
Water Pollution Abatement Plan & Organized Sewage Collection System Application

O'REILLY AUTO PARTS

Prepared for:

O'REILLY AUTO ENTERPRISES, LLC
233 S. PATTERSON AVENUE
SPRINGFIELD, MO 65802

Prepared by:

Bowman



Marisa Keiser

August 4, 2025

Bowman Consulting Group, Ltd. • 807 Las Cimas Parkway, Las Cimas II, Suite 350 •

Austin, Texas 78746 • P: 512.327.1180

TBPE Firm No. 14309

AUGUST 4, 2025

Water Pollution Abatement Plan Checklist

- **Edwards Aquifer Application Cover Page (TCEQ-20705)**
- **General Information Form (TCEQ-0587)**
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 - Attachment B - USGS / Edwards Recharge Zone Map
 - Attachment C - Project Description
- **Geologic Assessment Form (TCEQ-0585)**
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- **Temporary Stormwater Section (TCEQ-0602)**
 - Attachment A - Spill Response Actions
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 - Attachment C - Sequence of Major Activities
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 - Attachment G - Drainage Area Map
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 - Attachment D - BMPs for Surface Streams
 - Attachment E - Request to Seal Features (if sealing a feature)
 - Attachment F - Construction Plans
 - Attachment G - Inspection, Maintenance, Repair and Retrofit Plan
 - Attachment H - Pilot-Scale Field Testing Plan (if proposed)
 - Attachment I - Measures for Minimizing Surface Stream Contamination

- **Agent Authorization Form (TCEQ-0599), if application submitted by agent**
- **Application Fee Form (TCEQ-0574)**
- **Check Payable to the “Texas Commission on Environmental Quality”**
- **Core Data Form (TCEQ-10400)**

Organized Sewage Collection System Plan Checklist

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 - Attachment B - Stratigraphic Column
 - Attachment C - Site Geology
 - Attachment D - Site Geologic Map(s)
- **Organized Sewage Collection System Plan (TCEQ-0582)**
 - Attachment A - SCS Engineering Design Report
 - Attachment B - Justification and Calculations for Deviation in Straight Alignment Without Manholes
 - Attachment C - Justification for Variance from Maximum Manhole Spacing
 - Attachment D – Calculations for Slopes for Flows Greater Than 10.0 Feet Per Second Site Plan
 - Final Plan and Profile Sheets
- **Lift Station / Force Main System Application (TCEQ-0624) if applicable**
 - Attachment A - Engineering Design Report
 - Site Plan
 - Final Plan and Profile Sheets
- **Temporary Stormwater Section (TCEQ-0602)**
 - Attachment A - Spill Response Actions
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1 – TCEQ - 20705 Attachments

Edwards Aquifer Application Cover Page

Texas Commission on Environmental Quality

Edwards Aquifer Application Cover Page

Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with [30 TAC 213](#).

Administrative Review

1. [Edwards Aquifer applications](#) must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.

To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: <http://www.tceq.texas.gov/field/eapp>.

2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.

An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.

5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
6. If the geologic assessment was completed before October 1, 2004 and the site contains “possibly sensitive” features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

Technical Review

1. When an application is deemed administratively complete, the technical review period begins. The regional office will distribute copies of the application to the identified affected city, county, and groundwater conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.

3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited**.
4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

Mid-Review Modifications

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a “Mid-Review Modification”. Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ’s Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ’s San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: O'Reilly Auto Parts					2. Regulated Entity No.:				
3. Customer Name: Rick Hicks					4. Customer No.:				
5. Project Type: (Please circle/check one)	New		Modification			Extension		Exception	
6. Plan Type: (Please circle/check one)	WPAP 	CZP	SCS 	UST	AST	EXP	EXT	Technical Clarification	Optional Enhanced Measures
7. Land Use: (Please circle/check one)	Residential		Non-residential 			8. Site (acres):			1.41
9. Application Fee:	\$4,650		10. Permanent BMP(s):				Jellyfish Device		
11. SCS (Linear Ft.):	184.0		12. AST/UST (No. Tanks):				N/A		
13. County:	Williamson		14. Watershed:				North Fork San Gabriel River		

Application Distribution

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the “Texas Groundwater Conservation Districts within the EAPP Boundaries” map found at:

http://www.tceq.texas.gov/assets/public/compliance/field_ops/eapp/EAPP%20GWCD%20map.pdf

For more detailed boundaries, please contact the conservation district directly.

Austin Region			
County:	Hays	Travis	Williamson
Original (1 req.)	—	—	X
Region (1 req.)	—	—	X
County(ies)	—	—	X
Groundwater Conservation District(s)	☐ Edwards Aquifer Authority ☐ Barton Springs/ Edwards Aquifer ☐ Hays Trinity ☐ Plum Creek	☐ Barton Springs/ Edwards Aquifer	NA
City(ies) Jurisdiction	☐ Austin ☐ Buda ☐ Dripping Springs ☐ Kyle ☐ Mountain City ☐ San Marcos ☐ Wimberley ☐ Woodcreek	☐ Austin ☐ Bee Cave ☐ Pflugerville ☐ Rollingwood ☐ Round Rock ☐ Sunset Valley ☐ West Lake Hills	☐ Austin ☐ Cedar Park ☐ Florence ☒ Georgetown ☐ Jerrell ☐ Leander ☐ Liberty Hill ☐ Pflugerville ☐ Round Rock

San Antonio Region					
County:	Bexar	Comal	Kinney	Medina	Uvalde
Original (1 req.)	—	—	—	—	—
Region (1 req.)	—	—	—	—	—
County(ies)	—	—	—	—	—
Groundwater Conservation District(s)	☐ Edwards Aquifer Authority ☐ Trinity-Glen Rose	☐ Edwards Aquifer Authority	☐ Kinney	☐ EAA ☐ Medina	☐ EAA ☐ Uvalde
City(ies) Jurisdiction	☐ Castle Hills ☐ Fair Oaks Ranch ☐ Helotes ☐ Hill Country Village ☐ Hollywood Park ☐ San Antonio (SAWS) ☐ Shavano Park	☐ Bulverde ☐ Fair Oaks Ranch ☐ Garden Ridge ☐ New Braunfels ☐ Schertz	NA	☐ San Antonio ETJ (SAWS)	NA

I certify that to the best of my knowledge, that the application is complete and accurate. This application is hereby submitted to TCEQ for administrative review and technical review.

Marisa Keiser

Print Name of Customer/Authorized Agent

Marisa Keiser

August 4, 2025

Signature of Customer/Authorized Agent

Date

****FOR TCEQ INTERNAL USE ONLY****

Date(s) Reviewed:		Date Administratively Complete:	
Received From:		Correct Number of Copies:	
Received By:		Distribution Date:	
EAPP File Number:		Complex:	
Admin. Review(s) (No.):		No. AR Rounds:	
Delinquent Fees (Y/N):		Review Time Spent:	
Lat./Long. Verified:		SOS Customer Verification:	
Agent Authorization Complete/Notarized (Y/N):		Fee Check:	Payable to TCEQ (Y/N):
Core Data Form Complete (Y/N):			Signed (Y/N):
Core Data Form Incomplete Nos.:			Less than 90 days old (Y/N):

2 – TCEQ - 0587 Attachments

General Information Form

General Information Form

Texas Commission on Environmental Quality

For Regulated Activities on the Edwards Aquifer Recharge and Transition Zones and Relating to 30 TAC §213.4(b) & §213.5(b)(2)(A), (B) Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **General Information Form** is hereby submitted for TCEQ review. The application was prepared by:

Print Name of Customer/Agent: Marisa Keiser, PE

Date: 07/25/2025

Signature of Customer/Agent:

Marisa Keiser

Project Information

1. Regulated Entity Name: O'Reilly Auto Parts
2. County: Williamson
3. Stream Basin: Brazos River
4. Groundwater Conservation District (If applicable): _____
5. Edwards Aquifer Zone:
☒ Recharge Zone
☐ Transition Zone
6. Plan Type:
☒ WPAP
☒ SCS
☐ Modification

- ☐ AST
☐ UST
☐ Exception Request

7. Customer (Applicant):

Contact Person: Rick Hicks

Entity: O'Reilly Auto Enterprises, LLC

Mailing Address: 233 S Patterson Avenue

City, State: Springfield, MO

Zip: 65802

Telephone: 417-427-8062 EXT65680

FAX: _____

Email Address: rhicks28@oreillyauto.com

8. Agent/Representative (If any):

Contact Person: Marisa Keiser

Entity: Bowman Consulting Group

Mailing Address: 807 Las Cimas Pkwy, Las Cimas II, Suite 350, Austin, TX 78746

City, State: Austin, Texas

Zip: 78746

Telephone: 512.327.1180

FAX: _____

Email Address: mkeiser@bowman.com

9. Project Location:

☒ The project site is located inside the city limits of Georgetown.

☐ The project site is located outside the city limits but inside the ETJ (extra-territorial jurisdiction) of _____.

☐ The project site is not located within any city's limits or ETJ.

10. ☒ The location of the project site is described below. The description provides sufficient detail and clarity so that the TCEQ's Regional staff can easily locate the project and site boundaries for a field investigation.

The project site is located 5510 Williams Drive, Georgetown, Texas 78633

Property ID R476274.

11. ☒ **Attachment A – Road Map.** A road map showing directions to and the location of the project site is attached. The project location and site boundaries are clearly shown on the map.

12. ☒ **Attachment B - USGS / Edwards Recharge Zone Map.** A copy of the official 7 ½ minute USGS Quadrangle Map (Scale: 1" = 2000') of the Edwards Recharge Zone is attached. The map(s) clearly show:

☒ Project site boundaries.

☒ USGS Quadrangle Name(s).

☒ Boundaries of the Recharge Zone (and Transition Zone, if applicable).

☐ Drainage path from the project site to the boundary of the Recharge Zone.

13. ☒ **The TCEQ must be able to inspect the project site or the application will be returned.** Sufficient survey staking is provided on the project to allow TCEQ regional staff to locate the boundaries and alignment of the regulated activities and the geologic or manmade features noted in the Geologic Assessment.

☒ Survey staking will be completed by this date: _____

14. ☒ **Attachment C – Project Description.** Attached at the end of this form is a detailed narrative description of the proposed project. The project description is consistent throughout the application and contains, at a minimum, the following details:

- ☒ Area of the site
- ☒ Offsite areas
- ☒ Impervious cover
- ☒ Permanent BMP(s)
- ☒ Proposed site use
- ☒ Site history
- ☒ Previous development
- ☒ Area(s) to be demolished

15. Existing project site conditions are noted below:

- ☐ Existing commercial site
- ☐ Existing industrial site
- ☐ Existing residential site
- ☐ Existing paved and/or unpaved roads
- ☐ Undeveloped (Cleared)
- ☒ Undeveloped (Undisturbed/Uncleared)
- ☐ Other: _____

Prohibited Activities

16. ☒ I am aware that the following activities are prohibited on the Recharge Zone and are not proposed for this project:

- (1) Waste disposal wells regulated under 30 TAC Chapter 331 of this title (relating to Underground Injection Control);
- (2) New feedlot/concentrated animal feeding operations, as defined in 30 TAC §213.3;
- (3) Land disposal of Class I wastes, as defined in 30 TAC §335.1;
- (4) The use of sewage holding tanks as parts of organized collection systems; and
- (5) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41(b), (c), and (d) of this title (relating to Types of Municipal Solid Waste Facilities).
- (6) New municipal and industrial wastewater discharges into or adjacent to water in the state that would create additional pollutant loading.

17. ☐ I am aware that the following activities are prohibited on the Transition Zone and are not proposed for this project:

- (1) Waste disposal wells regulated under 30 TAC Chapter 331 (relating to Underground Injection Control);

- (2) Land disposal of Class I wastes, as defined in 30 TAC §335.1; and
- (3) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41 (b), (c), and (d) of this title.

Administrative Information

18. The fee for the plan(s) is based on:

- ☒ For a Water Pollution Abatement Plan or Modification, the total acreage of the site where regulated activities will occur.
- ☒ For an Organized Sewage Collection System Plan or Modification, the total linear footage of all collection system lines.
- ☐ For a UST Facility Plan or Modification or an AST Facility Plan or Modification, the total number of tanks or piping systems.
- ☐ A request for an exception to any substantive portion of the regulations related to the protection of water quality.
- ☐ A request for an extension to a previously approved plan.

19. ☒ Application fees are due and payable at the time the application is filed. If the correct fee is not submitted, the TCEQ is not required to consider the application until the correct fee is submitted. Both the fee and the Edwards Aquifer Fee Form have been sent to the Commission's:

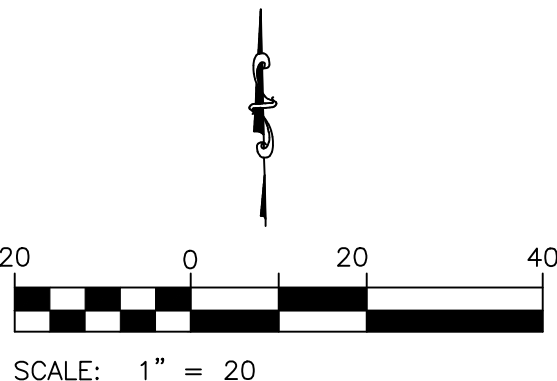
- ☐ TCEQ cashier
- ☒ Austin Regional Office (for projects in Hays, Travis, and Williamson Counties)
- ☐ San Antonio Regional Office (for projects in Bexar, Comal, Kinney, Medina, and Uvalde Counties)

20. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

21. ☒ No person shall commence any regulated activity until the Edwards Aquifer Protection Plan(s) for the activity has been filed with and approved by the Executive Director.

Attachment A – Road Map

V:\250519 - O'Reilly's Auto Parts\250519-01-002 (ENG) - Georgetown, TX (GX3)\Engineering\Design Documents\TCEQ\WPAP\ General Information Form TCEQ 0587\250519-01-002_XBASE.dwg, ATTACHMENT A - ROAD MAP, June 05, 2025, 2:07 PM, herica.silva



O'REILLY'S AUTO PARTS
WPAP ATTACHMENT A - ROAD MAP



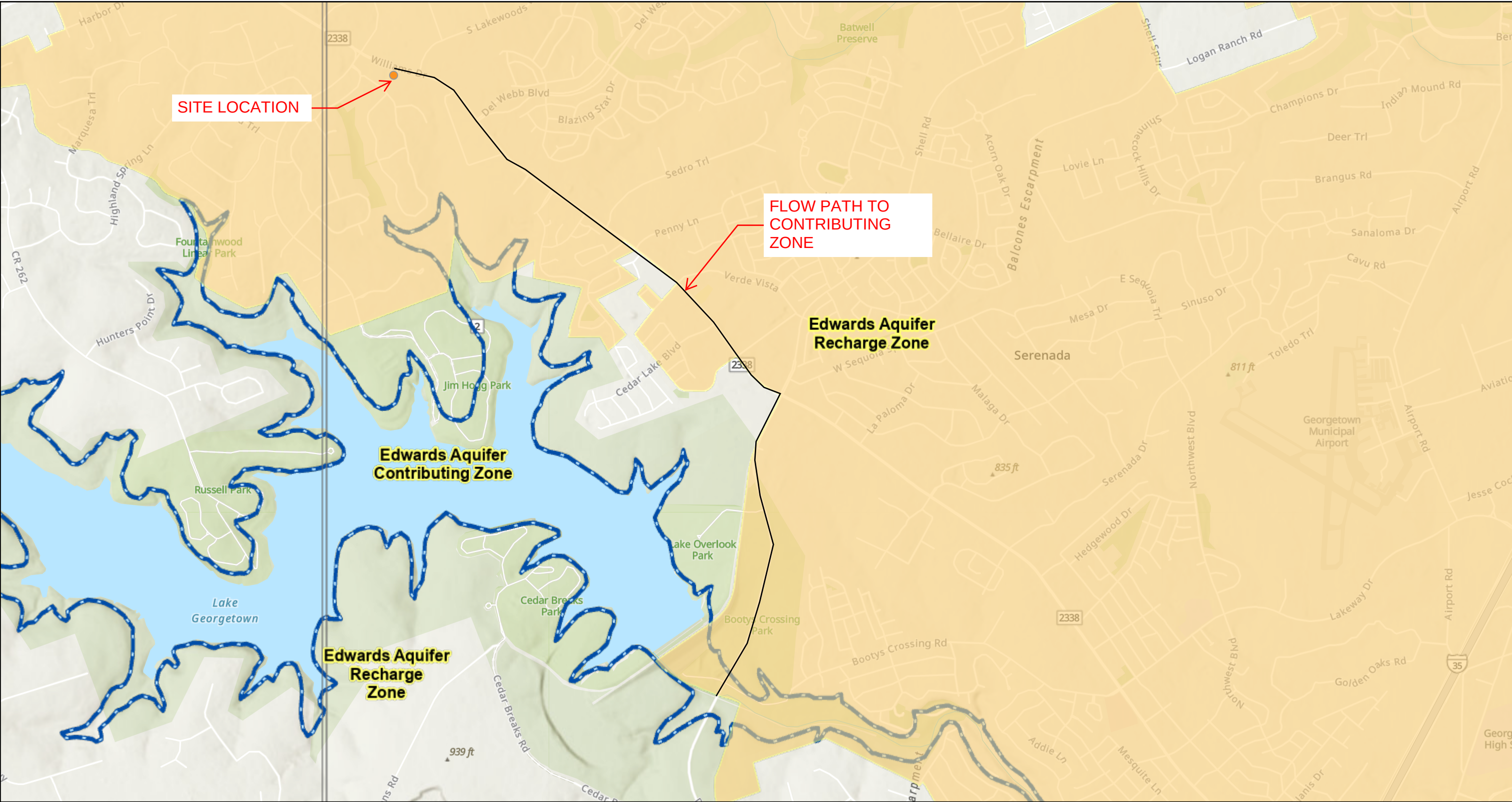
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Phone: (512) 327-1180 • Fax: (512) 327-4062 • www.bowman.com
TBPE Firm Registration No. F-14309

THIS DOCUMENT IS RELEASED FOR
THE PURPOSE OF INTERIM REVIEW,
MARK-UP, AND/OR DRAFTING
UNDER THE AUTHORITY OF
MARISA KEISER, P.E.
#141397
ON Jun. D, YY.
IT IS NOT TO BE USED FOR
CONSTRUCTION, BIDDING, OR
PERMIT PURPOSES.

PRELIMINARY NOT FOR CONSTRUCTION

Attachment B – USGS/Edwards Aquifer Recharge Zone Map

Edwards Aquifer Viewer Custom Print



7/25/2025, 3:35:55 PM

1:30,146

- ArcGIS World Geocoding Service

TCEQ_EDWARDS_OFFICIAL_MAPS

7.5 Minute Quad Grid

TX Counties

City/Place

Edwards Aquifer Boundary central line

Edwards Aquifer Boundary

Edwards Aquifer Label

World_Hillshade
- Esri, NASA, NGA, USGS, FEMA, Sources: Esri, TomTom, Garmin, FAO, NOAA, USGS, (c) OpenStreetMap contributors, and the GIS User Community, TCEQ

TCEQ-0587

Attachment C – Project Description

This WPAP application covers a total area of approximately 1.41 Acres per the Fountainwood Plaza Plat boundary.

The project is an Auto Parts Sales commercial development which includes utility improvements, parking lot, detention and water quality ponds and commercial building to serve O'Reilly Auto Parts. There are no areas being demolished as the area is currently undeveloped and vacant. Approximately 35,100SF of impervious cover is being proposed for this development. There will be 1 permanent BMP(s) on site and the runoff from the proposed impervious cover is captured in the sand filter pond to achieve 89% TSS removal efficiency. The pond can be found in the construction plans included in this application.

Project Information:

Limits of Construction	=	1.16 Acres
Total Project Site Area	=	1.41 Acres
Total Disturbance Area	=	1.16 Acres
Total Impervious cover	=	0.67 Acres
	=	47.51%

Total water quality treatment flow for:

JELLYFISH = 0.62 CFS

3 – TCEQ - 0585 Attachments

Geological Assessment Form

GEOLOGIC ASSESSMENT

O'Reilly Auto Enterprises, LLC – Georgetown GX3

Williamson County Property ID R476274
5510 Williams Drive
Georgetown, Texas 78633

Prepared for:

O'Reilly Auto Enterprises, LLC
233 S Patterson Ave
Springfield, Missouri 65802

Prepared by:

Bowman

151 Stagecoach Trail, Suite 130
San Marcos, Texas 78666
Phone: 512.396.2600
TBPB Firm Registration #50698



Project Number: 250519-01-002

Report Issuance Date: June 3, 2025

Geologic Assessment

Texas Commission on Environmental Quality

For Regulated Activities on The Edwards Aquifer Recharge/transition Zones and Relating to 30 TAC §213.5(b)(3), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

Print Name of Geologist: Gilberto S. Barajas

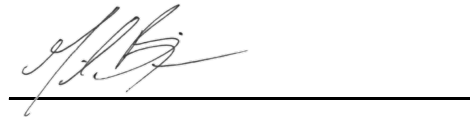
Telephone: (737) 315-3013

Date: 2 June 2025

Fax: _____

Representing: Bowman (TBPG 50698) (Name of Company and TBPG or TBPE registration number)

Signature of Geologist:



Regulated Entity Name: Georgetown GX3



Project Information

1. Date(s) Geologic Assessment was performed: 5/8/2025

2. Type of Project:

☒ WPAP
☐ SCS

☐ AST
☐ UST

3. Location of Project:

☒ Recharge Zone
☐ Transition Zone
☐ Contributing Zone within the Transition Zone

4. ☒ **Attachment A - Geologic Assessment Table.** Completed Geologic Assessment Table (Form TCEQ-0585-Table) is attached.
5. ☒ Soil cover on the project site is summarized in the table below and uses the SCS Hydrologic Soil Groups* (Urban Hydrology for Small Watersheds, Technical Release No. 55, Appendix A, Soil Conservation Service, 1986). If there is more than one soil type on the project site, show each soil type on the site Geologic Map or a separate soils map.

Table 1 - Soil Units, Infiltration Characteristics and Thickness

Soil Name	Group*	Thickness(feet)
DnB - Denton silty clay, 1 to 3 percent slopes	D	< 3.0
DoC - Doss silty clay, moist, 1 to 5 percent slopes	D	< 3.0

Soil Name	Group*	Thickness(feet)

** Soil Group Definitions (Abbreviated)*

- A. Soils having a high infiltration rate when thoroughly wetted.
- B. Soils having a moderate infiltration rate when thoroughly wetted.
- C. Soils having a slow infiltration rate when thoroughly wetted.
- D. Soils having a very slow infiltration rate when thoroughly wetted.

6. ☒ **Attachment B – Stratigraphic Column.** A stratigraphic column showing formations, members, and thicknesses is attached. The outcropping unit, if present, should be at the top of the stratigraphic column. Otherwise, the uppermost unit should be at the top of the stratigraphic column.
7. ☒ **Attachment C – Site Geology.** A narrative description of the site specific geology including any features identified in the Geologic Assessment Table, a discussion of the potential for fluid movement to the Edwards Aquifer, stratigraphy, structure(s), and karst characteristics is attached.
8. ☒ **Attachment D – Site Geologic Map(s).** The Site Geologic Map must be the same scale as the applicant's Site Plan. The minimum scale is 1": 400'
- Applicant's Site Plan Scale: 1" = 33'
- Site Geologic Map Scale: 1" = 33'
- Site Soils Map Scale (if more than 1 soil type): 1" = 33'
9. Method of collecting positional data:
- ☒ Global Positioning System (GPS) technology.
- ☐ Other method(s). Please describe method of data collection: _____

10. ☒ The project site and boundaries are clearly shown and labeled on the Site Geologic Map.
11. ☒ Surface geologic units are shown and labeled on the Site Geologic Map.
12. ☐ Geologic or manmade features were discovered on the project site during the field investigation. They are shown and labeled on the Site Geologic Map and are described in the attached Geologic Assessment Table.
- ☒ Geologic or manmade features were not discovered on the project site during the field investigation.
13. ☒ The Recharge Zone boundary is shown and labeled, if appropriate.
14. All known wells (test holes, water, oil, unplugged, capped and/or abandoned, etc.): If applicable, the information must agree with Item No. 20 of the WPAP Application Section.
- ☐ There are _____ (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply.)
- ☐ The wells are not in use and have been properly abandoned.
- ☐ The wells are not in use and will be properly abandoned.
- ☐ The wells are in use and comply with 16 TAC Chapter 76.
- ☒ There are no wells or test holes of any kind known to exist on the project site.

Administrative Information

15. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

APPENDIX A

Geologic Assessment Table

APPENDIX B

Stratigraphic Column

STRATIGRAPHIC COLUMN

Period	Epoch	Stratigraphic Unit	Hydrologic Unit	Thickness (ft)
Quaternary		Alluvium (Qal)		Up to 30
Cretaceous	Upper	Taylor Clay (Ktl)	Confining Units	330
		Austin Chalk (Kau)		430
		Eagle Ford Shale (Kef)		30-40
		Buda Limestone (Kbu)		20-35
		Del Rio Clay (Kdr)		60-75
	Lower	Georgetown Limestone (Kgt)	Edwards Aquifer	Up to 100
		Edwards Formation (Ked)		Up to 200
		Comanche Peak Formation (Kcp)	Edwards Aquifer and Confining Unit in areas without caves	80
		Walnut Formation (Kw)		Up to 120
		Upper Glen Rose member (Kgrr)	Upper Trinity Aquifer	Up to 480

Gray shaded cells highlight lithologies underlying the site. Stratigraphic column based on The University of Texas at Austin Bedrock Geology of Round Rock and Surrounding Areas, Williamson and Travis Counties, Texas by Todd B. Housh, PhD, PG, 2007.

APPENDIX C

Site Geology

EXECUTIVE SUMMARY

Bowman has conducted a Geologic Assessment (GA) for the site identified as Georgetown GX3 in Williamson County, Texas, per Edwards Aquifer rules. No sensitive geologic features, sensitive manmade features, nor potential pathways for contaminant movement to the Edwards Aquifer were identified on the site.

INTRODUCTION

This GA narrative has been prepared for the Georgetown GX3 site (hereafter, referred to as the "site") by Bowman (Texas Board of Professional Geoscientists [TBPG] Registration Number 50698). Bowman believes this GA, which includes Texas Commission on Environmental Quality (TCEQ) Geologic Assessment Form TCEQ-0585, satisfies the requirements of the Edwards Aquifer rules set forth in Title 30 Texas Administrative Code (TAC) Chapter 213. This GA was completed for the approximately 1.36-acre site located at 5510 Williams Drive Georgetown, Williamson County, Texas (Williamson County Property ID R476274). The site is in the Edwards Aquifer Recharge Zone. The site consists of undeveloped grassland with a sparse compilation of Plateau live oak (*Quercus fusiformis*), Ashe juniper (*Juniperus ashei*), and cedar elm (*Ulmus crassifolia*). The purpose of this GA is to determine which, if any, geologic and manmade features on the site are sensitive, to identify all potential pathways on the site for contaminant movement to the Edwards Aquifer, and to provide sufficient geologic information so that the appropriate Best Management Practices (BMPs) can be proposed in the Edwards Aquifer Protection Plan (EAPP).

METHODOLOGY

This GA was prepared by Gilberto Steven Barajas, a qualified geologist per Rules and Technical Guidance for the Edwards Aquifer Protection Program and Professional Geoscientist (License #15461), of Bowman with assistance from Nick Gladstone, a Project Manager, of Bowman. This GA was prepared using TCEQ ***Instructions to Geologists for Geologic Assessments on the Edwards Aquifer Recharge/Transition Zones*** (Rev. 10-01-04).

Published reports and area geology maps were studied for information regarding caves or other karst features and to evaluate land uses on the site and in proximity to the site. A field survey was then conducted which included a general visual inspection of the site as well as a more detailed, systematic visual inspection in which transects were walked at 50-foot intervals. The purpose of walking the transects is to identify features within varying topography and vegetation. To define the type and extent of a feature, if any, various techniques were utilized where applicable including non-conductive probing, hand clearing/hand excavation, and sensory analysis to assess air flow. A return field

survey was not necessary and was not conducted. Similarly, tests such as excavation, cave mapping, infiltrometer tests, geophysical studies, or tracer studies for geologic assessment of features were not necessary and were not conducted. The GA did not include a path of proposed sewer lines as there are no proposed sewer lines for this project.

RESULTS

Soils

Soil information is based on data from the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS) Conservation Engineering Division's *Urban Hydrology for Small Watersheds* Technical Release Number 55 (TR-55). The majority of the site is underlain by the Denton silty clay, 1 to 3 percent slopes (DnB), and a relatively small area in the southern portion of the site is underlain by the Doss silty clay, 1 to 5 percent slopes (DoC). A map depicting the soils underlying the site is shown in Figure 1. Both (BtE) and (Kr) soils have very slow infiltration rate when thoroughly wetted (Soil Group D). The typical profile for the DnB soil unit, from ground surface to subsurface, is approximately 0 to 33 inches of silty clay and 3 inches of gravelly silty clay. The typical profile for the DoC soil unit, from ground surface to subsurface, is approximately 17 inches of silty clay. The underlying material for DnB is silty and clayey slope alluvium over residuum weathered from limestone. The underlying material for DoC is residuum weathered from limestone.

Geology and Hydrogeology

The site is underlain by the Georgetown Formation (Kgt) in the Washita Group overlying the Cretaceous-age Edwards Limestone (Ked) Formation in the Fredricksburg Group. The Kgt Formation is characterized by limestone and marl; mostly limestone, fine grained, argillaceous, nodular, moderately indurated, light gray; some shale, marly, soft, light gray to yellowish gray. The Kgt Formation is the uppermost unit of the Edwards Aquifer and is generally considered non-cavernous, though several caves have been mapped within this Formation where it thinly covers the Ked Formation. The Ked Formation is characterized by limestone, dolomite, and chert; limestone is characteristically aphanitic to fine grained, massive to thin bedded, hard, brittle, in part rudistid biostromes, in part miliolid biosparite; dolomite is characteristically fine to very fine grained, porous, medium gray to grayish brown. The Ked Formation is among the most extensively karstified and cavernous rock units within the Austin and San Antonio regions; karst development in the Ked is generally lateral and associated with cave, sinkhole, and solution fracture development.

Observations made during the field survey agree with the published classification of the site in the Edwards Aquifer Recharge Zone as defined by the TCEQ. Based on topography,

surface soils, and lack of geologic or manmade features, precipitation likely gathers on-site and flows south-southeast to an Unnamed Tributary of the North Fork San Gabriel River. Geologic or manmade features mapped on-site were not found in publications. Furthermore, the Texas Water Development Board (TWDB) database was searched for active and plugged water wells, and there were no wells mapped on-site. However, there was a water well report for a domestic-use well approximately 0.25 miles west of the site (State Well Number 5819402). The well was reportedly drilled to approximately 110 feet below ground surface (ft-bgs) and is not expected to increase the rate of recharge to the subsurface at the site.

Feature Descriptions

No sensitive geologic features, sensitive manmade features, nor potential pathways for contaminant movement to the Edwards Aquifer were identified on the site based on published reports and field survey data.

CONCLUSION

A GA was conducted at the site per Edwards Aquifer rules. No sensitive geologic features, sensitive manmade features, nor potential pathways for contaminant movement to the Edwards Aquifer were identified for this GA. If features are discovered during subsequent site activity, they should be investigated by a qualified geologist to determine if action is necessary. However, as of the date of this report, Bowman recommends no further assessments or testing to determine if sensitive geologic features, sensitive manmade features, or potential pathways for contaminant movement to the Edwards Aquifer exist.

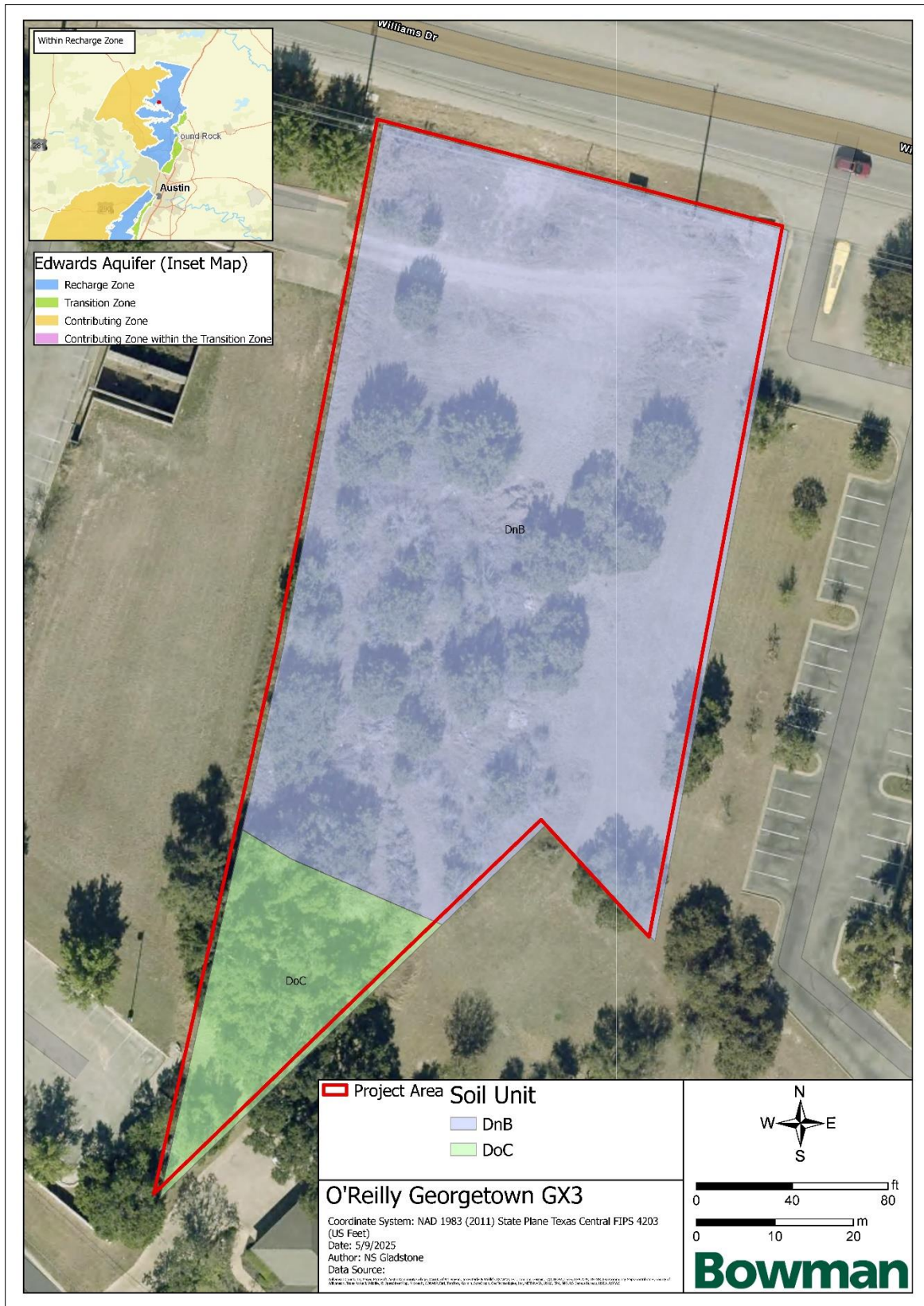
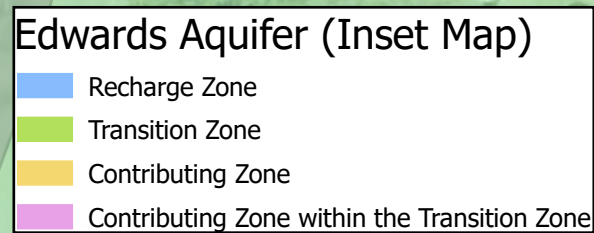
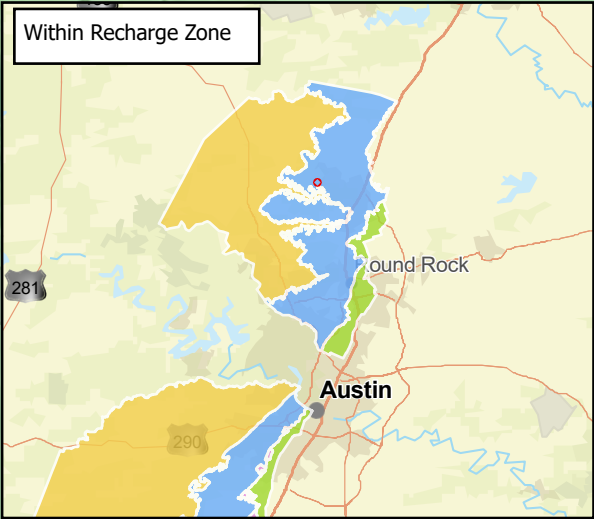


Figure 1. Map of soils underlying the site.

APPENDIX D

Site Geologic Map

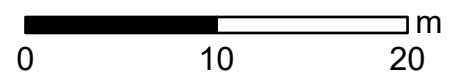
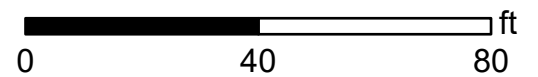


- Project Area
- Georgetown Formation

O'Reilly Georgetown GX3

Coordinate System: NAD 1983 (2011) State Plane Texas Central FIPS 4203 (US Feet)
Date: 5/9/2025
Author: NS Gladstone
Data Source:

Williamson County TX, Maec, Microsoft, Austin Community College, County of Williamson, Texas Parks & Wildlife, CONANP, Esri, TomTom, Garmin, FAO, NOAA, USGS, EPA, NPS, USFWS, Esri Community Maps Contributors, County of Williamson, Texas Parks & Wildlife, © OpenStreetMap, Microsoft, CONANP, Esri, TomTom, Garmin, SafeGraph, GeoTechnologies, Inc, METI/NASA, USGS, EPA, NPS, US Census Bureau, USDA, USFWS



Bowman

4– TCEQ - 0582 Attachments

Organized Sewage Collection System Plan

Organized Sewage Collection System Application

Texas Commission on Environmental Quality

For Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(c), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Regulated Entity Name: O'Reilly Auto Parts

1. ☒ **Attachment A – SCS Engineering Design Report.** This Engineering Design Report is provided to fulfill the requirements of 30 TAC Chapter 217, including 217.10 of Subchapter A, §§217.51 – 217.70 of Subchapter C, and Subchapter D as applicable, and is required to be submitted with this SCS Application Form.

Customer Information

2. The entity and contact person responsible for providing the required engineering certification of testing for this sewage collection system upon completion (including private service connections) and every five years thereafter to the appropriate TCEQ region office pursuant to 30 TAC §213.5(c) is:

Contact Person: Rick Hicks

Entity: O'Reilly Auto Parts

Mailing Address: 233 S Patterson Ave

City, State: Springfield, MO

Zip: 65802

Telephone: (417) 427-8062 Ext: 65680

Fax: _____

Email Address: rhicks28@oreillyauto.com

The appropriate regional office must be informed of any changes in this information within 30 days of the change.

3. The engineer responsible for the design of this sewage collection system is:

Contact Person: Marisa Keiser

Texas Licensed Professional Engineer's Number: 141397

Entity: Bowman Consulting Group

Mailing Address: 807 Las Cimas Pkwy, Suite 350

City, State: Austin, TX

Zip: 78746

Telephone: (512) 327-1180

Fax: _____

Email Address: mkeiser@bowman.com

Project Information

4. Anticipated type of development to be served (estimated future population to be served, plus adequate allowance for institutional and commercial flows):

- ☐ Residential: Number of single-family lots: _____
☐ Multi-family: Number of residential units: _____
☒ Commercial
☐ Industrial
☐ Off-site system (not associated with any development)
☐ Other: _____

5. The character and volume of wastewater is shown below:

100% Domestic _____ gallons/day
_____% Industrial _____ gallons/day
_____% Commingled 125 gallons/day
Total gallons/day: 125

6. Existing and anticipated infiltration/inflow is _____ gallons/day. This will be addressed by: _____.

7. A Water Pollution Abatement Plan (WPAP) is required for construction of any associated commercial, industrial or residential project located on the Recharge Zone.

- ☐ The WPAP application for this development was approved by letter dated _____. A copy of the approval letter is attached.
☒ The WPAP application for this development was submitted to the TCEQ on _____, but has not been approved.
☐ A WPAP application is required for an associated project, but it has not been submitted.
☐ There is no associated project requiring a WPAP application.

8. Pipe description:

Table 1 - Pipe Description

<i>Pipe Diameter(Inches)</i>	<i>Linear Feet (1)</i>	<i>Pipe Material (2)</i>	<i>Specifications (3)</i>
4	184	PVC SDR 26	ASTM D3034

Total Linear Feet: 184

- (1) Linear feet - Include stub-outs and double service connections. Do not include private service laterals.
(2) Pipe Material - If PVC, state SDR value.
(3) Specifications - ASTM / ANSI / AWWA specification and class numbers should be included.

9. The sewage collection system will convey the wastewater to the City of Georgetown (name) Treatment Plant. The treatment facility is:

- ☒ Existing
☐ Proposed

10. All components of this sewage collection system will comply with:

- ☒ The City of Georgetown standard specifications.
☐ Other. Specifications are attached.

11. ☒ No force main(s) and/or lift station(s) are associated with this sewage collection system.
☐ A force main(s) and/or lift station(s) is associated with this sewage collection system and the **Lift Station/Force Main System Application** form (TCEQ-0624) is included with this application.

Alignment

12. ☒ There are no deviations from uniform grade in this sewage collection system without manholes and with open cut construction.
13. ☒ There are no deviations from straight alignment in this sewage collection system without manholes.
- ☐ **Attachment B - Justification and Calculations for Deviation in Straight Alignment without Manholes.** A justification for deviations from straight alignment in this sewage collection system without manholes with documentation from pipe manufacturer allowing pipe curvature is attached.
- ☐ For curved sewer lines, all curved sewer line notes (TCEQ-0596) are included on the construction plans for the wastewater collection system.

Manholes and Cleanouts

14. ☒ Manholes or clean-outs exist at the end of each sewer line(s). These locations are listed below: (Please attach additional sheet if necessary)

Table 2 - Manholes and Cleanouts

<i>Line</i>	<i>Shown on Sheet</i>	<i>Station</i>	<i>Manhole or Clean-out?</i>
A	12 Of 25	N/A	Cleanout
A	12 Of 25	N/A	Manhole
	Of		
	Of		
	Of		
	Of		
	Of		

<i>Line</i>	<i>Shown on Sheet</i>	<i>Station</i>	<i>Manhole or Clean-out?</i>
	Of		
	Of		
	Of		

15. ☒ Manholes are installed at all Points of Curvature and Points of Termination of a sewer line.

16. ☒ The maximum spacing between manholes on this project for each pipe diameter is no greater than:

Pipe Diameter (inches)	Max. Manhole Spacing (feet)
6 - 15	500
16 - 30	800
36 - 48	1000
≥54	2000

☐ **Attachment C – Justification for Variance from Maximum Manhole Spacing.** The maximum spacing between manholes on this project (for each pipe diameter used) is greater than listed in the table above. A justification for any variance from the maximum spacing is attached, and must include a letter from the entity which will operate and maintain the system stating that it has the capability to maintain lines with manhole spacing greater than the allowed spacing.

17. ☐ All manholes will be monolithic, cast-in-place concrete.

☐ The use of pre-cast manholes is requested for this project. The manufacturer's specifications and construction drawings, showing the method of sealing the joints, are attached.

Site Plan Requirements

Items 18 - 25 must be included on the Site Plan.

18. ☒ The Site Plan must have a minimum scale of 1" = 400'.

Site Plan Scale: 1" = 20'.

19. ☒ The Site Plan must include the sewage collection system general layout, including manholes with station numbers, and sewer pipe stub outs (if any). Site plan must be overlain by topographic contour lines, using a contour interval of not greater than ten feet and showing the area within both the five-year floodplain and the 100-year floodplain of any drainage way.

20. Lateral stub-outs:

☐ The location of all lateral stub-outs are shown and labeled.

☒ No lateral stub-outs will be installed during the construction of this sewer collection system.

21. Location of existing and proposed water lines:

- ☐ The entire water distribution system for this project is shown and labeled.
- ☒ If not shown on the Site Plan, a Utility Plan is provided showing the entire water and sewer systems.
- ☐ There will be no water lines associated with this project.

22. 100-year floodplain:

- ☒ After construction is complete, no part of this project will be in or cross a 100-year floodplain, either naturally occurring or manmade. (Do not include streets or concrete-lined channels constructed above of sewer lines.)
- ☐ After construction is complete, all sections located within the 100-year floodplain will have water-tight manholes. These locations are listed in the table below and are shown and labeled on the Site Plan. (Do not include streets or concrete-lined channels constructed above sewer lines.)

Table 3 - 100-Year Floodplain

<i>Line</i>	<i>Sheet</i>	<i>Station</i>
	of	to
	of	to
	of	to
	of	to

23. 5-year floodplain:

- ☒ After construction is complete, no part of this project will be in or cross a 5-year floodplain, either naturally occurring or man-made. (Do not include streets or concrete-lined channels constructed above sewer lines.)
- ☐ After construction is complete, all sections located within the 5-year floodplain will be encased in concrete or capped with concrete. These locations are listed in the table below and are shown and labeled on the Site Plan. (Do not include streets or concrete-lined channels constructed above sewer lines.)

Table 4 - 5-Year Floodplain

<i>Line</i>	<i>Sheet</i>	<i>Station</i>
	of	to
	of	to
	of	to
	of	to

- 24. ☒ Legal boundaries of the site are shown.
- 25. ☒ The ***final plans and technical specifications*** are submitted for the TCEQ's review. Each sheet of the construction plans and specifications are dated, signed, and sealed by the Texas Licensed Professional Engineer responsible for the design on each sheet.

Items 26 - 33 must be included on the Plan and Profile sheets.

26. ☒ All existing or proposed water line crossings and any parallel water lines within 9 feet of sewer lines are listed in the table below. These lines must have the type of pressure rated pipe to be installed shown on the plan and profile sheets. Any request for a variance from the required pressure rated piping at crossings must include a variance approval from 30 TAC Chapter 290.

☐ There will be no water line crossings.

☒ There will be no water lines within 9 feet of proposed sewer lines.

Table 5 - Water Line Crossings

<i>Line</i>	<i>Station or Closest Point</i>	<i>Crossing or Parallel</i>	<i>Horizontal Separation Distance</i>	<i>Vertical Separation Distance</i>
A	Manhole	Crossing	13.8'	1.14

27. Vented Manholes:

☒ **No part** of this sewer line is within the 100-year floodplain and vented manholes are not required by 30 TAC Chapter 217.

☐ **A portion** of this sewer line is within the 100-year floodplain and vented manholes will be provided at less than 1500 foot intervals. These water-tight manholes are listed in the table below and labeled on the appropriate profile sheets.

☐ **A portion** of this sewer line is within the 100-year floodplain and an alternative means of venting shall be provided at less than 1500 feet intervals. A description of the alternative means is described on the following page.

☐ **A portion** of this sewer line is within the 100-year floodplain; however, there is no interval longer than 1500 feet located within. No vented manholes will be used.

Table 6 - Vented Manholes

<i>Line</i>	<i>Manhole</i>	<i>Station</i>	<i>Sheet</i>

<i>Line</i>	<i>Manhole</i>	<i>Station</i>	<i>Sheet</i>

28. Drop manholes:

- ☒ There are no drop manholes associated with this project.
- ☐ Sewer lines which enter new or existing manholes or "manhole structures" higher than 24 inches above the manhole invert are listed in the table below and labeled on the appropriate profile sheets. These lines meet the requirements of 30 TAC §217.55(l)(2)(H).

Table 7 - Drop Manholes

<i>Line</i>	<i>Manhole</i>	<i>Station</i>	<i>Sheet</i>

29. Sewer line stub-outs (For proposed extensions):

- ☐ The placement and markings of all sewer line stub-outs are shown and labeled.
- ☒ No sewer line stub-outs are to be installed during the construction of this sewage collection system.

30. Lateral stub-outs (For proposed private service connections):

- ☒ The placement and markings of all lateral stub-outs are shown and labeled.
- ☐ No lateral stub-outs are to be installed during the construction of this sewage collection system.

31. Minimum flow velocity (From Appendix A)

- ☒ Assuming pipes are flowing full; all slopes are designed to produce flows equal to or greater than 2.0 feet per second for this system/line.

32. Maximum flow velocity/slopes (From Appendix A)

- ☒ Assuming pipes are flowing full, all slopes are designed to produce maximum flows of less than or equal to 10 feet per second for this system/line.
- ☐ **Attachment D – Calculations for Slopes for Flows Greater Than 10.0 Feet per Second.** Assuming pipes are flowing full, some slopes produce flows which are greater than 10 feet per second. These locations are listed in the table below. Calculations are attached.

Table 8 - Flows Greater Than 10 Feet per Second

<i>Line</i>	<i>Profile Sheet</i>	<i>Station to Station</i>	<i>FPS</i>	<i>% Slope</i>	<i>Erosion/Shock Protection</i>

33. Assuming pipes are flowing full, where flows are ≥ 10 feet per second, the provisions noted below have been made to protect against pipe displacement by erosion and/or shock under 30 TAC §217.53(l)(2)(B).

- ☐ Concrete encasement shown on appropriate Plan and Profile sheets for the locations listed in the table above.
- ☐ Steel-reinforced, anchored concrete baffles/retards placed every 50 feet shown on appropriate Plan and Profile sheets for the locations listed in the table above.
- ☒ N/A

Administrative Information

34. ☒ The final plans and technical specifications are submitted for TCEQ review. Each sheet of the construction plans and specifications are dated, signed, and sealed by the Texas Licensed Professional Engineer responsible for the design on each sheet.
35. ☒ Standard details are shown on the detail sheets, which are dated, signed, and sealed by the Texas Licensed Professional Engineer, as listed in the table below:

Table 9 - Standard Details

<i>Standard Details</i>	<i>Shown on Sheet</i>
Lateral stub-out marking [Required]	12 of 25
Manhole, showing inverts comply with 30 TAC §217.55(l)(2) [Required]	12 of 25
Alternate method of joining lateral to existing SCS line for potential future connections [Required]	20 of 25
Typical trench cross-sections [Required]	20 of 25
Bolted manholes [Required]	20 of 25
Sewer Service lateral standard details [Required]	20 of 25
Clean-out at end of line [Required, if used]	of
Baffles or concrete encasement for shock/erosion protection [Required, if flow velocity of any section of pipe >10 fps]	of
Detail showing Wastewater Line/Water Line Crossing [Required, if crossings are proposed]	12 of 25
Mandrel detail or specifications showing compliance with 30 TAC §217.57(b) and (c) [Required, if Flexible Pipe is used]	of

Standard Details	Shown on Sheet
Drop manholes [Required, if a pipe entering a manhole is more than 24 inches above manhole invert]	of

36. ☒ All organized sewage collection system general construction notes (TCEQ-0596) are included on the construction plans for this sewage collection system.
37. ☒ All proposed sewer lines will be sufficiently surveyed/staked to allow an assessment prior to TCEQ executive director approval. If the alignments of the proposed sewer lines are not walkable on that date, the application will be deemed incomplete and returned.
- ☒ Survey staking was completed on this date: _____
38. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
39. ☒ Any modification of this SCS application will require TCEQ approval, prior to construction, and may require submission of a revised application, with appropriate fees.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Organized Sewage Collection System Application** is hereby submitted for TCEQ review and executive director approval. The system was designed in accordance with the requirements of 30 TAC §213.5(c) and 30 TAC §217 and prepared by:

Print Name of Licensed Professional Engineer: Marisa Keiser

Date: August 4, 2025

Place engineer's seal here:



Signature of Licensed Professional Engineer:

Marisa Keiser
August 4, 2025

Marisa Keiser

Appendix A-Flow Velocity Table

Flow Velocity (Flowing Full) All gravity sewer lines on the Edwards Aquifer Recharge Zone shall be designed and constructed with hydraulic slopes sufficient to give a velocity when flowing full of not less than 2.0 feet per second, and not greater than 10 feet per second. The grades shown in the following table are based on Manning's formula and an n factor of 0.013 and shall be the minimum and maximum acceptable slopes unless provisions are made otherwise.

Table 10 - Slope Velocity

<i>Pipe Diameter(Inches)</i>	<i>% Slope required for minimum flow velocity of 2.0 fps</i>	<i>% Slope which produces flow velocity of 10.0 fps</i>
6	0.50	12.35
8	0.33	8.40
10	0.25	6.23
12	0.20	4.88
15	0.15	3.62
18	0.11	2.83
21	0.09	2.30
24	0.08	1.93
27	0.06	1.65
30	0.055	1.43
33	0.05	1.26
36	0.045	1.12
39	0.04	1.01
>39	*	*

**For lines larger than 39 inches in diameter, the slope may be determined by Manning's formula (as shown below) to maintain a minimum velocity greater than 2.0 feet per second when flowing full and a maximum velocity less than 10 feet per second when flowing full.*

$$v = \frac{1.49}{n} \times R_h^{0.67} \times \sqrt{S}$$

Figure 1 - Manning's Formula

Where:

v = velocity (ft/sec)

n = Manning's roughness coefficient (0.013)

R_h = hydraulic radius (ft)

S = slope (ft/ft)

Attachment A – SCS Engineering Design Report

TCEQ Engineering Design Report

For

O'Reilly Auto Parts

JULY 2025

Prepared By:
Bowman Consulting Group
807 Las Cimas Pkwy
Suite 350
Austin, TX 78746

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PVC PIPE STANDARDS

The American Society for Testing and Materials (ASTM) also known as ASTM International (Reference: www.astm.org) governs the manufacturing specifications for Polyvinyl Chloride (PVC) pipes, including the dimension ratio and water pressure allowable for use of each pipe, through its D-3034 standard. ASTM D-3034 lists its pipe dimensions and pipe classes using the “SDR” mark up, such as SDR-13.5, SDR-21, SDR-26 and SDR-41. The SDR refers to the standard dimension ratio (SDR) of the outside pipe diameter and the wall thickness. This project specifies the use of SDR-26 PVC pipe, which are to meet the ASTM pressure rating of greater than 115 psi and fall in the size category listed below. ASTM D-3034 standards must be meticulously adhered to by all PVC pipe manufacturers and is recognized as the standard during PVC pressure pipe testing and quality checks. Other in-depth information can be found published in Thermoplastic Pressure Pipe Design and Selection UNI-TR-7, by the Uni-Bell PVC Pipe Association.

SDR 26 Pipe Size Matrix (Per ASTM D-3034)			
Size (in)	O.D. (in)	Calc I.D. (in)	Thickness (in)
4	4.215	3.891	0.162
6	6.275	5.793	0.241
8	8.400	7.754	0.323
10	10.500	9.692	0.404
12	12.500	11.538	0.481
15	15.300	14.124	0.588

PROPOSED TYPE OF PIPE

Type I, Grade I, Polyvinyl Chloride (PVC) Specifications:

Size of Pipe: 4.00 in.

SDR 26 Properties

Pipe Compliance:	ASTM D-3034
Joint Compliance:	ASTM D-3212
Cell Classification:	12454
Minimum Tensile Strength (psi):	7,000
Minimum Modulus of Elasticity (psi):	400,000
Calculated Inner Diameter (in) = (Outer Diameter - 2t)	3.891
Outer Diameter (inch):	4.215
Wall Thickness (inch):	0.162
Mean Pipe Diameter (in) = (Outer Diameter - Thickness)	4.053
Approximate Trenching Width (feet):	1.35

Minimum Pipe Depth (Cover) used (feet): **3.00**

Maximum Pipe Depth (Cover) used (feet): **6.70**

Proposed Waste Water Usage: **125.00 GPD**

Q_{max} (As determined in Attachment A) = 0.0002 CFS

$$Q_{full} = \frac{1.486}{n} \times A \times R^{\frac{2}{3}} \times \sqrt{S}$$

A = Cross-Sectional Area, (ft²) = 0.083

S = Slope, decimal, minimum used = 0.030

R_h = hydraulic radius = 0.081

For the Specified Pipe at the Minimum Design Slope, the full flow is

$$Q_{full} = 0.306 \text{ CFS}$$

$$0.0002 < 0.306$$

Design meets TCEQ Guidelines

MINIMUM AND MAXIMUM GRADES FOR PIPES (30 TAC §217.53(1)(2)(A))

Minimum and Maximum Pipe Slopes		
Size of Pipe	Minimum Slope (%)	Maximum Slope (%)
6	0.5	12.35
8	0.33	8.4
10	0.25	6.23
12	0.2	4.88
15	0.15	3.62
18	0.11	2.83
21	0.09	2.3
24	0.08	1.93
27	0.06	1.65
30	0.055	1.43
33	0.05	1.26
36	0.045	1.12
39	0.04	1.01
>39	*	*
* For pipes larger than 39 inches in diameter, the slope is determined by Manning's formula to maintain a velocity greater than 2.0 feet per second and less than 10.0 feet per second when flowing full.		

MINIMUM AND MAXIMUM VELOCITY FOR THE PROPOSED SYSTEM:

So, using 4.00 inch PVC Pipe:

$$V = \frac{1.49}{n} \times R_h^{0.67} \times \sqrt{S}$$

V = velocity (ft/sec)	=	(solve)
n = Manning's coefficient	=	0.013
Calc. Inner Diameter (in)	=	3.891
A = Cross-Sectional Area, ft ²	=	0.083
Wp = Wetted Perimeter, ft	=	1.019
R _h = hydraulic radius, A/Wp	=	0.081
S = slope (ft/ft)	=	0.030

Minimum Slope Used (%): **3.00**

Maximum Slope Used (%): **3.09**

V_{min} = **3.72** ft/sec

V_{max} = **3.77** ft/sec

3.72 > **2.00** ft/sec

3.77 < **10.00** ft/sec

Design meets TCEQ Guidelines

Design meets TCEQ Guidelines

AVERAGE VALUES OF MODULUS OF SOIL REACTION, E'

Soil type-pipe bedding material (Unified Classification System)	Dumped	E' for Degree of Compaction of Bedding, in pounds per square inch		
		Slight <85% Proctor, <40% relative density	Moderate 85%-95% Proctor, 40%-70% relative density	High, > 95% Proctor, > 70% relative density
(1)	(2)	(3)	(4)	(5)
Fine-grained Soils (LL>50%) Soils with medium to high plasticity CH, MH, CH-MH	No data available; consult a competent soils engineer; Otherwise use E=0			
Fine-grained Soils (LL<50) Soils with medium to no plasticity, CL, ML, ML-CL, with less than 25% coarse-grained particles	50	200	400	1000
Fine-grained Soils (LL<50) Soils with medium to no plasticity, CL, ML, ML-CL, with more than 25% coarse-grained particles	100	400	1000	2000
Coarse-grained Soils with Fines GM, GC, SM, SC ^c contains more than 12% fines				
Coarse-grained Soils with Little or no Fines GW, GP, SW, SP ^c contains less than 12% fines	200	1000	2000	3000
Crushed Rock	1000	3000	3000	3000
Accuracy in Terms of Percentage Deflection	± 2	± 2	± 1	± 0.5

Taken from: Howard, Amster K. "Soil Reaction for Buried Flexible Pipe"
U.S. Bureau of Reclamation, Denver, CO and the American Society of Civil Engineers.

Modulus of Soil Reaction for the in-situ soil is determined to be = 1000 psi

PIPE BEDDING CLASS

Taken from the American Society for Testing and Material (ASTM) D 2321 and American Association of State Highway and Transportation Officials (AASHTO) M43, and as published on Table 7, in Deflection: The Pipe/Soil Mechanism UNI-TR-1-97, Uni-Bell PVC Pipe Association, Pg 24.

Pipe Embedment Material						E', psi (kPa) for Degree of Embedment Compaction				
ASTM D 2321*		ASTM D 2487		AASHTO M43	Min. Std. Proctor Density (%)	Lift Placement Depth	Dumped	Slightly < 85%	Moderate 85% - 95%	High > 95%
Class	Description	Notation	Description	Notation						
IA	Open-graded, clean manufactured aggregates	N/A	Angular crushed stone or rock, crushed gravel, crushed slag; large voids with little or no fines	5 56	Dumped	18" (0.45 m)	1000 (6,900)	3000 (20,700)	3000 (20,700)	3000 (20,700)
IB	Dense-graded, clean manufactured, processed aggregates	N/A	Angular crushed stone or other Class IA material and stone/sand mixtures; little or no fines							
II	Clean, coarse-grained soils	GW	Well-graded gravel, gravel/sand mixtures; little or no fines	57 6 67	85%	12" (0.30 m)	N/R	1000 (6,900)	2000 (13,800)	3000 (20,700)
		GP	Poorly graded gravel, gravel/sand mixtures; little or no fines							
		SW	Well-graded sands, gravelly sands; little or no fines							
		SP	Poorly graded sands, gravelly sands; little or no fines							
III	Coarse-grained soils with fines	GM	Silty gravels, gravel/sand/silt mixtures	Gravel and sand with <10% fines	90%	9" (0.20 m)	N/R	N/R	1000 (6,900)	2000 (13,800)
		GC	Clayey gravels, gravel/sand/clay mixtures							
		SM	Silty sands, sand/silt mixtures							
		SC	Clayey sands, sand/clay mixtures							

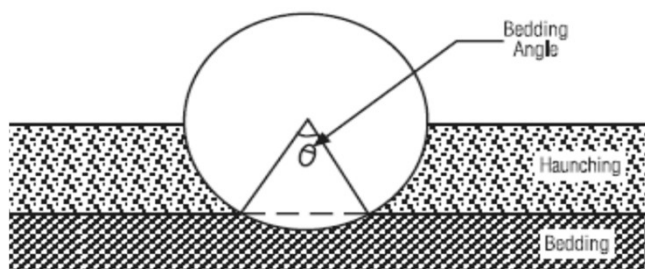
NOTE:

Per TCEQ guidelines, a contractor is allowed to use ASTM D 2321 Bedding Class 1A, 1B, II, or III at no less than 85% percent compaction. To grant the contractor its ability to make the proper judgment of which bedding class to use, the calculations provided in this Engineering Design Report reflect the use of **Bedding Class III, at 85%-95% compaction**, with an E' value of 1000 psi. This provides the "worst case" scenario for the SCS line. All other Bedding Class options will provide an improved value for the zeta factor as well as pipe deflection.

For Bedding Class III, 85%-95% Compaction, $E_b = 1000$ psi

PIPE BEDDING ANGLE

As Published on Figure 8 and Table 5, in Deflection: The Pipe/Soil Mechanism UNI-TR-1-97, Uni-Bell PVC Pipe Association, Pgs 18-19.



Bedding Constant Values

Bedding Angle, degrees	Bedding Constant
0	0.110
30	0.108
45	0.105
60	0.102
90	0.096
120	0.090
180	0.083

LIVE LOAD DETERMINATION

Source: AASHTO H20 and E80 Loads and as Published on Table 4, in Deflection: The Pipe/Soil Mechanism UNI-TR-1-97, Uni-Bell PVC Pipe Association, Pg 14.

Height of Cover (ft)	Live Load Transferred to Pipe, lb/in ²			Height of Cover (ft)	Live Load Transferred to Pipe, lb/in ²		
	Highway H20 ¹	Railway E80 ²	Airport ³		Highway H20 ¹	Railway E80 ²	Airport ³
1	12.50			14	*	4.17	3.06
2	5.56	26.39	13.14	16	*	3.47	2.29
3	4.17	23.61	12.28	18	*	2.78	1.91
4	2.78	18.40	11.27	20	*	2.08	1.53
5	1.74	16.67	10.09	22	*	1.91	1.14
6	1.39	15.63	8.79	24	*	1.74	1.05
7	1.22	12.15	7.85	26	*	1.39	*
8	0.69	11.11	6.93	28	*	1.04	*
10	*	7.64	6.09	30	*	0.69	*
12	*	5.56	4.76	35	*	*	*
				40	*	*	*

¹ Simulates 20 ton truck + impact

² Simulates 80,000 lb/ft railway load + impact

³ 180,000 lbs. dual tandem gear assembly. 26 inch spacing between tires and 66 inch center-to-center spacing between fore and aft tires under a rigid pavement 12 inches thick + impact.

* Negligible live load influence

PRISM LOAD DETERMINATION

Also referred to as the ‘dead’ load, the prism load is the pressure acting on the pipe by the weight of the soil column above a given section of the pipe. The following prism load columns are industry standards as referenced from Table 3, Deflection: The Pipe/Soil Mechanism UNI-TR-1-97, Uni-Bell PVC Pipe Association, Pg 13.

Table 3 Prism Load Soil Pressure (lbs/in ²)					
Height of Cover (ft)	Soil Unit Weight (lb/ft ³)				
	100	110	120	125	130
1	0.69	0.76	0.83	0.87	0.90
2	1.39	1.53	1.67	1.74	1.81
3	2.08	2.29	2.50	2.60	2.71
4	2.78	3.06	3.33	3.47	3.61
5	3.47	3.82	4.17	4.34	4.51
6	4.17	4.58	5.00	5.21	5.42
7	4.86	5.35	5.83	6.08	6.32
8	5.56	6.11	6.67	6.94	7.22
9	6.25	6.88	7.50	7.81	8.13
10	6.94	7.64	8.33	8.68	9.03
11	7.64	8.40	9.17	9.55	9.93
12	8.33	9.17	10.00	10.42	10.83
13	9.03	9.93	10.83	11.28	11.74
14	9.72	10.69	11.67	12.15	12.64
15	10.42	11.46	12.50	13.02	13.54
16	11.11	12.22	13.33	13.89	14.44
17	11.81	12.99	14.17	14.76	15.35
18	12.50	13.75	15.00	15.63	16.25
19	13.19	14.51	15.83	16.49	17.15
20	13.89	15.28	16.67	17.36	18.06
21	14.58	16.04	17.50	18.23	18.96
22	15.28	16.81	18.33	19.10	19.86
23	15.97	17.57	19.17	19.97	20.76
24	16.67	18.33	20.00	20.83	21.67
25	17.36	19.10	20.83	21.70	22.57
26	18.06	19.86	21.67	22.57	23.47
27	18.75	20.63	22.50	23.44	24.38
28	19.44	21.39	23.33	24.31	25.28
29	20.14	22.15	24.17	25.17	26.18
30	20.83	22.92	25.00	26.04	27.08
31	21.53	23.68	25.83	26.91	27.99
32	22.22	24.44	26.67	27.78	28.89
33	22.92	25.21	27.50	28.65	29.79
34	23.61	25.97	28.33	29.51	30.69
35	24.31	26.74	29.17	30.38	31.60
36	25.00	27.50	30.00	31.25	32.50
37	25.69	28.26	30.83	32.12	33.40
38	26.39	29.03	31.67	32.99	34.31
39	27.08	29.79	32.50	33.85	35.21
40	27.78	30.56	33.33	34.72	36.11
41	28.47	31.32	34.17	35.59	37.01
42	29.17	32.08	35.00	36.46	37.92
43	29.86	32.85	35.83	37.33	38.82
44	30.56	33.61	36.67	38.19	39.72
45	31.25	34.38	37.50	39.06	40.63
46	31.94	35.14	38.33	39.93	41.53
47	32.64	35.90	39.17	40.80	42.43
48	33.33	36.67	40.00	41.67	43.33
49	34.03	37.43	40.83	42.53	44.24
50	34.72	38.19	41.67	43.40	45.14

Note that the Prism Loads are calculated based upon the Marston Theory of Loads, developed by Professor Anson Marston, circa 1913, and is calculated using the formula:

$$P = \frac{\gamma_s * H}{144}$$

This formula determines the earth load on a flexible pipe and is regarded as a conservative approach to determining the dead load placed upon a buried flexible pipe.

At maximum burial depth of 6.70
feet, prism load = 5.00 psi

BUCKLING PRESSURE (ALLOWABLE)

Where:	q_a	=	Allowable buckling pressure (psi)	
	h	=	Height of soil above top of pipe (in) =	80.40 in
	H	=	Depth of burial, feet, from ground surface to top of pipe	
	B'	=	Empirical coefficient of elastic support	
	E_b	=	Modulus of soil reaction for the bedding material (psi)	
	E	=	Modulus of elasticity of the pipe material (psi)	
	I	=	Moment of inertia of the pipe, per linear inch of pipe (in ³)	
	t	=	Pipe wall thickness (in)	
	D	=	Mean Pipe Diameter (in)	$D = 4.053$ in

Solving for the Empirical coefficient of elastic support, given by Luscher in 1966, as referenced on Pg 113 of Moser, A.P., Buried Pipe Design. 2nd Ed., McGraw-Hill:

$$B' = \frac{4(h^2 + Dh)}{1.5(2h + D)^2}$$

$$I = \left(\frac{t^3}{12} \right) = \left(\frac{\text{inches}^3}{\text{in}_{\text{linear}}} \right) =$$

$$B' = \frac{27160.1}{40764.8} = 0.666 \quad I = \frac{0.0042515}{12} = 0.0004$$

Using the Allowable Buckling Pressure Equation as shown in Moser, A.P., Buried Pipe Design. 2nd Ed., McGraw-Hill, Pg 112, and an initial factor of safety (SF) of 2.5, the Allowable Buckling Pressure is then:

$$q_a = \frac{1}{FS} * \sqrt{32 * R_w * B' * E_b * \left(E * \frac{I}{D^3} \right)} \quad \text{Where,}$$

$$R_w = 1 - 0.33 (h_w / h)$$

$$q_a = \frac{1}{2.5} \sqrt{\left[32 \right] \left[1 \right] \left[0.666 \right] \left[1000 \right] \left[400000 \frac{0.0004}{66.58} \right]}$$

$$q_a = 85.21 \quad \text{psi}$$

BUCKLING PRESSURE (INSTALLED CONDITION)

Where:	q_p	=	Pressure applied to pipe under installed conditions (psi)	
	γ_w	=	Specific Weight of Water = 0.0361 (pci)	
	γ_s	=	Specific Weight of Soil (pcf)	
	W_c	=	Vertical Soil Load on the pipe per unit length (lb/in)	
	L_L	=	Live load as determined from chart	
	h_w	=	Height of Groundwater above pipe, typically = 0	
	D	=	Mean Pipe Diameter (in)	$D = 4.053 \text{ in}$
	t	=	Pipe Wall Thickness (in)	$t = 0.162 \text{ in}$

The Vertical Soil Load can be calculated using Equation 6.6 of Uni-Bell's Handbook of PVC Pipe , Ch VI Superimposed Loads on Buried Pipe, Pg 183

$$W_c = H \times \gamma_s \times (D + t)$$

Where: $\gamma_s = 120$ Value taken from: Assumed Worst Case Scenario

$$W_c = \left[6.70 \right] \left[12 \text{ in/ft} \right] \left[120.00 \right] \left[\frac{1 \text{ ft}^3}{1728 \text{ in}^3} \right] \left[4.22 \right]$$

$$W_c = 23.53 \text{ lb/in}$$

At Max Pipe Depth (H) of 6.70 ft

Using the Equation on Pg 114 of Moser, A.P., Buried Pipe Design. 2nd Ed., McGraw-Hill, Pressure Applied to Pipe under installed conditions at its deepest installed depth (Note, since $h_w = 0$, the Water Buoyancy Factor (R_w) = 1) is calculated to be:

$$q_p = \gamma_w h_w + R_w \left(\frac{W_c + L_L}{D} \right) \text{ and } L_L = 0 \quad R_w = 1 - 0.33 (h_w / h)$$

$$q_p = 0.0361 \times 0 + 1 \times \left[\frac{23.53}{4.053} \right]$$

$$q_p = 5.81 \text{ psi}$$

Note: The pressure applied to the pipe under installed conditions is less than the Allowable Buckling Pressure of the specified pipe, (i.e.. $q_a > q_p$) therefore the design is acceptable for installation.

WALL CRUSHING CALCULATION

Where:	D _o	=	outside pipe diameter, in.	=	4.215 in
	P _c	=	Compressive stress or hydrostatic design basis (HDB). For typical PVC pipe assume 4,000 psi. For any other pipe material the HDB must be supplied by the pipe manufacturer.		
	A	=	surface area of the pipe wall, in. ² /ft	=	0.162 in. ² /ft
	γ _s	=	specific weight of soil, pcf,	=	120 pcf
	H	=	Depth of burial (ft) from ground surface to crown of pipe		

Using the Wall Crushing and Wall Thrust equations, as referenced in Plastic Pipe Design Manual published by Vylon Pipe, Pg 14 the Wall Crushing due to compressive stress can be found using the following:

$$P_c = \frac{T}{A} \text{ where } T, \text{ Thrust, is calculated as } T = \frac{P_y D}{2}$$

Substituting the Thrust equation into the Wall Crushing equation:

$$P_c = \frac{\frac{P_y D}{2}}{A} = \frac{P_y D}{2A}$$

From the Marston Equation determining the Prism Load Calculation (See previous section on Prism Load), substitute the equation for P_y:

$$P_c = \frac{\frac{\gamma_s * H}{144} D}{2A} \text{ Rearranging this equation, it becomes: } 2AP_c = \frac{\gamma_s * H}{144} D$$

And simplifies to:

$$288AP_c = \gamma_s HD$$

Note that the Surface Area of the Pipe Wall, A, is per unit length in inches² per foot, a conversion factor (from feet to inches) of 12 must be applied, therefore,

$$24AP_c = \gamma_s HL$$

Solving for H, the equation becomes:

$$H = \frac{24 * P_c * A}{\gamma_s * D_o}$$

(Continued on next page)

Using this equation, and converting all units, solve for “height” of the soil column, or in other words, the depth of burial of the PVC pipe:

$$H = \frac{[24][4000][0.162 \times 12]}{120 \times 4.215} = 368.97$$

$$H = 368.97 \text{ feet}$$

Note: The resulting Wall Crushing will occur at a greater depth than the deepest burial depth of the proposed SCS lines, therefore pipe design is acceptable.

DEFLECTION ANALYSIS: LEONHARDT'S ZETA FACTOR

The Leonhardt's Zeta Factor Equation can be calculated using Equation 7.32 of Uni-Bell's Handbook of PVC Pipe , Ch VII Design of Buried PVC Pipe, Pg 268

Where:	Do	=	Pipe Outer Diameter, in	=	4.215
	B	=	Trench Width, in,	=	16.22 in
	E _b	=	Modulus of soil reaction, bedding material (psi)	=	1000
	E _n	=	Modulus of soil reaction for the in-situ soil (psi)	=	1000

$$\text{where,} \quad zeta = \frac{1.44}{f + [1.44 - f] \times \left[\frac{E_b}{E'_n} \right]}$$

$$f = \frac{\frac{B}{Do} - 1}{1.154 + 0.444 \left[\frac{B}{Do} - 1 \right]}$$

$$f = \frac{2.846975}{2.416057} = 1.178356$$

Substituting f into the zeta equation:

$$zeta = \frac{1.44}{[1.178] [0.262] [1.000]}$$

The Leonhardt Zeta factor is then determined as: **1.000**

PIPE STIFFNESS (Figure: 30 TAC §217.53(k)(3))

Using Equation B.1, as directed in 30 TAC §217.53(k)(3), to Calculate the Pipe Stiffness:

$$PS = C \times RSC \times \left(\frac{8.337}{D} \right)$$

Where: PS = Pipe Stiffness, for SDR-26 PVC (psi) = 115 p 115 psi
C = Conversion factor = 0.8
RSC = Ring Stiffness Constant
D = Mean Pipe Diameter (in), D = 4.053 in

The RSC can be supplied by the manufacturer or calculated by rearranging Equation B.1

$$RSC = \frac{PS}{C \times \left(\frac{8.337}{D} \right)}$$

$$RSC = \frac{115}{1.645596}$$

$$RSC = 69.884$$

PREDICTED PIPE DEFLECTION

Using the Modified Iowa Equation, referenced and published by the Uni-Bell PVC Pipe association and found at <http://www.uni-bell.org/faq.html>, and Equation 14 of Deflection: The Pipe/Soil Mechanism UNI-TR-1-97, Uni-Bell PVC Pipe Association Pgs 17, the predicted pipe deflection can be calculated.

Where:	% $\Delta Y/D$	=	Predicted % vertical deflection under load
	P	=	Prism Load, psi
	K	=	Bedding angle constant, Assumed to = 0.096
	W'	=	Live Load, psi, = 0 At max depth (ft) : 6.70
	DR	=	Dimension Ratio = 26
	E	=	Modulus of tensile elasticity of the pipe material, psi
	E'	=	Modulus of Soil Reaction (zeta x Eb) = 1000.00
	D _L	=	Deflection Lag Factor = 1.5

And using the Modified Iowa Equation:

$$(\%) \frac{\Delta Y}{D} = \frac{(D_L KP + KW') \times 100}{[2E / (3(DR - 1)^3)] + 0.061E'}$$

$$\text{Where, Prism Load, } P = \frac{\gamma_s * H}{144}$$

$$\text{and/or from previous chart, prism load} = 5.00 \text{ psi}$$

The Predicted Deflection is determined as:

$$(\%) \frac{\Delta Y}{D} = \frac{\left[\left[\frac{1.5 \times 0.480}{46875} \right] + 0 \right] \times 100}{\left[\frac{800000}{46875} \right] + [0.061 \times 1000.00]} = 0.92 \%$$

NOTE: 0.92 < 5%, therefore pipe design is acceptable

A deflection lag factor of 1.0 is typical for new pipes. Over the life of the pipe, the pipe will tend to deflect. Therefore, 1.5 is a conservative factor for the 50 year life.

PIPE STRAIN

Pipe strain is also known as the elongation of the pipe over the original length of the pipe. Under normal loading conditions of the PVC pipe, the variable that affects the elongation or straining of the pipe stems from either the flexure or deflection (i.e., bending) of the pipe within the bedding material (i.e. increased or excessive pipe deflection causing the pipe to elongate) or hoop stress within the pipe wall. Please note that pipe strain is not generally known to be the limiting performance factor during pipe failure. For this system, pipe deflection is limited to 5% for a SDR 26 pipe. This 5% deflection value is the industry accepted value placing the pipe within its straining limits. Therefore, as the calculated deflection above is shown to be less than 5%, the pipe and bedding class used in this system is within the acceptable straining limits for this pipe.

However, total Pipe strain is calculated as the combination of the before mentioned hoop stress and the maximum strain due to deflection. Both items are calculated below using Equations 15 and 16 found in Deflection: the Pipe/Soil Mechanism, UNI-TR-1-97, Published by the Uni-Bell PVC Pipe Association (Pgs 28-30):

Where:	ϵ_h	=	Maximum Pipe Strain due to Hoop Stress, in/in
	P	=	Pressure on the pipe (Live + Prism Loads), psi
	E	=	Modulus of Elasticity of the Pipe, psi
	t	=	Pipe Wall thickness (in) = 0.162
	D	=	Pipe Diameter, Outer (in) = 4.215

$$\epsilon_h = \frac{PD}{2tE}$$

Using the maximum cover for both live loads and prism loads as well as the previous unit weight of the soil:

$$\frac{[0.00 + 5.00] \times 4.215}{2 \times 0.162 \times 400,000} = 1.626E-04 \quad \frac{\text{in}}{\text{in}}$$

(Continued on following page)

Where:	ϵ_f	=	Maximum Pipe Strain due to Ring Deflection, in/in
	ΔY	=	Change in vertical pipe diameter under load, in, (numerator in the deflection equation, but in decimal form)
	t	=	Pipe Wall thickness (in) = 0.162
	D	=	Pipe Diameter, Outer (in) = 4.215
	DR	=	Dimension Ratio, PVC Pipe = 26

$$\epsilon_f = \frac{t}{D} \left[\frac{3\Delta Y / D}{1 - 2\Delta Y / D} \right] = \frac{1}{DR} \left[\frac{3\Delta Y}{D - 2\Delta Y} \right]$$

$$\frac{0.162}{4.215} \times \frac{2.160}{4.215 - 1.440} = 0.0299163 \frac{\text{in}}{\text{in}}$$

$$\epsilon_{total} = 0.0301 \frac{\text{in}}{\text{in}}$$

TCEQ PIPE BEDDING AND TRENCHING REQUIREMENTS (30 TAC 217.54)

0 **These notes are provided in the Construction Documents on Plan Sheet 3 of 25**

a. Pipe Embedment

1. A rigid pipe must be laid with the adequate bedding, haunching, and initial backfill to support the anticipated load. The bedding classes that are allowed are A, B, or C, as described in American Society for Testing and Materials (ASTM) C 12, American National Standards Institute (ANSI) A 106.2, Water Environment Federation Manual of Practice No. 9 or American Society of Civil Engineers (ASCE) MOP 37.
2. A flexible pipe must be laid with the adequate bedding, haunching, and initial backfill to support the anticipated load. The bedding classes that are allowed are IA, IB, II, or III, as described in ASTM D-2321 or ANSI K65.171.
3. Debris, large clods, or stones that are greater than six inches in diameter, organic matter, or other unstable materials are prohibited as bedding, haunching, or initial backfill.
4. Backfill must not disturb the alignment of a collection system pipe.
5. If trenching encounters significant fracture, fault zones, caves, or solutional modification to the rock strata, an owner must halt construction until an engineer prepares a written report detailing how construction will accommodate these site conditions.

b. Compaction.

1. Compaction of an embedment envelope must meet the manufacturer's recommendations for the collection system pipe used in a project.
2. Compaction of an embedment envelope must provide the modulus of soil reaction for the bedding material necessary to ensure a wastewater collection system pipe's structural integrity as required by §217.53 of this title (relating to Pipe Design).
3. The placement of the backfill above a pipe must not affect the structural integrity of a pipe.

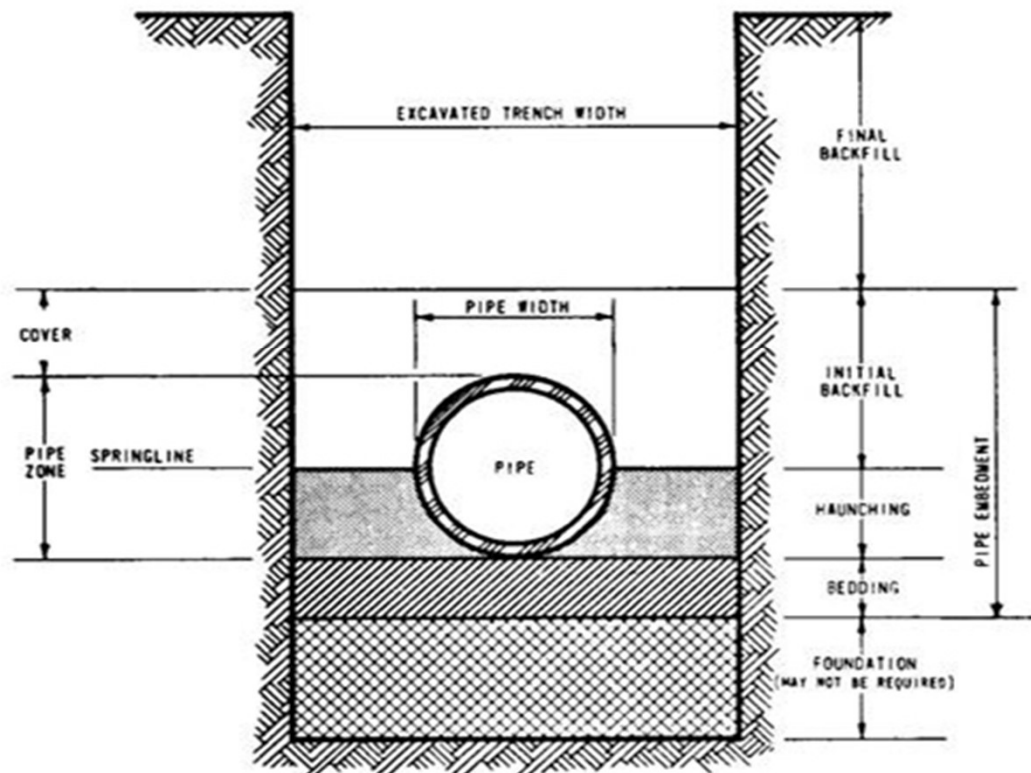
c. Envelope Size.

1. A minimum clearance of 6.0 inches below and on each side of the bell of all pipes to the trench walls and floor is required.
2. The embedment material used for haunching and initial backfill must be installed to a minimum depth of 12 inches above the crown of a pipe.

d. Trench Width.

1. The width of a trench must allow a pipe to be laid and jointed properly and must allow the backfill to be placed and compacted as needed.
2. The maximum and minimum trench width needed for safety and a pipe's structural integrity must be included in the report.
3. The width of a trench must be sufficient to properly and safely place and compact haunching materials.
4. The space between a pipe and a trench wall must be wider than the compaction equipment used in the pipe zone.

TRENCH CROSS-SECTION (30 TAC 217.54)



NOTE:

Trenching Details along with 30 TAC 217.54 are annotated in the Construction Documents/Plan Sheets on Sheet 20 of 25

MANHOLE SPECIFICATIONS

30 TAC 217.55 Requirements with design comments:

- a. An owner must include manholes in a wastewater collection system at:
 - 1. All points of change in alignment, grade, or size;
 - 2. At the intersection of all pipes; and
 - 3. At the end of all pipes that may be extended at a future date. **All WW SCS Lines end with a Manhole**
- b. Manholes placed at the end of a wastewater collection system pipe that may be extended in the future must include pipe stub outs with plugs. **(Self explanatory, see item a above)**
- c. A clean-out with watertight plugs may be installed in lieu of a manhole at the end of a wastewater collection system pipe if no extensions are anticipated. **(Self explanatory)**
- d. Cleanout installations must pass all applicable testing requirements outlined for gravity collection pipes in §217.57 of this title (relating to Testing Requirements for Installation of Gravity Collection System Pipes). **(Self explanatory, see Item d above)**
- e. A manhole must be made of monolithic, cast-in-place concrete, fiberglass, pre-cast concrete, high-density polyethylene, or equivalent material that provides adequate structural integrity. **See the Pre-Cast Manhole Details following these construction notes along with CoSM MH Details in Construction Documents**
- f. The use of bricks to adjust a manhole cover to grade or construct a manhole is prohibited. **(Self explanatory, See Details following these notes)**
- g. Manholes may be spaced no further apart than the distances specified in the following table for a wastewater collection system with straight alignment and uniform grades, unless a variance based on the availability of cleaning equipment that is capable of servicing greater distances is granted by the executive director. **(Manholes are spaced no greater than 500 L.F.)**

Table C.2. - Maximum Manhole Spacing	
Pipe Diameter	Maximum Manhole
6-15	500
18-30	800
36-48	1000
54 or larger	2000

- h. Tunnels are exempt from manhole spacing requirements because of construction constraints. **(Self explanatory and not applicable)**
- i. An intersection of three or more collection pipes must have a manhole. **(Self explanatory and maintained throughout the design of the SCS)**
- j. A manhole must not be located in the flow path of a watercourse, or in an area where ponding of surface water is probable. **(Self explanatory and maintained throughout the design of the SCS)**
- k. The inside diameter of a manhole must be no less than 48 inches. A manhole diameter must be sufficient to allow personnel and equipment to enter, exit, and work in the manhole and to allow proper joining of the collection system pipes in the manhole wall. **(See Manhole Details following these notes)**
- l. Manholes must meet the following requirements for covers, inlets, and bases.

- 1. Manhole Covers

- A.

- A manhole where personnel entry is anticipated requires at least a 30 inch diameter clear opening. (**Lid to have 30” min openings per CoG Specifications and Details within the plan sets.**

- B. A manhole located within a 100-year flood plain must have a means of preventing inflow. **(Self explanatory but not applicable for this project)**

- C. A manhole cover construction must be constructed of impervious material. **(Self explanatory, See Manhole Details)**

- D. A manhole cover that is located in a roadway must meet or exceed the American Association of State Highways and Transportation Officials standard M-306 for load bearing. **(Self explanatory, See Manhole Details)**

- 2. Manhole Inverts

- A. The bottom of a manhole must contain a U-shaped channel that is a smooth continuation of the inlet and outlet pipes. **(Self explanatory, see CoG Details within plan sets.)**

- B. A manhole connected to a pipe less than 15 inches in diameter must have a channel depth equal to at least half the largest pipe's diameter **(Self explanatory)**

- C. A manhole connected to a pipe at least 15 inches in diameter but not more than 24 inches in diameter must have a channel depth equal to at least three-fourths of the largest pipe's diameter **(Self explanatory, but not applicable for this project)**

- D. A manhole connected to a pipe greater than 24 inches in diameter must have a channel depth equal to at least the largest pipe's diameter **(Self explanatory, but not applicable for this project)**.
- E. A manhole with pipes of different sizes must have the tops of the pipes at the same elevation and flow channels in the invert sloped on an even slope from pipe to pipe. **(Self explanatory and maintained throughout the design of the SCS)**
- F. A bench provided above a channel must slope at a minimum of 0.5 inch per foot. **(Self Explanatory)**
- G. An invert must be filleted to prevent solids from being deposited if a wastewater collection system pipe enters a manhole higher than 24 inches above a manhole invert. **(Self Explanatory)**
- H. A wastewater collection system pipe entering a manhole more than 24 inches above an invert must have a drop pipe. **(Self Explanatory)**
- m. The inclusion of steps in a manhole is prohibited. **(Self Explanatory, steps are not included in CoG manhole Details)**
- n. Connections. A manhole-pipe connection must use watertight, size-on-size resilient connectors that allow for differential settlement and must conform to American Society for Testing and Materials C-923. **(Self Explanatory, see Details within plan sets.)**
- o. Venting. An owner must use an alternate means of venting if manholes are at more than 1,500 foot intervals and gasketed manhole covers are required for more than three manholes in sequence. Vents must meet the following requirements: **(Self Explanatory, MH's are vented every third MH)**
 - 1. Vent design must minimize inflow;
 - 2. Vents must be located above a 100-year flood event elevation; and
 - 3. Tunnels must be vented in compliance with this subsection.
- p. Cleanouts. The size of a cleanout must be equal to the size of the wastewater collection system main. **(Self Explanatory)**

Precast Manhole Information is provided on the detail sheets as per COG

5– TCEQ - 0584 Attachments

Water Pollution Abatement Plan Application Form

Water Pollution Abatement Plan Application

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Water Pollution Abatement Plan Application Form** is hereby submitted for TCEQ review and Executive Director approval. The form was prepared by:

Print Name of Customer/Agent: Marisa Keiser, PE

Date: August 4, 2025

Signature of Customer/Agent:



Regulated Entity Name: O'Reilly Auto Parts

Regulated Entity Information

1. The type of project is:

- ☐ Residential: Number of Lots: _____
- ☐ Residential: Number of Living Unit Equivalents: _____
- ☒ Commercial
- ☐ Industrial
- ☐ Other: _____

2. Total site acreage (size of property): 1.41

3. Estimated projected population: _____

4. The amount and type of impervious cover expected after construction are shown below:

Table 1 - Impervious Cover Table

Impervious Cover of Proposed Project	Sq. Ft.	Sq. Ft./Acre	Acres
Structures/Rooftops	7885	$\div 43,560 =$	0.181
Parking	6009	$\div 43,560 =$	0.137
Other paved surfaces	15,333	$\div 43,560 =$	0.352
Total Impervious Cover	29,227	$\div 43,560 =$	0.67

Total Impervious Cover 0.67 $\div$ Total Acreage 1.41 X 100 = 47.51% Impervious Cover

5. ☒ **Attachment A - Factors Affecting Surface Water Quality.** A detailed description of all factors that could affect surface water and groundwater quality that addresses ultimate land use is attached.
6. ☒ Only inert materials as defined by 30 TAC §330.2 will be used as fill material.

For Road Projects Only

Complete questions 7 - 12 if this application is exclusively for a road project.

7. Type of project:

- ☐ TXDOT road project.
- ☐ County road or roads built to county specifications.
- ☐ City thoroughfare or roads to be dedicated to a municipality.
- ☐ Street or road providing access to private driveways.

8. Type of pavement or road surface to be used:

- ☐ Concrete
- ☐ Asphaltic concrete pavement
- ☐ Other: _____

9. Length of Right of Way (R.O.W.): _____ feet.

Width of R.O.W.: _____ feet.

$L \times W =$ _____ $\text{Ft}^2 \div 43,560 \text{ Ft}^2/\text{Acre} =$ _____ acres.

10. Length of pavement area: _____ feet.

Width of pavement area: _____ feet.

$L \times W =$ _____ $\text{Ft}^2 \div 43,560 \text{ Ft}^2/\text{Acre} =$ _____ acres.

Pavement area _____ acres $\div$ R.O.W. area _____ acres $\times 100 =$ _____ % impervious cover.

11. ☐ A rest stop will be included in this project.

☐ A rest stop will not be included in this project.

12. ☐ Maintenance and repair of existing roadways that do not require approval from the TCEQ Executive Director. Modifications to existing roadways such as widening roads/adding shoulders totaling more than one-half (1/2) the width of one (1) existing lane require prior approval from the TCEQ.

Stormwater to be generated by the Proposed Project

13. ☒ **Attachment B - Volume and Character of Stormwater.** A detailed description of the volume (quantity) and character (quality) of the stormwater runoff which is expected to occur from the proposed project is attached. The estimates of stormwater runoff quality and quantity are based on the area and type of impervious cover. Include the runoff coefficient of the site for both pre-construction and post-construction conditions.

Wastewater to be generated by the Proposed Project

14. The character and volume of wastewater is shown below:

<u>100%</u> Domestic	<u>125</u> Gallons/day
<u> </u> % Industrial	<u> </u> Gallons/day
<u> </u> % Commingled	<u> </u> Gallons/day
TOTAL gallons/day <u>125</u>	

15. Wastewater will be disposed of by:

☐ On-Site Sewage Facility (OSSF/Septic Tank):

☐ **Attachment C - Suitability Letter from Authorized Agent.** An on-site sewage facility will be used to treat and dispose of the wastewater from this site. The appropriate licensing authority's (authorized agent) written approval is attached. It states that the land is suitable for the use of private sewage facilities and will meet or exceed the requirements for on-site sewage facilities as specified under 30 TAC Chapter 285 relating to On-site Sewage Facilities.

☐ Each lot in this project/development is at least one (1) acre (43,560 square feet) in size. The system will be designed by a licensed professional engineer or registered sanitarian and installed by a licensed installer in compliance with 30 TAC Chapter 285.

☒ Sewage Collection System (Sewer Lines):

☒ Private service laterals from the wastewater generating facilities will be connected to an existing SCS.

☐ Private service laterals from the wastewater generating facilities will be connected to a proposed SCS.

☐ The SCS was previously submitted on _____.

☐ The SCS was submitted with this application.

☐ The SCS will be submitted at a later date. The owner is aware that the SCS may not be installed prior to Executive Director approval.

☐ The sewage collection system will convey the wastewater to the _____ (name) Treatment Plant. The treatment facility is:

☐ Existing.

☐ Proposed.

16. ☒ All private service laterals will be inspected as required in 30 TAC §213.5.

Site Plan Requirements

Items 17 – 28 must be included on the Site Plan.

17. ☒ The Site Plan must have a minimum scale of 1" = 400'.

Site Plan Scale: 1" = 20'.

18. 100-year floodplain boundaries:

☐ Some part(s) of the project site is located within the 100-year floodplain. The floodplain is shown and labeled.

☒ No part of the project site is located within the 100-year floodplain.

The 100-year floodplain boundaries are based on the following specific (including date of material) sources(s): _____

19. ☒ The layout of the development is shown with existing and finished contours at appropriate, but not greater than ten-foot contour intervals. Lots, recreation centers, buildings, roads, open space, etc. are shown on the plan.

☐ The layout of the development is shown with existing contours at appropriate, but not greater than ten-foot intervals. Finished topographic contours will not differ from the existing topographic configuration and are not shown. Lots, recreation centers, buildings, roads, open space, etc. are shown on the site plan.

20. All known wells (oil, water, unplugged, capped and/or abandoned, test holes, etc.):

☐ There are _____ (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply)

☐ The wells are not in use and have been properly abandoned.

☐ The wells are not in use and will be properly abandoned.

☐ The wells are in use and comply with 16 TAC §76.

☒ There are no wells or test holes of any kind known to exist on the project site.

21. Geologic or manmade features which are on the site:

☐ All sensitive geologic or manmade features identified in the Geologic Assessment are shown and labeled.

☒ No sensitive geologic or manmade features were identified in the Geologic Assessment.

☐ **Attachment D - Exception to the Required Geologic Assessment.** A request and justification for an exception to a portion of the Geologic Assessment is attached.

- 22. ☒ The drainage patterns and approximate slopes anticipated after major grading activities.
- 23. ☒ Areas of soil disturbance and areas which will not be disturbed.
- 24. ☒ Locations of major structural and nonstructural controls. These are the temporary and permanent best management practices.
- 25. ☒ Locations where soil stabilization practices are expected to occur.
- 26. ☐ Surface waters (including wetlands).
☒ N/A
- 27. ☐ Locations where stormwater discharges to surface water or sensitive features are to occur.
☒ There will be no discharges to surface water or sensitive features.
- 28. ☒ Legal boundaries of the site are shown.

Administrative Information

- 29. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
- 30. ☒ Any modification of this WPAP will require Executive Director approval, prior to construction, and may require submission of a revised application, with appropriate fees.

TCEQ-0584

Attachment A – Factors Affecting Water Quality

Potential sources of pollution that may be expected to affect the quality of the stormwater discharges from the construction site include the following:

- Soil erosion due to the clearing of the site for pavement, buildings, utilities and pond structures.
- Oil, grease, fuel and hydraulic fluid contamination from construction equipment and vehicle drippings.
- Hydrocarbons from concrete paving operations.
- Miscellaneous trash and litter from construction.
- Concrete truck washout.

TCEQ-0584

Attachment B – Volume and Character of Storm Water

The site generally drains northeast conveying the flows to a drainage system discharging in a Detention Pond. It is expected that the character of surface water and ground water runoff would be consistent with a development used for commercial purposes. The point of discharges was analyzed. Please refer to the attached exhibits for drainage area maps. Please see the summary table below for peak volumes of storm water.

Existing Development Conditions - HEC-HMS						
BASIN	Q (CFS)					
	2-yr	10-yr	25-yr	100-yr		
EA1	0.30	0.50	0.60	0.70		
EB1	1.10	1.90	2.50	2.90		
EC1	4.00	6.50	8.20	9.60		
Proposed Development Conditions - HEC-HMS						
BASIN	Q (CFS)					
	2-yr	10-yr	25-yr	100-yr		
PA1	0.30	0.50	0.60	0.70		
PB1	1.00	1.70	2.20	2.60		
PC1	3.30	5.00	6.10	7.00		
PC2	1.90	2.70	3.20	3.60		
PC3	0.50	0.90	1.20	1.40		
Pond A	0.20	0.30	0.30	0.40		
Existing X Proposed Development Conditions - HEC-HMS						
BASIN		AREA (ac)	FLOW (CFS)			
			2 YR	10 YR	25 YR	100 YR
Existing POA A		0.15	0.30	0.50	0.60	0.70
Proposed POA A		0.15	0.30	0.50	0.60	0.70
Δ CFS			0.00	0.00	0.00	0.00
Existing POA B		0.58	1.10	1.90	2.50	2.90
Proposed POA B		0.51	1.00	1.70	2.20	2.60
Δ CFS			-0.10	-0.20	-0.30	-0.30
Existing POA C		2.13	4.00	6.50	8.20	9.60
Proposed POA C		2.18	4.00	6.20	7.50	8.70
Δ CFS			0.00	-0.30	-0.70	-0.90

Attachment C – Suitability Letter from Authorized Agent

N/A

Attachment D - Exception to the required Geological Assessment

N/A

6– TCEQ - 0602 Attachments

Temporary Stormwater Section

Temporary Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: Marisa Keiser, PE

Date: August 4, 2025

Signature of Customer/Agent:



Regulated Entity Name: O'Reilly Auto Parts

Project Information

Potential Sources of Contamination

Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.

1. Fuels for construction equipment and hazardous substances which will be used during construction:

☐ The following fuels and/or hazardous substances will be stored on the site: _____

These fuels and/or hazardous substances will be stored in:

- ☐ Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

- ☐ Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year.
- ☐ Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
- ☒ Fuels and hazardous substances will not be stored on the site.
- 2. ☒ **Attachment A - Spill Response Actions.** A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
- 3. ☒ Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
- 4. ☒ **Attachment B - Potential Sources of Contamination.** A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.

Sequence of Construction

- 5. ☒ **Attachment C - Sequence of Major Activities.** A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
 - ☐ For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given.
 - ☒ For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
- 6. ☐ Name the receiving water(s) at or near the site which will be disturbed or which will receive discharges from disturbed areas of the project: _____

Temporary Best Management Practices (TBMPs)

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

- 7. ☒ **Attachment D – Temporary Best Management Practices and Measures.** TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

- ☒ A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.
 - ☒ A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.
 - ☒ A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer.
 - ☒ A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.
8. ☒ The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
- ☐ **Attachment E - Request to Temporarily Seal a Feature.** A request to temporarily seal a feature is attached. The request includes justification as to why no reasonable and practicable alternative exists for each feature.
- ☒ There will be no temporary sealing of naturally-occurring sensitive features on the site.
9. ☒ **Attachment F - Structural Practices.** A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.
10. ☒ **Attachment G - Drainage Area Map.** A drainage area map supporting the following requirements is attached:
- ☐ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided.
 - ☐ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used.
 - ☐ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area.
 - ☒ There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.

☐ There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. Erosion and sediment controls other than sediment basins or sediment traps within each disturbed drainage area will be used.

11. ☐ **Attachment H - Temporary Sediment Pond(s) Plans and Calculations.** Temporary sediment pond or basin construction plans and design calculations for a proposed temporary BMP or measure have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information must be signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed temporary BMPs and measures are attached.

☒ N/A

12. ☒ **Attachment I - Inspection and Maintenance for BMPs.** A plan for the inspection of each temporary BMP(s) and measure(s) and for their timely maintenance, repairs, and, if necessary, retrofit is attached. A description of the documentation procedures, recordkeeping practices, and inspection frequency are included in the plan and are specific to the site and/or BMP.
13. ☒ All control measures must be properly selected, installed, and maintained in accordance with the manufacturer's specifications and good engineering practices. If periodic inspections by the applicant or the executive director, or other information indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations.
14. ☒ If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain).
15. ☒ Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50%. A permanent stake will be provided that can indicate when the sediment occupies 50% of the basin volume.
16. ☒ Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source for stormwater discharges (e.g., screening outfalls, picked up daily).

Soil Stabilization Practices

Examples: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

17. ☒ **Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices.** A schedule of the interim and permanent soil stabilization practices for the site is attached.

18. ☒ Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
19. ☒ Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

Administrative Information

20. ☒ All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
21. ☒ If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
22. ☒ Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

Attachment A – Spill Response Actions

The objective of this section is to describe measures to prevent or reduce the discharge of pollutants to drainage systems or watercourses. Measures include reducing the chance for spills, stopping the source of spills, containing and cleaning up spills, properly disposing of spill materials, and training employees.

The following steps will help reduce the stormwater impacts of leaks and spills:

Education

- (1) Be aware that different materials pollute in different amounts. Make sure that each employee knows what a “significant spill” is for each material they use, and what is the appropriate response for “significant” and “insignificant” spills. Employees should also be aware of when spill must be reported to the TCEQ. Information is available in 30 TAC 327.4 and 40 CFR 302.4.
- (2) Educate employees and subcontractors on potential dangers to humans and the environment from spills and leaks.
- (3) Hold regular meetings to discuss and reinforce appropriate disposal procedures (incorporate into regular safety meetings).
- (4) Establish a continuing education program to indoctrinate new employees.
- (5) Have a contractor’s superintendent or representative oversee and enforce proper spill prevention and control measures.

General Measures

- (1) To the extent that the work can be accomplished safely, spills of oil, petroleum, products, substances listed under 40 CFR parts 110, 117, and 302, and sanitary and septic wastes should be contained and cleaned up immediately.
- (2) Store hazardous materials and wastes in covered containers and protect from vandalism.
- (3) Place a stockpile of spill cleanup materials where it will be readily accessible.
- (4) Train employees in spill prevention and cleanup.
- (5) Designate responsible individuals to oversee and enforce control measures.
- (6) Spills should be covered and protected from stormwater runoff during rainfall to the extent that it doesn’t compromise cleanup activities.
- (7) Do not bury or wash spills with water.
- (8) Store and dispose of used clean up materials, contaminated materials, and recovered spill material that is no longer suitable for the intended purpose in conformance with the provisions in applicable BMP’s.
- (9) Do not allow water used for cleaning and decontamination to enter storm drains or watercourses. Collect and dispose of contaminated water in accordance with applicable regulations.
- (10) Contain water overflow or minor water spillage and do not allow it to discharge into drainage facilities or watercourses.

- (11) Place Material Safety Data Sheets (MSDS), as well as proper storage, cleanup, and spill reporting instructions for hazardous materials stored or used on the project site in an open, conspicuous, and accessible location.
- (12) Keep waste storage areas clean, well-organized, and equipped with ample cleanup supplies as appropriate for the materials being stored. Perimeter controls, containment structures, covers, and liners should be repaired or replaced as needed to maintain proper function.

Cleanup

- (1) Clean up leaks and spills immediately
- (2) Use a rag for small spills on paved surfaces, a damp mop for general cleanup, and absorbent material for larger spills. If the spilled material is hazardous, then the used cleanup materials are also hazardous and must be disposed of as hazardous waste.
- (3) Never hose down or bury dry material spills. Clean up as much of the material as possible and dispose of properly. See the waste management BMPs in this section for specific information.

Minor Spills

- (1) Minor spills typically involve small quantities of oil, gasoline, paint, etc. which can be controlled by the first responder at the discovery of the spill.
- (2) Use absorbent materials on small spills rather than hosing down or burying the spill.
- (3) Absorbent materials should be promptly removed and disposed of properly.
- (4) Follow the practice below for a minor spill:
- (5) Contain the spread of the spill
- (6) Recover spilled materials
- (7) Clean the contaminated area and properly dispose of contaminated materials.

Semi-Significant Spills

Semi-significant spills still can be controlled by the first responder along with the aid of other personnel such as laborers and the foreman, etc. This response may require the cessation of all other activities.

Spills should be cleaned up immediately using the following steps:

- (1) Contain spread of the spill
- (2) Notify the project foreman immediately
- (3) If the spill occurs on paved or impermeable surfaces, clean up using “dry” methods (absorbent materials, cat litter and/or rags). Contain the spill by encircling with absorbent materials and do not let the spill spread widely.
- (4) If the spill occurs in dirt areas, immediately contain the spill by constructing an earthen dike. Dig up and properly dispose of contaminated soil.
- (5) If the spill occurs during rain, cover spill with tarps or other material to prevent contaminating runoff.

Significant/Hazardous Spills

- (1) Notify the TCEQ by telephone as soon as possible and within 24 hours at 512-339-2929 (Austin) or 210-490-3096 (San Antonio) between 8 AM and 5 PM. After hours, contact the Environmental Release Hotline at 1-800-832-8224. It is the contractor's responsibility to have all emergency phone numbers at the construction site.
- (2) For spills of federal reportable quantities, in conformance with the requirements in 40 CFR parts 110, 119, and 302, the contractor should notify the National Response Center at (800) 424-8802.
- (3) Notification should first be made by telephone and followed up with a written report.
- (4) The services of a spills contractor or a Haz-Mat team should be obtained immediately. Construction personnel should not attempt to clean up until the appropriate and qualified staffs have arrived at the job site.
- (5) Other agencies which may need to be consulted include, but are not limited to, the City Police Department, County Sheriff Office, Fire Departments, etc.

More information on spill rules and appropriate responses is available on the TCEQ website at : <http://www.tceq.texas.gov/response/>

Vehicle and Equipment Maintenance

- (1) If maintenance must occur onsite, use a designated area and a secondary containment, located away from drainage courses, to prevent the runoff of stormwater and the runoff of spills.
- (2) Regularly inspect onsite vehicles and equipment for leaks and repair immediately.
- (3) Check incoming vehicles and equipment (including delivery trucks, and employee and subcontractor vehicles) for leaking oil and fluids. Do not allow leaking vehicles or equipment onsite.
- (4) Always use secondary containment, such as drain pan or drop cloth, to catch spills or leaks when removing or changing fluids.
- (5) Place drip pans or absorbent materials under paving equipment when not in use.
- (6) Use absorbent materials on small spills rather than hosing down or burying the spill. Remove the absorbent materials promptly and dispose of properly.
- (7) Promptly transfer used fluids to the proper waste or recycling drums. Don't leave full drip pans or other open containers lying around.
- (8) Oil filters disposed of in trashcans or dumpsters can leak oil and pollute stormwater. Place the oil filter in a funnel over the waste oil-recycling drum to drain excess oil before disposal. Oil filters can also be recycled. Ask the oil supplier or recycler about recycling oil filters.
- (9) Store cracked batteries in a non-leaking secondary container. Do this with all cracked batteries even if you think all of the acid has drained out. If you drop a battery, treat it as if it is cracked. Put it into the containment area until you are sure it is not leaking.

Vehicle and Equipment Fueling

- (1) If fueling must occur on site, use designated areas, located away from drainage courses, to prevent the runoff of stormwater and the runoff of spills.
- (2) Discourage “topping off” of fuel tanks.
- (3) Always use secondary containment, such as a drain pan, when fueling to catch spills/ leaks.

Attachment B – Potential Sources of Contamination

Potential Source:	Oil, grease, fuel, and hydraulic fluid contamination from construction equipment and vehicle drippings
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Preventative Measure:	Vehicle maintenance, when possible, will be performed within the construction staging areas.
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Potential Source:	Miscellaneous trash and litter from construction
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Preventative Measure:	Trash containers will be placed throughout the site to encourage proper trash disposal.
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Potential Source:	Construction debris
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Preventative Measure:	Construction debris will be monitored daily by contractor. Debris will be collected weekly and placed in disposal bins. Situations requiring immediate attention will be addressed on a case by case basis.
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Attachment C – Sequence of Major Activities

The sequence of major activities will be divided into two stages: Site Preparation and Site Construction. For all activities listed below, Erosion and Sediment control measures have been included in the construction plans to lessen the impact of disturbed soils during the major activities in construction. Please refer to these sheets in the Construction Drawings for more detailed information.

Site Preparation:

- Submit written notice of construction to TCEQ regional office at least 48 hours prior to the start of any regulated activities.
- Install Temporary BMPs
- Clearing and grubbing of vegetation
- Demolish of existing utilities, curb and gutter

Construction (Approximately 1.16 AC to be disturbed):

- Excavate/Sawcut for roadway improvements
- Utility trenching and installation, including water, wastewater, and stormsewers.
- Final grading
- Grading for pond
- Install Paving/Infrastructure
- Revegetating all disturbed areas

Attachment D – Temporary Best Management Practices and Measures

Temporary measures are intended to provide a method of slowing the flow or runoff from the construction site in order to allow sediment and suspended solids to settle out of the water. By containing the sediment and solids within the site, they will not enter surface streams and/or sensitive features. BMP measures utilized in this plan are intended to allow storm water to continue downstream after passing through for treatment. This will allow stormwater runoff to continue downstream to any existing sensitive features.

Site Preparation:

The clearing and grading of the land will disturb the largest area of soil, so erosion control measures will be installed as the first step in construction. The methodology for pollution prevention of all on-site stormwater will include a) installation of inlet protection, b) the erection of silt fences along the downgradient boundary of the construction activities, c) installation of rock berms with silt fence covering downgradient from areas of concentrated stormwater flow, d) installation of stabilized construction entrances to reduce the dispersion of sediment from the site, e) installation of a construction staging and spoil area and f) installation of concrete washout.

Construction:

All installed erosion control measures will be inspected, and if necessary, repaired before any additional construction begins, as well as periodically throughout the construction process. The contractor will be responsible for all maintenance of erosion control measures, as well as the installation of all remaining on-site control measures, including the concrete truck washout, as necessary.

Attachment E - Request to Temporarily Seal a Feature, if sealing a feature

N/A

Attachment F – Structural Practices

The following structural measures will be installed prior to the initiation of site construction:

- Silt fences along the downstream boundary of all construction activity, and rock berms with silt fence covering for secondary protection
- Installation of stabilized construction entrances and construction staging areas
- Installation of concrete truck washout pits, as required

Attachment G – Drainage Map

Attachment H - Temporary Sediment Pond(s) Plans and Calculations

N/A

Attachment I – Inspection and Maintenance for BMPs

Inspections

Designated and qualified person(s) shall inspect BMPs every seven days, and within 24 hours after a storm event greater than 0.5 inches of rainfall. An inspection report that summarizes the scope of the inspection, names and qualifications of personnel conducting the inspection, date of the inspection, major observations, and actions taken as a result of the inspection shall be recorded and maintained as part of the Storm Water TPDES data for a period of three years after the date of the inspection. A copy of the Inspection Report Form is provided in the Storm Water Pollution Prevention Plan.

As a minimum, the inspector shall observe: (1) significant disturbed areas for evidence of erosion, (2) inspection and maintenance of inlet protection, (3) storage areas for evidence of leakage from the exposed stored materials, (4) structural controls (rock berm outlets, silt fences, drainage swales, etc.) for evidence of failure or excess siltation (over 6 inches deep), (5) vehicle exit point for evidence of off-site sediment tracking, (6) vehicle storage areas for signs of leaking equipment or spills, and (7) concrete truck rinse-out pit for signs of potential failure. Deficiencies noted during the inspection will be corrected and documented within seven (8) calendar days following the inspection or before the next anticipated storm event if practicable.

SWPPP Inspection Report

Project Name: _____ **Date of Inspection:** _____

Inspection Frequency: (Every 7 Days, 14 Days, or Post Rain) _____

Post Significant Rainfall: N/A / Rainfall Amount: _____

Is inspector qualified to perform inspections? Yes

Are inspector qualifications present in SWPPP? Yes

Was the entire site inspected? _____

If no, please list conditions limiting the scope of the inspection: _____

General Notes:

Please note if the following areas or controls were observed in compliance during the inspection.

Do the following items comply with SWPPP regulation?	Yes/No or Note Corrective Action Taken
Copy of the NOI with the SWPPP?	
Construction Site Notice posted at entrance(s) to site?	
Copy of the NOI at the site entrance?	
Do storage areas show signs of erosion?	
Do disturbed areas show signs of erosion?	
Are there signs of erosion at outfalls?	
BMPs working properly? (If no, make list of issue locations in area of concern/corrective action section below)	
Do BMPs need maintenance? (If yes, make a detailed list of issue locations in area of concern/corrective action section below.)	
Are new BMPs required on-site?	
Did the site map/BMP map get updated?	

SWPPP Inspection Report

Control	Compliant (Yes - No - N/A)
General	
Revegetation	
Silt Fence	
Rock Berm	
Sediment Traps	
Tree Protection	
Site Stabilization	
Detention and/or Water Quality Pond	
Stabilized Construction Entrance	
Concrete Washout	
Spoils/Materials Site	
Drainage Channels	
Outfall/Outlet Protections	
Inlet Protections	
No Off-site Discharge	
Equipment Area	
Trash receptacles	
Construction Debris	
Infrastructure	
Roadway clearing	
Utility clearing	
Roadway grading	
Utility construction	
Drainage construction	
Roadway base	
Roadway surfaces	
Site cleanups	

Inspector Qualifications: _____

By my signature below, I certify that all terms are acceptable and the project site is in compliance with SWPPP.

Inspector's Name

Inspector's Signature

Name of Owner/Operator (Firm)

Date

SWPPP Inspection Report**Project Milestone Dates**

Date when major site grading activities begin:

Construction Activity

Date

Dates when construction activities temporarily or permanently cease on all or a portion of the project:

Construction Activity

Date

Dates when stabilization measures are initiated:

Stabilization Activity

Date

Attachment J – Schedule of Interim and Permanent Soil Stabilization Practices**STABILIZATION PRACTICES**

Installation and utilization of stabilization measures will begin as soon as practicable in any portion of the site where construction activities have either temporarily or permanently ceased. Stabilization measures must be initiated immediately, where construction activities have temporarily ceased and will not resume for a period exceeding 14 calendar days. The term “immediately” is used to define the deadline for initiating stabilization measures. In the context of this requirement, “immediately” means as soon as practicable, but no later than the end of the next work day, following the day when the earth- disturbing activities have temporarily or permanently ceased. Temporary / Interim stabilization methods should be utilized in situations where development and/or construction practices have ceased temporarily, and permanent stabilization methods should be utilized after development and/or construction activities have been completed.

Disturbed areas to receive paving, landscape treatment and turfing shall be covered by erosion control blankets. All other rough graded slopes, disturbed ground surfaces and discharge channels shall receive seeding with native seed mix and then covered by erosion control blankets or straw mulching or other approved BMP. Stockpile materials shall be seeded and covered by soil erosion blankets. A storm water perimeter control device shall be established at a minimum distance of 10 feet from the toe of the stockpile. The materials excavated from utility trenching shall be protected from up gradient storm run- on. The excavated materials shall be covered by erosion control blankets.

TEMPORARY STABILIZATION

Temporary (Interim) Stabilization

Seed Specification: INTERIM SEEDING: N/A

Temporary vegetation - establishment of natural grassy areas that are intended to be re-disturbed during later phases of construction or development. Temporary vegetation is usually accomplished by spreading rapidly growing grasses via the process of hydro-seeding or hydro-mulching.

Mulching - the process of spreading a ground layer of chipped wood or brush to protect disturbed and unstable topsoil against erosion by storm water runoff by slowing run-off velocities, promoting sediment deposition, filtering sediment, and promoting increased ground infiltration rates. Mulching also provides the added benefits of reducing soil water loss, which is beneficial when attempting to establish newly planted vegetation. Applied in thicker layers and the size of mulch chips, mulching can also be used to prevent erosion on areas of steeper slope.

Geo-textiles - Geo-textiles (i.e. fiber matting, coir, filter fabrics) are porous materials or ground coverings which allow storm water run-off to pass through, but block the passage of most sediment and larger suspended particles. Geo-textiles matting can be used on newly seeded slopes to lessen seed and soil loss, or next to riprap to prevent run-off from washing out the soil beneath.

Vegetative buffer strips - areas where vegetation has been left undisturbed or where vegetation has been re-established, typically in long, narrow strips. Buffer strip areas retard the speed of storm water runoff, promote sediment filtration, increase ground infiltration, and improve site aesthetics. Vegetative buffer strips are extremely effective on steep, unstable slopes, or within floodplains, and along the bank slopes of waterways.

Tree Protection - is a required practice by most regulatory agencies. Only trees of certain sizes are required to be protected. Refer to your specific governing jurisdiction for specific regulations. However, even if tree protection is not a required, regulated practice it is still and important and cost effective erosion control method. (reference: **Preservation of mature vegetation** for specific details)

Preservation of mature vegetation - provides a natural buffer zone and promotes improved storm water run-off quality by helping minimize topsoil erosion as well as providing cost effective aesthetic benefits. Established, mature vegetation can withstand and tolerate heavier storm events than newly planted vegetation, due to a deeper, more established root system. It is necessary that preservation of existing, mature vegetation be planned for in advance of site construction. Areas to be preserved should be clearly marked and possibly even barricaded to prevent damage during construction.

Interim Stabilization Practices:	When Implemented:	Located:	Purpose:	In Use:
Temporary Vegetation	Throughout site development	N/A	Temporary vegetation growth is recommended to reduce soil erosion in areas that are not actively under development.	NO
Mulching	Throughout site development	N/A	Mulching is utilized to reduce topsoil erosion and to prevent soil water loss. This method can be used in planted/landscaped areas to prevent soil movement and water loss until vegetation is established.	NO

Geo-textiles	Throughout site development	N/A	Geo-textiles (i.e. matting, Curlex) can be used to temporarily stabilize soil in areas where it is not feasible to utilize mulching or temporary vegetation.	NO
Vegetative Buffer Strips	Throughout site development	Located at perimeters of the site and along natural creek beds	Vegetative buffer strips will be utilized throughout the site for both drainage and aesthetic purposes, as well as for the secondary benefits of improved water quality due to sediment deposition and improved infiltration.	NO
Tree Protection	Throughout site development	Located around all desirable trees to be retained, per plan	Desirable trees throughout the site are to be protected during and after construction to promote both water quality and aesthetics.	YES
Preservation of Existing Mature Vegetation	Throughout site development	Desirable existing vegetation to be preserved throughout the site, per plan	Desirable existent mature vegetation (i.e. under-story) is to be preserved throughout the site to promote water quality via sediment deposition and improved infiltration.	YES

PERMANENT STABILIZATION: Permanent drainage structures, including concrete curbs and gutters, concrete pavement, asphalt pavement, drainage swales, drainage ditch, turfing, vegetative strips, concrete culvert and pipe culvert will provide permanent erosion control at this project site. After initial stabilization, the Contractor shall inspect the site once a month until project acceptance as been granted by the Customer Representative/Contract Manager. Unsatisfactory stabilized areas shall be future stabilized at the request of the Customer Representative/Contract Manager. Final or permanent stabilization shall be in accordance with the specification sections: [2300 Earthwork], [02916 Mulching for erosion control],[02921 Seeding],[02922 Sodding],[02923 Sprigging],[02919 Top soil], [02924 Seeding] and [02925or 02926 Establishment of Turf].

Seed Specification: PERMANENT SEEDING: Permanent stabilization to be according to site specific re- stabilization / landscape plan and / or the San Antonio Ordinances.

Permanent vegetation - the process of establishing a permanent vegetative ground cover that helps reduce topsoil erosion by holding and stabilizing soil particles, which in turn slows storm water run-off velocity, promotes ground infiltration, promoting sediment deposition, and by providing secondary aesthetic benefits. Permanent vegetation is established by planting and seeding in areas where the soil needs stabilization due to existing soil structure, texture, or steeper grade slopes. Permanent vegetation can include trees, grasses and shrubs.

Mulching - the process of spreading a ground layer of chipped wood or brush to protect disturbed and unstable topsoil against erosion by storm water runoff by slowing run-off velocities, promoting sediment deposition, filtering sediment, and promoting increased ground infiltration rates. Mulching also provides the added benefits of reducing soil water loss, which is beneficial when attempting to establish newly planted vegetation. Applied in thicker layers and the size of mulch chips, mulching can also be used to prevent erosion on areas of steeper slope.

Geo-textiles - Geo-textiles (i.e. fiber matting, coir, filter fabrics) are porous materials or ground coverings which allow storm water run-off to pass through, but block the passage of most sediment and larger suspended particles. Geo-textiles matting can be used on newly seeded slopes to lessen seed and soil loss, or next to riprap to prevent run-off from washing out the soil beneath.

Sod stabilization - the practice of installing grass sod strips or squares over a disturbed or unprotected topsoil surface to provide instant protection of soil from the erosive forces of storm water run-off. Sod stabilization is an effective and feasible practice in areas where construction activities are complete increasing the chances that the grass cover will have the opportunity to become established. This measure requires maintenance such as the installation of sub-sod topsoil and frequent watering to promote sod growth.

Hydro-mulch/seeding stabilization - the practice of applying seed mixtures hydraulically with paper or wood mulch material over a disturbed or unprotected topsoil surface to provide vegetative protection of soil from the erosive forces of storm water run-off. Hydro-mulch/seeding stabilization is an effective and feasible practice in areas where construction activities are complete increasing the chances that the grass cover will have the opportunity to become established. This measure requires maintenance such as the placement of topsoil and frequent watering to promote sod growth.

Vegetative buffer strips - areas where vegetation has been left undisturbed or where vegetation has been re-established, typically in long, narrow strips. Buffer strip areas retard the speed of storm water runoff, promote sediment filtration, increase ground infiltration, and improve site aesthetics. Vegetative buffer strips are extremely effective on steep, unstable slopes, or within floodplains, and along the bank slopes of waterways

Paved or impervious surfaces - provides permanent stabilization by protecting soil from exposure of impact erosion by rainfall with a layer of concrete, asphalt or other impervious cover.

Preservation of mature vegetation - provides a natural buffer zone and promotes improved storm water run-off quality by helping minimize topsoil erosion as well as providing cost effective aesthetic benefits. Established, mature vegetation can withstand and tolerate heavier storm events than newly planted vegetation, due to a deeper, more established root system. It is necessary that preservation of existing, mature vegetation be planned for in advance of site construction. Areas to be preserved should be clearly marked and possibly even barricaded to prevent damage during construction.

Permanent Stabilization Practices:	When Implemented:	Located:	Purpose:	In Use:
Permanent Vegetation (i.e. grasses, shrubbery, trees)	Installed during the last phase of site development	To be located throughout site, per plan	Installation of permanent vegetation is a method of reducing and preventing soil erosion, improved infiltration and increases site aesthetics.	YES
Mulching	Installed during the last phase of site development	N/A	Mulching is utilized to reduce topsoil erosion and to prevent soil water loss. This method can be used in planted/landscaped areas to prevent soil movement and water loss until vegetation is well established.	NO
Geo-textiles	Installed during the last phase of site development	To be located in areas of significant soil disturbance	Geo-textiles are utilized to reduce soil erosion and promote vegetation growth in high slope and/or high water flow areas.	NO
Sod Stabilization	Installed during the last phase of site development	To be located throughout the site, per landscaping plan	Sod stabilization is used to establish a complete and instant vegetative ground cover in an effort to prevent topsoil erosion.	YES
Hydro-mulch/Seeding	Installed during the last phase of site development	To be used throughout the site, per landscaping plan	Hydro-mulch/seeding stabilization is used to establish a complete vegetative ground cover in an effort to prevent topsoil erosion.	YES
Vegetative Buffer Strips	Installed during the last phase of site development	To be located at perimeter of site	Vegetative buffer strips will be utilized throughout the site for both drainage and aesthetic purposes, as well as for the secondary benefits of improved water quality due to sediment deposition and improved infiltration.	YES
Paved and/or Impervious Surfaces	Installed during the last phase of site development	Throughout the site	Areas where structural concrete are located within the site; minimize and prevent erosion at those locations	YES
Preservation of Existing Mature Vegetation	Installed during the last phase of site development	Located at perimeters of site	Desirable existent mature vegetation (i.e. under-story) is to be preserved throughout the site to promote water quality via sediment deposition and improved infiltration.	YES

7– TCEQ - 0600 Attachments

Permanent Stormwater Section

Permanent Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(C), (D)(li), (E), and (5), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Permanent Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: Marisa Keiser, PE

Date: August 4, 2025

Signature of Customer/Agent



Regulated Entity Name: O'Reilly Auto Parts

Permanent Best Management Practices (BMPs)

Permanent best management practices and measures that will be used during and after construction is completed.

1. ☒ Permanent BMPs and measures must be implemented to control the discharge of pollution from regulated activities after the completion of construction.
☐ N/A
2. ☒ These practices and measures have been designed, and will be constructed, operated, and maintained to insure that 80% of the incremental increase in the annual mass loading of total suspended solids (TSS) from the site caused by the regulated activity is removed. These quantities have been calculated in accordance with technical guidance prepared or accepted by the executive director.
☒ The TCEQ Technical Guidance Manual (TGM) was used to design permanent BMPs and measures for this site.

☐ A technical guidance other than the TCEQ TGM was used to design permanent BMPs and measures for this site. The complete citation for the technical guidance that was used is: _____

☐ N/A

3. ☒ Owners must insure that permanent BMPs and measures are constructed and function as designed. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the appropriate regional office within 30 days of site completion.

☐ N/A

4. Where a site is used for low density single-family residential development and has 20 % or less impervious cover, other permanent BMPs are not required. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.

☐ The site will be used for low density single-family residential development and has 20% or less impervious cover.

☐ The site will be used for low density single-family residential development but has more than 20% impervious cover.

☒ The site will not be used for low density single-family residential development.

5. The executive director may waive the requirement for other permanent BMPs for multi-family residential developments, schools, or small business sites where 20% or less impervious cover is used at the site. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner must notify the appropriate regional office of these changes.

☐ **Attachment A - 20% or Less Impervious Cover Waiver.** The site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is attached.

☐ The site will be used for multi-family residential developments, schools, or small business sites but has more than 20% impervious cover.

☒ The site will not be used for multi-family residential developments, schools, or small business sites.

6. ☒ **Attachment B - BMPs for Upgradient Stormwater.**

- ☐ A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is attached.
- ☒ No surface water, groundwater or stormwater originates upgradient from the site and flows across the site, and an explanation is attached.
- ☐ Permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site, and an explanation is attached.
7. ☒ **Attachment C - BMPs for On-site Stormwater.**
- ☐ A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is attached.
- ☐ Permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, and an explanation is attached.
8. ☒ **Attachment D - BMPs for Surface Streams.** A description of the BMPs and measures that prevent pollutants from entering surface streams, sensitive features, or the aquifer is attached. Each feature identified in the Geologic Assessment as sensitive has been addressed.
- ☐ N/A
9. ☒ The applicant understands that to the extent practicable, BMPs and measures must maintain flow to naturally occurring sensitive features identified in either the geologic assessment, executive director review, or during excavation, blasting, or construction.
- ☒ The permanent sealing of or diversion of flow from a naturally-occurring sensitive feature that accepts recharge to the Edwards Aquifer as a permanent pollution abatement measure has not been proposed.
- ☐ **Attachment E - Request to Seal Features.** A request to seal a naturally-occurring sensitive feature, that includes, for each feature, a justification as to why no reasonable and practicable alternative exists, is attached.
10. ☒ **Attachment F - Construction Plans.** All construction plans and design calculations for the proposed permanent BMP(s) and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer, and are signed, sealed, and dated. The plans are attached and, if applicable include:
- ☒ Design calculations (TSS removal calculations)
- ☒ TCEQ construction notes
- ☒ All geologic features
- ☒ All proposed structural BMP(s) plans and specifications
- ☐ N/A

11. ☐ **Attachment G - Inspection, Maintenance, Repair and Retrofit Plan.** A plan for the inspection, maintenance, repairs, and, if necessary, retrofit of the permanent BMPs and measures is attached. The plan includes all of the following:
- ☐ Prepared and certified by the engineer designing the permanent BMPs and measures
 - ☐ Signed by the owner or responsible party
 - ☐ Procedures for documenting inspections, maintenance, repairs, and, if necessary retrofit
 - ☐ A discussion of record keeping procedures
- ☒ N/A
12. ☐ **Attachment H - Pilot-Scale Field Testing Plan.** Pilot studies for BMPs that are not recognized by the Executive Director require prior approval from the TCEQ. A plan for pilot-scale field testing is attached.
- ☒ N/A
13. ☒ **Attachment I - Measures for Minimizing Surface Stream Contamination.** A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is attached. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity, which increase erosion that results in water quality degradation.
- ☐ N/A

Responsibility for Maintenance of Permanent BMP(s)

Responsibility for maintenance of best management practices and measures after construction is complete.

14. ☒ The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
- ☐ N/A
15. ☒ A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development, or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.
- ☐ N/A

Attachment A - 20% or Less Impervious Cover Declaration, if project is multi-family residential, a school, or a small business and 20% or less impervious cover is proposed for the site

N/A

Attachment B - BMPs for Upgradient Stormwater

All disturbed areas are provided with perimeter erosion controls. There will be no surface water originating upstream flowing across the site. Offsite storm water runoff bypasses the disturbed areas unless it is included in the BMP. Reference construction plan set.

Attachment C - BMPs for On-Site Stormwater

Flows from the proposed commercial development will be treated by Jellyfish Device. The system will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site. Please see the attached construction plans.

Attachment D - BMPs for Surface Streams

All disturbed areas are provided with perimeter erosion controls. This development will be provided with an on-site water quality system that will be used to prevent pollutants of surface water from entering surface streams. See Construction Plans.

Attachment E - Request to Seal Features, if sealing a feature

N/A

Attachment F – Construction Plans

The full set of Construction Plans are attached, included but not limited to the following:

- General Notes
- TCEQ SCS General Notes
- SWPPP
- Proposed Conditions Plan
- Erosion & Sedimentation Control Plan
- Erosion & Sedimentation Control Details
- Proposed Water Quality Device and Drainage Area Map

PROJECT INFORMATION:

ADDRESS: 5510 WILLIAMS DRIVE
GEORGETOWN, WILLIAMSON COUNTY, TEXAS 78633
SITE ACREAGE: 1.41 ACRES
PROPOSED USE: AUTOMOBILE PARTS AND ACCESSORY SALES, INDOOR
ZONING DISTRICT: C-3 GENERAL COMMERCIAL
PROPOSED LIMITS OF CONSTRUCTION AREA: 50466 SF (1.16 AC)
PROPOSED TOTAL IMPERVIOUS COVER: 29,211.90 SF (.67 AC)
LEGAL DESCRIPTION: LOT 4, FOUNTAINWOOD PLAZA, A SUBDIVISION IN WILLIAMSON COUNTY, TEXAS, ACCORDING TO THE MAP OR PLAT THEREOF RECORDED IN CABINET CC, SLIDE -41, PLAT RECORDS OF WILLIAMSON COUNTY TEXAS
BASIS OF BEARING: NAD 83/93 HARN - TEXAS CENTRAL ZONE AS REFLECTED IN THE BEARING OF NORTH 04°04'21" WEST CALCULATED BETWEEN THE MONUMENTS FOUND AT THE SOUTHWEST CORNER OF LOT 4 AND ON NORTH LINE OF LOT 3, AS SHOWN ON THE PLAT OF FOUNTAINWOOD PLAZA, ACCORDING TO THE PLAT THEREOF RECORDED IN CABINET CC, SLIDE -41, PLAT RECORDS OF WILLIAMSON COUNTY, TEXAS.
VERTICAL DATUM: FOR THIS PROJECT IS NAVD88 BASED UPON THE CONCRETE MONUMENT AT THE SOUTHEAST CORNER OF LOT 15, FOUNTAINWOOD ESTATES PHASE 7, AS SHOWN ON THE PLAT OF FOUNTAINWOOD PLAZA, AND HAVING AN ELEVATION OF 921.22 FEET (NAVD88)
SITE BENCHMARK: NAIL SET IN CURB IN THE DRIVE AISLE ADJACENT TO THE NORTHWEST CORNER OF THE SUBJECT PROPERTY ELEVATION = 930.21 FEET

FLOODPLAIN NOTE:

FEDERAL FLOODPLAIN MAPS: 48491C0280E, WITH REVISION DATE OF SEPTEMBER 26, 2008 AS PUBLISHED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY SHOWS THIS SITE AS BEING WITHIN ZONE X (AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN) AND IS NOT IN A SPECIAL FLOOD HAZARD AREA.

WATERSHED STATUS:

THIS PROJECT IS LOCATED IN THE NORTH FORK SAN GABRIEL RIVER WATERSHED.

AQUIFER NOTE:

THIS PROJECT IS IN THE EDWARDS AQUIFER RECHARGE ZONE.

GENERAL NOTES:

- IT IS THE RESPONSIBILITY OF THE PROPERTY OWNER, AND SUCCESSORS TO THE CURRENT PROPERTY OWNER, TO ENSURE THE SUBJECT PROPERTY AND ANY IMPROVEMENTS ARE MAINTAINED IN CONFORMANCE WITH THIS SITE DEVELOPMENT PLAN.
- THIS DEVELOPMENT SHALL COMPLY WITH ALL STANDARDS OF THE UNIFIED DEVELOPMENT CODE (UDC), THE CITY OF GEORGETOWN CONSTRUCTION STANDARDS AND SPECIFICATIONS MANUAL, THE DEVELOPMENT MANUAL AND ALL OTHER APPLICABLE CITY STANDARDS.
- THIS SITE DEVELOPMENT PLAN SHALL MEET THE UDC STORMWATER REQUIREMENTS.
- ALL SIGNAGE REQUIRES A SEPARATE APPLICATION AND APPROVAL FROM THE INSPECTION SERVICES DEPARTMENT. NO SIGNAGE IS APPROVED WITH THE SITE DEVELOPMENT PLAN.
- SIDEWALKS SHALL BE PROVIDED IN ACCORDANCE WITH THE UDC.
- DRIVEWAYS WILL REQUIRE APPROVAL BY THE DEVELOPMENT ENGINEER OF THE CITY OF GEORGETOWN.
- OUTDOOR AND SITE LIGHTING SHALL COMPLY WITH SECTION 7.04 OF THE UDC.
- SCREENING OF MECHANICAL EQUIPMENT, DUMPSTERS AND PARKING SHALL COMPLY WITH CHAPTER 8 OF THE UDC. THE SCREENING IS SHOWN ON THE LANDSCAPE AND ARCHITECTURAL PLANS, AS APPLICABLE.
- THE COMPANION LANDSCAPE PLAN HAS BEEN DESIGNED AND PLANT MATERIALS SHALL BE INSTALLED TO MEET ALL REQUIREMENTS OF THE UDC.
- ALL MAINTENANCE OF REQUIRED LANDSCAPE SHALL COMPLY WITH THE MAINTENANCE STANDARDS OF CHAPTER 8 OF THE UDC.
- A SEPARATE IRRIGATION PLAN SHALL BE REQUIRED AT THE TIME OF BUILDING PERMIT APPLICATION. PROJECTS PROPOSING PUBLIC PARKLAND SHALL INCLUDE IRRIGATION PLANS AS PART OF THIS PLAN AS REQUIRED BY THE PARKLAND DEVELOPMENT GUIDELINES.
- FIRE FLOW REQUIREMENTS OF 1500 PER MINUTE ARE BEING MET BY THIS PLAN.
- ANY HERITAGE TREE NOTED ON THIS SITE DEVELOPMENT PLAN IS SUBJECT, IN PERPETUITY, TO THE MAINTENANCE, CARE, PRUNING AND REMOVAL REQUIREMENTS OF THE UNIFIED DEVELOPMENT CODE.
- THE CONSTRUCTION PORTION OF THESE PLANS WERE PREPARED, SEALED, SIGNED AND DATED BY A TEXAS LICENSED PROFESSIONAL ENGINEER. THEREFORE, BASED ON THE ENGINEER'S CONCURRENCE OF COMPLIANCE, THE CONSTRUCTION PLANS FOR CONSTRUCTION OF THE PROPOSED PROJECT ARE HEREBY APPROVED SUBJECT TO THE STANDARD CONSTRUCTION SPECIFICATIONS AND DETAILS MANUAL AND ALL OTHER APPLICABLE CITY, STATE AND FEDERAL REQUIREMENTS AND CODES.
- THIS PROJECT IS SUBJECT TO ALL CITY STANDARD CONSTRUCTION SPECIFICATIONS AND DETAILS IN EFFECT AT THE TIME OF SUBMITTAL OF THE PROJECT TO THE CITY.
- WHERE NO EXISTING OVERHEAD INFRASTRUCTURE EXISTS, UNDERGROUND ELECTRIC UTILITY LINES SHALL BE LOCATED ALONG THE STREET AND WITHIN THE SITE. WHERE EXISTING OVERHEAD INFRASTRUCTURE IS TO BE RELOCATED, IT SHALL BE RE-INSTALLED UNDERGROUND AND THE EXISTING FACILITIES SHALL BE REMOVED AT THE DISCRETION OF THE DEVELOPMENT ENGINEER.
- ALL ELECTRIC AND COMMUNICATION INFRASTRUCTURE SHALL COMPLY WITH UDC SECTION 13.06.
- A (VARIANCE OR SPECIAL EXCEPTION) WAS APPROVED BY THE ZONING BOARD OF ADJUSTMENT ON (DATE) TO ALLOW (DESCRIBALLOWANCE) (IF APPLICABLE)
- ALTERNATIVE IMPERVIOUS COVER STANDARDS HAVE BEEN APPROVED IN ACCORDANCE WITH SECTION 11.02 OF THE UDC FOR _____ (IF APPLICABLE)
- SCREENING AND LOCATION OF OUTDOOR STORAGE SHALL COMPLY WITH SECTION 5.09 OF THE UDC. (IF APPLICABLE)
- THE PROPERTY SUBJECT TO THIS APPLICATION IS SUBJECT TO THE WATER QUALITY REGULATIONS OF THE CITY OF GEORGETOWN. (FOR PROPERTIES LOCATED OVER THE EDWARDS AQUIFER RECHARGE ZONE)
- A GEOLOGIC ASSESSMENT, IN ACCORDANCE WITH THE CITY OF GEORGETOWN WATER QUALITY REGULATIONS, WAS COMPLETED ON JUNE 3, 2025. ANY SPRINGS AND STREAMS AS IDENTIFIED IN THE GEOLOGIC ASSESSMENT ARE SHOWN HEREIN.

APPROVED BY:

Marisa Keiser
MARISA KEISER, P.E.



August 4, 2025
DATE

BOWMAN CONSULTING GROUP
TBPE FIRM NO. 14309
PRELIMINARY - FOR INTERIM REVIEW ONLY

THESE DOCUMENTS ARE FOR INTERIM REVIEW AND NOT INTENDED FOR CONSTRUCTION, BIDDING, OR PERMIT PURPOSES. THEY WERE PREPARED BY, OR UNDER THE SUPERVISION OF:

CITY OF GEORGETOWN PLANNING DEPARTMENT

DATE

MARISA KEISER - PER 141397 ON August 1, 2025

REVISION & CORRECTIONS

NUMBER	DESCRIPTION	REVISE (R) CORRECTION (C) ADD (A) VOID (V) SHEET NO.'S	NET CHANGE IMPERVIOUS COVER (SQ. FT.)	TOTAL IMPERVIOUS COVER (SQ. FT.)/%	DESIGN ENGINEER SIGNATURE	CITY OF GEORGETOWN APPROVAL	APPROVAL DATE

O'REILLY AUTO PARTS

CONSTRUCTION PLANS

5510 WILLIAMS DRIVE

GEORGETOWN, WILLIAMSON COUNTY, TEXAS 78633



VICINITY MAP

SCALE: 1" = 400'

SUBMITTAL DATE: AUGUST 4, 2025

DEVELOPER/OWNER:

O'REILLY AUTO PARTS
233 S PATTERSON AVENUE
SPRINGFIELD, MO
EMAIL: RH4K526@O'REILLYAUTO.COM
(417)427-4062 EXT 65680
CONTACT: RICK HICKS

ENGINEER:

BOWMAN CONSULTING GROUP, LTD.
807 LAS CIMAS PKWY, LAS CIMAS II, SUITE 350
AUSTIN, TEXAS 78746
512-327-1160
CONTACT: MARISA KEISER, PE

SURVEYOR:

WINDROSE
9380 CORPORATE DRIVE, STE 102
SELMA, TEXAS 78154
210-634-1565
CONTACT: PAUL JONES

Bowman

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BENCHMARK NOTES

1. BENCHMARK #1
NAIL SET IN CURB IN THE DRIVE AISLE ADJACENT TO THE NORTHWEST CORNER OF THE SUBJECT PROPERTY
ELEV.: 930.21

CONSTRUCTION GENERAL

1. CONTRACTORS SHALL HAVE AN APPROVED SET OF PLANS WITH APPROVED REVISIONS ON SITE AT ALL TIMES. FAILURE TO HAVE APPROVED PLANS ON SITE MAY RESULT IN ISSUANCE OF WORK STOPPAGE.
2. CONTACT OF UTILITY COMPANIES NEAR PROJECT:
- 2.1. SEWER, WATER, STORM & ROADS
- CITY OF GEORGETOWN
308-1 INDUSTRIAL AVE
GEORGETOWN, TEXAS 78633
(NO CONTACT PROVIDED)
512-930-3640
- 2.2. NATURAL GAS
- ATMOS ENERGY CORP.
5420 LBJ FREEWAY STE 1700
DALLAS, TX 75420
DAVID DOBERNECKI
512-310-3854
- 2.3. ELECTRIC
- FEDERNALES ELECT COOP. INC.
201 S. AVENUE F
JOHNSON CITY, TEXAS 78636
(NO CONTACT PROVIDED)
512-355-2131
- 2.4. FIBER OPTIC
- FRONTIER COMMUNICATIONS
911 S CHURCH ST, STE 5931
GEORGETOWN, TX 78626
TREY VALDEZ
512-866-2231
- 2.5. OTHER
- OPTIMUM/ALTICE USA
911 S CHURCH ST, STE 5931
GEORGETOWN, TX 78626
DEAN COLE
512-931-2965
3. CONTACT 811 SYSTEM FOR EXISTING WATER AND WASTEWATER LOCATIONS 48 HOURS PRIOR TO CONSTRUCTION.
- REFRESH ALL LOCATES BEFORE 14 DAYS – LOCATE REFRESH REQUESTS MUST INCLUDE A COPY OF YOUR 811 TICKET. TEXAS PIPELINE DAMAGE PREVENTION LAWS REQUIRE THAT A LOCATE REFRESH REQUEST BE SUBMITTED BEFORE 14 DAYS, OR IF LOCATION MARKERS ARE NO LONGER VISIBLE.
- a. REPORT PIPELINE DAMAGE IMMEDIATELY – IF YOU WITNESS OR EXPERIENCE PIPELINE EXCAVATION DAMAGE, PLEASE CONTACT THE CITY OF GEORGETOWN BY PHONE 512-930-3640.
4. THE CONTRACTOR SHALL CONTACT THE CITY INSPECTOR 48 HOURS BEFORE:
- a. BEGINNING EACH PHASE OF CONSTRUCTION. CONTACT ASSIGNED CITY INSPECTOR.
- b. ANY TESTING. CONTRACTOR SHALL PROVIDE QUALITY TESTING FOR ALL INFRASTRUCTURES TO BE ACCEPTED AND MAINTAINED BY THE CITY OF GEORGETOWN AFTER COMPLETION.
- c. PRIOR ROLLING SUB-GRADE AND EVERY LIFT OF ROADWAY EMBANKMENT, IN-PLACE DENSITY TESTING OF EVERY BASE COURSE, AND ASPHALT CORES. ALL OF THIS TESTING MUST BE WITNESSED BY A CITY OF GEORGETOWN REPRESENTATIVE.
- d. CONNECTING TO THE EXISTING WATER LINES.
- e. THE INSTALLATION OF ANY DRAINAGE FACILITY WITHIN A DRAINAGE EASEMENT OR STREET ROW. THE METHOD OF PLACEMENT AND COMPACTION OF BACKFILL IN THE CITY'S ROW MUST BE APPROVED PRIOR TO THE START OF BACKFILL OPERATIONS.
4. ALL RESPONSIBILITY FOR THE ACCURACY OF THESE PLANS REMAINS WITH THE ENGINEER OF RECORD WHO PREPARED THEM. IN REVIEWING THESE PLANS, THE CITY MUST RELY ON THE ADEQUACY OF THE WORK OF THE ENGINEER OF RECORD.
5. EXCESS SOIL SHALL BE REMOVED AT THE CONTRACTOR'S EXPENSE. NOTIFY THE CITY OF GEORGETOWN IF THE DISPOSAL SITE IS INSIDE THE CITY'S JURISDICTIONAL BOUNDARIES.
6. BURNING IS PROHIBITED UNLESS THE GEORGETOWN FIRE DEPARTMENT ISSUE AN OPERATIONAL PERMIT INSIDE THE CITY LIMITS AND E.S.D. 8 FOR OUTDOOR BURNING.
7. DAYTIME MEANS THE TIME BETWEEN 7:00 A.M. AND 10:00 P.M. NO WORK IS TO BE PERFORMED AT NIGHTTIME. CHRISTMAS, NEW YEAR'S OR THANKSGIVING. THE CITY INSPECTOR RESERVES THE RIGHT TO REQUIRE THE CONTRACTOR TO UNCOVER ALL WORK PERFORMED WITHOUT INSPECTION.
8. CONTACT THE CITY INSPECTOR 4 DAYS PRIOR TO WORK FOR APPROVAL TO SCHEDULE ANY INSPECTIONS ON WEEKENDS OR CITY HOLIDAYS.
9. NO BLASTING IS ALLOWED.
10. ANY CHANGES OR REVISIONS TO THESE PLANS MUST FIRST BE SUBMITTED TO THE CITY BY THE DESIGN ENGINEER FOR REVIEW AND WRITTEN APPROVAL PRIOR TO CONSTRUCTION OF THE REVISION. ALL CHANGES AND REVISIONS SHALL USE REVISION CLOUDS TO HIGHLIGHT ALL REVISIONS AND CHANGES WITH EACH SUBMITTAL. REVISION TRIANGLE MARKERS AND NUMBERS SHALL BE USED TO MARK REVISIONS. ALL CLOUDS AND TRIANGLE MARKERS FROM PREVIOUS REVISIONS MUST BE REMOVED. REVISION INFORMATION SHALL BE UPDATED ON COVER SHEET AND AFFECTED PLAN SHEET TITLE BLOCK.
11. THE CONTRACTOR AND ENGINEER SHALL KEEP ACCURATE RECORDS OF ALL CONSTRUCTION THAT DEVIATES FROM THE PLANS. THE ENGINEER SHALL FURNISH THE CITY OF GEORGETOWN ACCURATE "RECORD DRAWINGS" FOLLOWING THE COMPLETION OF ALL CONSTRUCTION. THESE "RECORD DRAWINGS" SHALL MEET THE SATISFACTION OF THE ENGINEERING DEPARTMENTS PRIOR TO FINAL ACCEPTANCE.
12. THE CONTRACTOR WILL REIMBURSE THE CITY FOR ALL REPAIR AND/OR COST INCURRED AS A RESULT OF ANY DAMAGE TO ANY PUBLIC INFRASTRUCTURE WITHIN CITY EASEMENT OR PUBLIC RIGHT-OF-WAY, REGARDLESS OF THESE PLANS.
13. WHEN CONSTRUCTION IS BEING CARRIED OUT WITHIN EASEMENTS, THE CONTRACTOR SHALL CONFINE HIS WORK TO WITHIN THE PERMANENT AND TEMPORARY EASEMENTS. PRIOR TO ACCEPTANCE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING ALL TRASH AND DEBRIS WITHIN THE PERMANENT EASEMENTS. CLEANUP SHALL BE TO THE SATISFACTION OF THE ENGINEER OF RECORD AND CITY.
14. CONTRACTOR TO LOCATE, PROTECT, AND MAINTAIN BENCHMARKS, MONUMENTS, CONTROL POINTS AND PROJECT ENGINEERING REFERENCE POINTS. RE-ESTABLISH DISTURBED OR DESTROYED ITEMS BY REGISTERED PROFESSIONAL LAND SURVEYOR IN THE STATE OF TEXAS, AT NO ADDITIONAL COST TO THE PROPERTY OWNER.
15. ALL CONSTRUCTION OPERATIONS SHALL BE ACCOMPLISHED IN ACCORDANCE WITH APPLICABLE REGULATIONS OF THE U.S. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA). OSHA STANDARDS MAY BE PURCHASED FROM THE GOVERNMENT PRINTING OFFICE. INFORMATION AND RELATED REFERENCE MATERIALS MAY BE PURCHASED FROM OSHA, 1033 LA POSADA DR, SUITE 375, AUSTIN, TEXAS 78752-3832.
16. ALL MANHOLE FRAMES/COVERS AND WATER VALVE/METER BOXES MUST BE ADJUSTED TO FINISHED GRADE AT THE OWNER'S EXPENSE BY THE CONTRACTOR FOR CITY CONSTRUCTION INSPECTOR INSPECTION. ALL UTILITY ADJUSTMENTS SHALL BE COMPLETED PRIOR TO FINAL CONTRACTOR SHALL BACKFILL AROUND MANHOLES AND VALVE BOXES WITH CLASS A CONCRETE.
17. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THE SCOPE OF THIS CONTRACT WHERE NOT SPECIFICALLY COVERED IN THE PROJECT SPECIFICATIONS SHALL CONFORM TO ALL CITY OF GEORGETOWN DETAILS AND CITY OF AUSTIN STANDARD SPECIFICATIONS.
18. PROJECT SPECIFICATIONS TAKE PRECEDENCE OVER PLANS AND SPECIAL CONDITIONS GOVERN OVER TECHNICAL SPECIFICATIONS.
19. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ALL PERMITS, TESTS, APPROVALS AND ACCEPTANCES REQUIRED TO COMPLETE CONSTRUCTION OF THIS PROJECT.
20. THE CONTRACTOR MUST OBTAIN A CONSTRUCTION WATER METER FOR ALL WATER USED DURING CONSTRUCTION. A COPY OF THIS PERMIT MUST BE CARRIED AT ALL TIMES BY ALL WHO USE WATER.
21. THE CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING ROADS AND DRIVES ADJACENT TO AND NEAR THE SITE FREE FROM SOIL, SEDIMENT AND DEBRIS. CONTRACTOR WILL NOT REMOVE SOIL, SEDIMENT OR DEBRIS FROM ANY AREA OR VEHICLE BY MEANS OF WATER. ONLY SHOVELING AND SWEEPING WILL BE ALLOWED. THE CONTRACTOR WILL BE RESPONSIBLE FOR DUST CONTROL FROM THE SITE. THE CONTRACTOR SHALL KEEP THE SITE AREA CLEAN AND MAINTAINED AT ALL TIMES, TO THE SATISFACTION OF THE CITY. THE SUBDIVISION (OR SITE) WILL NOT BE ACCEPTED (OR CERTIFICATE OF OCCUPANCY ISSUED) UNTIL THE SITE HAS BEEN CLEARED TO THE SATISFACTION OF THE CITY.
22. TREES IN EXISTING ROW SHOULD BE PROTECTED OR NOTED IN THE PLANS TO BE REMOVED.

CONSTRUCTION SEQUENCE NOTES

- BELOW IS GENERAL SEQUENCE OF CONSTRUCTION. THE ENGINEER OF RECORD SHALL UPDATE BELOW WITH NOTES SPECIFIC TO THE PROJECT.
1. REACH OUT TO THE CITY FOR PRE-CONSTRUCTION MEETING AND CONSTRUCTION PERMIT.
2. SET-UP EIS CONTROLS AND TREE PROTECTION AND REACH OUT TO CITY FOR INSPECTION.
3. SET UP TEMPORARY TRAFFIC CONTROLS.
4. CONSTRUCT THE DRAINAGE PONDS AND STORM WATER FEATURES.
5. START UTILITY, ROAD, GRADING, FRANCHISE UTILITY AND ALL NECESSARY INFRASTRUCTURE CONSTRUCTION. [NOTE: PLEASE UPDATE AS PER THE PROJECT]
6. REQUEST FINAL WALKTHROUGH AND CONDUCT WALKTHROUGH WITH ENGINEER OF RECORD AND CITY DEPARTMENT.
7. ENGINEER OF RECORD IS RESPONSIBLE TO PREPARE AND SUBMIT CLOSEOUT DOCUMENTS FOR PROJECT CLOSEOUT.

EROSION CONTROL NOTES

1. THE CONTRACTOR IS REQUIRED TO INSPECT THE CONTROLS AND FENCES AT WEEKLY INTERVALS AND AFTER SIGNIFICANT RAINFALL EVENTS TO ENSURE THAT THEY ARE FUNCTIONING PROPERLY. THE CONTRACTOR IS RESPONSIBLE FOR MAINTENANCE OF CONTROLS AND FENCES AND SHALL IMMEDIATELY MAKE ANY NECESSARY REPAIRS TO DAMAGED AREAS. SILT ACCUMULATION AT CONTROLS MUST BE REMOVED WHEN THE DEPTH REACHES SIX (6) INCHES.
2. THE TEMPORARY SPOILS DISPOSAL SITE IS TO BE SHOWN IN THE EROSION CONTROL MAP.
3. ANY ON-SITE SPOILS DISPOSAL SHALL BE REMOVED PRIOR TO ACCEPTANCE UNLESS SPECIFICALLY SHOWN ON THE PLANS. THE DEPTH OF SPOIL SHALL NOT EXCEED 10 FEET IN ANY.
4. ALL AREAS DISTURBED OR EXPOSED DURING CONSTRUCTION SHALL BE RESTORED WITH A MINIMUM OF 6 INCHES OF TOPSOIL AND COMPOST BLEND, THE TOPSOIL AND COMPOST BLEND SHALL CONSIST OF 75% TOPSOIL AND 25% COMPOST.
5. SEEDING FOR REESTABLISHING VEGETATION SHALL COMPLY WITH THE AUSTIN GROW GREEN GUIDE OR WILLIAMSON COUNTY'S PROTOCOL FOR SUSTAINABLE ROADSIDES (SPEC 164-WC001 SEEDING FOR EROSION CONTROL). RESEEDING VARIETIES OF BERMDUA SHALL NOT BE USED.
6. STABILIZATION CONSTRUCTION ENTRANCE IS REQUIRED AT ALL POINTS WHERE CONSTRUCTION TRAFFIC IS EXITING THE PROJECT ONTO EXISTING PAVEMENT. LINEAR CONSTRUCTION PROJECTS MAY REQUIRE SPECIAL CONSIDERATION. ROADWAYS SHALL REMAIN CLEAR OF SILT AND MUD.
7. TEMPORARY STOP SIGNS SHOULD BE INSTALLED AT ALL CONSTRUCTION ENTRANCES WHERE A STOP CONDITION DOES NOT ALREADY EXIST.
8. IN THE EVENT OF INCLEMENT WEATHER THAT MAY RESULT IN A FLOODING SITUATION, THE CONTRACTOR SHALL REMOVE INLET PROTECTION MEASURES UNTIL SUCH TIME AS THE WEATHER EVENT HAS PASSED.

WATER AND WASTEWATER GENERAL NOTES

1. ALL NEWLY INSTALLED PIPES AND RELATED PRODUCTS MUST CONFORM TO AMERICAN NATIONAL STANDARDS INSTITUTE/NATIONAL SANITATION FOUNDATION (ANSI/NSF) STANDARD 61 AND MUST BE CERTIFIED BY AND ORGANIZATION ACCREDITED BY ANSI.
2. ALL WATER SERVICE, WASTEWATER SERVICE AND VALVE LOCATIONS SHALL BE APPROPRIATELY STAMPED AS FOLLOWS:
- f. WATER SERVICE "W" ON TOP OF CURB
- g. WASTEWATER SERVICE "S" ON TOP OF CURB
- h. VALVE "V" ON TOP OF CURB.
3. OPEN UTILITIES SHALL NOT BE PERMITTED ACROSS THE EXISTING PAVED SURFACES. WATER AND WASTEWATER LINES ACROSS THE EXISTING PAVED SURFACES SHALL BE BORED AND INSTALLED IN STEEL ENCASEMENT PIPES. BELL RESTRAINTS SHALL BE PROVIDED AT JOINTS.
4. INTERIOR SURFACES OF ALL DUCTILE IRON POTABLE OR RECLAIMED WATER PIPE SHALL BE CEMENT-MORTAR LINED AND SEAL COATED AS REQUIRED BY AWWA C104.
5. SAND, AS DESCRIBED IN AUSTIN SPECIFICATION ITEM 510 PIPE, SHALL NOT BE USED AS BEDDING FOR WATER AND WASTEWATER LINES. ACCEPTABLE BEDDING MATERIALS ARE PIPE BEDDING STONE, PEA GRAVEL AND IN LIEU OF SAND, A NATURALLY OCCURRING OR MANUFACTURED STONE MATERIAL CONFORMING TO ASTM C33 FOR STONE QUALITY AND MEETING THE FOLLOWING GRADATION SPECIFICATION:
- | SIEVE SIZE | PERCENT RETAINED BY WEIGHT |
|------------|----------------------------|
| 1/2" | 0 |
| 3/8" | 0-2 |
| #4 | 40-85 |
| #10 | 95-100 |
6. DENSITY TESTING FOR TRENCH BACKFILL SHALL BE DONE IN MAXIMUM 12" LIFTS.

WATER

1. SAMPLING TAPS SHALL BE BROUGHT UP TO 3 FEET ABOVE GRADE AND SHALL BE EASILY ACCESSIBLE FOR CITY PERSONNEL. AT THE CONTRACTORS' REQUEST, AND IN HIS PRESENCE, SAMPLES FOR BACTERIOLOGICAL TESTING WILL BE COLLECTED BY THE CITY OF GEORGETOWN NOT LESS THAN 24 HOURS AFTER THE TREATED LINE HAS BEEN FLUSHED OF THE CONCENTRATED CHLORINE SOLUTION AND CHARGED WITH WATER APPROVED BY THE CITY.
2. CITY PERSONNEL WILL OPERATE OR AUTHORIZE THE CONTRACTOR TO OPERATE ALL WATER VALVES THAT WILL PASS THROUGH THE CITY'S POTABLE WATER. THE CONTRACTOR MAY BE FINED, INCLUDING ADDITIONAL THEFT OF WATER FINES, IF A WATER VALVE IS OPERATED IN AN UNAUTHORIZED MANNER, REGARDLESS OF WHO OPERATED THE VALVE.
3. THE CONTRACTOR IS HEREBY NOTIFIED THAT CONNECTING TO, SHUTTING DOWN, OR TERMINATING EXISTING UTILITY LINES MAY HAVE TO OCCUR AT OFF-PEAK HOURS. SUCH HOURS ARE USUALLY OUTSIDE NORMAL WORKING HOURS AND POSSIBLY BETWEEN 12 AM AND 6 AM AFTER COORDINATING WITH CITY CONSTRUCTION INSPECTORS AND INFORMING AFFECTED PROPERTIES.
4. PRESSURE TAPS OR HOT TAPS SHALL BE IN ACCORDANCE WITH CITY OF GEORGETOWN STANDARD SPECIFICATIONS. THE CONTRACTOR SHALL PERFORM ALL EXCAVATION AND SHALL FURNISH, INSTALL AND AIR TEST THE SLEEVE AND VALVE. A CITY OF GEORGETOWN INSPECTOR MUST BE PRESENT WHEN THE CONTRACTOR MAKES A TAP, AND/OR ASSOCIATED TESTS. A MINIMUM OF TWO (2) WORKING DAYS NOTICE IS REQUIRED. "SIZE ON SIZE" TAPS SHALL NOT BE PERMITTED UNLESS MADE BY THE USE OF AN APPROVED FULL-CIRCLE GASKETED TAPPING SLEEVE.
- CONCRETE THRUST BLOCKS SHALL BE PLACED BEHIND AND UNDER ALL TAP SLEEVES A MINIMUM OF 24 HOURS PRIOR TO THE BRANCH BEING PLACED INTO SERVICE. THRUST BLOCKS SHALL BE INSPECTED PRIOR TO BACKFILL.
5. FIRE HYDRANTS ON MAINS UNDER CONSTRUCTION SHALL BE SECURELY WRAPPED WITH A BLACK POLY WRAP BAG AND TAPED INTO PLACE. THE POLY WRAP SHALL BE REMOVED WHEN THE MAINS ARE ACCEPTED AND PLACED INTO SERVICE.
6. THRUST BLOCKS OR RESTRAINTS SHALL BE IN ACCORDANCE WITH THE CITY OF GEORGETOWN STANDARD SPECIFICATIONS AND REQUIRED AT ALL FITTINGS PER DETAIL OR MANUFACTURER'S RECOMMENDATION. ALL FITTINGS SHALL HAVE BOTH THRUST BLOCKS AND RESTRAINTS.
7. ALL DEAD END WATER MAINS SHALL HAVE "FIRE HYDRANT ASSEMBLY" OR "BLOW-OFF VALVE AND THRUST BLOCK" OR "BLOW-OFF VALVE AND THRUST RESTRAINTS". THRUST RESTRAINTS SHALL BE INSTALLED ON THE MINIMUM LAST THREE PIPE LENGTHS (STANDARD 20' LAYING LENGTH). ADDITIONAL THRUST RESTRAINTS MAY BE REQUIRED BASED UPON THE MANUFACTURERS RECOMMENDATION AND/OR ENGINEER'S DESIGN.
8. PIPE MATERIAL FOR PUBLIC WATER MAINS SHALL BE PVC (AWWA C900-DR14 MIN. 305 PSI PRESSURE RATING). WATER SERVICES (2" OR LESS) SHALL BE POLYETHYLENE TUBING (BLACK, 20PSI, AND SDR-9), COPPER PIPES AND FITTINGS ARE NOT ALLOWED IN THE PUBLIC RIGHT OF ALL PLASTIC PIPES FOR USE IN PUBLIC WATER SYSTEMS MUST BEAR THE NATIONAL SANITATION FOUNDATION SEAL OF APPROVAL (NSF-PW).
10. ALL FIRE HYDRANT LEADS SHALL BE DUCTILE IRON PIPE (AWWA C115/C151 PRESSURE CLASS 350).
11. ALL IRON PIPE AND FITTINGS SHALL BE WRAPPED WITH MINIMUM 8-MIL POLYETHYLENE.
12. LINE FLUSHING OR ANY ACTIVITY USING A LARGE QUANTITY OF WATER MUST BE COORDINATED WITH THE PUBLIC WORKS DEPARTMENT.
13. ALL WATER METER BOXES SHALL MEET THE CITY CITY STANDARDS. SEE DETAILS SHEET #20
14. ALL WATER VALVE COVERS ARE TO BE PAINTED BLUE.

WASTEWATER

13. CURVILINEAR WASTEWATER DESIGN LAYOUT IS NOT PERMITTED.
14. MANHOLE TESTING SHALL BE CONDUCTED AFTER THE FINAL BACKFILL HAS BEEN IN PLACE AT LEAST 30 DAYS.
15. MANHOLES SHALL BE COATED PER CITY OF GEORGETOWN SPL WW-511 (RAVEN 405 OR SPRAYWALL). PENETRATIONS TO EXISTING WASTEWATER MANHOLES REQUIRE THE CONTRACTOR TO RECOAT THE ENTIRE MANHOLE IN ACCORDANCE WITH CITY OF AUSTIN STANDARD SPECIFICATIONS SECTION NO. 506.5.
16. RECLAIMED AND RECYCLED WATER LINE SHALL BE CONSTRUCTED OF "PURPLE PIPE." ALL RECLAIMED AND RECYCLED WATER VALVE COVERS SHALL BE SQUARE AND PAINTED PURPLE.
17. FORCE MAIN PIPES NEED TO HAVE SWEEPING WYES FOR JOINTS.

STREET AND DRAINAGE NOTES

1. THE CITY OF GEORGETOWN HAS NOT REVIEWED THESE PLANS FOR COMPLIANCE WITH THE AMERICANS WITH DISABILITIES ACT (ADA). IT IS THE RESPONSIBILITY OF THE OWNER TO PROVIDE COMPLIANCE WITH ALL LEGISLATION RELATED TO ACCESSIBILITY WITHIN THE LIMITS OF CONSTRUCTION SHOWN IN THESE PLANS. ALL SIDEWALKS SHALL COMPLY WITH THE AMERICANS WITH DISABILITIES ACT AND TEXAS ACCESSIBILITY STANDARDS (TAS).
2. BACKFILL BEHIND THE CURB SHALL BE COMPACTED TO OBTAIN A MINIMUM OF 95% MAXIMUM DENSITY TO WITHIN 6" OF TOP OF CURB. MATERIAL USED SHALL BE PRIMARILY GRANULAR WITH NO ROCKS LARGER THAN 6" IN THE GREATEST DIMENSION. THE REMAINING 6" SHALL BE CLEAN TOPSOIL FREE FROM ALL CLOUDS AND SUITABLE FOR SUSTAINING PLANT LIFE.
3. A MINIMUM OF 6" OF TOPSOIL SHALL BE PLACED BETWEEN THE CURB AND RIGHT-OF-WAY AND IN ALL DRAINAGE CHANNELS EXCEPT CHANNELS CUT IN STABLE ROCK.
4. DEPTH OF COVER FOR ALL CROSSINGS UNDER PAVEMENT, INCLUDING GAS, ELECTRIC TELEPHONE, CABLE TV, ETC., SHALL BE A MINIMUM OF 36" BELOW SUBGRADE.
5. ALL DRAINAGE PIPE IN PUBLIC RIGHT OF WAY OR EASEMENTS SHALL BE REINFORCED CONCRETE PIPE MINIMUM CLASS III OF TONGUE AND GROOVE OR O-RING JOINT DESIGN. CORRUGATED METAL PIPE IS NOT ALLOWED IN PUBLIC RIGHT OR WAY OR EASEMENTS.
6. ALL STRIPING, WITH THE EXCEPTION OF STOP BARS, CROSS WALKS, WORDS AND ARROWS, IS TO BE TYPE II (WATER BASED). STOP BARS, CROSS WALKS, WORDS AND ARROWS REQUIRE TYPE I THERMOPLASTIC.
7. MANHOLE FRAMES, COVERS, VALVES, CLEAN-OUTS, ETC. SHALL BE RAISED TO GRADE PRIOR TO FINAL PAVEMENT CONSTRUCTION.
8. A STOP BAR SHALL BE PLACED AT ALL STOP SIGN LOCATIONS.
9. THE GEOTECHNICAL ENGINEER SHALL INSPECT THE SUBGRADE FOR COMPLIANCE WITH THE DESIGN ASSUMPTIONS MADE DURING PREPARATION OF THE SOILS REPORT. ANY ADJUSTMENTS THAT ARE REQUIRED SHALL BE MADE THROUGH REVISIONS OF THE APPROVED CONSTRUCTION PLANS.
10. GEOTECHNICAL INVESTIGATION INFORMATION AND PAVEMENT RECOMMENDATIONS WERE PROVIDED BY ALPHA TESTING. REPORT NUMBER A230982. SEE PAVEMENT PLAN FOR REFERENCE.
11. A TRAFFIC CONTROL PLAN, IN ACCORDANCE WITH THE TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, CITY OF AUSTIN TRANSPORTATION CRITERIA MANUAL, CITY OF GEORGETOWN STANDARD DETAILS AND TEXAS DEPARTMENT OF TRANSPORTATION CRITERIA, SHALL BE SUBMITTED TO THE CITY OF GEORGETOWN FOR REVIEW AND APPROVAL PRIOR TO ANY PARTIAL OR COMPLETE ROADWAY CLOSURES. TRAFFIC CONTROL PLANS MUST BE SITE SPECIFIC AND SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER.
12. TEMPORARY ROCK CRUSHING IS NOT ALLOWED. ALL SOURCES OF FLEXIBLE BASE MATERIAL ARE REQUIRED TO BE APPROVED BY THE CITY. PRIOR TO BASE PLACEMENT ALL CURRENT TRIAXIAL TEST REPORTS FOR PROPOSED STOCK PILES ARE TO BE SUBMITTED TO THE CITY CONSTRUCTION INSPECTOR FOR REVIEW AND APPROVAL.
13. AT ROAD INTERSECTIONS THAT HAVE A VALLEY GUTTER, THE CROWN TO THE INTERSECTING ROAD WILL BE CULMINATED AT A DISTANCE OF 40 FEET FROM THE INTERSECTING CURB LINE UNLESS OTHERWISE NOTED.
14. NO PONDING OF WATER SHALL BE ALLOWED TO COLLECT ON OR NEAR THE INTERSECTION OF PRIVATE DRIVEWAYS AND PUBLIC STREETS. RECONSTRUCTION OF THE DRIVEWAY APPROACH SHALL BE AT THE CONTRACTOR'S EXPENSE.
15. IN A MANNER WHICH RETAINS OPERATIONS OF NOT LESS THAN HALF OF THE DRIVEWAY TO REMAIN OPEN AT ALL TIMES, FULL CLOSURE OF SUCH DRIVEWAY CAN BE CONSIDERED WITH WRITTEN AUTHORIZATION OBTAINED BY THE CONTRACTOR FROM ALL PROPERTY OWNERS AND ACCESS EASEMENT RIGHT HOLDERS ALLOWING THE FULL CLOSURE OF THE DRIVEWAY.
20. CONTRACTOR MUST CLEAR FIVE (5) FEET BEYOND ALL PUBLIC RIGHT OF WAY TO PREVENT FUTURE VEGETATIVE GROWTH INTO THE SIDEWALK AREAS.
21. SLOPE OF NATURAL GROUND ADJACENT TO THE PUBLIC RIGHT OF WAY SHALL NOT EXCEED 3:1 IF A 3:1 SLOPE IS NOT POSSIBLE, SLOPE PROTECTION OR RETAINING WALL MUST BE SUBMITTED TO THE CITY FOR REVIEW AND APPROVAL PRIOR TO FINAL ACCEPTANCE.
22. THERE SHALL BE NO WATER, WASTEWATER OR DRAINAGE APPURTENANCES, INCLUDING BUT NOT LIMITED TO VALVES, FITTINGS, METERS, CLEAN-OUTS, MANHOLES, OR VAULTS IN ANY DRIVEWAY, SIDEWALK, TRAFFIC OR PEDESTRIAN AREA.

23. PUBLIC SIDEWALKS SHALL NOT USE CURB INLETS AS PARTIAL WALKING SURFACE. SIDEWALKS SHALL NOT USE TRAFFIC CONTROL BOXES, METERS, CHECK VALVE VAULTS, COMMUNICATION VAULTS, OR OTHER BURIED OR PARTIALLY BURIED INFRASTRUCTURE AS A VEHICULAR OR PEDESTRIAN SURFACE.
24. ALL WET UTILITIES SHALL BE INSTALLED AND ALL DENSITIES MUST HAVE PASSED INSPECTION(S) PRIOR TO THE INSTALLATION OF DRY UTILITIES.
25. DRY UTILITIES SHALL BE INSTALLED AFTER SUBGRADE IS CUT AND BEFORE THE FIRST COURSE OF NO TRENCHING COMPACTED BASE. IF NECESSARY DRY UTILITIES INSTALLED AFTER FIRST COURSE BASE SHALL BE BORED ACROSS THE FULL WIDTH OF THE PUBLIC RIGHT-OF-WAY.
26. A MINIMUM OF SEVEN (7) DAYS OF CURE TIME IS REQUIRED FOR HMAC PRIOR TO THE INTRODUCTION OF VEHICULAR TRAFFIC TO ALL STREETS.

TRENCH SAFETY NOTES

1. TRENCH SAFETY SYSTEMS TO BE UTILIZED FOR THIS PROJECT ARE DESCRIBED IN ITEM 509S "TRENCH SAFETY SYSTEMS" OF THE CITY OF AUSTIN STANDARD SPECIFICATIONS AND SHALL BE IN ACCORDANCE WITH THE LAWS OF THE STATE OF TEXAS AND THE U.S. OCCUPATION SAFETY AND HEALTH ADMINISTRATION REGULATIONS.

GRADING NOTES

1. POSITIVE DRAINAGE SHALL BE MAINTAINED ON ALL SURFACE AREAS WITHIN THE SCOPE OF THIS PROJECT. CONTRACTOR SHOULD TAKE PRECAUTIONS NOT TO ALLOW ANY PONDING OF AREA.
2. THE CONTRACTOR SHALL CONSTRUCT EARTHEN EMBANKMENTS WITH SLOPES NO STEEPER THAN 3:1 AND COMPACT SOIL TO 95% OF MAXIMUM DENSITY IN ACCORDANCE WITH THE CITY OF AUSTIN STANDARD SPECIFICATIONS.
3. AREAS OF SOIL DISTURBANCE ARE LIMITED TO GRADING AND IMPROVEMENTS SHOWN. ALL OTHER AREAS WILL NOT BE DISTURBED.

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY WATER POLLUTION ABATEMENT PLAN GENERAL CONSTRUCTION NOTES

EDWARDS AQUIFER PROTECTION PROGRAM CONSTRUCTION NOTES - LEGAL DISCLAIMER

THE FOLLOWING LISTED "CONSTRUCTION NOTES" ARE INTENDED TO BE ADVISORY IN NATURE ONLY AND DO NOT CONSTITUTE AN APPROVAL OR CONDITIONAL APPROVAL BY THE EXECUTIVE DIRECTOR (ED). NOR DO THEY CONSTITUTE A COMPREHENSIVE LISTING OF RULES OR CONDITIONS TO BE FOLLOWED DURING CONSTRUCTION. FURTHER ACTIONS MAY BE REQUIRED TO ACHIEVE COMPLIANCE WITH TCEQ REGULATIONS FOUND IN TITLE 30, TEXAS ADMINISTRATIVE CODE (TAC), CHAPTERS 213 AND 217, AS WELL AS LOCAL ORDINANCES AND REGULATIONS PROVIDING FOR THE PROTECTION OF WATER QUALITY. ADDITIONALLY, NOTHING CONTAINED IN THE FOLLOWING LISTED "CONSTRUCTION NOTES" RESTRICTS THE POWERS OF THE ED. THE COMMISSION OR ANY OTHER GOVERNMENTAL ENTITY TO PREVENT, CORRECT, OR CURTAIL ACTIVITIES THAT RESULT OR MAY RESULT IN POLLUTION OF THE EDWARDS AQUIFER OR HYDROLOGICALLY CONNECTED SURFACE WATERS. THE HOLDER OF ANY EDWARDS AQUIFER PROTECTION PLAN CONTAINING "CONSTRUCTION NOTES" IS STILL RESPONSIBLE FOR COMPLIANCE WITH TITLE 30, TAC, CHAPTERS 213 OR ANY OTHER APPLICABLE TCEQ REGULATION, AS WELL AS ALL CONDITIONS OF AN AQUIFER PROTECTION PLAN THROUGH ALL PHASES OF PLAN IMPLEMENTATION. FAILURE TO COMPLY WITH ANY CONDITION OF THE ED'S APPROVAL, WHIF OF BRAND, A VIOLATION OF THE ED'S APPROVAL, OR NOT IN CONTRADICTION OF ANY "CONSTRUCTION NOTES" IS A VIOLATION OF TCEQ REGULATIONS AND ANY VIOLATION IS SUBJECT TO ADMINISTRATIVE RULES, ORDERS, AND PENALTIES AS PROVIDED UNDER TITLE 30, TAC § 213.10 (RELATING TO ENFORCEMENT). SUCH VIOLATIONS MAY ALSO BE SUBJECT TO CIVIL PENALTIES AND INJUNCTION. THE FOLLOWING LISTED "CONSTRUCTION NOTES" IN NO WAY REPRESENT AN APPROVED EXCEPTION BY THE ED TO ANY PART OF TITLE 30 TAC, CHAPTERS 213 AND 217, OR ANY OTHER TCEQ APPLICABLE REGULATION.

1. A WRITTEN NOTICE OF CONSTRUCTION MUST BE SUBMITTED TO THE TCEQ REGIONAL OFFICE AT LEAST 48 HOURS PRIOR TO THE START OF ANY REGULATED ACTIVITIES. THIS NOTICE MUST INCLUDE:
- THE NAME OF THE APPROVED PROJECT;
 - THE ACTIVITY START DATE; AND
 - THE CONTACT INFORMATION OF THE PRIME CONTRACTOR.
2. ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROJECT MUST BE PROVIDED WITH COMPLETE COPIES OF THE APPROVED WATER POLLUTION ABATEMENT PLAN (WPAP) AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS ARE REQUIRED TO KEEP ON-SITE COPIES OF THE APPROVED PLAN AND APPROVAL LETTER.
3. IF ANY SENSITIVE FEATURE(S) (CAVES, SOLUTION CAVITY, SINK HOLE, ETC.) IS DISCOVERED DURING CONSTRUCTION, ALL REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE STOPPED IMMEDIATELY. THE APPROPRIATE REGIONAL OFFICE MUST BE IMMEDIATELY NOTIFIED OF ANY SENSITIVE FEATURES ENCOUNTERED DURING CONSTRUCTION. CONSTRUCTION ACTIVITIES MAY NOT BE RESUMED UNTIL THE TCEQ HAS REVIEWED AND APPROVED THE APPROPRIATE PROTECTIVE MEASURES IN ORDER TO PROTECT ANY SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM POTENTIALLY ADVERSE IMPACTS TO WATER QUALITY.
4. NO TEMPORARY OR PERMANENT HAZARDOUS SUBSTANCE STORAGE TANK SHALL BE INSTALLED WITHIN 150 FEET OF A WATER SUPPLY SOURCE, DISTRIBUTION SYSTEM, WELL, OR SENSITIVE FEATURE.
5. PRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITY, ALL TEMPORARY EROSION AND SEDIMENTATION (E&S) CONTROL MEASURES MUST BE PROPERLY INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE APPROVED PLANS AND MANUFACTURERS SPECIFICATIONS. IF INSPECTIONS INDICATE A CONTROL HAS BEEN USED INAPPROPRIATELY, OR INCORRECTLY, THE APPLICANT MUST REPLACE OR MODIFY THE CONTROL FOR SITE SITUATIONS. THESE CONTROLS MUST REMAIN IN PLACE UNTIL THE DISTURBED AREAS HAVE BEEN PERMANENTLY STABILIZED.
6. ANY SEDIMENT THAT ESCAPES THE CONSTRUCTION SITE MUST BE COLLECTED AND PROPERLY DISPOSED OF BEFORE THE NEXT RAIN EVENT TO ENSURE IT IS NOT WASHED INTO SURFACE STREAMS, SENSITIVE FEATURES, ETC.
7. SEDIMENT MUST BE REMOVED FROM THE SEDIMENT TRAPS OR SEDIMENTATION BASINS NOT LATER THAN WHEN IT OCCUPIES 50% OF THE BASIN'S DESIGN CAPACITY.
8. LITTER, CONSTRUCTION DEBRIS, AND CONSTRUCTION CHEMICALS EXPOSED TO STORMWATER SHALL BE PREVENTED FROM BEING DISCHARGED OFFSITE.
9. ALL SPOILS (EXCAVATED MATERIAL) GENERATED FROM THE PROJECT SITE MUST BE STORED ON-SITE WITH PROPER E&S CONTROLS. FOR STORAGE OR DISPOSAL OF SPOILS AT ANOTHER SITE ON THE EDWARDS AQUIFER RECHARGE ZONE, THE OWNER OF THE SITE MUST RECEIVE APPROVAL OF A WATER POLLUTION ABATEMENT PLAN FOR THE PLACEMENT OF FILL MATERIAL OR MASS GRADING PRIOR TO THE PLACEMENT OF SPOILS AT THE OTHER SITE.
10. IF PORTIONS OF THE SITE WILL HAVE A TEMPORARY OR PERMANENT CEASE IN CONSTRUCTION ACTIVITY LASTING LONGER THAN 14 DAYS, SOIL STABILIZATION IN THOSE AREAS SHALL BE INITIATED AS SOON AS POSSIBLE PRIOR TO THE 14TH DAY OF INACTIVITY. IF ACTIVITY WILL RESUME PRIOR TO THE 21ST DAY, STABILIZATION MEASURES ARE NOT REQUIRED. IF DROUGHT CONDITIONS OR INCLEMENT WEATHER PREVENT ACTION BY THE 14TH DAY, STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS POSSIBLE.
11. THE FOLLOWING RECORDS SHALL BE MAINTAINED AND MADE AVAILABLE TO THE TCEQ UPON REQUEST:
- THE DATES WHEN MAJOR GRADING ACTIVITIES OCCUR;
 - THE DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE; AND
 - THE DATES WHEN STABILIZATION MEASURES ARE INITIATED.
12. THE HOLDER OF ANY APPROVED EDWARD AQUIFER PROTECTION PLAN MUST NOTIFY THE APPROPRIATE REGIONAL OFFICE IN WRITING AND OBTAIN APPROVAL FROM THE EXECUTIVE DIRECTOR PRIOR TO INITIATING ANY OF THE FOLLOWING:

- A. ANY PHYSICAL OR OPERATIONAL MODIFICATION OF ANY WATER POLLUTION ABATEMENT STRUCTURE(S), INCLUDING BUT NOT LIMITED TO PONDS, DAMS, BERMS, SEWAGE TREATMENT PLANTS, AND DIVERSIONARY STRUCTURES;
- B. ANY CHANGE IN THE NATURE OR CHARACTER OF THE REGULATED ACTIVITY FROM THAT WHICH WAS ORIGINALLY APPROVED OR A CHANGE WHICH WOULD SIGNIFICANTLY IMPACT THE ABILITY OF THE PLAN TO PREVENT POLLUTION OF THE EDWARDS AQUIFER;
- C. ANY DEVELOPMENT OF LAND PREVIOUSLY IDENTIFIED AS UNDEVELOPED IN THE ORIGINAL WATER POLLUTION ABATEMENT PLAN.

CONTACT:
AUSTIN REGIONAL OFFICE 12100 PARK 35 CIRCLE, BUILDING A
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\$217.54. CRITERIA FOR LAYING PIPE.

- (A) PIPE EMBEDMENT.
- (1) A RIGID PIPE MUST BE LAID WITH BEDDING, HAUNCHING, AND INITIAL BACKFILL THAT WILL MEET THE APPROPRIATE STANDARDS AND WILL SUPPORT THE ANTICIPATED LOAD. THE BEDDING CLASSES THAT ARE ALLOWED ARE A, B, OR C, AS DESCRIBED IN AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) C 12, AMERICAN NATIONAL STANDARDS INSTITUTE (ANSI) A 106.2, WATER ENVIRONMENT FEDERATION MANUAL OF PRACTICE NO. 9, OR AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE) MOP 37.
- (2) A FLEXIBLE PIPE MUST BE LAID WITH BEDDING, HAUNCHING, AND INITIAL BACKFILL THAT WILL MEET THE APPROPRIATE STANDARDS AND WILL SUPPORT THE ANTICIPATED LOAD. THE BEDDING CLASSES THAT ARE ALLOWED ARE A, B, OR C, AS DESCRIBED IN ASTM D-2221 OR ANSI/K65.171 DEBRIS, LARGE CLOUDS, STONES THAT ARE GREATER THAN SIX INCHES IN DIAMETER, ORGANIC MATTER, AND OTHER UNDESIRABLE MATERIALS ARE PROHIBITED AS BEDDING, HAUNCHING, OR INITIAL BACKFILL.
- (3) BACKFILL MUST NOT DISTURB THE ALIGNMENT OF A COLLECTION SYSTEM PIPE.
- (4) IF A FRACTURE, FAULT ZONE, CAVE, OR SOLUTIONAL MODIFICATION TO THE ROCK STRATA THAT WOULD PREVENT PIPE BEDDING IS ENCOUNTERED DURING CONSTRUCTION, OWNER MUST HALT CONSTRUCTION UNTIL AN ENGINEER PREPARES A WRITTEN REPORT, WHICH MUST BE SUBMITTED WITH THE FINAL ENGINEERING REPORT, DETAILING HOW CONSTRUCTION WILL ACCOMMODATE THESE SITE CONDITIONS.
- (B) COMPACTION.
- (1) COMPACTION OF THE PIPE EMBEDMENT MUST MEET THE MANUFACTURER'S RECOMMENDATIONS FOR THE COLLECTION SYSTEM PIPES USED IN A PROJECT. COMPACTION OF THE PIPE EMBEDMENT MUST PROVIDE THE MODULUS OF SOIL REACTION FOR THE BEDDING MATERIAL NECESSARY TO ENSURE A COLLECTION SYSTEM PIPE'S STRUCTURAL INTEGRITY AS REQUIRED BY §217.53 OF THIS TITLE (RELATING TO PIPE DESIGN). THE PLACEMENT OF THE BACKFILL ABOVE A PIPE MUST NOT AFFECT THE STRUCTURAL INTEGRITY OF A PIPE.
- (C) EMBEDMENT THICKNESS.
- (1) A MINIMUM CLEARANCE OF 6.0 INCHES BELOW AND ON EACH SIDE OF THE BELL OF ALL PIPES TO THE TRENCH WALLS AND FLOOR IS REQUIRED.
- (2) THE EMBEDMENT MATERIAL USED FOR HAUNCHING AND INITIAL BACKFILL MUST BE INSTALLED TO A MINIMUM DEPTH OF 12 INCHES ABOVE THE CROWN OF A PIPE.
- (D) TRENCH WIDTH.
- (1) THE WIDTH OF A TRENCH MUST ALLOW A PIPE TO BE LAID AND JOINTED PROPERLY AND MUST ALLOW THE BACKFILL TO BE PLACED AND COMPACTED AS NEEDED.
- (2) THE MAXIMUM AND MINIMUM TRENCH WIDTH NEEDED FOR SAFETY AND A PIPE'S STRUCTURAL INTEGRITY MUST BE INCLUDED IN THE ENGINEERING REPORT.
- (3) THE WIDTH OF A TRENCH MUST ALLOW PROPER AND SAFE PLACEMENT AND COMPACTION OF HAUNCHING MATERIALS IN ACCORDANCE WITH THE STANDARDS IN SUBSECTION A OF THIS SECTION.
- (4) THE SPACE BETWEEN A PIPE AND A TRENCH WALL MUST BE WIDER THAN THE COMPACTION EQUIPMENT USED IN THE PIPE ZONE.



THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTORS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.



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REVISIONS	DESCRIPTION	DATE			

GENERAL NOTES	O'REILLY AUTO PARTS 5510 WILLIAMS DR. GEORGETOWN, TEXAS 78633



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JOB No.	250519-01-002	
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TEXAS COMMISSION ON ENVIRONMENTAL QUALITY
ORGANIZED SEWAGE COLLECTION SYSTEM
GENERAL CONSTRUCTION NOTES

EDWARDS AQUIFER PROTECTION PROGRAM CONSTRUCTION NOTES – LEGAL
DISCLAIMER

THE FOLLOWING/LISTED "CONSTRUCTION NOTES" ARE INTENDED TO BE ADVISORY IN NATURE ONLY AND DO NOT CONSTITUTE AN APPROVAL OR CONDITIONAL APPROVAL BY THE EXECUTIVE DIRECTOR, NOR DO THEY CONSTITUTE A COMPREHENSIVE LISTING OF RULES OR CONDITIONS TO BE FOLLOWED DURING CONSTRUCTION. FURTHER ACTIONS MAY BE REQUIRED TO ACHIEVE COMPLIANCE WITH TCEQ REGULATIONS FOUND IN TITLE 30, TEXAS ADMINISTRATIVE CODE, CHAPTERS 213 AND 217, AS WELL AS LOCAL ORDINANCES AND REGULATIONS PROVIDING FOR THE PROTECTION OF WATER QUALITY. ADDITIONALLY, NOTHING CONTAINED IN THE FOLLOWING/LISTED "CONSTRUCTION NOTES" RESTRICTS THE POWERS OF THE EXECUTIVE DIRECTOR, THE COMMISSION OR ANY OTHER GOVERNMENTAL ENTITY TO PREVENT, CORRECT, OR CURTAIL ACTIVITIES THAT RESULT OR MAY RESULT IN POLLUTION OF THE EDWARDS AQUIFER OR HYDROLOGICALLY CONNECTED SURFACE WATERS. THE HOLDER OF ANY EDWARDS AQUIFER PROTECTION PLAN CONTAINING "CONSTRUCTION NOTES" IS STILL RESPONSIBLE FOR COMPLIANCE WITH TITLE 30, TEXAS ADMINISTRATIVE CODE, CHAPTERS 213 OR ANY OTHER APPLICABLE TCEQ REGULATION, AS WELL AS ALL CONDITIONS OF AN EDWARDS AQUIFER PROTECTION PLAN THROUGH ALL PHASES OF PLAN IMPLEMENTATION. FAILURE TO COMPLY WITH ANY CONDITION OF THE EXECUTIVE DIRECTOR'S APPROVAL, WHETHER OR NOT IN CONTRADICTION OF ANY "CONSTRUCTION NOTES," IS A VIOLATION OF TCEQ REGULATIONS AND ANY VIOLATION IS SUBJECT TO ADMINISTRATIVE RULES, ORDERS, AND PENALTIES AS PROVIDED UNDER TITLE 30, TEXAS ADMINISTRATIVE CODE § 213.10 (RELATING TO ENFORCEMENT). SUCH VIOLATIONS MAY ALSO BE SUBJECT TO CIVIL PENALTIES AND INJUNCTION. THE FOLLOWING/LISTED "CONSTRUCTION NOTES" IN NO WAY REPRESENT AN APPROVED EXCEPTION BY THE EXECUTIVE DIRECTOR TO ANY PART OF TITLE 30 TEXAS ADMINISTRATIVE CODE, CHAPTERS 213 AND 217, OR ANY OTHER TCEQ APPLICABLE REGULATION.

1. THIS ORGANIZED SEWAGE COLLECTION SYSTEM (SCS) MUST BE CONSTRUCTED IN ACCORDANCE WITH 30 TEXAS ADMINISTRATIVE CODE (TAC) §213.5(C), THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY'S (TCEQ) EDWARDS AQUIFER RULES AND ANY LOCAL GOVERNMENT STANDARD SPECIFICATIONS.

2. ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROPOSED REGULATED PROJECT MUST BE PROVIDED WITH COPIES OF THE SCS PLAN AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS MUST BE REQUIRED TO KEEP ON-SITE COPIES OF THE PLAN AND THE APPROVAL LETTER.

3. A WRITTEN NOTICE OF CONSTRUCTION MUST BE SUBMITTED TO THE PRESIDING TCEQ REGIONAL OFFICE AT LEAST 48 HOURS PRIOR TO THE START OF ANY REGULATED ACTIVITIES. THIS NOTICE MUST INCLUDE:

- THE NAME OF THE APPROVED PROJECT;
- THE ACTIVITY START DATE; AND
- THE CONTACT INFORMATION OF THE PRIME CONTRACTOR.

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4. ANY MODIFICATION TO THE ACTIVITIES DESCRIBED IN THE REFERENCED SCS APPLICATION FOLLOWING THE DATE OF APPROVAL MAY REQUIRE THE SUBMITTAL OF AN SCS APPLICATION TO MODIFY THIS APPROVAL, INCLUDING THE PAYMENT OF APPROPRIATE FEES AND ALL INFORMATION NECESSARY FOR ITS REVIEW AND APPROVAL.

5. PRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITY, ALL TEMPORARY EROSION AND SEDIMENTATION (E&S) CONTROL MEASURES MUST BE PROPERLY INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS. THESE CONTROLS MUST REMAIN IN PLACE UNTIL THE DISTURBED AREAS HAVE BEEN PERMANENTLY STABILIZED.

6. IF ANY SENSITIVE FEATURES ARE DISCOVERED DURING THE WASTEWATER LINE TRENCHING ACTIVITIES, ALL REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE SUSPENDED IMMEDIATELY. THE APPLICANT MUST IMMEDIATELY NOTIFY THE APPROPRIATE REGIONAL OFFICE OF THE TCEQ OF THE FEATURE DISCOVERED. A GEOLOGIST'S ASSESSMENT OF THE LOCATION AND EXTENT OF THE FEATURE DISCOVERED MUST BE REPORTED TO THAT REGIONAL OFFICE IN WRITING AND THE APPLICANT MUST SUBMIT A PLAN FOR ENSURING THE STRUCTURAL INTEGRITY OF THE SEWER LINE OR FOR MODIFYING THE PROPOSED COLLECTION SYSTEM ALIGNMENT AROUND THE FEATURE. THE REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MAY NOT PROCEED UNTIL THE EXECUTIVE DIRECTOR HAS REVIEWED AND APPROVED THE METHODS PROPOSED TO PROTECT THE SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM ANY POTENTIALLY ADVERSE IMPACTS TO WATER QUALITY WHILE MAINTAINING THE STRUCTURAL INTEGRITY OF THE LINE.

7. SEWER LINES LOCATED WITHIN OR CROSSING THE 5-YEAR FLOODPLAIN OF A DRAINAGE WAY WILL BE PROTECTED FROM INUNDATION AND STREAM VELOCITIES WHICH COULD CAUSE EROSION AND SCOURING OF BACKFILL. THE TRENCH MUST BE CAPPED WITH CONCRETE TO PREVENT SCOURING OF BACKFILL, OR THE SEWER LINES MUST BE ENCASED IN CONCRETE. ALL CONCRETE SHALL HAVE A MINIMUM THICKNESS OF 6 INCHES.

8. BLASTING PROCEDURES FOR PROTECTION OF EXISTING SEWER LINES AND OTHER UTILITIES WILL BE IN ACCORDANCE WITH THE NATIONAL FIRE PROTECTION ASSOCIATION CRITERIA. SAND IS NOT ALLOWED AS BEDDING OR BACKFILL IN TRENCHES THAT HAVE BEEN BLASTED. IF ANY EXISTING SEWER LINES ARE DAMAGED, THE LINES MUST BE REPAIRED AND RETESTED.

9. ALL MANHOLES CONSTRUCTED OR REHABILITATED ON THIS PROJECT MUST HAVE WATERTIGHT SIZE ON SIZE RESILIENT CONNECTORS ALLOWING FOR DIFFERENTIAL SETTLEMENT. IF MANHOLES ARE CONSTRUCTED WITHIN THE 100-YEAR FLOODPLAIN, THE COVER MUST HAVE A GASKET AND BE BOLTED TO THE RING. WHERE GASKETED MANHOLE COVERS ARE REQUIRED FOR MORE THAN THREE MANHOLES IN SEQUENCE OR FOR MORE THAN 1500 FEET, ALTERNATE MEANS OF VENTING WILL BE PROVIDED. BRICKS ARE NOT AN ACCEPTABLE CONSTRUCTION MATERIAL FOR ANY PORTION OF THE MANHOLE.

THE DIAMETER OF THE MANHOLES MUST BE A MINIMUM OF FOUR FEET AND THE MANHOLE FOR ENTRY MUST HAVE A MINIMUM CLEAR OPENING DIAMETER OF 30 INCHES. THESE DIMENSIONS AND OTHER DETAILS SHOWING COMPLIANCE WITH THE COMMISSION'S RULES CONCERNING MANHOLES AND SEWER LINE/MANHOLE INVERTS DESCRIBED IN 30 TAC §217.55 ARE INCLUDED ON PLAN SHEET ____ OF ____.

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IT IS SUGGESTED THAT ENTRANCE INTO MANHOLES IN EXCESS OF FOUR FEET DEEP BE ACCOMPLISHED BY MEANS OF A PORTABLE LADDER. THE INCLUSION OF STEPS IN A MANHOLE IS PROHIBITED.

10. WHERE WATER LINES AND NEW SEWER LINE ARE INSTALLED WITH A SEPARATION DISTANCE CLOSER THAN NINE FEET (I.E., WATER LINES CROSSING WASTEWATER LINES, WATER LINES PARALLELING WASTEWATER LINES, OR WATER LINES NEXT TO MANHOLES) THE INSTALLATION MUST MEET THE REQUIREMENTS OF 30 TAC §217.53(D) (PIPE DESIGN) AND 30 TAC §290.44(E) (WATER DISTRIBUTION).

11. WHERE SEWERS LINES DEVIATE FROM STRAIGHT ALIGNMENT AND UNIFORM GRADE ALL CURVATURE OF SEWER PIPE MUST BE ACHIEVED BY THE FOLLOWING PROCEDURE WHICH IS RECOMMENDED BY THE PIPE MANUFACTURER: _____

IF PIPE FLEXURE IS PROPOSED, THE FOLLOWING METHOD OF PREVENTING DEFLECTION OF THE JOINT MUST BE USED: _____

SPECIFIC CARE MUST BE TAKEN TO ENSURE THAT THE JOINT IS PLACED IN THE CENTER OF THE TRENCH AND PROPERLY BEDDED IN ACCORDANCE WITH 30 TAC §217.54.

12. NEW SEWAGE COLLECTION SYSTEM LINES MUST BE CONSTRUCTED WITH STUB OUTS FOR THE CONNECTION OF ANTICIPATED EXTENSIONS. THE LOCATION OF SUCH STUB OUTS MUST BE MARKED ON THE GROUND SUCH THAT THEIR LOCATION CAN BE EASILY DETERMINED AT THE TIME OF CONNECTION OF THE EXTENSIONS. SUCH STUB OUTS MUST BE MANUFACTURED WYES OR TEES THAT ARE COMPATIBLE IN SIZE AND MATERIAL WITH BOTH THE SEWER LINE AND THE EXTENSION. AT THE TIME OF ORIGINAL CONSTRUCTION, NEW STUB-OUTS MUST BE CONSTRUCTED SUFFICIENTLY TO EXTEND BEYOND THE END OF THE STREET PAVEMENT. ALL STUB-OUTS MUST BE SEALED WITH A MANUFACTURED CAP TO PREVENT LEAKAGE. EXTENSIONS THAT WERE NOT ANTICIPATED AT THE TIME OF ORIGINAL CONSTRUCTION OR THAT ARE TO BE CONNECTED TO AN EXISTING SEWER LINE NOT FURNISHED WITH STUB OUTS MUST BE CONNECTED USING A MANUFACTURED SADDLE AND IN ACCORDANCE WITH ACCEPTED PLUMBING TECHNIQUES.

IF NO STUB-OUT IS PRESENT AN ALTERNATE METHOD OF JOINING LATERALS IS SHOWN IN THE DETAIL ON PLAN SHEET ____ OF ____ (FOR POTENTIAL FUTURE LATERALS).

THE PRIVATE SERVICE LATERAL STUB-OUTS MUST BE INSTALLED AS SHOWN ON THE PLAN AND PROFILE SHEETS ON PLAN SHEET ____ OF ____ AND MARKED AFTER BACKFILLING AS SHOWN IN THE DETAIL ON PLAN SHEET ____ OF ____.

13. TRENCHING, BEDDING AND BACKFILL MUST CONFORM WITH 30 TAC §217.54. THE BEDDING AND BACKFILL FOR FLEXIBLE PIPE MUST COMPLY WITH THE STANDARDS OF ASTM D-2321, CLASSES IA, IB, II, OR III. RIGID PIPE BEDDING MUST COMPLY WITH THE REQUIREMENTS OF ASTM C 12 (ANSI A 106.2) CLASSES A, B OR C.

14. SEWER LINES MUST BE TESTED FROM MANHOLE TO MANHOLE. WHEN A NEW

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SEWER LINE IS CONNECTED TO AN EXISTING STUB OR CLEAN-OUT, IT MUST BE TESTED FROM EXISTING MANHOLE TO NEW MANHOLE. IF A STUB OR CLEAN-OUT IS USED AT THE END OF THE PROPOSED SEWER LINE, NO PRIVATE SERVICE ATTACHMENTS MAY BE CONNECTED BETWEEN THE LAST MANHOLE AND THE CLEANOUT UNLESS IT CAN BE CERTIFIED AS CONFORMING WITH THE PROVISIONS OF 30 TAC §213.5(C)(3)(E).

15. ALL SEWER LINES MUST BE TESTED IN ACCORDANCE WITH 30 TAC §217.57. THE ENGINEER MUST RETAIN COPIES OF ALL TEST RESULTS WHICH MUST BE MADE AVAILABLE TO THE EXECUTIVE DIRECTOR UPON REQUEST. THE ENGINEER MUST CERTIFY IN WRITING THAT ALL WASTEWATER LINES HAVE PASSED ALL REQUIRED TESTING TO THE APPROPRIATE REGIONAL OFFICE WITHIN 30 DAYS OF TEST COMPLETION AND PRIOR TO USE OF THE NEW COLLECTION SYSTEM. TESTING METHOD WILL BE:

- (a) FOR A COLLECTION SYSTEM PIPE THAT WILL TRANSPORT WASTEWATER BY GRAVITY FLOW, THE DESIGN MUST SPECIFY AN INFILTRATION AND EXFILTRATION TEST OR A LOW-PRESSURE AIR TEST. A TEST MUST CONFORM TO THE FOLLOWING REQUIREMENTS:
 - (1) **LOW PRESSURE AIR TEST.**
 - (A) A LOW PRESSURE AIR TEST MUST FOLLOW THE PROCEDURES DESCRIBED IN AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) C-828, ASTM C-924, OR ASTM F-1417 OR OTHER PROCEDURE APPROVED BY THE EXECUTIVE DIRECTOR, EXCEPT AS TO TESTING TIMES AS REQUIRED IN TABLE C.3 IN SUBPARAGRAPH (C) OF THIS PARAGRAPH OR EQUATION C.3 IN SUBPARAGRAPH (B)(II) OF THIS PARAGRAPH.
 - (B) FOR SECTIONS OF COLLECTION SYSTEM PIPE LESS THAN 36 INCH AVERAGE INSIDE DIAMETER, THE FOLLOWING PROCEDURE MUST APPLY, UNLESS A PIPE IS TO BE TESTED AS REQUIRED BY PARAGRAPH (2) OF THIS SUBSECTION.
 - (I) A PIPE MUST BE PRESSURIZED TO 3.5 POUNDS PER SQUARE INCH (PSI) GREATER THAN THE PRESSURE EXERTED BY GROUNDWATER ABOVE THE PIPE.
 - (II) ONCE THE PRESSURE IS STABILIZED, THE MINIMUM TIME THE PRESSURE TO DROP FROM 3.5 PSI GAUGE TO 2.5 PSI GAUGE IS COMPUTED FROM THE FOLLOWING EQUATION:
$$T = \frac{0.085 \times D \times K}{Q}$$
WHERE:
 - T = TIME FOR PRESSURE TO DROP 1.0 POUND PER SQUARE INCH GAUGE IN SECONDS
 - K = 0.000419 X D X L, BUT NOT LESS THAN 1.0
 - D = AVERAGE INSIDE PIPE DIAMETER IN INCHES
 - L = LENGTH OF LINE OF SAME SIZE BEING TESTED, IN FEET
 - Q = RATE OF LOSS, 0.0015 CUBIC FEET PER MINUTE PER SURFACE

SQUARE FOOT INTERNAL

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(C) SINCE A K VALUE OF LESS THAN 1.0 MAY NOT BE USED, THE MINIMUM TESTING TIME FOR EACH PIPE DIAMETER IS SHOWN IN THE FOLLOWING TABLE C.3:

PIPE DIAMETER (INCHES)	MINIMUM TIME (SECONDS)	MAXIMUM LENGTH FOR MINIMUM TIME (FEET)	TIME FOR LONGER LENGTH (SECONDS/FO)
6	340	398	0.855
8	454	298	1.520
10	567	239	2.374
12	680	199	3.419
15	850	159	5.342
18	1020	133	7.693
21	1190	114	10.471
24	1360	100	13.676
27	1530	88	17.309
30	1700	80	21.369
33	1870	72	25.856

(D) AN OWNER MAY STOP A TEST IF NO PRESSURE LOSS HAS OCCURRED DURING THE FIRST 25% OF THE CALCULATED TESTING TIME.

(E) IF ANY PRESSURE LOSS OR LEAKAGE HAS OCCURRED DURING THE FIRST 25% OF A TESTING PERIOD, THEN THE TEST MUST CONTINUE FOR THE ENTIRE TEST DURATION AS OUTLINED ABOVE OR UNTIL FAILURE.

(F) WASTEWATER COLLECTION SYSTEM PIPES WITH A 27 INCH OR LARGER AVERAGE INSIDE DIAMETER MAY BE AIR TESTED AT EACH JOINT INSTEAD OF FOLLOWING THE PROCEDURE OUTLINED IN THIS SECTION.

(G) A TESTING PROCEDURE FOR PIPE WITH AN INSIDE DIAMETER GREATER THAN 33 INCHES MUST BE APPROVED BY THE EXECUTIVE DIRECTOR.

(2) **INFILTRATION/EXFILTRATION TEST.**

- (A) THE TOTAL EXFILTRATION, AS DETERMINED BY A HYDROSTATIC HEAD TEST, MUST NOT EXCEED 50 GALLONS PER INCH OF DIAMETER PER MILE OF PIPE PER 24 HOURS AT A MINIMUM TEST HEAD OF 2.0 FEET ABOVE THE CROWN OF A PIPE AT AN UPSTREAM MANHOLE.
- (B) AN OWNER SHALL USE AN INFILTRATION TEST IN LIEU OF AN EXFILTRATION TEST WHEN PIPES ARE INSTALLED BELOW THE GROUNDWATER LEVEL.
- (C) THE TOTAL EXFILTRATION, AS DETERMINED BY A HYDROSTATIC HEAD TEST, MUST NOT EXCEED 50 GALLONS PER INCH DIAMETER PER MILE OF PIPE PER 24 HOURS AT A MINIMUM TEST HEAD OF TWO FEET ABOVE THE CROWN OF A PIPE AT AN UPSTREAM MANHOLE.

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OR AT LEAST TWO FEET ABOVE EXISTING GROUNDWATER LEVEL, WHICHEVER IS GREATER.

(D) FORTION CONSTRUCTION WITHIN A 25-YEAR FLOOD PLAIN, THE INFILTRATION OR EXFILTRATION MUST NOT EXCEED 10 GALLONS PER INCH DIAMETER PER MILE OF PIPE PER 24 HOURS AT THE SAME MINIMUM TEST HEAD AS IN SUBPARAGRAPH (C) OF THIS PARAGRAPH.

(E) IF THE QUANTITY OF INFILTRATION OR EXFILTRATION EXCEEDS SPECIFIED, AN OWNER SHALL UNDERTAKE REMEDIAL ACTION IN ORDER TO REDUCE THE INFILTRATION OR EXFILTRATION TO AN AMOUNT WITHIN THE LIMITS SPECIFIED. AN OWNER SHALL RETEST A PIPE FOLLOWING A REMEDIATION ACTION.

(B) IF A GRAVITY COLLECTION PIPE IS COMPOSED OF FLEXIBLE PIPE, DEFLECTION TESTING IS ALSO REQUIRED. THE FOLLOWING PROCEDURES MUST BE FOLLOWED:

- (1) FOR A COLLECTION PIPE WITH INSIDE DIAMETER LESS THAN 27 INCHES, DEFLECTION MEASUREMENT REQUIRES A RIGID MANDREL.
- (A) **MANDREL SIZING.**
 - (I) A RIGID MANDREL MUST HAVE AN OUTSIDE DIAMETER GREATER THAN 95% OF THE BASE INSIDE DIAMETER (OD) OR AVERAGE ID OF A PIPE, AS SPECIFIED IN THE APPROPRIATE STANDARD BY THE ASTM'S, WATER WORKS ASSOCIATION, UNI-BELL, OR AMERICAN NATIONAL STANDARDS INSTITUTE, OR ANY RELATED APPENDIX.
 - (II) IF A MANDREL SIZING DIAMETER IS NOT SPECIFIED IN THE STANDARD, THE MANDREL MUST HAVE AN OD EQUAL TO 95% OF THE ID OF A PIPE. IN THIS CASE, THE ID OF THE PIPE, FOR THE PURPOSE OF DETERMINING THE OD OF THE MANDREL, MUST BE THE AVERAGE DIAMETER MINUS TWO MINIMUM WALL THICKNESSES FOR OD CONTROLLED PIPE AND THE AVERAGE INSIDE DIAMETER FOR ID CONTROLLED PIPE.
 - (III) ALL DIMENSIONS MUST MEET THE APPROPRIATE STANDARD.
- (B) **MANDREL DESIGN.**
 - (I) A RIGID MANDREL MUST BE CONSTRUCTED OF A METAL MATERIAL THAT CAN WITHSTAND 200 PSI WITHOUT BEING DEFORMED.
 - (II) A MANDREL MUST HAVE NINE OR MORE ODD NUMBER OF LEGS.
 - (III) A BARREL SECTION LENGTH MUST EQUAL AT LEAST 75% DIAMETER OF A PIPE.
 - (IV) EACH SIZE MANDREL MUST USE A SEPARATE PROVING RING.
- (C) **METHOD OPTIONS.**
 - (I) AN ADJUSTABLE OR FLEXIBLE MANDREL IS PROHIBITED.
 - (II) A TEST MAY NOT USE TELEVISION INSPECTION AS A SUBSTITUTE FOR A DEFLECTION TEST.

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(II) IF REQUESTED, THE EXECUTIVE DIRECTOR MAY APPROVE THE USE OF A DEFLECTOMETER OR A MANDREL WITH REMOVABLE LEGS OR RUNNERS ON A CASE-BY-CASE BASIS.

(2) FOR A GRAVITY COLLECTION SYSTEM PIPE WITH AN INSIDE DIAMETER 27 INCHES AND GREATER, OTHER TEST METHODS MAY BE USED TO DETERMINE VERTICAL DEFLECTION.

(3) A DEFLECTION TEST METHOD MUST BE ACCURATE TO WITHIN PLUS OR MINUS 0.2% DEFLECTION.

(4) AN OWNER SHALL NOT CONDUCT A DEFLECTION TEST UNTIL AT LEAST 30 DAYS AFTER THE FINAL BACKFILL.

(5) GRAVITY COLLECTION SYSTEM PIPE DEFLECTION MUST NOT EXCEED FIVE PERCENT (5%).

(6) IF A PIPE SECTION FAILS A DEFLECTION TEST, AN OWNER SHALL CORRECT THE PROBLEM AND CONDUCT A SECOND TEST AFTER THE FINAL BACKFILL HAS BEEN IN PLACE AT LEAST 30 DAYS.

16. ALL MANHOLES MUST BE TESTED TO MEET OR EXCEED THE REQUIREMENTS OF 30 TAC §217.58.

- (A) ALL MANHOLES MUST PASS A LEAKAGE TEST.
- (B) AN OWNER SHALL TEST EACH MANHOLE (AFTER ASSEMBLY AND BACKFILLING) FOR LEAKAGE, SEPARATE AND INDEPENDENT OF THE COLLECTION SYSTEM PIPES, BY HYDROSTATIC EXFILTRATION TESTING, VACUUM TESTING, OR OTHER METHOD APPROVED BY THE EXECUTIVE DIRECTOR.
- (1) **HYDROSTATIC TESTING.**
 - (A) THE MAXIMUM LEAKAGE FOR HYDROSTATIC TESTING OR ANY METHODS IS 0.025 GALLONS PER FOOT DIAMETER PER FOOT OF MANHOLE DEPTH PER HOUR.
 - (B) TO PERFORM A HYDROSTATIC EXFILTRATION TEST, AN OWNER SHALL SEAL ALL WASTEWATER PIPES COMING INTO A MANHOLE WITH AN INTERNAL PIPE PLUG, FILE THE JOINTS, AND SEAL THE MANHOLE WITH WATER, AND MAINTAIN THE TEST FOR AT LEAST ONE HOUR.
 - (C) A TEST FOR CONCRETE MANHOLES MAY USE A 24-HOUR WETTING PERIOD BEFORE TESTING TO ALLOW SATURATION OF THE CONCRETE.
 - (2) **VACUUM TESTING.**
 - (A) TO PERFORM A VACUUM TEST, AN OWNER SHALL PLUG ALL LIFT HOLES AND EXTERIOR JOINTS WITH A NON-SHRINK GROUT AND PLUG ALL PIPES ENTERING A MANHOLE.
 - (B) NO GROUT MUST BE PLACED IN HORIZONTAL JOINTS BEFORE TESTING.
 - (C) STUB-OUTS, MANHOLE BOOTS, AND PIPE PLUGS MUST BE SECURED TO PREVENT MOVEMENT WHILE A VACUUM IS DRAWN.
 - (D) AN OWNER SHALL USE A MINIMUM 60 INCH/LB TORQUE WRENCH TO TIGHTEN THE EXTERNAL CLAMPS THAT SECURE A TEST COVER TO THE TOP OF A MANHOLE.
 - (E) A TEST HEAD MUST BE PLACED AT THE INSIDE OF THE TOP OF A CONE SECTION, AND THE SEAL INFLATED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
 - (F) THERE MUST BE A VACUUM OF 10 INCHES OF MERCURY INSIDE A MANHOLE TO PERFORM A VALID TEST.

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OFF.

(G) A TEST DOES NOT BEGIN UNTIL AFTER THE VACUUM PUMP IS

ALL VALVES CLOSED, THE VACUUM IS AT LEAST 9.0 INCHES OF MERCURY.

(H) A MANHOLE PASSES THE TEST IF AFTER 2.0 MINUTES AND WITH

17. ALL PRIVATE SERVICE LATERALS MUST BE INSPECTED AND CERTIFIED IN ACCORDANCE WITH 30 TAC §213.5(C)(3)(I). AFTER INSTALLATION OF AND, PRIOR TO COVERING AND CONNECTING A PRIVATE SERVICE LATERAL TO AN EXISTING ORGANIZED SEWAGE COLLECTION SYSTEM, A TEXAS LICENSED PROFESSIONAL ENGINEER, TEXAS REGISTERED SANITARIAN, OR APPROPRIATE CITY INSPECTOR MUST VISUALLY INSPECT THE PRIVATE SERVICE LATERAL AND THE CONNECTION TO THE SEWAGE COLLECTION SYSTEM, AND CERTIFY THAT IT IS CONSTRUCTED IN CONFORMITY WITH THE APPLICABLE PROVISIONS OF THIS SECTION. THE OWNER OF THE COLLECTION SYSTEM MUST MAINTAIN SUCH CERTIFICATIONS FOR FIVE YEARS AND FORWARD COPIES TO THE APPROPRIATE REGIONAL OFFICE UPON REQUEST. CONNECTIONS MAY ONLY BE MADE TO AN APPROVED SEWAGE COLLECTION SYSTEM.

AUSTIN REGIONAL OFFICE
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AUSTIN, TEXAS 78753-1808
PHONE (512) 339-2929
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SAN ANTONIO, TEXAS 78233-4480
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\$217.54. **CRITERIA FOR LAYING PIPE.**

(A) PIPE EMBEDMENT.

(1) A RIGID PIPE MUST BE LAID WITH BEDDING, HAUNCHING, AND INITIAL BACKFILL THAT WILL MEET THE APPROPRIATE STANDARDS AND WILL SUPPORT THE ANTICIPATED LOAD. THE BEDDING CLASSES THAT ARE ALLOWED ARE A, B, OR C, AS DESCRIBED IN AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) C 12, AMERICAN NATIONAL STANDARDS INSTITUTE (ANSI) A 106.2, WATER ENVIRONMENT FEDERATION MANUAL OF PRACTICE NO. 9, OR AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE) MOP 37.

(2) A FLEXIBLE PIPE MUST BE LAID WITH BEDDING, HAUNCHING, AND INITIAL BACKFILL THAT WILL MEET THE APPROPRIATE STANDARDS AND WILL SUPPORT THE ANTICIPATED LOAD. THE BEDDING CLASSES THAT ARE ALLOWED ARE IA, IB, II, OR III, AS DESCRIBED IN ASTM D-2321 OR ANSI K65.1710EBRIS, LARGE CLODS, STONES THAT ARE GREATER THAN SIX INCHES IN DIAMETER, ORGANIC MATTER, AND OTHER UNSTABLE MATERIALS ARE PROHIBITED AS BEDDING, HAUNCHING, OR INITIAL BACKFILL.

(3) BACKFILL MUST NOT DISTURB THE ALIGNMENT OF A COLLECTION SYSTEM PIPE.

(4) IF A FRACTURE, FAULT ZONE, CAVE, OR SOLUTIONAL MODIFICATION TO THE ROCK STRATA THAT WOULD PREVENT PIPE BEDDING IS ENCOUNTERED DURING CONSTRUCTION, AN OWNER MUST HALT CONSTRUCTION UNTIL AN ENGINEER PREPARES A WRITTEN REPORT, WHICH MUST BE SUBMITTED WITH THE FINAL ENGINEERING REPORT, DETAILING HOW CONSTRUCTION WILL ACCOMMODATE THESE SITE CONDITIONS.

(B) COMPACTION.

(1) COMPACTION OF THE PIPE EMBEDMENT MUST MEET THE MANUFACTURER'S RECOMMENDATIONS FOR THE COLLECTION SYSTEM PIPES USED IN A PROJECT. COMPACTION OF THE PIPE EMBEDMENT MUST PROVIDE THE MODULUS OF SOIL REACTION FOR THE BEDDING MATERIAL NECESSARY TO ENSURE A COLLECTION SYSTEM PIPE'S STRUCTURAL INTEGRITY AS REQUIRED BY §217.53 OF THIS TITLE (RELATING TO PIPE DESIGN). THE PLACEMENT OF THE BACKFILL ABOVE A PIPE MUST NOT AFFECT THE STRUCTURAL INTEGRITY OF A PIPE.

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EMBODMENT THICKNESS.

(1) A MINIMUM CLEARANCE OF 6.0 INCHES BELOW AND ON EACH SIDE OF THE BELL OF ALL PIPES TO THE TRENCH WALLS AND FLOOR IS REQUIRED.

(2) THE EMBEDMENT MATERIAL USED FOR HAUNCHING AND INITIAL BACKFILL MUST BE INSTALLED TO A MINIMUM DEPTH OF 12 INCHES ABOVE THE CROWN OF A PIPE.

(3) TRENCH WIDTH.

(1) THE WIDTH OF A TRENCH MUST ALLOW A PIPE TO BE LAID AND JOINTED PROPERLY AND MUST ALLOW THE BACKFILL TO BE PLACED AND COMPACTED AS NEEDED.

(2) THE MAXIMUM AND MINIMUM TRENCH WIDTH NEEDED FOR SAFETY AND A PIPE'S STRUCTURAL INTEGRITY MUST BE INCLUDED IN THE ENGINEERING REPORT.

(3) THE WIDTH OF A TRENCH MUST ALLOW PROPER AND SAFE PLACEMENT AND COMPACTION OF HAUNCHING MATERIALS IN ACCORDANCE WITH THE STANDARDS IN SUBSECTION (A) OF THIS SECTION.

(4) THE SPACE BETWEEN A PIPE AND A TRENCH WALL MUST BE WIDER THAN THE COMPACTION EQUIPMENT USED IN THE PIPE ZONE.

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THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTORS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

Bowman

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REVISIONS

REVISION	DESCRIPTION	DATE
1		

TCEQ NOTES

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633

STATE OF TEXAS
MARISA KEISER
141397
LICENSED PROFESSIONAL ENGINEER

Marisa Keiser
August 4, 2025

DESIGN	DRAWN	CHKD
MC	LB	MK

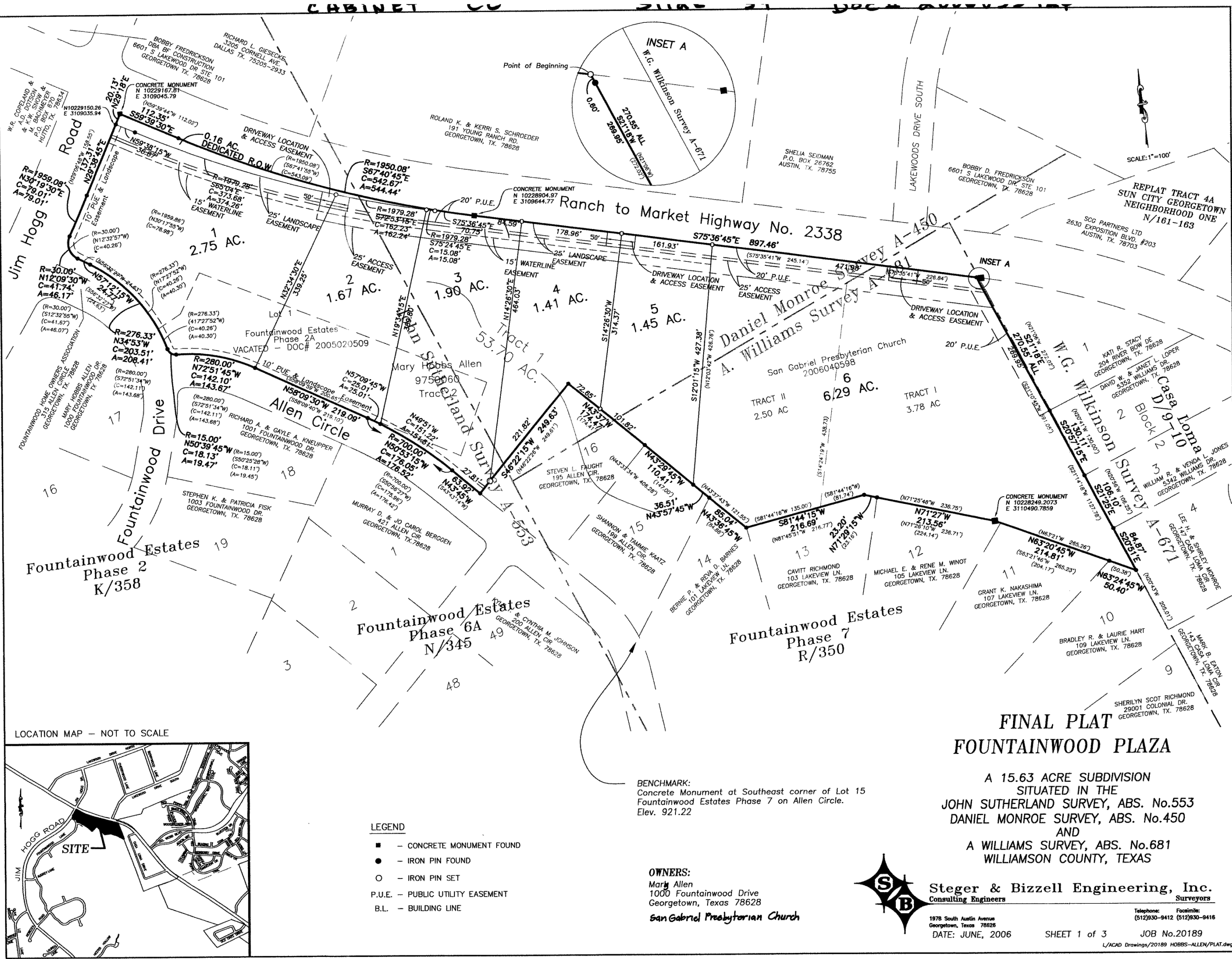
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3	OF 25

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14-C-6
FOUNTAINWOOD PLAZA

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REVISIONS	DATE	DESCRIPTION

FINAL PLAT (1 OF 3)
O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633

DESIGN	DRAWN	CHKD
MC	LB	MK

JOB No. 250519-01-002
SHEET
4 OF 25

V:\250519 - O'Reilly's Auto Parts\250519-01-002 (ENG) - Georgetown, TX (GX3)\Engineering\Engineering Plans\250519-01-002_FINAL PLAT.dwg, FINAL PLAT (2 OF 3), August 01, 2025, 9:19 AM, jrgm.rwg

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PERIMETER DESCRIPTION

BEING 15.63 acres of land, situated in the John Sutherland Survey, Abstract No. 553, the Daniel Monroe Survey, Abstract No. 450, and the A. Williams Survey, Abstract No. 681, in Williamson County, Texas, said land being a portion of that certain Tract 1, called 53.70 acres, and all of that certain Tract 2, (Tract 2 being Lot 1 of Fountainwood Estates, Phase 2A, a subdivision of record in Cabinet K, Slide 360, of the Plat Records of Williamson County, Texas) as conveyed to Mary Hobbs Allen of the Plat Records of Williamson County, Texas, as conveyed to Williamson County, Texas, record as Document No. 9750060 of the Official Records of Williamson County, Texas, as being that certain Tract I, called 3.78 acres and Tract II, called 2.50 acres, as conveyed to San Gabriel Presbyterian Church, of record as Document No. 2006040598 of the Official Public Records of Williamson County, Texas. Surveyed on the ground in the month of June, 2003, under the supervision of Brian F. Peterson, Registered Professional Land Surveyor, and being more particularly described as follows;

BEGINNING at a point on the Southwest line of Ranch to Market Highway No. 2338, being on, or near, the east line of the A. Williams Survey, Abstract No. 681, being on, or near, the west line of the W.G. Wilkinson Survey, Abstract No. 671, for the Northeast corner of the above-referenced 53.70 acre Allen Tract 1, being the Northeast corner of the said 3.78 acre San Gabriel Presbyterian Church Tract I, and the most northerly corner of Casa Loma, a subdivision of record in Cabinet D, Slides 9-10, of the Plat Records of Williamson County, Texas, from said point the Northeast corner of the said A. Williams Survey, A-681, bears approximately N 21° W, 110 feet, more or less;

THENCE, along, or near, the east line of the said A. Williams Survey, A-681, being along, or near, the west line of the said W.G. Wilkinson Survey, A-671, along the east line of the said 53.70 acre Allen Tract 1, being the west line of the said Casa Loma, S 21° 18' E, at 0.60 of a foot pass an iron pin found for a total distance of 270.55 feet, in all, to an iron pin found; S 20° 57' 15" E, 130.11 feet to an iron pin found; S 21° 25' E, 106.10 feet to an iron pin found and S 20° 51' E, 84.87 feet to an iron pin found marking the Southeast corner of the said 3.78 acre San Gabriel Presbyterian Church Tract I, being the Northeast corner of Fountainwood Estates, Phase 7, a subdivision of record in Cabinet R, Slide 350, of the Plat Records of Williamson County, Texas, for the Southeast corner hereof;

THENCE, along the north line of the said Fountainwood Estates, Phase 7, along the south line of the said 3.78 acre San Gabriel Presbyterian Church Tract I, N 63° 24' 45" W, 50.40 feet to a railroad spike found and N 63° 20' 45" W, 214.81 feet to a concrete monument found; N 71° 27' W, 213.56 feet to an iron pin found; N 71° 29' 15" W, 23.20 feet to an iron pin found; S 81° 44' 15" W, at 81.74 feet, more or less, pass the Southwest corner of the said 3.78 acre San Gabriel Presbyterian Church Tract I, being the Southeast corner of the above-referenced 2.50 acre San Gabriel Presbyterian Church Tract II, and continuing along the south line of the said 2.50 acre San Gabriel Presbyterian Church Tract II, for a total distance of 218.69 feet, in all, to an iron pin found; N 43° 36' 45" W, 85.04 feet to an iron pin found and N 43° 57' 45" W, 36.51 feet to an iron pin found marking the Southwest corner of the said 2.50 acre San Gabriel Presbyterian Church Tract II, N 43° 29' 45" W, 110.41 feet to an iron pin found and N 43° 37' W, passing the north line of the A. Williams Survey, A-681, being the south line of the Daniel Monroe Survey, A-450, for a total distance of 174.47 feet, in all, to an iron pin found and S 46° 22' 15" W, passing the west line of the said Daniel Monroe Survey, A-450, being the east line of the John Sutherland Survey, A-553, for a total distance of 249.63 feet, in all, to an iron pin found on the north line of Allen Circle, a dedicated roadway, being the north line of Fountainwood Estates, Phase 6A, a subdivision of record in Cabinet N, Slide 345, of the Plat Records of Williamson County, Texas, being the most westerly corner of the said Fountainwood Estates, Phase 7, being a westerly line of the said 53.70 acre Allen tract, for a southerly corner hereof;

THENCE, along the said north line of Allen Circle, N 43° 45' W, 63.92 feet to an iron pin found at the beginning of a curve to the left, (Radius = 700.00 feet, Long Chord bears N 50° 53' 15" W, 176.05 feet),
Thence, along the said curve for an arc distance of 176.52 feet to an iron pin found on the south line of the above-referenced Allen Tract 2, being a corner of the said 53.70 acre Allen Tract 1;
Thence, continuing along the south line of the said Tract 2, N 58° 09' 30" W, 219.09 feet to an iron pin found at the beginning of a curve to the left, (Radius = 280.00 feet, Long Chord bears N 72° 51' 45" W, 142.10 feet);
Thence, along the said curve for an arc distance of 143.67 feet to an iron pin found at the beginning of a curve to the right, (Radius = 15.00 feet, Long Chord bears N 50° 39' 45" W, 18.13 feet);
Thence, along the said curve for an arc distance of 19.47 feet to an iron pin found at the intersection of the said north line of Allen Circle and the east line of Fountainwood Drive, a dedicated roadway, being the Northwest corner of the said Fountainwood Estates, Phase 6A, being on the Northeast line of Fountainwood Estates, Phase 2, a subdivision of record in Cabinet K, Slide 358, of the Plat Records of Williamson County, Texas, and continuing along the said east line of Fountainwood Drive, along a curve to the left, (Radius = 276.33 feet, Long Chord bears N 34° 53' W, 203.51 feet);
Thence, along the said curve, at an arc distance of 40.30 feet, more or less, pass the Northwest corner of the said Allen Tract 2, being a southerly corner of the said 53.70 acre Allen Tract 1, and continuing along the west line of the said 53.70 acre Allen Tract 1, for a total arc distance of 208.41 feet, in all, to an iron pin found;
Thence, N 57° 12' 15" W, 24.77 feet to an iron pin found at the beginning of a curve to the right, (Radius = 30.00 feet, Long Chord bears N 12° 09' 30" W, 41.74 feet);
Thence, along the said curve for an arc distance of 46.17 feet to an iron pin found at the intersection of Fountainwood Drive and the Southeast line of Jim Hogg Road, marking the most northerly corner of the said Fountainwood Estates, Phase 21, for the most westerly corner of the said 53.70 acre Allen Tract 1, for the most westerly corner hereof;

THENCE, along the said Southeast line of Jim Hogg Road, along a curve to the left, (Radius = 1959.08 feet, Long Chord bears N 30° 19' 30" E, 79.01 feet), an arc distance of 79.01 feet to an iron pin found;
Thence, N 29° 38' 45" E, 137.31 feet to an iron pin found and N 29° 18' E, 20.13 feet to a concrete monument found at the intersection of the said Southeast line of Jim Hogg Road and the said Southwest line of R.M. Highway No. 2338, for the most northerly corner of the said 53.70 acre Allen Tract 1, for the most northerly corner hereof;

THENCE, along the said Southwest line of R.M. Highway No. 2338, being the north line of the said 53.70 acre Allen Tract 1, as follows; S 59° 39' 30" E, 112.35 feet to an iron pin found at the beginning of a curve to the left, (Radius = 1950.08 feet, Long Chord bears S 67° 40' 45" E, 542.67 feet);
Thence, along the said curve for an arc distance of 544.44 feet to a concrete monument found;
Thence, S 75° 36' 45" E, at 425.48 feet pass an iron pin set for the Northeast corner of the said 53.70 acre Allen Tract 1, being the Northwest corner of the said 2.50 acre San Gabriel Presbyterian Church Tract II, at 670.62 feet pass the Northeast corner of the said 2.50 acre San Gabriel Presbyterian Church Tract II, being the Northwest corner of the said 3.78 acre San Gabriel Presbyterian Church Tract I, for a total distance of 897.46 feet, in all, to the Place of BEGINNING and containing 15.63 acres of land.

STATE OF TEXAS }
COUNTY OF WILLIAMSON }

I, Brian F. Peterson, Registered Professional Land Surveyor, in the State of Texas, do hereby certify that this plat is true and correctly made from an actual survey on the ground of the property legally described hereon, and that there are no apparent discrepancies, conflicts, overlapping of improvements, visible utility lines or roads in place, except as shown on the accompanying plat, and that the perimeter field notes hereon have a mathematical closure in compliance with standards as set out in the "Manual of Practice" of the Texas Society of Professional Surveyors, to the best of my knowledge and belief.

TO CERTIFY WHICH, WITNESS my hand and seal at Georgetown, Williamson County, Texas,
this the 26th day of June, 2006, A.D.

Brian F. Peterson
Registered Professional Land Surveyor, No. 3967
State of Texas



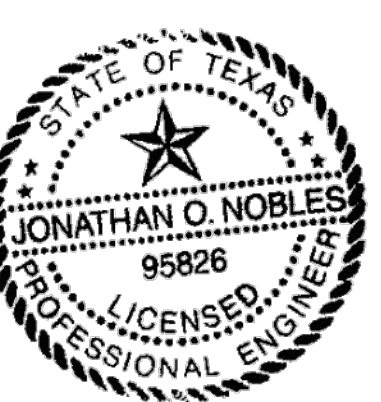
STATE OF TEXAS }
COUNTY OF WILLIAMSON }

I, Jonathan O. Nobles, Licensed Professional Engineer, do hereby certify that this subdivision is in the Edwards Aquifer Recharge Zone. An approved Water Pollution Abatement Plan is required by the Texas Natural Resource Conservation Commission prior to construction. No lot within this subdivision is encroached by any Special Flood Hazard Areas inundated by 100-Year Flood as identified by the U.S. Federal Emergency Management Agency Boundary map (Flood Insurance Rate Map), Community-Panel Numbers 48491C0100C and 48491C0115C, effective date September 27, 1991.

The engineering work for the roads and drainage (including driveway drain pipes) within the subdivision have been completed in compliance with the Williamson County Subdivision Regulations (including the Engineering Guidelines incorporated as Appendix B) and all generally accepted engineering standards.

TO CERTIFY WHICH, WITNESS my hand and seal at Georgetown, Williamson County, Texas,
this the 26th day of JUNE, 2006, A.D.

Jonathan O. Nobles
Professional Engineer, No. 95826
State of Texas



GENERAL NOTES:

- Total Acres: 15.63 Acres
- Number of lots: 6
- Number of blocks: 1
- Area of smallest lot: 1.414 Acres
- There is no area within the boundaries of this subdivision in the 100-year Flood plain as defined by FIRM Map Number 48491C0115C, effective date of September 27, 1991.
- Water service will be provided by Chisholm Trail Special Utility District, wastewater service shall be provided by the City of Georgetown. Electric service provided by P.E.C..
- In order to promote drainage away from a structure, the slab elevation should be built at least one foot above the surrounding ground, and the ground should be graded away from the structure at a slope of 1/2" per foot for a distance of a least 10 feet.
- Maximum impervious cover is 50% allowable up to 65% if appropriate impervious coverage waivers are met in accordance with UDC Section 11.02.020
- The bearings on this plat have been rotated to the NAD 83/93 HARN - Texas Central Zone and elevation is based upon NAVD 88.
- This subdivision is located in the limits of the City of Georgetown.
- This tract is located in the Edwards Aquifer Recharge Zone. An approved Water Pollution Abatement Plan is required by the Texas Natural Resource Conservation Commission prior to construction.
- All driveway pipes to be minimum 18" RCP or dip driveway.
- Building setbacks will be in accordance with the City of Georgetown UDC.
- Sidewalks and street trees will be constructed in accordance with the City of Georgetown UDC.
- A TIA, Traffic Impact Analysis will be required during the site plan review process when the cumulative Average Daily Trips (ADT) exceeds 2000 according to Institute of Transportation Engineer (ITE) manual.
- No access shall be permitted onto either Fountainwood Drive or Allen Circle from this development. Access to Williams Drive for all lots shall be from the driveway locations shown on this plat.
- A rock wall will be built by the developer along the boundary length of Fountainwood Drive and Allen Circle with the submittal of a site plan within this subdivision and prior to final certificate of occupancy..

FINAL PLAT
FOUNTAINWOOD PLAZA

A 15.63 ACRE SUBDIVISION
SITUATED IN THE
JOHN SUTHERLAND SURVEY, ABS. No.553
DANIEL MONROE SURVEY, ABS. No.450
AND
A WILLIAMS SURVEY, ABS. No.681
WILLIAMSON COUNTY, TEXAS



Steger & Bizzell Engineering, Inc.
Consulting Engineers Surveyors

1978 South Austin Avenue
Georgetown, Texas 78626
DATE: June, 2006

Telephone: (512)930-9412 Facsimile: (512)930-9416
SHEET 2 of 3 JOB No.20189
L/ACAD Drawings/20189 HOBBS-ALLEN/PLAT.dwg



THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTORS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

DESIGN	DRAWN	CHKD
MC	LB	MK
JOB No. 250519-01-002		
SHEET		
5	OF	25

REVISIONS	DATE				
	DESCRIPTION				
REVISION					

FINAL PLAT (2 OF 3)
O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633

Bowman
© 2024 Bowman Consulting Group LLC
189 E. FIRM Registration No. F-14009
8071 Las Colinas Parkway
Suite 350
Austin, Texas 78746
Phone: (512) 322-1800
Fax: (512) 327-4062
www.bowman.com

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STATE OF TEXAS {
COUNTY OF WILLIAMSON {
KNOW ALL MEN BY THESE PRESENTS:

That I, Mary Hobbs Allen, owner of that certain tract of land shown hereon and described in a deed recorded in Documents No. 9750060 of the Official Records of Williamson County, Texas, do hereby join, approve, and consent to all dedications and plat note requirements shown hereon. I do hereby approve the recordation of this subdivision plat and dedicate to the public use forever any easements and roads that are shown hereon. This subdivision is to be known as FOUNTAINWOOD PLAZA. I hereby acknowledge that I am the owner of this property and do hereby state that there are no lienholders or any unpaid debt for which this property represents collateral on any loan. It is also understood that it is the responsibility of the owner, not the County, to assure compliance with the provisions of all applicable state, federal, and local laws and regulations relating to the environment, including, but not limited to the Endangered Species Act, State Aquifer Regulations, and Municipal Watershed Ordinances.

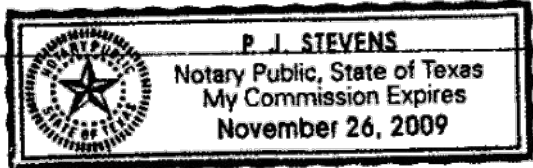
Mary Hobbs Allen
Mary Hobbs Allen
1000 Fountainwood Drive
Georgetown, Texas 78628

Before me, the undersigned authority, on this day personally appeared Mary Hobbs Allen known by me to be the person whose name is subscribed to the foregoing instrument. It has been acknowledged to me that he/she executed the foregoing instrument as the owner of the property described hereon.

[Signature] Notary Public in and for the State of Texas
Signature and Date

P. J. Stevens Printed Name of
Notary and Notary Stamp

Commission Expires November 26, 2009 Date Notary



STATE OF TEXAS {
COUNTY OF WILLIAMSON {
KNOW ALL MEN BY THESE PRESENTS:

That we, San Gabriel Presbyterian Church of Georgetown, owner of the certain tract of land shown hereon and described in a deed recorded in Document #2006040598 of the Deed Records of Williamson County, Texas, do hereby join, approve, and consent to all dedications and plat note requirements shown hereon. I do hereby approve the recordation of this subdivision plat and dedicate to the public use forever any easements and roads that are shown hereon. This subdivision is to be known as FOUNTAINWOOD PLAZA. It is also understood that it is the responsibility of the owner, not the County, to assure compliance with the provisions of all applicable state, federal, and local laws and regulations relating to the environment, including, but not limited to the Endangered Species Act, State Aquifer Regulations, and Municipal Watershed Ordinances.

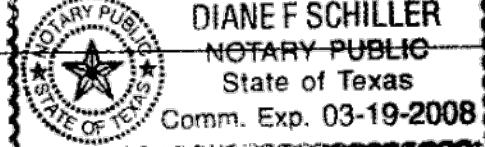
Donald J. Harper
Donald J. Harper, Building Committee Chairman
San Gabriel Presbyterian Church of Georgetown
6920 RR 2338
Georgetown, Texas 78628

Before me, the undersigned authority, on this day personally appeared Donald J. Harper, known by me to be the person whose name is subscribed to the foregoing instrument. It has been acknowledged to me that she executed the foregoing instrument as the owner of the property described hereon.

Diane F. Schiller Notary Public in and for the State of Texas
Signature and Date

Diane F. Schiller Printed Name of Notary and
Notary Stamp

3-19-08 Date Notary Commission
Expires



STATE OF TEXAS {
COUNTY OF WILLIAMSON {
KNOW ALL MEN BY THESE PRESENTS:

That I, First Texas Bank, the lien holder of the certain tract of land shown hereon and described in a DEED OF TRUST recorded in Document #2006040597 of the Official Records of Williamson County, Texas, do hereby join, approve, and consent to all dedications and plat note requirements shown hereon. I do hereby approve the recordation of this subdivision plat and dedicate to the public use forever any easements and roads that are shown hereon. This subdivision is to be known as FOUNTAINWOOD PLAZA.

Kevin R. Kelly
Kevin R. Kelly, V.P.
First Texas Bank
900 S. Austin Ave.
Georgetown, TX 78626

Before me, the undersigned authority, on this day personally appeared Kevin R. Kelly known by me to be the person whose name is subscribed to the foregoing instrument. It has been acknowledged to me that he/she executed the foregoing instrument as the legal representative of the lienholder of the property described hereon.

APRIL WEATHERS Notary Public in and for the State of Texas
Signature and Date
Notary Stamp

12/2/07 Printed Name of Notary and
Date of Notary Commission
Expires

I, Bobby Ray, AICP, Director of the Planning and Development Services of the City of Georgetown, do hereby certify this plat as approved for filing of record with the County Clerk of Williamson County, Texas.

Date: 6/29/06
Bobby Ray Bobby Ray, Director
Planning and Development Services

The subdivision, known as FOUNTAINWOOD PLAZA, has been approved for filing for record according to the minutes of the meeting of the Georgetown City Council on the 27th day of April, 2004, A.D.

Attest:
Gary Melon
Gary Melon, Mayor
City of Georgetown, Texas

Sandra Lee
Sandra Lee, City Secretary
City of Georgetown, Texas

Based upon the above representations of the Engineer or Surveyor whose seal is affixed hereto, and after a review of the plat as represented by the said Engineer or Surveyor, I find that this plat complies with the requirements of Chapter 15.44, Flood Damage Prevention, of the Georgetown Municipal Code. This certification is made solely upon such representations and should not be relied upon for verifications of the facts alleged. The City of Georgetown disclaims any responsibility to any member of the public or independent verifications of the representation, factual or otherwise, contained in this plat and the documents associated with it.

Date: 6/29/06

Dave Hall Flood Plain Coordinator
City of Georgetown

STATE OF TEXAS {
COUNTY OF WILLIAMSON {
KNOW ALL MEN BY THESE PRESENTS

I, Nancy Rister, Clerk of the County Court of said County, do hereby certify that the foregoing instrument in writing, with its certificate of authentication was filed for record in my office on the 30th day of JUNE, 2006 A.D., at 8:36 o'clock, A.M., and duly recorded this the 6th day of JULY, 2006 A.D., at 11:49 o'clock, A.M., in the Plat Records of said County in Cabinet CC, Slides 39, 40 and 41.

TO CERTIFY WHICH, WITNESS my hand and seal at the County Court of said County, at my office in Georgetown, Texas, the date last shown above written.

Nancy Rister, Clerk County Court
of Williamson County, Texas
Nancy Rister Deputy



FINAL PLAT FOUNTAINWOOD PLAZA

A 15.63 ACRE SUBDIVISION
SITUATED IN THE
JOHN SUTHERLAND SURVEY, ABS. No.553
DANIEL MONROE SURVEY, ABS. No.450
AND
A WILLIAMS SURVEY, ABS. No.681
WILLIAMSON COUNTY, TEXAS



Steger & Bizzell Engineering, Inc.
Consulting Engineers Surveyors

1978 South Austin Avenue
Georgetown, Texas 78626

Telephone: (512)930-9412 Facsimile: (512)930-9416

DATE: JUNE, 2006

SHEET 3 of 3

JOB No.20189

L/ACAD Drawings/20189 HOBBS-ALLEN/PLAT.dwg



Know what's below.
Call before you dig.

THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

REVISION	DATE	DESCRIPTION

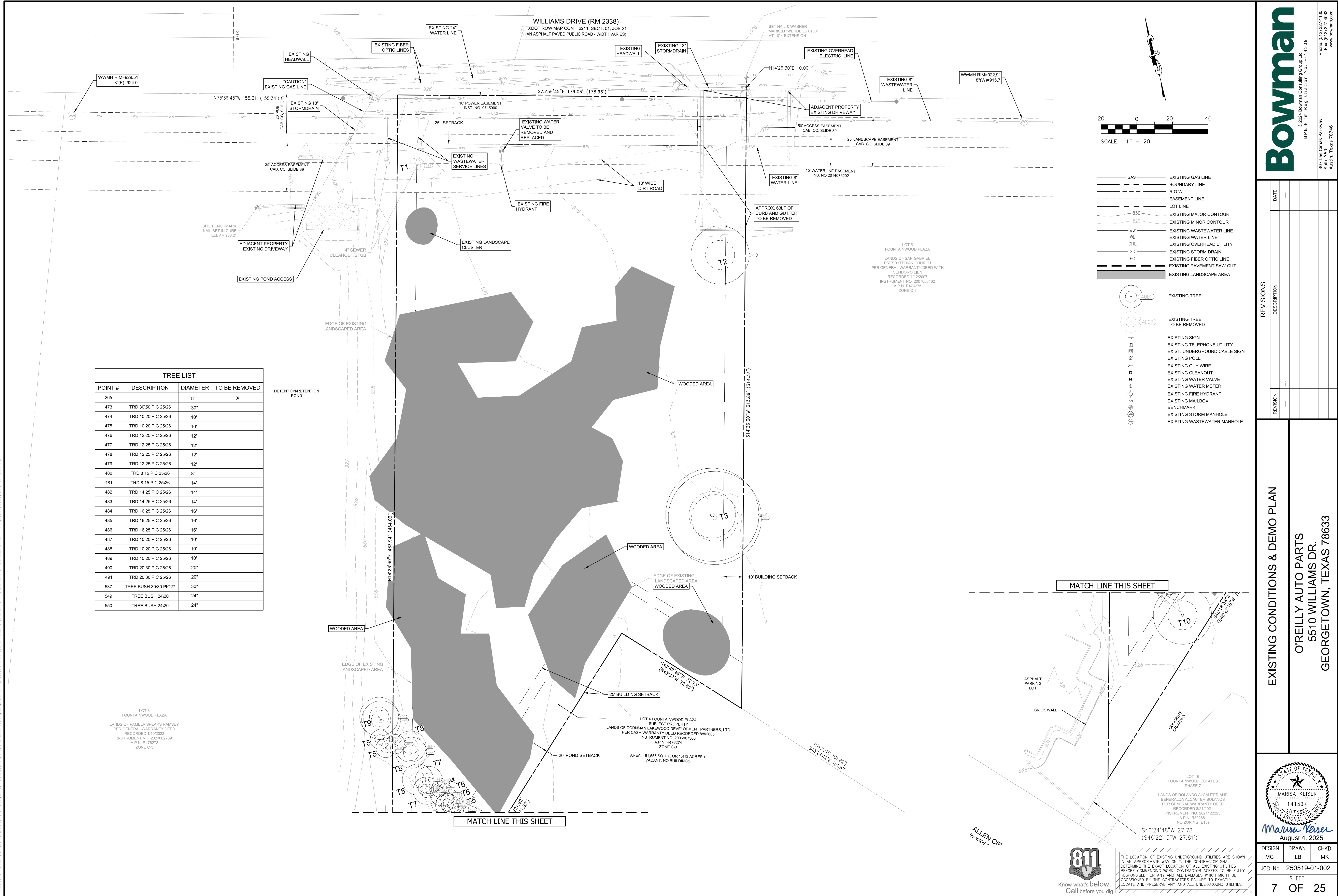
FINAL PLAT (3 OF 3)

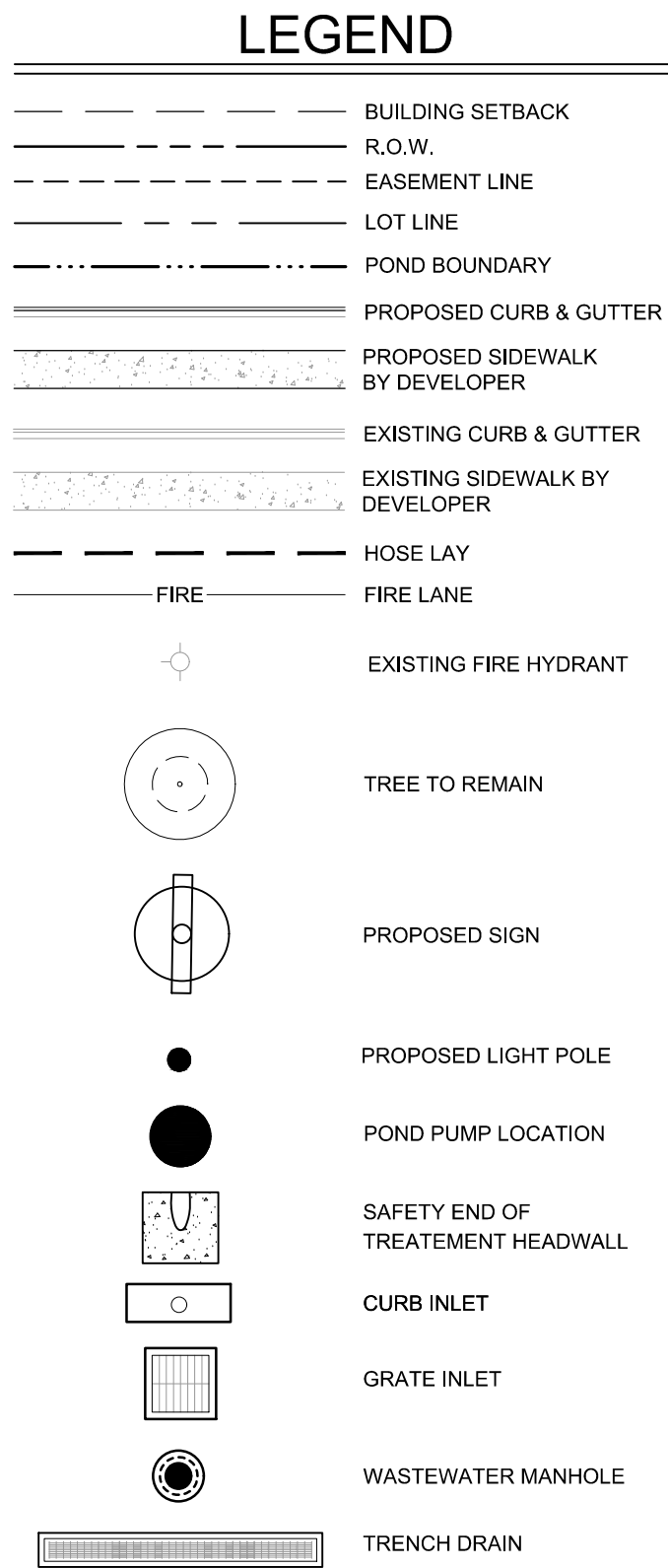
O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633

DESIGN	DRAWN	CHKD
MC	LB	MK
JOB No.	250519-01-002	
SHEET	6	25

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189 E. Fm. 1931 Station N.G. F-14109
8071 Las Cimas Parkway
Suite 350
Austin, Texas 78746
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www.bowman.com

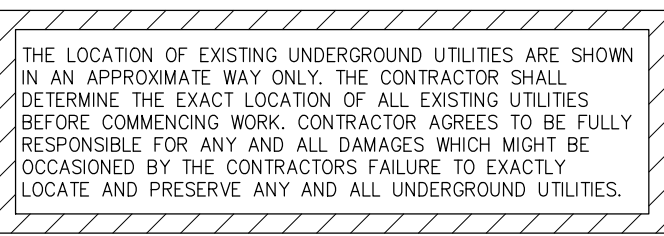




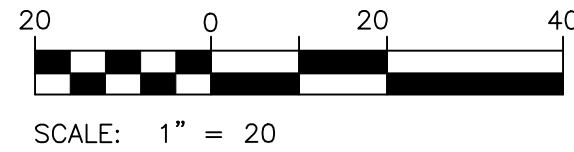
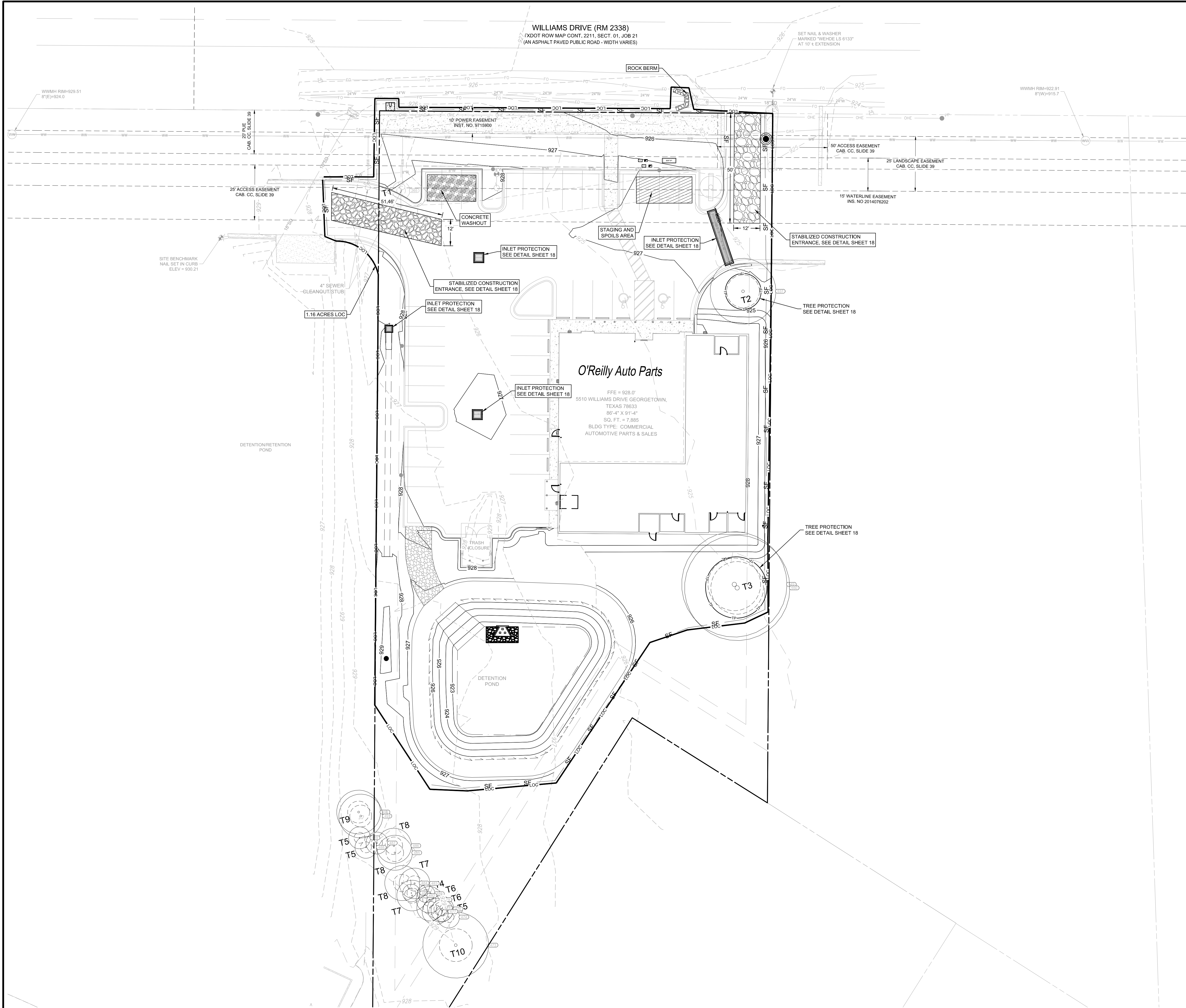
SITE DATA	
PROPERTY AREA:	1.41 AC
PROPOSED BUILDING AREA	7,885 SF
PROPOSED IMPERVIOUS COVER	.67 AC
MAX. IMPERVIOUS COVER ALLOWABLE	50% - 0.705 AC
IMPERVIOUS COVER WAIVER #	

DIMENSIONAL SITE PLAN NOTES:

1. ALL LIGHTING FIXTURES SHALL BE DESIGNED TO COMPLETELY CONCEAL AND FULLY SHIELD, WITHIN AN OPAQUE HOUSING, THE LIGHT SOURCE FROM VISIBILITY FROM ANY STREET OR RIGHT-OF-WAY. THE CONE OF LIGHT SHALL NOT CROSS ANY ADJACENT PROPERTY LINE. THE ILLUMINATION SHALL NOT EXCEED 2-FOOT CANDLES AT A HEIGHT OF THREE FEET AT THE PROPERTY LINE, ONLY INCANDESCENT, FLUORESCENT, XOLOR-CORRECTED HIGH-PRESSURE SODIUM OR METAL HALIDE MAY BE USED. ALL VEHICLE OR PEDESTRIAN ACCESS SHALL BE SUFFICIENTLY LIGHTED TO ENSURE SECURITY OF PROPERTY AND PERSONS.
2. ALL ROOF, WALL AND GROUND MOUNTED MECHANICAL EQUIPMENT MUST BE SCREENED IN ACCORDANCE WITH CHAPTER 8 OF THE UDC. IF ROOF AND WALL MOUNTED EQUIPMENT OF ANY TYPE INCLUDING DUCT WORK AND LARGE VENTS IS PROPOSED IT SHALL BE SHOWN ON THE SITE PLAN AND SCREENING IDENTIFIED. SCREENING OF MECHANICAL EQUIPMENT SHALL RESULT IN THE MECHANICAL EQUIPMENT BLENDING IN WITH THE PRIMARY BUILDING AND NOT APPEARING SEPARATE FROM THE BUILDING AND SHALL BE SCREENED FROM VIEW OF ANY RIGHTS-OF-WAY OR ADJOINING PROPERTIES.
3. PER CHAPTER 8, THE DUMPSTER ENCLOSURES MUST BE ONE (1) FOOT ABOVE THE HEIGHT OF THE WASTE CONTAINER, USE PROTECTIVE POLES IN CORNERS AND AT IMPACT AREAS, FENCE POSTS SHALL BE OF RUST PROTECTED METAL OR CONCRETE. A MINIMUM 6" SLAB IS REQUIRED AND MUST BE SLOPED TO DRAIN. THE ENCLOSURE MUST HAVE STEEL FRAMING GATES WITH SPRING LOADED HINGES AND FASTENERS TO KEEP CLOSED. SCREENING MUST BE ON ALL FOUR SIDES BY MASONRY WALL OR APPROVED FENCE OR SCREENING WITH OPAQUE GATES.



V:\250519 - O'Reilly's Auto Parts\250519-01-002 (ENG) - Georgetown, TX (GX3)\Engineering\250519-01-002_EROSION & SEDIMENTATION CONTROL PLAN August 01, 2025 9:19 AM greg.may



LEGEND

- PROPOSED R.O.W.
- PROPOSED LOT LINE
- EXISTING R.O.W.
- EASEMENT LINE
- POND BOUNDARY
- PROPOSED CURB & GUTTER
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- LOC
- LIMITS OF CONSTRUCTION
- PROPOSED SILT FENCE
- PROPOSED TREE PROTECTION
- INLET PROTECTION
- STABILIZED CONSTRUCTION ENTRANCE
- STAGING / SPOILS AREA
- CONCRETE WASHOUT
- PROPOSED ROCK BERM
- EXISTING TREE
- PROPOSED LIGHT POLE

QUANTITIES

LIMITS OF CONSTRUCTION	1.16 AC
STABILIZED CONSTRUCTION ENTRANCE	2
SILT FENCE	687 LF
CONCRETE WASHOUT AREA	1
STAGING AND SPOIL AREA	1
TREE PROTECTION FENCE	823 LF
INLET PROTECTION	4
ROCK BERM	1
RIP RAP	77 SF



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EROSION & SEDIMENTATION CONTROL PLAN

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



August 4, 2025

DESIGN	DRAWN	CHKD
MC	LB	MK
JOB No.	250519-01-002	

SHEET
9 OF 25

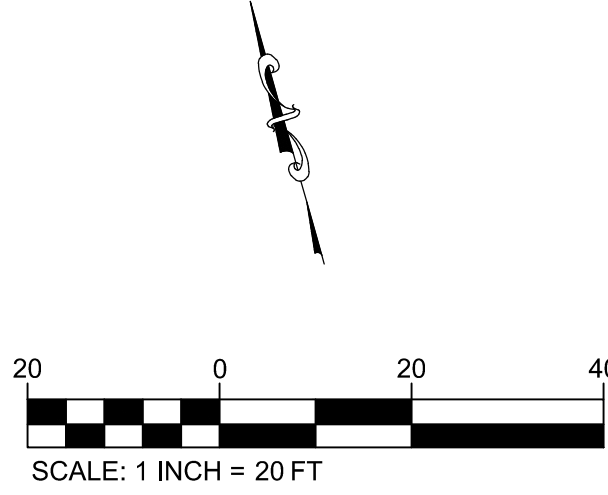
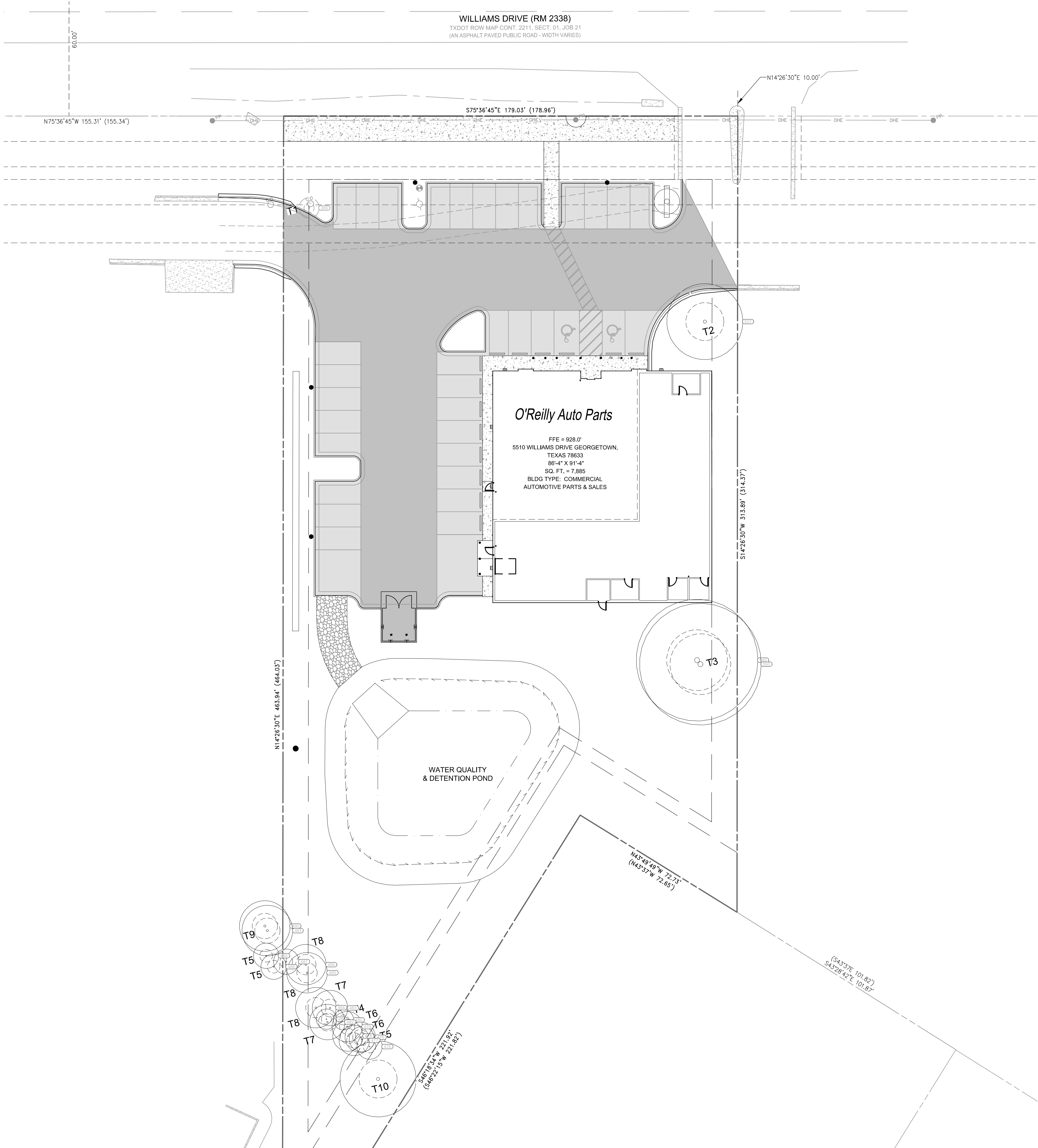
REVISIONS

REVISION	DESCRIPTION	DATE

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8071 Las Cimarras Parkway
Suite 350
Austin, Texas 78746
Phone: (512) 327-1801
Fax: (512) 327-4062
www.bowman.com

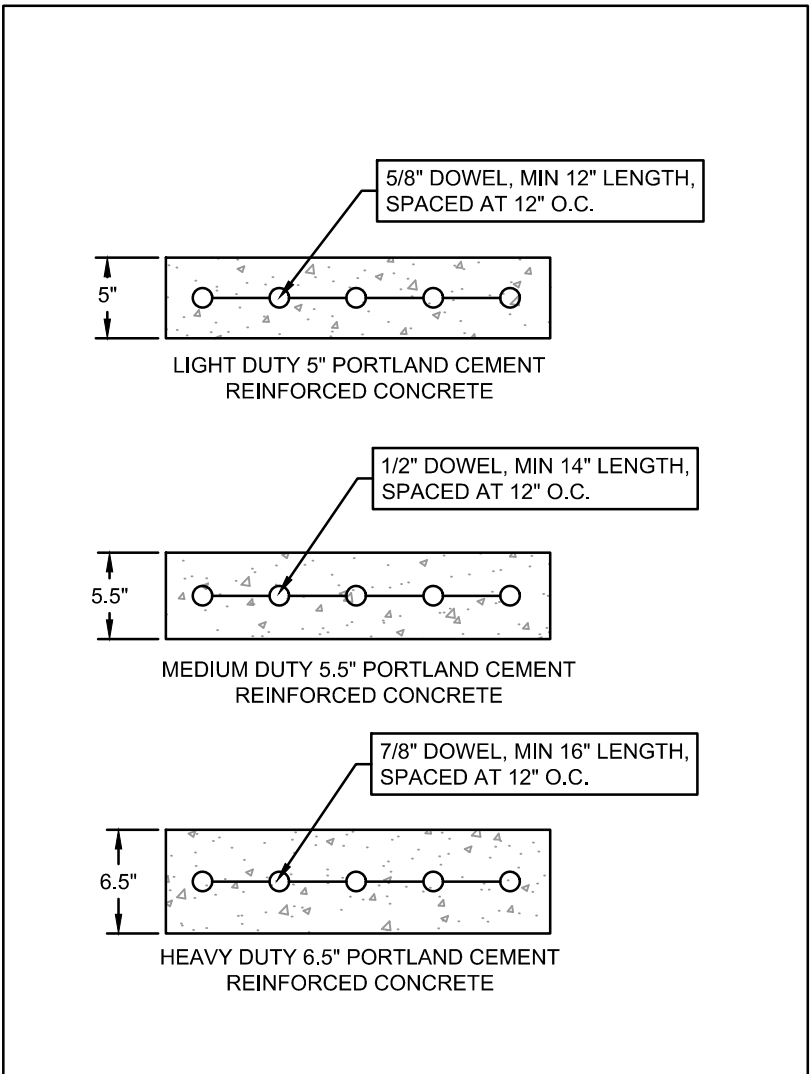
V:\250519-01-O'Reilly's Auto Parts\250519-01-002 (ENG) - Georgetown, TX (GX3)\Engineering\Plans\250519-01-002_PAV.dwg, PAVING PLAN, August 01, 2025, 9:19 AM, gpg.may



LEGEND

- R.O.W.
- EASEMENT LINE
- LOT LINE
- POND BOUNDARY
- PROPOSED CURB & GUTTER
- PROPOSED SIDEWALK BY DEVELOPER
- EXISTING WASTEWATER LINE
- EXISTING WATER LINE
- EXISTING OVERHEAD UTILITY
- EXISTING STORM DRAIN
- WASTEWATER LINE
- STORM DRAIN PIPE
- WASTEWATER PIPE
- HOSE LAY
- FIRE LANE
- FIRE
- FIRE HYDRANT
- HEADWALL
- CURB INLET
- STORMSEWER MANHOLE
- WASTEWATER MANHOLE
- TREE TO REMAIN
- PROPOSED SIGN
- LIGHT DUTY 5" CONCRETE PAVEMENT
- MODERATE DUTY 5.5" CONCRETE PAVEMENT
- HEAVY DUTY 6.5" CONCRETE PAVEMENT
- PROPOSED CONCRETE SIDEWALK
- PROPOSED LIGHT POLE
- POND PUMP LOCATION

CONCRETE PAVEMENT SECTIONS SCALE 1"=1'



*SEE GEOTECH REPORT NUMBER 17-8791 BY ESC SOUTHWEST, LLP
DATED DECEMBER 31, 2024



Know what's below.
Call before you dig.

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REVISIONS

REVISION	DESCRIPTION	DATE

PAVING PLAN

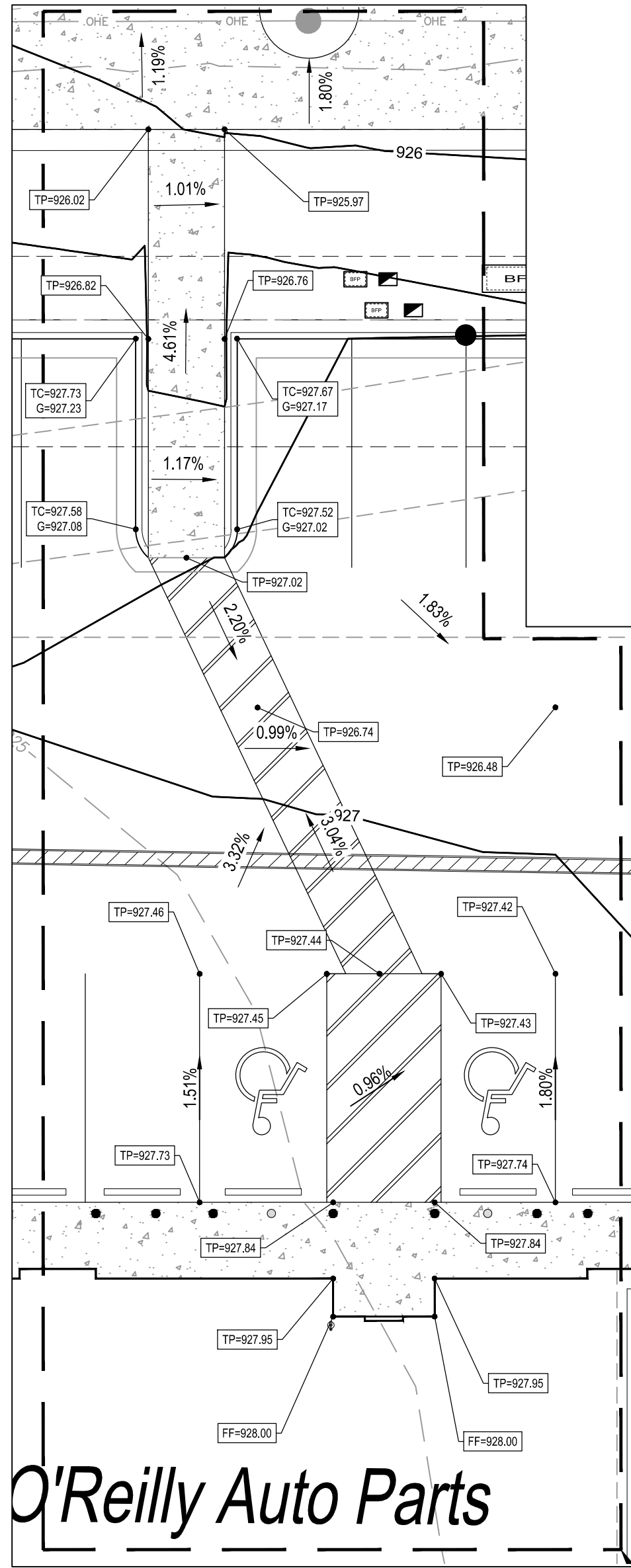
O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



August 4, 2025

DESIGN MC DRAWN LB CHKD MK
JOB No. 250519-01-002

SHEET
10 OF 25



A graphic scale bar and a north arrow. The scale bar is marked with 20, 0, 20, and 40. Below the scale bar, the text reads: SCALE: 1 INCH = 20 FT. The north arrow is a vertical line with a circular arrowhead pointing upwards, indicating North.

The diagram illustrates a cross-section of a proposed road layout. At the top, a solid line represents the 'PROPOSED R.O.W.' (Right of Way). Below it, a dashed line indicates the 'PROPOSED LOT LINE'. A solid line marks the 'EASEMENT LINE'. Further down, a solid line with a small circle at its base represents the 'POND BOUNDARY'. Below this, a solid line is labeled 'PROPOSED CURB & GUTTER'. The next line is a dashed line labeled '830', representing the 'EXISTING MAJOR CONTOUR'. Below that is a solid line labeled '829', representing the 'EXISTING MINOR CONTOUR'. This is followed by another dashed line labeled '830', representing the 'PROPOSED MAJOR CONTOUR'. Below that is a solid line labeled '829', representing the 'PROPOSED MINOR CONTOUR'. A hatched area represents the 'PROPOSED SIDEWALK'. Below the sidewalk is a solid line for the 'POND BOTTOM' and a dashed line for the 'POND BERM'. Two circles represent the 'PROPOSED LIGHT POLE' and 'POND PUMP'. At the bottom, a solid line represents the 'FINISHED FLOOR'.

PROPOSED R.O.W.

PROPOSED LOT LINE

EASEMENT LINE

POND BOUNDARY

PROPOSED CURB & GUTTER

830
EXISTING MAJOR CONTOUR

829
EXISTING MINOR CONTOUR

830
PROPOSED MAJOR CONTOUR

829
PROPOSED MINOR CONTOUR

PROPOSED SIDEWALK

POND BOTTOM

POND BERM

PROPOSED LIGHT POLE

POND PUMP

TP = TOP OF PAVEMENT
TC = TOP OF CURB
G = GRADE
ME = MATCH EXISTING
TG = TOP OF GRATE
FF = FINISHED FLOOR
TW = TOP OF WALL
BW = BOTTOM OF WALL
RIM = RIM

REVISIONS		
REVISION	DESCRIPTION	DATE
1	Initial design	10/10/2023
2	Revised design	11/10/2023
3	Final design	12/10/2023
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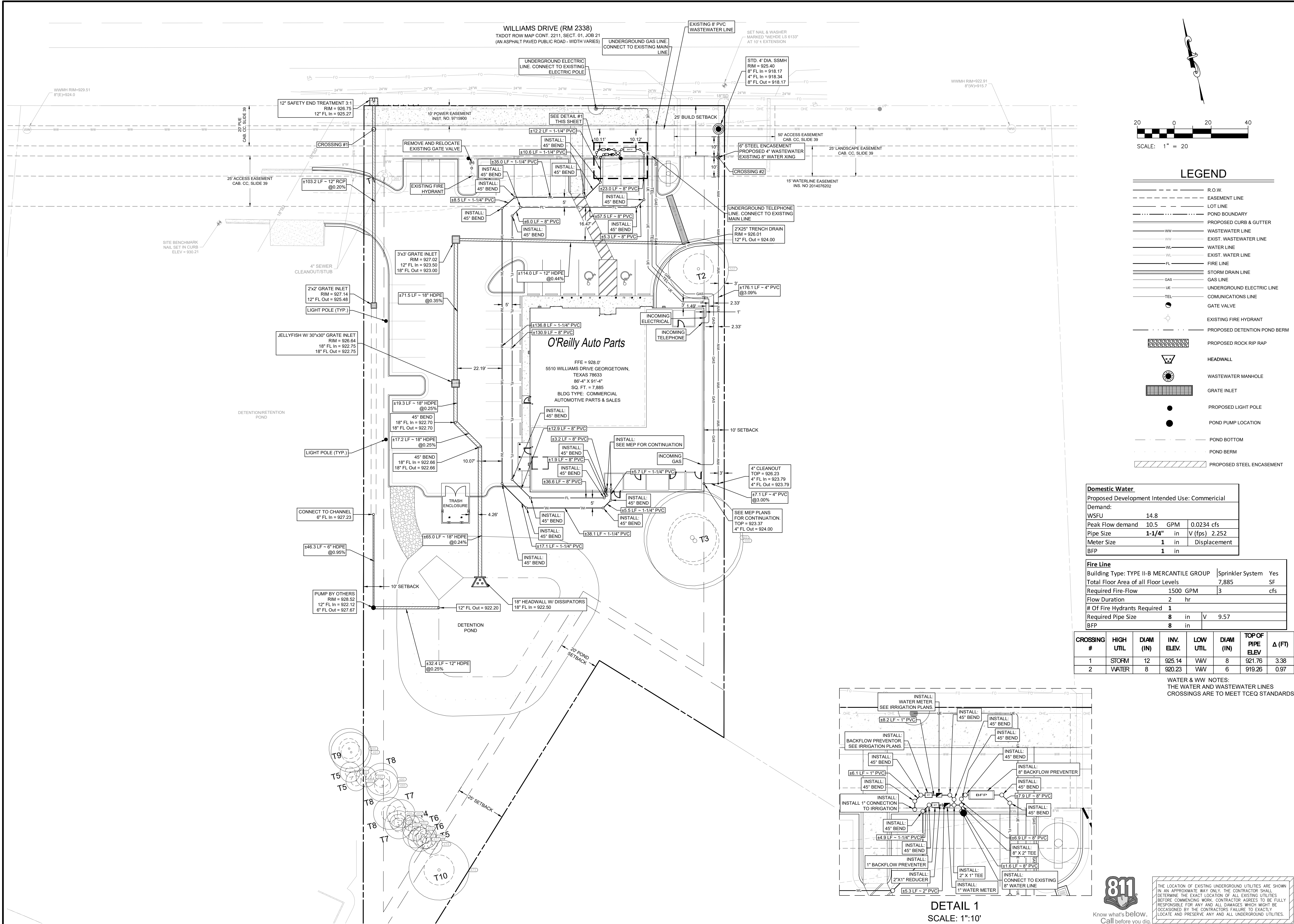
GRADING PLAN

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



DESIGN MC	DRAWN LB	CHKD MK
JOB No. 250519-01-002		
SHEET 11 OF 25		

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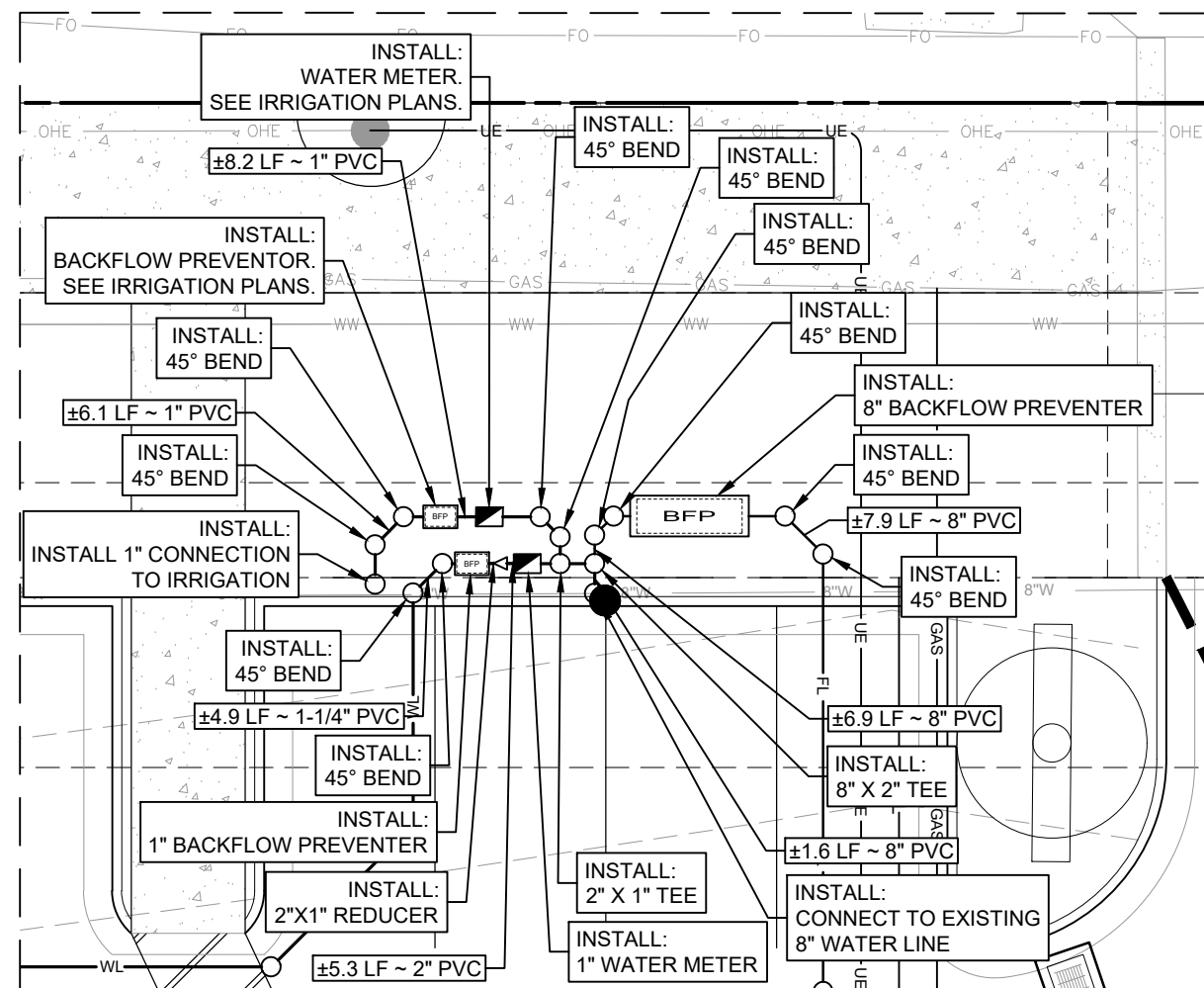


Domestic Water				
Proposed Development Intended Use: Commercial				
Demand:				
WSFU	14.8			
Peak Flow demand	10.5	GPM	0.0234 cfs	
Pipe Size	1-1/4"	in	V (fps)	2.252
Meter Size	1	in	Displacement	
BFP	1	in		

Fire Line				
Building Type: TYPE II-B MERCANTILE GROUP			Sprinkler System	Yes
Total Floor Area of all Floor Levels			7,885	SF
Required Fire-Flow	1500	GPM	3	cfs
Flow Duration	2	hr		
# Of Fire Hydrants Required	1			
Required Pipe Size	8	in	V	9.57
BFP	8	in		

CROSSING #	HIGH UTIL	DIAM (IN)	INV. ELEV.	LOW UTIL	DIAM (IN)	TOPOF PIPE ELEV	Δ (FT)
1	STORM	12	925.14	VWV	8	921.76	3.38
2	WATER	8	920.23	VWV	6	919.26	0.97

WATER & WW NOTES:
THE WATER AND WASTEWATER LINES
CROSSINGS ARE TO MEET TCEQ STANDARDS



DETAIL 1
SCALE: 1"=10'



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REVISIONS

REVISION	DATE	DESCRIPTION

UTILITY PLAN

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633

DESIGN
MC

DRAWN
LB

CHKD
MK

JOB No. 250519-01-002

SHEET
12 OF 25

STATE OF TEXAS

MARISA KEISER

141397

LICENSED PROFESSIONAL ENGINEER

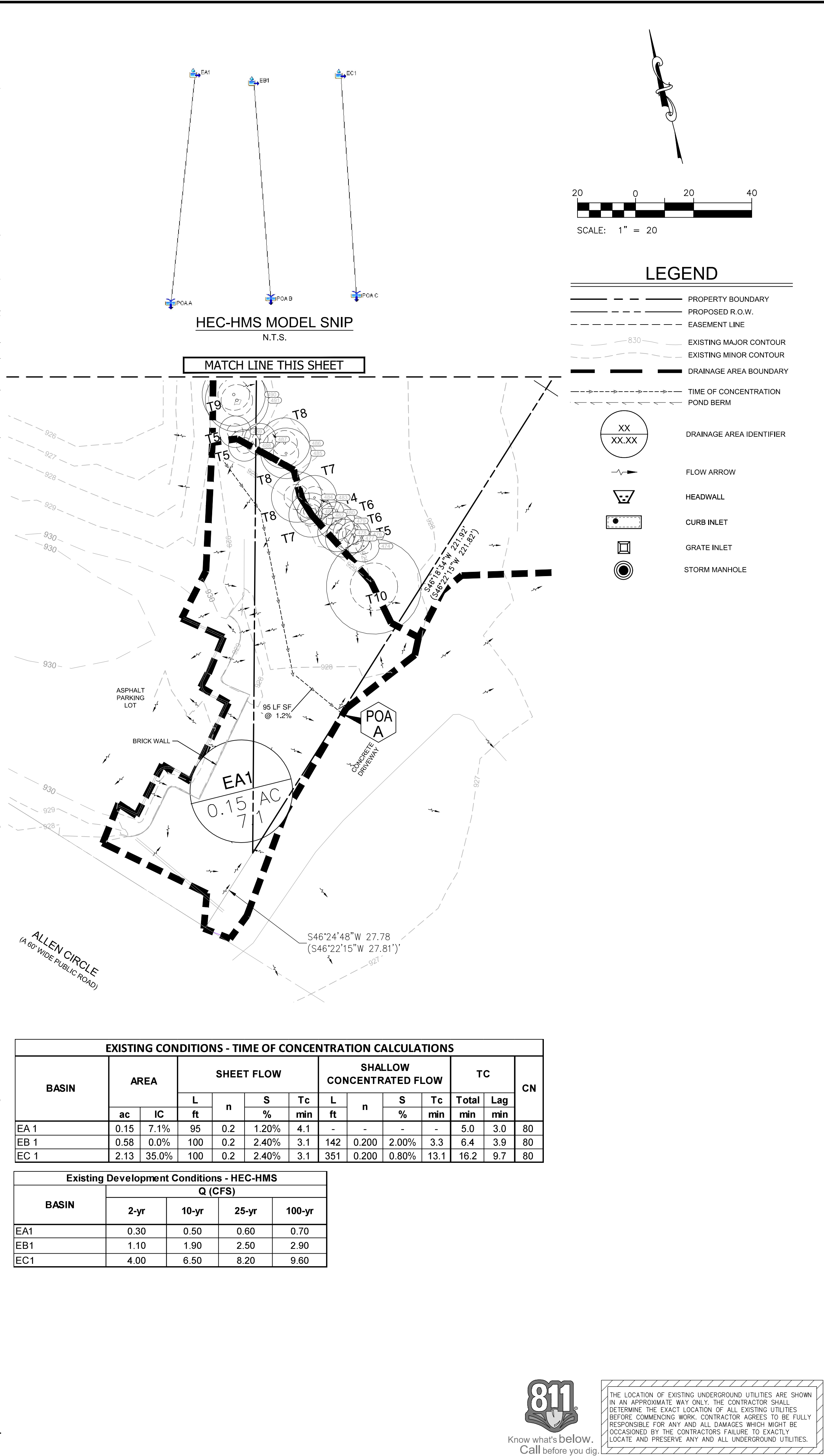
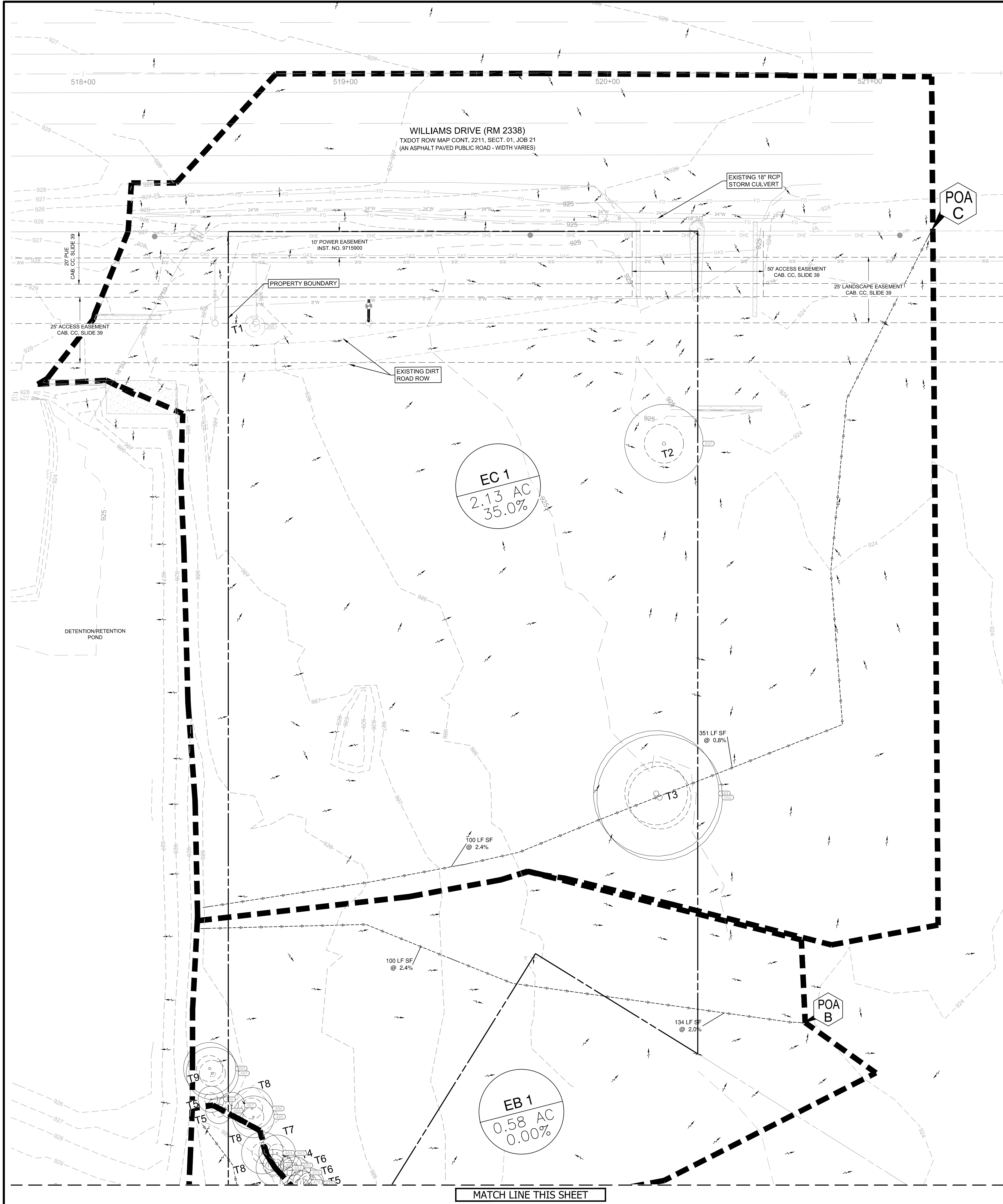
Marisa Keiser

August 4, 2025

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REVISIONS

DATE

REVISION

DESCRIPTION

EXISTING DRAINAGE AREA MAP

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633

STATE OF TEXAS
MARISA KEISER
141397
LICENSED PROFESSIONAL ENGINEER
Marisa Keiser
August 4, 2025

DESIGN
MC

DRAWN
LB

CHKD
MK

JOB No. 250519-01-002

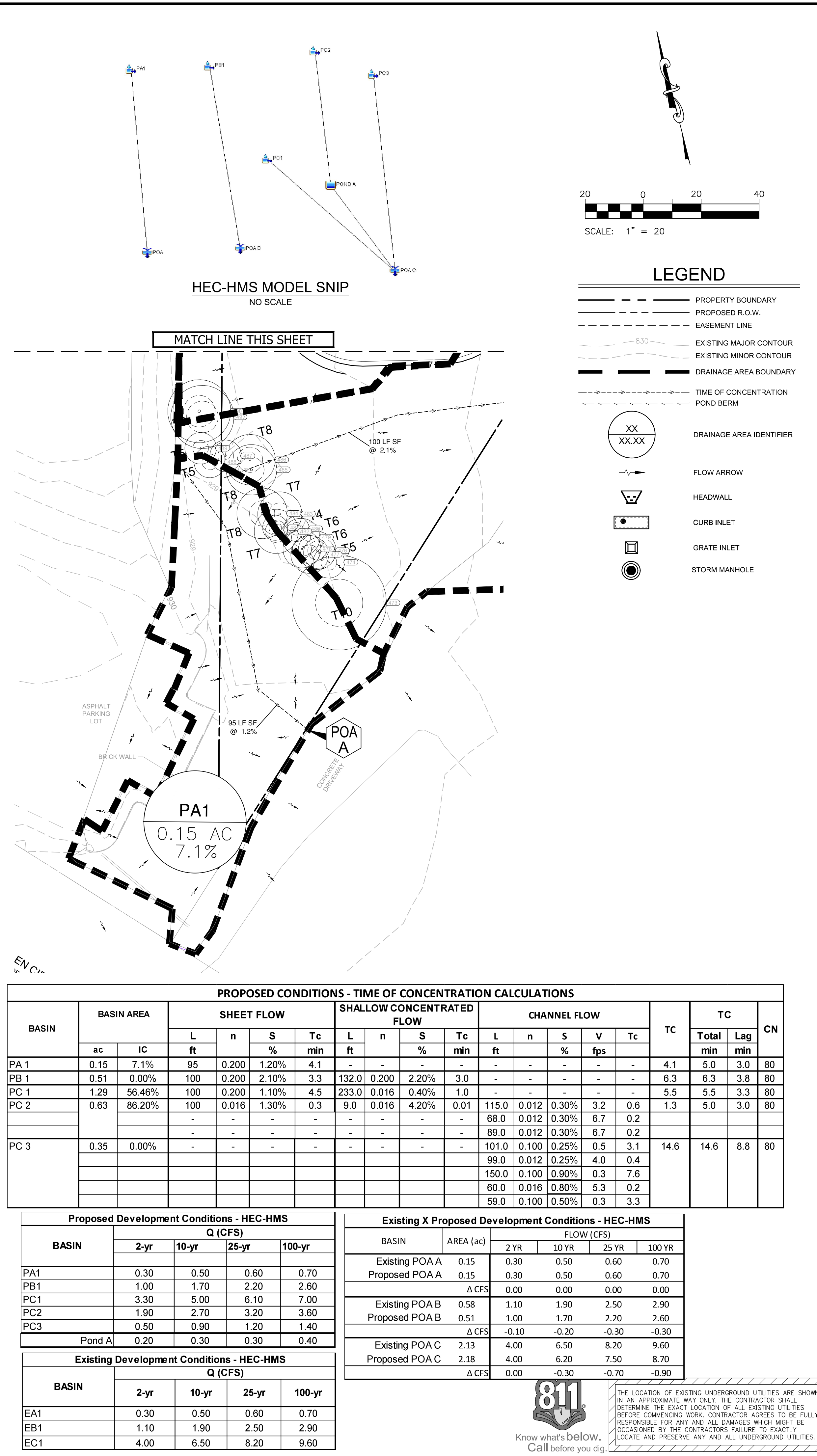
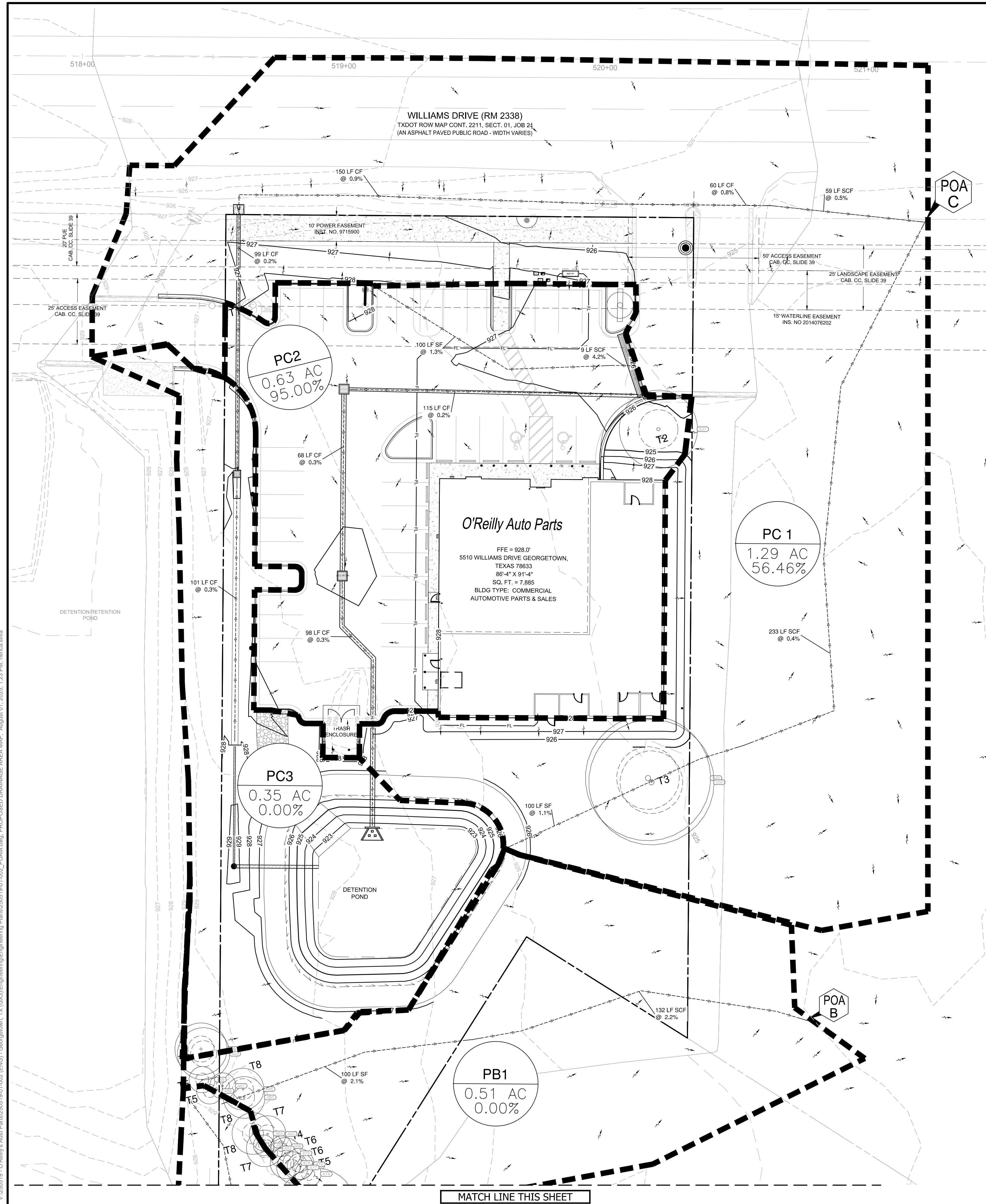
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13 OF 25

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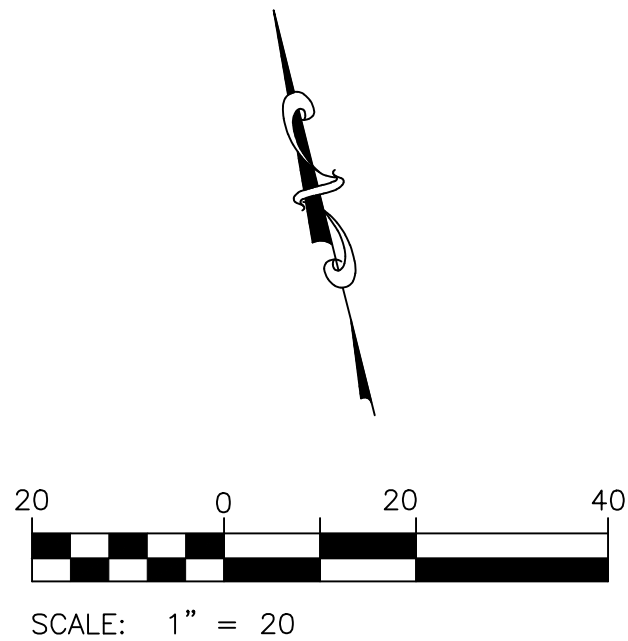
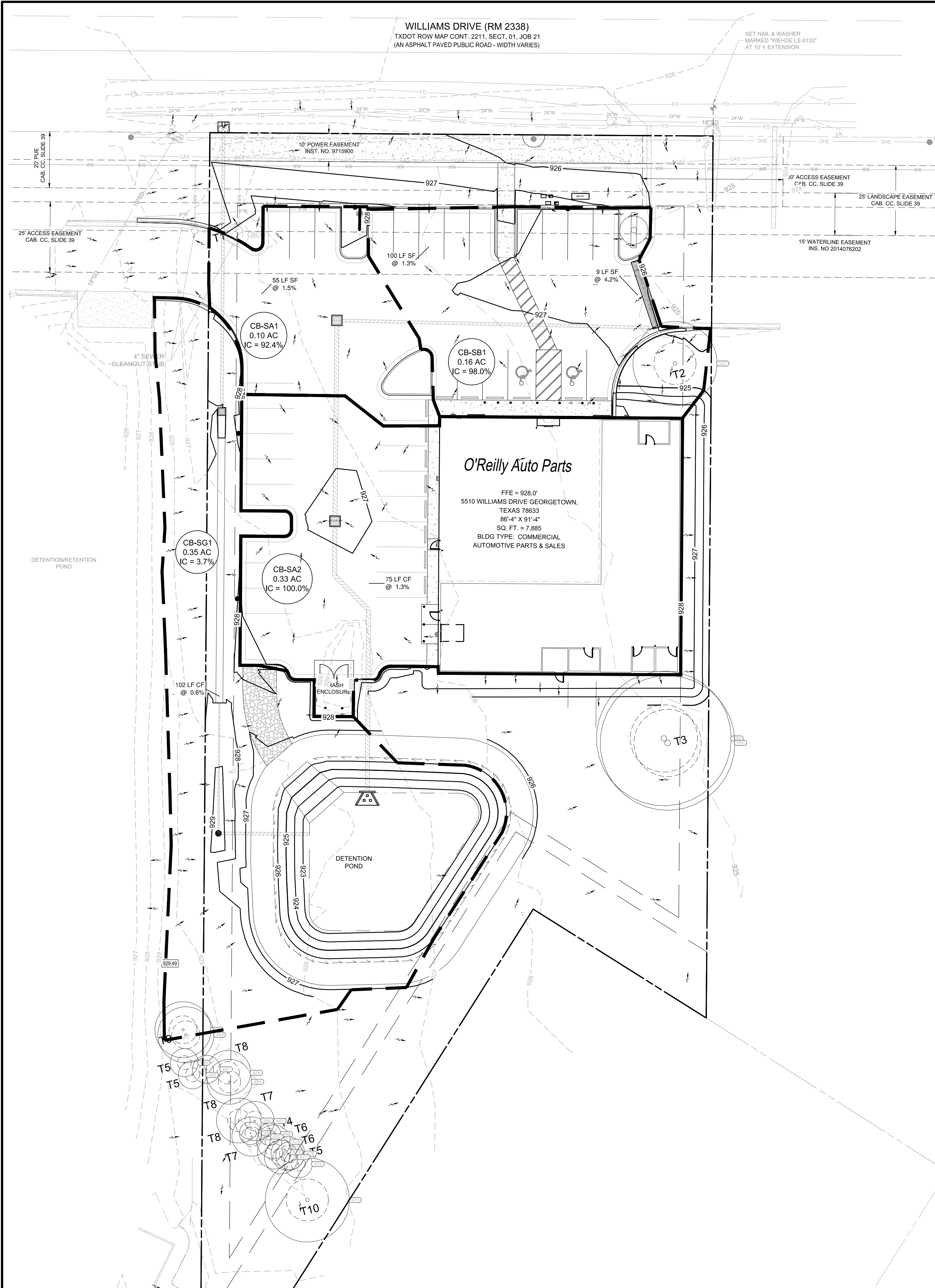
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LEGEND

- PROPERTY BOUNDARY
- PROPOSED R.O.W.
- EASEMENT LINE
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- DRAINAGE AREA BOUNDARY
- TIME OF CONCENTRATION
- POND BERM
- DRAINAGE AREA IDENTIFIER
- FLOW ARROW
- HEADWALL
- CURB INLET
- GRATE INLET
- STORM MANHOLE

O'REILLY'S - PROPOSED CONDITIONS					
BASIN NAME	TOTAL AREA (SF)	TOTAL AREA (AC)	CONCRETE (SF)	GRASS (SF)	IMPERVIOUS COVER (%)
CB-SA1	4406	0.10	4070	336	92.4
CB-SA2	14584	0.33	14584	0	100.0
CB-SB1	7172	0.16	7028	172	98.0
CB-SG1	15179	0.35	563	14616	3.7

O'REILLY INLET CALCULATIONS - 25 YR STORM											
Inlet ID	Drainage Area	Inlet Type	Length of Inlet (ft)	Q25 (cfs)	Peak Flow Including Bypass	Peak Flow Intercepted by Inlet	Peak Flow Bypassing Inlet	Inlet Accepting Bypass Flow	Depth in Gutter (ft)	Ponding Width (ft)	Inlet Efficiency
CB-SA1	0.10	3'x3' GRATE INLET	3	0.97	0.96	2.49	0.00	No	0.10	0.13	100%
CB-SA2	0.33	2'x2' GRATE INLET	2.5	3.20	3.17	5.50	0.00	No	0.30	0.20	100%
CB-SB1	0.16	2' X 25' TRENCH D.	25	1.57	1.54	1.54	0.00	No	0.10	0.36	100%
CB-SG1	0.35	30"x30" GRATE INLET	2.5	3.33	3.37	3.37	0.00	No	0.40	0.47	100%

O'REILLY INLET CALCULATIONS - 100 YR STORM											
Inlet ID	Drainage Area	Inlet Type	Length of Inlet (ft)	Q100 (cfs)	Peak Flow Including Bypass	Peak Flow Intercepted by Inlet	Peak Flow Bypassing Inlet	Inlet Accepting Bypass Flow	Depth in Gutter (ft)	Ponding Width (ft)	Inlet Efficiency
CB-SA1	0.10	3'x3' GRATE INLET	3	1.17	1.16	2.97	0.00	No	0.20	0.14	100%
CB-SA2	0.33	2'x2' GRATE INLET	2.5	3.86	3.83	6.66	0.00	No	0.40	0.22	100%
CB-SB1	0.16	2' X 25' TRENCH D.	25	1.88	1.86	1.86	0.00	No	0.10	0.38	100%
CB-SG1	0.35	30"x30" GRATE INLET	2.5	4.02	4.07	4.07	0.00	No	0.40	0.51	100%

O'REILLY PIPE CALCULATIONS - STORMCAD					
Conduit Label	Diameter (in)	25-YR		100-YR	
		Flow (cfs)	Velocity (fps)	Flow (cfs)	Velocity (fps)
SS C1	12	3.37	4.29	4.07	5.18
SS 1	12	1.54	3.40	1.86	3.55
SS 2	18	2.45	3.51	2.97	3.69
SS 3	18	5.50	3.73	6.66	3.77
SS 4	18	5.49	3.54	6.65	3.76
SS 5	18	5.47	3.64	6.63	3.75



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INLET DRAINAGE AREA MAP

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



DESIGN	DRAWN	CHKD
MC	LB	MK
JOB No. 250519-01-002		
SHEET 15 OF 25		

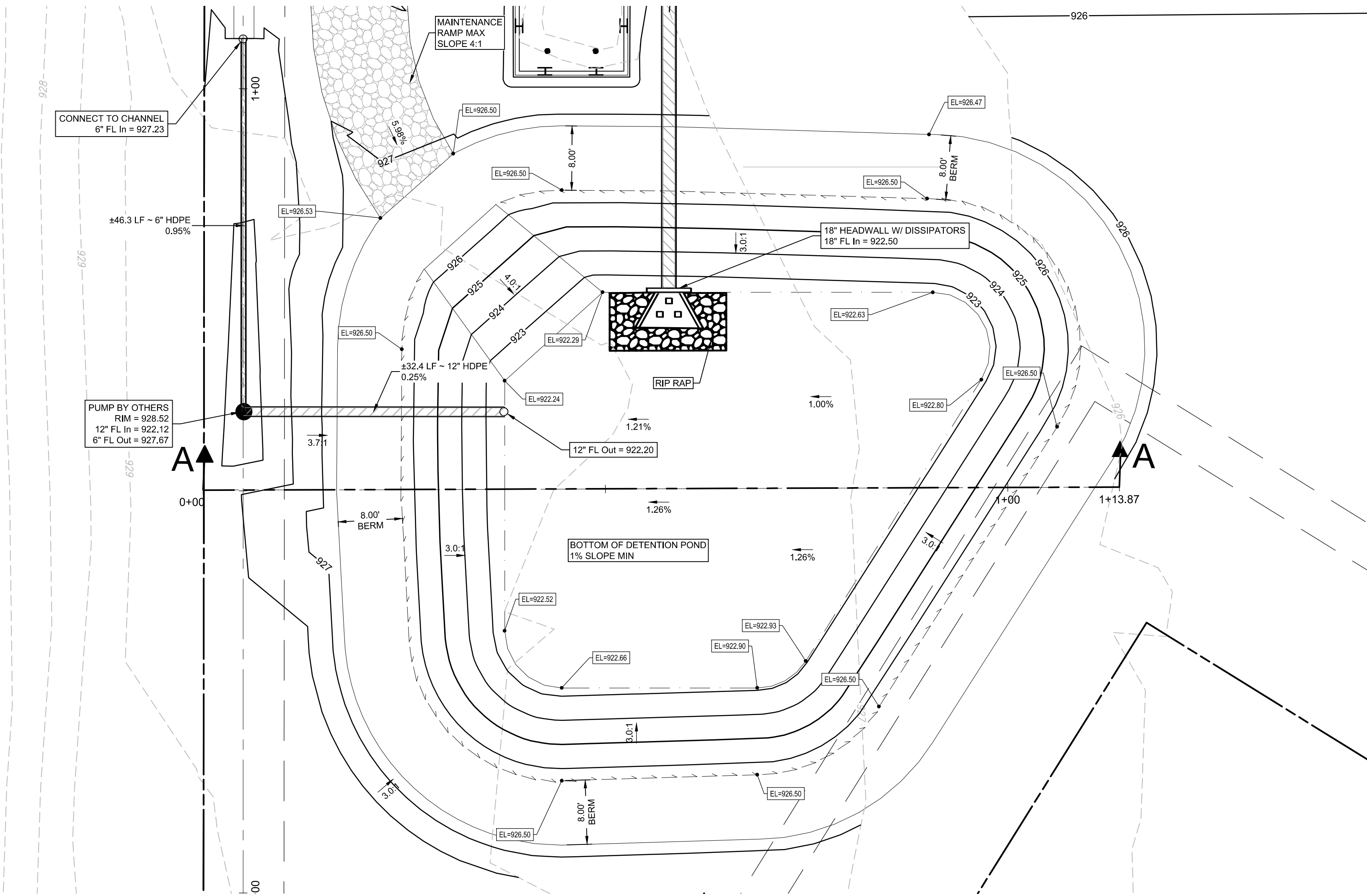
REVISIONS

REVISION	DATE	DESCRIPTION

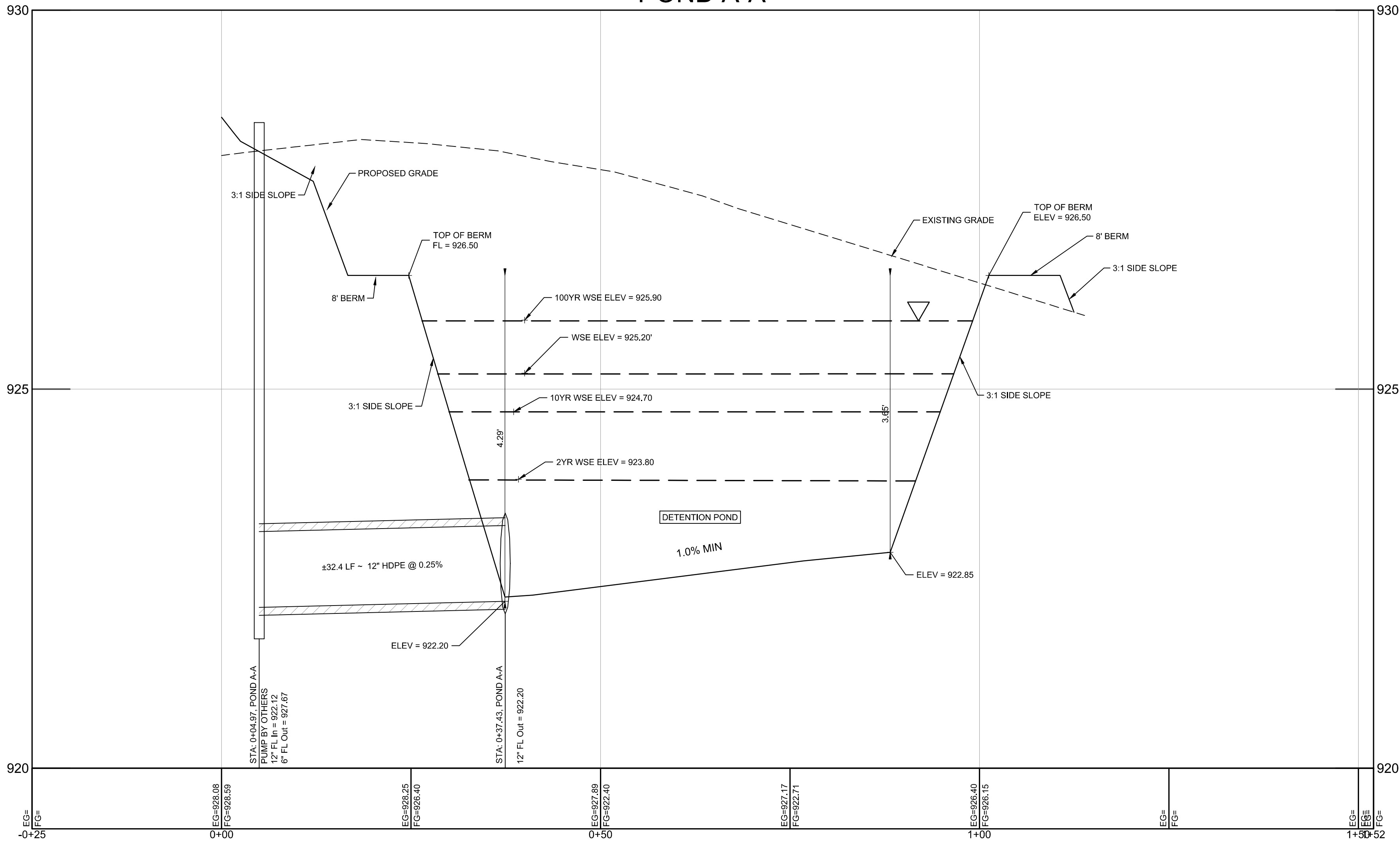
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POND A-A



Contech Engineered Solutions Calculations for Texas Commission on Environmental Quality
TSS Removal Calculations

Project Name: O'Reilly - Georgetown
Date Prepared: 7/17/2025

1. The Required Load Reduction for the total project:

Calculations from RG-348
Pages 3-27 to 3-30

Page 3-29 Equation 3.3: $L_M = 28.9(A_N \times P)$

$L_{M \text{ TOTAL PROJECT}}$ = Required TSS removal resulting from the proposed development = 85% of increased load
 A_N = Net increase in impervious area for the project
P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County = Williamson
Total project area included in plan * = 1.14 acres
Predevelopment impervious area within the limits of the plan * = 0.09 acres
Total post-development impervious area within the limits of the plan* = 0.68 acres
Total post-development impervious cover fraction * = 0.60
P = 32 inches

$L_{M \text{ TOTAL PROJECT}}$ = 546 lbs.

Number of drainage basins / outfalls areas leaving the plan area = 5

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. = PC1
Total drainage basin/outfall area = 0.63 acres
Predevelopment impervious area within drainage basin/outfall area = 0.06 acres
Post-development impervious area within drainage basin/outfall area = 0.54 acres
Post-development impervious fraction within drainage basin/outfall area = 0.86
 $L_{M \text{ THIS BASIN}}$ = 444 lbs.

3. Indicate the proposed BMP Code for this basin.

Proposed BMP = JF abbreviation
Removal efficiency = 86 percent

4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.

RG-348 Page 3-33 Equation 3.7:
 $LR = (\text{BMP efficiency}) \times P \times (A_i \times 34.6 + A_p \times 0.54)$

A_C = Total On-Site drainage area in the BMP catchment area
 A_I = Impervious area proposed in the BMP catchment area
 A_P = Pervious area remaining in the BMP catchment area
 L_R = TSS Load removed from this catchment area by the proposed BMP

A_C = 0.63 acres
 A_I = 0.54 acres
 A_P = 0.09 acres
 L_R = 516 lbs.

5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area

Desired $L_{M \text{ THIS BASIN}}$ = 465 lbs.
F = 0.90

6. Calculate Treated Flow required by the BMP Type for this drainage basin / outfall area.

Offsite area draining to BMP = 0.04 acres
Offsite impervious cover draining to BMP = 0.04 acres

Calculations from RG-348
Pages Section 3.2.22

Rainfall Intensity = 1.10 inches per hour
Effective Area = 0.52 acres
Cartridge Length = 54 inches

Peak Treatment Flow Required = 0.58 cubic feet per second

7. Jellyfish

Designed as Required in RG-348
Section 3.2.22

Flow Through Jellyfish Size	Vault
Jellyfish Size for Flow-Based Configuration =	JFPD0406-3-1
Jellyfish Treatment Flow Rate =	0.62 cfs

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WATER QUALITY & DETENTION POND (2 OF 2)

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



DESIGN MC DRAWN LB CHKD MK

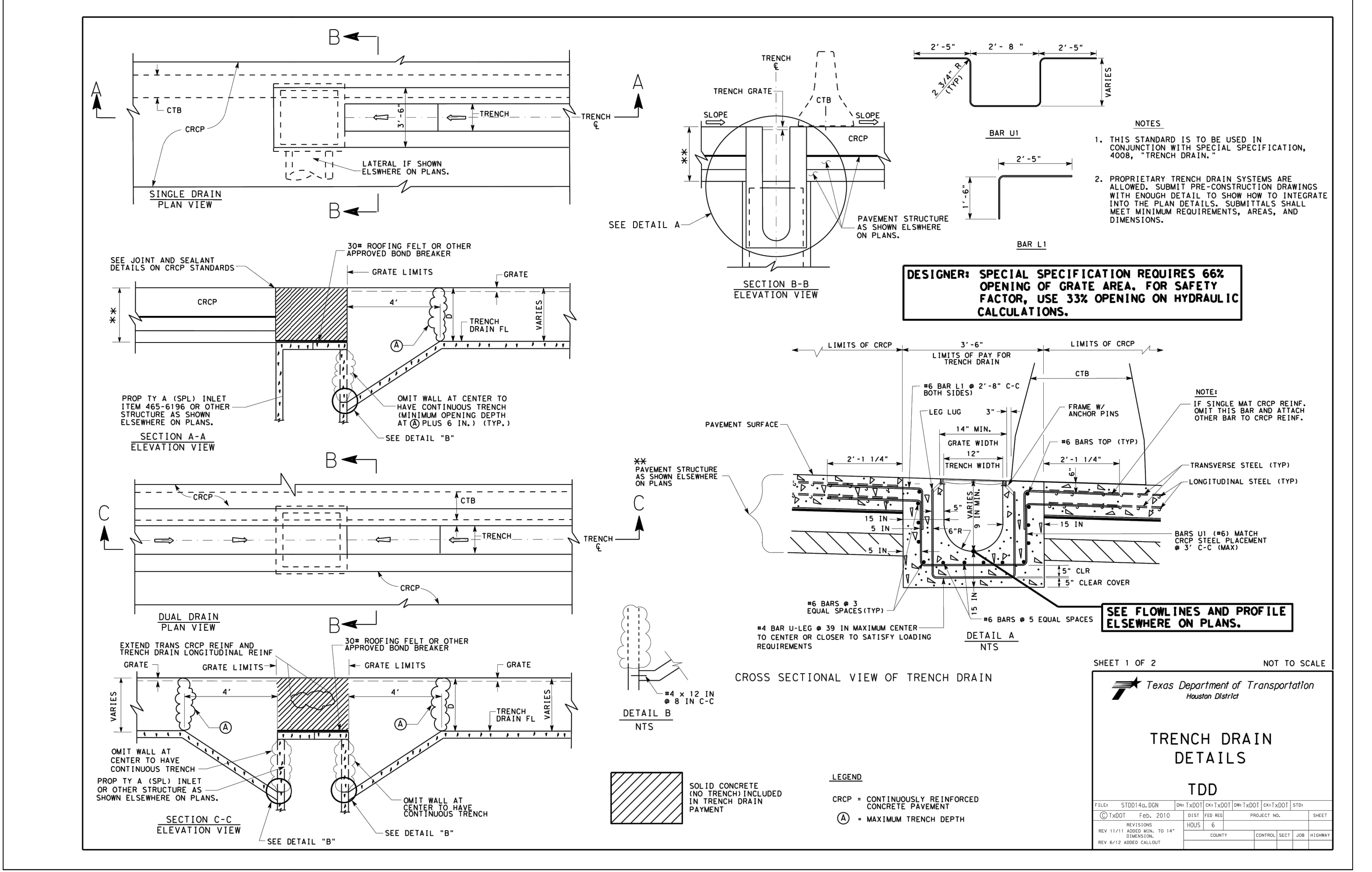
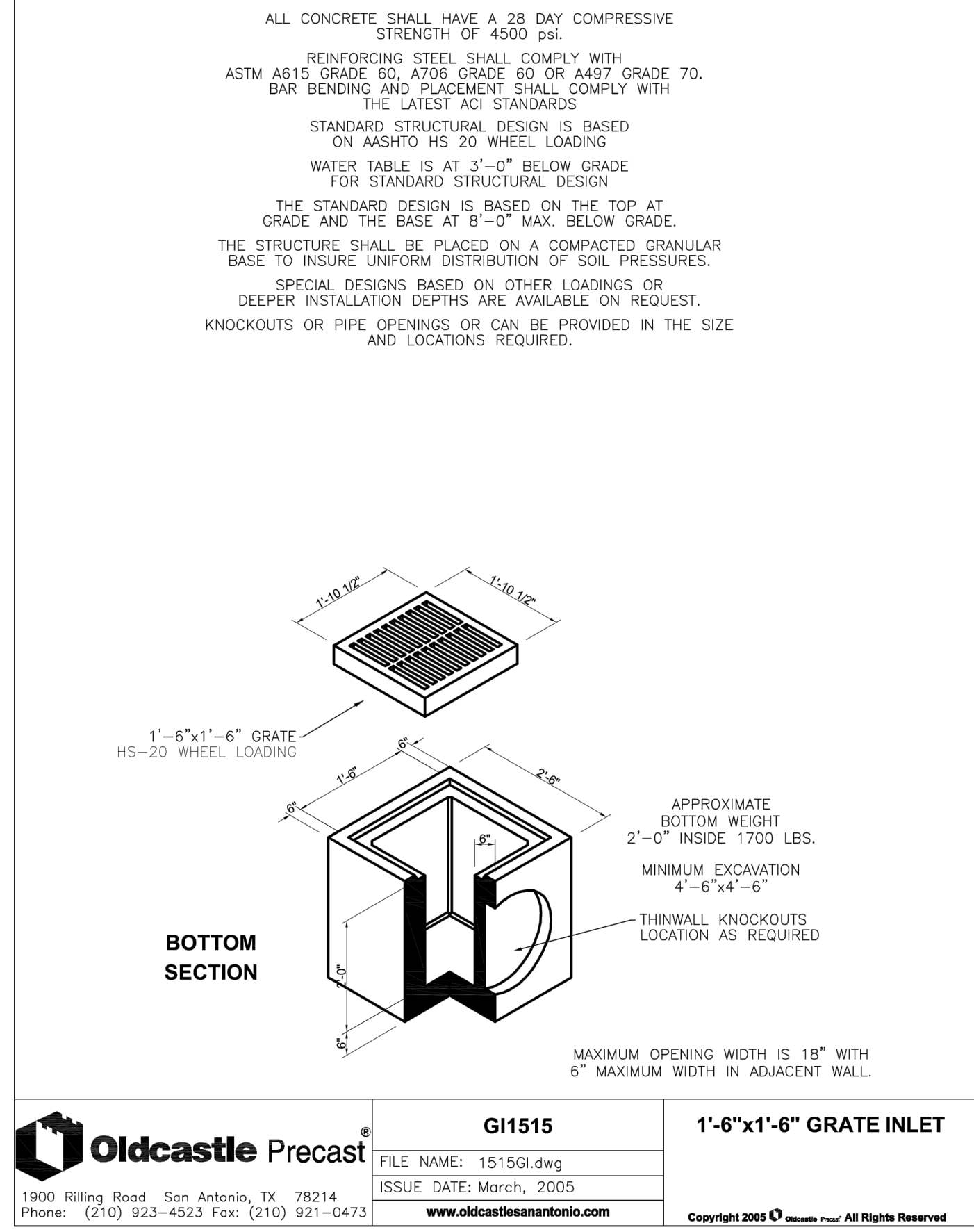
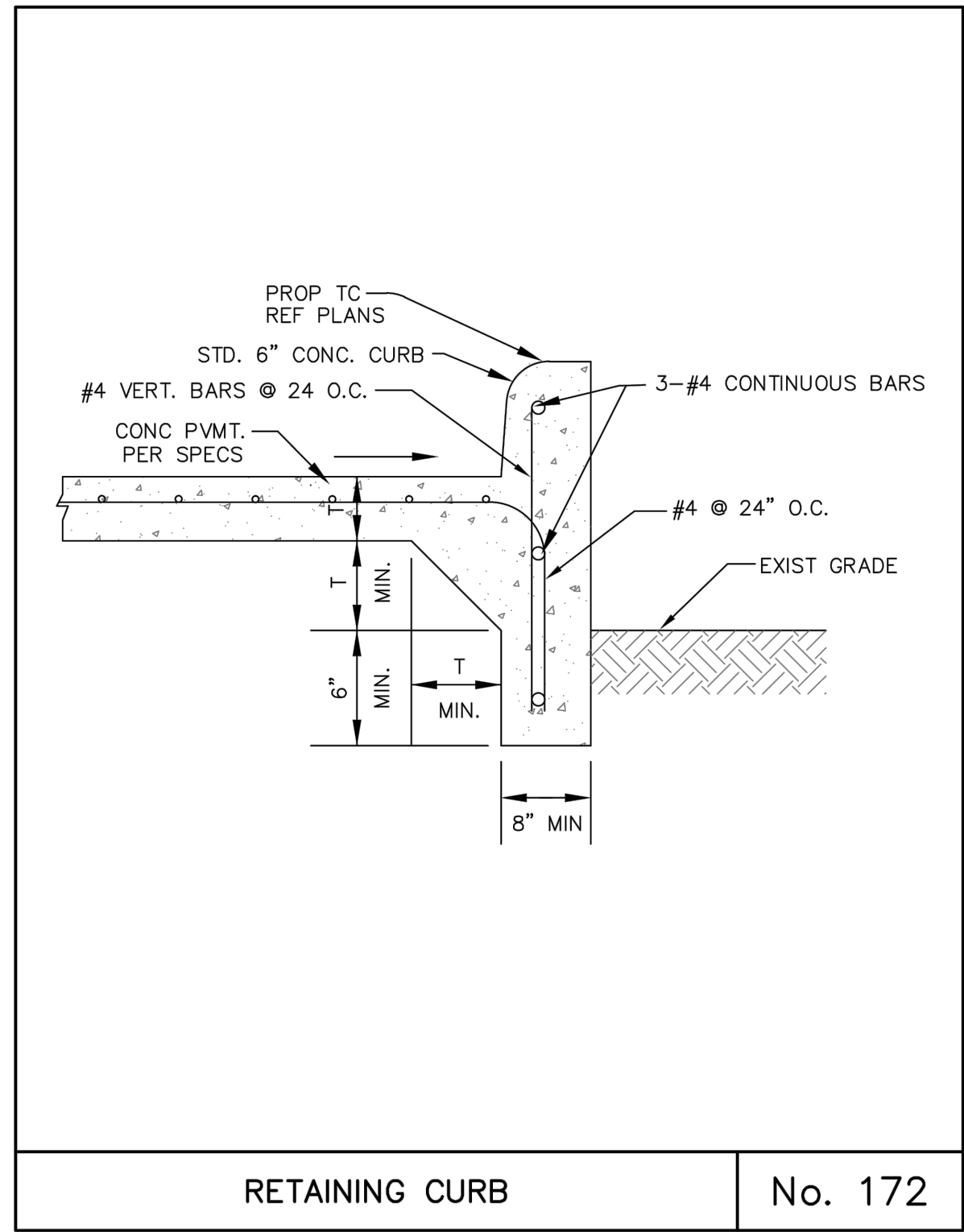
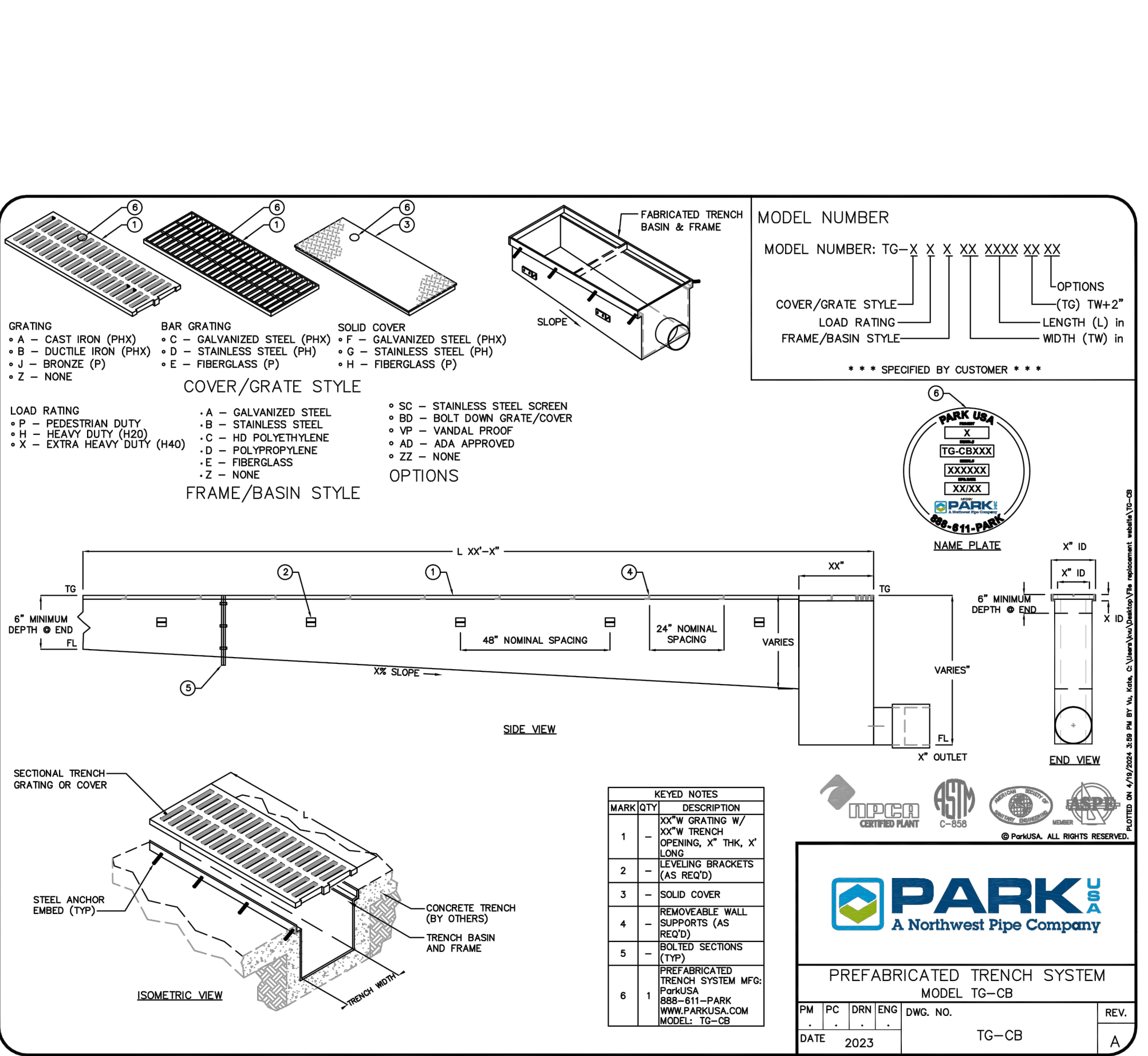
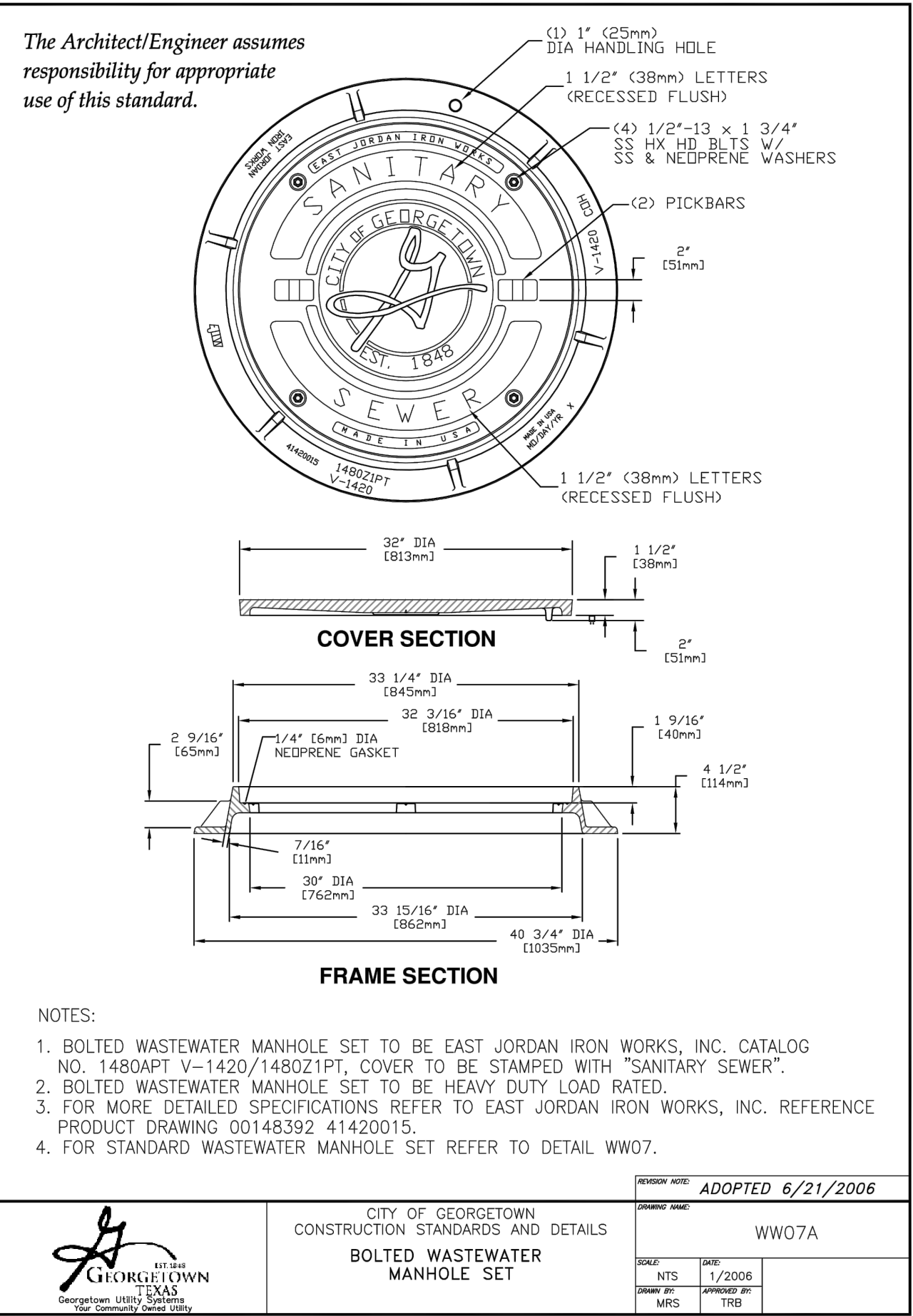
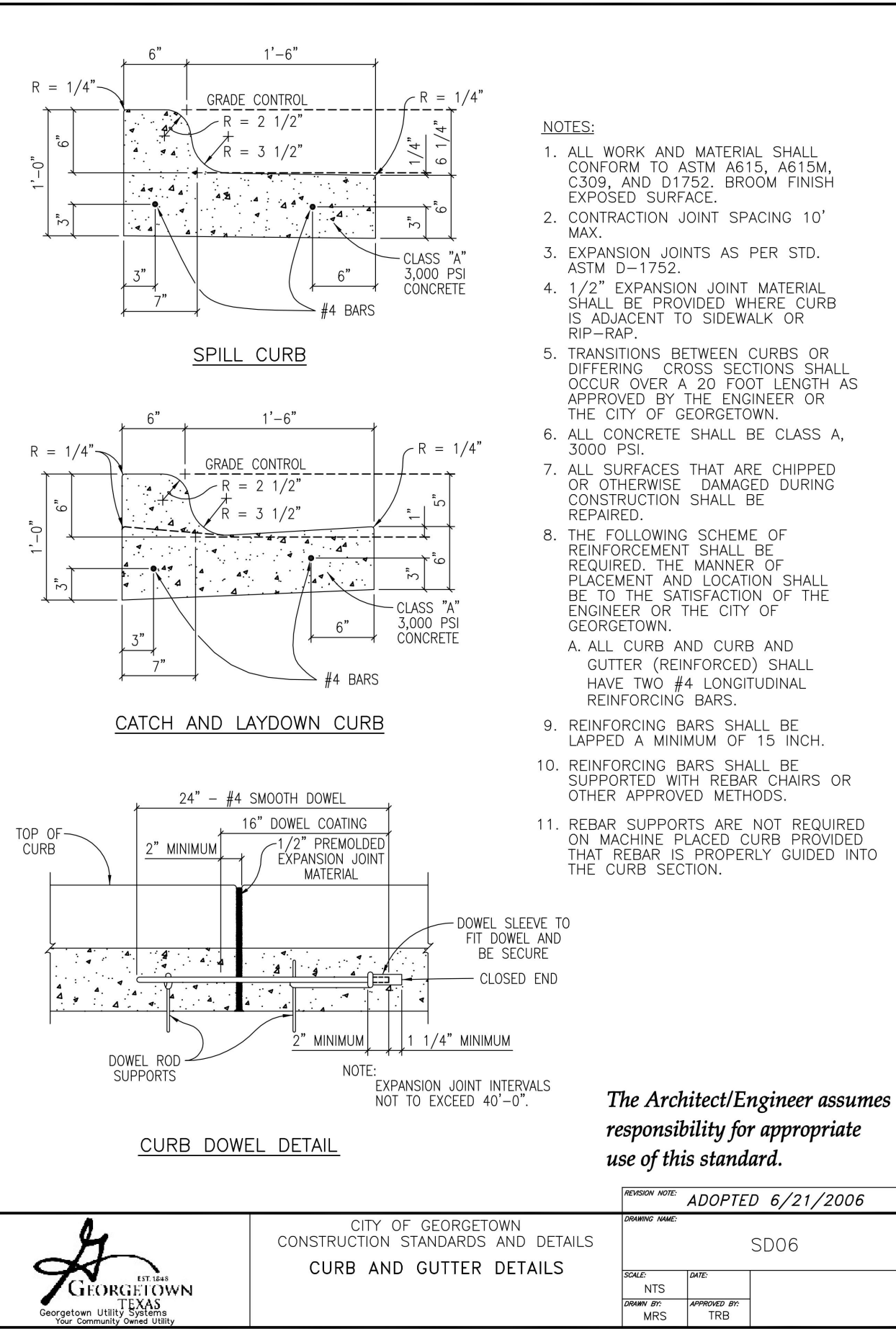
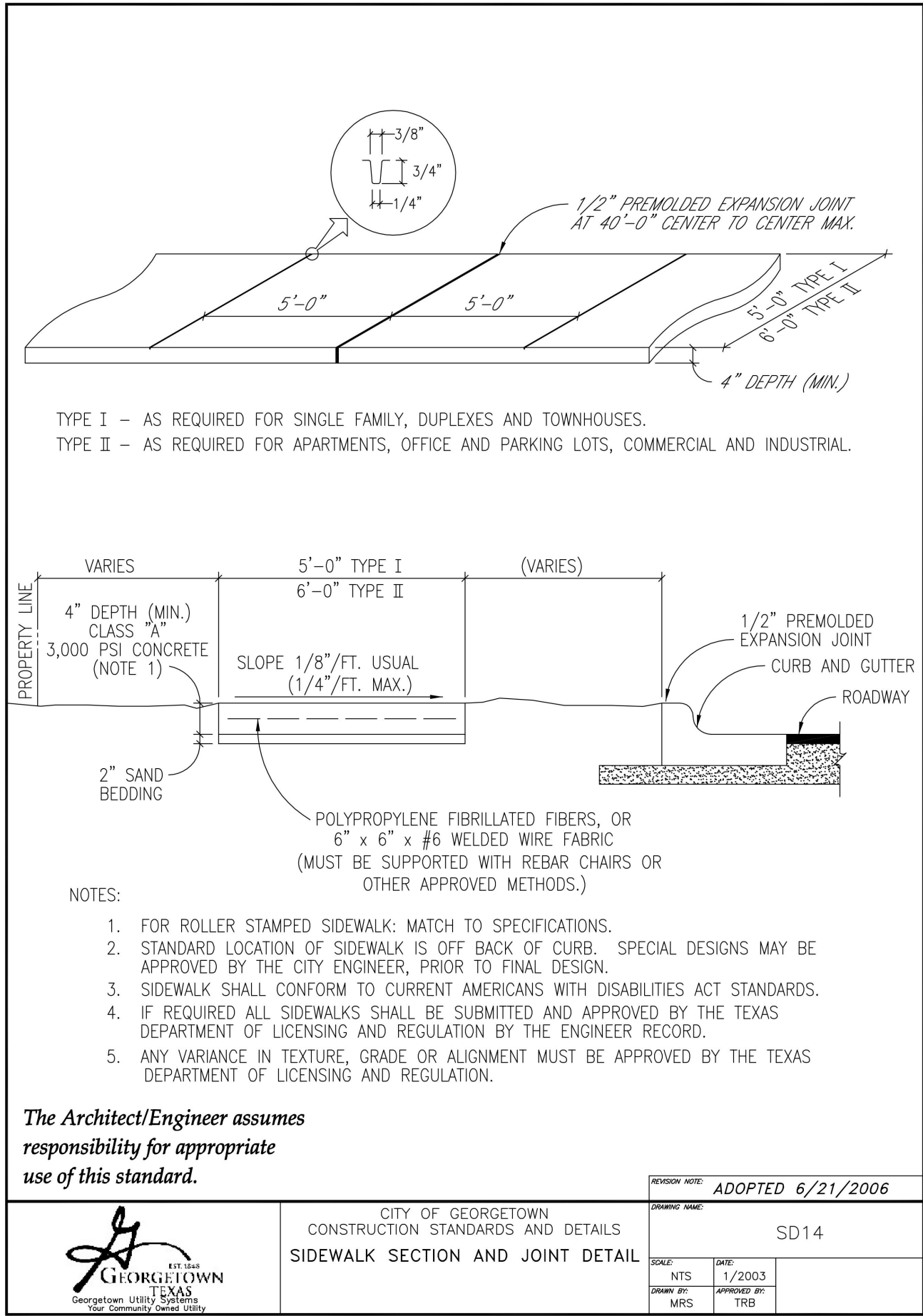
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17 OF 25



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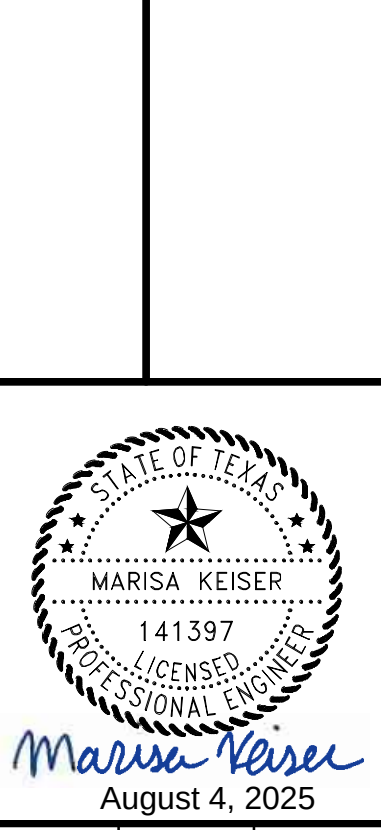
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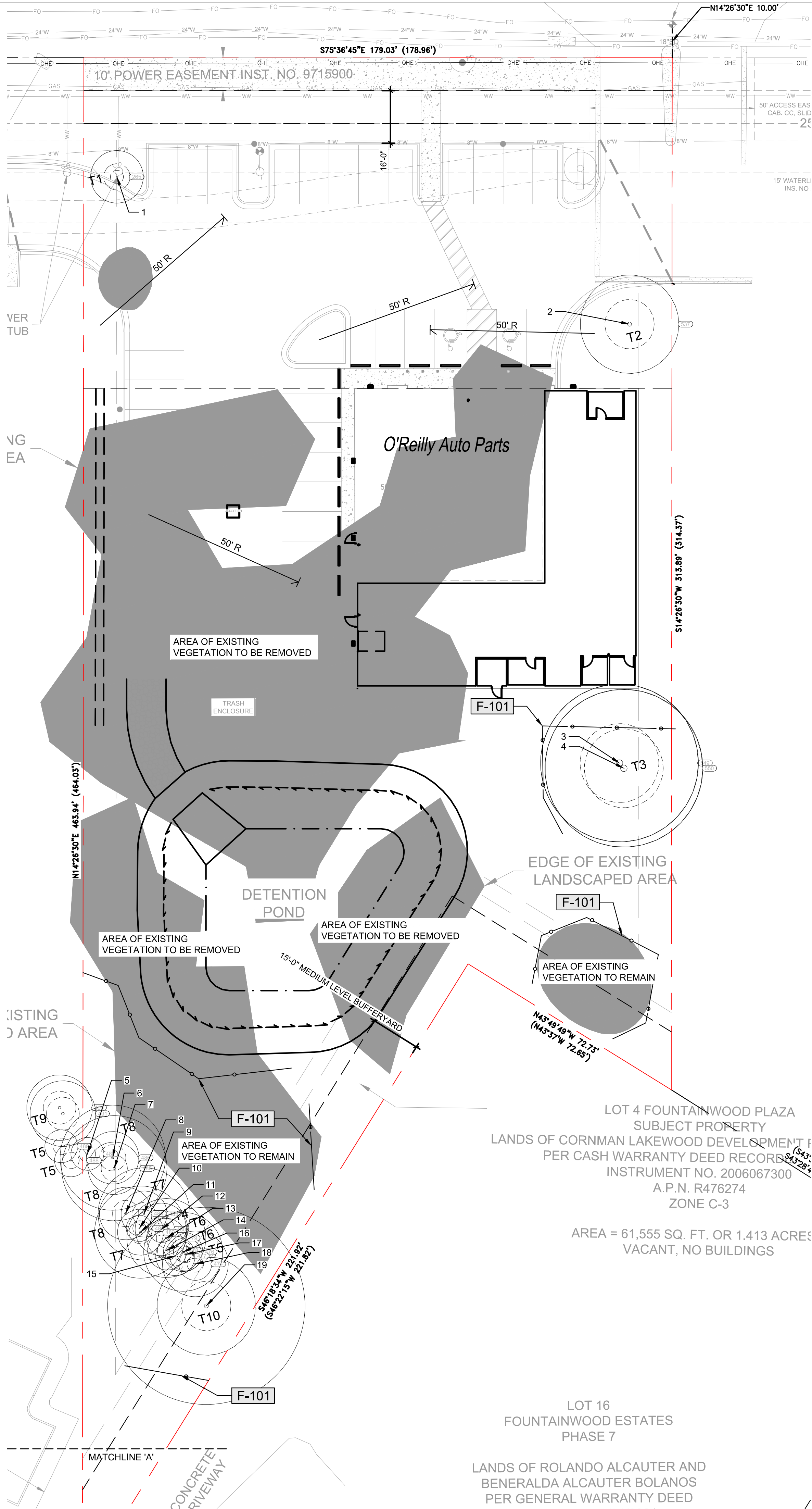
REVISION	DATE	DESCRIPTION

CONSTRUCTION DETAILS
O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



DESIGN	DRAWN	CHKD
MC	LB	MK
JOB No. 250519-01-002		
SHEET 19 OF 25		

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EXISTING ON-SITE TREE TABLE					
NO.	CATEGORY	DIAMETER"	TYPE	CLASSIFICATION	STATUS
1	T1	8	EVERGREEN	N/A	REMOVE
2	T2 (CLUSTER)	30	EVERGREEN	HERITAGE	REMAIN
3	T3 (CLUSTER)	24	EVERGREEN	PROTECTED	REMAIN
4	T3	24	EVERGREEN	PROTECTED	REMAIN
5	T5	10	DECIDUOUS	N/A	REMAIN
6	T8	16	DECIDUOUS	PROTECTED	REMAIN
7	T8	16	DECIDUOUS	PROTECTED	REMAIN
8	T8	16	DECIDUOUS	PROTECTED	REMAIN
9	T7	14	DECIDUOUS	PROTECTED	REMAIN
10	T4	8	DECIDUOUS	N/A	REMAIN
11	T7	14	DECIDUOUS	PROTECTED	REMAIN
12	T4	8	DECIDUOUS	N/A	REMAIN
13	T6	12	DECIDUOUS	PROTECTED	REMAIN
14	T6	12	DECIDUOUS	PROTECTED	REMAIN
15	T6	12	DECIDUOUS	PROTECTED	REMAIN
16	T6	12	DECIDUOUS	PROTECTED	REMAIN
17	T5	10	DECIDUOUS	N/A	REMAIN
18	T5	10	DECIDUOUS	N/A	REMAIN
19	T10	30	DECIDUOUS	HERITAGE	REMAIN

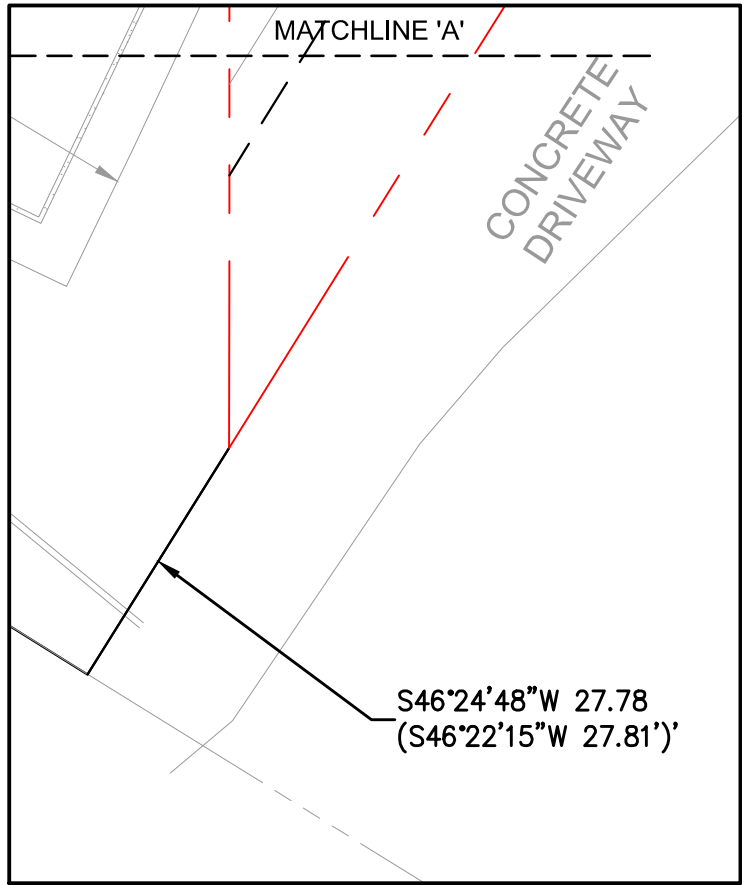
TOTAL CALIPER IN. ON SITE: 286
TOTAL CALIPER IN. HERITAGE TREES: 60
TOTAL CALIPER IN. PROTECTED TREES: 172

TOTAL CALIPER IN. REMOVED: 8
TOTAL CALIPER IN. HERITAGE TREES REMOVED: 0

TOTAL CALIPER IN. REMAINING: 278
TOTAL CALIPER IN. PROTECTED TREES REMAIN: 172 (100%)

NO HERITAGE TREES ARE REMOVED. NO MITIGATION IS
REQUIRED FOR THE 8 CALIPER INCHES THAT ARE REMOVED.

REFERENCE NOTES SCHEDULE		
FENCE		
F-101	TREE PRESERVATION FENCING	292 lf



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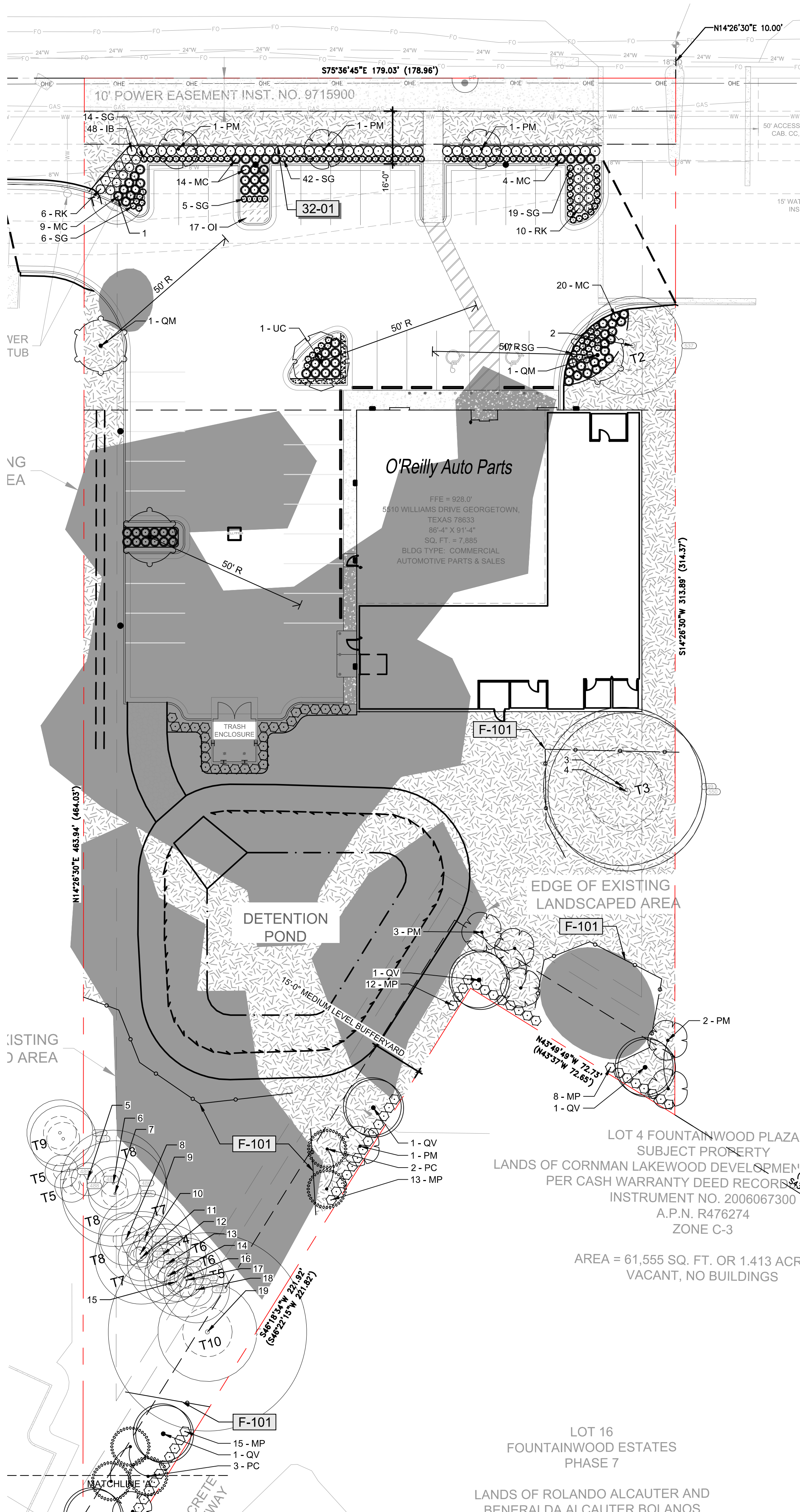
TREE PRESERVATION PLAN
O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



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PLANT SCHEDULE

SYMBOL	CODE	QTY	COMMON / BOTANICAL NAME	CONT.	CAL.	HT.	SP.	BR. HT.
TREES								
	PC	5	Carolina Cherry Laurel / Prunus caroliniana	30 gal	2"Cal	6'-8'	4'-5'	4'-5'
	PM	10	Mexican Plum / Prunus mexicana	30 gal	2"Cal	6'-8'	4'-5'	4'-5'
	QM	3	Burr Oak / Quercus macrocarpa	45 gal	3"Cal	10'-12'	5'-6'	5'+
	QV	6	Southern Live Oak / Quercus virginiana	45 gal	3"Cal	10'-12'	5'-6'	5'+
	UC	1	Cedar Elm / Ulmus crassifolia	45 gal	3"Cal	10'-12'	5'-6'	5'+

SYMBOL	CODE	QTY	COMMON / BOTANICAL NAME	CONT.	HT.	O.C.
SHRUBS						
	IB	46	Dwarf Burford Holly / Ilex cornuta 'Burfordii Nana'	5 gal	12"	36"
	MC	69	Pink Muhly Grass / Muhlenbergia capillaris	5 gal	10"	36"
	MP	80	Dwarf Wax Myrtle / Myrica cerifera 'Pumila'	5 gal	12"	36"
	RK	16	Knock Out Rose / Rosa shrub 'Knock Out'	5 gal	12"	36"
	SG	109	Autumn Sage / Salvia greggii	3 gal	10"	24"

GROUND COVERS						
	CT SOD	22,375 sf	Bermuda Grass / Cynodon dactylon 'Tif 419'	sod		
	OI	42	Aztec Grass / Ophiopogon intermedius	1 gal	8"	18"

REFERENCE NOTES SCHEDULE

CODE	DESCRIPTION	QTY
32-01	STEEL BED EDGE	340 lf
FENCE		
F-101	TREE PRESERVATION FENCING	292 lf
K-102	DECOMPOSED GRANITE	105 sf

REQUIREMENTS CALCULATIONS:

STREET YARDS BETWEEN 10,000 AND 100,000 SF:
1. LANDSCAPE AREA TOTALING 20% OF THE STREET YARD
2. FOUR SHADE TREES AND 12 SHRUBS PER THE FIRST 10,000 SF OF STREET YARD, PLUS ONE AND ONE-HALF SHADE TREES AND FOUR SHRUBS PER EVERY ADDITIONAL 10,000 SQUARE FEET OF STREET YARD THEREAFTER.

TOTAL STREET YARD AREA: 17,991 SF

17,991 SF X .2 = 3,598 SF LANDSCAPE AREA REQUIRED
4,458 SF LANDSCAPE AREA PROVIDED IN STREET YARD

17,991 SF - 10,000 = 7,991 SF
7,991 / 10,000 = .7991
.7991 X 1.5 = 1.1 ~ 2 TREES
.7991 X 4 = 3.1 ~ 4 SHRUBS

4 TREES + 2 TREES = 6 FRONTAGE TREES REQUIRED
12 SHRUBS + 4 SHRUBS = 16 FRONTAGE SHRUBS REQUIRED

6 FRONTAGE TREES AND 124 SHRUBS PROVIDED

LANDSCAPE AREA: 20 SF FOR EACH FRONTAGE PARKING STALL

20 PARKING STALLS IN THE FRONT YARD.

20 X 20 SF = 400 SF REQUIRED

615 SF LANDSCAPE AREA PROVIDED

LANDSCAPE AREA: 10 SF EACH ADDITIONAL PARKING STALL

14 PARKING STALLS X 10 = 140 SF LANDSCAPE AREA REQ.

159 SF LANDSCAPE AREA PROVIDED

SHADE TREES
1 SHADE TREE PER EVERY 12 PARKING STALLS

34 PARKING STALLS TOTAL / 12 = 2.8 ~ 3 SHADE TREES REQUIRED

3 SHADE TREES PROVIDED

BUFFER YARD LEVEL: DISTRICT C3 ADJACENT TO TF = TYPE C BUFFER - MEDIUM

294 LF

15' WIDE PLANTING AREA PROVIDED

ONE SHADE TREE, TWO EVERGREEN ORNAMENTAL TREES, AND EIGHT EVERGREEN SHRUBS PER EACH 50 LF OF BUFFERYARD.

294 LF / 50 = 5.88

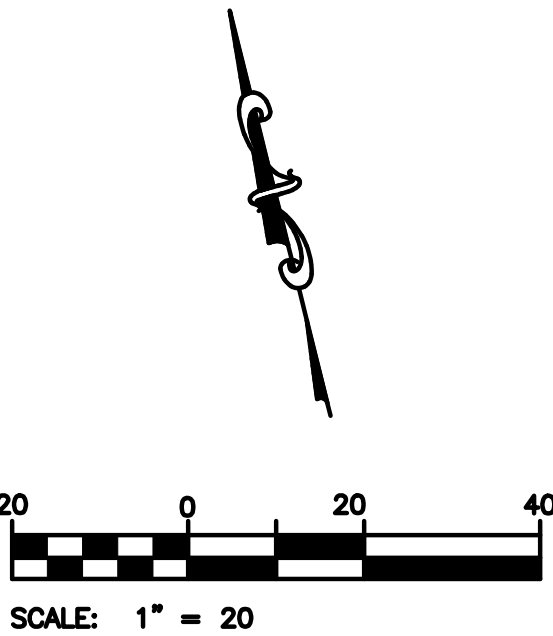
5.88 ~ 6 SHADE TREES REQUIRED
5.88 X 2 = 11.7 ~ 12 EVERGREEN ORNAMENTAL TREES REQUIRED
5.88 X 8 = 47 EVERGREEN SHRUBS REQUIRED

6 SHADE TREES, 12 EVERGREEN ORNAMENTAL TREES, AND 48 EVERGREEN SHRUBS PROVIDED

STATEMENT OF COMPLIANCE
THE PLANS SUBMITTED FOR APPROVAL COMPLY WITH
THE REQUIREMENTS OF THE GEORGETOWN, TX
UNIFIED DEVELOPMENT CODE CHAPTER 8.



THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.



LANDSCAPE NOTES

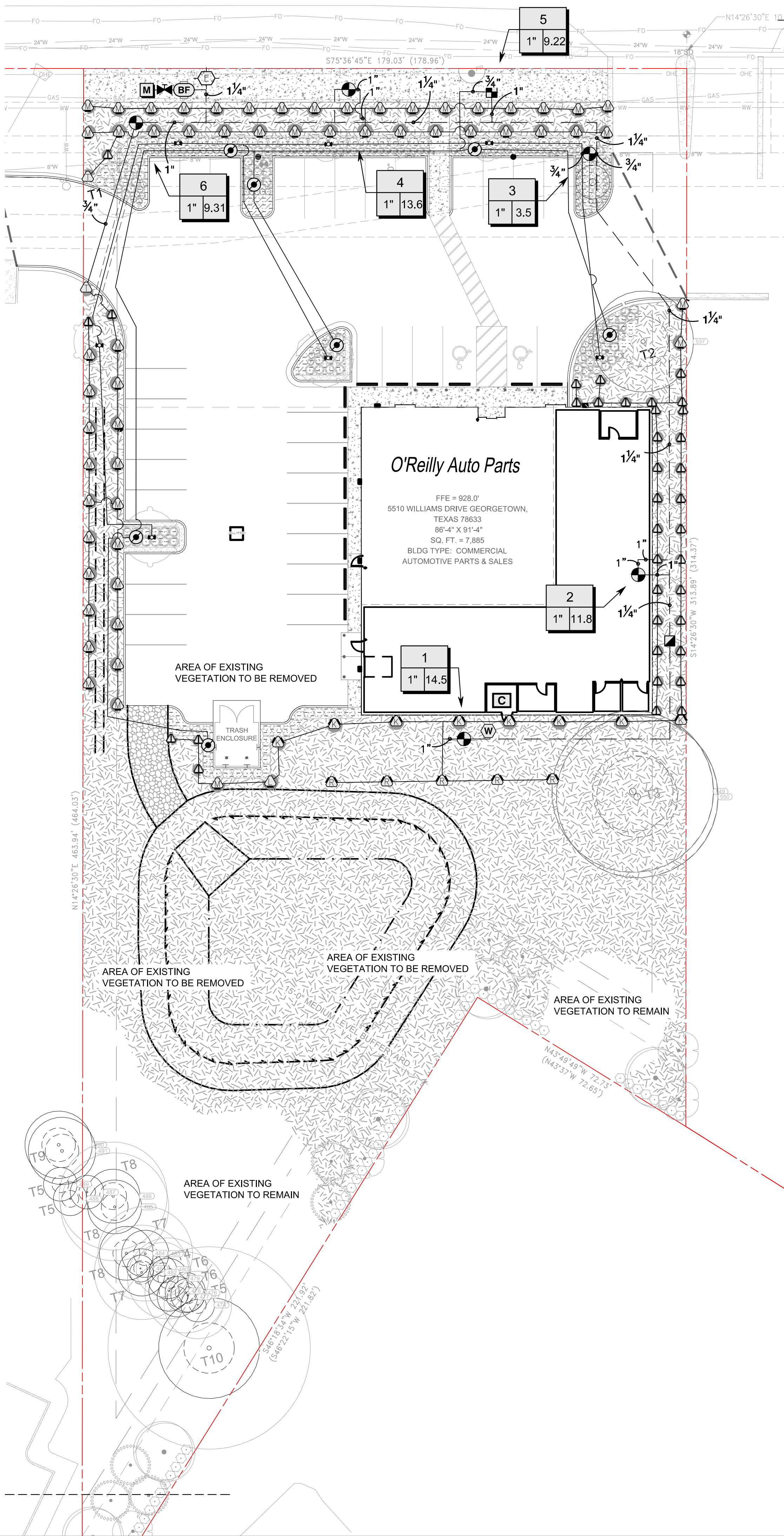
- ALL MATERIAL QUANTITIES ARE FOR THE CONVENIENCE OF THE CONTRACTOR ONLY. CONTRACTOR SHALL VERIFY ALL QUANTITIES.
- CONTRACTOR SHALL HAVE ALL UTILITY LINES LOCATED BEFORE PERFORMING ANY EXCAVATION FOR LANDSCAPE AND/OR IRRIGATION INSTALLATION. CONTRACTOR SHALL TAKE NECESSARY STEPS TO PROTECT EXISTING UTILITIES.
- PRIOR TO APPLYING SOD OR HYDROSEED, ALL STONES AND DEBRIS LARGER THAN 1 INCH IN ANY DIMENSION SHALL BE REMOVED AND SURFACE MUST BE TOP DRESSED WITH 2" OF TOPSOIL AND FINE GRADED SMOOTH.
- CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING A FULL AND HEALTHY STAND OF GRASS AT THE TIME OF POSSESSION BY THE OWNER.
- ALL AREAS DISTURBED BY CONSTRUCTION (INCLUDING AREAS OUTSIDE PROPERTY LINES) SHALL BE PLANTED AND WATERED A MINIMUM OF 10 WEEKS OR UNTIL GRASS IS FULLY ESTABLISHED.
- LANDSCAPE BED EDGING SHALL BE PROVIDED WHERE INDICATED ON PLANS AND BETWEEN PLANTING/STONE BEDS AND LAWN AREAS/SIDEWALKS.
- OPEN AREAS WITHIN LANDSCAPE BEDS SHALL BE MULCHED.
- CONTRACTOR IS RESPONSIBLE FOR OBTAINING PHOTOGRAPHS OF ACTUAL LANDSCAPE MATERIALS AND SUBMITTING THEM TO THE LANDSCAPE ARCHITECT OF RECORD FOR APPROVAL PRIOR TO INSTALLATION.
- PLANT MATERIAL SELECTED SHALL FOLLOW THE GUIDELINES OF THE AMERICAN STANDARD FOR NURSERY STOCK BY THE AMERICAN ASSOCIATION OF NURSERYMEN AND MEET OR EXCEED ALL SIZE REQUIREMENTS LISTED ON PLANT SCHEDULE.
- ALL PLANTINGS SHALL BE MAINTAINED IN A HEALTHY STATE BY LANDSCAPE CONTRACTOR AND SHALL BE WARRANTED (FOR REPLACEMENT) FOR ONE (1) CALENDAR YEAR AFTER ACCEPTANCE OF THE LANDSCAPE INSTALLATION.

REVISIONS		DATE	DESCRIPTION
REVISION			

PLANTING PLAN
O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633

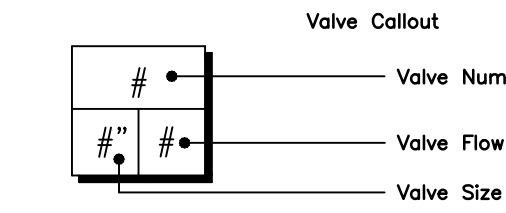


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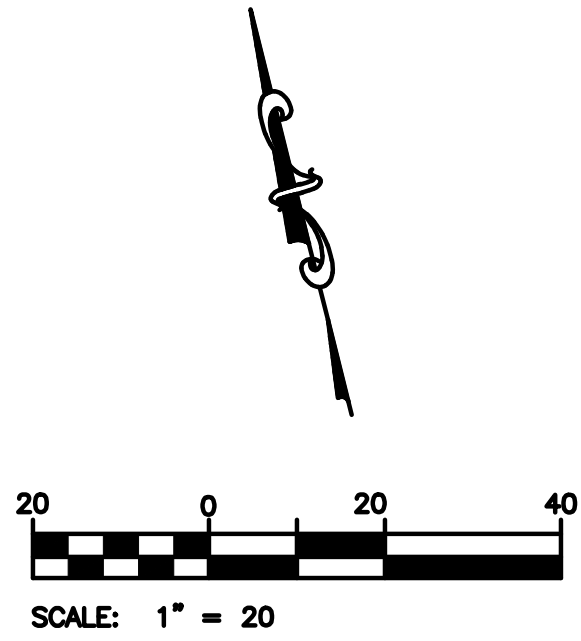
IRRIGATION SCHEDULE

SYMBOL	MANUFACTURER/MODEL/DESCRIPTION	QTY	PSI
	Hunter MP Corner PROS-04-PRS40-CV-F Turf Rotator, 4in. pop-up with factory installed check valve, foguard, pressure regulated to 40 psi, MP Rotator nozzle. T=Turquoise adj arc 45-105 on PRS40 body.	2	40
	Hunter MP1000 PROS-04-PRS40-CV-F Turf Rotator, 4in. pop-up with check valve, foguard, pressure regulated to 40 psi, MP Rotator nozzle on PRS40 body. M=Maroon adj arc 90 to 210, L=Light Blue 210 to 270 arc, O=Olive 360 arc.	57	40
	Hunter MP2000 PROS-04-PRS40-CV-F Turf Rotator, 4in. pop-up with factory installed check valve, foguard, pressure regulated to 40 psi, MP Rotator nozzle on PRS40 body. K=Black adj arc 90-210, G=Green adj arc 210-270, R=Red 360 arc.	13	40
	Hunter MP800SR PROS-04-PRS40-CV-F Turf Rotator, 4in. pop-up with check valve, foguard, pressure regulated to 40 psi, MP Rotator nozzle on PRS40 body. ADJ=Orange and Gray (arc 90-210), 360=Lime Green and Gray (arc 360)	36	40
	Hunter RZWS-SLEEVE-36-CV 36in. long RZWS with Filter Fabric Sleeve, 25 gpm or .50 gpm bubbler options, Check Valve, 1/2in. swing joint for connection to 1/2in. pipe	7	30
SYMBOL	MANUFACTURER/MODEL/DESCRIPTION	QTY	
	Hunter PCZ-101-25 Drip Control Valve Kit, 1in. PGV globe valve with 1in. HY100 filter system. Pressure Regulation: Flow range: 0.5 GPM to 15 GPM. 150 mesh stainless steel screen.	1	
	Pipe Transition Point in Drip Box Pipe transition point from PVC lateral to drip tubing with riser in 6in. drip box.	7	
	Area to Receive Dripline Hunter HDL-06-18-PC HDL-06-18-PC: Hunter Dripline with 0.6 GPH flow. Light brown tubing with gray striping. Emitters at 18" O.C. Dripline laterals spaced at 18" apart, with emitters offset for triangular pattern. Install with Hunter PLD barbed or PLD-LOC fittings.	1,383 lf	
SYMBOL	MANUFACTURER/MODEL/DESCRIPTION	QTY	
	Hunter ICV-G 1in., 1-1/2in., 2in., and 3in. Plastic Electric Remote Control Valves, Globe Configuration, with NPT Threaded Inlet/Outlet, for Commercial/Municipal Use.	5	
	Hunter HQ-33DLRC Quick coupler valve, yellow locking rubber cover, red brass and stainless steel, with 3/4in. NPT inlet, 2-piece body.	1	
	Shut Off Valve	1	
	Febco 825Y 1" Reduced Pressure Backflow Preventer	1	
	Hunter P2C-400 with (01) PCM-300 Light Commercial & Residential Controller, 7-station expanded module controller, 120 VAC, Outdoor/Indoor model	1	
	Hunter WRF-CLIK Rain/freeze Sensor, install within 1000 ft of controller, in line of sight. 22-28 VAC/VDC 100 mA power from timer transformer. Mount as noted. Includes Gutter Mount.	1	
	Strong Box SBBC-22CR Low profile, tube and wire construction smooth touch surface, cold rolled steel, backflow enclosure. 23.5in.L, 28in.H, 17.75in.W.	1	
	Water Meter 1" Reference Civil Utility Plans For Point Of Connection Location. This Plan Is For Graphical Representation Only. 65 P.S.I. Assumed Pressure.	1	
	Irrigation Lateral Line: PVC Class 200 SDR 21	2,094 lf	
	Irrigation Mainline: PVC Schedule 40	449.8 lf	
	Pipe Sleeve: PVC Schedule 40	30.9 lf	



IRRIGATION NOTES:

- IRRIGATION HAS BEEN DESIGNED WITH AN ASSUMED STATIC PRESSURE OF 60 PSI AT THE IRRIGATION METER. IF AVAILABLE PRESSURE IS LESS THAN ASSUMED NOTIFY LICENSED IRRIGATOR OF RECORD PRIOR TO SYSTEM INSTALLATION FOR DESIGN MODIFICATION INSTRUCTIONS.
- INSTALL WEATHER SENSING DEVICE AS SPECIFIED. AFFIX TO INSIDE OF PARAPET WALL ABOVE CONTROLLER WHERE OPEN TO THE SKY FROM ABOVE. COORDINATE EXACT LOCATION WITH OWNER AND ARCHITECT AND CONDUIT FOR WIRING WITH ELECTRICAL CONSULTANT.
- A LICENSED IRRIGATOR MUST BE ON-SITE AND PROVIDE SUPERVISION FOR THE ENTIRE DURATION OF THE IRRIGATION SYSTEM INSTALLATION. PER TAC RULE 344.36. ON-SITE SUPERVISING LICENSED IRRIGATOR ASSUMES ALL RESPONSIBILITY FOR THE IRRIGATION SERVICES PERFORMED IN ACCORDANCE WITH THESE DOCUMENTS.
- BACKFLOW PREVENTER MUST BE WINTERIZED UPON INSTALLATION.
- EACH IRRIGATION CONTROL VALVE SHALL BE ADJUSTED TO PROVIDE THE MINIMUM AMOUNT OF PRESSURE REQUIRED (PER MANUFACTURER'S RECOMMENDATIONS) TO OPERATE THE IRRIGATION ZONE IT IS SERVING.
- ALL IRRIGATION PIPING AND VALVES MUST MEET THE SEPARATION DISTANCES FROM THE ON-SITE SEWAGE FACILITIES SYSTEM AS REQUIRED FROM A PRIVATE WATER LINE IN 289.911(10) OF TAC TITLE 30 RELATING TO MINIMUM REQUIRED SEPARATION DISTANCES FOR ON-SITE SEWAGE FACILITIES.
- ALL IRRIGATION EMISSION DEVICES MUST DIRECT FLOW AWAY FROM ANY ADJACENT IMPERVIOUS SURFACE AND SHALL NOT BE INSTALLED CLOSER THAN FOUR INCHES FROM A HARDSCAPE AREA, SUCH AS, BUT NOT LIMITED TO, A BUILDING, FENCE, CONCRETE, OR ANY OTHER IMPERVIOUS MATERIAL.
- IRRIGATION EMISSION DEVICES MUST BE INSTALLED TO OPERATE AT THE MINIMUM AND NOT ABOVE THE MAXIMUM SPRINKLER HEAD PRESSURE AS PUBLISHED BY THE MANUFACTURER FOR THE NOZZLE AND HEAD SPACING THAT IS USED.
- ALL PVC IRRIGATION PIPING MUST NOT EXCEED THE MAXIMUM WATER VELOCITY OF FIVE FEET PER SECOND.
- ALL UNLABELED PVC IRRIGATION PIPING THAT IS DOWN STREAM OF PIPE SIZES LABELED 3/4" SHALL BE 1/2" CLASS 9 3.15 PVC.
- ALL PVC FITTINGS MUST BE PRIMED WITH A COLORED PRIMER PRIOR TO APPLYING THE PVC CEMENT IN ACCORDANCE WITH THE UNIFORM PLUMBING CODE (SECTION 316) OF THE INTERNATIONAL PLUMBING CODE (SECTION 605)
- RAIN/MOISTURE SHUT-OFF TECHNOLOGY MUST BE INSTALLED AND DONE SO IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS.
- AN ISOLATION VALVE MUST BE INCLUDED ON ALL IRRIGATION INSTALLATIONS AND SHALL BE PLACED BETWEEN THE WATER METER AND THE BACKFLOW PREVENTION DEVICE.
- ALL IRRIGATION PIPING MUST HAVE A MINIMUM DEPTH COVERAGE OF SIX (6) INCHES OF SELECT BACKFILL BETWEEN THE TOP OF THE PIPE AND THE FINISHED GRADE OF THE TOPSOIL. MOUNDING



VALVE SCHEDULE

NUMBER	MODEL	SIZE	TYPE	GPM	DESIGN PSI	PSI	PSI @ POC	PRECIP
1	Hunter ICV-G	1"	Turf Rotary	14.53	40	43.7	62.2	0.31 in/h
2	Hunter ICV-G	1"	Turf Rotary	11.79	40	43.8	59.9	0.73 in/h
3	Hunter ICV-G	1"	Bubbler	3.5	30	32.9	45.1	1.7 in/h
4	Hunter ICV-G	1"	Turf Rotary	13.6	40	43.8	57.6	0.75 in/h
5	Hunter PCZ-101-25	1"	Area for Dripline	9.22	25	36.3	50.2	0.43 in/h
6	Hunter ICV-G	1"	Turf Rotary	9.31	40	48.0	61.6	0.57 in/h

WATERING SCHEDULE

NUMBER	MODEL	TYPE	PRECIP	IN./WEEK	MIN./WEEK	GAL./WEEK	GAL./DAY
1	Hunter ICV-G	Turf Rotary	0.31 in/h	0.9	175	2,543	363
2	Hunter ICV-G	Turf Rotary	0.73 in/h	0.9	75	884	126
3	Hunter ICV-G	Bubbler	1.7 in/h	0.9	32	112	16
4	Hunter ICV-G	Turf Rotary	0.75 in/h	0.9	73	993	142
5	Hunter PCZ-101-25	Area for Dripline	0.43 in/h	0.9	127	1,171	167
6	Hunter ICV-G	Turf Rotary	0.57 in/h	0.9	95	885	126
TOTALS:					577	6,588	941.1

- SOIL TO MEET THIS REQUIREMENT MUST BE NOTED ON THE IRRIGATION PLAN AND DISCUSSED WITH THE IRRIGATION SYSTEM OWNER OR OWNERS REPRESENTATIVE AND LICENSED IRRIGATOR OF RECORD TO ADDRESS ANY SAFETY ISSUES.
- ALL TRENCHES AND HOLES CREATED DURING INSTALLATION OF AN IRRIGATION SYSTEM MUST BE BACKFILLED AND COMPACTED TO FINISHED GRADE.
 - ALL UNDERGROUND WIRING MUST BE LISTED BY UNDERWRITERS LABORATORIES AS ACCEPTABLE FOR BURIAL AND MUST BE BURIED WITH A MINIMUM OF SIX (6) INCHES OF BACKFILL.
 - ALL ELECTRICAL WIRE SPLICES EXPOSED TO MOISTURE MUST BE WATERPROOFED WITH 3M DBOIB-KKIT WATERPROOF WIRE CONNECTORS OR APPROVED EQUAL.
 - ALL QUICK COUPLERS MUST BE INSTALLED USING A QUICK COUPLER KEY AND PLACED IN A VALVE BOX. AN ISOLATION VALVE MUST BE INSTALLED UPSTREAM OF EACH QUICK COUPLER.
 - A FINAL WALK THROUGH WITH OWNERS REPRESENTATIVE MUST BE SCHEDULED PRIOR TO FINAL COMPLETION, TO EXPLAIN OPERATION OF THE SYSTEM.
 - UPON COMPLETION OF THE IRRIGATION SYSTEM INSTALLATION, PROVIDE THE OWNER OR OWNERS REPRESENTATIVE WITH A DOCUMENT CONTAINING, BUT NOT LIMITED TO THE FOLLOWING INFORMATION:
 - DRAWING SHOWING ACTUAL INSTALLATION (ALL VARIANCES FROM ORIGINAL PLAN MUST BE AUTHORIZED BY LICENSED IRRIGATOR OF RECORD). DRAWING MUST INCLUDE THE STATEMENT "THIS IRRIGATION SYSTEM HAS BEEN INSTALLED IN ACCORDANCE WITH ALL APPLICABLE STATE AND LOCAL LAWS, ORDINANCES, RULES, REGULATIONS OR ORDERS. I HAVE TESTED THE SYSTEM AND DETERMINED THAT IT HAS BEEN INSTALLED ACCORDING TO THE IRRIGATION PLAN AND IS PROPERLY ADJUSTED FOR THE MOST EFFICIENT APPLICATION OF WATER AT THIS TIME."
 - HOW TO OPERATE AND REPAIR THE IRRIGATION SYSTEM
 - MANUFACTURER'S MANUAL FOR THE AUTOMATIC CONTROLLER
 - HOW TO CHECK THE RAIN/MOISTURE SENSOR
 - A LIST OF COMPONENTS THAT REQUIRE MAINTENANCE, SUCH AS FILTERS, AND THE RECOMMENDED FREQUENCY FOR THE SERVICE.
 - HOW TO PRUNE GRASS AND PLANTS AWAY FROM IRRIGATION EMITTERS
 - LIST OF PRECIPITATION RATES OF EACH IRRIGATION ZONE WITHIN THE SYSTEM
 - DOCUMENTATION OUTLINING ANY WATER CONSERVATION MEASURES CURRENTLY IN EFFECT FROM THE WATER PURVEYOR
 - THE NAME OF THE WATER PURVEYOR
 - A SUGGESTED SEASONAL OR MONTHLY WATERING SCHEDULE BASED ON CURRENT EVAPOTRANSPIRATION DATA FOR THE GEOGRAPHIC REGION AND MINIMUM WATER REQUIREMENTS FOR THE PLANT MATERIAL IN EACH ZONE BASED ON THE SOIL TYPE AND PLANT MATERIAL WHERE THE SYSTEM IS INSTALLED.

- A WRITTEN WARRANTY COVERING MATERIALS AND LABOR FURNISHED IN THE NEW INSTALLATION OF THE IRRIGATION SYSTEM FOR A MINIMUM PERIOD OF ONE YEAR. WARRANTY MUST INCLUDE THE ON-SITE LICENSED IRRIGATION CONTRACTOR'S SEAL, NAME, SIGNATURE, DATE, BUSINESS ADDRESS AND BUSINESS TELEPHONE NUMBERS. WARRANTY MUST INCLUDE THE STATEMENT, "IRRIGATION IN TEXAS IS REGULATED BY THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ), MC-176, P.O. BOX 130897, AUSTIN, TEXAS 78711-3087. TCEQ'S WEBSITE IS: WWW.TCEQ.STATE.TX.US". IRRIGATION CONTRACTOR SHALL ALSO SUPPLY INFORMATION REGARDING APPLICABLE MANUFACTURER'S WARRANTIES.
- AFFIX A PERMANENT STICKER TO THE IRRIGATION CONTROLLER THAT LISTS THE ON-SITE LICENSED IRRIGATOR'S NAME, LICENSE NUMBER, COMPANY NAME, TELEPHONE NUMBER(S) AND THE DATES OF THE WARRANTY PERIOD.
 - AFFIX A LAMINATED AS-BUILT IRRIGATION ZONE MAP TO THE INSIDE COVER OF THE CONTROLLER. NUMBER ALL ZONES ON THE MAP. PROVIDE A CHART SHOWING THE PROGRAM, WATERING DAYS, START TIMES, AND RUN TIMES FOR EACH ZONE.
 - REFER TO IRRIGATION SPECIFICATIONS FOR MORE INFORMATION.
 - WHEN INSTALLING IRRIGATION UNDER OR NEAR EXISTING TREES, ALL TRENCHES MUST BE HAND DUG. ROOTS LARGER THAN TWO (2) INCHES IN DIAMETER MAY NOT BE CUT.
- DRIP IRRIGATION NOTES:**
- INSTALL MANUAL FLUSH VALVE(S) IN EACH ZONE AT LOW POINT(S), PER MANUFACTURER SPECIFICATIONS.
 - INSTALL ONE AIR/VACUUM RELIEF VALVE IN EACH SUB-SURFACE ZONE AT HIGHEST POINT PER MANUFACTURER SPECIFICATIONS.
 - INSTALL DRIP OPERATION INDICATOR(S) IN EACH ZONE. FOR END-FED ZONES PLACE INDICATOR AT OPPOSITE END FROM VALVE. FOR CENTER-FED ZONES AN INDICATOR SHALL BE PLACED AT EACH END OF ZONE. WHEN ZONE HAS MULTIPLE END AREAS, I.E. PARKING LOTS, PLACE ONE INDICATOR PER END AREA. PLACEMENT SHOULD BE ACCESSIBLE BUT SCREENED FROM PUBLIC VIEW.
 - ALL ON-SURFACE DRIFLINE SHALL BE BURIED TO MAXIMUM DEPTH RECOMMENDED BY MANUFACTURER AND COVERED BY A MULCH LAYER (MIN. 3" DEEP) EXCEPT IN AREAS OF SEASONAL COLOR AND SURFACE SPREADING GROUNDCOVERS.
 - ALL DRIFLINE, BOTH ON-SURFACE INSTALLATIONS AND BELOW GRADE, MUST BE SECURED EVERY THREE (3) LINEAR FEET WITH GALVANIZED TIE-DOWN STAKES.
 - INSTALL CONTROL VALVES WITH MAXIMUM 1/8 GAUGE WIRE.

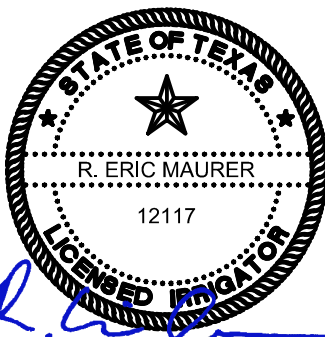


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REVISIONS

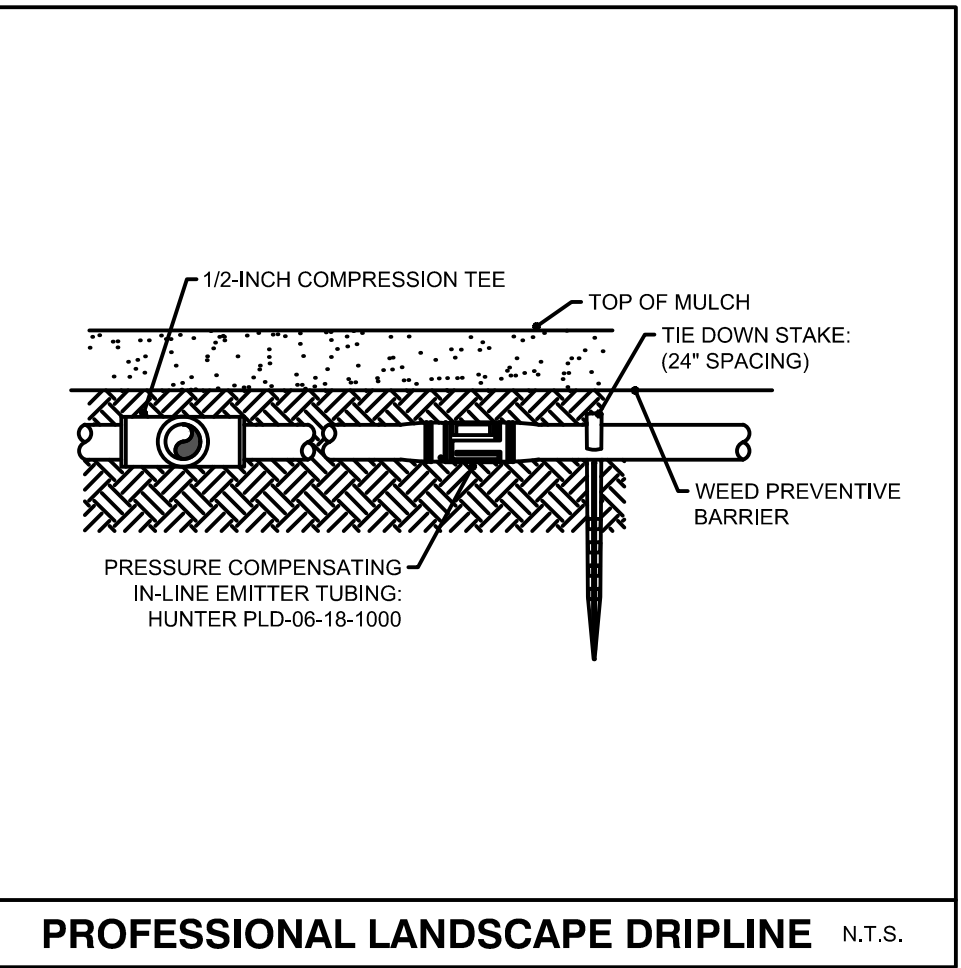
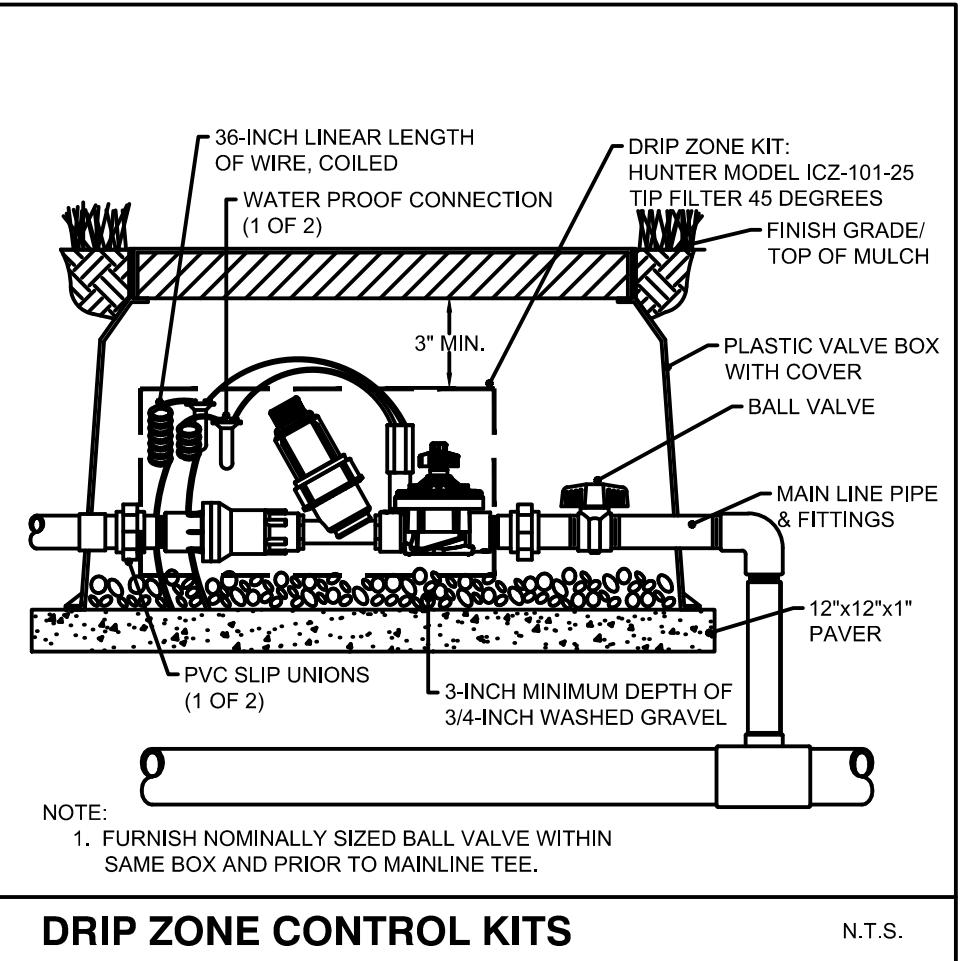
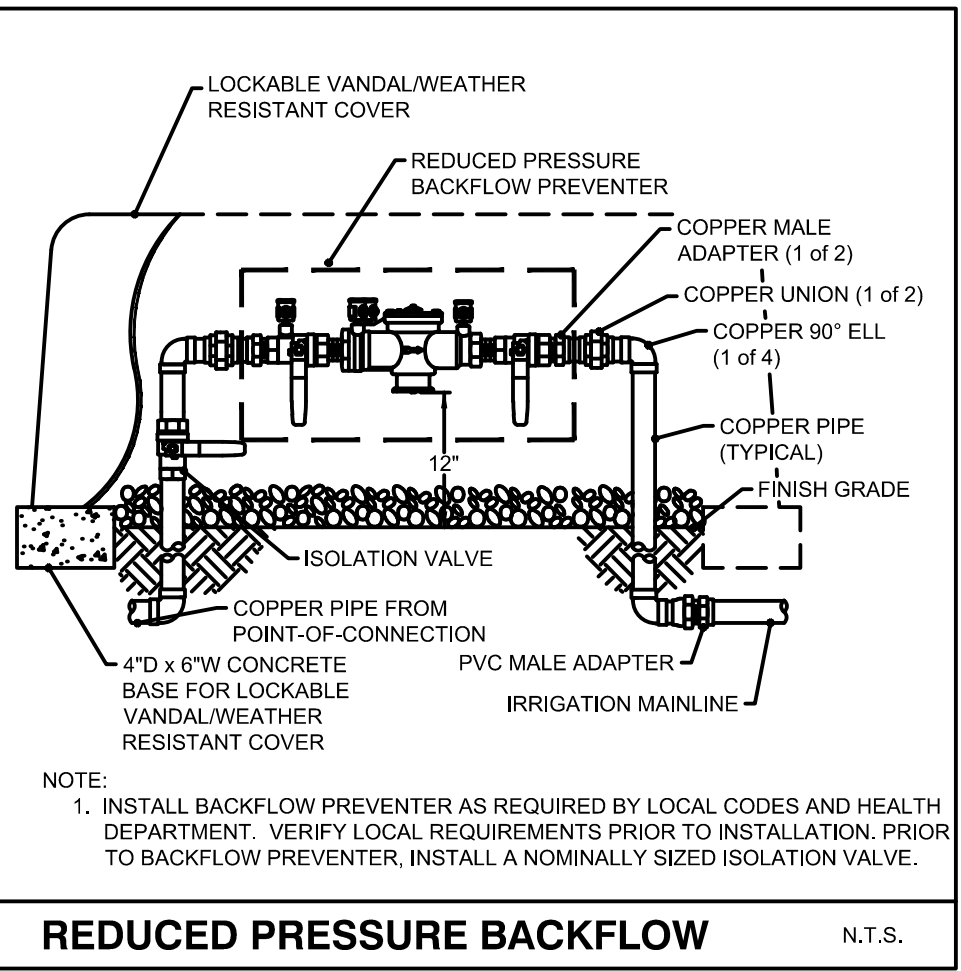
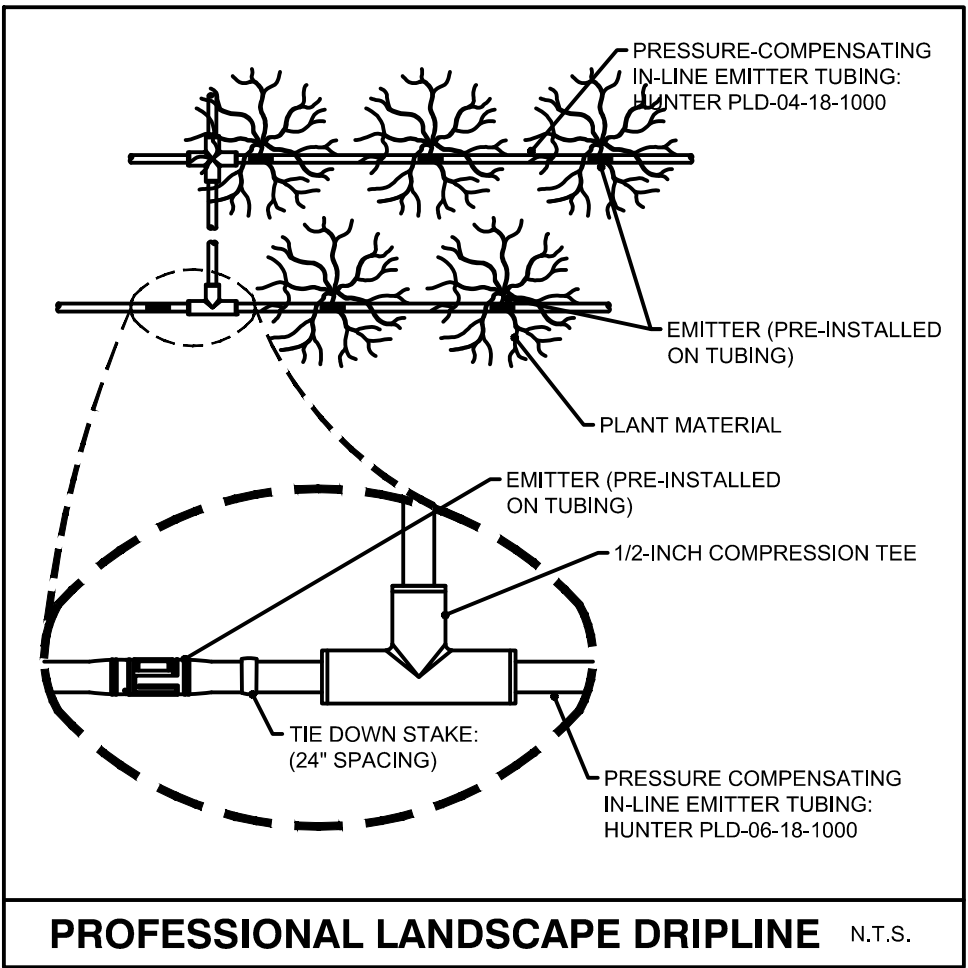
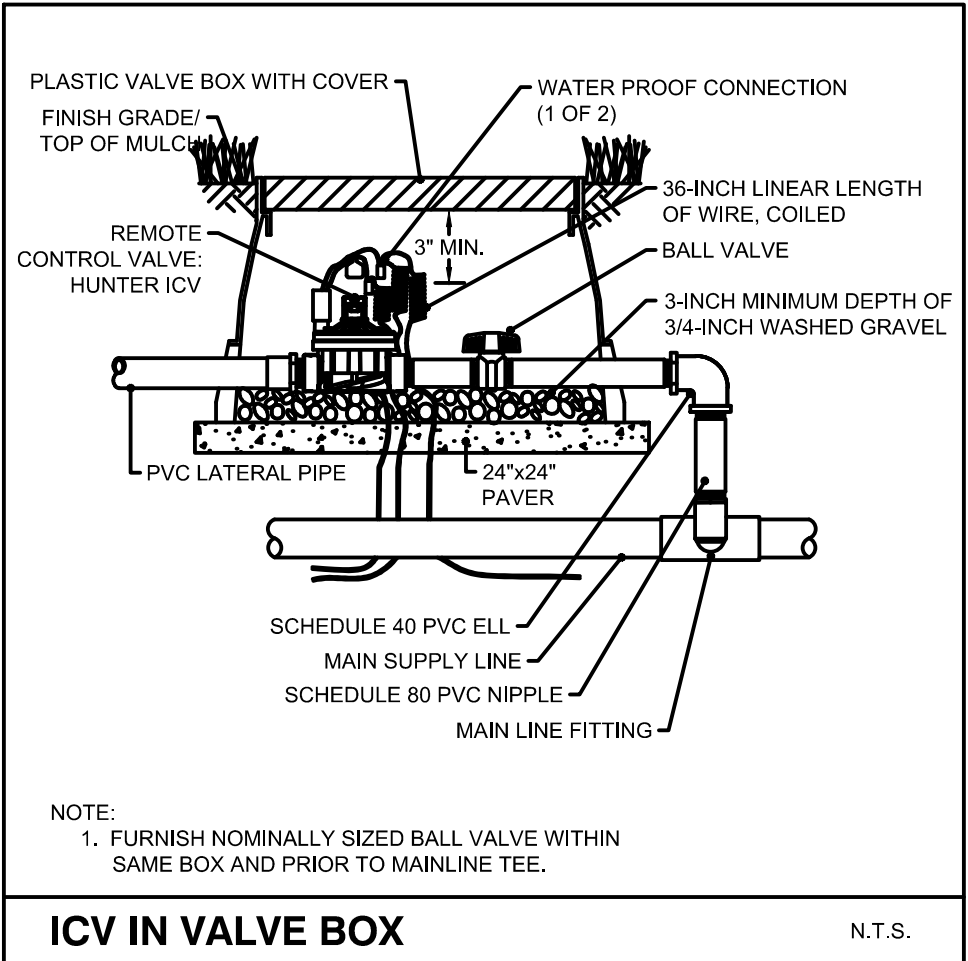
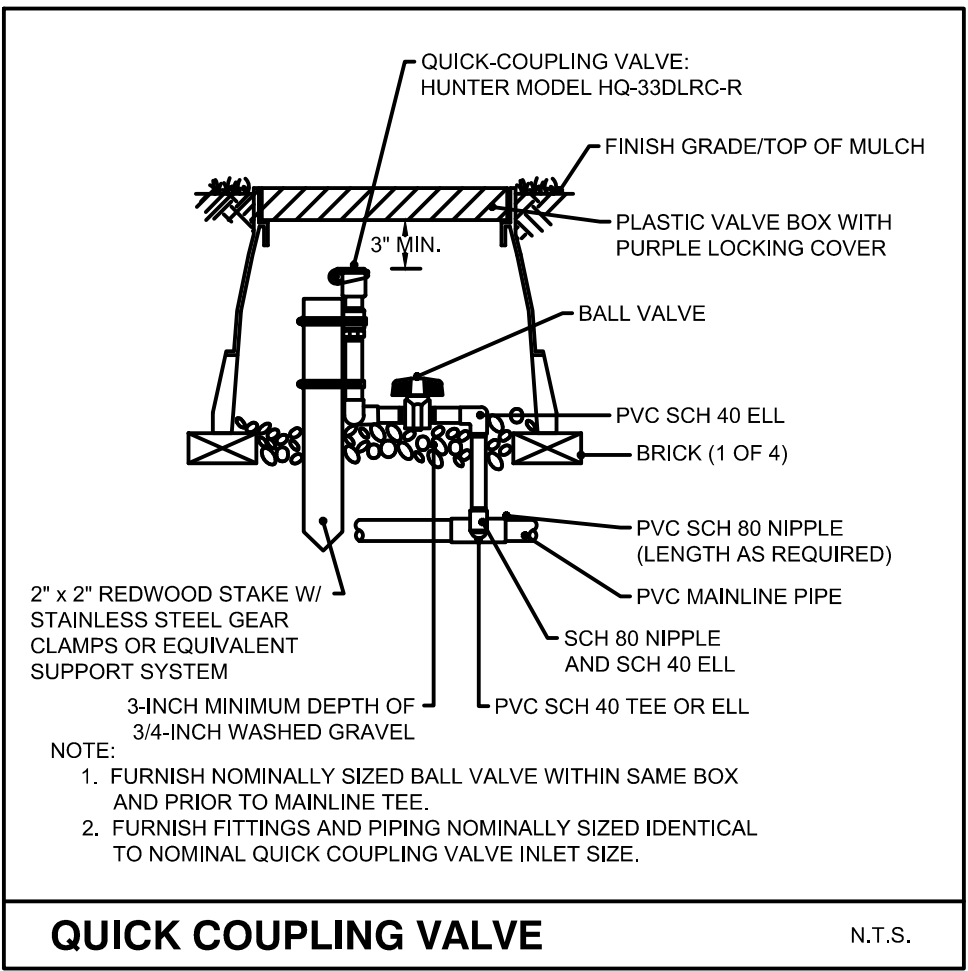
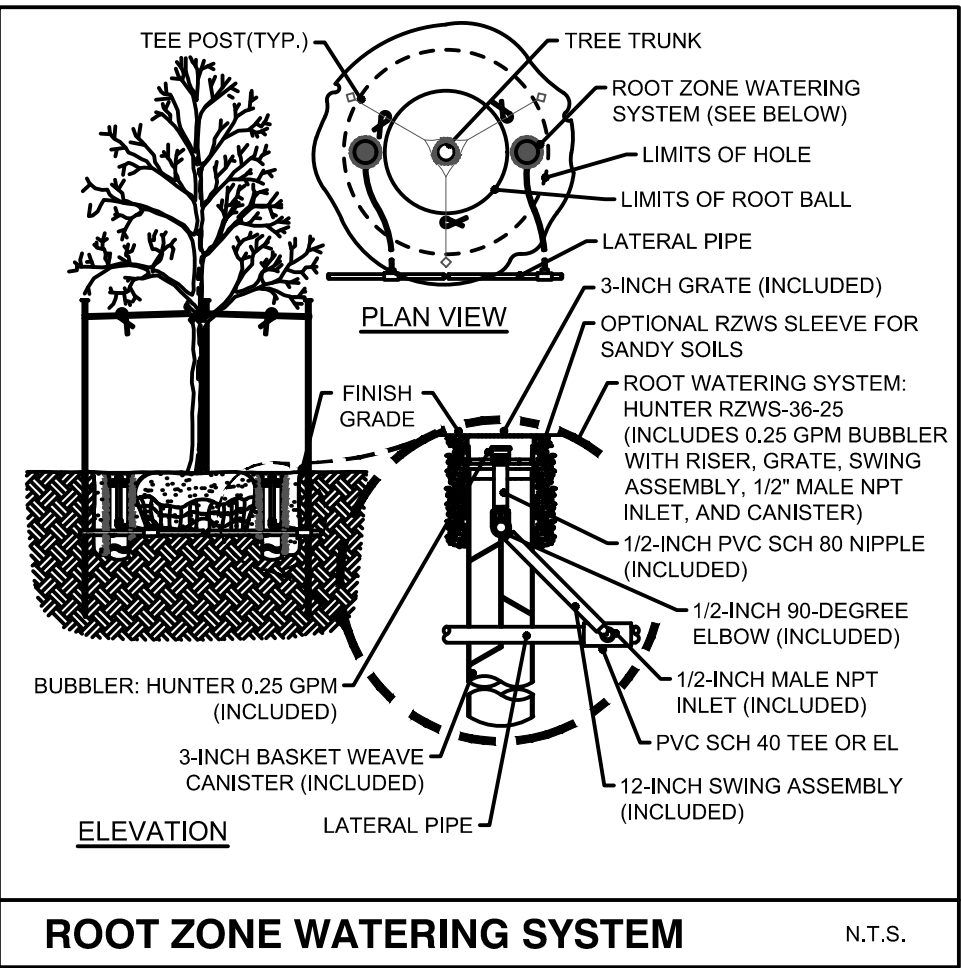
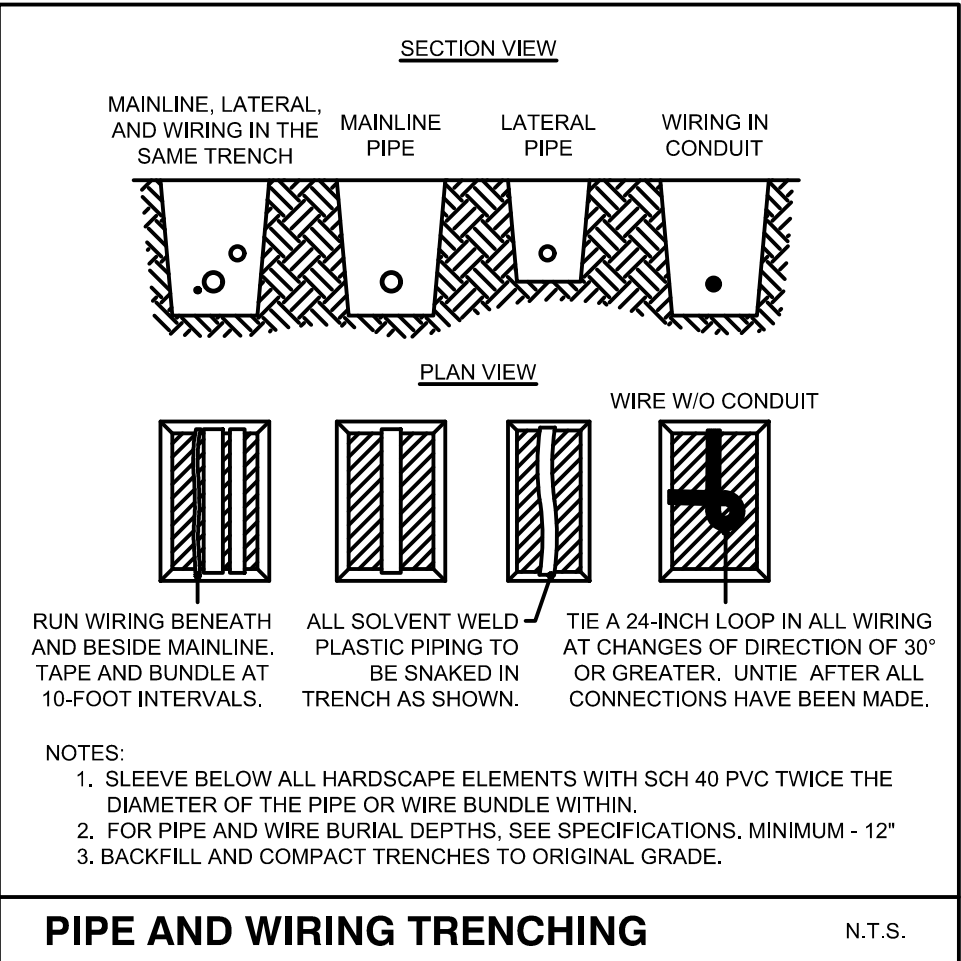
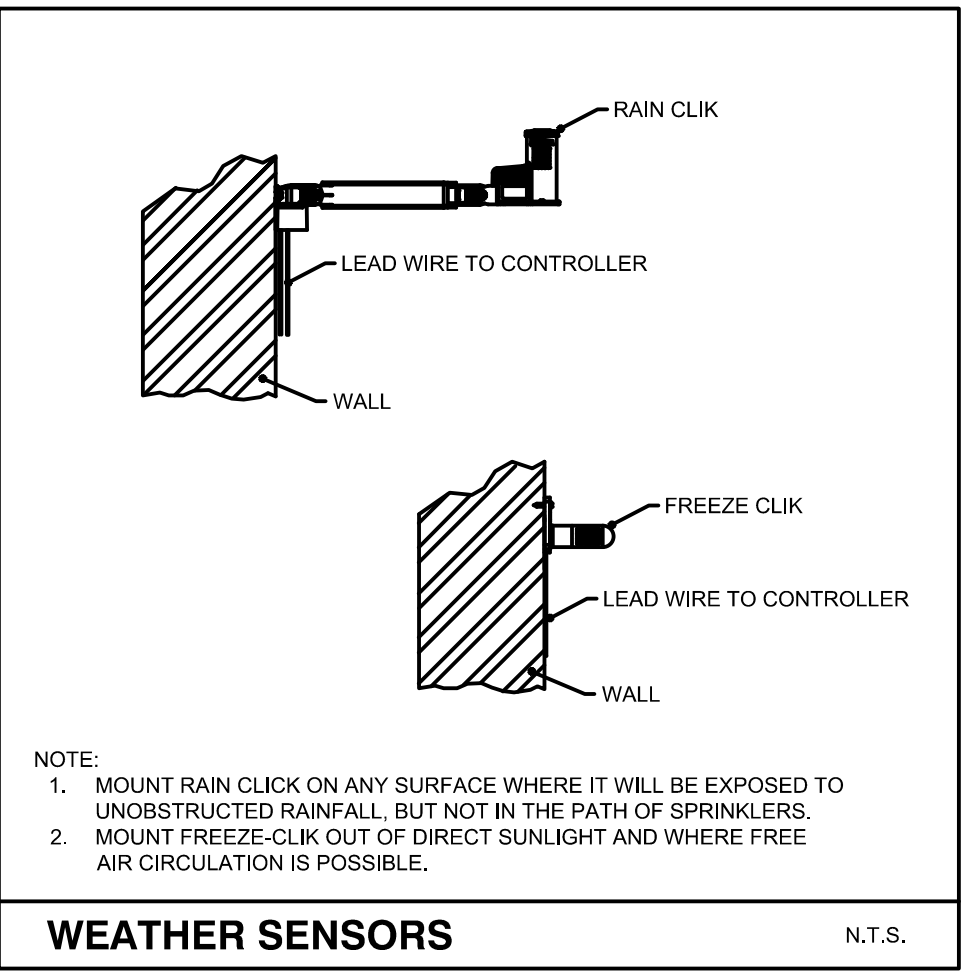
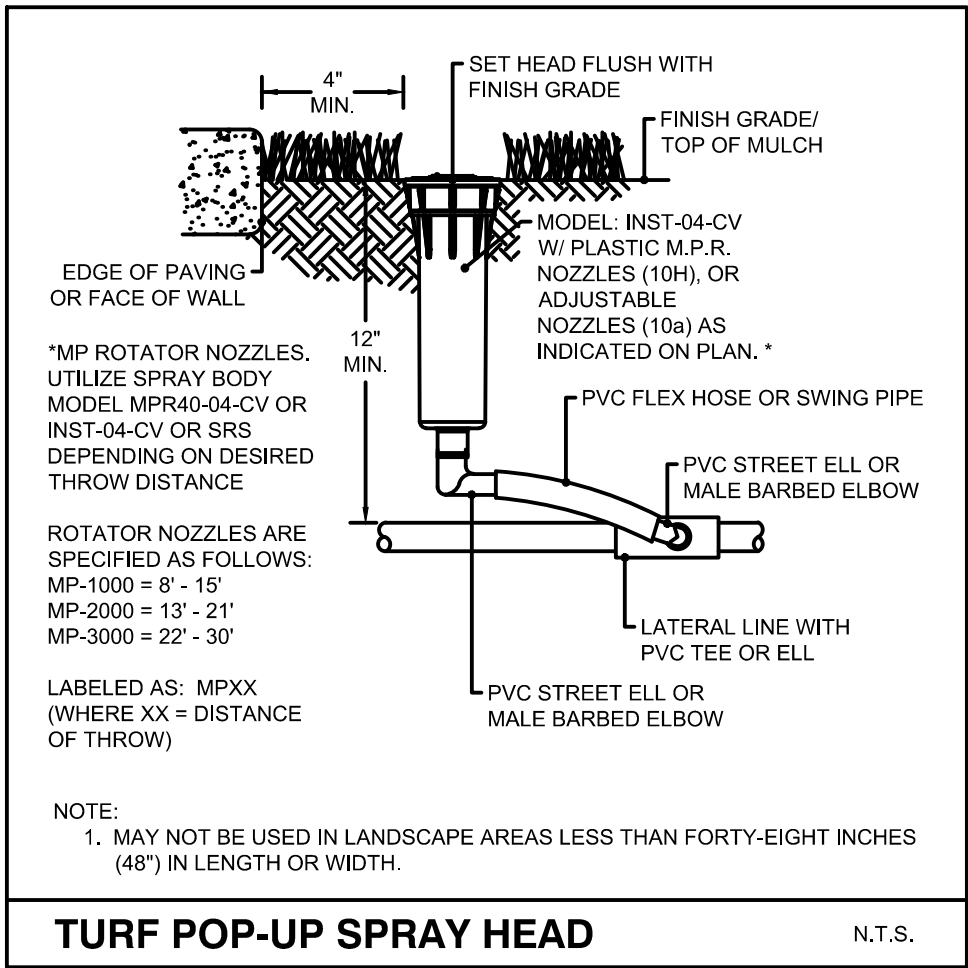
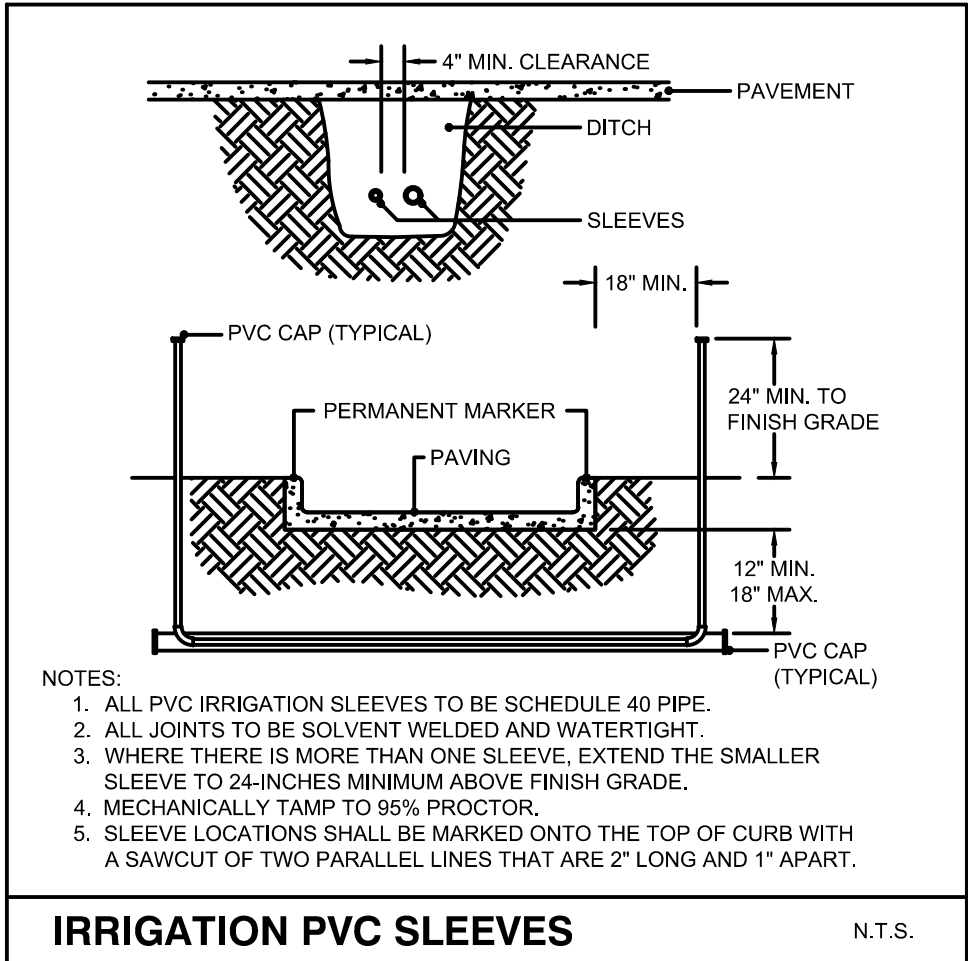
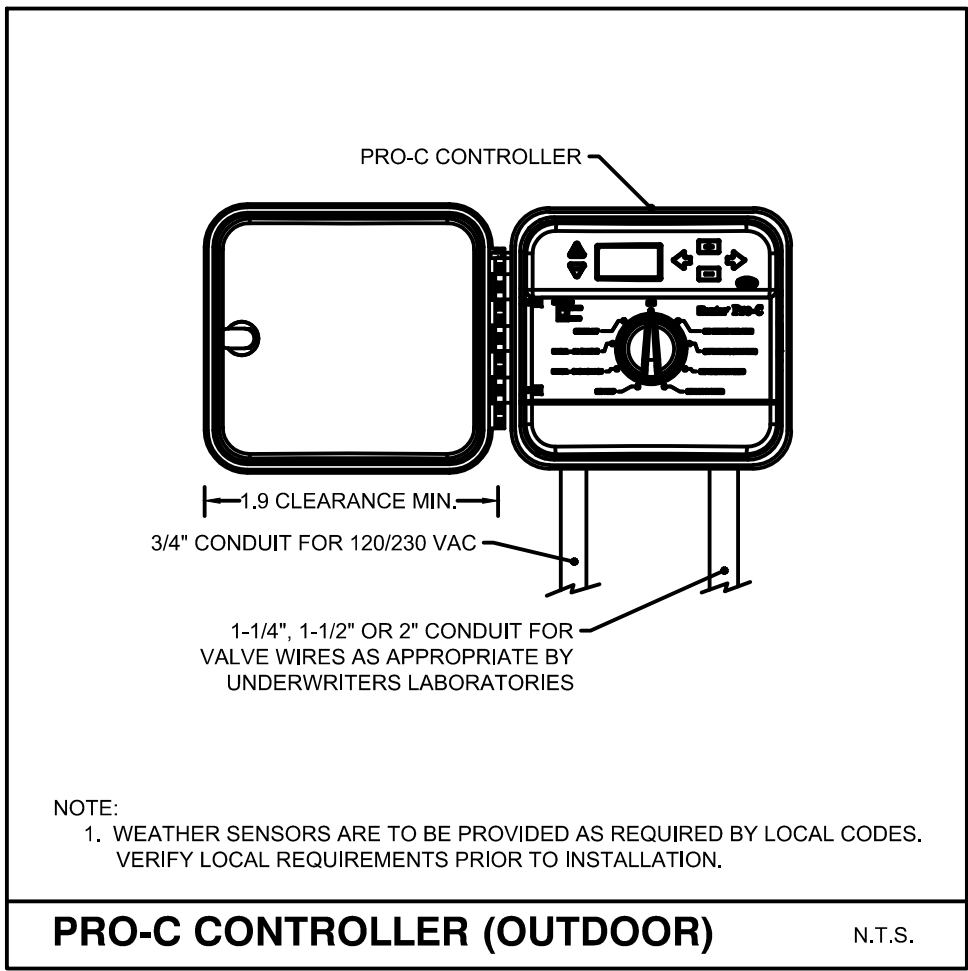
IRRIGATION PLAN

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



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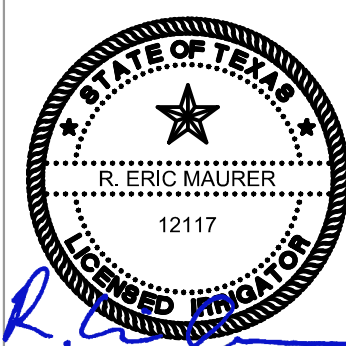


Bowman

REVISIONS

IRRIGATION DETAILS

O'REILLY AUTO PARTS
5510 WILLIAMS DR.
GEORGETOWN, TEXAS 78633



DESIGN MC DRAWN LB CHKD MK

JOB No. 250519-01-002

SHEET 25 OF 25



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Attachment G – Inspection, Maintenance, Repair and Retrofit Plan

Jellyfish Filter

Jellyfish cartridges are passively backwashed automatically after each storm event, which removes accumulated sediment from the membranes and significantly extends the service life of the cartridges and the maintenance interval. If required, the cartridges can be easily manually backwashed without removing the cartridges. Additionally, the lightweight cartridges can be removed by hand and externally rinsed, and rinsed cartridges then re-installed. These simple maintenance options allow for cartridge regeneration, thereby minimizing cartridge replacement costs and life-cycle treatment costs while ensuring long-term treatment performance.

Regular inspection and maintenance are proven, cost-effective ways to maximize water resource protection for all stormwater pollution control practices, and are required to insure proper functioning of the Jellyfish® Filter. Inspection of the Jellyfish® Filter is performed from the surface, while proper maintenance requires a combination of procedures conducted from the surface and with worker entry into the structure.

Please refer to the following information and guidelines before conducting inspection and maintenance activities:

When is inspection needed?

Post-construction inspection is required prior to putting the Jellyfish Filter into service.

Routine inspections are recommended quarterly during the first year of operation to accurately assess the sediment and floatable pollutant accumulation, and to ensure that the automatic backwash feature is functioning properly.

Inspection frequency in subsequent years is based on the maintenance plan developed in the first year, but must occur annually at a minimum.

Inspections should also be performed immediately after oil, fuel or other chemical spill.

When is maintenance service needed?

The unit must be cleaned annually. This cleaning includes removal and appropriate disposal of all water, sediment, oil and grease, and debris that has accumulated within the unit. The Jellyfish Filter is inspected and maintained by professional vacuum cleaning service providers with experience in the maintenance of underground tanks, sewers and catch basins. Since some of the maintenance procedures require manned entry into the Jellyfish structure, only professional maintenance service providers trained in confined space entry procedures should enter the vessel. Service provider companies typically have personnel who are trained and certified in confined space entry procedures according to local, state, and federal standards.

Filter cartridges should be tested for adequate flow rate, every 12 months and cleaned and re-commissioned, or replaced if necessary. A manual backflush must be performed on a single draindown cartridge using a Jellyfish Cartridge Backflush Pipe (described in the Jellyfish® Filter Owner's Manual). If the time required to drain 14 gallons of backflush water from the Backflush Pipe (from top of pipe to the top of the open flapper valve) exceeds 15 seconds, it is recommended to perform a manual backflush on each of the cartridges. After the manual backflush, the draindown test should be repeated on a single cartridge to determine if the cartridge can drain 14 gallons of water in 15 seconds. If the cartridge still does not achieve the design flow rate, it must be replaced.

The unit should be cleaned out immediately after an oil, fuel or chemical spill.

External Rinsing

This cartridge cleaning procedure is performed by removing the cartridge from the cartridge deck and externally rinsing the filtration tentacles using a low-pressure water sprayer, as described in the Jellyfish® Filter Owner's Manual. If this procedure is performed within the structure, the cartridge or individual filtration tentacles should be rinsed while safely suspended over the maintenance access wall opening in the cartridge deck, such that rinsate flows into the lower chamber of the Jellyfish® Filter. If the rinsing procedure is performed outside the structure, the cartridge or individual filtration tentacles should be rinsed in a suitable basin such as a plastic barrel or tub, and rinsate subsequently poured into the maintenance access wall opening in the cartridge deck. Sediment is subsequently removed from the lower chamber by standard vacuum service.



O'Reilly Auto Enterprises, LLC
233 S Patterson Avenue
Springfield, MO



Date

Statement of Certification: I Marisa Kaiser certify that the methods outlined within this attachment are adequate to adhere to the inspection, maintenance, and repair requirements outlined in TCEQ RG-348.

**Attachment H - Pilot-Scale Field Testing Plan, if BMPs not based on
Complying with the Edwards Aquifer Rules: Technical Guidance for BMPs**

N/A

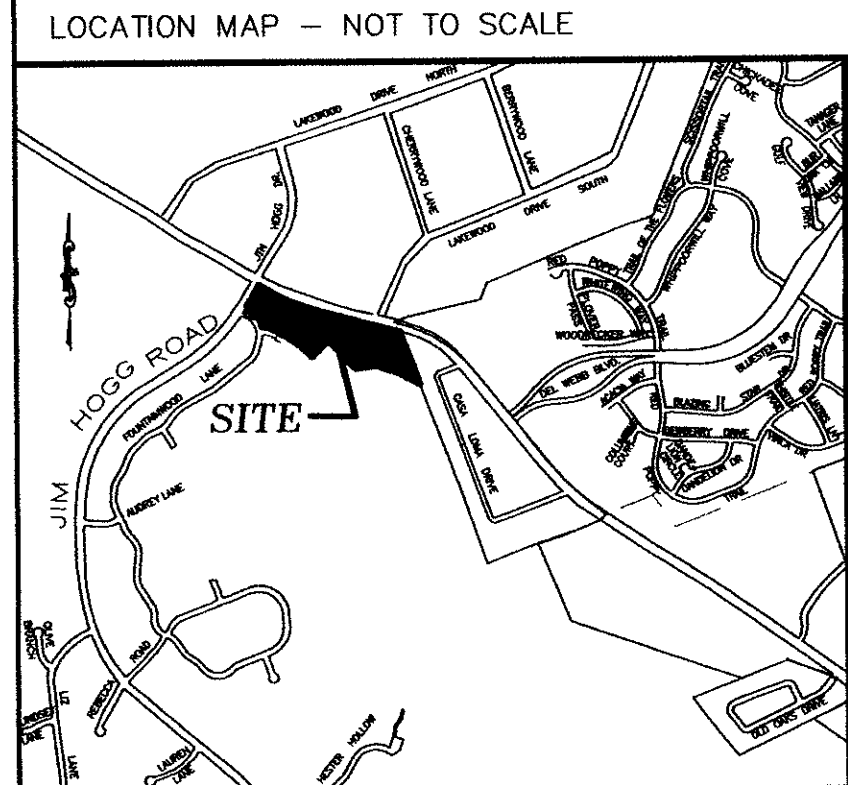
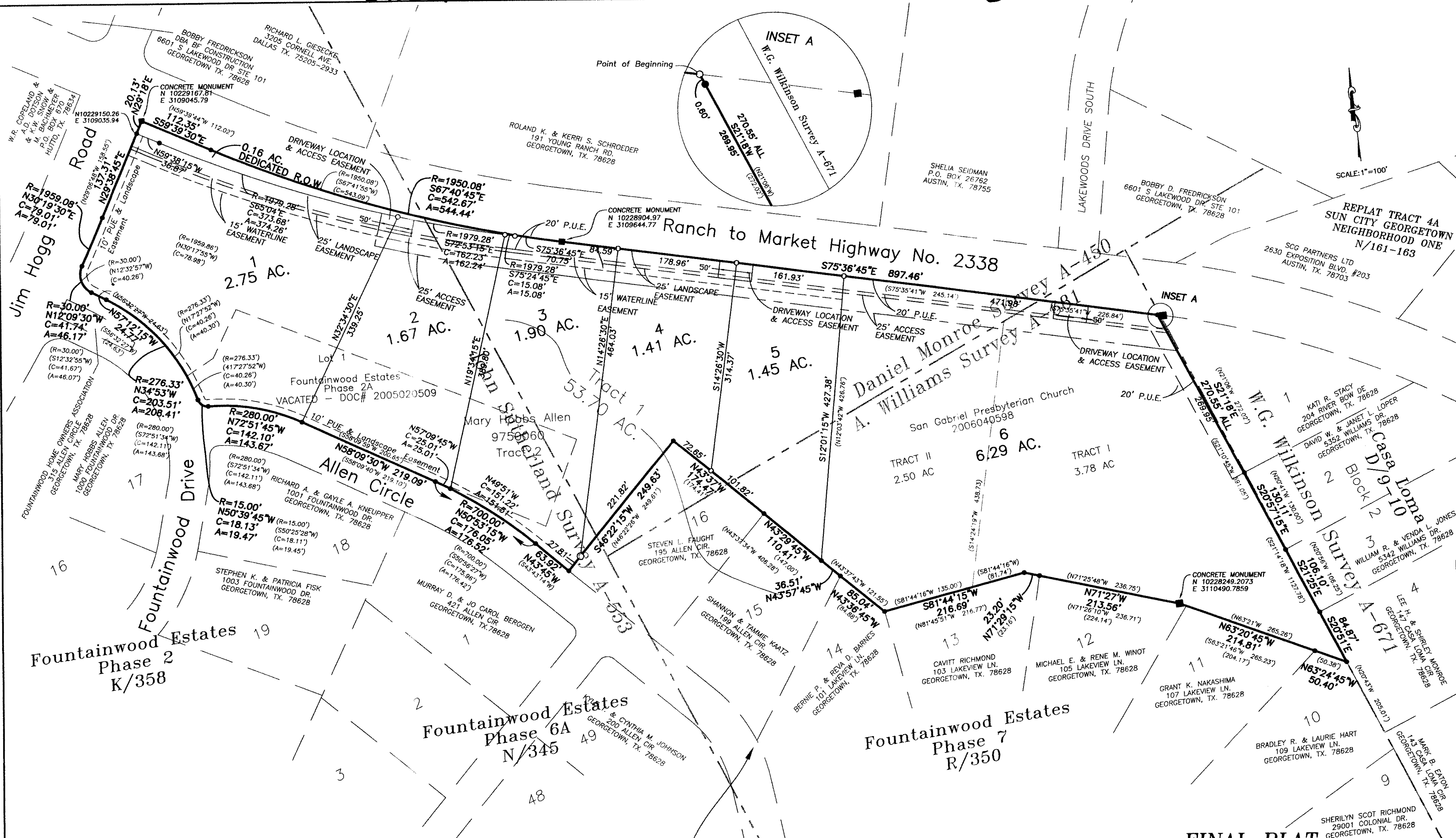
Attachment I – Measures for Minimizing Surface Stream Contamination

All flows from the proposed development will be treated by the Jellyfish device before discharging at detention pond and after that it will be directed to any surface streams. The proposed Permanent BMPs TSS removal will exceed the requirements of TCEQ 80% and the City of Georgetown 85%.

Detailed calculations are shown within the construction plans.

Attachment J – Final Plat

These drawings are the sole property of STEGER & BIZZELL ENGINEERING, INC. The use of these drawings is hereby restricted to the original site for which they were prepared. Reproduction or reuse of these drawings in whole or in part without written permission of STEGER & BIZZELL ENGINEERING, INC. is strictly prohibited.



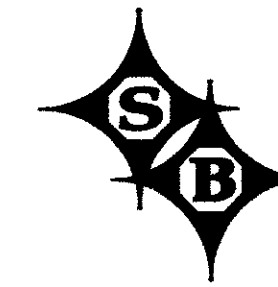
- LEGEND**
- - CONCRETE MONUMENT FOUND
 - - IRON PIN FOUND
 - - IRON PIN SET
 - P.U.E. - PUBLIC UTILITY EASEMENT
 - B.L. - BUILDING LINE

BENCHMARK:
Concrete Monument at Southeast corner of Lot 15
Fountainwood Estates Phase 7 on Allen Circle.
Elev. 921.22

OWNERS:
Mary Allen
1000 Fountainwood Drive
Georgetown, Texas 78628
San Gabriel Presbyterian Church

FINAL PLAT FOUNTAINWOOD PLAZA

A 15.63 ACRE SUBDIVISION
SITUATED IN THE
JOHN SUTHERLAND SURVEY, ABS. No.553
DANIEL MONROE SURVEY, ABS. No.450
AND
A WILLIAMS SURVEY, ABS. No.681
WILLIAMSON COUNTY, TEXAS



Steger & Bizzell Engineering, Inc.
Consulting Engineers
Surveyors

1978 South Austin Avenue
Georgetown, Texas 78626
DATE: JUNE, 2006
SHEET 1 of 3
JOB No.20189
L:\ACAD Drawings\20189 HOBBS-ALLEN\PLAT.dwg

PERIMETER DESCRIPTION

BEING 15.63 acres of land, situated in the John Sutherland Survey, Abstract No. 553, the Daniel Monroe Survey, Abstract No. 450, and the A. Williams Survey, Abstract No. 681, in Williamson County, Texas, said land being a portion of that certain Tract 1, called 53.70 acres, and all of that certain Tract 2, (Tract 2 being Lot 1 of Fountainwood Estates, Phase 2A, a subdivision of record in Cabinet K, Slide 360, of the Plat Records of Williamson County, Texas) as conveyed to Mary Hobbs Allen of record as Document No. 9750060 of the Official Records of Williamson County, Texas, being that certain Tract I, called 3.78 acres and Tract II, called 2.50 acres, as conveyed to San Gabriel Presbyterian Church, of record as Document No. 2006040598 of the Official Public Records of Williamson County, Texas. Surveyed on the ground in the month of June, 2003, under the supervision of Brian F. Peterson, Registered Professional Land Surveyor, and being more particularly described as follows;

BEGINNING at a point on the Southwest line of Ranch to Market Highway No. 2338, being on, or near, the east line of the A. Williams Survey, Abstract No. 681, being on, or near, the west line of the W.G. Wilkinson Survey, Abstract No. 671, for the Northeast corner of the above-referenced 53.70 acre Allen Tract 1, being the Northeast corner of the said 3.78 acre San Gabriel Presbyterian Church Tract I, and the most northerly corner of Casa Loma, a subdivision of record in Cabinet D, Sides 9-10, of the Plat Records of Williamson County, Texas, from said point the Northeast corner of the said A. Williams Survey, A-681, bears approximately N 21° W, 110 feet, more or less;

THENCE, along, or near, the east line of the said A. Williams Survey, A-681, being along, or near, the west line of the said W.G. Wilkinson Survey, A-671, along the east line of the said 53.70 acre Allen Tract 1, being the west line of the said Casa Loma, S 21° 18' E, at 0.60 of a foot pass an iron pin found for a total distance of 270.55 feet, in all, to an iron pin found; S 20° 57' 15" E, 130.11 feet to an iron pin found; S 21° 25' E, 106.10 feet to an iron pin found and S 20° 51' E, 84.87 feet to an iron pin found marking the Southeast corner of the said 3.78 acre San Gabriel Presbyterian Church Tract I, being the Northeast corner of Fountainwood Estates, Phase 7, a subdivision of record in Cabinet R, Slide 350, of the Plat Records of Williamson County, Texas, for the Southeast corner hereof;

THENCE, along the north line of the said Fountainwood Estates, Phase 7, along the south line of the said 3.78 acre San Gabriel Presbyterian Church Tract I, N 63° 24' 45" W, 50.40 feet to a railroad spike found and N 63° 20' 45" W, 214.81 feet to a concrete monument found; N 71° 27' W, 213.56 feet to an iron pin found; N 71° 29' 15" W, 23.20 feet to an iron pin found; S 81° 44' 15" W, at 81.74 feet, more or less, pass the Southwest corner of the said 3.78 acre San Gabriel Presbyterian Church Tract I, being the Southeast corner of the above-referenced 2.50 acre San Gabriel Presbyterian Church Tract II, and continuing along the south line of the said 2.50 acre San Gabriel Presbyterian Church Tract II, for a total distance of 216.69 feet, in all, to an iron pin found; N 43° 36' 45" W, 85.04 feet to an iron pin found and N 43° 57' 45" W, 36.51 feet to an iron pin found marking the Southwest corner of the said 2.50 acre San Gabriel Presbyterian Church Tract II, N 43° 29' 45" W, 110.41 feet to 2.50 acre San Gabriel Presbyterian Church Tract II, N 43° 37' W, passing the north line of the A. Williams Survey, an iron pin found and N 43° 37' W, passing the north line of the A. Williams Survey, A-681, being the south line of the Daniel Monroe Survey, A-450, for a total distance of 174.47 feet, in all, to an iron pin found and S 46° 22' 15" W, passing the west line of the said Daniel Monroe Survey, A-450, being the east line of the John Sutherland Survey, A-553, for a total distance of 249.63 feet, in all, to an iron pin found on the north line of Allen Circle, a dedicated roadway, being the north line of Fountainwood Estates, Phase 6A, a subdivision of record in Cabinet N, Slide 345, of the Plat Records of Williamson County, Texas, being the most westerly corner of the said Fountainwood Estates, Phase 7, being a westerly line of the said 53.70 acre Allen tract, for a southerly corner hereof;

THENCE, along the said north line of Allen Circle, N 43° 45' W, 63.92 feet to an iron pin found at the beginning of a curve to the left, (Radius = 700.00 feet, Long Chord bears N 50° 53' 15" W, 176.05 feet).
Thence, along the said curve for an arc distance of 176.52 feet to an iron pin found on the south line of the above-referenced Allen Tract 2, being a corner of the said 53.70 acre Allen Tract 1;
Thence, continuing along the south line of the said Tract 2, N 58° 09' 30" W, 219.09 feet to an iron pin found at the beginning of a curve to the left, (Radius = 280.00 feet, Long Chord bears N 72° 51' 45" W, 142.10 feet);
Thence, along the said curve for an arc distance of 143.67 feet to an iron pin found at the beginning of a curve to the right, (Radius = 15.00 feet, Long Chord bears N 50° 39' 45" W, 18.13 feet);
Thence, along the said curve for an arc distance of 19.47 feet to an iron pin found at the intersection of the said north line of Allen Circle and the east line of Fountainwood Drive, a dedicated roadway, being the Northwest corner of the said Fountainwood Estates, Phase 6A, being on the Northeast line of Fountainwood Estates, Phase 2, a subdivision of record in Cabinet K, Slide 358, of the Plat Records of Williamson County, Texas, and continuing along the said east line of Fountainwood Drive, along a curve to the left, (Radius = 276.33 feet, Long Chord bears N 34° 53' W, 203.51 feet);
Thence, along the said curve, at an arc distance of 40.30 feet, more or less, pass the Northwest corner the said Allen Tract 2, being a southerly corner of the said 53.70 acre Allen Tract 1, and continuing along the west line of the said 53.70 acre Allen Tract 1, for a total arc distance of 208.41 feet, in all, to an iron pin found;
Thence, N 57° 12' 15" W, 24.77 feet to an iron pin found at the beginning of a curve to the right, (Radius = 30.00 feet, Long Chord bears N 12° 09' 30" W, 41.74 feet);
Thence, along the said curve for an arc distance of 46.17 feet to an iron pin found at the intersection of Fountainwood Drive and the Southeast line of Jim Hogg Road, marking the most northerly corner of the said Fountainwood Estates, Phase 2, for the most westerly corner of the said 53.70 acre Allen Tract 1, for the most westerly corner hereof;

THENCE, along the said Southeast line of Jim Hogg Road, along a curve to the left, (Radius = 1959.08 feet, Long Chord bears N 30° 19' 30" E, 79.01 feet), an arc distance of 79.01 feet to an iron pin found;
Thence, N 29° 38' 45" E, 137.31 feet to an iron pin found and N 29° 18' E, 20.13 feet to a concrete monument found at the intersection of the said Southeast line of Jim Hogg Road and the said Southwest line of R.M. Highway No. 2338, for the most northerly corner of the said 53.70 acre Allen Tract 1, for the most northerly corner hereof;

THENCE, along the said Southwest line of R.M. Highway No. 2338, being the north line of the said 53.70 acre Allen Tract 1, as follows; S 59° 39' 30" E, 112.35 feet to an iron pin found at the beginning of a curve to the left, (Radius = 1950.08 feet, Long Chord bears S 67° 40' 45" E, 542.67 feet);
Thence, along the said curve for an arc distance of 544.44 feet to a concrete monument found;
Thence, S 75° 36' 45" E, at 425.48 feet pass an iron pin set for the Northeast corner of the said 53.70 acre Allen Tract 1, being the Northwest corner of the said 2.50 acre San Gabriel Presbyterian Church Tract II, at 670.62 feet pass the Northeast corner of the said 2.50 acre San Gabriel Presbyterian Church Tract II, being the Northwest corner of the said 3.78 acre San Gabriel Presbyterian Church Tract I, for a total distance of 897.46 feet, in all, to the Place of BEGINNING and containing 15.63 acres of land.

STATE OF TEXAS }

KNOW ALL MEN BY THESE PRESENTS

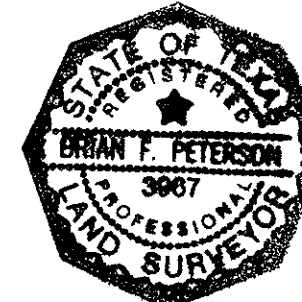
COUNTY OF WILLIAMSON }

I, Brian F. Peterson, Registered Professional Land Surveyor, in the State of Texas, do hereby certify that this plat is true and correctly made from an actual survey on the ground of the property legally described hereon, and that there are no apparent discrepancies, conflicts, overlapping of improvements, visible utility lines or roads in place, except as shown on the accompanying plat, and that the perimeter field notes hereon have a mathematical closure in compliance with standards as set out in the "Manual of Practice" of the Texas Society of Professional Surveyors, to the best of my knowledge and belief.

TO CERTIFY WHICH, WITNESS my hand and seal at Georgetown, Williamson County, Texas,

this the 26th day of June, 2006, A.D.

Brian F. Peterson
Registered Professional Land Surveyor, No. 3967
State of Texas



STATE OF TEXAS }

KNOW ALL MEN BY THESE PRESENTS

COUNTY OF WILLIAMSON }

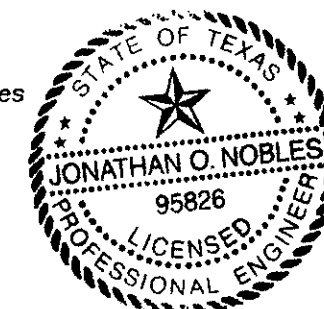
I, Jonathan O. Nobles, Licensed Professional Engineer, do hereby certify that this subdivision is in the Edwards Aquifer Recharge Zone. An approved Water Pollution Abatement Plan is required by the Texas Natural Resource Conservation Commission prior to construction. No lot within this subdivision is encroached by any Special Flood Hazard Areas inundated by 100-Year Flood as identified by the U.S. Federal Emergency Management Agency Boundary map (Flood Insurance Rate Map), Community-Panel Numbers 48491C0100C and 48491C0115C, effective date September 27, 1991.

The engineering work for the roads and drainage (including driveway drain pipes) within the subdivision have been completed in compliance with the Williamson County Subdivision Regulations (including the Engineering Guidelines incorporated as Appendix B) and all generally accepted engineering standards.

TO CERTIFY WHICH, WITNESS my hand and seal at Georgetown, Williamson County, Texas,

this the 26th day of JUNE, 2006, A.D.

Jonathan O. Nobles
Professional Engineer, No. 95826
State of Texas

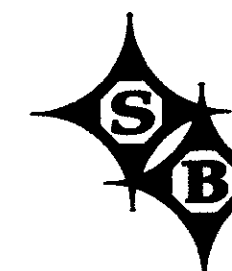


GENERAL NOTES:

1. Total Acres: 15.63 Acres
2. Number of lots: 6
3. Number of blocks: 1
4. Area of smallest lot: 1.414 Acres
5. There is no area within the boundaries of this subdivision in the 100-year Flood plain as defined by FIRM Map Number 48491C0115C, effective date of September 27, 1991.
6. Water service will be provided by Chisholm Trail Special Utility District, wastewater service shall be provided by the City of Georgetown. Electric service provided by P.E.C..
7. In order to promote drainage away from a structure, the slab elevation should be built at least one foot above the surrounding ground, and the ground should be graded away from the structure at a slope of 1/2" per foot for a distance of a least 10 feet.
8. Maximum impervious cover is 50% allowable up to 65% if appropriate impervious coverage waivers are met in accordance with UDC Section 11.02.020.
9. The bearings on this plat have been rotated to the NAD 83/93 HARN - Texas Central Zone and elevation is based upon NAVD 88.
10. This subdivision is located in the limits of the City of Georgetown.
11. This tract is located in the Edwards Aquifer Recharge Zone. An approved Water Pollution Abatement Plan is required by the Texas Natural Resource Conservation Commission prior to construction.
12. All driveway pipes to be minimum 18" RCP or dip driveway.
13. Building setbacks will be in accordance with the City of Georgetown UDC.
14. Sidewalks and street trees will be constructed in accordance with the City of Georgetown UDC.
15. A TIA, Traffic Impact Analysis will be required during the site plan review process when the cumulative Average Daily Trips (ADT) exceeds 2000 according to Institute of Transportation Engineer (ITE) manual.
16. No access shall be permitted onto either Fountainwood Drive or Allen Circle from this development. Access to Williams Drive for all lots shall be from the driveway locations shown on this plat.
17. A rock wall will be built by the developer along the boundary length of Fountainwood Drive and Allen Circle with the submittal of a site plan within this subdivision and prior to final certificate of occupancy..

FINAL PLAT FOUNTAINWOOD PLAZA

A 15.63 ACRE SUBDIVISION
SITUATED IN THE
JOHN SUTHERLAND SURVEY, ABS. No.553
DANIEL MONROE SURVEY, ABS. No.450
AND
A WILLIAMS SURVEY, ABS. No.681
WILLIAMSON COUNTY, TEXAS



Steger & Bizzell Engineering, Inc.
Consulting Engineers Surveyors

Telephone: (512)930-9412 (512)930-9416
Facsimile: (512)930-9412

1978 South Austin Avenue
Georgetown, Texas 78626

DATE: June, 2006

SHEET 2 of 3

JOB No.20189

L/ACAD Drawings/20189 HOBBS-ALLEN/PLAT.dwg

These drawings are the sole property of STEGER & BIZZELL ENGINEERING, INC. The use of these drawings is hereby restricted to the original site for which they were prepared. Reproduction or reuse of these drawings in whole or in part without written permission of STEGER & BIZZELL ENGINEERING, INC. is strictly prohibited.

STATE OF TEXAS {
COUNTY OF WILLIAMSON {
KNOW ALL MEN BY THESE PRESENTS;

That I, Mary Hobbs Allen, owner of that certain tract of land shown hereon and described in a deed recorded in Documents No. 9750060 of the Official Records of Williamson County, Texas, do hereby join, approve, and consent to all dedications and plat note requirements shown hereon. I do hereby approve the recordation of this subdivision plat and dedicate to the public use forever any easements and roads that are shown hereon. This subdivision is to be known as FOUNTAINWOOD PLAZA. I hereby acknowledge that I am the owner of this property and do hereby state that there are no lienholders or any unpaid debt for which this property represents collateral on any loan. It is also understood that it is the responsibility of the owner, not the County, to assure compliance with the provisions of all applicable state, federal, and local laws and regulations relating to the environment, including, but not limited to the Endangered Species Act, State Aquifer Regulations, and Municipal Watershed Ordinances.

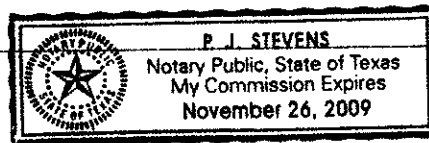
Mary Hobbs Allen
Mary Hobbs Allen
1000 Fountainwood Drive
Georgetown, Texas 78628

Before me, the undersigned authority, on this day personally appeared Mary Hobbs Allen known by me to be the person whose name is subscribed to the foregoing instrument. It has been acknowledged to me that he/she executed the foregoing instrument as the owner of the property described hereon.

P. J. Stevens Notary Public in and
Signature and Date

P. J. Stevens Printed Name of
Notary and Notary Stamp

Commission Expires November 26, 2009 Date Notary



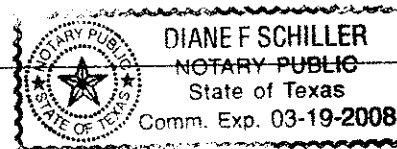
STATE OF TEXAS {
COUNTY OF WILLIAMSON {
KNOW ALL MEN BY THESE PRESENTS;

That we, San Gabriel Presbyterian Church of Georgetown, owner of the certain tract of land shown hereon and described in a deed recorded in Document #2006040598 of the Deed Records of Williamson County, Texas, do hereby join, approve, and consent to all dedications and plat note requirements shown hereon. I do hereby approve the recordation of this subdivision plat and dedicate to the public use forever any easements and roads that are shown hereon. This subdivision is to be known as: FOUNTAINWOOD PLAZA. It is also understood that it is the responsibility of the owner, not the County, to assure compliance with the provisions of all applicable state, federal, and local laws and regulations relating to the environment, including, but not limited to the Endangered Species Act, State Aquifer Regulations, and Municipal Watershed Ordinances.

Donald J. Harper
Donald J. Harper, Building Committee Chairman
San Gabriel Presbyterian Church of Georgetown
6920 RR 2338
Georgetown, Texas 78628

Before me, the undersigned authority, on this day personally appeared Donald J. Harper, known by me to be the person whose name is subscribed to the foregoing instrument. It has been acknowledged to me that she executed the foregoing instrument as the owner of the property described hereon.

Diane F. Schiller Notary Public in and for the State of Texas
Signature and Date
Diane F. Schiller Printed Name of Notary and
Notary Stamp
3-19-08 Date Notary Commission
Expires
Comm. Exp. 03-19-2008



STATE OF TEXAS {
COUNTY OF WILLIAMSON {
KNOW ALL MEN BY THESE PRESENTS;

That I, First Texas Bank, the lien holder of the certain tract of land shown hereon and described in a DEED OF TRUST recorded in Document #2006040597 of the Official Records of Williamson County, Texas, do hereby join, approve, and consent to all dedications and plat note requirements shown hereon. I do hereby approve the recordation of this subdivision plat and dedicate to the public use forever any easements and roads that are shown hereon. This subdivision is to be known as FOUNTAINWOOD PLAZA.

Kevin R. Kelly
Kevin R. Kelly, V.P.
First Texas Bank
900 S. Austin Ave.
Georgetown, TX 78626

Before me, the undersigned authority, on this day personally appeared Kevin R. Kelly known by me to be the person whose name is subscribed to the foregoing instrument. It has been acknowledged to me that he/she executed the foregoing instrument as the legal representative of the lienholder of the property described hereon.

April Weathersby Notary Public in and for the State of Texas
Signature and Date
April Weathersby Printed Name of Notary and
Notary Stamp
12/2/07 Date of Notary Commission
Expires



I, Bobby Ray, AICP, Director of the Planning and Development Services of the City of Georgetown, do hereby certify this plat as approved for filing of record with the County Clerk of Williamson County, Texas.

Date: 6/29/06
Bobby Ray
Bobby Ray, Director
Planning and Development Services

The subdivision, known as FOUNTAINWOOD PLAZA, has been approved for filing for record according to the minutes of the meeting of the Georgetown City Council on the 27th day of April, 2004, A.D.

Attest:
Gary Nelson
Gary Nelson, Mayor
City of Georgetown, Texas

Sandra Lee
Sandra Lee, City Secretary
City of Georgetown, Texas

Based upon the above representations of the Engineer or Surveyor whose seal is affixed hereto, and after a review of the plat as represented by the said Engineer or Surveyor, I find that this plat complies with the requirements of Chapter 15.44, Flood Damage Prevention, of the Georgetown Municipal Code. This certification is made solely upon such representations and should not be relied upon for verifications of the facts alleged. The City of Georgetown disclaims any responsibility to any member of the public or independent verifications of the representation, factual or otherwise, contained in this plat and the documents associated with it.

Date: 6/29/06
Dave Halk
Dave Halk, Flood Plain Coordinator
City of Georgetown

STATE OF TEXAS {
COUNTY OF WILLIAMSON {
KNOW ALL MEN BY THESE PRESENTS

I, Nancy Rister, Clerk of the County Court of said County, do hereby certify that the foregoing instrument in writing, with its certificate of authentication was filed for record in my office on the 30th day of JUNE, 2006 A.D., at 8:36 o'clock, A.M., and duly recorded this the 6th day of JULY, 2006 A.D., at 11:49 o'clock, A.M., in the Plat Records of said County in Cabinet CC, Slides 39, 40 and 41.

TO CERTIFY WHICH, WITNESS my hand and seal at the County Court of said County, at my office in Georgetown, Texas, the date last shown above written.

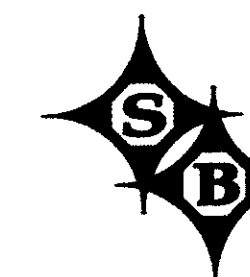
Nancy Rister, Clerk County Court
of Williamson County, Texas

Karen Miller Deputy



FINAL PLAT FOUNTAINWOOD PLAZA

A 15.63 ACRE SUBDIVISION
SITUATED IN THE
JOHN SUTHERLAND SURVEY, ABS. No.553
DANIEL MONROE SURVEY, ABS. No.450
AND
A WILLIAMS SURVEY, ABS. No.681
WILLIAMSON COUNTY, TEXAS



Steger & Bizzell Engineering, Inc.
Consulting Engineers Surveyors

1978 South Austin Avenue
Georgetown, Texas 78626
DATE: JUNE, 2006

SHEET 3 of 3 JOB No.20189

L/ACAD Drawings/20189 HOBBS-ALLEN/PLAT.dwg

8– TCEQ - 0599 Attachments

Agent Authorization Form

Agent Authorization Form
For Required Signature
Edwards Aquifer Protection Program
Relating to 30 TAC Chapter 213
Effective June 1, 1999

I Philip Hopper
Print Name

Senior Vice President of Real Estate Expansion
Title - Owner/President/Other

of O'Reilly Auto Enterprises, LLC
Corporation/Partnership/Entity Name

have authorized Marisa Keiser
Print Name of Agent/Engineer

of Bowman Consulting Group, LTD
Print Name of Firm

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:


Applicant's Signature

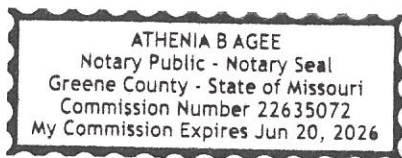
8-12-25
Date

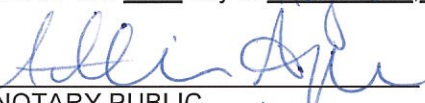
THE STATE OF Missouri §

County of Greene §

BEFORE ME, the undersigned authority, on this day personally appeared Philip Hopper known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this 12 day of August, 2025.




NOTARY PUBLIC
Athenia Bagee
Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 06/20/2026



Owner Authorization Form

Edwards Aquifer Protection Program

Instructions

Complete the following form by adding the requested information in the fields below. The form must be notarized for it to be considered complete. Attach it to other programmatic submittals required by 30 Texas Administrative Code (30 TAC), Chapter 213, and provide it to TCEQ's Edwards Aquifer Protection Program (EAPP) as part of your application.

If you have questions on how to fill out this form or about EAPP, please contact us by phone at 512-339-2929 or by e-mail at eapp@tceq.texas.gov.

Landowner Authorization

I, Terry Cornman of Cornman Lakewood Development, Partners Limited am the owner of the property located at:

Parcel #: R476274 5510 Williams Drive Georgetown TX

and am duly authorized in accordance with 30 TAC 213.4(c)(2) and 213.4(d)(1), or 30 TAC 213.23(c)(2) and 213.23(d), relating to the right to submit an application, signatory authority, and proof of authorized signatory.

I do hereby authorize CASCO
To conduct signing on behalf of the landowner
At 5510 Williams Drive Georgetown, TX

Landowner Acknowledgement

I understand that: O'Reilly Auto Enterprises LLC

Is ultimately responsible for the compliance with the approved or conditionally approved Edwards Aquifer protection plan and any special conditions of the approved plan through all phases of plan implementation even if the responsibility for compliance and the right to possess and control the property referenced in the application has been contractually assumed by another legal entity. I further understand that any failure to comply with any condition of the executive director's approval is a violation and subject to administrative rule or orders and penalties as provided under 30 TAC 213.10, relating to enforcement. Such violations may also be subject to civil penalties.

Landowner Signature

Signature
Landowner Signature

[Handwritten Signature]

Date
Date

10/02/2025

THE STATE § OF State Texas
County § of County Williamson

BEFORE ME, the undersigned authority, on this day personally appeared
O'Reilly Auto Enterprises LLC

known to me to be the person whose name is subscribed to the foregoing instrument and
acknowledged to me that (s)he executed same for the purpose and consideration therein
expressed.

GIVEN under my hand and seal of office on this Day day of Month

10.2.2025

Click or tap here to add ID
NOTARY PUBLIC

Typed or Printed Name of Notary
MY COMMISSION EXPIRES: Date



[Handwritten Signature]

Optional Attachments

Select All that apply:

- ☐ Lease Agreement
- ☐ Signed Contract
- ☐ Deed Restricted Easement
- ☐ Other legally binding documents

9– TCEQ - 0574 Attachments

Application Fee Form

Application Fee Form

Texas Commission on Environmental Quality

Name of Proposed Regulated Entity: O'Reilly Auto Parts

Regulated Entity Location: City of Georgetown

Name of Customer: O'Reilly Auto Enterprises, LLC

Contact Person: Rick Hicks

Phone: (417) 427-8062 Ext: 65680

Customer Reference Number (if issued):CN _____

Regulated Entity Reference Number (if issued):RN _____

Austin Regional Office (3373)

☐ Hays

☐ Travis

☒ Williamson

San Antonio Regional Office (3362)

☐ Bexar

☐ Medina

☐ Uvalde

☐ Comal

☐ Kinney

Application fees must be paid by check, certified check, or money order, payable to the **Texas Commission on Environmental Quality**. Your canceled check will serve as your receipt. **This form must be submitted with your fee payment.** This payment is being submitted to:

☒ Austin Regional Office

☐ San Antonio Regional Office

☒ Mailed to: TCEQ - Cashier

☐ Overnight Delivery to: TCEQ - Cashier

Revenues Section

Mail Code 214

P.O. Box 13088

Austin, TX 78711-3088

12100 Park 35 Circle

Building A, 3rd Floor

Austin, TX 78753

(512)239-0357

Site Location (Check All That Apply):

☒ Recharge Zone

☐ Contributing Zone

☐ Transition Zone

<i>Type of Plan</i>	<i>Size</i>	<i>Fee Due</i>
Water Pollution Abatement Plan, Contributing Zone Plan: One Single Family Residential Dwelling	Acres	\$
Water Pollution Abatement Plan, Contributing Zone Plan: Multiple Single Family Residential and Parks	Acres	\$
Water Pollution Abatement Plan, Contributing Zone Plan: Non-residential	1.41 Acres	\$ 4,000.0
Sewage Collection System	184 L.F.	\$ 650.0
Lift Stations without sewer lines	Acres	\$
Underground or Aboveground Storage Tank Facility	Tanks	\$
Piping System(s)(only)	Each	\$
Exception	Each	\$
Extension of Time	Each	\$

Signature: Marisa Keiser

Date: August 4, 2025

Application Fee Schedule

Texas Commission on Environmental Quality

Edwards Aquifer Protection Program 30 TAC Chapter 213 (effective 05/01/2008)

Water Pollution Abatement Plans and Modifications

Contributing Zone Plans and Modifications

<i>Project</i>	<i>Project Area in Acres</i>	<i>Fee</i>
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5	\$1,500
	5 < 10	\$3,000
	10 < 40	\$4,000
	40 < 100	\$6,500
	100 < 500	\$8,000
	≥ 500	\$10,000
Non-residential (Commercial, industrial, institutional, multi-family residential, schools, and other sites where regulated activities will occur)	< 1	\$3,000
	1 < 5	\$4,000
	5 < 10	\$5,000
	10 < 40	\$6,500
	40 < 100	\$8,000
	≥ 100	\$10,000

Organized Sewage Collection Systems and Modifications

<i>Project</i>	<i>Cost per Linear Foot</i>	<i>Minimum Fee- Maximum Fee</i>
Sewage Collection Systems	\$0.50	\$650 - \$6,500

Underground and Aboveground Storage Tank System Facility Plans and Modifications

<i>Project</i>	<i>Cost per Tank or Piping System</i>	<i>Minimum Fee- Maximum Fee</i>
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500

Exception Requests

<i>Project</i>	<i>Fee</i>
Exception Request	\$500

Extension of Time Requests

<i>Project</i>	<i>Fee</i>
Extension of Time Request	\$150

10 – TCEQ - 10400 Attachments

Core Data Form



TCEQ Use Only

TCEQ Core Data Form

For detailed instructions regarding completion of this form, please read the Core Data Form Instructions or call 512-239-5175.

SECTION I: General Information

1. Reason for Submission (If other is checked please describe in space provided.)		
☒ New Permit, Registration or Authorization (Core Data Form should be submitted with the program application.)		
☐ Renewal (Core Data Form should be submitted with the renewal form)	☐ Other	
2. Customer Reference Number (if issued)	Follow this link to search for CN or RN numbers in Central Registry**	3. Regulated Entity Reference Number (if issued)
CN		RN

SECTION II: Customer Information

4. General Customer Information		5. Effective Date for Customer Information Updates (mm/dd/yyyy)		7/25/2025	
☒ New Customer		☐ Update to Customer Information		☐ Change in Regulated Entity Ownership	
☐ Change in Legal Name (Verifiable with the Texas Secretary of State or Texas Comptroller of Public Accounts)					
The Customer Name submitted here may be updated automatically based on what is current and active with the Texas Secretary of State (SOS) or Texas Comptroller of Public Accounts (CPA).					
6. Customer Legal Name (If an individual, print last name first: eg: Doe, John)				<i>If new Customer, enter previous Customer below:</i>	
O'Reilly Auto Enterprises, LLC					
7. TX SOS/CPA Filing Number		8. TX State Tax ID (11 digits)		9. Federal Tax ID (9 digits)	
18602213128					
11. Type of Customer:		☒ Corporation		☐ Individual	
Government: ☐ City ☐ County ☐ Federal ☐ State ☐ Other		☐ Sole Proprietorship		Partnership: ☐ General ☒ Limited	
12. Number of Employees		☒ 0-20 ☐ 21-100 ☐ 101-250 ☐ 251-500 ☐ 501 and higher		13. Independently Owned and Operated?	
				☐ Yes ☒ No	
14. Customer Role (Proposed or Actual) – as it relates to the Regulated Entity listed on this form. Please check one of the following					
☐ Owner		☐ Operator		☒ Owner & Operator	
☐ Occupational Licensee		☐ Responsible Party		☐ Voluntary Cleanup Applicant ☐ Other: DEVELOPER	
15. Mailing Address:					
233 S Patterson Ave					
City		Springfield		State MO ZIP 65802 ZIP + 4 2298	
16. Country Mailing Information (if outside USA)				17. E-Mail Address (if applicable)	
				Rick Hicks <RHICKS28@OREILLYAUTO.COM>	
18. Telephone Number		19. Extension or Code		20. Fax Number (if applicable)	
(417) 427-8062		65680		() -	

SECTION III: Regulated Entity Information

21. General Regulated Entity Information (If 'New Regulated Entity' is selected below this form should be accompanied by a permit application)	
☒ New Regulated Entity ☐ Update to Regulated Entity Name ☐ Update to Regulated Entity Information	
The Regulated Entity Name submitted may be updated in order to meet TCEQ Agency Data Standards (removal of organizational endings such as Inc, LP, or LLC).	
22. Regulated Entity Name (Enter name of the site where the regulated action is taking place.)	
O'Reilly Auto Enterprises	

23. Street Address of the Regulated Entity: (No PO Boxes)	233 S Patterson Ave							
	City	Springfield	State	MO	ZIP	65802	ZIP + 4	2298
24. County								

Enter Physical Location Description if no street address is provided.

25. Description to Physical Location:	This project site is located at 5510 Williams Dr.									
26. Nearest City	Georgetown					State	TX	Nearest ZIP Code	78633	
27. Latitude (N) In Decimal:	30.702849			28. Longitude (W) In Decimal:	-97.74559					
Degrees	Minutes	Seconds	Degrees	Minutes	Seconds					
30	42	10	97	44	44					
29. Primary SIC Code (4 digits)	5531		30. Secondary SIC Code (4 digits)			31. Primary NAICS Code (5 or 6 digits)	441310		32. Secondary NAICS Code (5 or 6 digits)	
33. What is the Primary Business of this entity? (Do not repeat the SIC or NAICS description.)										
Commercial										
34. Mailing Address:	233 S Patterson Ave									
	City	Springfield	State	MO	ZIP	65802	ZIP + 4	2298		
35. E-Mail Address:	Rick Hicks <RHICKS28@OREILLYAUTO.COM>									
36. Telephone Number	(417) 427-8062		37. Extension or Code	65680		38. Fax Number (if applicable)	() -			

39. TCEQ Programs and ID Numbers Check all Programs and write in the permits/registration numbers that will be affected by the updates submitted on this form. See the Core Data Form instructions for additional guidance.

☐ Dam Safety	☐ Districts	☒ Edwards Aquifer	☐ Emissions Inventory Air	☐ Industrial Hazardous Waste
☐ Municipal Solid Waste	☐ New Source Review Air	☐ OSSF	☐ Petroleum Storage Tank	☐ PWS
☐ Sludge	☒ Storm Water	☐ Title V Air	☐ Tires	☐ Used Oil
☐ Voluntary Cleanup	☒ Waste Water	☐ Wastewater Agriculture	☐ Water Rights	☐ Other:

SECTION IV: Preparer Information

40. Name:	Marisa Keiser, P.E.		41. Title:	Team Lead
42. Telephone Number	43. Ext./Code	44. Fax Number	45. E-Mail Address	
(512) 327-1180		() -	mkeiser@bowman.com	

SECTION V: Authorized Signature

46. By my signature below, I certify, to the best of my knowledge, that the information provided in this form is true and complete, and that I have signature authority to submit this form on behalf of the entity specified in Section II, Field 6 and/or as required for the updates to the ID numbers identified in field 39.

Company:	Bowman Consulting	Job Title:	Team Lead
Name (In Print):	Marisa Keiser, P.E.	Phone:	(512) 327- 1180
Signature:		Date:	