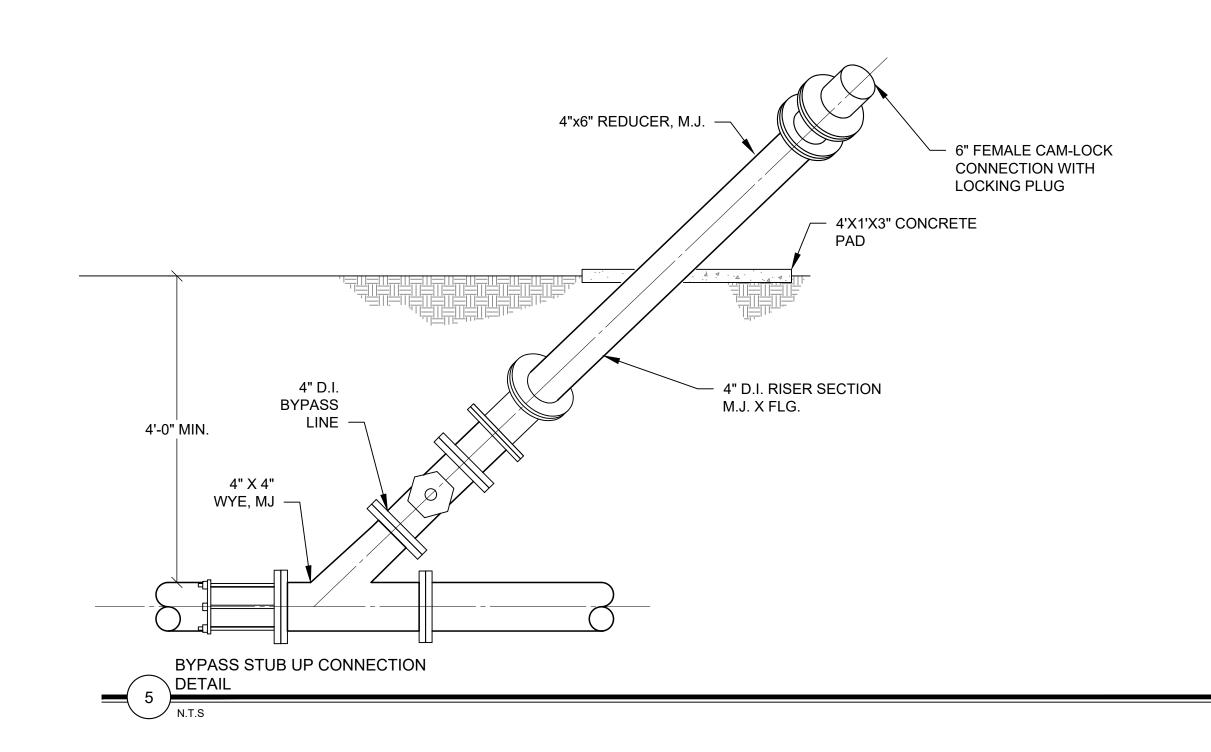
/6/2024 I.R.R							
069284918	DATE	MAY 2024	SCALE: AS SHOWN	DESIGNED BY: ATN	DRAWN BY: ATN	CHECKED BY: TRR	

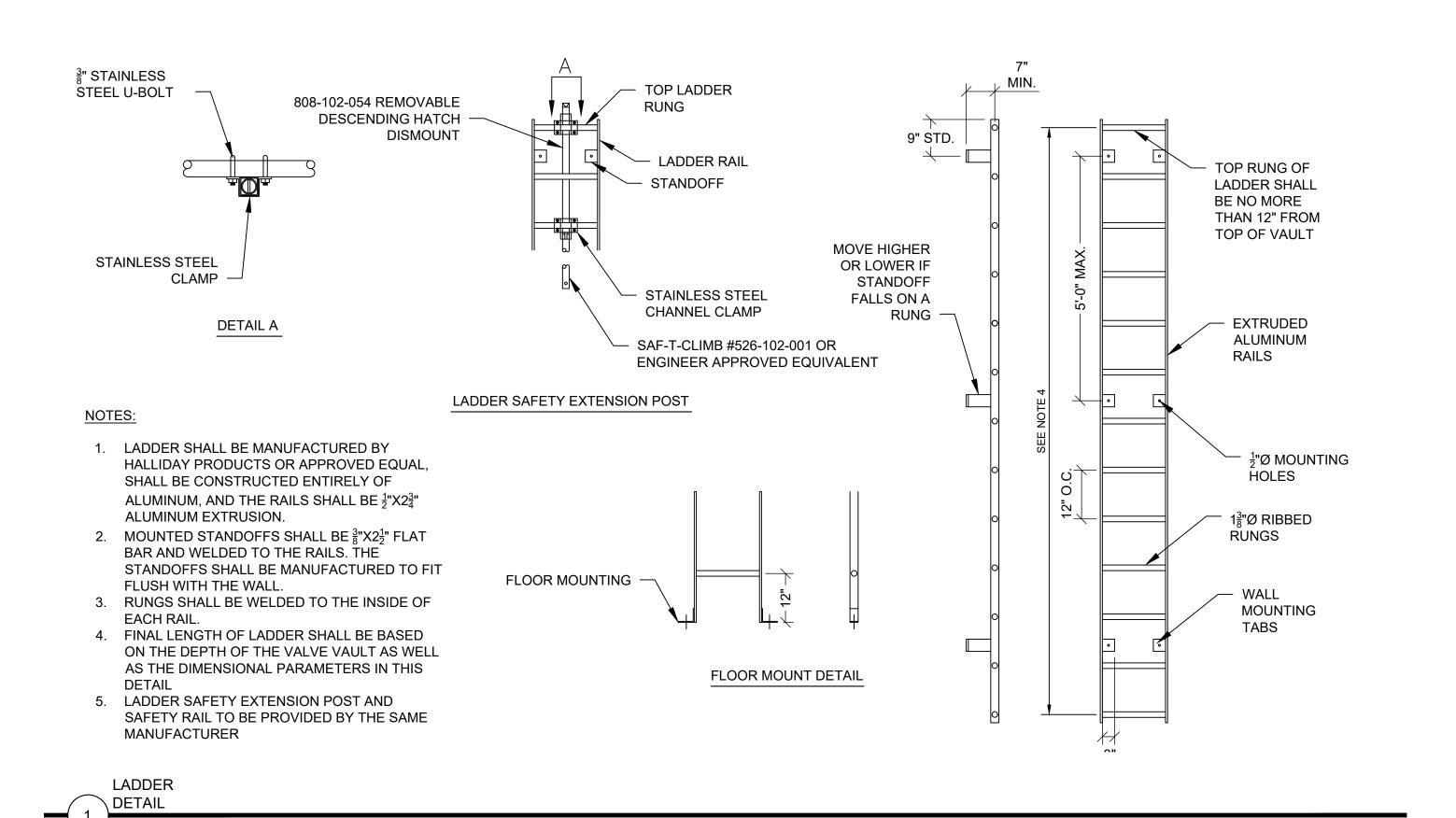
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SHEET NUMBER 49 OF 204

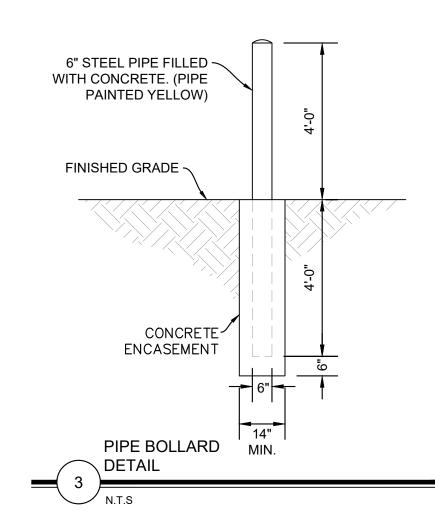
½" BALL VALVE —— — 2" VENTED COMBINATION AIR RELEASE VALVE (ARI D-025L OR EQUAL) 2" BRONZE 1/4 TURN BALL VALVE — 2" SCH 80 BRASS THREADED PIPE — 2" CORPORATION STOP MUELLER H-15020 OR APPROVED EQUAL <sup>1</sup> PVC SCH 80 AIR BLEED PIPE SMITH-BLAIR 372 TAPPING SADDLE OR APPROVED EQUAL — BRACE 2" BRASS PIPE W/
UNISTRUT CROSS BRACING
AND STAINLESS STEEL
STRAPS DUCTILE IRON CLAMP SADDLE CLAMPING — PIPE BACK TO 4" FROM VAULT FLOOR DRAIN AIR RELEASE VALVE
DETAIL

N.T.S





1. PRESSURE GAUGES SHALL HAVE 4-INCH DIAMETER FACES AND BE CALIBRATED FROM 0 TO 150 PSI. PRESSURE GAUGE~ 2. GAUGE SHALL BE FILLED WITH GLYCOL AND INSTALLED WITH A BRASS PRESSURE PRESSURE SNUBBER AND SHALL SNUBBER BE OF STAINLESS STEEL S.S. PROTECTIVE CONSTRUCTION. DIAPHRAGM 3. INSTALL GAUGE WITH A 0.25-INCH NPT CONNECTION AND %" THREADED INCLUDE A CORPORATION STOP BRONZE VALVE AND AIR BLEED. 4. VALVES TO BE RATED AT 200 1"X½" 300 LB. BRONZĒ BUSHING 5. ALL WELDED COUPLINGS SHALL 3000 LB. COUPLING / WELDED TO PIPE BE RATED FOR 3000 LBS. 6. GAUAGE SHALL BE TRERICE MANUFACTURED OR APPROVED PRESSURE GAUGE



Synchronous Speed: 3450 RPM 2<sup>1</sup>/<sub>2</sub> / 3 inch Discharge EBG-31 EBG-33 (3HP)

4.25" DIA

120

Capacity [USGPM]

160

Note: Full diameter impeller included in price of pump. Consult factory for reduced diameter impeller and pricing.

80



PUMP INFORMATION

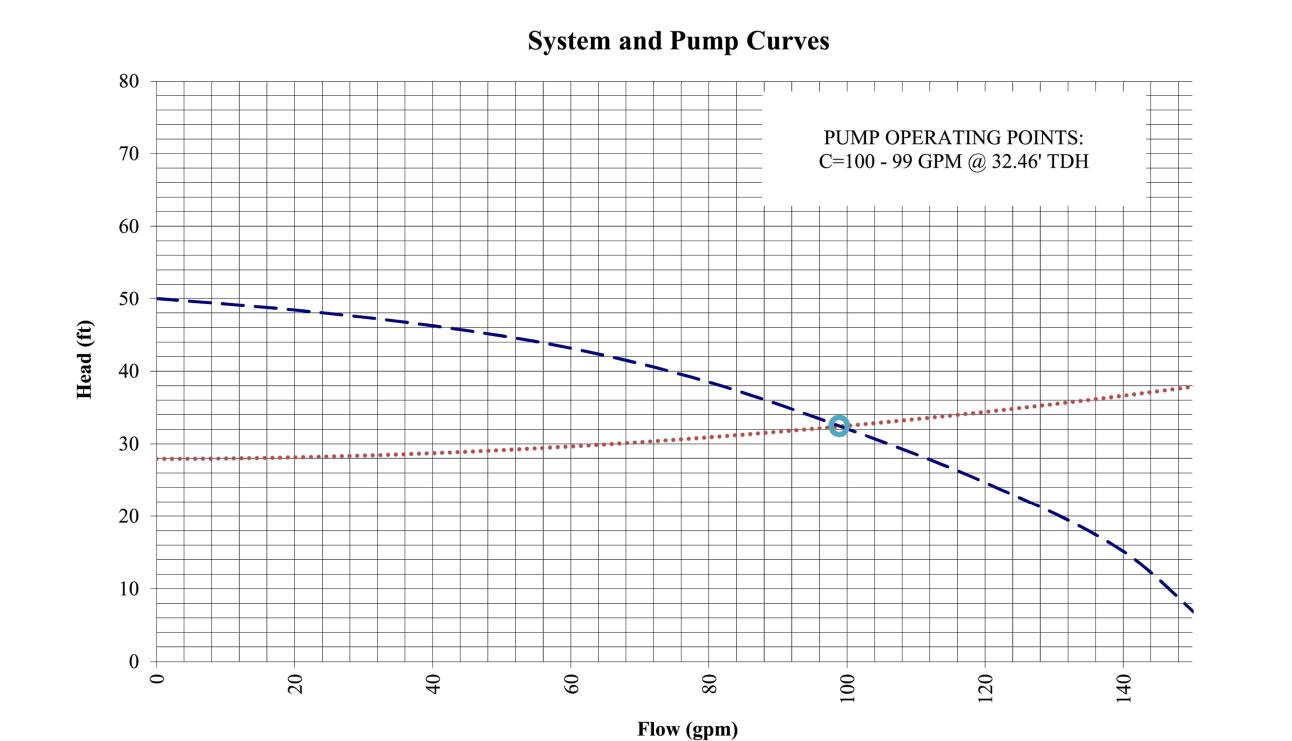
40

Head / [Ft]

1-197

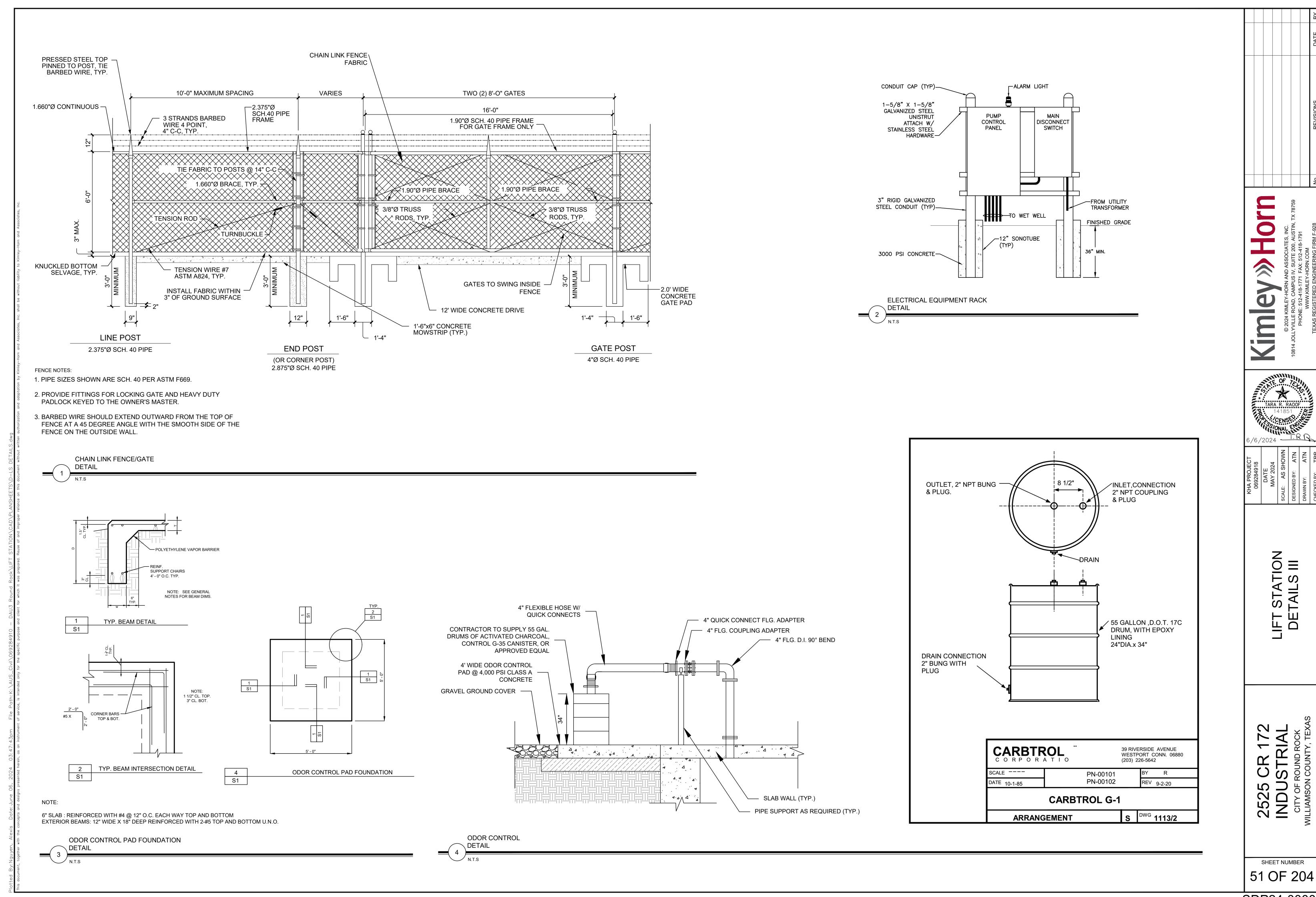
200

rev. 05/15



— Pump Model Information

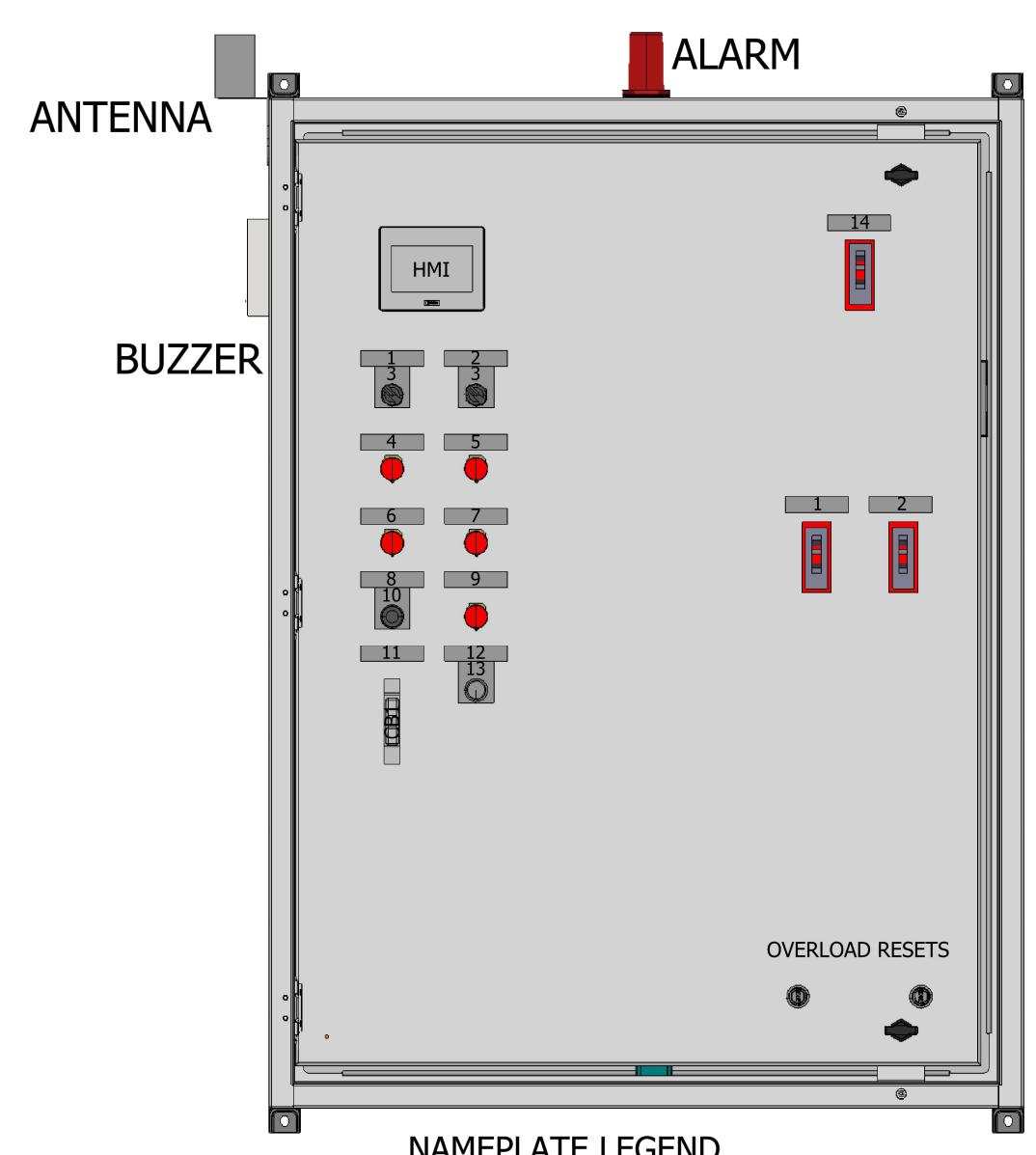
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# NEMA 4X STAINLESS STEEL ENCLOSURE 48"H x 36"W x 12"DP OUTER DOOR NOT SHOWN

**BACK PANEL** 

SENSAPHONE

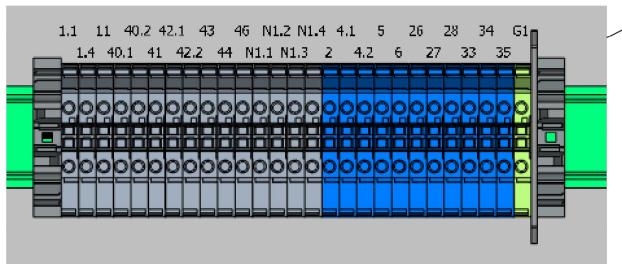


NAMEPLATE LEGEND 11. CONTROL

**FRONT** 

- 1. PUMP 1
- 2. PUMP 2

- 8. ALARM
- 9. BACKUP ON
- 10. SILENCE



LAMINATED DRAWING REQUIRED

RECEPTACLE	MAIN  PMR  A  A
F3 TRF F4 EPMR4A #1 MTH  SENTRY-ANALOG K1 K2 HLR ALT DT1	1CB 2CB
SF1 SF2 OT2 OT1 HLF CR1 CR2 BUR ASR	TRF1
HEATER    Occupation production p	1M 2M

l	JSEMCO I	NC	ORF	PORAT	ED						
1602 REZIN RD, TOMAH, WI 54660 PHONE: (608) 372-5911 FAX: (608) 372-5016 WWW.USEMCO.COM											
	JSTOMER:							ESCRIPTION:			
M.	MANSFIELD PUMPCOMPANY 2525 CR 172 INDUSTRIAL										
REV. NO.	REVISION	NAME	DATE		NAME	DATE	PAGE DES	CRIPTION			
				DESIGNED BY:	nchawla	05/16/24	DANIEL LAVOLIT				
				DRAWN BY:	NIKESH CHAWLA	05/17/24			1		
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&EMC/1 JOB NO.

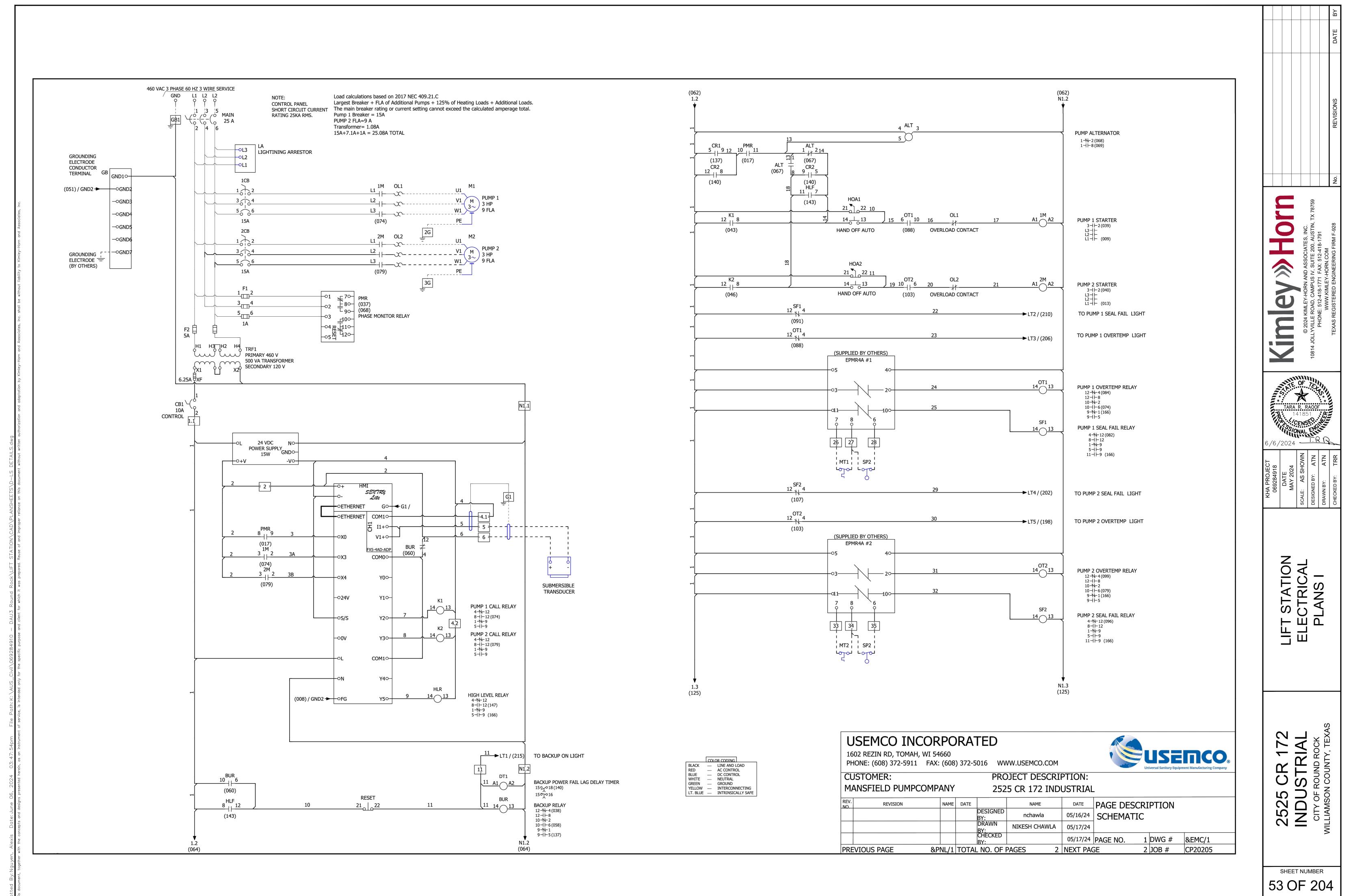
&/3 TOTAL NO. OF PAGES

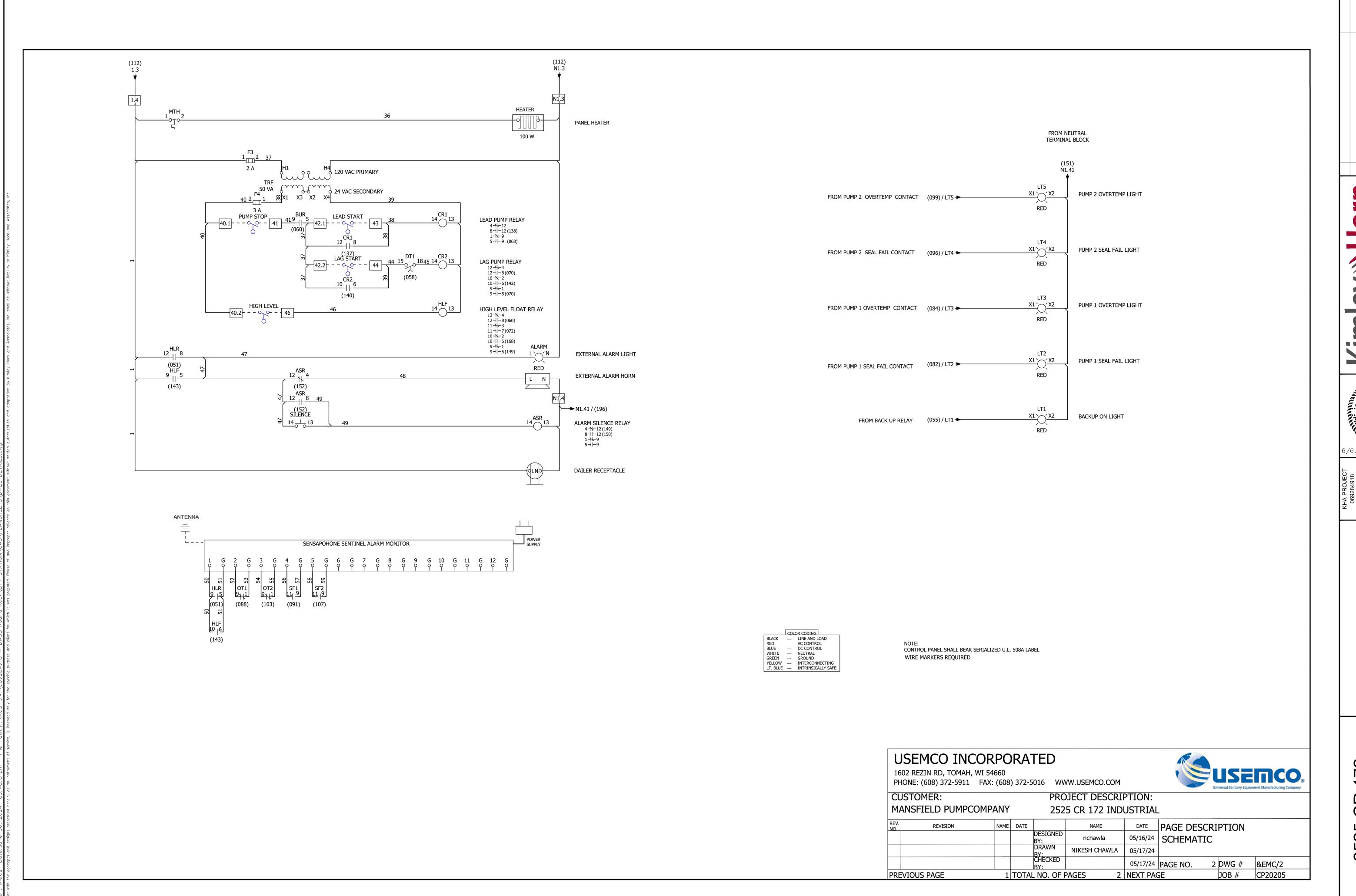
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SDP24-00001

12. BACKUP 3. HAND OFF AUTO 13. RESET 4. PUMP 1 OVERTEMP 14. MAIN 5. PUMP 2 OVERTEMP 6. PUMP 1 SEAL FAIL 7. PUMP 2 SEAL FAIL

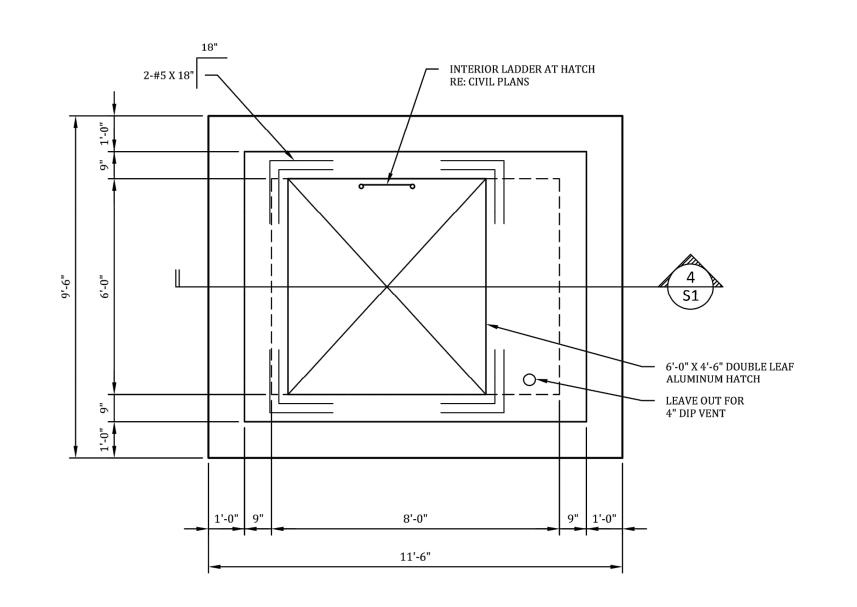




LIFT STATION ELECTRICAL PLANS

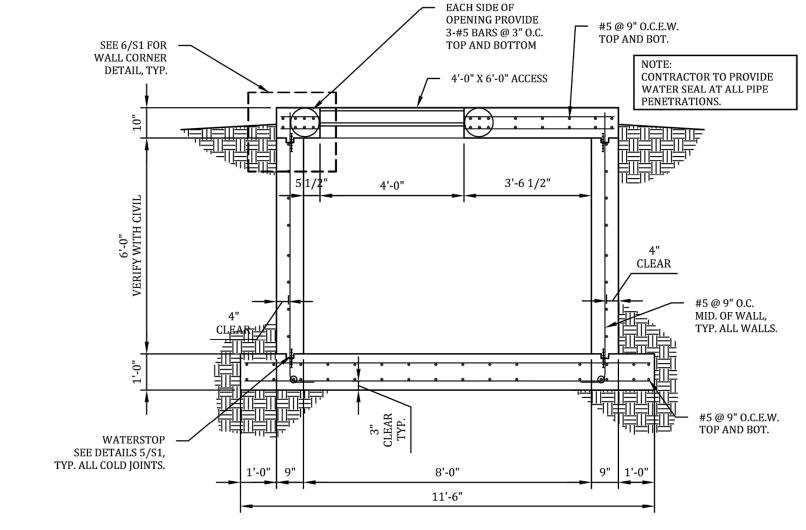
2525 CR 172
INDUSTRIAL
CITY OF ROUND ROCK
WILLIAMSON COUNTY, TEXAS

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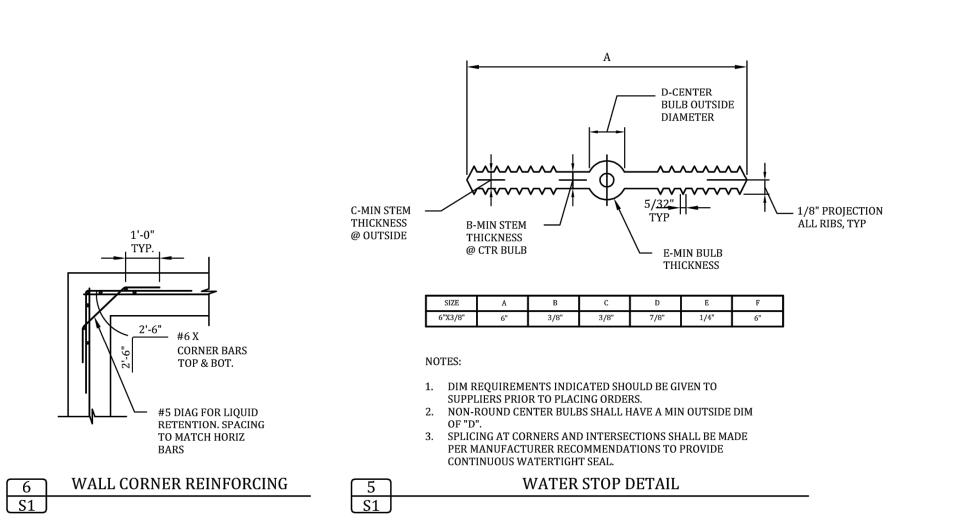
EDGE OF FOUNDATION EDGE OF TOP SLAB 36" X 48" ACCESS COVER, 2-#5 X 3'-0" DIAGONAL. CAST IN PLACE CONCRETE LID 2-#5 X 3'-0" DIAGONAL AT CORNER OF HATCH OPENINGS EACH SIDE OF OPENING PROVIDE #5 @ 6" O.C. 3-#6 BARS @ 3" O.C. #5 @ 6" O.C. TOP AND BOTTOM #5 @ 3" O.C. 6'-0" VERIFY - WET WELL 9'-4" VERIFY

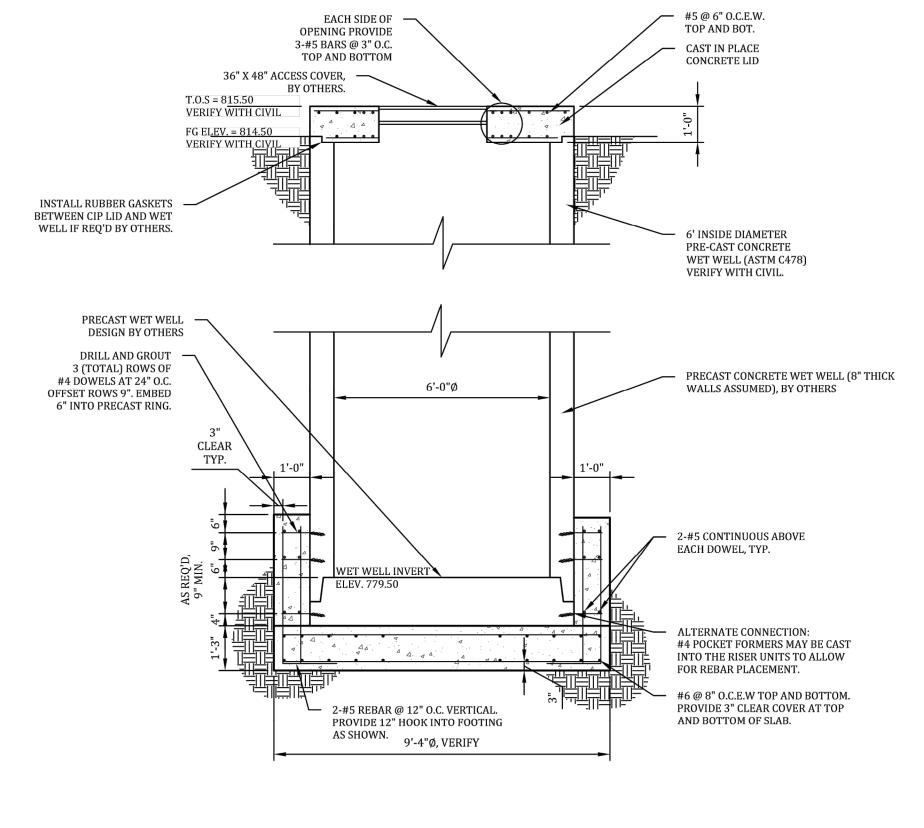
WET WELL PLAN VIEW AND CAP REINF. AND DIM CONTROL



VALVE VAULT PLAN VIEW - DIM CONTROL

VALVE VAULT CROSS SECTION





COORDINATE ALL PENETRATIONS WET WELL ANTI-FLOTATION RING AND ANCHORING WITH THE CIVIL, HAS BEEN DESIGNED FOR 35'-0" MECHANICAL, AND ELECTRICAL DRAWINGS WET WELL DEPTH PER KIMLEY HORN PLANS DATE MAY 22, 2024. ALL GROUTING IS TO BE DONE USING HILTI HIT-RE 500 V3 EPOXY MORTAR. ALL REBAR TO BE EPOXY-COATED. CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY ENGINEER OF DISCREPANCIES BEFORE PROCEEDING WITH CONSTRUCTION. AN 8" WET WELL WALL THICKNESS IS ASSUMED IN GEOMETRY OF ANTI-FLOTATION RING/FOUNDATION AND CIP WET WELL ROOF.

> WET WELL ANTI-FLOTATION RING/FOUNDATION AND CIP WET WELL ROOF CROSS SECTION

GENERAL NOTES

CONCRETE MIX DESIGN:

CONCRETE REINFORCEMENT:

1. ALL CONCRETE SHALL HAVE MINIMUM COMPRESSIVE STRENGTH f'c=3600 psi, TxDOT CLASS C CONCETE, AT 28 DAYS. PROVIDE 5" SLUMP. ADDING WATER AT SITE IS NOT PERMITTED.

2. THE CONCRETE MIX DESIGNS SHALL BE PROPORTIONED TO MINIMIZE THE ADVERSE EFFECTS OF CLIMATE AT THE TIME OF YEAR THE CONCRETE IS PLACED. USE OF WORKABILITY ADMIXTURES AND

3. USE OF CALCIUM CHLORIDE ADMIXTURES IS NOT PERMITTED.

4. THE BUILDER/CONTRACTOR IS RESPONSIBLE FOR MAKING CONCRETE COMPRESSIVE STRENGTH CYLINDER TESTS AS REQUIRED BY A.C.I. 318-14.

1. ALL REINFORCING STEEL SHALL BE NEW BILLET STEEL CONFORMING TO A.S.T.M. A-615 GRADE 60. REINFORCEMENT SHALL BE FREE OF RUST AND DELETERIOUS MATERIALS.

1. SUFFICIENT SUPPORT BARS AND CHAIRS SHALL BE PROVIDED TO MAINTAIN PROPER

REINFORCEMENT PROFILE THROUGHOUT CONCRETE PLACEMENT. BARS SHALL BE SUPPORTED ON CHAIRS AND TIED AT ALL INTERSECTIONS. USE 4'-0" MAXIMUM SPACING FOR SUPPORT CHAIRS.

LAYER OF LEVELING SAND MAY BE PLACED ON TOP OF THE POLYETHYLENE.

6. CONTRACTOR NOTE: BEFORE PROCEEDING WITH ANY WORK OR ORDERING OF MATERIALS, THE BUILDING COMPONENTS AND THEIR INTERRELATIONSHIP AT THE BUILDING SITE AND FOR THEIR

4. HEAVY VEGETATION SUCH AS TREES WITHIN 10-15 FEET OF THE PERIMETER OF THE FOUNDATION MAY CAUSE EXCESSIVE WATER REMOVAL FROM THE SOIL NEAR OR UNDER THE FOUNDATION. SUCH UNANTICIPATED PARAMETERS ARE NOT CONSIDERED UNLESS REPORTED BY THE CONTRACTOR TO FALKOFSKE ENGINEERING, INC. IN WRITING PRIOR TO DESIGN.

5. GEO-TECHNICAL INVESTIGATION PROVIDED BY: ROCK ENGINEERING & TESTING LABORATORY, INC. REPORT NO. G221847

ALLOWABLE SOIL BEARING PRESSURE (BEARING IN CLAY) ALLOWABLE SOIL BEARING PRESSURE (BEARING IN BEDROCK) MINIMUM DEPTH OF FOUNDATION INTO BEARING SOIL

6. ALL FILL AREAS AND MATERIALS SHALL BE PLACED, COMPACTED, AND TESTED PER THE GEO-TECHNICAL ENGINEERS RECOMMENDATIONS.

7. BUILDING CODE: 2015 INTERNATIONAL BUILDING CODE

1. INSPECTIONS ARE TO BE AS REQUIRED BY LOCAL BUILDING DEPARTMENT.

2. FALKOFSKE ENGINEERING INC. SHALL BE CALLED FOR CONSTRUCTION REVIEW OF FOUNDATION BEAMS AND REINFORCEMENT PRIOR TO PLACING CONCRETE.

The use of precast concrete bar supports (commonly called "Dobies") to support slab reinforcement or beam reinforcement for concrete slab on grade foundations is acceptable. It is important that the precast are from the Aztec Concrete Accessories, Inc. which produces them, then they are pre-approved by Falkofske Engineering, Inc.

As referenced by Aztec Concrete Accessories, Inc., the Concrete Reinforcing Steel Institute (CRSI) references the use of concrete bar supports in the Manual of Standard Practice. The Manual of Standard Practice shows the use of wire dobies, plain dobies, combination (those with grooves), and dowel dobies (those with a hole for dowel bar support). The ACI also references the use of dobies as a standard practice, and the CRSI

It is our opinion that precast concrete bar supports do a much better job of supporting slab reinforcement

foundation design.

AIR ENTRAINMENT IS PERMITTED AND MUST BE NOTED ON MIX DESIGNS.

2. INSURE PROPER VIBRATION OF CONCRETE AROUND ALL REINFORCEMENT.

3. TRENCHES FOR DEEP PLUMBING LINES SHOULD NOT BE LOCATED DIRECTLY UNDER (PARALLEL TO)

4. ALL SLABS AND BEAMS SHALL BE PROPERLY GRADED AND LEVELED. A THIN (1"±) LAYER OF SAND MAY BE USED UNDER THE SLAB.

5. A POLYETHYLENE VAPOR BARRIER SHALL BE PLACED BETWEEN THE CONCRETE AND SOIL. A THIN

CONTRACTOR AND/OR SUBCONTRACTORS SHALL VERIFY ALL MEASUREMENTS AND LOCATION OF

1. THE REINFORCED CONCRETE DESIGN OF THIS FOUNDATION STRUCTURE HAS BEEN ANALYZED USING STANDARD ENGINEERING PRACTICES.

2. THE REINFORCED CONCRETE SLAB-ON-GRADE DESIGN SHOWN REFLECTS THE MINIMUM REQUIREMENTS NEEDED TO ALLOW THIS STRUCTURE SYSTEM TO ADEQUATELY PERFORM IN CONJUNCTION WITH THE SOIL PARAMETERS NOTED BELOW. THE INTENT IS NOT TO LIMIT SOIL MOVEMENT, BUT INSTEAD TO LIMIT EXCESSIVE FOUNDATION FLEXURE SO THAT DIFFERENTIAL MOVEMENT WILL NOT CAUSE UNREASONABLE DISTRESS.

3. FOUNDATION MAINTENANCE: DRAINAGE AWAY FROM THE FOUNDATION SHALL BE MAINTAINED AT ALL TIMES SO THAT GROUND WATER WILL NOT COLLECT ADJACENT TO OR UNDER THE SLAB. THE INTENT IS TO MAINTAIN AN EVEN SOIL MOISTURE CONTENT AROUND THE BUILDING SO THAT DIFFERENTIAL SOIL MOVEMENT IS LIMITED.

DATED: 12/23/21

Qa = 15,000 psf

CONTRACTORS OPTION:

concrete bar supports are in fact made of concrete with an allowable f c of 3000 psi. If the pads purchased

Manual of Standard Practice as a source for placing layouts.

than plastic chairs which is typical in the today's Slab on Grade foundation construction.

Brick veneer of either clay brick or concrete brick shall not be used in any Falkofske Engineering, Inc.

ERIK GARCIA

147076



SHEET NUMBER 55 OF 204

CONTRACTOR TO VERIFY ALL DIMENSIONS WITH EQUIPMENT SIZES PRIOR TO CONSTRUCTION



# SECTION 7: TEMPORARY STORMWATER SECTION

# **Temporary Stormwater Section**

### **Texas Commission on Environmental Quality**

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

# Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: Nicholas C. Brown, P.E.
Date: <u>05/15/2025</u>
Signature of Customer/Agent:
Hilly
Regulated Entity Name: 2525 CR 172 Industrial

# **Project Information**

## **Potential Sources of Contamination**

Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.

1.	Fuels for construction equipment and hazardous substances which will be used during construction:
	The following fuels and/or hazardous substances will be stored on the site:
	These fuels and/or hazardous substances will be stored in:
	Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

- Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year.
   Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
- X Fuels and hazardous substances will not be stored on the site.
- 2. X Attachment A Spill Response Actions. A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
- 3. X Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
- 4. X Attachment B Potential Sources of Contamination. A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.

## Sequence of Construction

- 5. X Attachment C Sequence of Major Activities. A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
  - X For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given.
  - X For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
- 6. X Name the receiving water(s) at or near the site which will be disturbed or which will receive discharges from disturbed areas of the project: N/A

# Temporary Best Management Practices (TBMPs)

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

7. X Attachment D – Temporary Best Management Practices and Measures. TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

	<ul> <li>A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.</li> <li>A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.</li> <li>A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer.</li> <li>A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.</li> </ul>
8.	The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
	<ul> <li>Attachment E - Request to Temporarily Seal a Feature. A request to temporarily seal a feature is attached. The request includes justification as to why no reasonable and practicable alternative exists for each feature.</li> <li>There will be no temporary sealing of naturally-occurring sensitive features on the site.</li> </ul>
9.	Attachment F - Structural Practices. A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.
10.	Attachment G - Drainage Area Map. A drainage area map supporting the following requirements is attached:
	<ul> <li>For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided.</li> <li>For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used.</li> <li>For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area.</li> <li>There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.</li> </ul>

- ☐ There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. Erosion and sediment controls other than sediment basins or sediment traps within each disturbed drainage area will be used.
   11. ☒ Attachment H Temporary Sediment Pond(s) Plans and Calculations. Temporary sediment pond or basin construction plans and design calculations for a proposed temporary BMP or measure have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information must be signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed temporary BMPs and measures are attached.
   ☐ N/A
   12. ☒ Attachment I Inspection and Maintenance for BMPs. A plan for the inspection of each temporary BMP(s) and measure(s) and for their timely maintenance, repairs, and, if necessary, retrofit is attached. A description of the documentation procedures, recordkeeping practices, and inspection frequency are included in the plan and are
- 13. X All control measures must be properly selected, installed, and maintained in accordance with the manufacturer's specifications and good engineering practices. If periodic inspections by the applicant or the executive director, or other information indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations.
- 14. X If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain).
- 15. X Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50%. A permanent stake will be provided that can indicate when the sediment occupies 50% of the basin volume.
- 16. X Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source for stormwater discharges (e.g., screening outfalls, picked up daily).

### Soil Stabilization Practices

specific to the site and/or BMP.

Examples: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

17. X Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices. A schedule of the interim and permanent soil stabilization practices for the site is attached.

- 18. Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 19. X Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

### Administrative Information

- 20. X All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
- 21. X If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
- 22. X Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

# SPILL RESPONSE REACTIONS

If there is an accidental spill on site, the contractor shall respond with appropriate action. All onsite personnel will be trained to perform and be knowledgeable of the spill response actions. The contractor will be required to contact the owner and in turn the owner will contact the TCEQ in the event of a spill on site. In addition to the following guidance, reference the latest version of TCEQ's Technical Guidance Manual (TGM) RG-348 Section 2.4.16.

### Cleanup

- Clean up leaks and spills immediately.
- Use a rag for small spills on paved surfaces, a damp mop for general cleanup, and absorbent material for larger spills. If the spilled material is hazardous, then the used cleanup materials are also hazardous and must be disposed of as hazardous waste.
- Never hose down or bury dry material spills. Clean up as much of the material as possible and dispose of properly. See the waste management BMPs in this section for specific information.

### **Minor Spills**

- Minor spills typically involve small quantities of oil, gasoline, paint, etc. which can be controlled by the first responder at the discovery of the spill.
- Use absorbent materials on small spills rather than hosing down or burying the spill.
- Absorbent materials should be promptly removed and disposed of properly.
- Follow the practice below for a minor spill:
- Contain the spread of the spill.
- Recover spilled materials.
- Clean the contaminated area and properly dispose of contaminated materials.

### **Semi-Significant Spills**

Semi-significant spills still can be controlled by the first responder along with the aid of other personnel such as laborers and the foreman, etc. This response may require the cessation of all other activities.

Spills should be cleaned up immediately:

- Contain spread of the spill.
- Notify the project foreman immediately.
- If the spill occurs on paved or impermeable surfaces, clean up using "dry" methods (absorbent materials, cat litter and/or rags). Contain the spill by encircling with absorbent materials and do not let the spill spread widely.
- If the spill occurs in dirt areas, immediately contain the spill by constructing an earthen dike. Dig up and properly dispose of contaminated soil.
- If the spill occurs during rain, cover spill with tarps or other material to prevent contaminating runoff.

### Significant/Hazardous Spills

For significant or hazardous spills that are in reportable quantities:

- Notify the TCEQ by telephone as soon as possible and within 24 hours at (512)339-2929 (Austin) or 210-490-3096 (San Antonio) between 8 AM and 5 PM. After hours, contact the Environmental Release Hotline at 1-800-832-8224. It is the contractor's responsibility to have all emergency phone numbers at the construction site.
- For spills of federal reportable quantities, in conformance with the requirements in 40 CFR parts 110,119, and 302, the contractor should notify the National Response Center at (800) 424-8802.
- Notification should first be made by telephone and followed up with a written report.

- The services of a spills contractor or a Haz-Mat team should be obtained immediately. Construction personnel should not attempt to clean up until the appropriate and qualified staffs have arrived at the job site.
- Other agencies which may need to be consulted include, but not limited to, the City Police Department, County Sheriff Office, Fire Departments, etc.

# POTENTIAL SOURCES OF CONTAMINATION

Potential Source: Oil, grease, fuel, and hydraulic fluid contamination from construction equipment and vehicle dripping.

Preventative Measures: Vehicle maintenance will be performed within the construction staging area or a local maintenance shop.

Potential Source: Miscellaneous trash and litter from construction workers and material wrappings.

Preventative Measures: Trash containers will be placed throughout the site to encourage proper disposal of trash.

Potential Source: Silt leaving the site.

Preventative Measures: Contractor will install all temporary best management practices prior to start of construction including the stabilized construction entrance to prevent tracking onto adjoining streets.

Potential Source: Construction Debris.

Preventative Measures: Construction debris will be monitored daily by contractor. Debris will be collected weekly and placed in disposal bins. Situations requiring immediate attention will be addressed on a case-by-case basis.

Potential Source: Soil and Mud from Construction Vehicle tires as they leave the site.

Preventative Measures: A stabilized construction exit shall be utilized as vehicles leave the site. Any soil, mud, etc. carried from the project onto public roads shall be cleaned up within 24 hours.

Potential Source: Sediment from soil, sand, gravel and excavated materials stock piled on site.

Preventative Measures: Silt fence shall be installed on the down gradient side of the stock piled materials. Reinforced rock berms shall be installed at all downstream discharge locations.

Potential Source: Portable toilet spill.

Preventative Measures: Toilets on the site will be emptied on a regular basis by the contracted toilet company.

## **SEQUENCE OF MAJOR ACTIVITIES**

The installation of erosion and sedimentation controls for this SCS shall occur prior to any excavation of materials or major disturbances on the site. The sequence of major construction activities will be as follows. Approximate acreage to be disturbed is listed in parentheses next to each activity.

# **Intended Schedule or Sequence of Major Activities:**

- 1. Construct Access (1.01 Acres)
- 2. Installation of Temporary BMPs (1.01 Acres)
- 3. Initiate Grubbing and Topsoil Stripping of Site (1.01 Acres)
- 4. Rough Subgrade Preparation (earthwork, grading, street and drainage excavation and embankment) (1.01 Acres)
- 5. Wet and Dry Utility Construction (1.01 Acres)
- 6. Final Subgrade Preparation (<u>1.01</u> Acres)
- 7. Installation of Base Materials (<u>1.01</u> Acres)
- 8. Concrete (foundations, curbs, flatwork) (<u>1.01</u> Acres)
- 9. Building Construction (<u>0</u> Acres)
- 10. Paving Activities (1.01 Acres)
- 11. Topsoil, Irrigation and Landscaping (1.01 Acres)
- 12. Site cleanup and Removal of Temporary BMPs (1.01 Acres)

Maximum total construction time is not expected to exceed 18 months.

# TEMPORARY BEST MANAGEMENT PRACTICES AND MEASURES

- **A.** No storm water originates up gradient that impacts the site. An upstream off-site area will bypass through an existing channel.
- **B.** Temporary BMPs will be installed prior to soil disturbing construction activity. Silt fencing will be placed along the down-gradient sides of the property to prevent silt from escaping the construction area. A temporary construction entrance will be placed on site to reduce vehicle "tracking" onto adjoining streets. A concrete washout pit will be used to collect all excess concrete during construction.

BMPs for this project will protect surface water or groundwater from turbid water, phosphorus, sediment, oil, and other contaminants, which may mobilize in storm water flows by slowing the flow of runoff to allow sediment and suspended solids to settle out of the runoff.

Practices may also be implemented on site for interim and permanent stabilization. Stabilization practices may include but are not limited to: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of existing trees and vegetation, and other similar measures.

**C.** There are six sensitive features located in on-site, as noted in the geologic assessment. A buffer will be provided for Feature 4 and 5 in all directions. Any excavation within 50 feet of the feature will be avoided. We are not proposing and fill or covering of the sensitive features.

There are no surface streams within the boundaries of the project. The temporary onsite BMPs will be used to treat stormwater runoff before it leaves the project and prevent pollutants from entering into surface streams or any sensitive features down-gradient of the site.

**D.** The BMPs for this project are designed to allow water to pass through after sedimentation has occurred. Existing flow patterns will be maintained to any naturally occurring sensitive features that are discovered during construction.

Temporary runoff protection measures will be installed according to the recommendations made in Chapter 1 of RG-348. Temporary erosion control will be placed as close to the site of construction soil disturbance as possible to minimize any disturbance with drainage areas. Frequent inspections of erosion controls will be warranted near the environmentally sensitive features, especially after every rainfall.

# **REQUEST TO TEMPORARY SEAL A FEATURE**

(NOT APPLICABLE)

### STRUCTURAL PRACTICES

Structural BMPs will be used to limit runoff discharge of pollutants from exposed areas of the site. BMPs will be installed prior to soil disturbing construction activity. Silt fencing will be placed along the down-gradient sides of the property to prevent silt from escaping the construction area. A temporary construction entrance will be placed at the site entry/exit point to reduce tracking onto adjoining streets. A construction staging area will be used onsite to perform all vehicle maintenance and for equipment and material storage. A concrete truck washout pit will be placed on site to provide containment and easier cleanup of waste from concrete operations. The location of all structural temporary BMP's are shown on the erosion control plan sheet and details and specifications are provided on the erosion control details sheet which can be found under Section 5, attachment E.

### **Description of Temporary BMPs**

### **Temporary Construction Entrance/Exit**

The purpose of a temporary gravel construction entrance is to provide a stable entrance/exit condition from the construction site and keep mud and sediment off public roads. A stabilized construction entrance is a stabilized pad of crushed stone located at any point traffic will be entering or leaving the construction site from a public right-of-way, street, alley, sidewalk or parking area. The purpose of a stabilized construction entrance is to reduce or eliminate the tracking or flowing of sediment onto public rights-of-way. This practice should be used at all points of construction ingress and egress.

Excessive amounts of mud can also present a safety hazard to roadway users. To minimize the amount of sediment loss to nearby roads, access to the construction site should be limited to as few points as possible and vegetation around the perimeter should be protected where access is not necessary. A rock stabilized construction entrance should be used at all designated access points.

Inspection and Maintenance Guidelines:

- (1) The entrance should be maintained in a condition, which will prevent tracking or flowing of sediment onto public rights-of-way. This may require periodic top dressing with additional stone as conditions demand and repair and/or cleanout of any measures used to trap sediment.
- (2) All sediment spilled, dropped, washed, or tracked onto public rights-of-way should be removed immediately by contractor.
- (3) When necessary, wheels should be cleaned to remove sediment prior to entrance onto public right-of-way.
- (4) When washing is required, it should be done on an area stabilized with crushed stone that drains into an approved sediment trap or sediment basin.
- (5) All sediment should be prevented from entering any storm drain, ditch or water course by using approved methods.

### Silt Fence

The purpose of a silt fence is to intercept and detain water-borne sediment from unprotected areas of a limited extent. Silt fence is used during the period of construction near the perimeter of a disturbed area to intercept sediment while allowing water to percolate through. This fence should remain in place until the disturbed area is permanently stabilized. Silt fence should not be used where there is a concentration of water in a channel or drainage way. If concentrated

flow occurs after installation, corrective action must be taken such as placing a rock berm in the areas of concentrated flow.

Silt fencing within the site may be temporarily moved during the day to allow construction activity provided it is replaced and properly anchored to the ground at the end of the day. Silt fences on the perimeter of the site or around drainage ways should not be moved at any time.

Inspection and Maintenance Guidelines:

- (1) Inspect all fencing weekly, and after any rainfall.
- (2) Remove sediment when buildup reaches 6 inches.
- (3) Replace any torn fabric or install a second line of fencing parallel to the torn section.
- (4) Replace or repair any sections crushed or collapsed in the course of construction activity. If a section of fence is obstructing vehicular access, consider relocating it to a spot where it will provide equal protection, but will not obstruct vehicles. A triangular filter dike may be preferable to a silt fence at common vehicle access points.
- (5) When construction is complete, the sediment should be disposed of in a manner that will not cause additional siltation and the prior location of the silt fence should be revegetated. The fence itself should be disposed of in an approved landfill.

#### **Concrete Washout Area**

The purpose of concrete washout areas is to prevent or reduce the discharge of pollutants to stormwater from concrete waste by conducting washout offsite, performing onsite washout in a designated area, and training employees and subcontractors.

The following steps will help reduce stormwater pollution from concrete wastes:

- Incorporate requirements for concrete waste management into material supplier and subcontractor agreements.
- Avoid mixing excess amounts of fresh concrete.
- Perform washout of concrete trucks in designated areas only.
- Do not wash out concrete trucks into storm drains, open ditches, streets, or streams.
- Do not allow excess concrete to be dumped onsite, except in designated areas.
- For onsite washout:
- Locate washout area at least 50 feet from sensitive features, storm drains, open ditches, or water bodies. Do not allow runoff from this area by constructing a temporary pit or bermed area large enough for liquid and solid waste.
- Wash out wastes into the temporary pit where the concrete can set, be broken up, and then disposed properly.

Below grade concrete washout facilities are typical. These consist of a lined excavation sufficiently large to hold expected volume of washout material. Above grade facilities are used if excavation is not practical. Temporary concrete washout facility (type above grade) should be constructed as shown on the details at the end of this section, with sufficient quantity and volume to contain all liquid and concrete waste generated by washout operations. Plastic lining material should be a minimum of 10 mil in polyethylene sheeting and should be free of holes, tears, or other defects that compromise the impermeability of the material.

When temporary concrete washout facilities are no longer required for the work, the hardened concrete should be removed and disposed of. Materials used to construct temporary concrete washout facilities should be removed from the site of the work and disposed of. Holes, depressions or other ground disturbance caused by the removal of the temporary concrete washout facilities should be backfilled and repaired.

#### **Rock Berm**

The purpose of a rock berm is to serve as a check dam in areas of concentrated flow, to intercept sediment-laden runoff, detain the sediment and release the water in sheet flow. The rock berm should be used when the contributing drainage area is less than 5 acres. Rock berms are used in areas where the volume of runoff is too great for a silt fence to contain. They are less effective for sediment removal than silt fences, particularly for fine particles, but are able to withstand higher flows than a silt fence. As such, rock berms are often used in areas of channel flows (ditches, gullies, etc.). Rock berms are most effective at reducing bed load in channels and should not be substituted for other erosion and sediment control measures further up the watershed.

Inspection and Maintenance Guidelines:

- (1) Inspection should be made weekly and after each rainfall by the responsible party. For installations in streambeds, additional daily inspections should be made.
- (2) Remove sediment and other debris when buildup reaches 6 inches and dispose of the accumulated silt in an approved manner that will not cause any additional siltation.
- (3) Repair any loose wire sheathing.
- (4) The berm should be reshaped as needed during inspection.
- (5) The berm should be replaced when the structure ceases to function as intended due to silt accumulation among the rocks, washout, construction traffic damage, etc.
- (6) The rock berm should be left in place until all upstream areas are stabilized and accumulated silt removed.

#### Inlet Protection

Storm sewers that are made operational prior to stabilization of the associated drainage areas can convey large amounts of sediment to natural drainage ways. In case of extreme sediment loading, the storm sewer itself may clog and lose a major portion of its capacity. To avoid these problems, it is necessary to prevent sediment from entering the system at the inlets. The following guidelines for inlet protection are based primarily on recommendations by the Virginia Dept. of Conservation and Recreation (1992) and the North Central Texas Council of Governments (NCTCOG, 1993b).

In developments for which drainage is to be conveyed by underground storm sewers (i.e., streets with curbs and gutters), all inlets that may receive storm runoff from disturbed areas should be protected. Temporary inlet protection is a series of different measures that provide protection against silt transport or accumulation in storm sewer systems. This clogging can greatly reduce or completely stop the flow in the pipes. The different measures are used for different site conditions and inlet types.

Care should be taken when choosing a specific type of inlet protection. Field experience has shown that inlet protection that causes excessive ponding in an area of high construction activity may become so inconvenient that it is removed or bypassed, thus transmitting sediment-laden flows unchecked. In such situations, a structure with an adequate overflow mechanism should be utilized.

It should also be noted that inlet protection devices are designed to be installed on construction sites and not on streets and roads open to the public. When used on public streets these devices will cause ponding of runoff, which can cause minor flooding and can present a traffic hazard. An example of appropriate siting would be a new subdivision where the storm drain system is installed before the area is stabilized and the streets open to the general public. When construction occurs adjacent to active streets, the sediment should be controlled on site and not on public thoroughfares. Occasionally,

roadwork or utility installation will occur on public roads. In these cases, inlet protection is an appropriate temporary BMP.

The following inlet protection devices are for drainage areas of one acre or less. Runoff from larger disturbed areas should be routed to a temporary sediment trap or basin.

Filter barrier protection using silt fence is appropriate when the drainage area is less than one acre and the basin slope is less than five percent. This type of protection is not applicable in paved areas.

Block and gravel protection is used when flows exceed 0.5 cubic feet per second and it is necessary to allow for overtopping to prevent flooding. This form of protection is also useful for curb type inlets as it works well in paved areas.

Wire mesh and gravel protection is used when flows exceed 0.5 cubic feet per second and construction traffic may occur over the inlet. This form of protection may be used with both curb and drop inlets.

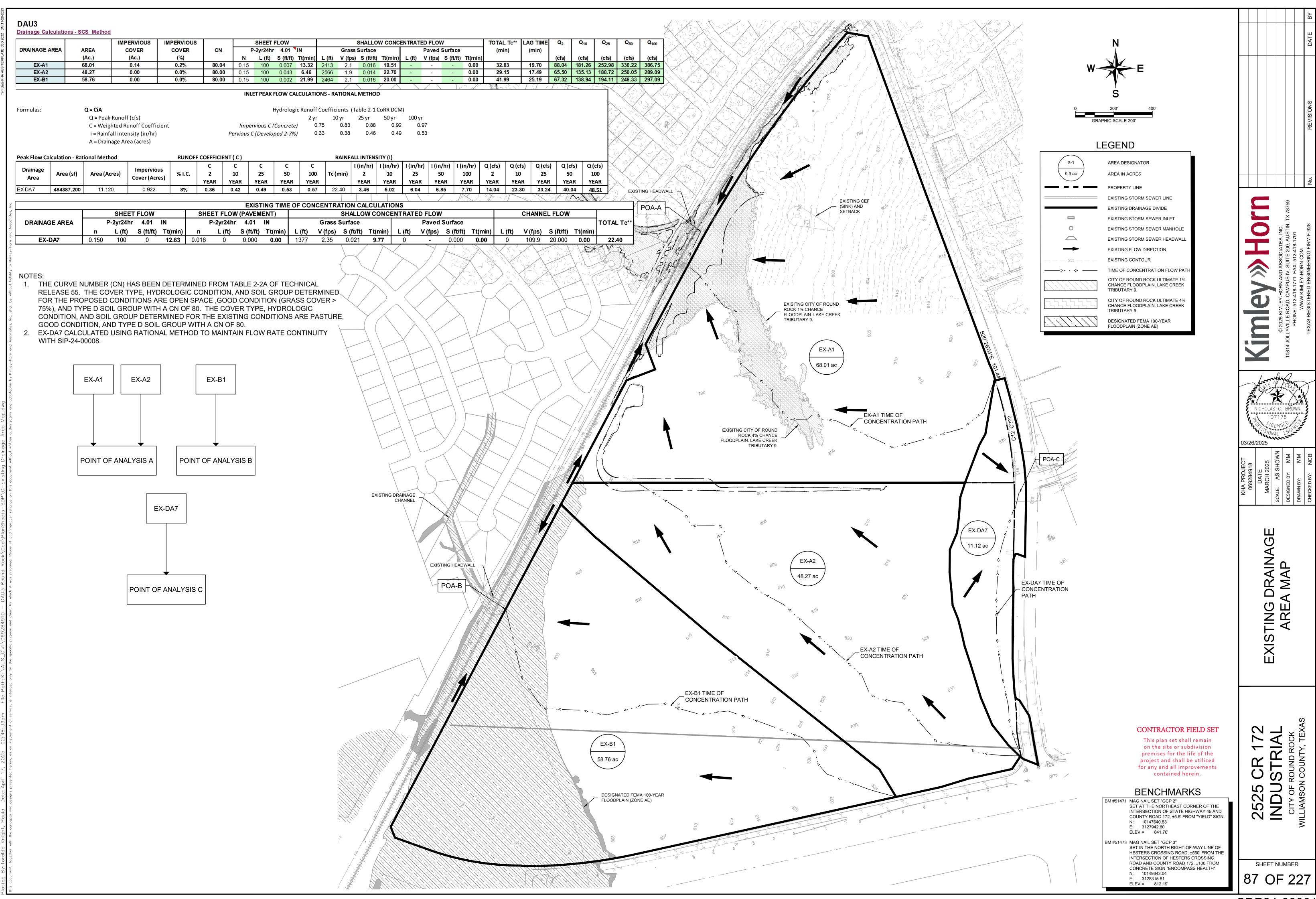
Excavated impoundment protection around a drop inlet may be used for protection against sediment entering a storm drain inlet. With this method, it is necessary to install weep holes to allow the impoundment to drain completely. If this measure is implemented, the impoundment should be sized such that the volume of excavation is 3,600 cubic feet per acre (equivalent to 1 inch of runoff) of disturbed area entering the inlet.

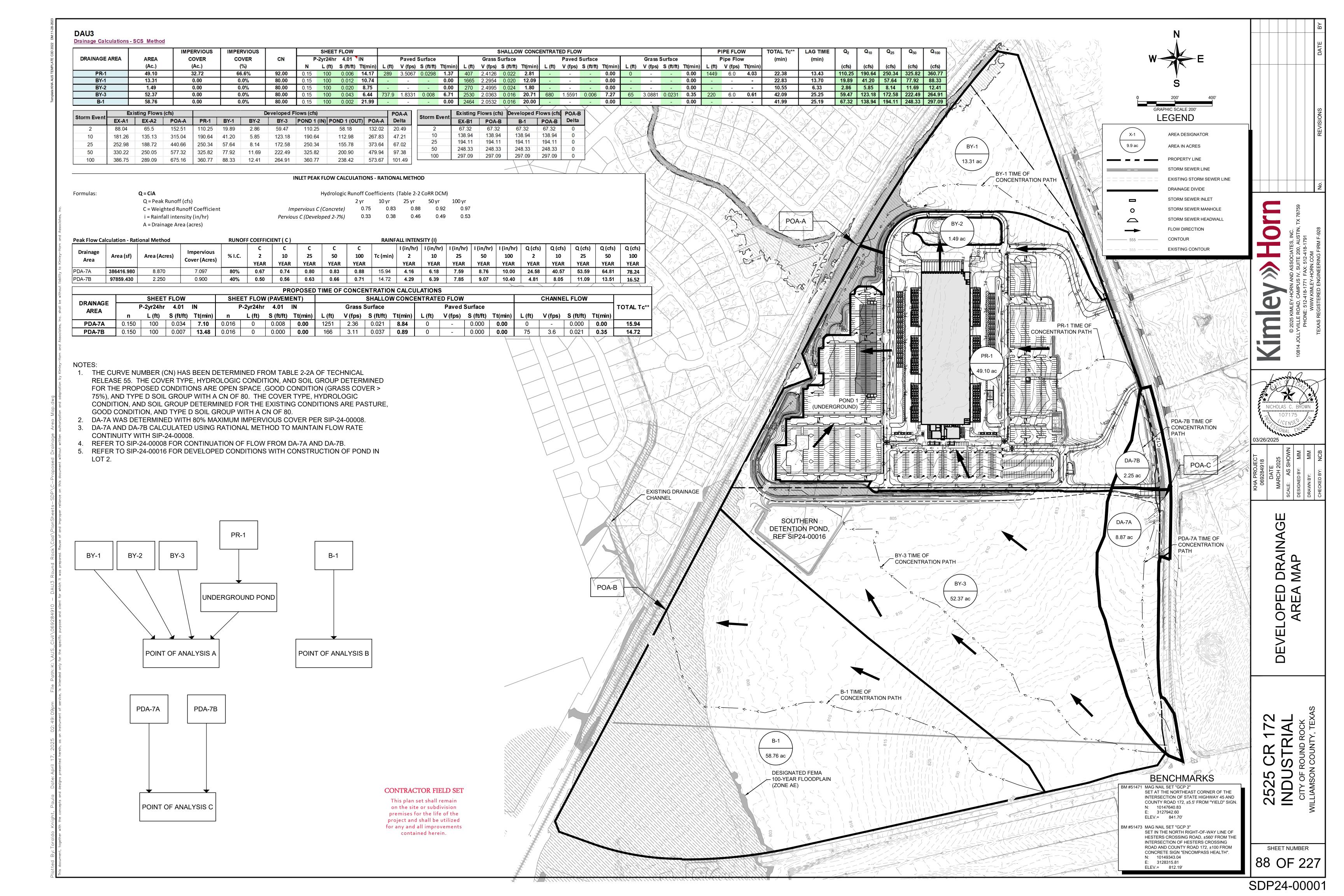
Inspection and Maintenance Guidelines:

- (1) Inspection should be made weekly and after each rainfall. Repair or replacement should be made promptly as needed by the contractor.
- (2) Remove sediment when buildup reaches a depth of 3 inches. Removed sediment should be deposited in a suitable area and in such a manner that it will not erode.
- (3) Check placement of device to prevent gaps between device and curb.
- (4) Inspect filter fabric and patch or replace if torn or missing.
- (5) Structures should be removed and the area stabilized only after the remaining drainage area has been properly stabilized.

# **DRAINAGE AREA MAP**

PR-A is greater than 10 acres and will be disturbed. An existing and proposed drainage area map is provided on the next page.





# TEMPORARY SEDIMENT POND(S) PLANS AND CALCULATIONS

(NOT APPLICABLE)

Responsible Party: Amazon.com Services. LLC

### INSPECTION AND MAINTENANCE FOR BMPS

The inspection and maintenance plan outlines the procedures necessary to maintain the performance of the Permanent Best Management Practices for this project. It should be noted that the plan provides guidelines that may have to be adjusted dependent on site specific and weather-related conditions.

It is the responsibility of the owner to provide the inspections and maintenance as outlined in the plan for the duration of the project. The owner will maintain this responsibility until it is assumed or transferred to another entity in writing. If the property is leased or sold, the responsibility for the maintenance will be required to be transferred through the lease agreement, binding covenants, closing documents, or other binding legal instrument.

Disposal of accumulated silt shall be accomplished following Texas Commission on Environmental Quality guidelines and specifications.

Maintenance records shall be kept on the installation, maintenance, or removal of items necessary for the proper operation of the facilities. All inspections shall be documented in the Inspection Frequency Log, attached.

An amended copy of this document will be provided to the Texas Commission on Environmental Quality within thirty (30) days of any changes in the following information.

Mailing Address:	101 Platform Way N				
City, State:	Nashville, TN		Zip: <u>37203</u>		
Telephone:	<u>(901) 438-4156</u>	Fax:			
Maintenance Plan acknowledge that	re read and understa for the proposed Pe I will maintain respon ility is transferred to c	ermanent Best I sibility for the in or assumed by a	Management P nplementation a nother party in	ractices for my proje and execution of the	ect. I plan
Signature of Respo	onsible Party	Barnett	by David Barnett armettu@mazon.com, JU=Amazon, CN=David 5 14:13:24-05'00'	Date	
	Plan is based on Ci ntal Criteria Manual.	ty of Round Ro	ock Standard S	Specifications and Cit	ty of
By: Nicholas	C. Brown, P.E.	Date:	5/15/2024		

### Personnel Responsible for Inspections

The agent that performs the inspections should be knowledgeable of this general permit and familiar with the construction site. The contractor is to provide an inspector with a CPESC, CESSWI, or CISEC certification. Documentation of the inspector's qualifications is to be included in the attached Inspector Qualifications Log.

### **Inspection Schedule**

The primary operator is required to choose one of the two inspections listed below.

Option 1: Once every seven calendar days. If this alternative schedule is developed,
then the inspection must occur regardless of whether or not there has been a rainfall event
since the previous inspection.
Option 2: Once every 14 calendar days and within 24 hours of the end of a storm event
of two inches or greater.

The inspections may occur on either schedule provided that documentation reflects the current schedule and that any changes to the schedule are conducted in accordance with the following provisions: the schedule may be changed a maximum of one time each month, the schedule change must be implemented at the beginning of a calendar month, and the reason for the schedule change must be documented (e.g., end of "dry" season and beginning of "wet" season).

If option 2 is the chosen frequency of inspections a rain gauge must be properly maintained on site or the storm event information from a weather station that is representative of the site location. For any day of rainfall during normal business hours that measures 0.25 inches or greater, proper documentation of the total rainfall measured for that day must be recorded. Personnel provided by the permittee must inspect:

- disturbed areas of the construction site that have not been finally stabilized;
- areas used for storage of materials that are exposed to precipitation:
- structural controls (for evidence of, or the potential for, pollutants entering the drainage system);
- sediment and erosion control measures identified in the SWP3 (to ensure they are operating correctly); and
- locations where vehicles enter or exit the site (for evidence of off-site sediment tracking).

### **Reductions in Inspection Frequency**

Where sites have been finally or temporarily stabilized or where runoff is unlikely due to winter conditions (e.g. site is covered with snow, ice, or frozen ground exists), inspections must be conducted at least once every month. In arid, semi-arid, or drought-stricken areas, inspections must be conducted at least once every month and within 24 hours after the end of a storm event of 0.5 inches or greater. A record of the total rainfall measured, as well as the approximate beginning and ending dates of winter or drought conditions resulting in monthly frequency of inspections in the attached Rain Gauge Log.

In the event of flooding or other uncontrollable situations which prohibit access to the inspection sites, inspections must be conducted as soon as access is practicable.

### **Inspection Report Forms**

Use the Inspection Report Forms given as a checklist to ensure that all required areas of the construction site are addressed. There is space to document the inspector's name as well as when the inspections regularly take place. The tables will document that the required area was inspected. (If there were any areas of concern, briefly describe them in this space with a more detailed description in the narrative section. Use the last table to document any discharges found during the inspections).

Describe how effective the installed BMPs are performing. Describe any BMP failures that were noted during the investigation and describe any maintenance required due to the failure. If new BMPs are needed as the construction site changes, the inspector can use the space at the bottom of the section to list BMPs to be implemented before the next inspection.

Describe the inspector's qualifications, how the inspection was conducted, and describe any areas of non-compliance in detail. If an inspection report does not identify any incidents of non-compliance, then it must contain a certifying signature stating that the facility or site is in compliance. The report must be signed by a person and in a manner required by 30 TAC 305.128. There is space at the end of the form to allow for this certifying signature.

Whenever an inspection shows that BMP modifications are needed to better control pollutants in runoff, the changes must be completed within seven calendar days following the inspection. If existing BMPs are modified or if additional BMPs are needed, you must describe your implementation schedule, and wherever possible, make the required BMP changes before the next storm event.

The Inspection Report Form functions as the required report and must be signed in accordance with TCEQ rules at 30 TAC 305.128.

### Corrective Action

Personnel Responsible for Corrective Actions

Both Primary and Secondary Operators are responsible for maintaining all necessary Corrective Actions. If an individual is specifically identified as the responsible party for modifying the contact information for that individual should be documented in the attached Inspector Qualifications Log.

### **Corrective Action Forms**

The Temporary BMPs must be modified based on the results of inspections, as necessary, to better control pollutants in runoff. Revisions must be completed within seven (7) calendar days following the inspection. If existing BMPs are modified or if additional BMPs are necessary, an implementation schedule must be described in the attached forms and wherever possible those changes implemented before the next storm event. If implementation before the next anticipated storm event is impracticable, these changes must be implemented as soon as practicable. Actions taken as a result of inspections must be properly documented by completing the corrective action forms given.



## Inspector Qualifications Log\*

Inspector Name: Qualifications (Check as appropriate and provide description):  □ Training Course □ Supervised Experience □ Other
Inspector Name: Qualifications (Check as appropriate and provide description):  □ Training Course □ Supervised Experience □ Other
Inspector Name: Qualifications (Check as appropriate and provide description):  □ Training Course □ Supervised Experience □ Other
Inspector Name: Qualifications (Check as appropriate and provide description):  □ Training Course □ Supervised Experience □ Other
Inspector Name: Qualifications (Check as appropriate and provide description):  □ Training Course □ Supervised Experience □ Other
Inspector Name: Qualifications (Check as appropriate and provide description):  □ Training Course □ Supervised Experience □ Other

<sup>\*</sup> The agent that performs the inspections should be knowledgeable of this general permit and familiar with the construction site. The contractor is to provide an inspector with a CPESC, CESSWI, or CISEC certification.



## Amendment Log

No.	Description of the Amendment	Date of Amendment	Amendment Prepared by [Name(s) and Title]



## Construction Activity Sequence Log

Name of Operator	Projected dates Month/year	Activity Disturbing Soil clearing, excavation, etc.	Location on-site where activity will be conducted	Acreage being disturbed

<sup>\*</sup>Construction activity sequences for linear projects may be conducted on a rolling basis. As a result, construction activities may be at different stages at different locations in the project area. The Contractor is required to complete and update the schedule and adjust as necessary.



## Stormwater Control Installation and Removal Log

Stormwater Control	Location On-Site	Installation Date	Removal Date



### Stabilization Activities Log

Date Activity Initiated	Description of Activity	Description of Stabilization Measure and Location	Date Activity Ceased (Indicate Temporary or Permanent)	Date When Stabilization Measures Initiated

Stabilization and erosion control practices may include, but are not limited to: establishing temporary or permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, and protecting existing trees and vegetation. List practices used where they are located, when they will be implemented, and whether they are temporary (interim) or permanent.



## Inspection Frequency Log

Date	Frequency Schedule and Reason for Change



## Rain Gauge Log

Date	Location of Rain Gauge	Gauge Reading



General Information						
Name of Project			Tracking No.		Inspection Date	
Inspector Name Contact Information	, Title &					
Present Phase of Co	nstruction					
Inspection Location inspections are required location where this being conducted)	ıired, specify	/				
Inspection Frequency Standard Frequency:						
Was this inspection triggered by a 0.25" storm event? ☐ Yes ☐ No If yes, how did you determined whether a 0.25" storm event has occurred? ☐ Rain gauge on site ☐ Weather station representative of site. Specify weather station source:  Total rainfall amount that triggered the inspection (in inches):						
If "yes", con	ne that any ponplete the fol	ortion of your site was unsafe for inspection				
- Location(s) where conditions were found:						



Condition and Effectiveness of Erosion and Sediment (E&S) Controls						
Type/Location of E&S Control	Repairs or Other Maintenance Needed?	Corrective Action Required?	Date on Which Maintenance or Corrective Action First Identified?	Notes		
1.	□Yes □No	□Yes □No				
2.	□Yes □No	□Yes □No				
3.	□Yes □No	□Yes □No				
4.	□Yes □No	□Yes □No				
5.	□Yes □No	□Yes □No				
6.	□Yes □No	□Yes □No				
7.	□Yes □No	□Yes □No				
8.	□Yes □No	□Yes □No				
9.	□Yes □No	□Yes □No				
10.	□Yes □No	□Yes □No				



Condition and Effectiveness of Pollution Prevention (P2) Practices						
Type/Location of P2 Practices	Repairs or Other Maintenance Needed?	Corrective Action Required?	Identification Date	Notes		
1.	□Yes □No	□Yes □No				
2.	□Yes □No	□Yes □No				
3.	□Yes □No	□Yes □No				
4.	□Yes □No	□Yes □No				
5.	□Yes □No	□Yes □No				
6.	□Yes □No	□Yes □No				
7.	□Yes □No	□Yes □No				
8.	□Yes □No	□Yes □No				
9.	□Yes □No	□Yes □No				
10.	□Yes □No	□Yes □No				



Stabilization of Exposed Soil				
Stabilization Area	Stabilization Method	Have You Initiated Stabilization?	Notes	
1.		☐ YES ☐ NO If yes, provide date:		
2.		☐ YES ☐ NO If yes, provide date:		
3.		☐ YES ☐ NO If yes, provide date:		
4.		☐ YES ☐ NO If yes, provide date:		
5.		☐ YES ☐ NO If yes, provide date:		
	Description of I	Discharges		
Was a stormwater discharge or other discharge occurring from any part of your site at the time of the inspection?   Yes No If "yes", provide the following information for each point of discharge:				
Discharge Location	Observations			
1.	Describe the discharge:			
	At points of discharge and the channels and banks of surface waters in the immediate vicinity, are there any visible signs of erosion and/or sediment accumulation that can be attributed to your discharge?   Yes  No If yes, describe what you see, specify the location(s) where these conditions were found, and indicate whether modification, maintenance, or corrective action is needed to resolve the issue:			
2.	Describe the discharge:			
	At points of discharge and the channels and banks of surface waters in the immediate vicinity, are there any visible signs of erosion and/or sediment accumulation that can be attributed to your discharge?   Yes  No If yes, describe what you see, specify the location(s) where these conditions were found, and indicate whether modification, maintenance, or corrective action is needed to resolve the issue:			
3.	Describe the discharge:			
	At points of discharge and the channels and banks of surface waters in the immediate vicinity, are there any visible signs of erosion and/or sediment accumulation that can be attributed to your discharge?   Yes No If yes, describe what you see, specify the location(s) where these conditions were found, and indicate whether modification, maintenance, or corrective action is needed to resolve the issue:			



Contractor or Subcontractor Certification and Signature		
"I certify under penalty of law that this document and all attachments were prepared under my directive assure that qualified personnel properly gathered and evaluated the information submitted. Based system, or those persons directly responsible for gathering the information, the information submaccurate, and complete. I am aware that there are significant penalties for submitting false information knowing violations."	on my inquiry of the person or persons who manage the nitted is, to the best of my knowledge and belief, true,	
Signature of Contractor or Subcontractor:	Date:	
Printed Name and Affiliation:		
Certification and Signature by Permittee	e	
"I certify under penalty of law that this document and all attachments were prepared under my directive assure that qualified personnel properly gathered and evaluated the information submitted. Based system, or those persons directly responsible for gathering the information, the information submaccurate, and complete. I am aware that there are significant penalties for submitting false information knowing violations."	on my inquiry of the person or persons who manage the nitted is, to the best of my knowledge and belief, true,	
Signature of Permittee or "Duly Authorized Representative":	Date:	
Printed Name and Affiliation:		



Section A – Initial Report  (Complete this section <u>within 24 hours</u> of discovering the condition that triggered corrective action)					
Name of Project	me of Project Tracking No.			Today's Date	
Date Problem First Disco	vered		Time Problem Firs	t Discovered	
Name and Contact Inform Form	nation of Individual Completing this				
What site conditions triggered the requirement to conduct corrective action:  A required stormwater control was never installed, was installed incorrectly, or not in accordance with the requirements in Part 2 and/or 3  The stormwater controls that have been installed and maintained are not effective enough for the discharge to meet applicable water quality standards  A prohibited discharge has occurred or is occurring					
Provide a description of t	he problem:				
	corrective action ( <i>Enter date that is eit</i> ork within the first 7 days, enter the da				e problem, or (2) if it is
If your estimated date of date you have established	completion falls after the 7-day deadlir I for making the new or modified storm	ne, explain (1) w nwater control d	why you believe it is in the soc	nfeasible to complete work within 7 onest practicable timeframe:	days, and (2) why the
	Section (Complete this section <u>no later than 7 c</u>	on B – Correc alendar days afte	ctive Action Progr r discovering the condi	ess tion that triggered corrective action)	
Section B.1 – Why the					
	d an additional sheet if necessary)			ermined and the Date You Determin	ned the Cause
1.					
2.			2.		
3.			3.		
Section B.2 – Stormwater Control Modifications to be Implemented to Correct the Problem					
List of Stormwater Contro Problem (Add an addition	ol Modification(s) Needed to Correct nal sheet if necessary)	Completion Date	Plan Updates Necessary?	Notes	
1.			□Yes □No Date:		
2.			☐Yes ☐No Date:		
3.			☐Yes ☐No Date:		
		•	-		

 $Section \ A-Initial \ Report \\ (Complete this section \ \underline{within 24 hours} \ of \ discovering \ the \ condition \ that \ triggered \ corrective \ action)$ 



Name of Project	Tracking	No.		Today's Date		
Date Problem First Discovered		Time Problem First Discovered				
Name and Contact Inform Form	nation of Individual Completing this					
What site conditions triggered the requirement to conduct corrective action:  A required stormwater control was never installed, was installed incorrectly, or not in accordance with the requirements in Part 2 and/or 3  The stormwater controls that have been installed and maintained are not effective enough for the discharge to meet applicable water quality standards  A prohibited discharge has occurred or is occurring						
Provide a description of t	he problem:					
	corrective action ( <i>Enter date that is el</i> ork within the first 7 days, enter the d				I the problem, or (2) if it is	
If your estimated date of date you have established	completion falls after the 7-day deadli I for making the new or modified stori	ne, explain (1) v nwater control (	vhy you believe it is i operational is the soc	nfeasible to complete work withir onest practicable timeframe:	n 7 days, and (2) why the	
	Section B — Corrective Action Progress (Complete this section <u>no later than 7 calendar days</u> after discovering the condition that triggered corrective action)					
Section B.1 – Why the Problem Occurred						
Cause(s) of Problem (Add an additional sheet if necessary)  How This Was Determined and the Date You Determined the Cause						
1.						
2.	2.					
3.			3.			
Section B.2 – Stormw	ater Control Modifications to be	Implemented	I to Correct the Pr	oblem		
List of Stormwater Control Modification(s) Needed to Correct		Completion Date	Plan Updates Necessary?	Notes		
1.			□Yes □No Date:			
2.			☐Yes ☐No Date:			
3.	3.		☐Yes ☐No Date:			

**Contractor or Subcontractor Certification and Signature** 



upervision in accordance with a system designed nquiry of the person or persons who manage the to the best of my knowledge and belief, true, uding the possibility of fine and imprisonment for
Date:
upervision in accordance with a system designed inquiry of the person or persons who manage the to the best of my knowledge and belief, true, adding the possibility of fine and imprisonment for
r

## SCHEDULE OF INTERIM AND PERMANENT SOIL STABILIZATION PRACTICES

Construction practices shall disturb the minimal amount of existing ground cover as required for land clearing, grading, and construction activity for the shortest amount of time possible to minimize the potential of erosion and sedimentation from the site. Existing vegetation shall be maintained and left in place until it is necessary to disturb for construction activity. For this project the following stabilization practices will be implemented:

- 1. Hydraulic Mulch and Seeding: Disturbed areas subject to erosion shall be stabilized with hydraulic mulch and/or seeded and watered to provide interim stabilization. For areas that are not to be sodded as per the project landscaping plan, vegetative cover will be established to provide permanent stabilization.
- 2. Sodding and Wood Mulch: As per the project landscaping plan, sodding and wood mulch will be applied to landscaped areas to provide permanent stabilization prior to project completion.

Records of the following shall be maintained:

- a) The dates when major grading activities occur;
- b) The dates when construction activities temporarily or permanently cease on a portion of the site; and
- c) The dates when stabilization measures are initiated.

Stabilization measures must be initiated as soon as practical in portions of the site where construction activities have temporarily or permanently ceased, and except as provided in the following, must be initiated no more that fourteen (14) days after the construction activity in that portion of the site has temporarily or permanently ceased:

Where the initiation of stabilization measures by the 14<sup>th</sup> day after construction activity temporarily or permanently ceased is precluded by snow cover or frozen ground conditions, stabilization measures must be initiated as soon as practical.

Where construction activity on a portion of the site is temporarily ceased and earth disturbing activities will be resumed within twenty-one (21) days, temporary stabilization measures do not have to be initiated on that portion of the site.

In arid areas (areas with an average rainfall of 0-10 inches), semiarid areas (areas with an average annual rainfall of 10 to 20 inches), and areas experiencing droughts where the initiation of stabilization measures by the 14<sup>th</sup> day after construction activity has temporarily or permanently ceased is precluded by seasonably arid conditions, stabilization measures must be initiated as soon as practical.

#### Maintenance

Below are some maintenance practices to be used to maintain erosion and sediment controls:

## 069284918 – 2525 CR 172 INDUSTRIAL ORGANIZED SEWAGE COLLECTION SYSTEM

- All measures will be maintained in good working order. The operator should correct any damage or deficiencies as soon as practicable after the inspection, but in no case later than seven (7) calendar days after the inspection.
- BMP Maintenance (as applicable)
- Sediment must be removed from sediment traps and sedimentation ponds no later than the time that design capacity has been reduced by 50%. For perimeter controls such as silt fences, berms, etc., the trapped sediment must be removed before it reaches 50% of the above-ground height.
- Silt fence will be inspected for depth of sediment, tears, to see of the fabric is securely attached to the fence posts, and to see that the fence posts are firmly in the ground.
- Drainage swale will be inspected and repaired as necessary.
- Inlet control will be inspected and repaired as necessary.
- Check dam will be inspected and repaired as necessary.
- Straw bale dike will be inspected and repaired as necessary.
- Diversion dike will be inspected and any breaches promptly repaired.
- Temporary and permanent seeding and planting will be inspected for bare spots, washouts, and healthy growth.
- If sediment escapes the site, accumulations must be removed at a frequency that minimizes off-site impacts, and prior to the next rain event, if feasible. If the permittee does not own or operate the off-site conveyance, then the permittee must to work with the owner or operator of the property to remove the sediment.
- Locations where vehicles enter or exit the site must be inspected for evidence of offsite sediment tracking.

To maintain the above practices, the following will be performed:

 Maintenance and repairs will be conducted before the next anticipated storm event or as necessary to maintain the continued effectiveness of storm water controls.
 Following an inspection, deficiencies should be corrected no later than seven (7) calendar days after the inspection.

Kimley»Horn

# SECTION 8: ADDITIONAL FORMS

## **AGENT AUTHORIZATION FORM (TCEQ-0599)**

### Agent Authorization Form

For Required Signature
Edwards Aquifer Protection Program
Relating to 30 TAC Chapter 213
Effective June 1, 1999

I	David Barnett	
	Print Name	
	Senior Pre-Construction Manager	
	Title - Owner/President/Other	
of	Amazon.com Services, LLC	
	Corporation/Partnership/Entity Name	
have authorized	Nicholas C. Brown	
	Print Name of Agent/Engineer	
of	Kimley-Horn and Associates	
	Print Name of Firm	

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

### I also understand that:

- 1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
- 2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
- 3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
- 4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
- 5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

### SIGNATURE PAGE:

Applicant's Signature Date

THE STATE OF Tenessees

County of Shelby §

BEFORE ME, the undersigned authority, on this day personally appeared <u>David Barnett</u> known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this 25th day of April , 2014.

STATE OF TENNESSEE NOTARY PUBLIC

NOTARY PUBLIC

Elizabeth Garantee Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 17/02/7025

## **APPLICATION FEE FORM (TCEQ-0574)**

## **Application Fee Form**

### **Texas Commission on Environmental Quality** Name of Proposed Regulated Entity: 2525 CR 172 Industrial Regulated Entity Location: 2525 CR 172, Round Rock, TX 78681 Name of Customer: David Barnett Phone: 512-418-1771 Contact Person: Nicholas C. Brown Customer Reference Number (if issued):CN 605619519 Regulated Entity Reference Number (if issued):RN N/A **Austin Regional Office (3373)** Hays Travis X Williamson San Antonio Regional Office (3362) Medina Uvalde Bexar Comal Kinney Application fees must be paid by check, certified check, or money order, payable to the Texas Commission on Environmental Quality. Your canceled check will serve as your receipt. This form must be submitted with your fee payment. This payment is being submitted to: X Austin Regional Office San Antonio Regional Office Mailed to: TCEQ - Cashier Overnight Delivery to: TCEQ - Cashier **Revenues Section** 12100 Park 35 Circle Mail Code 214 Building A, 3rd Floor P.O. Box 13088 Austin, TX 78753 Austin, TX 78711-3088 (512)239-0357 Site Location (Check All That Apply): X Recharge Zone Contributing Zone **Transition Zone**

Type of Plan	Size	Fee Due
Water Pollution Abatement Plan, Contributing Zone		
Plan: One Single Family Residential Dwelling	0 Acres	\$ 0
Water Pollution Abatement Plan, Contributing Zone		
Plan: Multiple Single Family Residential and Parks	0 Acres	\$ 0
Water Pollution Abatement Plan, Contributing Zone		
Plan: Non-residential	0 Acres	\$ 0
Sewage Collection System	494 L.F.	\$ 650
Lift Stations without sewer lines	0 Acres	\$ 0
Underground or Aboveground Storage Tank Facility	0 Tanks	\$ 0
Piping System(s)(only)	0 Each	\$ 0
Exception	0 Each	\$ 0
Extension of Time	0 Each	\$ 0

Signature:	July		Date:	05/15/2025
	- 1		='	

-1 · DR

### **Application Fee Schedule**

**Texas Commission on Environmental Quality** 

Edwards Aquifer Protection Program 30 TAC Chapter 213 (effective 05/01/2008)

### Water Pollution Abatement Plans and Modifications

Contributing Zone Plans and Modifications

	Project Area in	_
Project	Acres	Fee
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5	\$1,500
	5 < 10	\$3,000
	10 < 40	\$4,000
	40 < 100	\$6,500
	100 < 500	\$8,000
	≥ 500	\$10,000
Non-residential (Commercial, industrial, institutional,	< 1	\$3,000
multi-family residential, schools, and other sites	1 < 5	\$4,000
where regulated activities will occur)	5 < 10	\$5,000
	10 < 40	\$6,500
	40 < 100	\$8,000
	≥ 100	\$10,000

Organized Sewage Collection Systems and Modifications

Project	Cost per Linear Foot	Minimum Fee- Maximum Fee
Sewage Collection Systems	\$0.50	\$650 - \$6,500

## Underground and Aboveground Storage Tank System Facility Plans and Modifications

Project	Cost per Tank or Piping System	Minimum Fee- Maximum Fee
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500

**Exception Requests** 

Project	Fee	
Exception Request	\$500	

Extension of Time Requests

Project	Fee
Extension of Time Request	\$150

# CHECK PAYABLE TO THE "TEXAS COMMISSION ON ENVIRONMENTAL QUALITY"

## **CORE DATA FORM (TCEQ-10400)**



## **TCEQ Core Data Form**

For detailed instructions on completing this form, please read the Core Data Form Instructions or call 512-239-5175.

### **SECTION I: General Information**

1. Reason for	Submissi	<b>on</b> (If other is checked	l please describe	in space pr	rovided.)						
New Perr     New Perr	nit, Registra	ation or Authorization	(Core Data Forn	should be	submitted	d with the p	rogram a <sub>l</sub>	pplication.)			
Renewal (Core Data Form should be submitted with the renewal form)							Other				
2. Customer	er Reference Number (if issued)  Follow this link to search						Regulate	d Entity R	eference	Number (if	issued)
CN	for CN or RN numbers in Central Registry**						N				
SECTIO	N II:	Customer	Inform	ation	1						
4. General Cu	4. General Customer Information 5. Effective Date for Customer Information							tes (mm/do	d/yyyy)		
New Custo	mer		  pdate to Custon	ner Informa	ation		hange in F	Regulated E	ntity Own	ership	
Change in L	egal Name	(Verifiable with the Te	xas Secretary of	State or Te	xas Comp	troller of Pu	ıblic Acco	unts)			
The Custome	r Name sı	ıbmitted here may	be updated au	tomatical	lly based	on what i	s current	and activ	e with th	ne Texas Sec	retary of State
(SOS) or Texa	s Comptr	oller of Public Acco	unts (CPA).								
6. Customer	Legal Nan	ne (If an individual, pri	nt last name firs	t: eg: Doe, J	John)		<u>If ne</u>	w Customer	, enter pre	evious Custom	er below:
	Amazon.com Services LLC										
7. TX SOS/CP	A Filing N	umber	8. TX State T	ax ID (11 d	digits)		9. Fe	9. Federal Tax ID 10. DUNS Number			Number (if
0802761221			32054385284				(9 digits)				
						820544687					
						1	0203				
11. Type of C	ustomer:	☐ Corpora	tion			☐ Indi	vidual	dual Partnership: General Limited			
Government: [	City 🔲	County 🔲 Federal 🔲	Local   State	Other		Sole	e Proprietorship				
12. Number	of Employ	ees				l	13.	ndepende	ntly Ow	ned and Ope	erated?
0-20	□ 0-20 □ 21-100 □ 101-250 □ 251-500 ☑ 501 and higher							☐ Yes ☐ No			
14. Custome	r Role (Pro	posed or Actual) – as i	it relates to the I	Regulated E.	ntity listed	d on this for	m. Please	check one o	of the follo	owing	
Owner Operator Other:											
Occupational Licensee Responsible Party VCP/BSA Applicant											
David Barnett - Sr Pre-Construction Manager  15. Mailing											
	Amazon	Tower 1, 101 Platform	Way N.								
Address:	City	Nashville		State	TN	ZIP	3720	3		ZIP + 4	
16. Country I	Mailing In	 formation (if outside	USA)			 17. E-Mail	Address	(if applicat	ole)		
						barnettu@a	mazon.co	om			
18 Telephone Number 19 Extension or Code							20 Fax Number (if applicable)				

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( 901 ) 438-4156		( ) -
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### **SECTION III: Regulated Entity Information**

21. General Regulated Entity Information (If 'New Regulated Entity" is selected, a new permit application is also required.)										
New Regulated Entity ☐ Update to Regulated Entity Name ☐ Update to Regulated Entity Information										
The Regulated Entity Name submitted may be updated, in order to meet TCEQ Core Data Standards (removal of organizational endings such as Inc, LP, or LLC).										
22. Regulated Entity Name (Enter name of the site where the regulated action is taking place.)										
2525 CR172 Industrial										
23. Street Address of the Regulated Entity:	2525 CR 172									
(No PO Boxes)				1	1	1		Ι	T	
1	City	Round Rock	State	TX	ZIP	78681		ZIP + 4		
24. County	Williamson									
		If no Stree	et Address is provid	led, fields	25-28 are re	quired.				
25. Description to										
Physical Location:										
26. Nearest City						State		Nea	rest ZIP Code	
Round Rock						TX		7858	31	
Latitude/Longitude are required and may be added/updated to meet TCEQ Core Data Standards. (Geocoding of the Physical Address may be used to supply coordinates where none have been provided or to gain accuracy).										
27. Latitude (N) In Decima	al:	30.484519		28. l	ongitude (V	V) In Decin	nal:	-97.69714	40	
Degrees	Minutes		Seconds	Degr	ees	Mi	nutes		Seconds	
29. Primary SIC Code	30.	Secondary SIC	Code	31. Prima	ry NAICS Co	de	32. Seco	ndary NAI	CS Code	
(4 digits)	(4 c	ligits)		<b>(</b> 5 or 6 dig	ts)		(5 or 6 di	gits)		
4225				23621						
33. What is the Primary Business of this entity? (Do not repeat the SIC or NAICS description.)										
warehouse distribution										
24 84-11	David Barnett - Sr Pre-Construction Manager									
34. Mailing	Amazon Tower 1, 101 Platform Way N									
Address:	'	ower 1, 101 Platt	•							
	City	Nashville	State	TN	ZIP	37203		ZIP + 4		
35. E-Mail Address:	City		State	TN	ZIP	37203		ZIP + 4		
35. E-Mail Address:  36. Telephone Number	City	Nashville	State			37203 ax Numbe	r (if applica			
	City	Nashville	State		38. F		r (if applica			

**39. TCEQ Programs and ID Numbers** Check all Programs and write in the permits/registration numbers that will be affected by the updates submitted on this form. See the Core Data Form instructions for additional guidance.

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☐ Dam Safety		Districts	Edwards Aquifer		Emissions Inventory Air	ir 🔲 Industrial Hazardous Was	
Municipal Solid Waste		New Source Review Air	OSSF	1	Petroleum Storage Tank	PWS	
Sludge		Storm Water	☐ Title V Air		Tires	Used Oil	
☐ Voluntary Clean			Wastewater Agrico	ultura	☐ Water Rights	Other:	
☐ Voluntary Cleanup		M wastewater					
ECTION 1	[V: Pr	<u>eparer Inf</u>	<u>ormation</u>				
0. Name: Nick Brown, PE			41. Title:	Senior Project Manager			
2. Telephone Nu	mber	43. Ext./Code	44. Fax Number	45. E-M	ail Address		
	512)418-1771			Nick.Brov			

**46.** By my signature below, I certify, to the best of my knowledge, that the information provided in this form is true and complete, and that I have signature authority to submit this form on behalf of the entity specified in Section II, Field 6 and/or as required for the updates to the ID numbers identified in field 39.

Company:	Amazon.com Services, LLC	nstruction Mar	nager	
Name (In Print):	David Barnett		Phone:	( 901 ) 438- <b>4156</b>
Signature:	Dal Burdt		Date:	4/25/24

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