Texas Commission on Environmental Quality

Edwards Aquifer Application Cover Page

Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with 30 TAC 213.

Administrative Review

- Edwards Aquifer applications must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.
 - To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: http://www.tceq.texas.gov/field/eapp.
- 2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
- 3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
- 4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.
 - An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.
- 5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
- 6. If the geologic assessment was completed before October 1, 2004 and the site contains "possibly sensitive" features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

Technical Review

- When an application is deemed administratively complete, the technical review period begins. The regional
 office will distribute copies of the application to the identified affected city, county, and groundwater
 conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days
 to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
- 2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

- clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.
- 3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited**.
- 4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

Mid-Review Modifications

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a "Mid-Review Modification". Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ's Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ's San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: Georgetown Intersection Improvements – Scenic Drive at SH 29							2. Regulated Entity No.:					
3. Customer Name: City of Georgeto						4. Cu	4. Customer No.:					
5. Project Type: (Please circle/check one)	New		Modification			Exter	sion	Exception				
6. Plan Type: (Please circle/check one)				EXP	EXT	Technical Clarification	Optional Enhanced Measures					
7. Land Use: (Please circle/check one)	Resider	ntial	Non-r	Non-residential			8. Sit	e (acres):				
9. Application Fee:	\$500		10. Permanent I			BMP(s	s):	StormSceptor				
11. SCS (Linear Ft.):	n/a		12. AS	ST/US	ST (No	o. Tar	ıks):	none				
13. County:	Willian	nson	14. W	aters	hed:			San Gabriel Watershed				

Application Distribution

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the "Texas Groundwater Conservation Districts within the EAPP Boundaries" map found at:

http://www.tceq.texas.gov/assets/public/compliance/field_ops/eapp/EAPP%20GWCD%20map.pdf

For more detailed boundaries, please contact the conservation district directly.

	Austin 1	Region					
County:	Hays	Travis	Williamson				
Original (1 req.)	_	_	_				
Region (1 req.)	_	_	_				
County(ies)	_	_	_				
Groundwater Conservation District(s)	Edwards Aquifer AuthorityBarton Springs/ Edwards AquiferHays TrinityPlum Creek	Barton Springs/ Edwards Aquifer	NA				
City(ies) Jurisdiction	AustinBudaDripping SpringsKyleMountain CitySan MarcosWimberleyWoodcreek	AustinBee CavePflugervilleRollingwoodRound RockSunset ValleyWest Lake Hills	AustinCedar ParkFlorenceGeorgetownJerrellLeanderLiberty HillPflugervilleRound Rock				

	San Antonio Region											
County:	Bexar	Comal	Kinney	Medina	Uvalde							
Original (1 req.)	_	_	_	_	_							
Region (1 req.)	_			_	_							
County(ies)			_									
Groundwater Conservation District(s)	Edwards Aquifer Authority Trinity-Glen Rose	Edwards Aquifer Authority	Kinney	EAA Medina	EAA Uvalde							
City(ies) Jurisdiction	Castle HillsFair Oaks RanchHelotesHill Country VillageHollywood ParkSan Antonio (SAWS)Shavano Park	BulverdeFair Oaks RanchGarden RidgeNew BraunfelsSchertz	NA	San Antonio ETJ (SAWS)	NA							

I certify that to the best of my knowledge, that the application is hereby submitted to TCEQ for adm		
Md Kamrul Islam, P.E.		
Print Name of Customer/Authorized Agent	5/9/25	
Signature of Customer/Authorized Agent	Date	

**FOR TCEQ INTERNAL USE ONLY	* *						
Date(s)Reviewed:	Date Adr	Date Administratively Complete:					
Received From:	Correct N	Number of Copies:					
Received By:	Distribut	tion Date:					
EAPP File Number:	Complex	:					
Admin. Review(s) (No.):	No. AR F	Rounds:					
Delinquent Fees (Y/N):	Review T	Time Spent:					
Lat./Long. Verified:	SOS Cus	tomer Verification:					
Agent Authorization Complete/Notarized (Y/N):	Fee	Payable to TCEQ (Y/N):					
Core Data Form Complete (Y/N):	Check:	Signed (Y/N):					
Core Data Form Incomplete Nos.:		Less than 90 days old (Y/N):					

General Information Form

Texas Commission on Environmental Quality

For Regulated Activities on the Edwards Aquifer Recharge and Transition Zones and Relating to 30 TAC §213.4(b) & §213.5(b)(2)(A), (B) Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **General Information Form** is hereby submitted for TCEQ review. The application was prepared by:

Pri	nt Name of Customer/Agent: Md Kamrul Islam, P.E.
Da	te: <u>05-09-2025</u>
	roject Information
1.	Regulated Entity Name: Georgetown Intersection Improvements – Scenic Drive at SH 29
2.	County: Williamson
3.	Stream Basin: <u>Brazos River Basin</u>
4.	Groundwater Conservation District (If applicable): N/A
5.	Edwards Aquifer Zone:
	Recharge Zone Transition Zone
6.	Plan Type:
	WPAPSCSModificationASTUSTException Request

7.	Customer (Applicant):	
	Contact Person: <u>Dustin Clarkson</u> Entity: <u>City of Georgetown</u> Mailing Address: <u>300-1 Industrial Ave.</u> City, State: <u>Georgetown, TX</u> Telephone: <u>512-930-8147</u> Email Address: <u>dustin.clarkson@georgetown.org</u>	Zip: <u>78626</u> FAX: <u>n/a</u>
8.	Agent/Representative (If any):	
	Contact Person: Md Kamrul Islam, P.E Entity: RPS Infrastructure, Inc. Mailing Address: 4801 Southwest Pkwy, Pkwy 1, Su City, State: Austin, Texas Telephone: 210-299-7912 Email Address: kamrul.islam@tetratech.com	<u>uite 185</u> Zip: <u>78735</u> FAX: <u>n/a</u>
9.	Project Location:	
	 ☐ The project site is located inside the city limits ☐ The project site is located outside the city limit jurisdiction) of ☐ The project site is not located within any city's 	s but inside the ETJ (extra-territorial
10.	The location of the project site is described bel detail and clarity so that the TCEQ's Regional st boundaries for a field investigation.	• •
	Scenic Drive at SH 29 intersection	
11.	Attachment A – Road Map. A road map showing project site is attached. The project location and the map.	_
12.	Attachment B - USGS / Edwards Recharge Zon USGS Quadrangle Map (Scale: 1" = 2000') of th The map(s) clearly show:	• • • • • • • • • • • • • • • • • • • •
	 ☑ Project site boundaries. ☑ USGS Quadrangle Name(s). ☑ Boundaries of the Recharge Zone (and Trance) ☑ Drainage path from the project site to the keep some content of the left of the le	
13.	The TCEQ must be able to inspect the project solution Sufficient survey staking is provided on the protect the boundaries and alignment of the regulated features noted in the Geologic Assessment.	ject to allow TCEQ regional staff to locate
	Survey staking will be completed by this date: I	March 29, 2024.

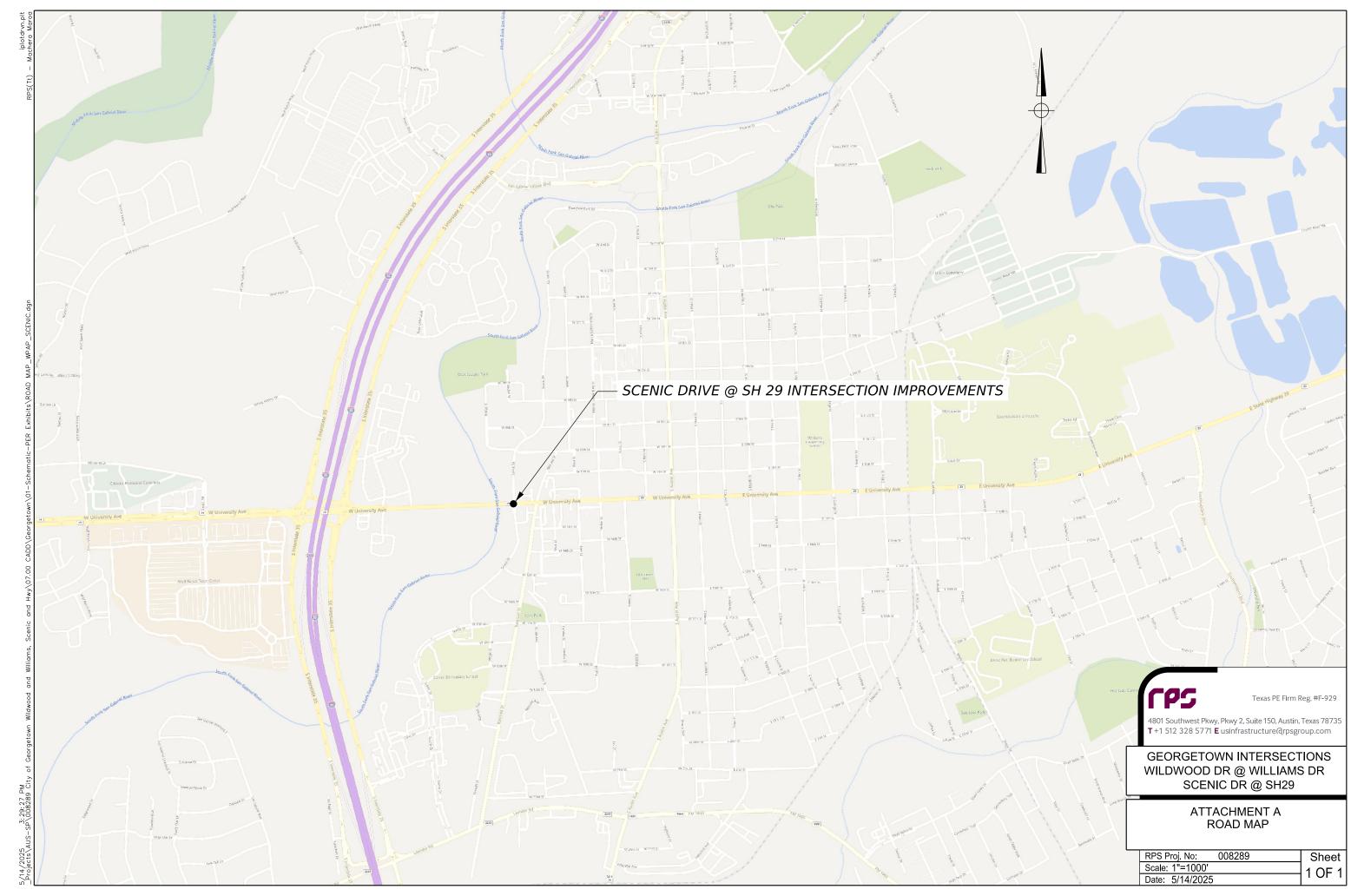
narrative of throughous Area of Site Site Site Site Site Site Site Site	vious cover nent BMP(s)
Site his	sed site use story us development) to be demolished
15. Existing project	ct site conditions are noted below:
Existin Existin Existin Undev	g commercial site g industrial site g residential site g paved and/or unpaved roads eloped (Cleared) eloped (Undisturbed/Uncleared)
Prohibited	Activities
	e that the following activities are prohibited on the Recharge Zone and are not for this project:
	disposal wells regulated under 30 TAC Chapter 331 of this title (relating to ground Injection Control);
(2) New fe	eedlot/concentrated animal feeding operations, as defined in 30 TAC §213.3;
(3) Land d	lisposal of Class I wastes, as defined in 30 TAC §335.1;
(4) The us	e of sewage holding tanks as parts of organized collection systems; and
standa	nunicipal solid waste landfill facilities required to meet and comply with Type I ards which are defined in §330.41(b), (c), and (d) of this title (relating to Types nicipal Solid Waste Facilities).
• •	nunicipal and industrial wastewater discharges into or adjacent to water in the hat would create additional pollutant loading.
	e that the following activities are prohibited on the Transition Zone and are sed for this project:
• •	disposal wells regulated under 30 TAC Chapter 331 (relating to Underground on Control);

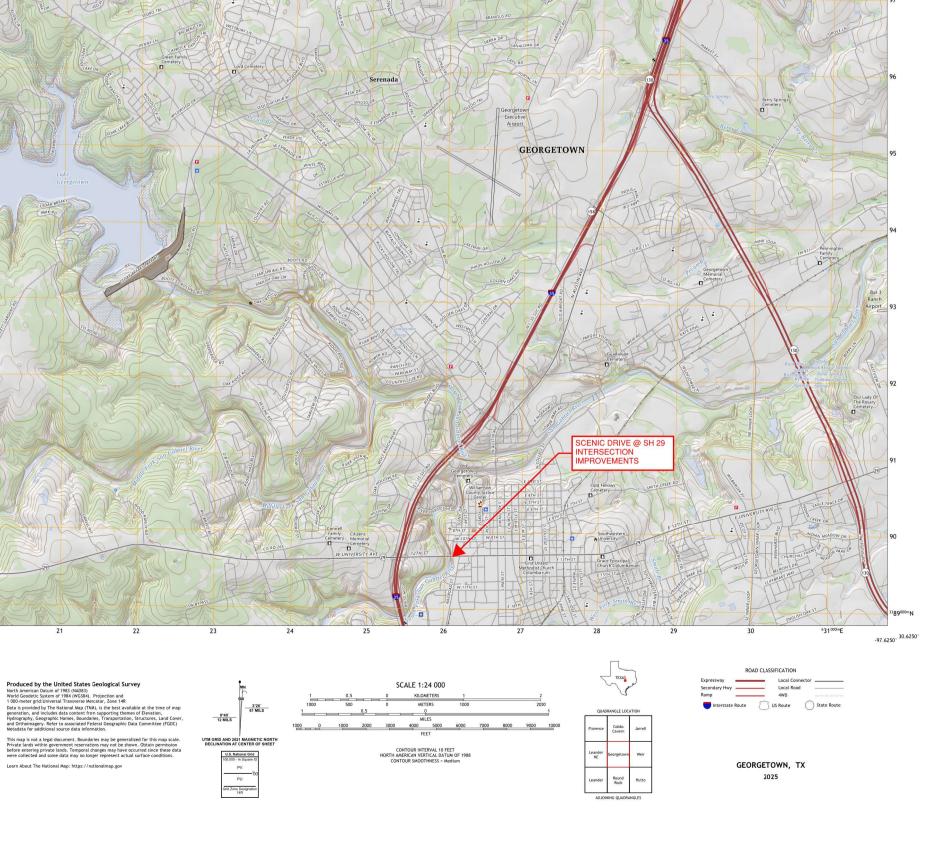
(2) Land disposal of Class I wastes, as defined in 30 TAC $\S 335.1$; and

(3) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41 (b), (c), and (d) of this title.

Administrative Information

18.	The	e fee for the plan(s) is based on:
		For a Water Pollution Abatement Plan or Modification, the total acreage of the site where regulated activities will occur. For an Organized Sewage Collection System Plan or Modification, the total linear footage of all collection system lines. For a UST Facility Plan or Modification or an AST Facility Plan or Modification, the total number of tanks or piping systems. A request for an exception to any substantive portion of the regulations related to the protection of water quality. A request for an extension to a previously approved plan.
19.		Application fees are due and payable at the time the application is filed. If the correct fee is not submitted, the TCEQ is not required to consider the application until the correct fee is submitted. Both the fee and the Edwards Aquifer Fee Form have been sent to the Commission's:
		 ☐ TCEQ cashier ☐ Austin Regional Office (for projects in Hays, Travis, and Williamson Counties) ☐ San Antonio Regional Office (for projects in Bexar, Comal, Kinney, Medina, and Uvalde Counties)
20.		Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
21.		No person shall commence any regulated activity until the Edwards Aquifer Protection Plan(s) for the activity has been filed with and approved by the Executive Director.





Attachment C - Project Description

Project Overview:

The proposed project involves intersection improvements at the intersection of Scenic Drive and SH 29. The primary objective is to enhance traffic flow and safety by adding northbound and southbound right-turn lanes. This project aims to facilitate smoother traffic movement and reduce congestion at this critical intersection.

Project Area:

The total project drainage area included in the plan is 1.83 acres. Of this area, 0.18 acres are attributed to off-site drainage, while the remaining 1.65 acres are on-site. The existing conditions indicate a predevelopment impervious area of 1.42 acres, which will increase to a total post-development impervious area of 1.48 acres, resulting in a 4.28% increase in impervious cover.

Existing Site Conditions:

The project site has been previously developed and currently includes two-lane roadways with sidewalks. The existing infrastructure is designed to accommodate traffic but requires enhancements to improve safety and efficiency. The project will involve minor modifications to the existing roadway layout to incorporate the new right-turn lanes.

Proposed Improvements:

The proposed improvements will include:

- Construction of northbound and southbound right-turn lanes on Scenic Drive at SH 29.
- Installation of curb inlet to collect stormwater runoff.
- Conveyance of collected stormwater through a pipe under Scenic Drive to a Stormceptor unit for treatment before discharge.

Permanent Best Management Practices (BMPs):

To ensure effective stormwater management and protect water quality, the project will implement a Stormceptor unit that his will treat stormwater runoff collected from the project site. The Stormceptor utilizes a combination of sedimentation and filtration to remove pollutants before the treated water is discharged into the existing storm sewer system. This system is designed to capture

and retain sediments, oils, and other contaminants, thereby improving the quality of runoff entering the aquifer.

Stormwater Management:

The project will maintain effective stormwater management practices to ensure that water quality is protected. The addition of the right-turn lanes will not significantly impact the overall drainage patterns, and the implementation of the BMPs will help manage runoff effectively.

Geologic Assessment

Texas Commission on Environmental Quality

For Regulated Activities on The Edwards Aquifer Recharge/transition Zones and Relating to 30 TAC §213.5(b)(3), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

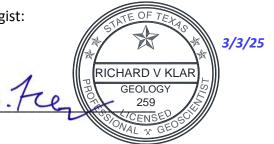
To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

Print Name of Geologist: Richard V. Klar, P.G. Telephone: 210-699-9090

Date: March 3, 2025 Fax: 210-699-6426

Representing: <u>Raba Kistner, Inc.</u>, TBPG Firm #50220 / TBPE Firm #3257 for RPS <u>Infrastructure</u>, Inc. on Behalf of the City of Georgetown (Name of Company and TBPG or TBPE registration number)

Signature of Geologist:



Regulated Entity Name: Georgetown Intersection Improvements – Scenic Drive at SH 29

Project Information

	1.	Date(s)	of Geo	logic Ass	essment	was per	formed:	<u>May</u>	<u>/ 18,</u>	<u>, 202:</u>
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2. Type of Project:

WPAP
SCS
SCS
UST

3. Location of Project:

Recharge Zone

Transition Zone

Contributing Zone within the Transition Zone

- 4. Attachment A Geologic Assessment Table. Completed Geologic Assessment Table (Form TCEQ-0585-Table) is attached.
- 5. Soil cover on the project site is summarized in the table below and uses the SCS Hydrologic Soil Groups* (Urban Hydrology for Small Watersheds, Technical Release No. 55, Appendix A, Soil Conservation Service, 1986). If there is more than one soil type on the project site, show each soil type on the Site Geologic Map or a separate soils map.

Table 1 - Soil Units, Infiltration Characteristics and Thickness

*Soil Group Definitions (Abbreviated)

- A. Soils having a high infiltration rate when thoroughly wetted.
- B. Soils having a moderate infiltration rate when thoroughly wetted.
- C. Soils having a slow infiltration rate when thoroughly wetted.
- D. Soils having a very slow infiltration rate when thoroughly wetted.
- Soil Name

 Group* Thickness (feet)

 Doss silty clay, 1 to 5 percent slopes (DoC)

 Georgetown stony clay loam, 1 to 3 percent slopes (GsB)
- 6. Attachment B Stratigraphic Column. A stratigraphic column showing formations, members, and thickness is attached. The outcropping unit, if present, should be at the top of the stratigraphic column. Otherwise, the uppermost unit should be at the top of the stratigraphic column.
- 7. Attachment C Project Geology. A narrative description of the site specific geology including any features identified in the Geologic Assessment Table, a discussion of the potential for fluid movement to the Edwards Aquifer, stratigraphy, structure(s), and karst characteristics is attached.
- 8. Attachment D Project Geologic Map(s). The Project Geologic Map must be the same scale as the applicant's Site Plan. The minimum scale is 1":400'.

Applicant's Site Plan Scale: 1" = <u>100</u>' Project Geologic Map Scale: 1" = 100'

Project Soils Map Scale (if more than 1 soil type): $1'' = \underline{60}'$

9.	Method of collecting positional data:
	☐ Global Positioning System (GPS) technology.☐ Other method(s). Please describe method of data collection:
10.	The project site boundaries are clearly shown and labeled on the Project Geologic Map.
11.	Surface geologic units are shown and labeled on the Project Geologic Map.
12.	Geologic or manmade features were discovered on the project site during the field investigation. They are shown and labeled on the Project Geologic Map and are described in the attached Geologic Assessment Table.
	Geologic or manmade features were not discovered on the project site during the field investigation.
13.	The Recharge Zone boundary is shown and labeled, if appropriate.
	All known wells (test holes, water, oil, unplugged, capped and/or abandoned, etc.): If applicable, the information must agree with Item No. 20 of the WPAP Application Section. There are _2_ test holes present on the project site and the locations are shown and labeled. (Check all of the following that apply.) The test holes are not in use and have been properly abandoned. The well is not in use and will be properly abandoned. The wells are in use and comply with 16 TAC Chapter 76. There are no wells or test holes of any kind known to exist on the project site.
Ac	dministrative Information
	Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

ATTACHMENTS

ATTACHMENT A

GEOLOGIC ASSESSMENT TABLE (TCEQ-0585-TABLE) COMMENTS TO GEOLOGIC ASSESSMENT TABLE SOIL PROFILE PROJECT SOILS MAP

GEOLOGIC ASSESSMENT TABLE						PROJ	ECT NAI	ME:						ents, Sceni exas (RKI P						
LOCATION FEATURE CHARACTERISTICS															EVAI	LUAT	ION	PHY	SICA	L SETTING
1A	1B *	1C*	2A	2B	3	4			5	5A	6	7	8A	8B	9	10		1	1	12
FEATURE ID	LATITUDE	LONGITUDE	FEATURE TYPE	POINTS	FORMATION	DIN	DIMENSIONS (FEET)		TREND (DEGREES)	DOM	DENSITY (NO/FT)	APERTURE (FEET)	INFILL	RELATIVE INFILTRATION RATE	TOTAL	SENSI	TIVITY	CATCHMENT AREA (ACRES)		TOPOGRAPHY
						X	Y	Z		10						<40	<u>>40</u>	<1.6	<u>>1.6</u>	
S-1	N30° 37' 59.71"	W97° 41' 3.08"	MB (W)	30	Ked	110	2.0-3.0	4.0-5.0					Х	6	36	✓		✓		Hilltop
S-2	N30° 38' 0.57"	W97° 41' 2.79"	MB (PTH, P-4)	30	Ked	0.5	0.5	10					Z	5	35	~		\		Hilltop
S-3	N30° 37' 58.13"	W97° 41' 2.53"	MB (PTH. P-5)	30	Ked	0.5	0.5	10				·	Z	5	35	✓		√		Hilltop

* DATUM: NAD83

Formation: Ked = Edwards Limestone Formation

Features: W = Potable Water line; PTH = Plugged Geotechnical Test Hole, test hole identifier

2A TYPE	TYPE	2B POINTS
С	Cave	30
SC	Solution cavity	20
SF	Solution-enlarged fracture(s)	20
F	Fault	20
0	Other natural bedrock features	5
MB	Manmade feature in bedrock	30
SW	Swallow hole	30
SH	Sinkhole	20
CD	Non-karst closed depression	5
Z	Zone, clustered or aligned features	30

	8A INFILLING
N	None, exposed bedrock
С	Coarse - cobbles, breakdown, sand, gravel
0	Loose or soft mud or soil, organics, leaves, sticks, dark colors
F	Fines, compacted clay-rich sediment, soil profile, gray or red colors
V	Vegetation. Give details in narrative description
FS	Flowstone, cements, cave deposits
Χ	Other materials: Granular bedding material for utility line (Feature S-1).
Z	Soil cuttings and bentonite for geotechnical test holes (Features S-2 and S-3).
	12 TOPOGRAPHY
Cliff,	Hilltop, Hillside, Drainage, Floodplain, Streambed

I have read, I understood, and I have followed the Texas Natural Resource Conservation Commission's Instructions to Geologists. The information presented here complies with that document and is a true representation of the conditions observed in the field.

My signature certifies that I am qualified as a geologist as defined by 30 TAC 213.

Date: *March 3, 2025*

Sheet <u>1</u> of <u>1</u>

TCEQ-0585-Table (Rev. 10-01-04)

Georgetown Intersection Improvements Scenic Drive at SH 29 Georgetown, Williamson County, Texas

The location of the following features are indicated on the **Project Geologic Map** provided as **Attachment D** to this report. The location of the existing potable water utility was taken from the Intersection Layout of the 100% Design Submittal prepared by RPS Group, dated October 3, 2024, review of the City of Georgetown online Geographic Information System (GIS) interactive maps for utility information, in addition to field observations.

Manmade Features in Bedrock (MB)

Feature S-1

Feature S-1 consists of a trench for an existing 6-inch potable water line owned by the City of Georgetown. The location of this utility was identified based on review of the design plan (RPS, 2024) and online interactive utility maps, https://georgetowntx.maps.arcgis.com/, in addition to field observations of paint markings and fire hydrants. On the basis of our observations and available utility information, it is inferred that the trench hosting the utility line is approximately 2-3 feet wide and installed to approximately 4-5 feet into the underlying Edwards Limestone (Ked) formation. The trench is located on the north side of State Highway (SH) 29 extending through its intersection with Scenic Drive, in addition along the SH 29 right-of-way. The length of the utility trench within the Project area is estimated on the order of 110 linear feet.



View to the south of a water utility valve along SH 29 on the southbound side of Scenic Drive.



View to the northwest of a fire hydrant along the northbound side of Scenic Drive, south of SH 29.

Features S-2 and S-3

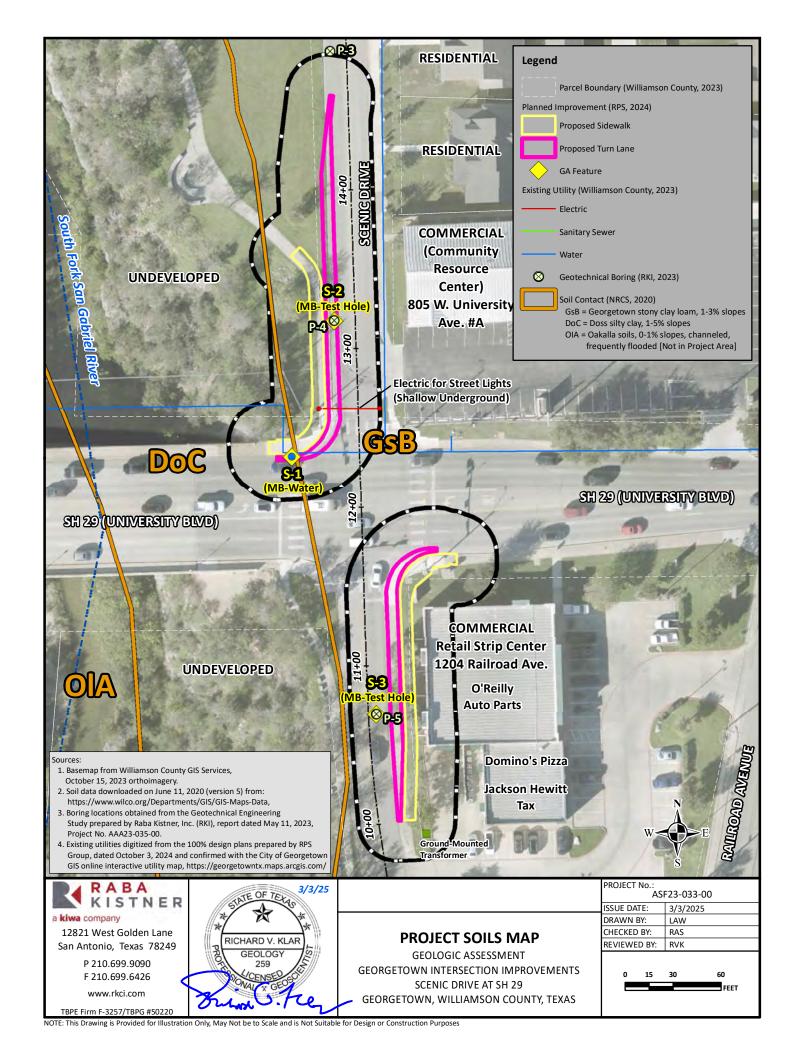
Features S-2 and S-3 consist of plugged geotechnical test holes installed by **Raba Kistner, Inc.** (**RKI**) on April 25, 2023 to evaluate soil conditions within the planned roadway improvements area. The borings were drilled to approximate depths of 10 feet below existing ground surface using a truck-mounted drilling rig. According to the geotechnical logging data, a dark brown to reddish-brown clay with weathered limestone fragments was encountered from 1 foot to approximately 3 feet underlain by tan limestone comprising the Ked. Shallow groundwater was not observed during drilling operations. Based on the referenced geotechnical report and observations in conjunction with field reconnaissance activities, the test holes were backfilled with soil cuttings and bentonite, and capped with a cold-mix patch to match the adjacent pavement surface following completion of drilling activities.

SOIL PROFILE Georgetown Intersection Improvements Scenic Drive at SH 29 Georgetown, Williamson County, Texas

SOIL SERIES	THICKNESS ON SITE	DESCRIPTION		
Doss	~0.8 to 2.5 feet	Doss silty clay, 1 to 5 percent slopes (DoC): Doss soils are complex comprising a shallow, gently sloping soil on convex slopes of low hills and ridges on uplands in the Edwards Plateau. The surface layer is typically a dark grayish brown silty clay approximately 9 inches thick. The subsoil is a yellowish brown clay loam extending to a depth of 18 inches. The underlying material is weakly cemented limestone and marl extending to a depth of 24 inches. The soil is moderately alkaline and calcareous throughout with a few noncalcareous surface layer area.		
Georgetown	~0.5 to 3 feet	Georgetown stony clay loam, 1 to 3 percent slopes (GsB): Georgetown soils are gently sloping found on the higher parts of uplands. The surface layer is a slightly acidic brown stony clay loam approximately 7 inches thick with few to common stones on or near the surface. The subsoil layer extends to a depth of approximately 35 inches and consists of a neutral, reddish brown cobbly clay. The underlying material is indurated fractured limestone with clay loam in the crevices and fractures.		

The preceding table was prepared based on *Soil Survey of Williamson County, Texas (January 1983)* in addition to field observations. As presented on the attached *Project Soils Map*, native soils mapped for the majority of the Project are classified as Georgetown stony clay loam, 1 to 3 percent slopes (GsB). A small portion of the Project along the northside of SH 29 at Scenic Drive is mapped as Doss silty clay, 1 to 5 percent slopes (DoC). The GsB and DoC soils form in clay occurring over hard, fractured limestone units of the Edwards Limestone. These soils are well drained, with medium surface runoff. Permeability is slow to moderately slow, ranging on the order of 0.06 to 0.6 inches per hour, which has the ability to impede rather than transmit fluids to the subsurface. Native soils were not directly observable as the Project area is a fully-developed urban corridor adjacent to Scenic Drive with sidewalks, pavements, and landscaping.

Test hole data reported in the *Geotechnical Engineering Study* prepared by **Raba Kistner, Inc.**, dated May 12, 2023 indicates a minimum of 1 foot of dark brown to reddish-brown clay soils overlying limestone within the Project area, which is generally consistent with the published soil information presented above.



ATTACHMENT B STRATIGRAPHIC COLUMN

STRATIGRAPHIC COLUMN Georgetown Intersection Improvements Scenic Drive at SH 29 Georgetown, Williamson County, Texas

STRATIGRAPHIC FORMATION THICKNESS		DESCRIPTION			
Edwards Limestone (Ked)	90-~300 feet	The Ked is comprised of massive to thin-bedded limestone, dolomitic limestone, and minor argillaceous limestone that have wackestone, packstone, and grainstone textures. The lower part of the Edwards Formation interfingers with the Comanche Peak and Walnut strata. Vuggy textures, collapse breccias, cavern systems, chert, and local rudistids are characteristic of this unit. Not exposed at the Project owing to roadway improvements, soil cover, and landscaping.			

Note: Stratigraphic Column adapted from the Geologic Map of the West Half of the Taylor, Texas, 30 x 60 Minute Quadrangle: Central Texas Urban Corridor, Encompassing Round Rock, Georgetown, Salado, Briggs, Liberty Hill, and Leander Sheet (Collins, 2005).

ATTACHMENT C NARRATIVE OF PROJECT SPECIFIC GEOLOGY

SITE GEOLOGY NARRATIVE Georgetown Intersection Improvements Scenic Drive at SH 29 Georgetown, Williamson County, Texas

Introduction

The following is a project-specific discussion of existing geological conditions and potential recharge features for the Edwards Aquifer identified within the area that will host planned roadway intersection improvements. The project site is located in Georgetown (Williamson County), Texas and consists of two areas located along Scenic Drive as follows: i) on the west side of Scenic Drive, approximately 250 feet long by 17 feet wide north of State Highway (SH) 29 (West University Avenue); and ii) on the east side of Scenic Drive, approximately 200 feet long by 16 feet wide south of SH 29. Based on the 100% Design Submittal prepared by RPS Group, dated October 3, 2024, the planned improvements will include extending the roadway for a right turn lane and widening the existing sidewalk along the southbound and northbound sides of Scenic Drive, just north and south of SH 29, respectively.

This assessment was performed by **Raba Kistner**, **Inc.** (**RKI**) for RPS Infrastructure, Inc. on behalf of the City of Georgetown, pursuant to applicable Edwards Aquifer Protection Program Rules as specified in *Title 30 of the Texas Administrative Code*, *Section 213 (30 TAC §213, effective April 24, 2008)*. This assessment report is in the format required by the Texas Commission on Environmental Quality (TCEQ) for the Geologic Assessment portion of the referenced Water Pollution Abatement Plan (WPAP) submittal and was prepared in accordance with the revised *Instructions to Geologists for Geologic Assessments on the Edwards Aquifer Recharge/Transition Zones (TCEQ-0585)*, which are applicable to submittals received by the TCEQ after October 1, 2004.

This geologic assessment (GA) report documents conditions observed by **RKI** on May 18, 2023 within the Project boundaries, which were defined to include a 25-foot buffer surrounding the planned improvements based on collaboration with RPS (hereinafter referred to as Project).

Project Description

Project Location. The Project consists of two discrete land areas located on the west side (i.e., north of SH 29) and east side (i.e., south of SH 29) of Scenic Drive. The Project is located within a primarily commercial area and consists of a fully-developed urban corridor. As depicted in the following **Photograph 1**, the property adjacent to the southbound lane is currently undeveloped with the exception of an 8-foot wide concrete sidewalk for a river trail extending from the central portion to the south of the project boundary. Utility/traffic poles are also present at the Scenic Drive and SH 29 intersection. **Photograph 2** depicts the property adjacent to the northbound lane of Scenic Drive is developed with impervious cover and landscaping for a commercial development (i.e., retail strip center), which is located at 1204 Railroad Avenue.

Photograph 1: Southbound Lane



West side of Scenic Drive, north of SH 29. View to the south.

Photograph 2: Northbound Lane



East side of Scenic Drive, south of SH 29. View to the south.

Based on review of official maps published by TCEQ, the SITE is fully located within the Edwards Aquifer Recharge Zone (EARZ). As such, the performance of a geologic assessment is required to facilitate planned WPAP construction activities in accordance with applicable provisions set forth in the EAPP rules as specified in *Title 30 of the Texas Administrative Code, Section 213 (30 TAC 213, effective April 24, 2008).*

Topography and Drainage. Topographic information for the Project was obtained from the Georgetown, Texas Topographic Quadrangle Map prepared by the United States Geological Survey (USGS, 2022) and 5-foot topographic contours obtained from the Texas Natural Resources Information System (TNRIS, 2015). The Project consists of gently-sloping hilltop topography, with a maximum elevation of approximately 746 feet relative to mean sea level (MSL) at the south end and a minimum elevation of approximately 743 feet at the north end. As indicated by field observations and topographic contours presented on the **Project Geologic Map**, the local surface drainage patterns are generally from the east to west following existing storm water conveyances along SH 29. Ultimate flow is towards South Fork San Gabriel River, which is located approximately 150 feet west of the Project. A review of the U.S. Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM), Map No. 48491C0293F indicates that no part of the Project is located within the designated 100-year floodplain.

Historical Property Use. Although research pertaining to past Project operations and historical land use activities was beyond the scope of this assessment, historical aerial imagery was reviewed to evaluate historical land use and the presence of lineations that could indicate the presence of normal faulting. The following aerial photographs from Google EarthTM were reviewed: 1995, 1997, 2002 through 2006, 2008, 2009, and 2011 through 2024. Information obtained from this review is presented below:

- The 1995 to 2004 aerial photographs depict undeveloped land at the northwest corner of SH 29 and Scenic Drive, in addition to several small structures at the southeast corner.
- The 2005 aerial photograph indicates development of a commercial building in the southwest portion of SH 29 at Scenic Drive. No other changes were observed.

- The 2011 aerial photograph depicts Americans with Disabilities Act (ADA) compliant ramps installed at the intersection corners of SH 29 at Scenic Drive
- The 2014 aerial photograph depicts an approximate 8-foot wide concrete sidewalk for the South Fork San Gabriel River trail system west adjacent to Scenic Drive, north of SH 29.
- The 2016 to 2024 aerial photographs depict no changes to the Project or adjacent properties.

Classification of Recharge Features: As further described herein, one manmade recharge feature was identified within Project boundaries, which included a potable water line. The significance of this feature was assessed using definitions and guidance provided in *Instructions to Geologists (TCEQ-0585-Instructions, revised October 1, 2004)*. All features within the Project that met the criteria presented in this reference were mapped. The characteristics of the mapped feature and the corresponding assessment information, as defined by the TCEQ, is presented in the attached *Geologic Assessment Table (TCEQ-0585)*.

Stratigraphy

Based on our review of the *Geologic Atlas of Texas – Austin Sheet (Barnes, 1974, Reprinted 1981)* and the *Geologic Map of the West Half of the Taylor, Texas, 30 x 60 Minute Quadrangle: Central Texas Urban Corridor, Encompassing Round Rock, Georgetown, Salado, Briggs, Liberty Hill, and Leander* (Collins, 2005), in addition to boring logs for adjacent plugged test holes (**RKI**, 2023), the entire Project is underlain by the Edwards Limestone (Ked) formation. The Ked is comprised of limestone, dolomitic limestone, and argillaceous limestone. The Edwards Formation, which is the most porous and prolific part of the aquifer thins northward from approximately 300 to 90 feet and is thinner than any other aquifer segment. The older Comanche Peak Formation and Walnut strata interfingers with the Edwards rocks.

Boring logs described a very stiff to hard dark brown clay to approximately 4 feet, then a hard reddish-brown clay (possible decomposed limestone) ranging in thickness from 2.5 to 6 feet, and followed by hard tan weathered limestone with ferrous staining from 4 feet to maximum boring termination depths of 10 feet. No exposures of the Ked were observed as the South Fork San Gabriel River, which is west of the Project area was heavily vegetated along the riverbanks and channel bottom.

Structure

This Project is located within the Balcones Fault Zone. This zone generally consists of a northeast-southwest trending, *en echelon* normal fault system, which juxtaposes Upper Cretaceous lithologies in the southeast with Lower Cretaceous lithologies in the northwest. As a result of this larger-scale regional faulting, minor internal fault sequences and fractures exist within this zone that follow the same structural trend and accommodate localized displacement.

Based on review of historical aerial photographs and published maps, no faults were mapped within the Project, and no evidence of faulting was observed during reconnaissance activities. The nearest mapped fault is located approximately 1,100 feet to the east (i.e., from the northbound project limit).

Project No. ASF23-033-00 March 3, 2025

Manmade Features

As presented on the *Project Geologic Map*, three manmade features were identified that may potentially serve to enhance the transmission of surface runoff to the subsurface. The features consist of a potable water utility line trench and two plugged geotechnical test holes, which meet the criteria for assessment as a manmade features in bedrock. The location of the existing utility trench was obtained from the 100% design plans, dated October 3, 2024 made available from RPS and review , the City of Georgetown online Geographic Information System (GIS) interactive maps for utility information, https://georgetowntx.maps.arcgis.com/, in addition to field observations of water valves, fire hydrants, and paint markings.

Feature S-1 is an existing potable water utility trench. installed into the underlying bedrock of the Ked. Although not directly observable, it is inferred that this subgrade trench is backfilled in accordance with standard construction practices that include the use of structural fill soils (e.g., limestone gravel, compacted clay soils, etc.) overlain by native or fill soils. This trench was not observed in conjunction with any naturally occurring recharge features. Although the backfilled trench may exhibit a somewhat greater relative infiltration rate than the surrounding soil/rock strata underlying the project boundaries, this manmade feature is classified as not sensitive, having a low potential of preferentially transmitting fluids to the Edwards Aquifer. This classification is based upon the point assignment criteria presented in the **Geologic Assessment Table (TCEQ-0585)** and professional judgment.

Information regarding the locations of the geotechnical test holes was obtained from the referenced geotechnical engineering study **RKI** (2023). There were field indications of these plugged test holes. The following is a discussion of the features that were reported:

Features S-2 and S-3 consist of geotechnical borings reportedly installed to maximum total depths of approximately 10 feet. Borings reported encountering very stiff to hard dark brown to reddish-brown clay soils, which overlie a hard, tan weathered limestone consisting of the Edwards Limestone. These logging observations are consistent with mapped soil and rock types. No shallow groundwater was observed during drilling operations. These features are collectively classified as not sensitive as they have been plugged and no longer exist. The former locations of these features are included on the **Project Geologic Map.**

Potential for Fluid Migration to the Edwards Aquifer

Based on a review of Project geology, topography and drainage conditions, and the results of our mapping efforts, the overall potential for direct fluid migration (i.e., surface-derived flow) to the Edwards Aquifer via infiltration is considered to be low to moderate. The following assessment findings support this conclusion:

The capacity for direct infiltration of surface runoff or other spilled substances is limited owing to
presence of clay soils and impervious cover along Scenic Drive, north and south of SH 29, in addition
to the presence of an existing conveyance structure (i.e., storm drain) present along the Scenic Drive
ROW that serves to rapidly transmit water downhill to the south/southeast.

- There were no natural karst features observed throughout the Project area would increase the potential for rapid infiltration. Owing to soil cover and improvements, no exposures of the Ked are present at the Project.
- The manmade features present at the Project are classified as not sensitive based on consideration of construction details and application of point assignment criteria and professional judgment.

References

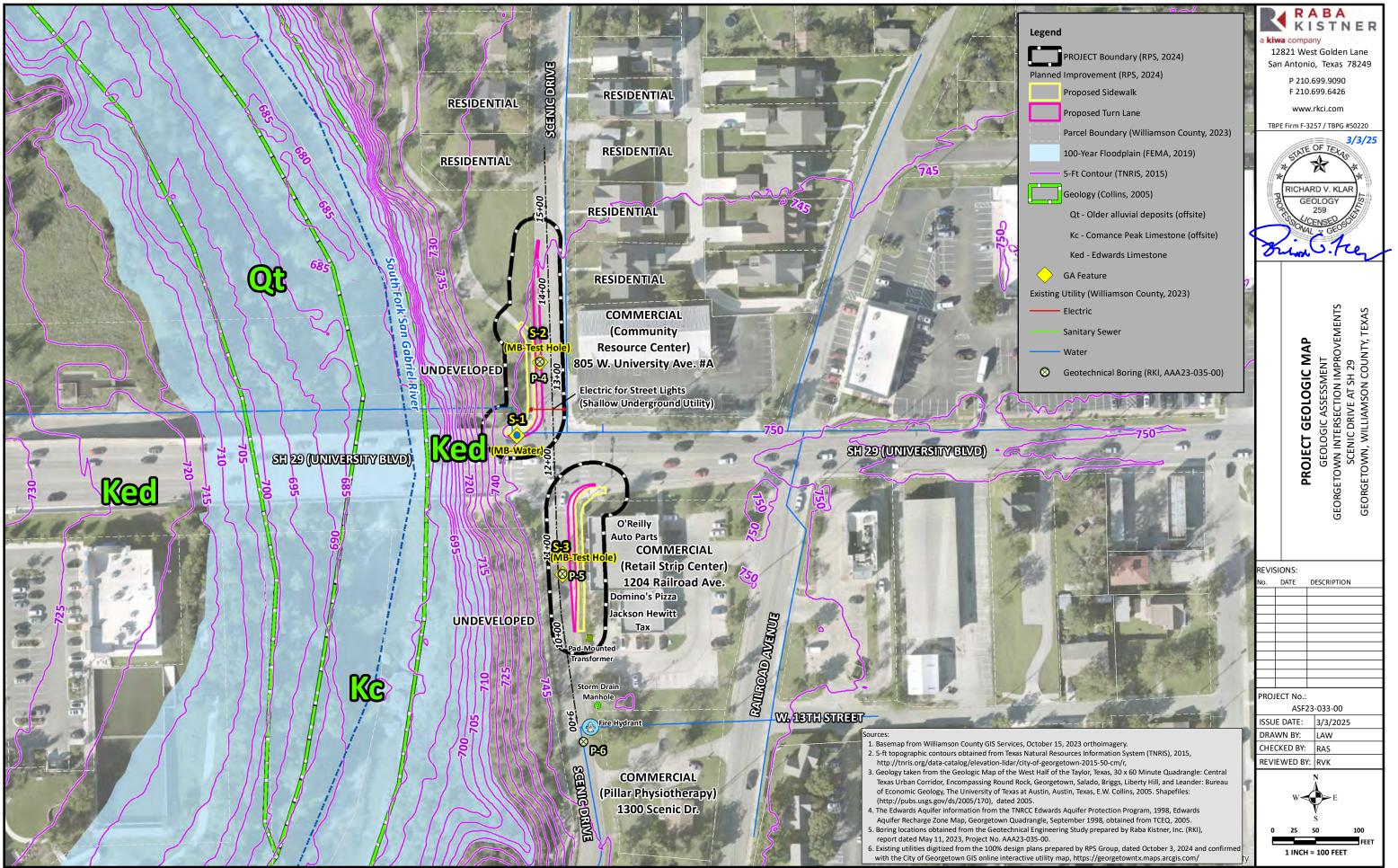
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- Collins, Edward W., 2005, Geologic Map of the West Half of the Taylor, Texas, 30 x 60 Minute Quadrangle: Central Texas Urban Corridor, Encompassing Round Rock, Georgetown, Salado, Briggs, Liberty Hill, and Leander: Bureau of Economic Geology, The University of Texas at Austin, Austin, Texas.
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Texas Water Development Board (TWDB), Water Data Interactive (WDI) Groundwater Data Viewer, https://www2.twdb.texas.gov/apps/WaterDataInteractive/GroundwaterDataViewer/?map=sdr, accessed February 24, 2025.

United States Geological Survey (USGS), 2013, Georgetown Quadrangle; USGS, Denver, Colorado.

United States Department of Agriculture (USDA), 1983, Soil Survey of Williamson County, Texas; USDA / Soil Conservation Service / Texas Agricultural Experiment Station.

United States Department of Agriculture (USDA), 1986, Urban Hydrology for Small Watersheds; USDA / Natural Resource Conservation Service, Technical Release (TR-55), June 1986.



ATTACHMENT D FEATURE POSITION TABLE (GPS COORDINATES) PROJECT GEOLOGIC MAP

March 3, 2025

FEATURE POSITION TABLE

Georgetown Intersection Improvements Scenic Drive at SH 29

Georgetown, Williamson County, Texas

RKI Project No. ASF23-033-00

Feature Designation	Feature Type	Date Collected	North Latitude	West Longitude	UTM Northing (meters)	UTM Easting (meters)
S-1	Manmade feature in bedrock (Potable Water Line)	5/18/2023	N30° 37' 59.71"	W97° 41' 3.08"	3389696	626099
S-2	Manmade feature in bedrock (Plugged Geotechnical Test Hole, P-4)	5/18/2023	N30° 38' 0.57"	W97° 41' 2.79"	3389723	626106
S-3	Manmade feature in bedrock (Plugged Geotechnical Test Hole, P-5)	5/18/2023	N30° 37' 58.13"	W97° 41' 2.53"	3389648	626114

NOTES:

- 1) Geographic coordinates are presented Degrees, Minutes, Decimal Seconds
- 2) Reference Datum is NAD 83
- 3) Data were collected utilizing a Garmin GPS 60cx Global Positioning System.
- 4) Horizontal Accuracy: RMS Value < 3 meter ground resolution
- 5) GPS data was collected by Rick Sample (**RKI** Project Professional).
- 6) GPS coordinates correlate to the points on the map for each feature.

Recharge and Transition Zone Exception Request Form

Texas Commission on Environmental Quality

30 TAC §213.9 Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Recharge and Transition Zone Exception Request Form** is hereby submitted for TCEQ review and executive director approval. The request was prepared by:

Print Name of Customer/Agent: Md Kamrul Islam, P.E.

Date: <u>05-09-2025</u>

Signature of Customer/Agent:

Regulated Entity Name: Georgetown Intersection Improvements – Scenic Drive at SH 29

Exception Request

- 1. Attachment A Nature of Exception. A narrative description of the nature of each exception requested is attached. All provisions of 30 TAC §213 Subchapter A for which an exception is being requested have been identified in the description.
- 2. Attachment B Documentation of Equivalent Water Quality Protection.

 Documentation demonstrating equivalent water quality protection for the Edwards Aquifer is attached.

Administrative Information

- 3. Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
- 4. The applicant understands that no exception will be granted for a prohibited activity in Chapter 213.
- 5. The applicant understands that prior approval under this section must be obtained from the executive director for the exception to be authorized.

Attachment A – Nature of Exception

Narrative for Exception Request: Scenic Drive and SH 29 Intersection

Project Overview:

The proposed project involves intersection improvements at Scenic Drive and SH 29, aimed at enhancing traffic flow and safety by adding northbound and southbound right-turn lanes. The project site has been previously developed, and the enhancements will involve minor modifications at the intersection.

Existing Conditions:

The total project drainage area included in the plan is 1.83 acres, with a predevelopment impervious area of 1.42 acres.

Proposed Changes:

The project will add right-turn lanes on Scenic Drive at SH 29, leading to a negligible increase in impervious cover. The transition from 1.42 acres to 1.48 acres of impervious area is minor and will not adversely affect stormwater management or groundwater recharge.

Justification for Exception:

- **Negligible Impact**: The 4.28% increase in impervious cover is minimal and will not lead to significant adverse effects on stormwater runoff or groundwater recharge. The project is designed to maintain effective water management practices.
- Water Quality Protection: To ensure that water quality is adequately protected, the project will implement Best Management Practices (BMPs). Stormwater from the proposed improvements will be collected via a curb inlet and conveyed through a pipe under Scenic Drive to a Stormceptor unit. This unit will treat the collected runoff before it is discharged, ensuring compliance with water quality standards.
- Appropriateness of Exception Request: Given that the site has been previously developed and the proposed changes involve a negligible increase in impervious cover, this exception request aligns with the conditions outlined in the Edwards Aquifer rules. The project meets the criteria for an exception as it does not involve significant construction activities and aims to enhance existing infrastructure while protecting water quality.

Attachment B: Documentation of Equivalent Water Quality Protection

Project Overview

The project aims to enhance traffic flow and safety by adding northbound and southbound right-turn lanes at the intersection.

Existing Conditions

- Total project area included in the plan: 1.83 acres.
- Predevelopment impervious area: 1.42 acres
- Total post-development impervious area: 1.48 acres
- Net Increase in Impervious cover: 0.06
- This results in a 4.28% increase in impervious cover.

Proposed Changes

The project will introduce additional pavement, leading to a negligible increase in impervious cover. The transition from 1.42 acres to 1.48 acres of impervious area is minor and will not adversely affect stormwater management or groundwater recharge.

Best Management Practices (BMPs)

A Stormceptor unit will treat stormwater runoff collected from the project area and the offsite area, utilizing sedimentation and filtration to remove pollutants before discharging treated water into the existing storm sewer system.

Per the TCEQ Technical Guidance Manual, the required TSS removal for the total project is calculated as follows:

- \circ $L_M = 27 \cdot (A_N \times P)$
 - L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed development
 - A_N = Net increase in impervious area for the project
 - P = Average annual precipitation = 32 inches
- L_{M TOTAL PROJECT} = 53 Lbs

- o Required TSS Removal in BMP Drainage Area: 26.88 lbs
- o TSS Removal for Uncaptured Area (Impervious Cover Overtreatment): 28.70 lbs
- Total TSS Treatment by BMP = LM+TSS Uncaptured
 - Total TSS Treatment = 55.58 lbs

The proposed project aligns with the intent of the Edwards Aquifer rules by implementing effective BMPs that ensure water quality is protected, thereby justifying the exception request.

Monitoring and Maintenance Plan

The Stormceptor unit will undergo regular inspections and maintenance to ensure its effectiveness in treating stormwater runoff. Maintenance activities will include sediment removal and system checks to confirm operational efficiency.

Temporary Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: Md Kamrul Islam, P.E.

Date: 05/14/2025

Signature of Customer/Agent:

Regulated Entity Name: Georgetown Intersection Improvements – Scenic Drive at SH 29

Project Information

Potential Sources of Contamination

Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.

1.	Fuels for construction equipment and hazardous substances which will be used during construction:
	The following fuels and/or hazardous substances will be stored on the site:
	These fuels and/or hazardous substances will be stored in:
	Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

	 Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year. Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
	igstyle igstyle Fuels and hazardous substances will not be stored on the site.
2.	Attachment A - Spill Response Actions. A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
3.	☐ Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
4.	Attachment B - Potential Sources of Contamination. A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.
S	equence of Construction
5.	Attachment C - Sequence of Major Activities. A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
	 For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given. For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
6.	Name the receiving water(s) at or near the site which will be disturbed or which will receive discharges from disturbed areas of the project:
T	emporary Best Management Practices (TBMPs)
sta co ba	osion control examples: tree protection, interceptor swales, level spreaders, outlet abilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized instruction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment sins. Please refer to the Technical Guidance Manual for guidelines and specifications. All ructural BMPs must be shown on the site plan.

7. Attachment D – Temporary Best Management Practices and Measures. TBMPs and

measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

	A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.	
	A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.	
	 A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer. A description of how, to the maximum extent practicable, BMPs and measures will 	
	maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.	
8.	The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.	е
	Attachment E - Request to Temporarily Seal a Feature. A request to temporarily seal a feature is attached. The request includes justification as to why no reasonab and practicable alternative exists for each feature.	le
	There will be no temporary sealing of naturally-occurring sensitive features on the site.	
9.	Attachment F - Structural Practices. A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runch discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.	
10.	Attachment G - Drainage Area Map . A drainage area map supporting the following requirements is attached:	
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided.	
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used.	
	For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area.	
	There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.	

	There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. Erosion and sediment controls other than sediment basins or sediment traps within each disturbed drainage area will be used.
	Attachment H - Temporary Sediment Pond(s) Plans and Calculations. Temporary sediment pond or basin construction plans and design calculations for a proposed temporary BMP or measure have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information must be signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed temporary BMPs and measures are attached.
\boxtimes	N/A
	Attachment I - Inspection and Maintenance for BMPs. A plan for the inspection of each temporary BMP(s) and measure(s) and for their timely maintenance, repairs, and, if necessary, retrofit is attached. A description of the documentation procedures, recordkeeping practices, and inspection frequency are included in the plan and are specific to the site and/or BMP.
	All control measures must be properly selected, installed, and maintained in accordance with the manufacturer's specifications and good engineering practices. If periodic inspections by the applicant or the executive director, or other information indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations.
	If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain).
	Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50%. A permanent stake will be provided that can indicate when the sediment occupies 50% of the basin volume.
	Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source for stormwater discharges (e.g., screening outfalls, picked up daily).
Soil	Stabilization Practices
mulchii	les: establishment of temporary vegetation, establishment of permanent vegetation, ng, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or vation of mature vegetation

preservation of mature vegetation.

17. X Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices. A schedule of the interim and permanent soil stabilization practices for the site is attached.

- 18. Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 19. Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

Administrative Information

- 20. All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
- 21. If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
- 22. Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

Attachment A - Spill Response Actions

This project will prioritize spill prevention through comprehensive worker training on potential hazards, strict adherence to Material Safety Data Sheets (MSDS) for all on-site materials, and the provision of appropriate Personal Protective Equipment (PPE). While on-site storage of hazardous substances is prohibited, the operation of construction equipment necessitates careful handling of fuels, lubricants, and potential paving materials. In the event of a spill, immediate containment and cleanup procedures as outlined in the MSDS will be enacted, with prompt notification of the designated project Spill Coordinator and relevant authorities. Contaminated materials will be properly disposed of at an approved facility, ensuring compliance with all applicable local, state, and federal regulations. All appropriate authorities will be notified as soon as a spill is discovered. The emergency response phone number for the State of Texas Spill-Reporting Hotline is 1-800-832-8224.

Attachment B - Potential Sources of Contamination

This intersection improvement project involves various construction activities that present potential sources of contamination. These sources can be broadly categorized into sediment-related and non-sediment-related issues. Sediment-related contamination includes clearing and grading, where the initial phases of construction will disturb existing soil, leading to potential sediment runoff during rainfall events. Excavation for curb and gutter installation, sidewalk construction, and utility relocation will further expose soil, increasing the risk of erosion. Vehicle tracking is another concern, as construction equipment and delivery vehicles can track sediment onto paved surfaces, which can then be washed into storm drains. Non-sediment-related contamination involves fuel and lubricants, where the operation and maintenance of construction equipment pose a risk of spills or leaks of diesel fuel, gasoline, hydraulic fluids, and motor oil. Paving materials such as asphalt paving operations may release asphaltic materials and solvents. Concrete operations, including concrete mixing and pouring, can introduce concrete fines and additives. To minimize these potential sources of contamination, the contractor will implement erosion and sediment control measures, including silt fences, temporary sediment traps, and stabilized construction entrances. Regular equipment maintenance will be conducted to prevent leaks and spills. Proper handling and storage of materials will be enforced, and spill response equipment will be readily available on-site. As outlined in Attachment A, in the event of a spill, all steps will be taken to contain and clean the area.

Attachment C - Sequence of Construction

This attachment outlines the sequence of major construction activities for the intersection improvements.

• Traffic Control and Site Preparation:

- o Installation of temporary traffic control devices and signage.
- Establishment of construction entrances and exits.
- Installation of initial erosion and sediment control measures (e.g., silt fence, construction exits).

• Clearing and Grubbing:

- Removal of existing vegetation, including trees, shrubs, and grass, within the construction limits.
- O Stripping and stockpiling of topsoil for later use in restoration.

• Excavation and Grading:

- o Excavation for the new right-turn lanes, curb and gutter, and sidewalk.
- o Rough grading to establish the proposed roadway alignment and drainage patterns.
- o Compaction of subgrade as required.

• Storm Sewer System Installation

- Excavate and install the Stormceptor®, storm sewer pipe, curb inlet, and the outfall structure.
- o Ensure all components are properly connected and set to design elevations.
- Stabilize disturbed areas around all drainage infrastructure.

• Curb and Gutter and Sidewalk Construction:

- o Forming and pouring of concrete curb and gutter.
- o Construction of new sidewalk sections.

• Pavement Construction:

- Placement of base course materials.
- o Paving of the new right-turn lane with asphalt.
- o Placement of pavement markings.

• Final Grading and Restoration:

- o Final grading of the site to ensure proper drainage.
- Placement of topsoil and seeding/sodding in disturbed areas.
- o Removal of temporary erosion and sediment control measures after site stabilization.
- o Final site cleanup and demobilization.

For each activity, appropriate temporary erosion and sediment control measures will be implemented and maintained throughout the construction process. The general timing of these measures will coincide with the commencement of each activity and will remain in place until final site stabilization is achieved.

Attachment D - Temporary Best Management Practices and Measures

The following Temporary Best Management Practices (BMPs) and measures will be implemented to prevent pollution of surface water, groundwater, and stormwater during construction of this project.

- a. Prevention of Pollution from Upgradient Sources:
 - **Description:** The project site is located in a developed area. Upgradient sources of potential pollutants may include runoff from adjacent roadways and properties.

• BMPs and Measures:

- Sediment fences will be installed along the upgradient perimeter of the site to intercept and filter runoff from these areas.
- Existing drainage patterns will be maintained to the extent practicable to minimize the concentration of runoff onto the site.
- b. Prevention of Pollution from On-Site or Off-Site Flows:
 - **Description:** Construction activities will disturb soil, creating the potential for sediment-laden runoff. Potential pollutants include sediment, fuels, and lubricants.

• BMPs and Measures:

- Temporary sediment fences and construction exits will be installed to control sediment runoff.
- o Disturbed areas will be stabilized with temporary seeding and mulching.
- Spill prevention and control measures will be implemented to prevent the release of fuels and lubricants.
- o Proper waste management practices will be followed.
- The site will be graded to direct runoff to stabilized outlets.
- c. Prevention of Pollutants from Entering Surface Streams, Sensitive Features, or the Aquifer:
 - **Description:** There are no surface streams or sensitive features located within the immediate project area.

• BMPs and Measures:

- Erosion and sediment control measures will be implemented to prevent pollutants from leaving the site and entering downstream waters.
- The Edwards Aquifer is located within the region, and therefore all precautions will be taken to prevent any pollutants from entering the aquifer.
- d. Maintenance of Flow to Naturally Occurring Sensitive Features:

• **Description:** There are no naturally occurring sensitive features within the project area that require the maintenance of flow.

• BMPs and Measures:

o Existing drainage patterns will be maintained to the extent practicable.

e. Additional Information:

- o All BMPs will be inspected and maintained regularly.
- O The construction contractor will be responsible for implementing and maintaining all BMPs.
- o This plan is based on the attached plans and TxDOT standards.

Attachment F - Structural Practices

This attachment outlines the structural practices implemented to manage runoff and minimize erosion during the construction of this project.

- a. Perimeter Sediment Control: A system of sediment control fence will be installed along the perimeter of the disturbed areas to intercept and divert runoff. This will prevent sediment-laden water from leaving the site and impacting surrounding areas. The fence will also provide localized storage by temporarily ponding runoff, allowing sediment to settle out.
- b. Localized Erosion Control & Site Stabilization: Localized erosion control measures will be strategically placed to minimize soil disturbance and prevent erosion within the construction footprint. Site stabilization techniques, such as temporary seeding and mulching, will be employed to protect exposed soils from erosion and establish a protective cover.
- c. Runoff Management & Limitation of Discharge: Runoff will be managed through strategically placed controls to slow down and filter water before it leaves the site. These controls, including the sediment control fence, will act as barriers, promoting sedimentation and limiting the discharge of pollutants.
- d. Floodplain Protection: All structural practices will be implemented outside of designated floodplain areas to avoid any potential impacts.
- e. Ongoing Monitoring and Maintenance: Regular inspections and maintenance will be conducted to ensure the effectiveness of all structural practices throughout the construction period.
- f. Contractor Responsibility: The construction contractor will be responsible for the proper installation, maintenance, and removal of all structural practices.

Attachment I - Inspection and Maintenance for BMPs

This attachment outlines the maintenance procedures for the temporary Best Management Practices (BMPs) implemented for erosion and sediment control during the construction of this project.

- a. Sediment Fence Maintenance
 - Frequency: Sediment fences will be inspected:
 - Weekly
 - o After each rainfall event of 0.5 inches or greater
 - o At a minimum of every 14 days

• Procedure:

- Inspect fence fabric for tears, sags, or deterioration. Repair or replace damaged sections immediately.
- o Check fence posts for stability. Reset or replace posts as needed.
- o Remove accumulated sediment when it reaches 1/3 to 1/2 of the fence height.
- o Ensure that the bottom of the fence remains properly trenched and anchored.
- O Dispose of removed sediment in a suitable location where it will not erode.
- b. Construction Exit Maintenance
 - **Frequency:** Construction exits will be inspected:
 - o Daily
 - o After significant rainfall events

• Procedure:

- Maintain the aggregate pad to ensure it is clean and free of sediment buildup.
- Add new aggregate as needed to maintain the required depth and prevent tracking of sediment off-site.
- o Remove any sediment that is tracked onto public roads.
- Repair any damage to the exit, such as rutting or displacement of aggregate.
- c. Temporary Seeding and Soil Stabilization Maintenance
 - Frequency: Seeded and mulched areas will be inspected:
 - Weekly
 - o After rainfall events

Procedure:

- o Check for erosion and re-seed or re-mulch as necessary.
- o Repair any rills or gullies that develop.
- o Ensure that vegetation is established and healthy.

d. General BMP Maintenance

- Frequency: All temporary BMPs will be inspected:
 - o Weekly
 - o After significant rainfall events

• Procedure:

- o Ensure that all BMPs are functioning as intended.
- o Repair or replace any damaged or ineffective BMPs.
- o Remove any accumulated debris or sediment that may be impairing BMP performance.

e. Record Keeping

- Detailed records of all inspections and maintenance activities will be maintained on-site.
- These records will include the date of inspection, the condition of the BMPs, and any maintenance performed.

f. Contractor Responsibility

• The construction contractor will be responsible for the proper maintenance of all temporary BMPs throughout the construction period.

Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices

This attachment details the schedule for both interim and permanent soil stabilization. Interim stabilization, encompassing temporary measures like seeding, mulching, and erosion control blankets, will be implemented as soon as practicable following the temporary or permanent cessation of construction activities in any area, ensuring stabilization occurs within seven days. Permanent stabilization, including final grading, topsoil placement, and the establishment of permanent vegetation, will commence promptly after final grading is completed. Records documenting the dates of major grading, activity cessation, and stabilization initiation will be maintained on-site. The construction contractor is responsible for the implementation and upkeep of all soil stabilization practices throughout the project.

Permanent Stormwater Section

Texas Commission on Environmental Quality

Print Name of Customer/Agent: Md Kamrul Islam, P.E.

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(C), (D)(Ii), (E), and (5), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Permanent Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Date: 05/09/2025
Signature of Customer/Agent

Regulated Entity Name: Georgetown Intersection Improvements – Scenic Drive at SH 29

Permanent Best Management Practices (BMPs)

Permanent best management practices and measures that will be used during and after

Construction is completed.
 Permanent BMPs and measures must be implemented to control the discharge of

	pollution from regulated activities after the completion of construction.
	□ N/A
2.	These practices and measures have been designed, and will be constructed, operated, and maintained to insure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity is removed. These quantities have been calculated in accordance with technical guidance prepared or accepted by the executive director.
	The TCEQ Technical Guidance Manual (TGM) was used to design permanent BMPs and measures for this site.

	A technical guidance other than the TCEQ TGM was used to design permanent BMPs and measures for this site. The complete citation for the technical guidance that was used is:
	□ N/A
3.	Owners must insure that permanent BMPs and measures are constructed and function as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the appropriate regional office within 30 days of site completion.
	□ N/A
4.	Where a site is used for low density single-family residential development and has 20 % or less impervious cover, other permanent BMPs are not required. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
	The site will be used for low density single-family residential development and has
	 20% or less impervious cover. The site will be used for low density single-family residential development but has more than 20% impervious cover. The site will not be used for low density single-family residential development.
5.	The executive director may waive the requirement for other permanent BMPs for multifamily residential developments, schools, or small business sites where 20% or less impervious cover is used at the site. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.
	 □ Attachment A - 20% or Less Impervious Cover Waiver. The site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is attached. □ The site will be used for multi-family residential developments, schools, or small business sites but has more than 20% impervious cover. □ The site will not be used for multi-family residential developments, schools, or small
6.	business sites. Attachment B - BMPs for Upgradient Stormwater.

		 □ A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is attached. □ No surface water, groundwater or stormwater originates upgradient from the site and flows across the site, and an explanation is attached. □ Permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site, and an explanation is attached.
7.	\boxtimes	Attachment C - BMPs for On-site Stormwater.
		 A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is attached. Permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, and an explanation is attached.
8.		Attachment D - BMPs for Surface Streams . A description of the BMPs and measures that prevent pollutants from entering surface streams, sensitive features, or the aquifer is attached. Each feature identified in the Geologic Assessment as sensitive has been addressed.
	\boxtimes	N/A
9.		The applicant understands that to the extent practicable, BMPs and measures must maintain flow to naturally occurring sensitive features identified in either the geologic assessment, executive director review, or during excavation, blasting, or construction.
		 The permanent sealing of or diversion of flow from a naturally-occurring sensitive feature that accepts recharge to the Edwards Aquifer as a permanent pollution abatement measure has not been proposed. Attachment E - Request to Seal Features. A request to seal a naturally-occurring sensitive feature, that includes, for each feature, a justification as to why no reasonable and practicable alternative exists, is attached.
10.		Attachment F - Construction Plans . All construction plans and design calculations for the proposed permanent BMP(s) and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer, and are signed, sealed, and dated. The plans are attached and, if applicable include:
		 ✓ Design calculations (TSS removal calculations) ☐ TCEQ construction notes ☐ All geologic features ✓ All proposed structural BMP(s) plans and specifications
		N/A

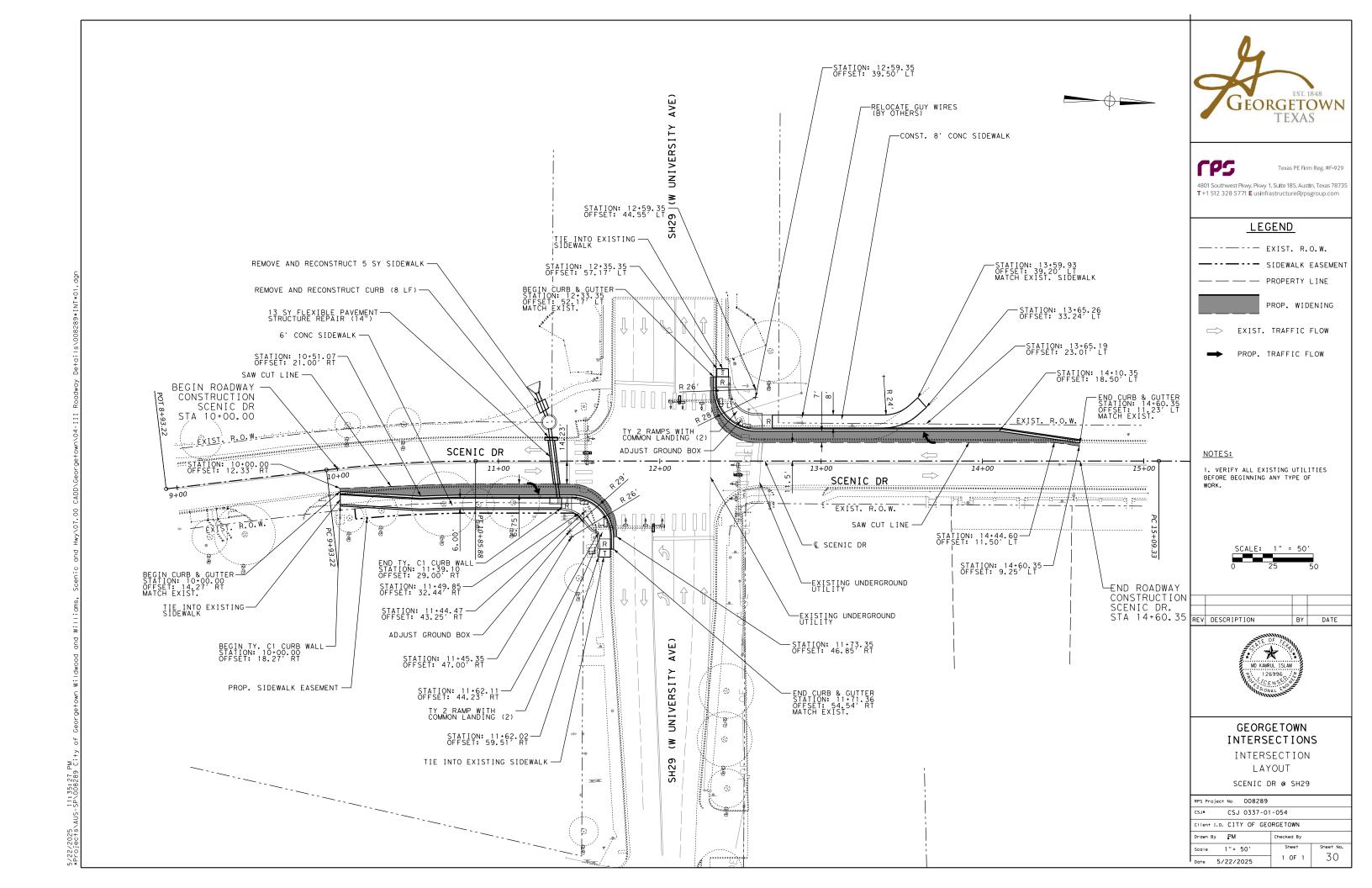
11. Attachment G - Inspection, Maintenance, Repair and Retrofit Plan. A plan for the inspection, maintenance, repairs, and, if necessary, retrofit of the permanent BMPs and measures is attached. The plan includes all of the following:
 □ Prepared and certified by the engineer designing the permanent BMPs and measures □ Signed by the owner or responsible party ☑ Procedures for documenting inspections, maintenance, repairs, and, if necessary retrofit ☑ A discussion of record keeping procedures
□ N/A
12. Attachment H - Pilot-Scale Field Testing Plan. Pilot studies for BMPs that are not recognized by the Executive Director require prior approval from the TCEQ. A plan for pilot-scale field testing is attached.
⊠ N/A
13. Attachment I -Measures for Minimizing Surface Stream Contamination. A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity, which increase erosion that results in water quality degradation.
⊠ N/A
Responsibility for Maintenance of Permanent BMP(s)
Responsibility for maintenance of best management practices and measures after construction is complete.
14. The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
□ N/A
15. A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development, or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.
□ N/A

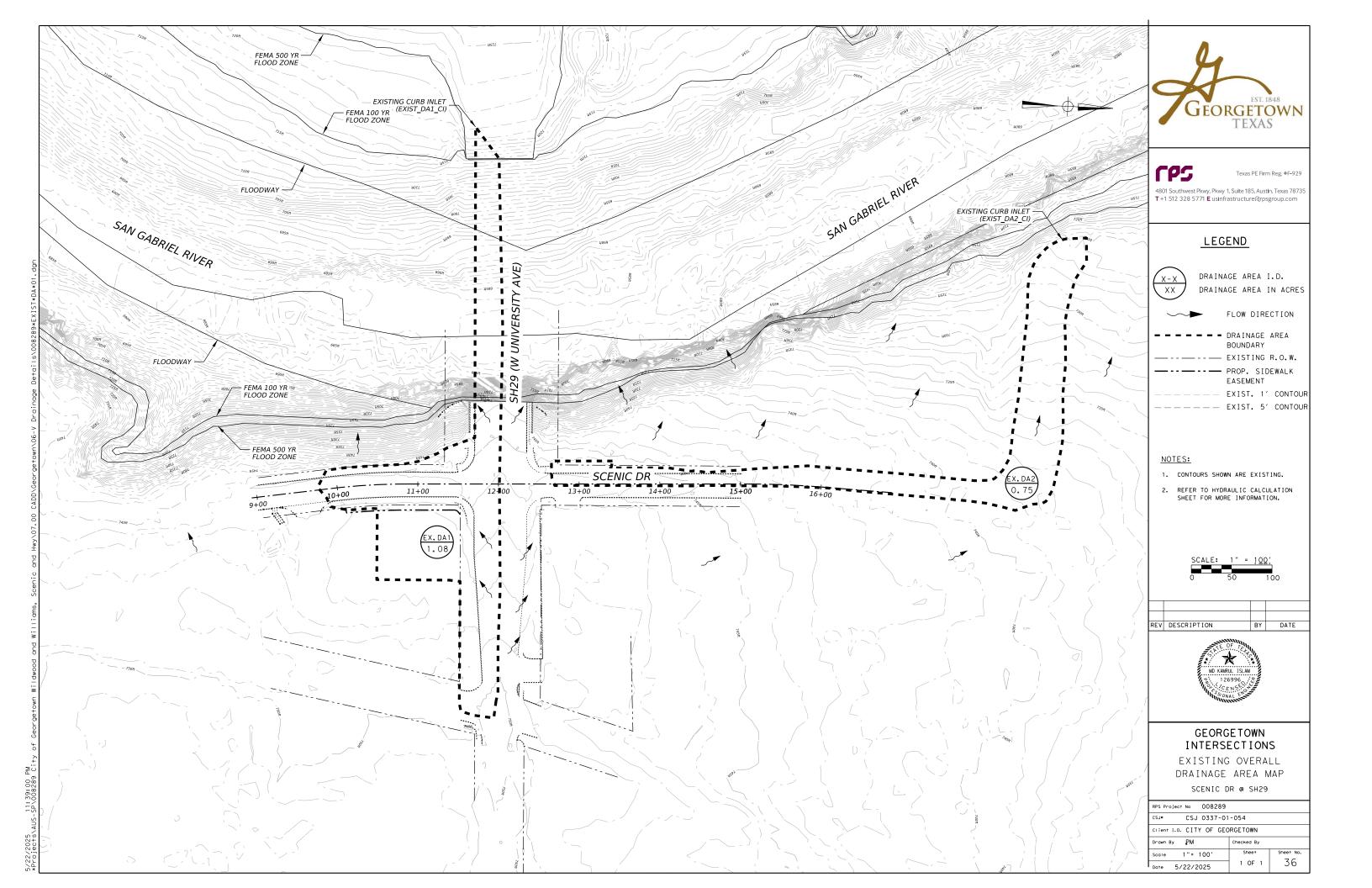
Attachment C – BMP for on-site Stormwater

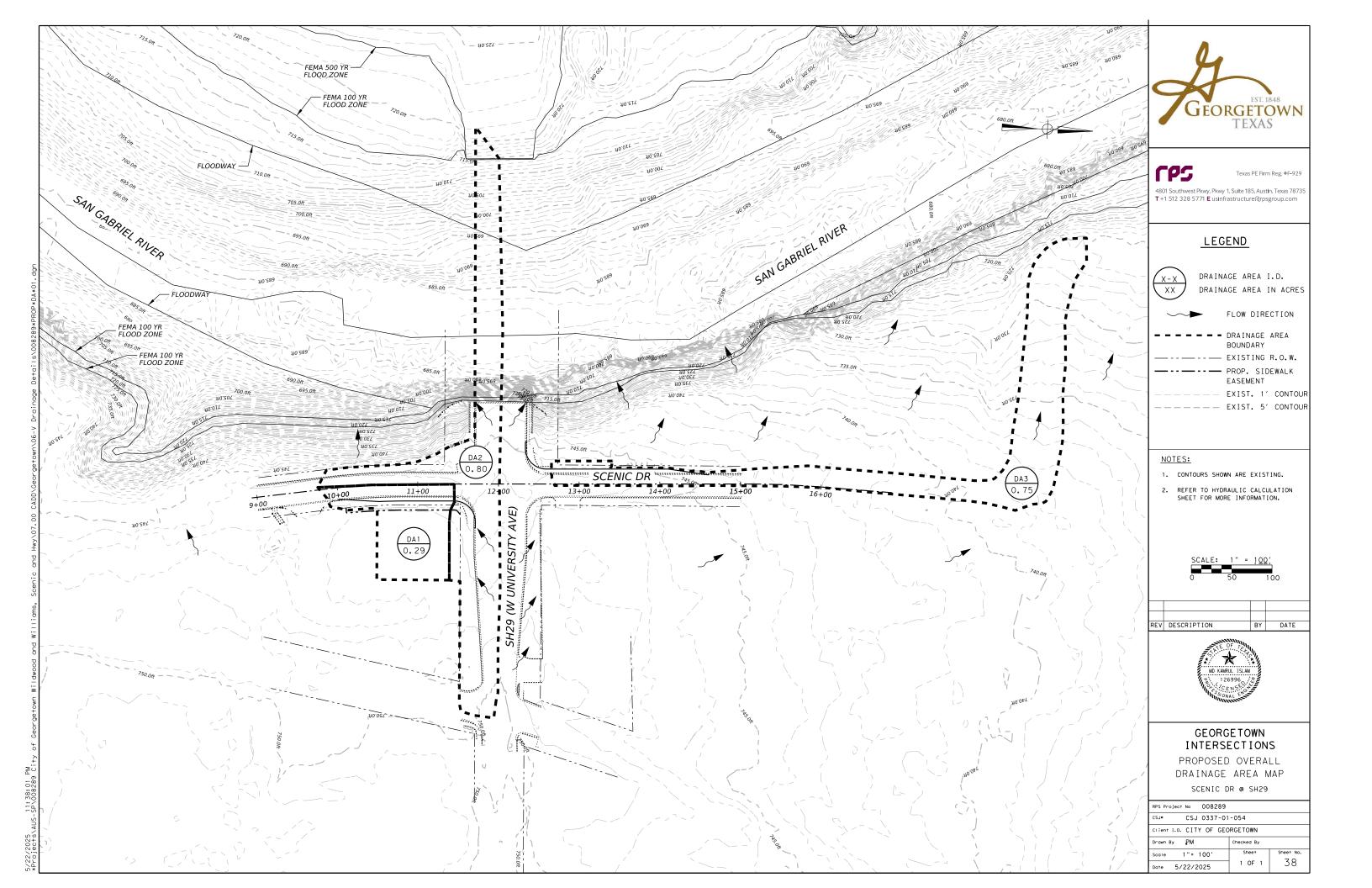
To effectively manage stormwater runoff at the intersection of Scenic Dr and SH 29, a Best Management Practice (BMP) has been implemented. Stormwater generated from the proposed roadway, along with runoff from the adjacent property, will be captured through a curb inlet. This water will then be directed through a pipe beneath Scenic Drive to a Stormceptor unit, where it will undergo treatment. After treatment, the treated runoff will be discharged through a pipe and outfall. The Stormceptor will serve as a permanent BMP for this intersection improvement project.

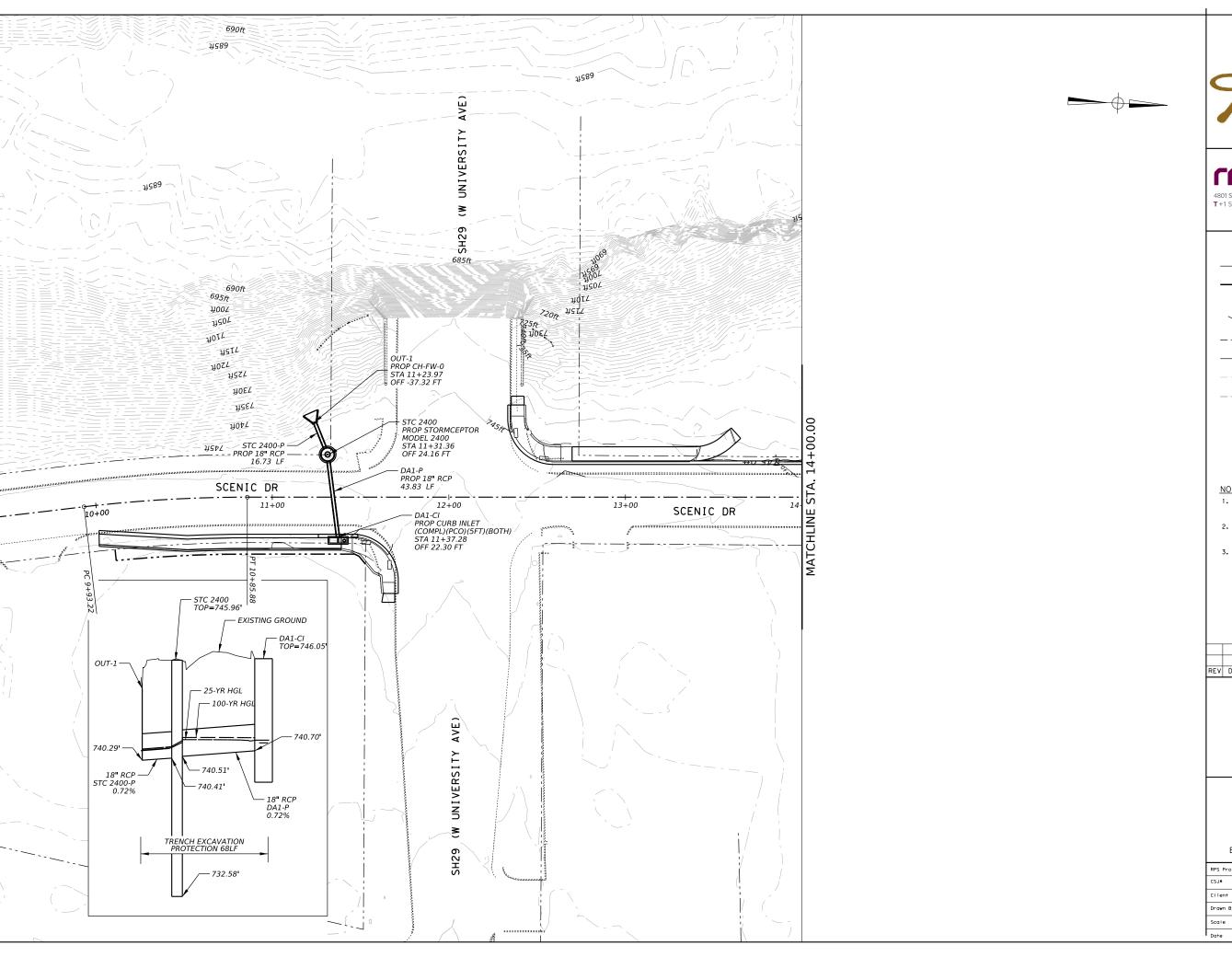
Attachment F – Construction Plans

The construction plans are attached on the following pages.











CPS

4801 Southwest Pkwy, Pkwy 1, Sulte 185, Austin, Texas 78735 T+1 512 328 5771 E usinfrastructure(qrpsgroup.com

LEGEND

____ -- __ EXIST ROW

---- PROP. SIDEWALK EASEMENT

✓► FLOW DIRECTION

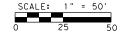
---- DITCH Q

EXIST. 1' CONTOUR

---- EXIST. 5' CONTOUR

NOTES:

- 1. VERIFY ALL EXISTING UTILITIES BEFORE BEGINNING ANY TYPE OF
- WORK.
 2. INLET STATIONS, OFFSETS, AND RADII ARE MEASURED FROM LIP OF GUTTER UNLESS OTHERWISE NOTED.
 3. REFER TO DRAINAGE CALCULATION SHEETS FOR STORMSCEPTOR SIZING CALCULATIONS.



REV DESCRIPTION BY DATE

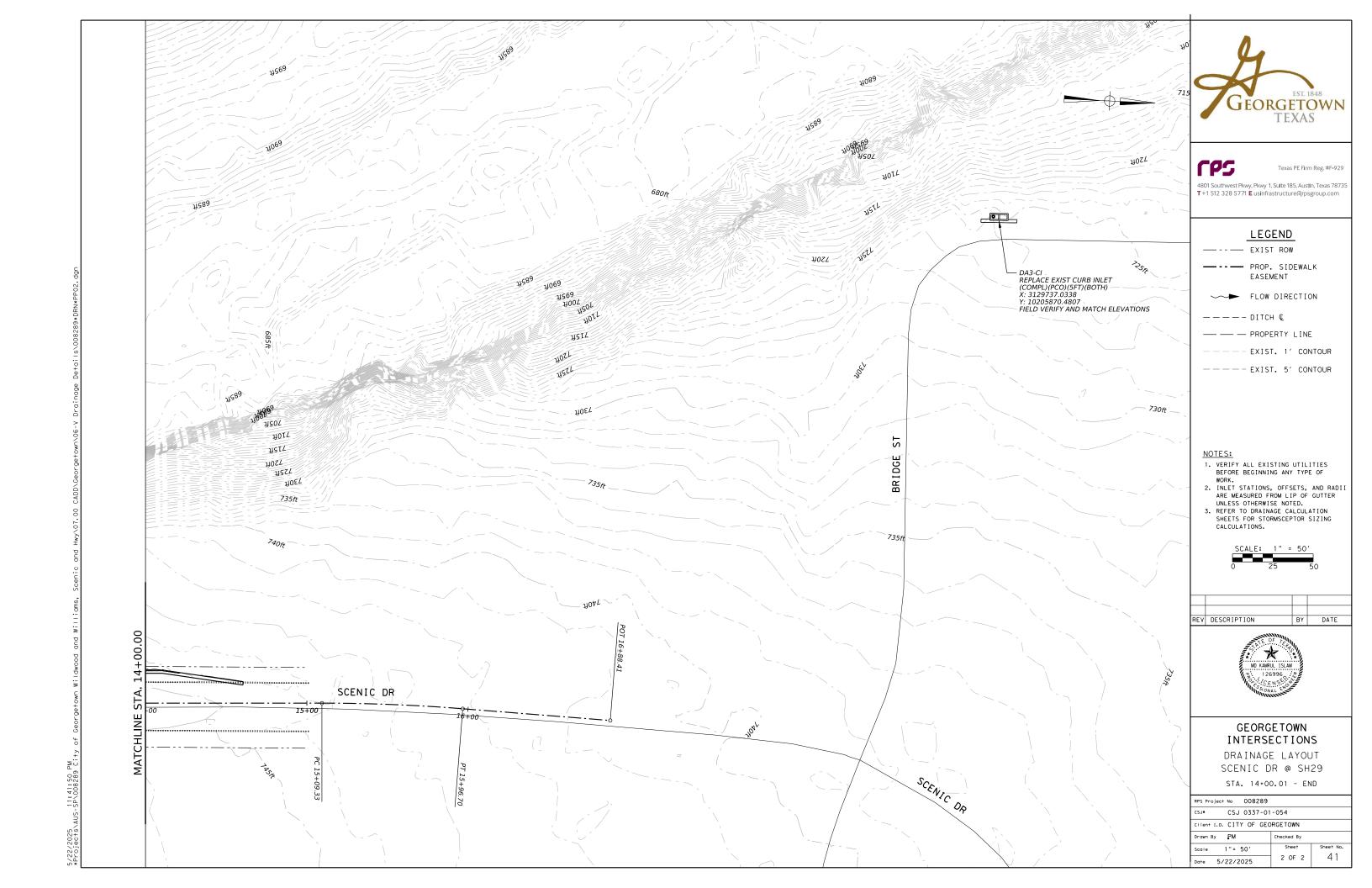


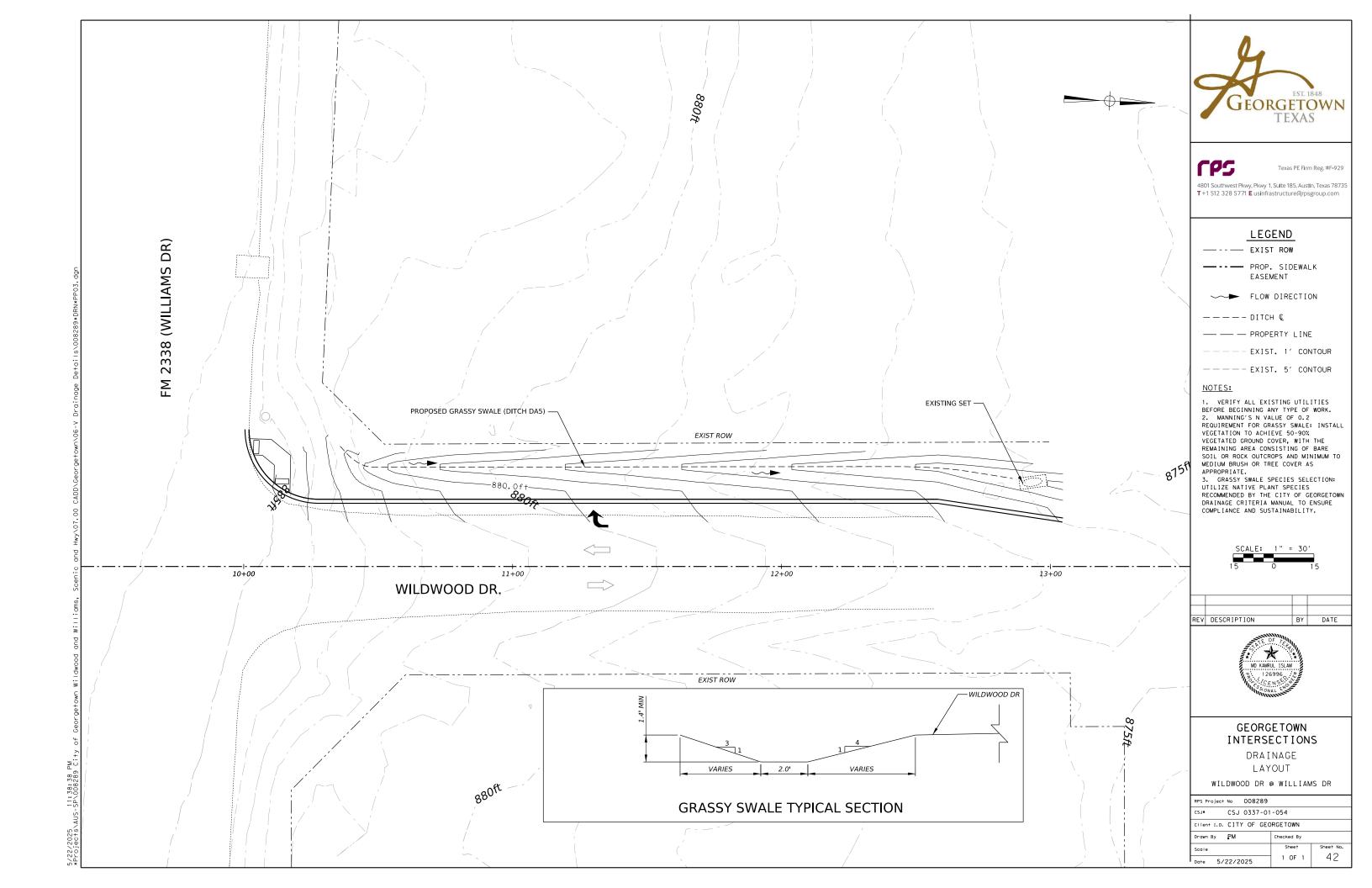
GEORGETOWN INTERSECTIONS

DRAINAGE LAYOUT SCENIC DR @ SH29

BEGINNING - STA. 14+00.00

RPS Project	No	008289		
CSJ=	CSJ	0337-01	-054	
Client I.D.	CITY	OF GEO	RGETOWN	
Drawn By	₽М		Checked By	
Scale	1"= 5	50′	Sheet	Sheet No.
Date 5/	22/2	025	1 OF 2	40





SCENIC DR @SH29 25-YEAR INLET HYDRAULIC DATA

Node ID	Baseline	Baseline	Type (Inlet)	Profile Type	Flow	Total Rational	Computed	Inlet Allowable	Longitudinal	Road Cross	Gutter Width	Curb Opening	Gutter Cross	Bypass to Node	Inlet Bypass Flow	Inlet Computed	Inlet Allowable
(Label)	Station	Offset		(Inlet Location)	(Local from Inflow	Flow to Inlet	Ponded	Ponded	Slope	Slope (%)	(ft)	Length	Slope	(Bypass Target)	(Bypassed	Ponded Depth	Ponded Depth
	(ft)	(ft)			Collection)	(cfs)	Width (Spread /	Width (Spread /	(%)			(ft)	(%)		Rational Flow)	(Depth Gutter)	(Depth Gutter)
					(cfs)		Top Width)	Top Width)							(cfs)	(ft)	(ft)
							(ft)	(ft)									
1043	DA1_CI	11+37	22.3	FTW I-CO(10'x3')6"curb	On Grade	2.54	2.54	5.10	2.03	2.00	1.30	10.00	25.00		0.00	4.91	6.00
1095	EXIST_DA1_CI	N/A	N/A	FTW I-CO(10'x3')6"curb	On Grade	9.45	7.99	11.40	2.50	2.00	1.30	10.00	25.00		1.45	6.41	6.00
1097	EXIST_DA2_CI	N/A	N/A	FTW I-CO(10'x3')6"curb	On Grade	5.62	5.46	7.40	3.96	2.00	1.30	10.00	25.00		0.16	5.46	6.00
1108	DA3_CI	N/A	N/A	FTW I-CO(10'x3')6"curb	On Grade	5.54	5.40	7.40	3.96	2.00	1.30	10.00	25.00		0.14	5.44	6.00

GEORGETOWN TEXAS

Texas PE Firm Reg. #F-929

4801 Southwest Pkwy, Pkwy 1, Sulte 185, Austin, Texas 78735 T+1 512 328 5771 E usinfrastructure@rpsgroup.com

SCENIC DR @SH29 100-YEAR INLET HYDRAULIC DATA

Node ID	Baseline	Baseline	Type (Inlet)	Profile Type	Flow	Total Rational	Computed	Inlet Allowable	Longitudinal	Road Cross	Gutter Width	Curb Opening	Gutter Cross	Bypass to Node	Inlet Bypass Flow	Inlet Computed	Inlet Allowable
(Label)	Station	Offset		(Inlet Location)	(Local from Inflow	Flow to Inlet	Ponded	Ponded	Slope	Slope (%)	(ft)	Length	Slope	(Bypass Target)	(Bypassed	Ponded Depth	Ponded Depth
	(ft)	(ft)			Collection)	(cfs)	Width (Spread /	Width (Spread /	(%)			(ft)	(%)		Rational Flow)	(Depth Gutter)	(Depth Gutter)
					(cfs)		Top Width)	Top Width)							(cfs)	(ft)	(ft)
							(ft)	(ft)									
1043	DA1_CI	11+37	22.3	FTW I-CO(10'x3')6"curb	On Grade	3.08	3.08	6.10	2.03	2.00	1.30	10.00	25.00		0.00	5.14	6.00
1095	EXIST_DA1_CI	N/A	N/A	FTW I-CO(10'x3')6"curb	On Grade	11.41	8.87	12.50	2.50	2.00	1.30	10.00	25.00		2.54	6.68	6.00
1097	EXIST_DA2_CI	N/A	N/A	FTW I-CO(10'x3')6"curb	On Grade	6.78	6.25	8.40	3.96	2.00	1.30	10.00	25.00		0.54	5.69	6.00
1108	DA3_CI	N/A	N/A	FTW I-CO(10'x3')6"curb	On Grade	6.89	6.31	8.40	3.96	2.00	1.30	10.00	25.00		0.58	5.71	6.00

SCENIC DR @SH29 25-YEAR LINK HYDRAULIC DATA

Pipe ID	Upstream	Downstream	Conduit	Number of	Size	Manning's n	Length	Upstream	Downstream	Slope	Discharge	Capacity	Velocity	HGL	HGL	Actual Depth	Normal Depth	Critical Depth
	Node	Node	Description	Barrels			(Construction)	Invert	Invert	(Construction	(Flow)	(Design)		Upstream	Downstream	Downstream		
								(Start)	(Stop))				(In)	(Out)	(Out)		
							(ft)	(ft)	(ft)	(%)	(cfs)	(cfs)	(ft/s)	(ft)	(ft)	(ft)	(ft)	(ft)
DA3-P	DA1_CI	STC 2400	RCP	1	18"	0.013	43.80	740.83	740.51	0.72	2.54	8.40	2.16	741.43	741.46	0.95	0.57	0.60
STC 2400-P	STC 2400	OUT-1	RCP	1	18"	0.013	16.70	740.41	740.29	0.72	2.53	8.18	4.08	741.02	740.87	0.57	0.57	0.60

SCENIC DR @SH29 100-YEAR LINK HYDRAULIC DATA

Pipe ID	Upstream	Downstream	Conduit	Number of	Size	Manning's n	Length	Upstream	Downstream	Slope	Discharge	Capacity	Velocity	HGL	HGL	Actual Depth	Normal Depth	Critical Depth
	Node	Node	Description	Barrels			(Construction)	Invert	Invert	(Construction	(Flow)	(Design)		Upstream	Downstream	Downstream		
								(Start)	(Stop))				(In)	(Out)	(Out)		
							(ft)	(ft)	(ft)	(%)	(cfs)	(cfs)	(ft/s)	(ft)	(ft)	(ft)	(ft)	(ft)
DA3-P	DA1_CI	STC 2400	RCP	1	18"	0.013	43.80	740.83	740.51	0.72	3.08	8.40	2.29	741.57	741.58	1.07	0.63	0.67
STC 2400-P	STC 2400	OUT-1	RCP	1	18"	0.013	16.70	740.41	740.29	0.72	3.06	8.18	4.30	741.08	740.93	0.64	0.64	0.67

NOTES:

- 1. CITY OF GEORGETOWN DRAINAGE
 CRITERIA MANUAL USED FOR
 HYDRAULIC CALCULATIONS.
 2. DESIGN STORM EVENT IS 25-YEAR.
 3. OPENROADS DRAINAGE UTILITIES
 SOFTWARE USED FOR HYDRAULIC
 CALCULATIONS.
 4. OUTFALL TAILWATER WAS ASSUMED
 TO BE THE CREEK WSEL.

REV DESCRIPTION BY DATE



GEORGETOWN INTERSECTIONS

DRAINAGE CALCULATIONS

RPS Project No 008289					
csJ# CSJ 0337-01	-054				
Client I.D. CITY OF GEORGETOWN					
Drawn By PM	Checked By				
Scale	Sheet	Sheet No.			
Date 5/22/2025	2 OF 3	44			

Texas Co	mmission on Environmental Quality							
TSS Remo	val Calculations 04-20-2009			Project Name: Date Prepared:			enic and	Hwy 2
ddiffonali	information is provided for cells with a red triangl	o in the un	nor right o	omer Place the	ourner ov	or the c	all.	
	in blue indicate location of instructions in the Technica				cursor ov	er ute c	en.	
	shown in red are data entry fields.	Guidance	vialiuai - NG	1540.				
	shown in black (Bold) are calculated fields. Cha	nace to the	ee fielde v	dill remove the e	nuatione u	ead in t	he enres	deboo
, iidi acters	shown in black (Bold) are calculated lields. Cha	inges to the	se lielus v	AIII TEITIOVE DIE E	qua uons u	seu III I	ile spiea	usileel
. The Requir	red Load Reduction for the total project	Calculations fo	rom RG-348		Pages 3-27	0 3-30		
	Page 3-29 Equation 3.3: L _M =	27.2(A _N x P)						
where	LM TOTAL PROJECT =	Required TSS	removal resul	ting from the propose	d developmer	t = 80% (of increased	load
				area for the project				
	P =	Average annua	al precipitation	, inches				
Site Data	: Determine Required Load Removal Based on the Entire Project							
	County =		•					
	Total project area included in plan *=	1.83	acres					
	Predevelopment impervious area within the limits of the plan * = lost-development impervious area within the limits of the plan * =	1.42						
iotai p	Total post-development impervious cover fraction * =	0.81						
	P =	32	inc hes					
	LM TOTAL PROJECT =	53	lbs.					
The values	entered in these fields should be for the total project area							
Nu	imber of drainage basins / outfalls areas leaving the plan area =	3	`					
. Drainage E	Basin Parameters (This information should be provided for	each basinj:						
	Drainage Basin/Outfall Area No. =	DA1	•					
	The state of the s							
Desid	Total drainage basin/outfall area = levelopment impervious area within drainage basin/outfall area =	0.29 0.26	acres acres					
Post-d	levelopment impervious area within drainage basin/outfall area =	0.26	acres					
Post-deve	elopment impervious fraction within drainage basin/outfall area =	0.99						
	L _{M THIS BASIN} =	27	lbs.					
Calculate F	Fraction of Annual Runoff to Treat the drainage basin / out	all area	•					
	Desired L _{M THIS BASIN} =	56	lbs.					
he following	sections are used to calculate the required water quality	volume(s) for	the selected	BMP.				
	r BMP Types not selected in cell C45 will show NA.							
0. Stormcep	Required TSS Removal in BMP Drainage Area=	26.88	The					
	Impervious Cover Overtreatment=	0.0330	ac					
	TSS Removal for Uncaptured Area =	28.70	lbs					
	BMP Sizing Effective Area =	0.26	FA					
	Calculated Model Size(s) =	2400, 3600	EA					
	Actual Model Size (if multiple values provided in Calculated							
	Model Size or if you are choosing a larger model size) =	2400	Model Size					
	Surface Area =	50.27	m ²					
	Overflow Rate =	0.005638	Π* V _{or}					
	Rounded Overflow Rate =	0.005810	V _{or}					
	BMP Efficiency % =	80.00	%					
	L _R Value =	253.55	lbs					
	TSS Load Credit =	226.67	lbs					
	Is Sufficient Treatment Available? (TSS Credit > TSS Uncapt.)	Yes						
	o common readment Available (100 creat 2 100 thrapt.)	100						
	TSS Treatment by BMP (LM + TSS Uncapt.) =	55.58						

					008289 COG -		
SS Remov	val Calculations 04-20-2009			Project Name: Date Prepared:	Wildwood DR @ Williams Dr 5/13/2025		
dditional i	nformation is provided for cells with a red triang	la in the un	ar right o				
ext shown i	n blue indicate location of instructions in the Technica				cursor over the cell.		
	shown in red are data entry fields. shown in black (Bold) are calculated fields. Cha	inges to the	se fields v	vill remove the e	quations used in the spreadsheet.		
The Requir	ed Load Reduction for the total project	Calculations fr	om RG-348		Pages 3-27 to 3-30		
	Page 3-29 Equation 3.3: L _M =	27.2(A _N x P)					
where:	LM TOTAL PROJECT =	Required TSS	removal resu	Iting from the propose	d development = 80% of increased load		
	A _N =	Net increase in Average annua	n impervious	area for the project			
Site Data:	Determine Dequired and Demovel Based on the Entire Drain	• •					
	County = Total project area included in plan *=	Williamson	acres				
Total p	ost-development impervious area within the limits of the plan" =	0.28 0.32 0.49					
	Total post-development impervious cover fraction * = P =	32	inches				
	L _M TOTAL PROJECT		lbs.				
The values	entered in these fields should be for the total project area						
Nu	mber of drainage basins / outfalls areas leaving the plan area =	1					
Drainage B	asin Parameters (This information should be provided for	each basin):					
. Dramage D	Drainage Basin/Outfall Area No. =						
	Total drainage basin/outfall area =	0.64	acres				
Post-de	evel opment impervious area within drainage basin/outfall area = evel opment impervious area within drainage basin/outfall area =	0.28	acres acres				
Post-deve	opment impervious fraction within drainage basin/outfall area = L _{M THIS BASIN} =	0.50 32	lbs.				
. Indicate the	proposed BMP Code for this basin.						
	Proposed BMP =	Grassy Swale	perc ent				
	Removal efficiency =	/0	porcult		Aqualogic Cartridge Filter Bioretention		
					Contech StormFilter Constructed Wetland		
					Extended Detention Grassy Swale		
					Retention / Imgation Sand Filter		
					Stormceptor Vegetated Filter Strips		
					Vortechs Wet Basin Wet Vault		
Calculate N	laximum TSS Load Removed (L _R) for this Drainage Basin	by the selecte	ed BMP Typ	<u>e.</u>	vvc. vauit		
	RG-348 Page 3-33 Equation 3.7: L _R =	(BMP efficience	y) x P x (A ₁ :	34.6 + A _P x 0.54)			
where				in the BMP catchme			
	A _P =	Pervious area	remaining in	n the BMP catchment the BMP catchment a	rea		
	L _R =	TSS Load rem	oved from thi	s catchment area by t	he proposed BMP		
	A _C = A ₁ =		acres acres				
	Ap = Lp =	0.32	acres lbs				
	LR -	250	106				
Calculate E	raction of Annual Puroff to Treat the drainage basin / our	fall area					
i. Calculate F	raction of Annual Runoff to Treat the drainage basin / out		lhe				
i. Calculate F	Desired L _{M THIS BASIN} =	32	lbs.				
	Desired L _{M THIS} BASIN =	0.13		Calculations from RG	-34R	Pages 3	34 to 3.36
	Desired L _{M THIS BASIN} =	0.13		Calculations from RG	-348	Pages 3	34 to 3-36
	Desired L _{a This Basin} = F = apture Volume required by the BMP Type for this drain plan Post Development Runoff Coefficient =	0.13 e basin / outf	all area.	Calculations from RG	-346	Pages 3	-34 to 3-36
	Desired $L_{H HH S} s_{AG N}=F$ = apture Volume required by the BMP Type for this drainated by the BMP Type for this	0.13 e basin / outf	ali area.	Calculations from RG	5-346	Pages 3	34 to 3-36
	Desired L _{a This Basin} = F = apture Volume required by the BMP Type for this drain plan Post Development Runoff Coefficient =	0.13 10 basin / outs 0.07 0.35 58	all area, inches cubic feet	Calculations from RG	-348	Pages 3	34 to 3-36
	Desired L _{to This Basin} = F = apture Volume required by the BMP Type for this drainate Rainfall Depth = Post Development Rainfall Depth = Cn-site Water Quarity Volume = Off-site area draining to BMP =	32 0.13 10 basin / outs 0.07 0.35 58 Calculations fr	inches cubic feet om RG-348 acres		-346	Pages 3	34 to 3-36
	Desired L _{tr Hits} sum = F = apture Volume required by the SMP Type for this draina; Rainfall Depth = Post Development Runoff Coefficient = Christle Water Quarity Volume =	0.13 0.13 0.13 0.07 0.35 58 Calculations fr	inches cubic feet		-348	Pages 3	34 to 3-36
	Desired L _{M THIS} BASIN = F= apture Volume required by the BMP Type for this drainage Post Deseignent Basin Coefficient = On-site Water Quarity Volume Offsite area draining to BMP = Offsite importuse core draining to BMP	32 0.13 (e basin / outf 0.07 0.35 58 Calculations fr 0.00 0.00 0	inches cubic feet om RG-348 acres		-348	Pages 3	34 to 3-36
6. Calculate C	Desired L _{to This Basin} = F = apture Volume required by the BMP Type for this drainate Rainfall Deptin = Post Development Raunoff Dediciont = Ch-site Water Quarity Volume = Off-site improvious cover draining to BMP = Off-site improvious cover draining to BMP = Improvious Tack of off-site reare a Off-site Raunoff Coefficient = Off-site Raunoff Coefficient = Off-site Volume = Storage for Sediment =	32 0.13 (e basin / outs 0.07 0.35 58 Calc utations fr 0.00 0.00 0.00 0	inches cubic feet om RG-348 acres acres cubic feet	Pages 3-36 to 3-37	-348	Pages 3	34 to 3-36
. Calculate C	Desired L _{th THIS BASIN} = F = apture Volume required by the BMP Type for this drainate Rainfall Depth = Post Development Rainoff Coefficient = Cn-site Water Quarity Volume = Off-site improvius cover draining to BMP = Improvius Exicut of off-site area Off-site improvius Cover draining to BMP = Improvius Exicut of off-site area Coff-site Volume (Stagle Stagle Stagl	32 0.13 te basin / outf 0.07 0.35 58 Calc ulations fr 0.00 0.00 0.00 0	inches cubic feet om RG-348 acres acres cubic feet	Pages 3-36 to 3-37	-346	Pages 3	34 to 3 36
Total C:	Desired L _{to This Basin} = F = apture Volume required by the BMP Type for this drainate Rainfall Deptin = Post Development Raunoff Dediciont = Ch-site Water Quarity Volume = Off-site improvious cover draining to BMP = Off-site improvious cover draining to BMP = Improvious Tack of off-site reare a Off-site Raunoff Coefficient = Off-site Raunoff Coefficient = Off-site Volume = Storage for Sediment =	32 0.13 10 basin / outf 0.07 0.35 58 Calculations fi 0.00 0 0 0 12 c9 volume(s) for	inches cubic feet om RG-348 acres cubic feet cubic feet cubic feet cubic feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 3-36
Total C:	Desired L _{M THIS} BASIN ** F = apture Volume required by the BMP Type for this drainage apture. Volume required by the BMP Type for this drainage apture. Post Development Fund Conficient * On-site Valer Quarky Youture * Off-site inspervious root or draining to BMP > Off-site inspervious root or draining to BMP = Site of the	32 0.13 0.07 0.35 58 Calculations fr 0.00 0.00 0 12 e9 volume(s) for	inches cubic feet om RG-348 acres cubic feet cubic feet cubic feet cubic feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT			34 to 336
Total Ca The following the values in the sizing Revalues of the Sizi	Desired Let the BABP. Type for this drainate provided by the BMP Type for this drainate. Rainfall Depth = Post Development Rainoff Depth = Post Development Rainoff Dediciont = Ch-site Water Quarity Volume = Ch-site Water Quarity Volume = Of-Site Impervious size of draining to BMP = Impervious train of of-Site interes of Site Rainoff Dediciont = Of-Site Rainoff Dediciont = Of-Site Rainoff Dediciont = Of-Site Rainoff Dediciont = Of-Site Rainoff Dediciont = Size open to Site Rainoff Dediciont = Size open to Volume (train of Julius and Lain of Site Rainoff Dediciont = Size open to Volume (train of Julius and Lain of Site Rainoff Dediciont = Size open to Volume (train of Julius and Lain of Site Rainoff Dediciont = Size open to Volume (train of Julius and Lain of Site Rainoff Dediciont = Size open to Volume (train of Julius and Lain of Site Rainoff Dediciont = Size open to Volume (train of Julius and Lain of Site Rainoff Dediciont = Size open to Volume (train of Julius and Lain of Site Rainoff Dediciont = Size open to Volume and Lain of Size o	32 0.13 10 basin / outf 0.07 0.35 58 Calculations fi 0.00 0 0 0 12 c9 volume(s) for	inches cubic feet om RG-348 acres cubic feet cubic feet cubic feet cubic feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total Ca he follows for the values of the salzing R	Desired Let miss beam = F = apture Volume required by the BMP Type for this drainage	32 0.13 1e basin / outs 10.07 0.35 58 Calc utations fi 0.00 0 0.00 0 12 12 es volume(s) for	inches cubic feet om RG-348 acres acres cubic feet cubic feet cubic feet cubic feet see UPON the selecte acres acres	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total Ca he follows for the values of the salzing R	Desired Let THE BARN F F = appture Volume required by the BMP Type for this drainac Rainfall Depth = Post Development Raunof Coefficient = On-site Water Quarity Volume = Off-site provious cover draining to BMP = Off-site improvious brown of draining to BMP = improvious brown of ordinaring to BMP = Off-site improvious cover draining to BMP = Off-site improvious brown of draining to BMP = Improvious brown of ordinaring to BMP = Off-site Water Quarity Volume = Stonge for Sediment = Stonge for Sediment = Stonge for Sediment = Design parameters. for the Folia Ovinting BMP is I could PREMA BEET TYPES not selected in cell Cast via theory Ma. Design parameters. for the Swale = A = Improvious Cover in Draininge Area = Rainfall Intersity = 1	32 0.13 1e basin / outs 1e bas	all area. Inches cubic feet om RG-348 acres acres cubic feet cubic feet cubic feet cubic feet acres acres acres acres acres	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		-34 to 336
Total Ca The following the values in the sizing Revalues of the Sizi	Desired L _{M THES} BASIN ** F = apture Volume required by the BMP Type for this drainage Paintal Depth - Post Deseignment Random Condicions * On-site Water Quarky Youther On-site Water Quarky Youther Of-site Impervous zero carraing to BMP * Of-site Impervous record carraing to BMP * Solder Water Quarky Volume * Solder Water Quarky Volume * Solder Volume (Pregulated water quality volume(sy x.1.50) * sections are used to activate the required water quality BMP Types not selected in cell C48 will show this. COURTEMENTS FOR THE FOLL OWING BMP */ LOAD REINCAPES* Design parameters. At the same Design parameters for the service of the same carrain impervous Cooper in Chainspace Area impervous Chainspace Area impervous Cooper in Chai	32 0.13 1e basin / outil 1e basin / outi	inches cubic feet acres	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		-34 to 396
Total Ca The following the values in the sizing Revalues of the Sizi	Desired Let miss beans = F = apture Volume required by the BMP Type for this drainate Post Development Raunoff Coefficient = Ch-site Water Quarty Volume Stoogle Sr Sediment = Stoogle Sr Sediment = Stoogle Sr Sediment = Ch-site Water Quarty Volume Stoogle Sr Sediment = Stoogle Sr Se	32 0.13 19 basin / outil 10.07 0.35 58 Calc utations in 0.00 0.00 0 0 12 e9 volume(s) for vVALS ARE BA Designed as F	all area. inches cubic feet om RG-348 acres acres cubic feet cubic feet cubic feet secution feet cubic feet cubic feet fine secution Re acres acres in/fr fift ft	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		-34 to 396
Total Ca The following the values in the sizing Revalues of the Sizi	Desired Let miss beam F = apture Volume required by the BMP Type for this drainate Post Development Raunof Coefficient = On-site Valer Quality Volume = On-site Water Quality Volume = On-site Improvious Exite for of of site area of site of of site area of site of of site area of site of of site of	0.13 0.13 0.13 0.07 0.36 0.00 0.00 0.00 0.00 0.00 0.00 0.00	inches cubic feet om RG-348 acres acres cubic feet cubic feet the selectic the selectic the selectic through the through	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total Ca The following the values in the sizing Revalues of the Sizi	Desired Lamma Basin = F = apture Volume required by the BMP Type for this drainage Rainfall Depth = Post Designment Runoff Costicent = Consider Volume required for this drainage Round Costicent = Consider Volume Runoff Costicent = Consider Volume Runoff Costicent = Consider Volume Runoff Costicent = Offsite improvas teaching to BMP = Offsite improvas teaching of Settle area = Offsite Runoff Costicent = Consider Volume Runoff Costicent = Consider Volume Runoff Costicent = Support to Settlement = Support to Settlement Runoff Costicent = Support to Settlement = Support to Settlement Runoff Runoff Costicent = Support to Settlement Runoff Runo	32 0.13 0.13 0.07 0.07 0.05 98 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	all area, inches cubic feet om RG-348 acres acres cubic feet cubic feet cubic feet the selective acres acres from RG-348 the selective from RG-348 the selective the selec	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total Ca The following the values in the sizing Revalues of the Sizi	Desired Let miss beam F = apture Volume required by the BMP Type for this drainate Post Development Raunof Coefficient = On-site Valer Quality Volume = On-site Water Quality Volume = On-site Improvious Exite for of of site area of site of of site area of site of of site area of site of of site of	0.13 0.13 0.13 0.07 0.36 0.00 0.00 0.00 0.00 0.00 0.00 0.00	inches inches cubic feet cubic feet acres acres cubic feet cubic feet feet feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		-34 to 336
Total C. Galculate C	Desired Let miss beam? F = apture Volume required by the BMP Type for this drainage. Rainfall Depth = Post Development Rainoff Coefficient = On-site Water Quartity Volume = Off-site improvious price of off-site price off-site price of off-site price off-site price of off-site price of off-site price off-site price of off-site price of off-site price off-site price of off-site price off-site price of off-site price of off-site price off-site price off-site price of off-site price off-site pri	0.13 0.13 0.13 0.07 0.35 0.00 0.00 0.00 0.00 0.00 0.00 0.00	inches inches cubic feet cubic feet acres acres cubic feet cubic feet feet feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		94 to 936
Total C. Galculate C	Desired L _{M THES} BASIN = F = apture Volume required by the BMP Type for this drainage Paintell Depth = Post Development Hundf Coefficient = On-site Water Quarkly Youther Off-site brown desired post of the Water Quarkly Volume = Off-site browns desired post of the product some of t	0.13 0.13 0.13 0.07 0.35 0.00 0.00 0.00 0.00 0.00 0.00 0.00	inches inches cubic feet cubic feet acres acres cubic feet cubic feet feet feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		3410 336
Total C. Galculate C	Desired Let miss beam F = appture Volume required by the BMP Type for this drainate Post Development Raunof Coefficient = On-site Water Quarity Volume = Of-site Improvious Earth of Of-site Improvious Coefficient = Of-site Improvious Earth of Of-site Impr	0.13 0.13 0.13 0.07 0.35 0.00 0.00 0.00 0.00 0.00 0.00 0.00	inches inches cubic feet cubic feet acres acres cubic feet cubic feet feet feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total C. Galculate C	Desired Let miss bean = F = apture Volume required by the BMP Type for this drainage	0.13 0.13 0.13 0.07 0.35 88 0.00 0.00 0.00 0.00 0.00 0.00 0.00	all area. Inches Cubic feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total C. Galculate C	Desired L _{M THES} BASIN = F = apture Volume required by the BMP Type for this drainage Paintell Depth = Post Development Hundf Coefficient = On-site Water Quarkly Youther Off-site brown desired post of the Water Quarkly Volume = Off-site browns desired post of the product some of t	0.13 0.13 0.13 0.07 0.35 0.00 0.00 0.00 0.00 0.00 0.00 0.00	all area. Inches Cubic feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		-34 to 336
Total C. Galculate C	Desired L _{M THES} BASIN = F = apture Volume required by the BMP Type for this drainage and the second services of	0.13 0.13 0.13 0.07 0.35 88 0.00 0.00 0.00 0.00 0.00 0.00 0.00	inches cubic feet feet feet feet feet feet feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		-34 to 336
Total Calendate C Total	Desired Let mine Bean P F = apture Volume required by the BMP Type for this drainage Rainfail Depth = Post Designment Raunof Coefficient = Christia area draining to BMP = Offsite improvas cord crising to BMP = Offsite improvas cord crising to BMP = Offsite improvas record crising to BMP = Offsite viviant on offsite area = Offsite viviant or sold area and the sold crising to BMP = Sitropy for Sediment = Comment of the sold crising to BMP = Sitropy for Sediment = Design parameters for the swale. Drainage Area to be Treated by the Swale = A = improvious Coxen in Drainage Area = Residence of Swale Stope =	32 0.13 0.13 0.17 0.07 0.05 08 0.00 0.00 0.00 0.00 0.00 0.00 0	inches cubic feet feet feet feet feet feet feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		-34 to 336
Total Calendate C Total	Desired Lames BARN Free Factors and Service Services Serv	32 0.13 0.13 0.13 0.07 0.35 0.00 0.00 0.00 0.00 0.00 0.00 0.00	inches cubic feet feet feet feet feet feet feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total C. Total	Desired Let THE BARN F F = apture Volume required by the BMP Type for this drainage Randia Depth = Post Deelopment Randia Oxficert = Chreste Water Quarky Volume = Storage for Sections are used to catculate the required water quarky columne(gx 1,20) = sections are used to catculate the required water quarky columne(gx 1,20) = sections are used to catculate the required water quarky columne(gx 1,20) = sections are used to catculate the required water quarky Collision (graph of the Chrester Ch	32 0.13 0.13 0.13 0.07 0.35 0.00 0.00 0.00 0.00 0.00 0.00 0.00	all area. Inches Cubic feet acres acres acres cubic feet cubic feet cubic feet feet feet feet feet	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total C. Total	Designed Let miles assays = appliers Volume required by the BMP Type for this drainage and the provided by the BMP Type for this drainage and the provided by the BMP Type for this drainage and the provided by the BMP Type for this drainage and the provided by the BMP Type for this drainage and the provided by the pro	32 0.13 0.13 0.07 0.07 0.35 58 Calc utations ft 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	all area. Inches Cubic feet acres acres cubic feet Cubic fee	Pages 3-36 to 3-37 If BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM		34 to 336
Total C. Total	Desired L _{th mid-Basin} = F = apture Volume required by the BMP Type for this drainage and the second control of the second control	32 0.13 0.13 0.13 0.07 0.05 98 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	all area. Inches cubic feet feet feet feet freet freet freet freet freet freet	Pages 3-36 to 5-37 d BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM Pages 3-51 to 3-54		34 to 336
Total C. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. C	Desired L _{th Third BASIN} = F= apture Volume required by the BMP Type for this drainag Post Designent Running Countries Charlet Water Quarity Volume = On-site Water Quarity Volume = Off-site impervious record craining to BMP = Off-site value countries Size to Sediment of Sediment Off-site value Off-site value (and Code vill show Vill SMP Types not selected in cell CdS vill show Vill SMP Types not selected in cell cdS vill show Vill SMP Types not selected vill show Vill SMP Types not selected in cell	32 0.13 0.13 0.13 0.07 0.05 98 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	all area. Inches cubic feet feet feet feet freet freet freet freet freet freet	Pages 3-36 to 5-37 d BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM Pages 3-51 to 3-54		-34 10 336
Total C. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. C	Desired L _{th Third BASIN} = F = apture Volume required by the BMP Type for this drainage of the second series of t	32 0.13 0.13 0.13 0.07 0.05 98 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	all area. Inches cubic feet feet feet feet freet freet freet freet freet freet	Pages 3-36 to 5-37 d BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM Pages 3-51 to 3-54		3410 336
Total C. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. C	Desired L _{bt mids} beans = F = apture. Volume required by the BMP Type for this drainage and the property of the BMP Type for this drainage in the property of the BMP Type for this drainage. Post of the property of the pro	32 0.13 0.07 0.07 0.38 68 Calc utations ft 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	all area. Inches cubic feet cubic feet cubic feet cubic feet cubic feet acres	Pages 3-35 to 3-37 il Burp. FLOW RATES - NOT 3-3-36	CALCULATED WATER QUALITY VOLUM Pages 3-51 to 3-54 st be modified and the solver rerun.		3410 336
Total C. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. C	Desired Laminus Basis = F = apture Volume required by the BMP Type for this drainage	0.13 0.13 0.13 0.13 0.07 0.35 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 12 49 volume(s) for 10 0.00 0.00 0.00 0.00 0.00 1.1.1 0.02.83 0.33 0.33 0.43 0.74 3.42 0.22 0.22 0.24 0.36 0.74 0.46 0.36 0.74 0.74 0.76	all area. Inches cubic feet acres	Pages 3-36 to 5-37 d BMP. FLOW RATES - NOT	CALCULATED WATER QUALITY VOLUM Pages 3-51 to 3-54		34 to 336
Total C. Catculate C. Co calculate C. Co calculate C.	Desired Lymers Basin = F = apture Volume required by the BMP Type for this drainage	32 0.13 0.13 0.13 0.07 0.05 0.00 0.00 0.00 0.00 0.00 0.00	all area. Inches cubic feet acres	Pages 3-35 to 3-37 il Burp. FLOW RATES - NOT 3-3-36	CALCULATED WATER QUALITY VOLUM Pages 3-51 to 3-54 st be modified and the solver rerun.		34 to 336
Total C. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. St. Calculate C. Total C. C	Desired Lammes Basin = F = apture Volume required by the BMP Type for this drainage	32 0.13 0.13 0.13 0.07 0.05 0.00 0.00 0.00 0.00 0.00 0.00	all area. Inches cubic feet acres	Pages 3-35 to 3-37 il Burp. FLOW RATES - NOT 3-3-36	CALCULATED WATER QUALITY VOLUM Pages 3-51 to 3-54 st be modified and the solver rerun.		34 to 336



CPS

Texas PE Firm Reg. #F-929

4801 Southwest Pkwy, Pkwy 1, Sulte 185, Austin, Texas 78735 T+1 512 328 5771 E usinfrastructure@rpsgroup.com

REV DESCRIPTION BY DATE



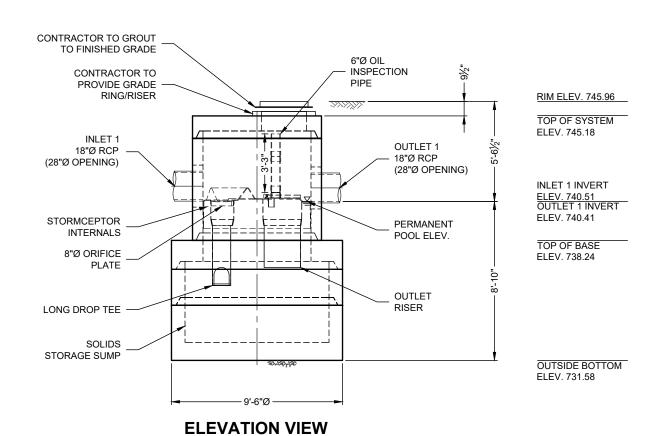
GEORGETOWN INTERSECTIONS

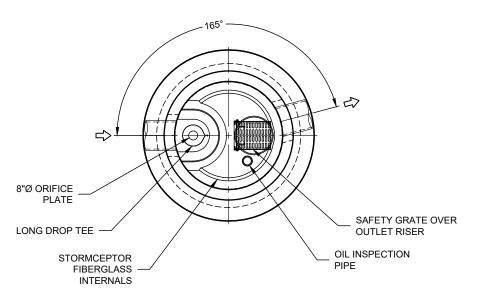
DRAINAGE CALCULATIONS

RPS Project	No	0082	89				
CSJ#	CSJ	0337	-01	-054	ı		
Client I.D.	CITY	OF	GEO	RGE1	OWN	1	
Drawn By	₽М			Checi	ked E	ly	
Scale				Sheet		Sheet No.	
Date 5/	22/2	025		3	OF	3	45

PLAN VIEW INTERNALS NOT SHOWN

CONTRACTOR TO INSTALL LONG DROP TEE AND OUTLET RISER EXTENSION ON-SITE. SEE INSTALLATION INSTRUCTIONS





SECTION FOR PIPE ORIENTATION

TOP SLAB NOT SHOWN

MATERIAL LIST - PROVIDED BY CONTECH

COUNT	DESCRIPTION	INSTALLED BY
1	STC2400 6'Ø FIBERGLASS SINGLE INSERT, STD	CONTECH
1	FIBERGLASS LONG DROP TEE	CONTRACTOR
1	FIBERGLASS 24"Ø OUTLET RISER EXTENSION PIPE	CONTRACTOR
1	FIBERGLASS 8"Ø ORIFICE PLATE	CONTRACTOR
1	ALUMINUM 24" SAFETY GRATE	CONTECH
2	6"Ø PVC OIL INSPECTION PIPE	CONTECH
1	6"Ø PVC COUPLING	CONTECH
1	SEALANT FOR JOINTS	CONTRACTOR
1	30"Ø X 4" FRAME AND COVER, EJ #41600483, OR EQUIV.	CONTRACTOR

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH REPRESENTATIVE. www.ContechES.com
- 3. STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS
- 4. STRUCTURE SHALL MEET AASHTO HS-20 LOAD RATING, ASSUMING EARTH COVER OF 0' 2', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.
- 5. STORMCEPTOR STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C-478 AND AASHTO LOAD FACTOR DESIGN METHOD.

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMCEPTOR MANHOLE STRUCTURE
- C. CONTRACTOR TO ADD JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS, AND ASSEMBLE STRUCTURE.
 D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH PIPE INVERTS WITH ELEVATIONS SHOWN. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES.
- E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

STRUCTURE WEIGHT APPROXIMATE HEAVIEST PICK = 20000 LBS.

MAXIMUM FOOTPRINT = 9.50'Ø

CONTECH **PROPOSAL DRAWING**

3/21/2025 MAA I.JK QUENCE No 844460 10

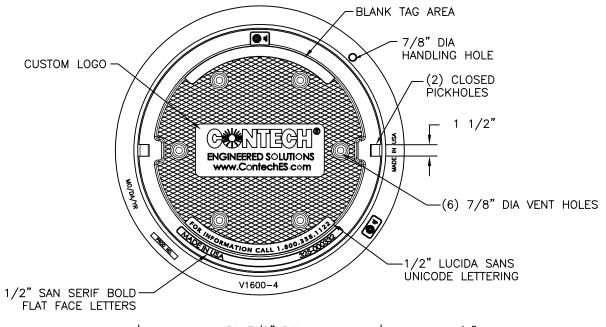
STC2400 - 844460-10 SCENIC DR @ SH 29 GEORGETOWN, TX E DESIGNATION: STC 2

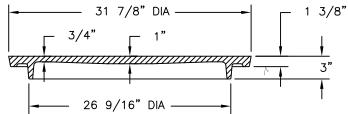
SITE

Stormceptor

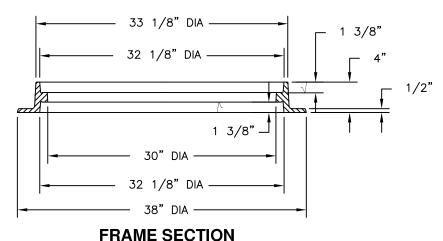
LAYOUT 1A/1A

1810B4 V1600-4 Assembly





COVER SECTION





Product Number 41600483

Design Features

-Materials Cover Gray Iron (CL35B) Frame Gray Iron (CL35B)

-Design Load

Heavy Duty

-Open Area

n/a

-Coating

Undipped

- √ Designates Machined Surface

Certification

.

-ASTM A48

-Country of Origin: USA

Major Components

00180783 41600410

Drawing Revision

05/09/2007 Designer: SMH 6/26/2017 Revised By: DAE

Disclaimer

Weights (lbs./kg) dimensions (inches/mm) and drawings provided for your guidance. We reserve the right to modify specifications without prior notice.

CONFIDENTIAL: This drawing is the property of EJ GROUP, Inc., and embodies confidential information, registered marks, patents, trade secret information, and/or know how that is the property of EJ GROUP, Inc. Copyright © 2012 EJ GROUP, Inc. All rights reserved.

Contact

800 626 4653 ejco.com

Attachment G – Inspection, Maintenance, Repair and Retrofit Plan for Stormceptor BMP

The Stormceptor BMP will be maintained by the City of Georgetown per their standard right-of-way maintenance agreements and procedures.

Once installed, the Stormceptor (STC 2400) will require the following maintenance procedures and guidelines based on the Stormceptor® STC Operation and Maintenance Guide (Contech Engineered Solutions LLC, 2020)

Sediment Removal

When the sediment captured in the system reaches a depth of 12 inches (300 mm), a vacuum truck shall be utilized to remove the sediment from the system. The sediment depth shall be measured from the surface using a dipstick tube equipped with a ball valve inserted through the riser pipe, thereby eliminating the need for confined space entry. Prior to sludge removal, any water in the unit shall be decanted in accordance with local regulations. Maintenance shall be performed once the sediment depth exceeds the 12-inch guideline.

Oil Removal

The depth of oil captured by the system shall be determined by inserting a dipstick in the oil cleanout port. Oil shall be removed through the oil inspection/cleanout port, using a small portable pump as necessary to expedite removal. Alternatively, oil may be removed from the 24-inch opening if water is removed from the lower chamber to lower the oil level below the drop pipes.

Inspection

The Stormceptor shall be inspected annually for sediment buildup, oil accumulation, and any signs of physical damage or corrosion. Based on local conditions, the frequency of inspection may be adjusted—if the unit fills with sediment more quickly than projected, inspections may be conducted semi-annually; once the site has stabilized, inspections may be performed every two to three years.

• Disposal

Requirements for the disposal of material from the Stormceptor System are like that of any other stormwater Best Management Practice (BMP) where permitted. Disposal options for the sediment may range from disposal in a sanitary trunk sewer upstream of a sewage treatment plant, to disposal in a sanitary landfill site. Petroleum waste products collected in the Stormceptor (free oil/chemical/fuel spills) should be removed by a licensed waste management company by hand or with flat-bottomed shovels.

Maintenance Records

All inspections, maintenance, and repair activities shall be documented and maintained in a binder labeled "Inspection, Maintenance, Repair and Retrofit Plan for Scenic Dr at Sh29 Intersection Improvements." This record shall be kept at the City of Georgetown offices and be available for TCEQ review.

Md	Kamrul	Islam	PΕ
IAICT	reallil ut	isiaiii.	1 . L.

Engineer

City of Georgetown representative

06/12/2025.

Date

Sion

Date

Agent Authorization Form

For Required Signature
Edwards Aquifer Protection Program
Relating to 30 TAC Chapter 213
Effective June 1, 1999

	Dustin Clarkson	- 1
	Print Name	
	CIP Project Manager	
	Title - Owner/President/Other	
of	City of Georgetown Corporation/Partnership/Entity Name	
have authorized	Md Kamrul Islam, P.E. Print Name of Agent/Engineer	
of	Tetratech	
	Print Name of Firm	

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

- 1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
- 2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
- Application fees are due and payable at the time the application is submitted. The
 application fee must be sent to the TCEQ cashier or to the appropriate regional office.
 The application will not be considered until the correct fee is received by the
 commission.
- 4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
- 5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:

Applicant's Signature

3/25/2025

Date

THE STATE OF <u>Texas</u> §
County of <u>Williamson</u> §

BEFORE ME, the undersigned authority, on this day personally appeared ______known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this 25th day of March ,2025

CHRISTOPHER BRIAN MAYS
Notary Public, State of Texas
Comm. Expires 08-02-2028
Notary ID 135021722

NOTARY PUBLIC

Christopher Brian Mays

Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 8-2-2028

Application Fee Form

Texas Commission on Environmental Quality

Name of Proposed Regulated Entity: Georgetown Intersection Improvements – Scenic Drive at Regulated Entity Location: Scenic Dr at SH 29 Name of Customer: City of Georgetowm Contact Person: Md Kamrul Islam, P.E. Phone: (210) 299-7912 Customer Reference Number (if issued):CN 600412043 Regulated Entity Reference Number (if issued):RN **Austin Regional Office (3373)** Hays Travis ✓ Williamson San Antonio Regional Office (3362) Bexar Medina Uvalde Comal Kinney Application fees must be paid by check, certified check, or money order, payable to the **Texas** Commission on Environmental Quality. Your canceled check will serve as your receipt. This form must be submitted with your fee payment. This payment is being submitted to: X Austin Regional Office San Antonio Regional Office Mailed to: TCEQ - Cashier Overnight Delivery to: TCEQ - Cashier **Revenues Section** 12100 Park 35 Circle Mail Code 214 Building A, 3rd Floor P.O. Box 13088 Austin, TX 78753 Austin, TX 78711-3088 (512)239-0357 Site Location (Check All That Apply): Recharge Zone **Contributing Zone Transition Zone** Type of Plan Size Fee Due Water Pollution Abatement Plan, Contributing Zone Plan: One Single Family Residential Dwelling \$ Acres Water Pollution Abatement Plan, Contributing Zone Plan: Multiple Single Family Residential and Parks Acres Water Pollution Abatement Plan, Contributing Zone Plan: Non-residential Acres \$ Sewage Collection System L.F. Lift Stations without sewer lines Acres | \$ Tanks \$ Underground or Aboveground Storage Tank Facility Piping System(s)(only) Each 1 Each | \$ 500 Exception **Extension of Time** Each | \$

Signature: _____ Date: <u>05/14/2025</u>

Application Fee Schedule

Texas Commission on Environmental Quality

Edwards Aquifer Protection Program 30 TAC Chapter 213 (effective 05/01/2008)

Water Pollution Abatement Plans and Modifications

Contributing Zone Plans and Modifications

	Project Area in	
Project	Acres	Fee
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5	\$1,500
	5 < 10	\$3,000
	10 < 40	\$4,000
	40 < 100	\$6,500
	100 < 500	\$8,000
	≥ 500	\$10,000
Non-residential (Commercial, industrial,	< 1	\$3,000
institutional, multi-family residential, schools, and	1 < 5	\$4,000
other sites where regulated activities will occur)	5 < 10	\$5,000
	10 < 40	\$6,500
	40 < 100	\$8,000
	≥ 100	\$10,000

Organized Sewage Collection Systems and Modifications

Project	Cost per Linear Foot	Minimum Fee- Maximum Fee
Sewage Collection Systems	\$0.50	\$650 - \$6,500

Underground and Aboveground Storage Tank System Facility Plans and Modifications

Project	Cost per Tank or Piping System	Minimum Fee- Maximum Fee
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500

Exception Requests

Project	Fee
Exception Request	\$500

Extension of Time Requests

Project	Fee				
Extension of Time Request	\$150				



TCEQ Core Data Form

For detailed instructions on completing this form, please read the Core Data Form Instructions or call 512-239-5175.

SECTION I: General Information

1. Reason for	r Submission (If other is check	ed please descr	ibe in space provid	led.)						
New Pern	nit, Registration or Authorizatio	n (<i>Core Data Fo</i>	rm should be subn	nitted with	the prog	ram application.)				
Renewal	(Core Data Form should be subi	mitted with the	renewal form)			ther				
2. Customer	Follow this link t		3. Re	3. Regulated Entity Reference Number (if issued)						
CN 600412043 for CN or RN number Central Registry*					RN	RN				
SECTIO	N II: Custome	r Infor	<u>mation</u>							
4. General Customer Information 5. Effective Date for Customer Information						Updates (mm/dd,	[/] yyyy)			
New Custor	mer \square	Update to Cust	omer Information		Char	nge in Regulated En	tity Owne	ership		
☐Change in Le	egal Name (Verifiable with the	Texas Secretary	of State or Texas C	omptrolle	r of Public	Accounts)				
The Custome	r Name submitted here ma	y be updated	automatically be	ased on t	what is c	urrent and active	with th	ne Texas Seci	retary of State	
(SOS) or Texa	s Comptroller of Public Acc	ounts (CPA).								
6. Customer	Legal Name (If an individual, p	orint last name j	first: eg: Doe, John,)		If new Customer,	enter pre	evious Custom	er below:	
City of George	town									
7. TX SOS/CPA Filing Number 8. TX Sta			e Tax ID (11 digits	9. Federal Tax II (9 digits)			D	10. DUNS Number (if applicable)		
11. Type of C	Customer: Corpo	ration			☐ Individ	ndividual Partne		rship: Gen	neral Limited	
	☐ County ☐ Federal [Local Sta	te 🗌 Other		Sole P	☐ Sole Proprietorship ☐ Other:				
	of Employees					13. Independe	ntly Ow	ned and Ope	erated?	
		1-500	1 and higher			Yes	□ No			
14. Customer	r Role (Proposed or Actual) – a	s it relates to th	e Regulated Entity	listed on	this form.	l Please check one o	f the follo	wing		
Owner	Operator al Licensee Responsible	_	Owner & Operator VCP/BSA Applicar	nt		Other:				
	<u> </u>									
15. Mailing										
Address:	au l					1			T	
	City		State		ZIP			ZIP + 4		
16. Country N	Mailing Information (if outside	de USA)		17.	E-Mail A	ddress (if applicab	le)			
18 Telenhon	a Number		19 Extension o	r Code		20 Eav N	lumbar	(if annlicable)		

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SECTION III:	Regula	ated Ent	tity Infor	mat	<u>tion</u>					
21. General Regulated En	itity Informa	tion (If 'New Re	gulated Entity" is sel	ected, a	new pe	rmit applica	ation is al	so required.)		
☑ New Regulated Entity ☐ Update to Regulated Entity Name ☐ Update to Regulated Entity Information										
The Regulated Entity Nar as Inc, LP, or LLC).	ne submitte	d may be upda	ited, in order to m	eet TC	EQ Cor	e Data Stai	ndards (removal of o	rganizatior	nal endings such
22. Regulated Entity Nam	ne (Enter nam	e of the site whe	re the regulated acti	on is ta	king pla	ce.)				
Georgetown Intersection Imp	provements –	Scenic Drive at S	H 29							
23. Street Address of the Regulated Entity:										
(No PO Boxes)	City		State			ZIP			ZIP + 4	
24. County							I		1	1
If no Street Address is provided, fields 25-28 are required.										
25. Description to	Canada Danat	CH 20 Lata and all								
Physical Location:	Scenic Dr at	SH 29 Intersection	on							
26. Nearest City State Nearest ZIP Code										
Georgetown TX 78626										
_	Latitude/Longitude are required and may be added/updated to meet TCEQ Core Data Standards. (Geocoding of the Physical Address may be used to supply coordinates where none have been provided or to gain accuracy).									
27. Latitude (N) In Decim	al:	30.633166			28. Lo	ongitude (V	W) In De	cimal:	-97.6840	70
Degrees	Minutes		Seconds		Degre	es		Minutes	_	Seconds
29. Primary SIC Code	30.	Secondary SIC	Code			y NAICS Co	ode	32. Seco	ondary NAI	CS Code
(4 digits)	(4 d	igits)		(5 c	or 6 digit	S)		(5 or 6 di	igits)	
33. What is the Primary E	Business of t	his entity? (D	o not repeat the SIC	or NAI	CS descri	ption.)				
	_									
24 Mailine										
34. Mailing										
Address:	City		State			ZIP			ZIP + 4	
35. E-Mail Address:							1			
36. Telephone Number			37. Extension o	r Code	1	38. F	ax Num	ber (if applica	ble)	
() -						() -			

39. TCEQ Programs and ID Numbers Check all Programs and write in the permits/registration numbers that will be affected by the updates submitted on this form. See the Core Data Form instructions for additional guidance.

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☐ Dam Safety		Districts	Edwards Aquifer			Emissions Inventory Air	☐ Industrial Hazardous Waste	
☐ Municipal Solid Waste ☐ New Source Review Air ☐ OSSF			Petroleum Storage Tank		☐ PWS			
Sludge		Storm Water	☐ Title V Air		Tires		Used Oil	
☐ Voluntary Cleanup		Wastewater	☐ Wastewater Agricul	ture	ure Water Rights		Other:	
SECTION IV: Preparer Information								
40. Name: Md Kamrul Islam, P.E.			41. Title:		Project Manager			

40. Name:	Md Kamrul Islam, P.E.			41. Title:	Project Manager
42. Telephone Number 43. Ext./Code 44. Fax Number			44. Fax Number	45. E-Mail <i>i</i>	Address
(512)299-7912			() -	KAMRUL.ISL	AM@tetratech.com

SECTION V: Authorized Signature

46. By my signature below, I certify, to the best of my knowledge, that the information provided in this form is true and complete, and that I have signature authority to submit this form on behalf of the entity specified in Section II, Field 6 and/or as required for the updates to the ID numbers identified in field 39.

Company:	RPS Infrastructure, Inc.	lanager			
Name (In Print):	Md Kamrul Islam, P.E.	Phone:	(512) 299- 7912		
Signature:				Date:	05/23/2025

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