

Texas Commission on Environmental Quality

Edwards Aquifer Application Cover Page

Our Review of Your Application

The Edwards Aquifer Program staff conducts an administrative and technical review of all applications. The turnaround time for administrative review can be up to 30 days as outlined in 30 TAC 213.4(e). Generally administrative completeness is determined during the intake meeting or within a few days of receipt. The turnaround time for technical review of an administratively complete Edwards Aquifer application is 90 days as outlined in 30 TAC 213.4(e). Please know that the review and approval time is directly impacted by the quality and completeness of the initial application that is received. In order to conduct a timely review, it is imperative that the information provided in an Edwards Aquifer application include final plans, be accurate, complete, and in compliance with [30 TAC 213](#).

Administrative Review

1. [Edwards Aquifer applications](#) must be deemed administratively complete before a technical review can begin. To be considered administratively complete, the application must contain completed forms and attachments, provide the requested information, and meet all the site plan requirements. The submitted application and plan sheets should be final plans. Please submit one full-size set of plan sheets with the original application, and half-size sets with the additional copies.

To ensure that all applicable documents are included in the application, the program has developed tools to guide you and web pages to provide all forms, checklists, and guidance. Please visit the below website for assistance: <http://www.tceq.texas.gov/field/eapp>.
2. This Edwards Aquifer Application Cover Page form (certified by the applicant or agent) must be included in the application and brought to the administrative review meeting.
3. Administrative reviews are scheduled with program staff who will conduct the review. Applicants or their authorized agent should call the appropriate regional office, according to the county in which the project is located, to schedule a review. The average meeting time is one hour.
4. In the meeting, the application is examined for administrative completeness. Deficiencies will be noted by staff and emailed or faxed to the applicant and authorized agent at the end of the meeting, or shortly after. Administrative deficiencies will cause the application to be deemed incomplete and returned.

An appointment should be made to resubmit the application. The application is re-examined to ensure all deficiencies are resolved. The application will only be deemed administratively complete when all administrative deficiencies are addressed.
5. If an application is received by mail, courier service, or otherwise submitted without a review meeting, the administrative review will be conducted within 30 days. The applicant and agent will be contacted with the results of the administrative review. If the application is found to be administratively incomplete, it can be retrieved from the regional office or returned by regular mail. If returned by mail, the regional office may require arrangements for return shipping.
6. If the geologic assessment was completed before October 1, 2004 and the site contains "possibly sensitive" features, the assessment must be updated in accordance with the *Instructions to Geologists* (TCEQ-0585 Instructions).

Technical Review

1. When an application is deemed administratively complete, the technical review period begins. The regional office will distribute copies of the application to the identified affected city, county, and groundwater conservation district whose jurisdiction includes the subject site. These entities and the public have 30 days to provide comments on the application to the regional office. All comments received are reviewed by TCEQ.
2. A site assessment is usually conducted as part of the technical review, to evaluate the geologic assessment and observe existing site conditions. The site must be accessible to our staff. The site boundaries should be

clearly marked, features identified in the geologic assessment should be flagged, roadways marked and the alignment of the Sewage Collection System and manholes should be staked at the time the application is submitted. If the site is not marked the application may be returned.

3. We evaluate the application for technical completeness and contact the applicant and agent via Notice of Deficiency (NOD) to request additional information and identify technical deficiencies. There are two deficiency response periods available to the applicant. There are 14 days to resolve deficiencies noted in the first NOD. If a second NOD is issued, there is an additional 14 days to resolve deficiencies. If the response to the second notice is not received, is incomplete or inadequate, or provides new information that is incomplete or inadequate, the application must be withdrawn or will be denied. Please note that because the technical review is underway, whether the application is withdrawn or denied **the application fee will be forfeited.**
4. The program has 90 calendar days to complete the technical review of the application. If the application is technically adequate, such that it complies with the Edwards Aquifer rules, and is protective of the Edwards Aquifer during and after construction, an approval letter will be issued. Construction or other regulated activity may not begin until an approval is issued.

Mid-Review Modifications

It is important to have final site plans prior to beginning the permitting process with TCEQ to avoid delays.

Occasionally, circumstances arise where you may have significant design and/or site plan changes after your Edwards Aquifer application has been deemed administratively complete by TCEQ. This is considered a "Mid-Review Modification". Mid-Review Modifications may require redistribution of an application that includes the proposed modifications for public comment.

If you are proposing a Mid-Review Modification, two options are available:

- If the technical review has begun your application can be denied/withdrawn, your fees will be forfeited, and the plan will have to be resubmitted.
- TCEQ can continue the technical review of the application as it was submitted, and a modification application can be submitted at a later time.

If the application is denied/withdrawn, the resubmitted application will be subject to the administrative and technical review processes and will be treated as a new application. The application will be redistributed to the affected jurisdictions.

Please contact the regional office if you have questions. If your project is located in Williamson, Travis, or Hays County, contact TCEQ's Austin Regional Office at 512-339-2929. If your project is in Comal, Bexar, Medina, Uvalde, or Kinney County, contact TCEQ's San Antonio Regional Office at 210-490-3096

Please fill out all required fields below and submit with your application.

1. Regulated Entity Name: Veritas Academy					2. Regulated Entity No.: RN108353129				
3. Customer Name: Veritas Academy					4. Customer No.: CN604817940				
5. Project Type: (Please circle/check one)	New		Modification		Extension		Exception		
6. Plan Type: (Please circle/check one)	WPAP	CZP	SCS	UST	AST	EXP	EXT	Technical Clarification	Optional Enhanced Measures
7. Land Use: (Please circle/check one)	Residential		Non-residential			8. Site (acres):		96.60 Acres	
9. Application Fee:	\$650		10. Permanent BMP(s):			Retention Irrigation			
11. SCS (Linear Ft.):	520		12. AST/UST (No. Tanks):						
13. County:	Hays		14. Watershed:			Bear Creek			

Application Distribution

Instructions: Use the table below to determine the number of applications required. One original and one copy of the application, plus additional copies (as needed) for each affected incorporated city, county, and groundwater conservation district are required. Linear projects or large projects, which cross into multiple jurisdictions, can require additional copies. Refer to the "Texas Groundwater Conservation Districts within the EAPP Boundaries" map found at:

http://www.tceq.texas.gov/assets/public/compliance/field_ops/eapp/EAPP%20GWCD%20map.pdf

For more detailed boundaries, please contact the conservation district directly.

Austin Region			
County:	Hays	Travis	Williamson
Original (1 req.)	—	—	—
Region (1 req.)	—	—	—
County(ies)	—	—	—
Groundwater Conservation District(s)	☐ Edwards Aquifer Authority ☐ Barton Springs/ Edwards Aquifer ☐ Hays Trinity ☐ Plum Creek	☐ Barton Springs/ Edwards Aquifer	NA
City(ies) Jurisdiction	☐ Austin ☐ Buda ☐ Dripping Springs ☐ Kyle ☐ Mountain City ☐ San Marcos ☐ Wimberley ☐ Woodcreek	☐ Austin ☐ Bee Cave ☐ Pflugerville ☐ Rollingwood ☐ Round Rock ☐ Sunset Valley ☐ West Lake Hills	☐ Austin ☐ Cedar Park ☐ Florence ☐ Georgetown ☐ Jerrell ☐ Leander ☐ Liberty Hill ☐ Pflugerville ☐ Round Rock

San Antonio Region					
County:	Bexar	Comal	Kinney	Medina	Uvalde
Original (1 req.)	—	—	—	—	—
Region (1 req.)	—	—	—	—	—
County(ies)	—	—	—	—	—
Groundwater Conservation District(s)	☐ Edwards Aquifer Authority ☐ Trinity-Glen Rose	☐ Edwards Aquifer Authority	☐ Kinney	☐ EAA Medina	☐ EAA Uvalde
City(ies) Jurisdiction	☐ Castle Hills ☐ Fair Oaks Ranch ☐ Helotes ☐ Hill Country Village ☐ Hollywood Park ☐ San Antonio (SAWS) ☐ Shavano Park	☐ Bulverde ☐ Fair Oaks Ranch ☐ Garden Ridge ☐ New Braunfels ☐ Schertz	NA	☐ San Antonio ETJ (SAWS)	NA

I certify that to the best of my knowledge, that the application is complete and accurate. This application is hereby submitted to TCEQ for administrative review and technical review.

Reese Hurley, PE

Print Name of Customer/Authorized Agent



4/16/2026

Signature of Customer/Authorized Agent

Date

****FOR TCEQ INTERNAL USE ONLY****

Date(s) Reviewed:		Date Administratively Complete:	
Received From:		Correct Number of Copies:	
Received By:		Distribution Date:	
EAPP File Number:		Complex:	
Admin. Review(s) (No.):		No. AR Rounds:	
Delinquent Fees (Y/N):		Review Time Spent:	
Lat./Long. Verified:		SOS Customer Verification:	
Agent Authorization Complete/Notarized (Y/N):		Fee Check:	Payable to TCEQ (Y/N):
Core Data Form Complete (Y/N):			Signed (Y/N):
Core Data Form Incomplete Nos.:			Less than 90 days old (Y/N):

General Information Form

Texas Commission on Environmental Quality

For Regulated Activities on the Edwards Aquifer Recharge and Transition Zones and Relating to 30 TAC §213.4(b) & §213.5(b)(2)(A), (B) Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

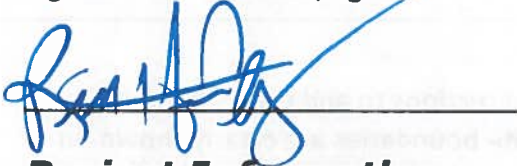
Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **General Information Form** is hereby submitted for TCEQ review. The application was prepared by:

Print Name of Customer/Agent: Reese Hurley, P.E.

Date: 04/16/2026

Signature of Customer/Agent:



Project Information

1. Regulated Entity Name: Veritas Academy
2. County: Hays
3. Stream Basin: Bear Creek
4. Groundwater Conservation District (If applicable): Barton Springs/Edwards Aquifer
5. Edwards Aquifer Zone:
 - ☒ Recharge Zone
 - ☐ Transition Zone
6. Plan Type:
 - ☐ WPAP
 - ☒ SCS
 - ☒ Modification
 - ☐ AST
 - ☐ UST
 - ☐ Exception Request

7. Customer (Applicant):

Contact Person: Scott Rhodes

Entity: Veritas Academy

Mailing Address: 13401 Escarpment Blvd

City, State: Austin, TX

Zip: 78739

Telephone: 512-891-1673

FAX: 512-891-1693

Email Address: scott.rhodes@veritasacademy.net

8. Agent/Representative (If any):

Contact Person: Reese Hurley, P.E.

Entity: LJA Engineering, Inc.

Mailing Address: 500 Rialto Blvd., Building II, Suite 100

City, State: Austin, Texas

Zip: 78735

Telephone: 512-439-4734

FAX: 512-439-4716

Email Address: rhurley@lja.com

9. Project Location:

- ☒ The project site is located inside the city limits of Austin.
- ☐ The project site is located outside the city limits but inside the ETJ (extra-territorial jurisdiction) of _____.
- ☐ The project site is not located within any city's limits or ETJ.

10. ☒ The location of the project site is described below. The description provides sufficient detail and clarity so that the TCEQ's Regional staff can easily locate the project and site boundaries for a field investigation.

13401 Escarpment Blvd. Austin, TX 78739

11. ☒ **Attachment A – Road Map.** A road map showing directions to and the location of the project site is attached. The project location and site boundaries are clearly shown on the map.
12. ☒ **Attachment B - USGS / Edwards Recharge Zone Map.** A copy of the official 7 ½ minute USGS Quadrangle Map (Scale: 1" = 2000') of the Edwards Recharge Zone is attached. The map(s) clearly show:
- ☒ Project site boundaries.
- ☒ USGS Quadrangle Name(s).
- ☒ Boundaries of the Recharge Zone (and Transition Zone, if applicable).
- ☒ Drainage path from the project site to the boundary of the Recharge Zone.
13. ☒ **The TCEQ must be able to inspect the project site or the application will be returned.** Sufficient survey staking is provided on the project to allow TCEQ regional staff to locate the boundaries and alignment of the regulated activities and the geologic or manmade features noted in the Geologic Assessment.
- ☒ Survey staking will be completed by this date: As needed

14. ☒ **Attachment C – Project Description.** Attached at the end of this form is a detailed narrative description of the proposed project. The project description is consistent throughout the application and contains, at a minimum, the following details:

- ☒ Area of the site
- ☒ Offsite areas
- ☒ Impervious cover
- ☒ Permanent BMP(s)
- ☒ Proposed site use
- ☒ Site history
- ☒ Previous development
- ☒ Area(s) to be demolished

15. Existing project site conditions are noted below:

- ☒ Existing commercial site
- ☐ Existing industrial site
- ☐ Existing residential site
- ☒ Existing paved and/or unpaved roads
- ☐ Undeveloped (Cleared)
- ☐ Undeveloped (Undisturbed/Uncleared)
- ☐ Other: _____

Prohibited Activities

16. ☒ I am aware that the following activities are prohibited on the Recharge Zone and are not proposed for this project:

- (1) Waste disposal wells regulated under 30 TAC Chapter 331 of this title (relating to Underground Injection Control);
- (2) New feedlot/concentrated animal feeding operations, as defined in 30 TAC §213.3;
- (3) Land disposal of Class I wastes, as defined in 30 TAC §335.1;
- (4) The use of sewage holding tanks as parts of organized collection systems; and
- (5) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41(b), (c), and (d) of this title (relating to Types of Municipal Solid Waste Facilities).
- (6) New municipal and industrial wastewater discharges into or adjacent to water in the state that would create additional pollutant loading.

17. ☒ I am aware that the following activities are prohibited on the Transition Zone and are not proposed for this project:

- (1) Waste disposal wells regulated under 30 TAC Chapter 331 (relating to Underground Injection Control);
- (2) Land disposal of Class I wastes, as defined in 30 TAC §335.1; and

- (3) New municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41 (b), (c), and (d) of this title.

Administrative Information

18. The fee for the plan(s) is based on:

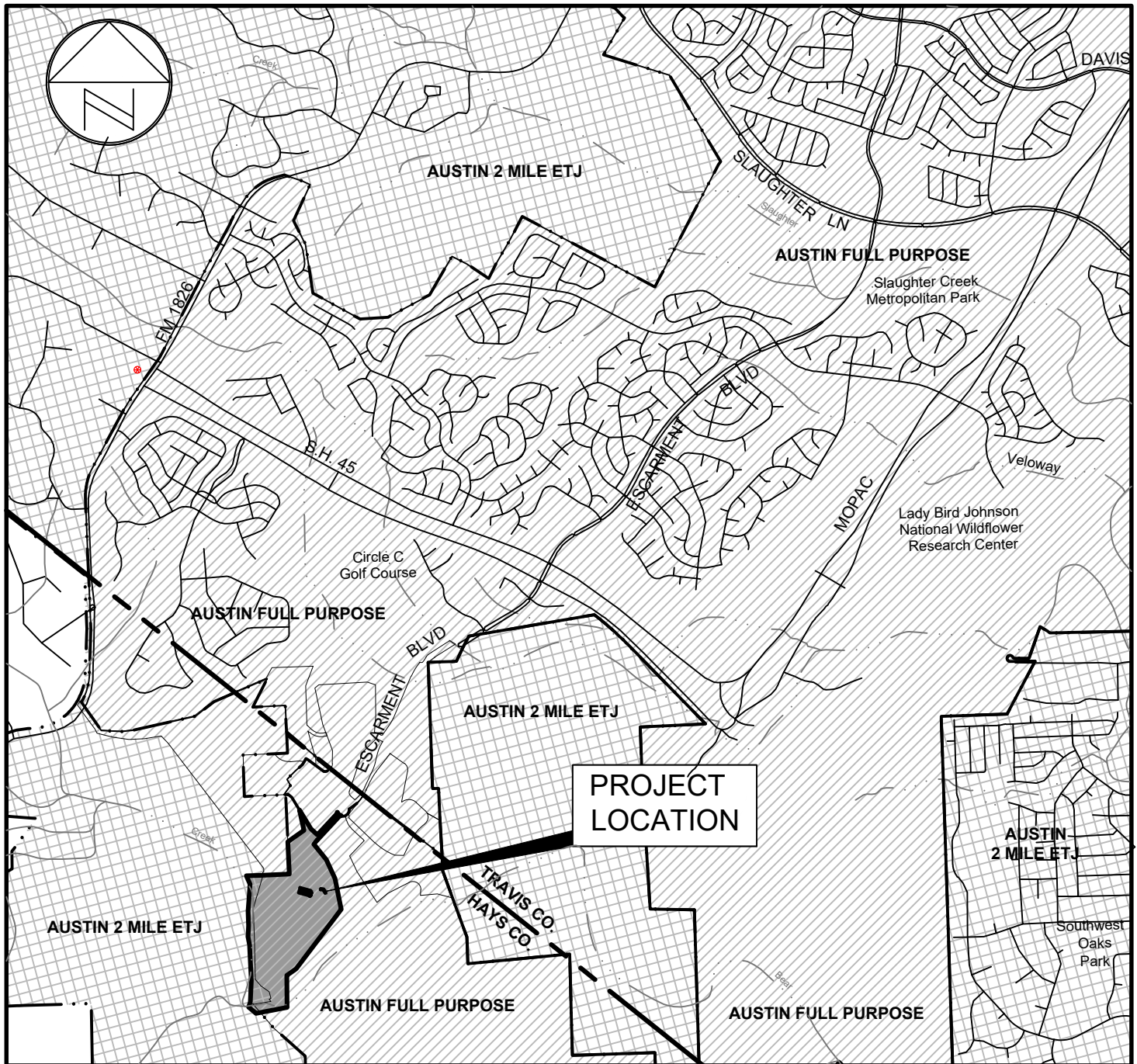
- ☐ For a Water Pollution Abatement Plan or Modification, the total acreage of the site where regulated activities will occur.
- ☒ For an Organized Sewage Collection System Plan or Modification, the total linear footage of all collection system lines.
- ☐ For a UST Facility Plan or Modification or an AST Facility Plan or Modification, the total number of tanks or piping systems.
- ☐ A request for an exception to any substantive portion of the regulations related to the protection of water quality.
- ☐ A request for an extension to a previously approved plan.

19. ☒ Application fees are due and payable at the time the application is filed. If the correct fee is not submitted, the TCEQ is not required to consider the application until the correct fee is submitted. Both the fee and the Edwards Aquifer Fee Form have been sent to the Commission's:

- ☐ TCEQ cashier
- ☒ Austin Regional Office (for projects in Hays, Travis, and Williamson Counties)
- ☐ San Antonio Regional Office (for projects in Bexar, Comal, Kinney, Medina, and Uvalde Counties)

20. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

21. ☒ No person shall commence any regulated activity until the Edwards Aquifer Protection Plan(s) for the activity has been filed with and approved by the Executive Director.



(NTS)

GRID NO.: A-13, A-14, WZ13

MAPSCO PAGE: 670V, 670Z, 671N, 671S, 671W

LJA Engineering, Inc.

5316 Highway 290 West
Suite 150
Austin, Texas 78735



Phone 512.439.4700
Fax 512.439.4716
FRN - F-1386

Veritas Academy
Gym Addition and Campus Improvements
Road Map

13401 Escarpment Boulevard, Austin TX

**FORM 0587,
ATTACHMENT A**

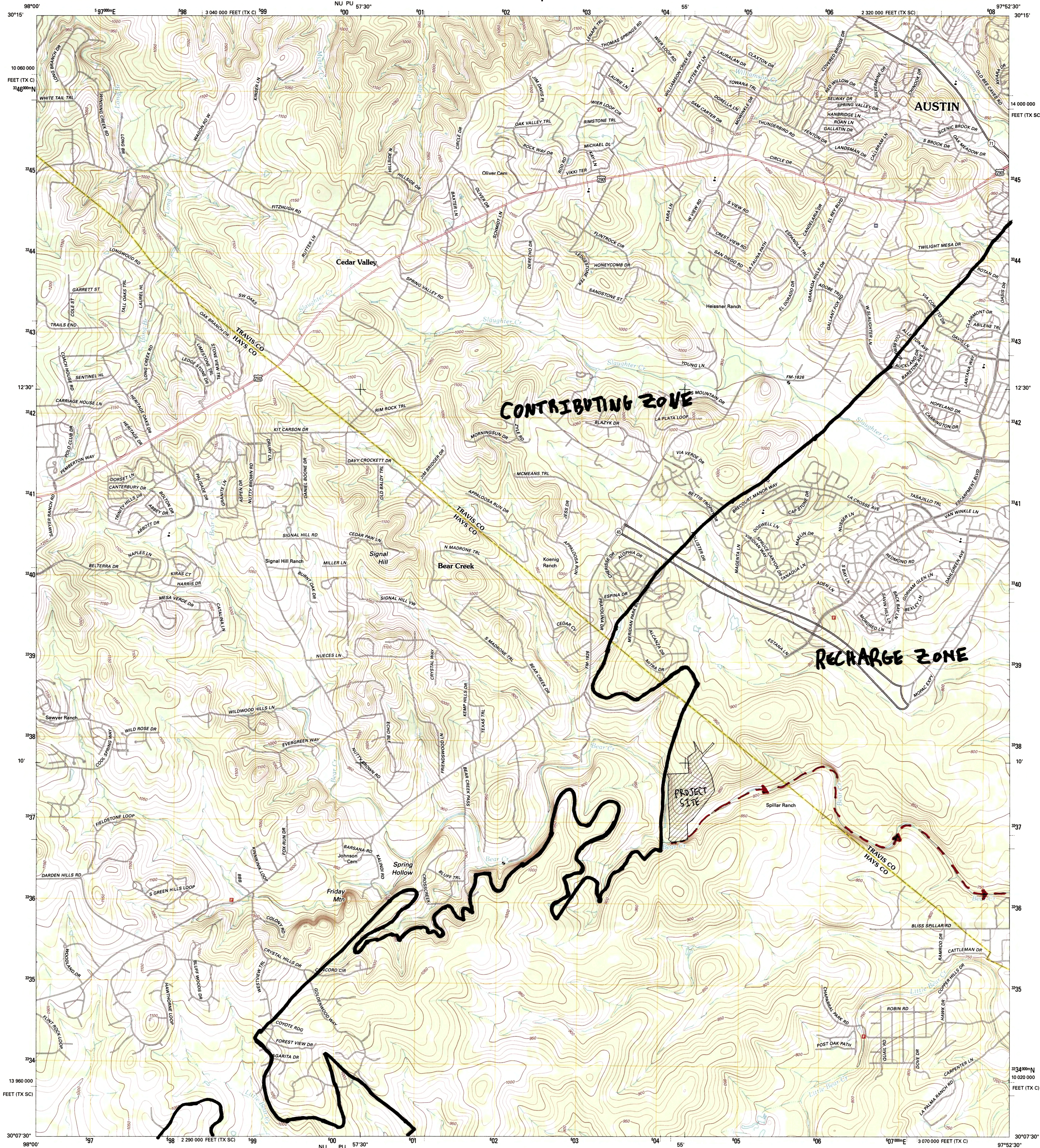
1 OF 3



U.S. DEPARTMENT OF THE INTERIOR
U. S. GEOLOGICAL SURVEY

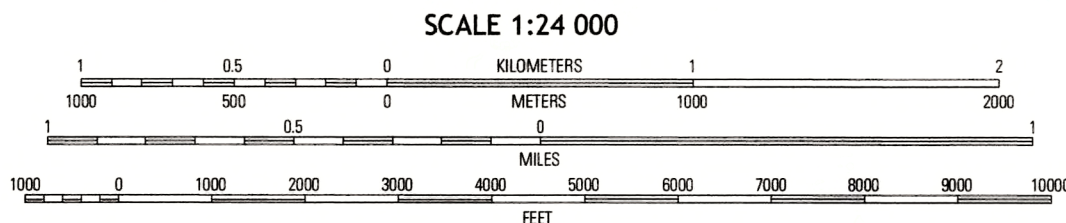
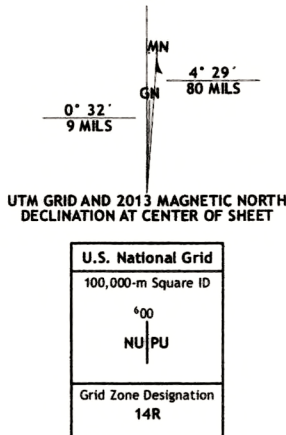


SIGNAL HILL QUADRANGLE
TEXAS
7.5-MINUTE SERIES



Produced by the United States Geological Survey
North American Datum of 1983 (NAD83)
World Geodetic System of 1984 (WGS84). Projection and
1 000-meter grid: Universal Transverse Mercator, Zone 14R
10 000-foot ticks: Texas Coordinate System of 1983 (south
central and central zones)

Imagery.....NAIP, May 2010
Roads.....©2006-2012 TomTom
Names.....©2012
Hydrography.....National Hydrography Dataset, 2010
Contours.....National Elevation Dataset, 2002
Boundaries.....Census, IBWC, IBC, USGS, 1972 - 2012



CONTOUR INTERVAL 10 FEET
NORTH AMERICAN VERTICAL DATUM OF 1988
This map was produced to conform with the
National Geospatial Program US Topo Product Standard, 2011.
A metadata file associated with this product is draft version 0.6.7



QUADRANGLE LOCATION		
Signal Hill	Bear Creek	Austin
Dripping Springs	Signal Hill	Oak Hill
Dripping Springs	Mountain City	Buda

ROAD CLASSIFICATION
Interstate Route
US Route
Ramp
State Route
Local Road
4WD
US Route
State Route

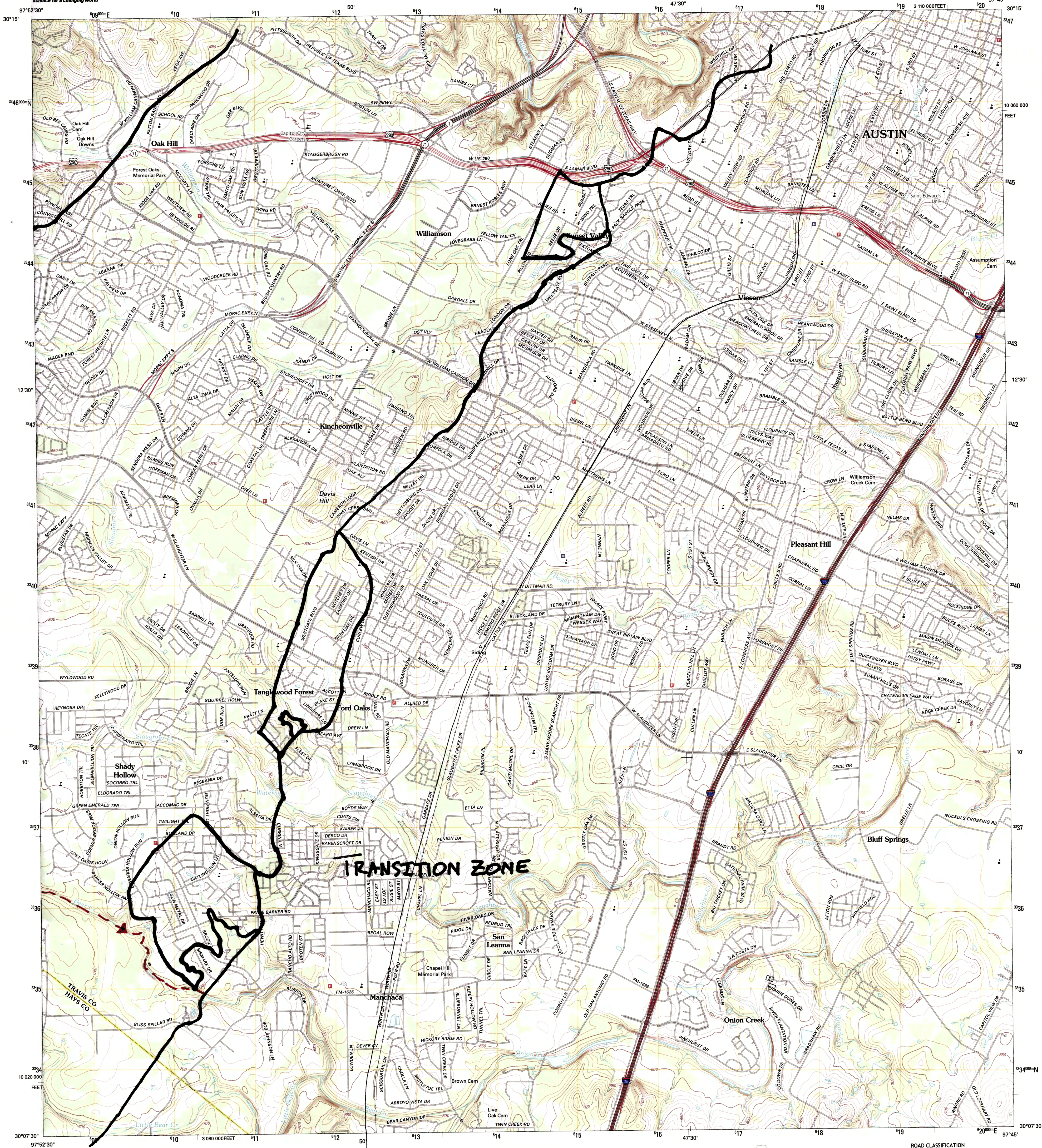
SIGNAL HILL, TX
2013



U.S. DEPARTMENT OF THE INTERIOR
U. S. GEOLOGICAL SURVEY

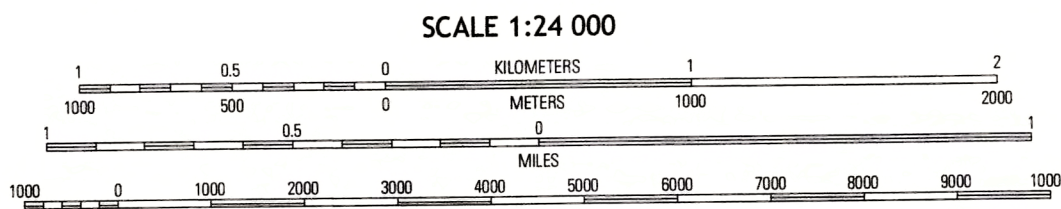
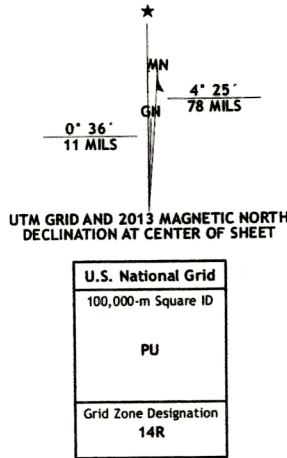


OAK HILL QUADRANGLE
TEXAS
7.5-MINUTE SERIES



Produced by the United States Geological Survey
North American Datum of 1983 (NAD83)
World Geodetic System of 1984 (WGS84) Projection and
1000-meter grid: Universal Transverse Mercator, Zone 14R
10 000-foot ticks: Texas Coordinate System of 1983 (central and
south central zones)

Imagery: NAD, May 2010
Roads: 2006-2012 TomTom
Names: GNIS, 2012
Hydrography: National Hydrography Dataset, 2010
Contours: National Elevation Dataset, 2002
Boundaries: Census, IBC, USGS, 1972 - 2012



CONTOUR INTERVAL 10 FEET
NORTH AMERICAN DATUM OF 1983
This map was produced to conform with the
National Geospatial Program US Topo Product Standard, 2011.
A metadata file associated with this product is draft version 0.6.7



ROAD CLASSIFICATION			
Interstate Route	State Route	US Route	Local Road
Interstate Route	State Route	US Route	Local Road
Interstate Route	State Route	US Route	Local Road

OAK HILL, TX
2013

ATTACHMENTS – FORM 0587

ATTACHMENT C – Project Description

The proposed Veritas Academy Gym Addition and Campus Improvements site plan is located on 96.60 acres of developed land. The site is located at the southernmost end of the Avana subdivision at 13401 Escarpment Boulevard, Austin, Texas 78739. The project is located in Northern Hays County within the City of Austin's full purpose jurisdiction. The project site is within the Bear Creek Watershed which is classified as the Barton Springs Zone by the City of Austin. The project site is subject to the Bradley Settlement Agreement, an agreement between the original owner and the City of Austin regarding the required water quality controls. To meet the requirements of the agreement, the water quality controls have been designed in conformance with the City of Austin's SOS Ordinance requirements. The proposed limits of construction will encompass **15.39** acres.

The site was undeveloped until Veritas Academy, a private school, was constructed in 2018. The previous development constructed 3.982 acres of impervious area consisting of a main school building, auditorium, eight pre-manufactured units with decking serving as school classrooms/offices/facilities, parking, and a drive aisle. Batting cages, an outdoor pavilion, and two faculty offices have also been constructed. The proposed improvements will include the removal of 0.661 acres of impervious area and construction of 2.904 acres of impervious area to consist of a gym, expanded driveway immediately surrounding the guard shack, relocated pre-manufactured units with wood decking to serve as school classrooms, two separate student and faculty restrooms, parking, a looped drive aisle and drop-off area, required fire lanes, and required accessible paths. A portion of the existing drive aisle, sidewalks, wood decking, **320** linear feet of 8-inch water line, **520** linear feet of 8-inch and 6-inch combined wastewater line, a restroom, a 3:1 safety end treatment, storm sewer pipes, and several inlets will be demolished. A transformer, two meter banks, two fire hydrants, and two portable classrooms will be relocated onsite. After the proposed improvements are constructed, the site will consist of **5.919** acres of impervious area.

Currently, storm sewer pipes and open channels convey developed runoff to an existing onsite water quality pond, which overflows into the existing detention pond during larger storm events. The northern portion of the site driveway sheet flows onto a vegetative filter strip located just west of the driveway and is not detained onsite. Developed runoff from the remainder of the site is conveyed to the existing retention/irrigation water quality pond. The existing onsite retention/irrigation pond includes sedimentation and filtration chambers for purposes of pre-treatment.

Onsite detention pond improvements are designed to keep developed conditions peak runoff rates to be no greater than the pre-developed rates. The project shall capture developed storm water runoff from onsite impervious cover and landscape areas using proposed and existing inlets. The runoff in drainage area **D2 (8.98 acres)** shall be conveyed via open channels and underground storm sewer pipe to an existing retention/irrigation water quality pond. The runoff from drainage area **D3 (4.46 acres)** shall be conveyed via open channels and underground storm sewer pipe to a proposed retention/irrigation water quality pond. Both water quality ponds will discharge into an existing onsite detention pond. HEC-HMS software was utilized to model the detention pond improvements. The existing detention pond has primarily 3:1 side slopes. To account for additional onsite runoff from the proposed improvements, the sides of the detention pond will be excavated, and walls will be installed to increase the detention volume.

The runoff associated with drainage area D4, located at the north portion of the site, is not conveyed to these ponds. The impervious improvements in this northern drainage area are related to the driveway improvements immediately surrounding the guard shack. The runoff from this drainage area is conveyed through an existing open channel, existing culvert, and existing flow spreader to an existing vegetative filter strip to provide water quality.

An existing 6" domestic water line and an existing 8" domestic fire line currently serve the site. The existing site has approximately 2,428 LF of water line for the purpose of providing the needed hydrant flows. There are approximately 2,256 linear feet of 4" domestic water service line. 2" domestic service lines extend from the 4" main to serve four of the existing portable units. Water and wastewater utilities are also required for the proposed improvements. The proposed onsite water and fire water lines will connect to the existing lines via proposed "tee joints" and extend through the site for the purpose of serving the proposed gym, restrooms, and relocated fire hydrants. The proposed improvements will install approximately 400 linear feet of 8" fire water line for purposes of providing the needed hydrant flows. Approximately 134 linear feet of 4-inch domestic water service line will be installed to serve the proposed gym. 187 linear feet of 2" domestic service line will be installed from the 4" main to serve the proposed restrooms.

Currently, a lift station with pump, pump controls, wet well, and a force main serve the site. The lift station is located at the south end (the lowest end) of the site. Approximately 2,303 linear feet of existing 3-inch force main mechanically conveys flow northward, where it connects to an existing 3-inch force main. The main school building, auditorium, and individual pre-manufactured units are served by a system of 6" and 8" wastewater collection lines before all flow ultimately reaches the existing lift station. Wastewater generated by the proposed improvements will ultimately gravity flow to the existing onsite sewage collection system. The proposed gym and restrooms will be served by a system of 4" and 6" wastewater collection lines before all wastewater flow ultimately reaches the existing lift station. Detailed information concerning the wastewater collection system is contained within the SCS application.

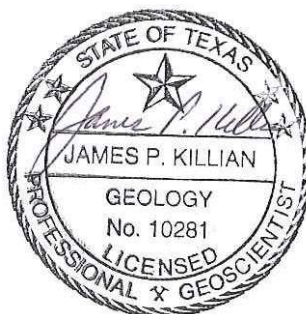
**GEOLOGIC ASSESSMENT
APPROXIMATELY 21.2-ACRE VERITAS ACADEMY AND GYM PROJECT
13401 ESCARPMENT BOULEVARD
AUSTIN, HAYS COUNTY, TEXAS
HJN 23138 GA**

PREPARED FOR:

**VERITAS ACADEMY
AUSTIN, TEXAS**

PREPARED BY:

**HORIZON ENVIRONMENTAL SERVICES, INC.
TBPG FIRM REGISTRATION NO. 50488**



JUNE 2023

TABLE OF CONTENTS

I. GEOLOGIC ASSESSMENT FORM (TCEQ-0585)

II. ATTACHMENTS:

- A GEOLOGIC ASSESSMENT TABLE
- B STRATIGRAPHIC COLUMN
- C DESCRIPTION OF SITE GEOLOGY
- D SITE GEOLOGIC MAP
- E SUPPORTING INFORMATION
- F ADDITIONAL SITE MAPS
- G SITE PHOTOGRAPHS

Geologic Assessment

Texas Commission on Environmental Quality

For Regulated Activities on The Edwards Aquifer Recharge/transition Zones and Relating to 30 TAC §213.5(b)(3), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. My signature certifies that I am qualified as a geologist as defined by 30 TAC Chapter 213.

Print Name of Geologist: James Killian

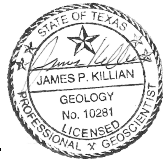
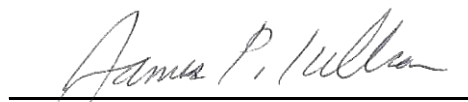
Telephone: 512-328-2430

Date: 15 June 2023

Fax: 512-328-1804

Representing: Horizon Environmental Services, Inc. and TBPG Form Registration No. 50488
(Name of Company and TBPG or TBPE registration number)

Signature of Geologist:



Regulated Entity Name: Approximately 21.2-acre Veritas Academy and Gym Project; 13401 Escarpment Boulevard, Austin, Hays County, Texas

Project Information

1. Date(s) Geologic Assessment was performed: 7 June 2023

2. Type of Project:

- ☒ WPAP
☒ SCS

- ☐ AST
☐ UST

3. Location of Project:

- ☒ Recharge Zone
☐ Transition Zone
☐ Contributing Zone within the Transition Zone

4. ☒ **Attachment A - Geologic Assessment Table.** Completed Geologic Assessment Table (Form TCEQ-0585-Table) is attached.
5. ☒ Soil cover on the project site is summarized in the table below and uses the SCS Hydrologic Soil Groups* (Urban Hydrology for Small Watersheds, Technical Release No. 55, Appendix A, Soil Conservation Service, 1986). If there is more than one soil type on the project site, show each soil type on the site Geologic Map or a separate soils map.

Table 1 - Soil Units, Infiltration Characteristics and Thickness

Soil Name	Group*	Thickness(feet)
Brackett-Rock Outcrop-Comfort Complex (BtD)	D	1.2
Comfort-Rock Outcrop Complex, 1-8% slopes (CrD)	D	1.1

Soil Name	Group*	Thickness(feet)

** Soil Group Definitions (Abbreviated)*

- A. Soils having a high infiltration rate when thoroughly wetted.
- B. Soils having a moderate infiltration rate when thoroughly wetted.
- C. Soils having a slow infiltration rate when thoroughly wetted.
- D. Soils having a very slow infiltration rate when thoroughly wetted.

6. ☒ **Attachment B – Stratigraphic Column.** A stratigraphic column showing formations, members, and thicknesses is attached. The outcropping unit, if present, should be at the top of the stratigraphic column. Otherwise, the uppermost unit should be at the top of the stratigraphic column.
7. ☒ **Attachment C – Site Geology.** A narrative description of the site specific geology including any features identified in the Geologic Assessment Table, a discussion of the potential for fluid movement to the Edwards Aquifer, stratigraphy, structure(s), and karst characteristics is attached.
8. ☒ **Attachment D – Site Geologic Map(s).** The Site Geologic Map must be the same scale as the applicant's Site Plan. The minimum scale is 1": 400'
Applicant's Site Plan Scale: 1" = 400'
Site Geologic Map Scale: 1" = 400'
Site Soils Map Scale (if more than 1 soil type): 1" = 500'
9. Method of collecting positional data:
☒ Global Positioning System (GPS) technology.

- ☐ Other method(s). Please describe method of data collection: _____
10. ☒ The project site and boundaries are clearly shown and labeled on the Site Geologic Map.
11. ☒ Surface geologic units are shown and labeled on the Site Geologic Map.
12. ☐ Geologic or manmade features were discovered on the project site during the field investigation. They are shown and labeled on the Site Geologic Map and are described in the attached Geologic Assessment Table.
- ☒ Geologic or manmade features were not discovered on the project site during the field investigation.
13. ☒ The Recharge Zone boundary is shown and labeled, if appropriate.
14. All known wells (test holes, water, oil, unplugged, capped and/or abandoned, etc.): If applicable, the information must agree with Item No. 20 of the WPAP Application Section.
- ☐ There are 0 (#) wells present on the project site and the locations are shown and labeled. (Check all of the following that apply.)
- ☐ The wells are not in use and have been properly abandoned.
- ☐ The wells are not in use and will be properly abandoned.
- ☐ The wells are in use and comply with 16 TAC Chapter 76.
- ☒ There are no wells or test holes of any kind known to exist on the project site.

Administrative Information

15. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.

ATTACHMENT A
GEOLOGIC ASSESSMENT TABLE

Horizon observed no features on the subject site that meet the TCEQ definition of a potential recharge feature; as such, the TCEQ Geologic Assessment Table was not completed.

ATTACHMENT B
STRATIGRAPHIC COLUMN

Geologic Unit	Hydrologic Unit	Approx. Thickness at Project Site (ft)	Elevation (ft msl)	Depth (ft)
Dolomitic Member (Kkd)	Edwards Aquifer	80	914	0
Basal Nodular Member (Kkbn)		50	834	80
Upper Glen Rose Limestone (Kgru)	Confining Unit	350	784	130
			434	480

Note: Unit elevation and thickness given with respect to a ground surface elevation of 914 ft on the northern property boundary.

	Date: 06/16/2023	Attachment B Stratigraphic Column Veritas Academy and Gym Project Austin, Hays County, Texas	
	Drawn: TRB/KRW		
	HJN NO: 23138		

ATTACHMENT C
DESCRIPTION OF SITE GEOLOGY

Geologic information for the subject site obtained via literature review is provided in Attachment E, Supporting Information.

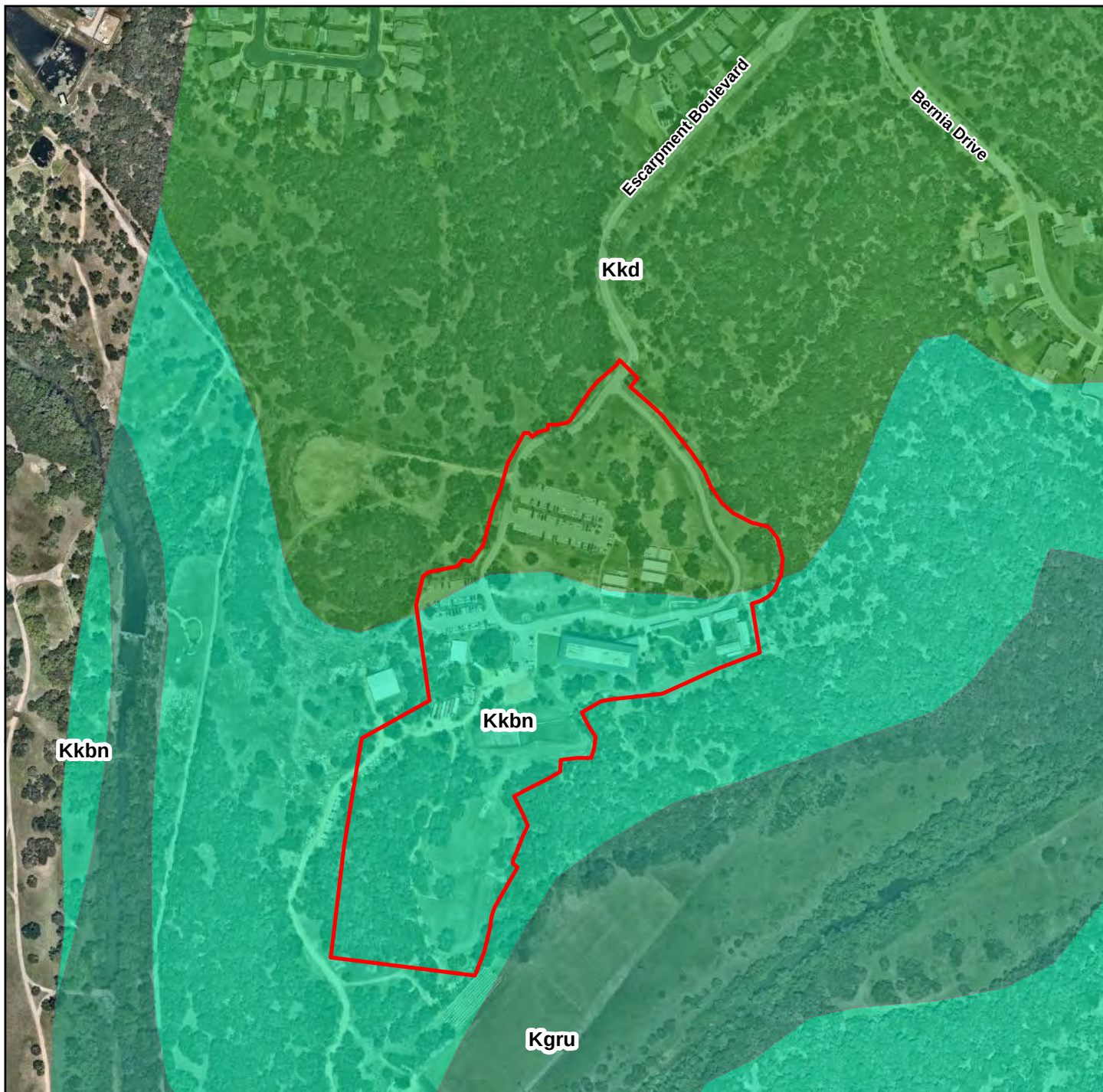
A geologic assessment of approximately 21.2 acres located at 13401 Escarpment Boulevard, Austin, Hays County, Texas, was conducted pursuant to Texas rules for regulated activities in the Edwards Aquifer Recharge Zone (EARZ) (30 TAC 213). The subject site consists of developed land used for a private educational facility. Assessment findings were used to develop recommendations for site construction measures intended to be protective of water resources at the subject site and adjacent areas.

The entire subject site is located within the Edwards Aquifer Recharge Zone (EARZ), as defined by the Texas Commission on Environmental Quality (TCEQ). The EARZ occurs where surface water enters the subsurface through exposed limestone bedrock containing faults, fractures, sinkholes, and caves.

The subject site is underlain by the Dolomitic Member (Kkd) and the Basal Nodular Member (Kkbn) of the Edwards Group-Kanier Formation (Blome, 2005; Stein and Ozuna, 1995; Small and Clark, 2000; Clark, 2003) which have estimated maximum thicknesses of about 80 feet and 50 feet, respectively.

No naturally occurring geologic features or man-made features were identified at this site. Further information pertaining to the subject site is presented in Attachments D, E, and F. Photographs of the subject site are presented in Attachment G.

ATTACHMENT D
SITE GEOLOGIC MAP



Legend

- Subject Site
- Dolomitic Member (Kkd)
- Basal Nodular Member (Kkbn)
- Upper Glen Rose Formation (Kgru)



Horizon
Environmental Services, Inc.

Date: 06/08/2023
 Drawn: ZHB/KRW
 HJN NO: 23138.001 GA
 Source: Nearmap, 2023;
 Blome et al., 2005

Attachment D

Site Geologic Map
 Veritas Academy and Gym Project
 Austin, Hays County, Texas



0 200 400
 Feet

ATTACHMENT E
SUPPORTING INFORMATION

1.0 INTRODUCTION AND METHODOLOGY

This report and any proposed abatement measures are intended to fulfill Texas Commission on Environmental Quality (TCEQ) reporting requirements (TCEQ, 2005). This geologic assessment includes a review of the subject site for potential aquifer recharge and documentation of general geologic characteristics for the subject site. Horizon Environmental Services, Inc. (Horizon) conducted the necessary field and literature studies according to TCEQ *Instructions to Geologists for Geologic Assessments on the Edwards Aquifer Recharge/Transition Zones* (TCEQ, 2004).

Horizon walked transects spaced 50 feet apart, mapped the locations of features using a sub-foot accurate Trimble Geo HX handheld GPS, and posted processed data utilizing GPS Pathfinder Office software, topographic maps, and aerial photographs. Horizon also searched the area around any potential recharge features encountered to look for additional features. When necessary, Horizon removed loose rocks and soil (by hand) to preliminarily assess each feature's subsurface extent while walking transects. However, labor-intensive excavation was not conducted during this assessment. Features that did not meet the TCEQ definition of a potential recharge feature (per TCEQ, 2004), such as surface weathering, karren, or animal burrows, were evaluated in the field and omitted from this report.

The results of this survey do not preclude the possibility of encountering subsurface voids or abandoned test or water wells during the clearing or construction phases of the proposed project. If a subsurface void is encountered during any phase of the project, work should be halted until the TCEQ (or appropriate agency) is contacted and a geologist can investigate the feature.

2.0 ENVIRONMENTAL SETTING

2.1 LOCATION AND GENERAL DESCRIPTION

The subject site consists of approximately 21.2 acres of partially developed land located at 13401 Escarpment Boulevard in Hays County, Texas (Appendix F, Figure 1).

2.2 LAND USE

The subject site is reportedly used for a private educational facility. Eight habitable structures were observed on the site. Escarpment Boulevard runs from the north end to the center of the site. Surrounding lands are generally used for rural and suburban residences, farming, and raising livestock.

2.3 TOPOGRAPHY AND SURFACE WATER

The subject site is situated on gently to moderately sloping terrain within the Bear Creek watershed (Appendix F, Figures 2 and 3). Surface elevations on the subject site vary from a minimum of approximately 840 feet above mean sea level (amsl) within a tributary of Bear Creek near the southeastern property boundary to a maximum of approximately 920 feet amsl near the

northern property boundary. Drainage on the site occurs primarily by overland sheet flow from northwest to southeast in the northern half and from north to south and northwest to southeast in the southern half toward unnamed tributaries of Bear Creek.

2.4 EDWARDS AQUIFER ZONE

The subject site is found within the Edwards Aquifer Recharge Zone (TCEQ, 2023) (Attachment F, Figure 2).

2.5 SURFACE SOILS

Two soil units are mapped within the subject site (NRCS, 2023) (Appendix F, Figure 4). Generally, the soil series are similar in their physical, chemical, and engineering properties, with the principal exception being rock fragment content and thickness. The soil units are described in further detail below.

Brackett-Rock Outcrop-Comfort Complex (BtD) consists of shallow, loamy and clayey soils and Rock outcrop on uplands in the Edwards Plateau. Slopes range from 1 to 8%. Many areas have a benched appearance along hill slopes because of the horizontal bands of Rock outcrop. The Brackett and Comfort soils are between the bands of Rock outcrop. Brackett soil makes up 30 to 60% of the complex. Rock outcrop makes up 10 to 45%. Comfort soil and similar soils make up 10 to 20%. The underlying material is weakly cemented limestone interbedded with thin layers of indurated limestone. The soil complex is moderately alkaline and calcareous throughout. Typically, the areas of Rock outcrop consist of exposures of limestone bedrock. There is some soil material in the narrow fractures in the rock. The soils in this complex are well-drained. Surface runoff is medium to rapid. Permeability is moderately slow in Brackett soil and slow in Comfort soil. The available water capacity is very low. Water erosion is a severe hazard. Seeps are common along the slopes after heavy rainfall. The medium to rapid runoff, the very low available water capacity, the shallow rooting zone, and the outcrops of rock are severe limitations, which affect roads and streets, corrosivity to uncoated steel, outcrops of rock, and slope (Batte et al., 1984).

Comfort-Rock Outcrop Complex, 1 to 8% slopes (CrD) consists of shallow, clayey soils and Rock outcrop on side slopes and on hilltops and ridgetops on uplands in the Edwards Plateau. Comfort extremely stony clay makes up 49 to more than 95% of the complex. Rock outcrop and areas of soil less than 4 inches deep make up 5 to 36%. Rumple, Purves, Eckrant, and Real soils make up less than 5 to 30%. The areas of Rock outcrop are long, narrow horizontal bands on hill slopes and along small drains. Cobbles and stones as much as 4 feet across cover about 45% of the surface. The subsoil extends to a depth of 13 inches. The underlying material is indurated fractured limestone. The soil is mildly alkaline and noncalcareous throughout. The Comfort soil is well-drained. Surface runoff is slow to medium. Permeability is slow, and the available water capacity is very low. Water erosion is a slight hazard. Typically, Rock outcrop is dolomitic limestone that is barren of soil except in narrow fractures in the rock. The areas of Rock outcrop, cobbles and stones on the surface and in the soil, the limited rooting depth, and the very low available water capacity are severe limitations. The stony surface layer, shallowness to bedrock,

and corrosivity to uncoated steel are severe limitations to use of the soils for recreation purposes (Batte et al., 1984).

2.6 WATER WELLS

A review of TCEQ and Texas Water Development Board (TWDB) records revealed no water wells on the subject site and 15 wells within 0.5 miles of the subject site (TCEQ, 2023; TWDB, 2023). According to the TWDB records, all the off-site wells are reportedly completed within the Edwards Aquifer and/or Trinity Aquifer at total depths ranging from 100 to 895 feet below surface. Horizon observed no wells on the subject site.

The results of this assessment do not preclude the existence of undocumented/abandoned wells on the site. If a water well or casing is encountered during construction, work should be halted near the feature until the TCEQ is contacted.

2.7 GEOLOGY

Literature Review

A review of existing literature shows the subject site is underlain by 2 geologic members of the Edwards Group-Kainer Formation (Dolomitic Member [Kkd] and Basal Nodular Member [Kkbn]) (Blome et al., 2005). The estimated maximum thickness of the Edwards Group-Kainer Formation is about 130 feet at higher elevations located near the northern boundary of the subject site.

The Dolomitic Member (Kkd) is mudstone to grainstone and chert-bearing, crystalline limestone. The massive bedded Dolomitic Member weathers light gray in outcrop and has abundant *Toucasia*. Cavern development is directly related to faults, fractures, and bedding planes; thus, it is considered to have nonfabric-selective porosity except where solutions along bedding planes yield water. The thickness of the member ranges from 110 to 140 feet (Stein and Ozuna, 1995; Small and Clark, 2000).

The Basal Nodular Member (Kkbn) is shaly, nodular limestone and burrowed mudstone to wackestone. It exhibits minor lateral cavern development at surface and nonfabric-selective porosity. It is identified in the field as gray nodular mudstone, containing black rotund bodies and *miliolids*, gastropods, and *Exogyra texana*. The member is considered regionally as a lower confining unit and is locally water bearing through dissolution along bedding planes. The thickness of this member ranges from 20 to 70 feet (Stein and Ozuna, 1995; Clark, 2003).

In general, the rock strata beneath the site dip to the southeast at about 10 to 30 feet per mile. The subject site is located within the Balcones Fault Zone and available geologic reports indicate the immediate area has been affected by geologically inactive, normal faulting. A normal fault is an inclined fault in which the hanging wall appears to have slipped downward relative to the footwall. The nearest mapped faults are approximately 0.5 miles to the west and 0.5 miles to the east, striking N30°E and N20°E, respectively (Blome et al., 2005).

The site Stratigraphic Column is provided as Attachment B, and the Site Geologic Map is Attachment D.

Field Assessment

Horizon observed no man-made or geologic features on the subject site that meet the TCEQ definition of a potential recharge feature.

3.0 CONCLUSIONS AND RECOMMENDATIONS

No geologic or man-made features were identified at the subject site that would require protection or mitigation pursuant to TCEQ rules for protection of the Edwards Aquifer (30 TAC 213). The site generally appears well-suited to development prospectuses. It should be noted that soil and drainage erosion would increase with ground disturbance. Native grasses and the cobbly content of the soil aid to prevent erosion. Soil and sedimentation fencing should be placed in all appropriate areas prior to any site disturbing activities.

Because the subject site is located over the Edwards Aquifer Recharge Zone, it is possible that subsurface voids underlie the site. If any subsurface voids are encountered during site development, work should halt immediately so that a geologist may assess the potential for the void(s) to provide meaningful contribution to the Edwards Aquifer.

4.0 REFERENCES

- Batte, Charles D.; Trevino, Rosando; Divin, James; and Bearden, Eddie D. Soil Survey of Comal and Hays Counties, Texas. US Department of Agriculture, Natural Resources Conservation Service (formerly Soil Conservation Service), in cooperation with the Texas Agricultural Experiment Station. 1984.
- (Blome et al.) Blome, Charles D., Jason R. Faith, Diana E. Pedraza, George B. Ozuna, James C. Cole, Allan K. Clark, Ted A. Small, and Robert R. Morris. *Geologic Map of the Edwards Aquifer Recharge Zone, South-Central Texas*. US Geological Survey Scientific Investigations Map 2873, Version 1.1. 2005.
- (CAPCOG) Capital Area Council of Governments. 5-foot contours, CAPCOG Center for Regional Development, Austin, Texas. 2007.
- Clark, A.K. Geologic framework and hydrogeologic characteristics of the Edwards aquifer, Uvalde County, Texas: U.S. Geological Survey Water-Resources Investigations Report 03–4010, 17 p., 1 sheet. 2003.
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- (NRCS) US Department of Agriculture, Natural Resources Conservation Service. Web Soil Survey, <<http://websoilsurvey.sc.egov.usda.gov/App/HomePage.htm>>. Soil map data layer updated 12 September 2019. Accessed 15 June 2023.
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- (TCEQ) Texas Commission on Environmental Quality. Instructions to Geologists for Geologic Assessments on the Edwards Aquifer Recharge/Transition Zones. Revised October 2004.
- _____. RG-348, Complying with the Edwards Aquifer Rules: Technical Guidance on Best

Management Practices. Revised July 2005.

_____. Optional Enhanced Measures for the Protection of Water Quality in the Edwards Aquifer (Revised). Appendix A to RG-348, Complying with the Edwards Aquifer Rules: Technical Guidance on Best Management Practices. September 2007.

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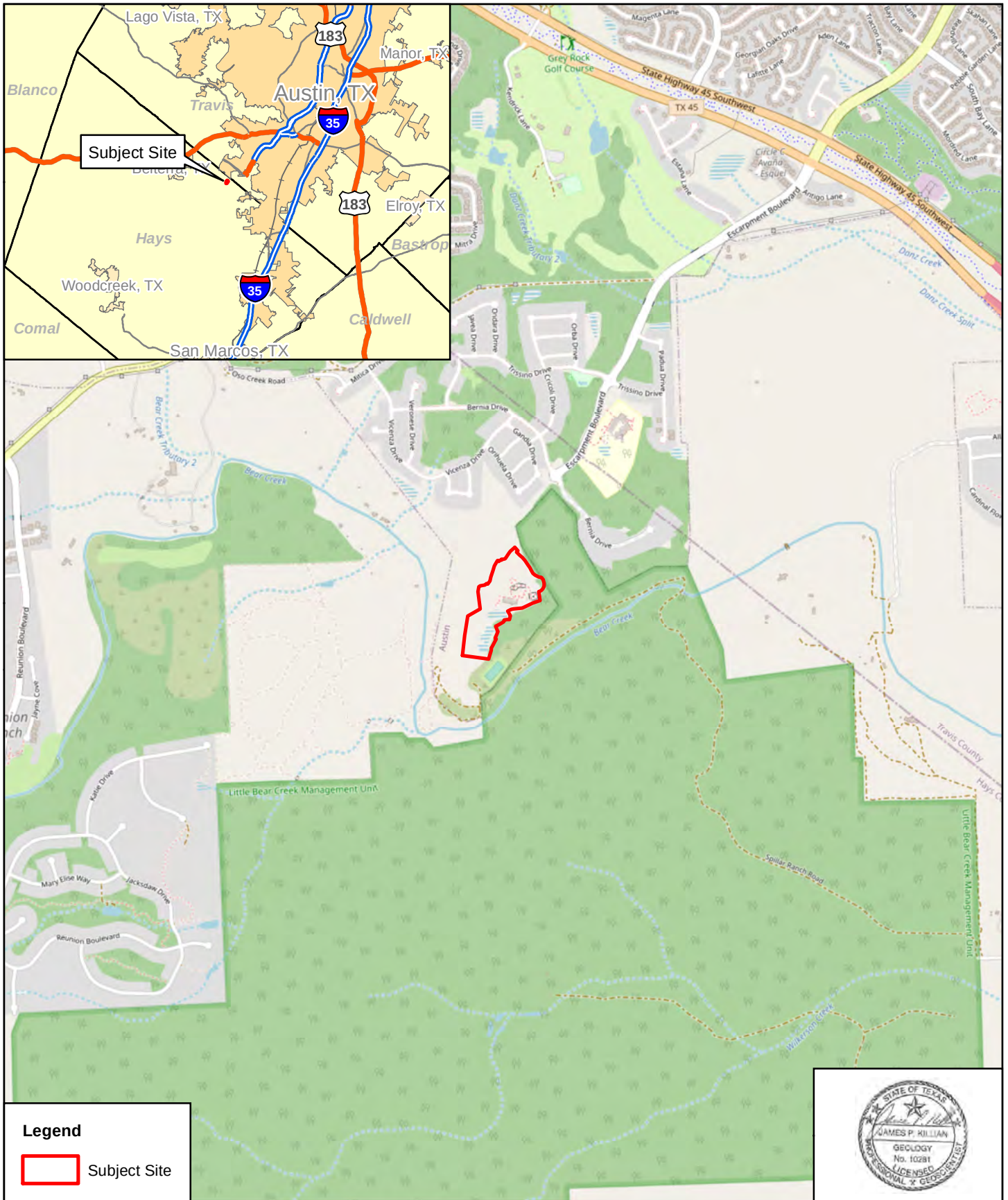
(TWDB) Texas Water Development Board. Water Information Integration and Dissemination System. TWDB Groundwater Database (ArcIMS), <http://wiid.twdb.state.tx.us/ims/www_drl/viewer.htm?>>. Accessed 15 June 2023.

(TWSC) United States Geological Survey, Texas Water Science Center. Geologic Database of Texas, <<https://txpub.usgs.gov/txgeology/>>. Updated 1 February 2014; Accessed 15 June 2023.

(UT-BEG) University of Texas Bureau of Economic Geology, C.V. Proctor, Jr., T.E. Brown, J.H. McGowen, N.B. Waechter, and V.E. Barnes. *Geologic Atlas of Texas*, Austin Sheet, Francis Luther Whitney Memorial Edition. 1974; revised 1981.

(USGS) US Geological Survey. 7.5-minute series topographic maps, Signal Hill, Texas, quadrangle. 1986.

ATTACHMENT F
ADDITIONAL SITE MAPS



Horizon
Environmental Services, Inc.

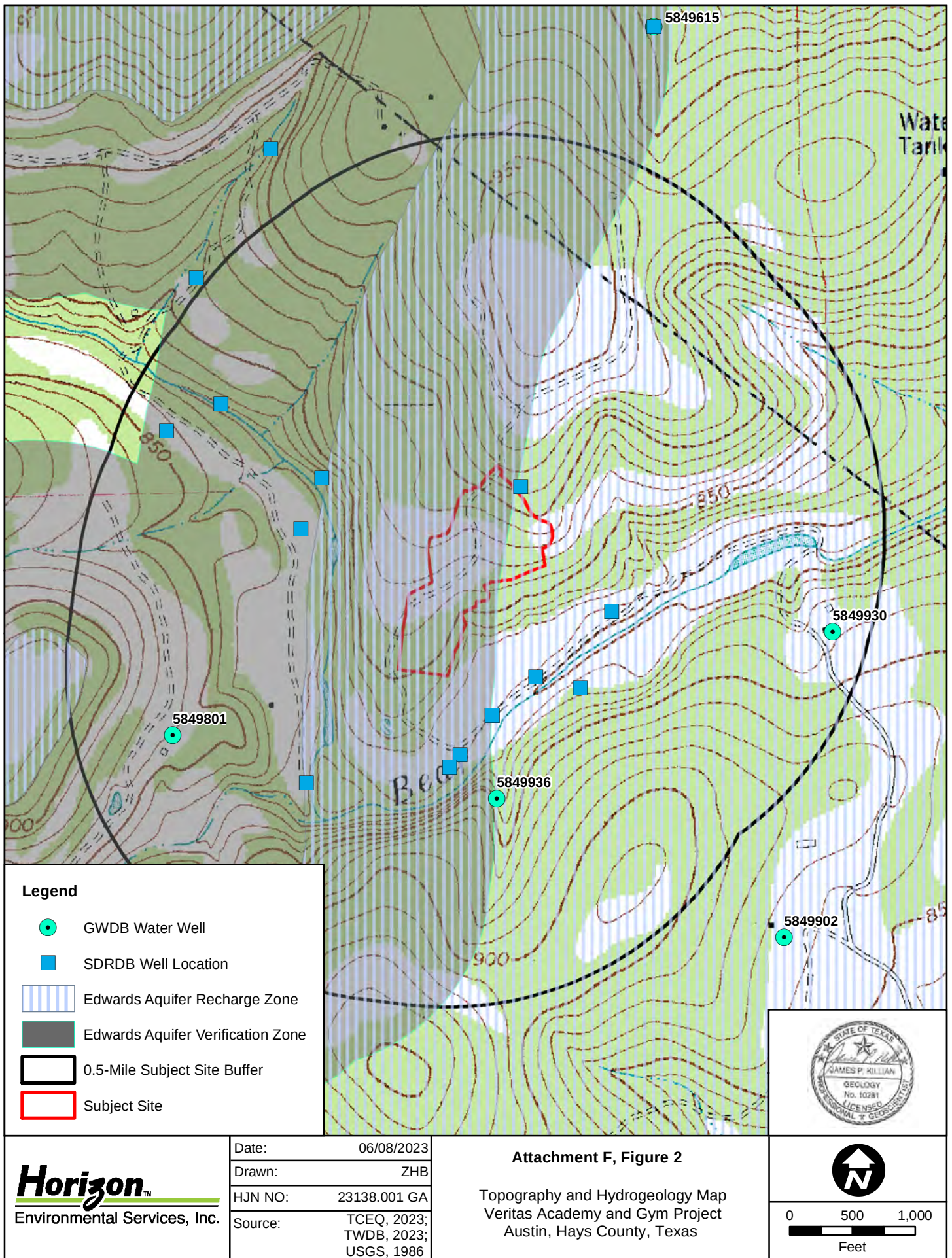
Date: 06/08/2023
 Drawn: ZHB
 HJN NO: 23138.001 GA
 Source: OSM, 2023

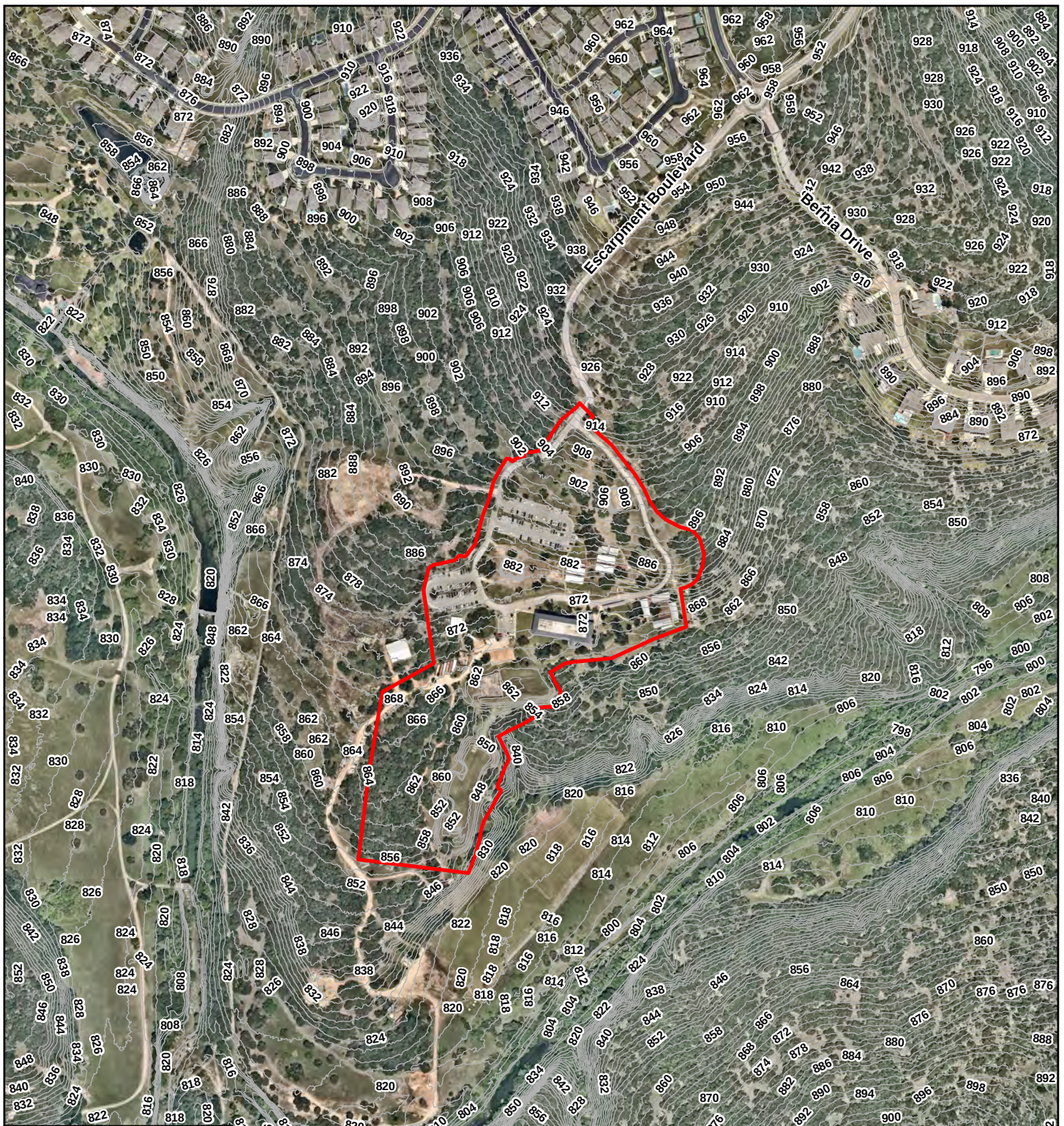
Attachment F, Figure 1

Vicinity Map
 Veritas Academy and Gym Project
 Austin, Hays County, Texas



0 1,000 2,000
 Feet





Legend

— 2-Foot Contours

Subject Site

Horizon
Environmental Services, Inc.

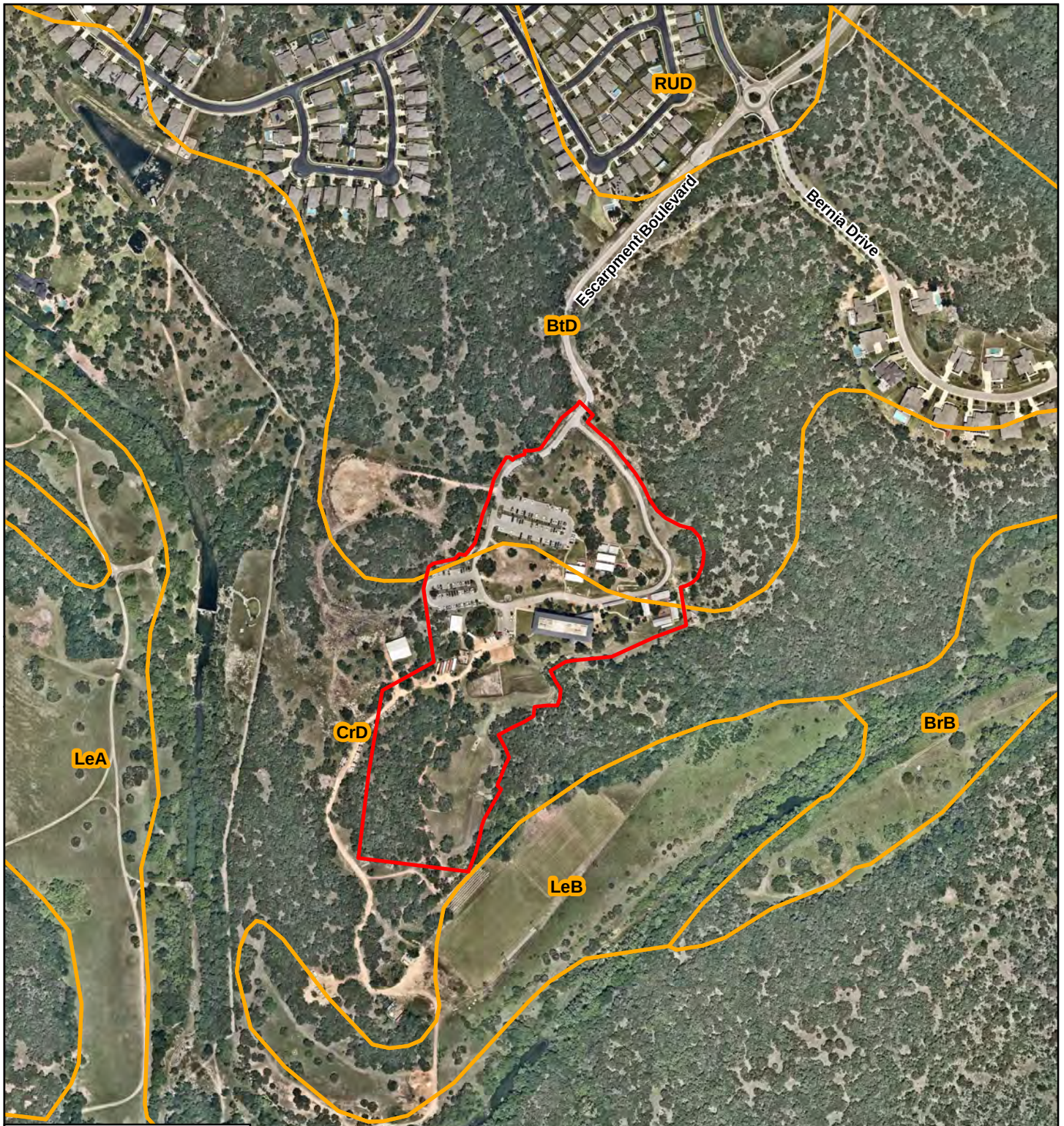
Date:	06/08/2023
Drawn:	ZHB
HJN NO:	23138.001 GA
Source:	COA, 2019; Nearmap, 2023

Attachment F, Figure 3

Topography Map
Veritas Academy and Gym Project
Austin, Hays County, Texas



0 250 500
Feet



Legend

- Soil Unit Boundary
- Subject Site

Horizon
Environmental Services, Inc.

Date:	06/08/2023
Drawn:	ZHB
HJN NO:	23138.001 GA
Source:	Nearmap, 2023; NRCS, 2019

Attachment F, Figure 4
Site Soil Map
Veritas Academy and Gym Project
Austin, Hays County, Texas



0 250 500
Feet

ATTACHMENT G
SITE PHOTOGRAPHS



PHOTO 1
General view of subject site, facing south



PHOTO 2
General view of subject site, facing east

Modification of a Previously Approved Plan

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Transition Zone and
Relating to 30 TAC 213.4(j), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This request for a **Modification of a Previously Approved Plan** is hereby submitted for TCEQ review and executive director approval. The request was prepared by:

Print Name of Customer/Agent: Reese Hurley, P.E. - LJA Engineering, Inc.

Date: 05-11-2026

Signature of Customer/Agent:



Project Information

1. Current Regulated Entity Name: Veritas Academy
Original Regulated Entity Name: Veritas Academy
Regulated Entity Number(s) (RN): 108353129
Edwards Aquifer Protection Program ID Number(s): 11003859
☒ The applicant has not changed and the Customer Number (CN) is: 604817940
☐ The applicant or Regulated Entity has changed. A new Core Data Form has been provided.
2. ☒ **Attachment A: Original Approval Letter and Approved Modification Letters.** A copy of the original approval letter and copies of any modification approval letters are attached.

3. A modification of a previously approved plan is requested for (check all that apply):
- ☐ Physical or operational modification of any water pollution abatement structure(s) including but not limited to ponds, dams, berms, sewage treatment plants, and diversionary structures;
 - ☐ Change in the nature or character of the regulated activity from that which was originally approved or a change which would significantly impact the ability of the plan to prevent pollution of the Edwards Aquifer;
 - ☐ Development of land previously identified as undeveloped in the original water pollution abatement plan;
 - ☒ Physical modification of the approved organized sewage collection system;
 - ☐ Physical modification of the approved underground storage tank system;
 - ☐ Physical modification of the approved aboveground storage tank system.
4. ☒ Summary of Proposed Modifications (select plan type being modified). If the approved plan has been modified more than once, copy the appropriate table below, as necessary, and complete the information for each additional modification.

<i>WPAP Modification</i>	<i>Approved Project</i>	<i>Proposed Modification</i>
<i>Summary</i>		
Acres	_____	_____
Type of Development	_____	_____
Number of Residential Lots	_____	_____
Impervious Cover (acres)	_____	_____
Impervious Cover (%)	_____	_____
Permanent BMPs	_____	_____
Other	_____	_____
<i>SCS Modification</i>	<i>Approved Project</i>	<i>Proposed Modification</i>
<i>Summary</i>		
Linear Feet	<u>462.21</u>	<u>519.86</u>
Pipe Diameter	<u>6"</u>	<u>6"</u>
Other	_____	_____

<i>AST Modification</i>	<i>Approved Project</i>	<i>Proposed Modification</i>
--------------------------------	--------------------------------	-------------------------------------

Summary

Number of ASTs	_____	_____
Volume of ASTs	_____	_____
Other	_____	_____

<i>UST Modification</i>	<i>Approved Project</i>	<i>Proposed Modification</i>
--------------------------------	--------------------------------	-------------------------------------

Summary

Number of USTs	_____	_____
Volume of USTs	_____	_____
Other	_____	_____

5. ☒ **Attachment B: Narrative of Proposed Modification.** A detailed narrative description of the nature of the proposed modification is attached. It discusses what was approved, including any previous modifications, and how this proposed modification will change the approved plan.

6. ☐ **Attachment C: Current Site Plan of the Approved Project.** A current site plan showing the existing site development (i.e., current site layout) at the time this application for modification is attached. A site plan detailing the changes proposed in the submitted modification is required elsewhere.
 - ☐ The approved construction has not commenced. The original approval letter and any subsequent modification approval letters are included as Attachment A to document that the approval has not expired.
 - ☐ The approved construction has commenced and has been completed. Attachment C illustrates that the site was constructed as approved.
 - ☐ The approved construction has commenced and has been completed. Attachment C illustrates that the site was **not** constructed as approved.
 - ☒ The approved construction has commenced and has **not** been completed. Attachment C illustrates that, thus far, the site was constructed as approved.
 - ☐ The approved construction has commenced and has **not** been completed. Attachment C illustrates that, thus far, the site was **not** constructed as approved.

7. ☐ The acreage of the approved plan has increased. A Geologic Assessment has been provided for the new acreage.
 - ☒ Acreage has not been added to or removed from the approved plan.

8. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.



COPY

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

Protecting Texas by Reducing and Preventing Pollution

July 30, 2015

Mr. Jef Fowler
Veritas Academy
P. O. Box 90517
Austin, Texas 78709

Re: Edwards Aquifer, Hays County
Veritas Academy, Located at the end of Escarpment Boulevard in the City of Austin, in Hays
County, Texas
Request for Approval of a Water Pollution Abatement Plan (WPAP) and Organized Sewage
Collection System (SCS);
30 Texas Administrative Code (TAC) Chapter 213 and Chapter 217 Edwards Aquifer
Edwards Aquifer Protection Program ID No. 11-15051301&2

Dear Mr. Fowler:

The Texas Commission on Environmental Quality (TCEQ) has completed its review of the WPAP/SCS applications for the above-referenced project submitted to the Austin Regional Office on behalf of Veritas Academy by LJA Engineering, Inc. on May 13, 2015. Final reviews of the applications were completed after additional materials were received on July 28 and July 30, 2015. As presented to the TCEQ, the Temporary and Permanent Best Management Practices (BMPs) and construction plans were prepared by a Texas licensed professional engineer to be in general compliance with the requirements of 30 TAC Chapter 213. These planning materials were sealed, signed and dated by a Texas licensed professional engineer. Therefore, based on the engineer's concurrence of compliance, the planning materials for construction of the proposed project and pollution abatement measures are hereby approved subject to applicable state rules and the conditions in this letter. The applicant or a person affected may file with the chief clerk a motion for reconsideration of the executive director's final action on this Edwards Aquifer Protection Plan. A motion for reconsideration must be filed no later than 23 days after the date of this approval letter. *This approval expires two (2) years from the date of this letter unless, prior to the expiration date, more than 10 percent of the construction has commenced on the project or an extension of time has been requested.*

PROJECT DESCRIPTION

The proposed Veritas Academy site is located on 96.6 acres of undeveloped land on the Edwards Aquifer Recharge Zone. The proposed project will disturb an area of 16.4 acres, 4.14 acres of which will be impervious cover. Proposed construction includes a main school building, auditorium, and pre-manufactured units with decking to serve as school/classroom/offices/facilities. Other improvements include parking, drive aisle, drainage, water and wastewater utilities, a retention/irrigation water quality pond and detention pond for larger storms. Upon treatment, outflow is into the Bear Creek watershed.

The proposed sewage collection system includes both gravity lines and a force main. The gravity lines consist of 771 linear feet of 8-inch SDR 26 pipe (service lines are 6-inch), which will convey the wastewater to a lift station. Wastewater pumped from the lift station will be carried using 2,303 linear feet of 3-inch force main and fittings. The force main piping will consist of 250 psi Certa-Lok Yelomine (SDR 17).

The proposed lift station will consist of a six-foot diameter wet well with an approximate depth of seventeen (17) feet, two duplex submersible non-clog pumps in parallel, and a 6-foot x 7-foot valve vault. An odor control system and back-up emergency generator will also be provided. Each pump will have a pumping capacity of 70 gallons per minute at a total dynamic head of 175 feet. Additional equipment will include a control panel, an audio visual alarm, a rail-type pump support system, level pump controllers, and discharge piping with valves (in vault beside wet well), and a security fence with controlled access.

The system will collect wastewater into a gravity system and be routed to the lift station located at the south end of the proposed site, and then pumped to an existing 3-inch force main in Escarpment Boulevard, which connects to an 8-inch line owned by Midtex Utilities and eventually to a 15-inch wastewater line in Escarpment Boulevard en route for treatment and disposal at the South Austin Regional Wastewater Treatment Plant. The project is located within the City of Austin ETJ and will conform to all applicable codes, ordinances, and requirements of the City of Austin. System designs and calculations were sealed by Reece Hurley, P.E., and Daniel Ryan, P.E.

PERMANENT POLLUTION ABATEMENT MEASURES

To prevent the pollution of stormwater runoff originating on-site or upgradient of the site and potentially flowing across and off the site after construction, a retention/irrigation pond, designed using the TCEQ technical guidance document, Complying with the Edwards Aquifer Rules: Technical Guidance on Best Management Practices (2005), will be constructed to treat stormwater runoff. The required total suspended solids (TSS) treatment for this project is 3,716 pounds of TSS generated from the 4.14 acres of impervious cover.

The individual treatment measures will consist of a retention/irrigation pond. The total capture volume required is 17,722 cubic feet; 57,464 cubic feet are provided. The required irrigation area is 0.41 acres; the provided irrigation area is 1.30 acres. The approved measures meet the required 80 percent removal of the increased load in TSS caused by the project.

Treatment design calculations were sealed by Reece Hurley, P.E., on May 12, 2015 to demonstrate the total treatment load removal to exceed the required additional total suspended solids (TSS) loading.

GEOLOGY

The geologic assessment (GA) was conducted in May 2011 by J. Jackson Harper, P.G. Geological & Hydrogeological Consulting, as part of the GA for a larger project, including two properties totaling 555 acres.

According to the geologic assessment included with the application, the site is underlain by the Upper Member of the Glen Rose Formation (Kgru), the Basal Nodular Member of the Kainer Formation (Kbn), the Dolomitic Member of the Kainer Formation (Kdo), and Quaternary Alluvium (Qal). The Austin Regional Office site assessment conducted on July 2, 2015 revealed the site to be generally as described.

Natural buffers were proposed for five features (SR-1/G37, SR-2/G40, SR-3/G10, SR-5/Go9, and SR-8/Go8). No regulated activities will take place within the natural buffers. The size is generally based on the drainage area for each sensitive feature; these odd shaped setbacks for features are illustrated on the construction plans (plan sheet 3 of 43).

SPECIAL CONDITIONS

- I. All sediment and/or media removed from the basin during maintenance activities shall be properly disposed of according to 30 TAC 330 or 30 TAC 335, as applicable.
- II. Permanent pollution abatement measures shall be operational prior to occupancy of the facility.
- III. It is emphasized that where wastewater lines must bridge faults, caverns, sinkholes, or solution features the lines shall be constructed in a manner that will maintain the structural integrity of the pipe. When such sensitive features are encountered, 30 TAC §213.5(f)(2) requires that all regulated activities near the feature must be immediately suspended and the owner/developer shall immediately notify the Austin Regional Office. Additionally, when such geologic features are encountered which are bridged by construction, the location and extent of those features must be assessed by a geologist and must be reported to the Austin Regional Office in writing within two working days of discovery as required by 30 TAC §213.5(c)(3)(K). Construction may not resume in the area of the feature until the executive director has reviewed and approved the methods proposed to protect the aquifer from any potential adverse impacts.
- IV. Upon completion of any lift station excavation, a geologist shall certify that the excavation has been inspected for the presence of sensitive features. Certification that the excavation has been inspected must be submitted to the Austin Regional Office.

STANDARD CONDITIONS

1. Pursuant to Chapter 7 Subchapter C of the Texas Water Code, any violations of the requirements in 30 TAC Chapter 213 may result in administrative penalties.
2. In addition to the rules of the Commission, the applicant may also be required to comply with state and local ordinances and regulations providing for the protection of water quality.

Prior to Commencement of Construction:

3. All contractors conducting regulated activities at the referenced project location shall be provided a copy of this notice of approval. At least one complete copy of the approved WPAP/SCS and this notice of approval shall be maintained at the project location until all regulated activities are completed.
4. Modification to the activities described in the referenced WPAP/SCS application following the date of approval may require the submittal of a plan to modify this approval, including the payment of appropriate fees and all information necessary for its review and approval prior to initiating construction of the modifications.

5. The applicant must provide written notification of intent to commence construction, replacement, or rehabilitation of the referenced project. Notification must be submitted to the Austin Regional Office no later than 48 hours prior to commencement of the regulated activity. Written notification must include the date on which the regulated activity will commence, the name of the approved plan and program ID number for the regulated activity, and the name of the prime contractor with the name and telephone number of the contact person. The executive director will use the notification to determine if the approved plan is eligible for an extension.
6. Temporary erosion and sedimentation (E&S) controls, i.e., silt fences, rock berms, stabilized construction entrances, or other controls described in the approved WPAP, must be installed prior to construction and maintained during construction. Temporary E&S controls may be removed when vegetation is established and the construction area is stabilized. The TCEQ may monitor stormwater discharges from the site to evaluate the adequacy of temporary E&S control measures. Additional controls may be necessary if excessive solids are being discharged from the site.
7. All borings with depths greater than or equal to 20 feet must be plugged with non-shrink grout from the bottom of the hole to within three (3) feet of the surface. The remainder of the hole must be backfilled with cuttings from the boring. All borings less than 20 feet must be backfilled with cuttings from the boring. All borings must be backfilled or plugged within four (4) days of completion of the drilling operation. Voids may be filled with gravel.

During Construction:

8. During the course of regulated activities related to this project, the applicant or agent shall comply with all applicable provisions of 30 TAC Chapter 213, Edwards Aquifer. The applicant shall remain responsible for the provisions and conditions of this approval until such responsibility is legally transferred to another person or entity.
9. If any sensitive feature (caves, solution cavities, sink holes, etc.) is discovered during construction, all regulated activities near the feature must be suspended immediately. The applicant or his agent must immediately notify the Austin Regional Office of the discovery of the feature. Regulated activities near the feature may not proceed until the executive director has reviewed and approved the methods proposed to protect the feature and the aquifer from potentially adverse impacts to water quality. The plan must be sealed, signed, and dated by a Texas licensed professional engineer.
10. If sediment escapes the construction site, the sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain). Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50 percent. Litter, construction debris, and construction chemicals shall be prevented from becoming stormwater discharge pollutants.
11. Intentional discharges of sediment laden water are not allowed. If dewatering becomes necessary, the discharge will be filtered through appropriately selected best management practices. These may include vegetated filter strips, sediment traps, rock berms, silt fence rings, etc.
12. The following records shall be maintained and made available to the executive director upon request: the dates when major grading activities occur, the dates when construction activities

temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.

13. Two wells exist on site. All water wells, including injection, dewatering, and monitoring wells must be in compliance with the requirements of the Texas Department of Licensing and Regulation under Title 16 TAC Chapter 76 (relating to Water Well Drillers and Pump Installers) and all other locally applicable rules, as appropriate.
14. Stabilization measures shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, and construction activities will not resume within 21 days. When the initiation of stabilization measures by the 14th day is precluded by weather conditions, stabilization measures shall be initiated as soon as practicable.
15. No part of the SCS system shall be used as a holding tank for a pump-and-haul operation.

After Completion of Construction:

16. A Texas licensed professional engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the Austin Regional Office within 30 days of BMP site completion. Certification by a Texas licensed professional engineer of the testing of sewage collection systems required by 30 TAC Chapter 213 and Chapter 217 shall be submitted to the Austin Regional Office within 30 days of test completion and prior to the new sewage collection system being put into service. The certification should include the project name as it appeared on the approved application, the program ID number, and two copies of a site plan sheet(s) indicating the wastewater lines that were tested and are being certified as complying with the appropriate regulations.
17. The applicant shall be responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. The regulated entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred. A copy of the transfer of responsibility must be filed with the executive director through Austin Regional Office within 30 days of the transfer.
18. Upon legal transfer of this property or sewage utility, the new owner(s) is required to comply with all terms of the approved Edwards Aquifer protection plan. If the new owner intends to commence any new regulated activity on the site, a new Edwards Aquifer protection plan that specifically addresses the new activity must be submitted to the executive director. Approval of the plan for the new regulated activity by the executive director is required prior to commencement of the new regulated activity.
19. An Edwards Aquifer protection plan approval or extension will expire and no extension will be granted if more than 50 percent of the total construction has not been completed within ten years from the initial approval of a plan. A new Edwards Aquifer protection plan must be submitted to the Austin Regional Office with the appropriate fees for review and approval by the executive director prior to commencing any additional regulated activities.
20. At project locations where construction is initiated and abandoned, or not completed, the site shall be returned to a condition such that the aquifer is protected from potential contamination.

21. Every five years after the initial certification, the sewage collection system shall be retested. Any lines that fail the test must be repaired and retested. Certification that the system continues to meet the requirements of 30 TAC Chapter 213 and Chapter 217 shall be submitted to the Austin Regional Office.
22. If ownership of this organized sewage collection system is legally transferred (e.g., developer to city or Municipal Utility District), the new owner(s) is required to comply with all terms of the approved Edwards Aquifer protection plan. If the new owner intends to commence any new regulated activity on the site, a new Edwards Aquifer protection plan that specifically addresses the new activity must be submitted to the executive director. Approval of the plan for the new regulated activity by the executive director is required prior to commencement of the new regulated activity.

This action is taken under authority delegated by the Executive Director of the Texas Commission on Environmental Quality. If you have any questions or require additional information, please contact Ms. Tejashri Kyle, P.E. of the Edwards Aquifer Protection Program of the Austin Regional Office at 512-339-2929.

Sincerely,



Carolyn Runyon, Water Section Manager
Austin Region Office
Texas Commission on Environmental Quality

CDR/tjk/jcs

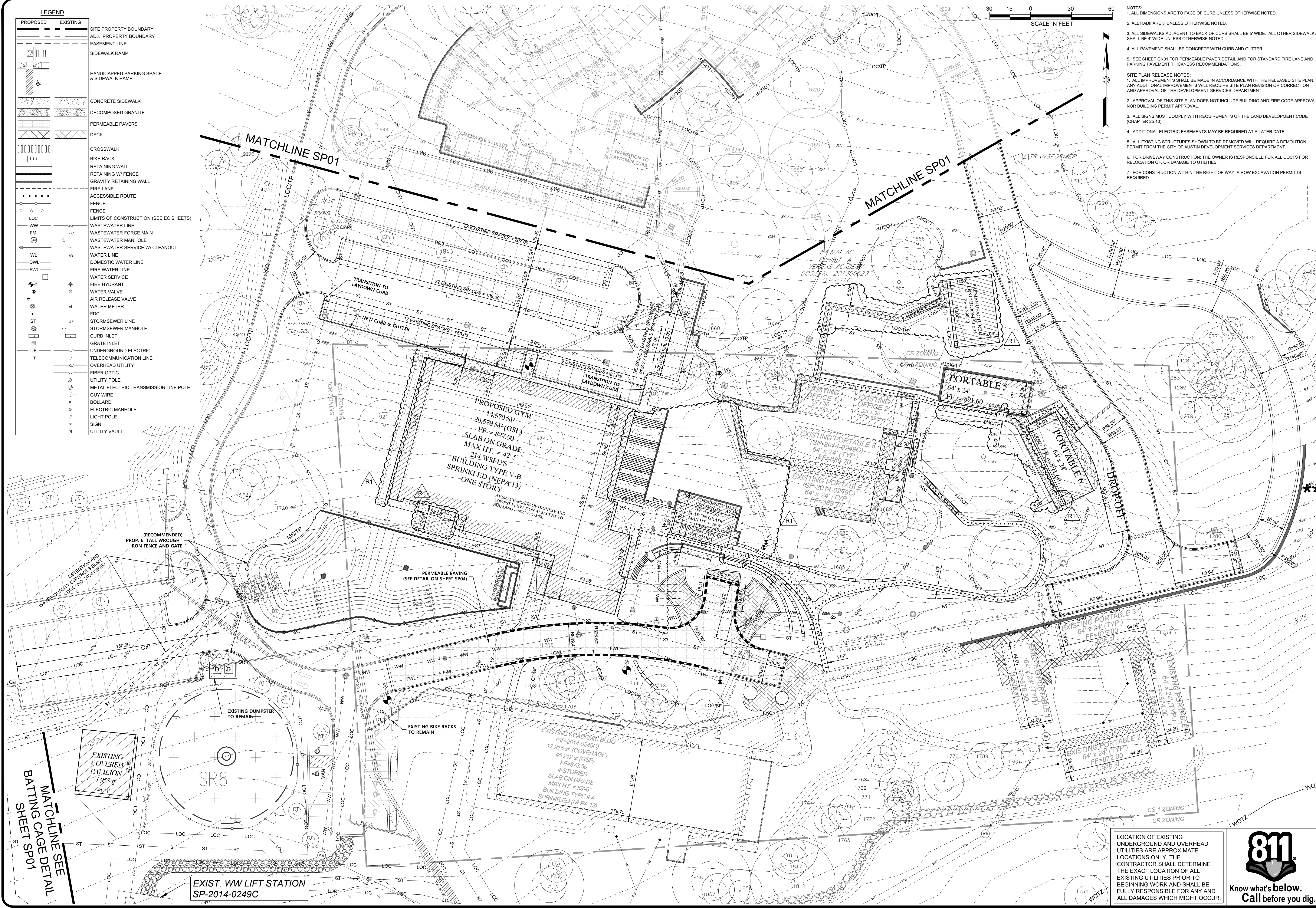
Enclosure: Deed Recordation Affidavit, Form TCEQ-0625
Change in Responsibility for Maintenance of Permanent BMPs, Form TCEQ-10263

cc: Reece Hurley, P.E., LJA Engineering, Inc., Austin
The Honorable Bert Cobb, M.D., County Judge, Hays County
Ms. Brooke Leftwich, Environmental Compliance Specialist, Hays County
Mr. David Johns, P.G., Watershed Protection Dept., City of Austin
Mr. John Dupnik, P.G., General Manager, Barton Springs/Edwards Aquifer Conservation District
TCEQ Central Records, Building F, MC 212

Attachment B – Narrative of Proposed Modification

As part of the Veritas Academy Site Plan Revision 1, a community hall and associated wastewater service and manhole have been added to the site plan. This modification extends the TCEQ SCS approximately 57.7 feet to the point at which the community hall wastewater service and associated manhole have been added to the overall proposed wastewater collection system. The resulting dry and wet weather peak flows remain between 2 and 10 feet per second.

1:\A201\A202 - Veritas Gym\Drawings\A202-0426-SP01.dwg
User: dourney May 18, 2022 - 16:22
Last Modified: May 15, 2022 - 11:41:13
Plot Date/Time: May 15, 2022 - 11:41:13



VERITAS ACADEMY
GYM ADDITION & CAMPUS IMPROVEMENTS
SITE PLAN 2

13401 ESCARPMENT BLVD., AUSTIN TEXAS

REVISIONS		DATE	BY
NO.	DESCRIPTION		
C1	UPDATE BUILDING FOOTPRINT AND SITE WALKWAYS	01/08/2025	DSM
R1	DATE	04/10/2025	DSM

JOB NUMBER:	DATE:	DESIGNED BY:	DRAWN BY:	CHECKED BY:	DRAWING NAME:
A219-0402	5/17/20	DSM	JKK	RBH	A219-0402-SP01.DWG

REESSE B. HURLEY
98211
LICENSED PROFESSIONAL ENGINEER
STATE OF TEXAS

04/30/2026

LJA Engineering, Inc.
Phone 512.439.4700
Fax 512.439.4716
7500 Riata Boulevard
Building II, Suite 100
Austin, Texas 78735

JOB NUMBER:	DATE:	DESIGNED BY:	DRAWN BY:	CHECKED BY:	DRAWING NAME:
A219-0402	5/17/20	DSM	JKK	RBH	A219-0402-SP01.DWG

SP02

SHEET NO.

20

OF 53 SHEETS



SP-2023-0426C

R1 C1 REPLACEMENT SHEET

Organized Sewage Collection System Application

Texas Commission on Environmental Quality

For Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(c), Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

Regulated Entity Name: Veritas Academy

1. ☒ **Attachment A – SCS Engineering Design Report.** This Engineering Design Report is provided to fulfill the requirements of 30 TAC Chapter 217, including 217.10 of Subchapter A, §§217.51 – 217.70 of Subchapter C, and Subchapter D as applicable, and is required to be submitted with this SCS Application Form.

Customer Information

2. The entity and contact person responsible for providing the required engineering certification of testing for this sewage collection system upon completion (including private service connections) and every five years thereafter to the appropriate TCEQ region office pursuant to 30 TAC §213.5(c) is:

Contact Person: Scott Rhodes

Entity: Veritas Academy

Mailing Address: 13401 Escarpment Blvd.

City, State: Austin, TX

Zip: 78739

Telephone: 512-891-1674

Fax: 512-891-1693

Email Address: scott.rhodes@veritasacademy.com

The appropriate regional office must be informed of any changes in this information within 30 days of the change.

3. The engineer responsible for the design of this sewage collection system is:

Contact Person: Reese Hurley, P.E.

Texas Licensed Professional Engineer's Number: 98211

Entity: LJA Engineering

Mailing Address: 7500 Rialto Blvd. Building II, Suite 100

City, State: Austin, TX

Zip: 78735

Telephone: 512-439-4734

Fax: 512-439-4716

Email Address: rhurley@lja.com

Project Information

4. Anticipated type of development to be served (estimated future population to be served, plus adequate allowance for institutional and commercial flows):

- ☐ Residential: Number of single-family lots: _____
☐ Multi-family: Number of residential units: _____
☒ Commercial
☐ Industrial
☐ Off-site system (not associated with any development)
☐ Other: _____

5. The character and volume of wastewater is shown below:

100% Domestic 5,625 gallons/day
 _____% Industrial _____ gallons/day
 _____% Commingled _____ gallons/day
 Total gallons/day: _____

6. Existing and anticipated infiltration/inflow is 2,250 gallons/day. This will be addressed by: All gravity lines will be SDR-26 to minimize infiltration, will be low pressure air tested, and all manholes will be hydrostatically tested.

7. A Water Pollution Abatement Plan (WPAP) is required for construction of any associated commercial, industrial or residential project located on the Recharge Zone.

- ☒ The WPAP application for this development was approved by letter dated 4/5/2024. A copy of the approval letter is attached.
☐ The WPAP application for this development was submitted to the TCEQ on _____, but has not been approved.
☐ A WPAP application is required for an associated project, but it has not been submitted.
☐ There is no associated project requiring a WPAP application.

8. Pipe description:

Table 1 - Pipe Description

<i>Pipe Diameter(Inches)</i>	<i>Linear Feet (1)</i>	<i>Pipe Material (2)</i>	<i>Specifications (3)</i>
6	365	PVC SDR-26	ASTM D3034
8	156	PVC SDR- 26	ASTM D3034

Total Linear Feet: 521

(1) Linear feet - Include stub-outs and double service connections. Do not include private service laterals.

(2) Pipe Material - If PVC, state SDR value.

(3) Specifications - ASTM / ANSI / AWWA specification and class numbers should be included.

9. The sewage collection system will convey the wastewater to the South Austin Regional (name) Treatment Plant. The treatment facility is:

- ☒ Existing
☐ Proposed

10. All components of this sewage collection system will comply with:

- ☒ The City of Austin standard specifications.
☐ Other. Specifications are attached.

11. ☒ No force main(s) and/or lift station(s) are associated with this sewage collection system.
☐ A force main(s) and/or lift station(s) is associated with this sewage collection system and the **Lift Station/Force Main System Application** form (TCEQ-0624) is included with this application.

Alignment

12. ☒ There are no deviations from uniform grade in this sewage collection system without manholes and with open cut construction.
13. ☒ There are no deviations from straight alignment in this sewage collection system without manholes.
- ☐ **Attachment B - Justification and Calculations for Deviation in Straight Alignment without Manholes.** A justification for deviations from straight alignment in this sewage collection system without manholes with documentation from pipe manufacturer allowing pipe curvature is attached.
- ☐ For curved sewer lines, all curved sewer line notes (TCEQ-0596) are included on the construction plans for the wastewater collection system.

Manholes and Cleanouts

14. ☒ Manholes or clean-outs exist at the end of each sewer line(s). These locations are listed below: (Please attach additional sheet if necessary)

Table 2 - Manholes and Cleanouts

<i>Line</i>	<i>Shown on Sheet</i>	<i>Station</i>	<i>Manhole or Clean-out?</i>
A	33 Of 52	3+88.00	Manhole
A	33 Of 52	4+62.21	Cleanout
A	33 Of 52	5+62.21	Manhole
A	33 Of 52	6+19.86	Manhole
A	33 Of 52	6+54.93	Cleanout
A	33 Of 52	7+54.93	Cleanout

<i>Line</i>	<i>Shown on Sheet</i>	<i>Station</i>	<i>Manhole or Cleanout?</i>
A	33 Of 52	8+37.43	Cleanout
A	33 Of 52	9+03.29	Cleanout
	Of		
	Of		

15. ☐ Manholes are installed at all Points of Curvature and Points of Termination of a sewer line.

16. ☒ The maximum spacing between manholes on this project for each pipe diameter is no greater than:

Pipe Diameter (inches)	Max. Manhole Spacing (feet)
6 - 15	500
16 - 30	800
36 - 48	1000
≥54	2000

☐ **Attachment C – Justification for Variance from Maximum Manhole Spacing.** The maximum spacing between manholes on this project (for each pipe diameter used) is greater than listed in the table above. A justification for any variance from the maximum spacing is attached, and must include a letter from the entity which will operate and maintain the system stating that it has the capability to maintain lines with manhole spacing greater than the allowed spacing.

17. ☐ All manholes will be monolithic, cast-in-place concrete.

☒ The use of pre-cast manholes is requested for this project. The manufacturer's specifications and construction drawings, showing the method of sealing the joints, are attached.

Site Plan Requirements

Items 18 - 25 must be included on the Site Plan.

18. ☒ The Site Plan must have a minimum scale of 1" = 400'.

Site Plan Scale: 1" = 30'.

19. ☒ The Site Plan must include the sewage collection system general layout, including manholes with station numbers, and sewer pipe stub outs (if any). Site plan must be overlain by topographic contour lines, using a contour interval of not greater than ten feet and showing the area within both the five-year floodplain and the 100-year floodplain of any drainage way.

20. Lateral stub-outs:

☒ The location of all lateral stub-outs are shown and labeled.

- ☐ No lateral stub-outs will be installed during the construction of this sewer collection system.

21. Location of existing and proposed water lines:

- ☒ The entire water distribution system for this project is shown and labeled.
- ☐ If not shown on the Site Plan, a Utility Plan is provided showing the entire water and sewer systems.
- ☐ There will be no water lines associated with this project.

22. 100-year floodplain:

- ☒ After construction is complete, no part of this project will be in or cross a 100-year floodplain, either naturally occurring or manmade. (Do not include streets or concrete-lined channels constructed above of sewer lines.)
- ☐ After construction is complete, all sections located within the 100-year floodplain will have water-tight manholes. These locations are listed in the table below and are shown and labeled on the Site Plan. (Do not include streets or concrete-lined channels constructed above sewer lines.)

Table 3 - 100-Year Floodplain

<i>Line</i>	<i>Sheet</i>	<i>Station</i>
	of	to
	of	to
	of	to
	of	to

23. 5-year floodplain:

- ☒ After construction is complete, no part of this project will be in or cross a 5-year floodplain, either naturally occurring or man-made. (Do not include streets or concrete-lined channels constructed above sewer lines.)
- ☐ After construction is complete, all sections located within the 5-year floodplain will be encased in concrete or capped with concrete. These locations are listed in the table below and are shown and labeled on the Site Plan. (Do not include streets or concrete-lined channels constructed above sewer lines.)

Table 4 - 5-Year Floodplain

<i>Line</i>	<i>Sheet</i>	<i>Station</i>
	of	to
	of	to
	of	to
	of	to

24. ☒ Legal boundaries of the site are shown.

25. ☒ The **final plans and technical specifications** are submitted for the TCEQ's review. Each sheet of the construction plans and specifications are dated, signed, and sealed by the Texas Licensed Professional Engineer responsible for the design on each sheet.

Items 26 - 33 must be included on the Plan and Profile sheets.

26. ☒ All existing or proposed water line crossings and any parallel water lines within 9 feet of sewer lines are listed in the table below. These lines must have the type of pressure rated pipe to be installed shown on the plan and profile sheets. Any request for a variance from the required pressure rated piping at crossings must include a variance approval from 30 TAC Chapter 290.

☐ There will be no water line crossings.

☒ There will be no water lines within 9 feet of proposed sewer lines.

Table 5 - Water Line Crossings

Line	Station or Closest Point	Crossing or Parallel	Horizontal Separation Distance	Vertical Separation Distance

27. Vented Manholes:

- ☒ **No part** of this sewer line is within the 100-year floodplain and vented manholes are not required by 30 TAC Chapter 217.
- ☐ **A portion** of this sewer line is within the 100-year floodplain and vented manholes will be provided at less than 1500 foot intervals. These water-tight manholes are listed in the table below and labeled on the appropriate profile sheets.
- ☐ **A portion** of this sewer line is within the 100-year floodplain and an alternative means of venting shall be provided at less than 1500 feet intervals. A description of the alternative means is described on the following page.
- ☐ **A portion** of this sewer line is within the 100-year floodplain; however, there is no interval longer than 1500 feet located within. No vented manholes will be used.

Table 6 - Vented Manholes

Line	Manhole	Station	Sheet

<i>Line</i>	<i>Manhole</i>	<i>Station</i>	<i>Sheet</i>

28. Drop manholes:

- ☐ There are no drop manholes associated with this project.
- ☒ Sewer lines which enter new or existing manholes or "manhole structures" higher than 24 inches above the manhole invert are listed in the table below and labeled on the appropriate profile sheets. These lines meet the requirements of 30 TAC §217.55(I)(2)(H).

Table 7 - Drop Manholes

<i>Line</i>	<i>Manhole</i>	<i>Station</i>	<i>Sheet</i>

29. Sewer line stub-outs (For proposed extensions):

- ☐ The placement and markings of all sewer line stub-outs are shown and labeled.
- ☒ No sewer line stub-outs are to be installed during the construction of this sewage collection system.

30. Lateral stub-outs (For proposed private service connections):

- ☒ The placement and markings of all lateral stub-outs are shown and labeled.
- ☐ No lateral stub-outs are to be installed during the construction of this sewage collection system.

31. Minimum flow velocity (From Appendix A)

- ☒ Assuming pipes are flowing full; all slopes are designed to produce flows equal to or greater than 2.0 feet per second for this system/line.

32. Maximum flow velocity/slopes (From Appendix A)

- ☒ Assuming pipes are flowing full, all slopes are designed to produce maximum flows of less than or equal to 10 feet per second for this system/line.
- ☐ **Attachment D – Calculations for Slopes for Flows Greater Than 10.0 Feet per Second.** Assuming pipes are flowing full, some slopes produce flows which are greater than 10 feet per second. These locations are listed in the table below. Calculations are attached.

Table 8 - Flows Greater Than 10 Feet per Second

<i>Line</i>	<i>Profile Sheet</i>	<i>Station to Station</i>	<i>FPS</i>	<i>% Slope</i>	<i>Erosion/Shock Protection</i>

33. Assuming pipes are flowing full, where flows are ≥ 10 feet per second, the provisions noted below have been made to protect against pipe displacement by erosion and/or shock under 30 TAC §217.53(l)(2)(B).

- ☐ Concrete encasement shown on appropriate Plan and Profile sheets for the locations listed in the table above.
- ☐ Steel-reinforced, anchored concrete baffles/retards placed every 50 feet shown on appropriate Plan and Profile sheets for the locations listed in the table above.
- ☒ N/A

Administrative Information

34. ☒ The final plans and technical specifications are submitted for TCEQ review. Each sheet of the construction plans and specifications are dated, signed, and sealed by the Texas Licensed Professional Engineer responsible for the design on each sheet.
35. ☒ Standard details are shown on the detail sheets, which are dated, signed, and sealed by the Texas Licensed Professional Engineer, as listed in the table below:

Table 9 - Standard Details

<i>Standard Details</i>	<i>Shown on Sheet</i>
Lateral stub-out marking [Required]	N/A (-- of --)
Manhole, showing inverts comply with 30 TAC §217.55(l)(2) [Required]	34 of 52
Alternate method of joining lateral to existing SCS line for potential future connections [Required]	N/A (-- of --)
Typical trench cross-sections [Required]	35 of 52
Bolted manholes [Required]	34 of 52
Sewer Service lateral standard details [Required]	See notes 5,7,& 8 on sheet 33 of 52
Clean-out at end of line [Required, if used]	35 of 52
Baffles or concrete encasement for shock/erosion protection [Required, if flow velocity of any section of pipe >10 fps]	N/A (- of -)
Detail showing Wastewater Line/Water Line Crossing [Required, if crossings are proposed]	N/A (- of -)
Mandrel detail or specifications showing compliance with 30 TAC §217.57(b) and (c) [Required, if Flexible Pipe is used]	35 of 52

Standard Details	Shown on Sheet
Drop manholes [Required, if a pipe entering a manhole is more than 24 inches above manhole invert]	34 of 52

36. ☒ All organized sewage collection system general construction notes (TCEQ-0596) are included on the construction plans for this sewage collection system.
37. ☐ All proposed sewer lines will be sufficiently surveyed/staked to allow an assessment prior to TCEQ executive director approval. If the alignments of the proposed sewer lines are not walkable on that date, the application will be deemed incomplete and returned.
- ☒ Survey staking was completed on this date: _____
38. ☒ Submit one (1) original and one (1) copy of the application, plus additional copies as needed for each affected incorporated city, groundwater conservation district, and county in which the project will be located. The TCEQ will distribute the additional copies to these jurisdictions. The copies must be submitted to the appropriate regional office.
39. ☐ Any modification of this SCS application will require TCEQ approval, prior to construction, and may require submission of a revised application, with appropriate fees.

Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Organized Sewage Collection System Application** is hereby submitted for TCEQ review and executive director approval. The system was designed in accordance with the requirements of 30 TAC §213.5(c) and 30 TAC §217 and prepared by:

Print Name of Licensed Professional Engineer: Reese Hurley, PE

Date: 4/10/2026

Place engineer's seal here:



Signature of Licensed Professional Engineer:

Appendix A-Flow Velocity Table

Flow Velocity (Flowing Full) All gravity sewer lines on the Edwards Aquifer Recharge Zone shall be designed and constructed with hydraulic slopes sufficient to give a velocity when flowing full of not less than 2.0 feet per second, and not greater than 10 feet per second. The grades shown in the following table are based on Manning's formula and an n factor of 0.013 and shall be the minimum and maximum acceptable slopes unless provisions are made otherwise.

Table 10 - Slope Velocity

<i>Pipe Diameter(Inches)</i>	<i>% Slope required for minimum flow velocity of 2.0 fps</i>	<i>% Slope which produces flow velocity of 10.0 fps</i>
6	0.50	12.35
8	0.33	8.40
10	0.25	6.23
12	0.20	4.88
15	0.15	3.62
18	0.11	2.83
21	0.09	2.30
24	0.08	1.93
27	0.06	1.65
30	0.055	1.43
33	0.05	1.26
36	0.045	1.12
39	0.04	1.01
>39	*	*

**For lines larger than 39 inches in diameter, the slope may be determined by Manning's formula (as shown below) to maintain a minimum velocity greater than 2.0 feet per second when flowing full and a maximum velocity less than 10 feet per second when flowing full.*

$$v = \frac{1.49}{n} \times R_h^{0.67} \times \sqrt{S}$$

Figure 1 - Manning's Formula

Where:

v = velocity (ft/sec)

n = Manning's roughness coefficient (0.013)

R_h = hydraulic radius (ft)

S = slope (ft/ft)

**ORGANIZED SEWAGE COLLECTION SYSTEM (SCS)
ENGINEERING DESIGN REPORT**

FOR

**VERITAS ACADEMY GYM ADDITION AND CAMPUS
IMPROVEMENTS**

APRIL 2026

PREPARED FOR

**VERITAS ACADEMY
13401 ESCARPMENT BLVD.
AUSTIN, TEXAS 78739
(512) 891-1673**



PREPARED BY

**LJA ENGINEERING, INC.
7500 RIALTO BLVD. BUILDING II, SUITE 100
AUSTIN, TEXAS 78735
(512) 439-4700
FRN-F-1386**

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Veritas Academy

SCS Engineering Design Report

A. SITE DESCRIPTION

1. Project Name: Veritas Academy Gym Addition and Campus Improvements
2. Location: 13401 Escarpment Blvd., Austin, Texas 78739
3. Treatment Facility: The South Austin Regional Wastewater Treatment plant will receive and treat flows from this project.
4. Project Summary: This project proposes to add a gym, looped driveway and drop-off area, parking improvements, required fire lanes, sidewalks and accessible paths, wood decking, drainage improvements, water quality improvements, detention improvements, and water and wastewater improvements. Two portable classrooms and a restroom will be relocated onsite. The existing guard shack will be demolished and replaced. To provide the site with wastewater service, approximately 283 LF of 4" domestic wastewater service, 364 linear feet of 6" domestic wastewater service, and 155 LF of 8" gravity wastewater pipe will be installed. **Only the 155 LF of 8" and 365 LF of 6" gravity wastewater pipe is part of this SCS.** Approximately 721 linear feet of water line, including both domestic and fire line, are also proposed.

B. CAPACITY DESIGN

1. Basis for Average Flow: A population-based method, based on the City of Austin's Utility Criteria Manual, was utilized to determine the average daily flow and peaks flow of the proposed gym and restroom trailer. The maximum population using the gym and restroom trailer was calculated to be 500 people. See Appendix B for the peak wet weather flow calculations.

2. Flow/Capacity Analysis:

8" Wastewater Pipe

Q_{max} (from Appendix B) = 39.07 gpm

Pipe Size = 8 in., $n = 0.013$

For the specified pipe at the design slope of 1.00%, the

Line Capacity (Q_{full}) = $(1.49/n) * A * R^{2/3} * S^{1/2} = 542.34$ gpm

$Q_{max} = 39.07$ gpm < $Q_{full} = 542.34$ gpm

Therefore, the line is of sufficient size to carry the peak flows.

3. Minimum/Maximum Slopes: All pipes must be designed with a slope that will provide a minimum velocity of at least 2 ft/s and a maximum velocity of at least 10 ft/s when flowing full. All gravity pipes are 8 and 6 inches.

6" Pipe, $S = 1.50\%$, $n = 0.013$, $V = 3.25 \text{ ft/s}$ $2 \text{ ft/s} < 3.26 \text{ ft/s} < 10 \text{ ft/s}$

4. Existing Sewage Collection System: The proposed SCS will tie into the existing Veritas Academy Sewage Collection System (Edwards Aquifer ID No. 11-15051301).

C. STRUCTURAL COMPONENTS

1. Type of Pipe:

6" SDR-26 ASTM D3034 Polyvinyl Chloride (PVC)

Product Standard:	ASTM 3034
Pipe Compound:	ASTM D1784 Cell Class 12454
Gasket:	ASTM F477
Integral Bell Joint:	ASTM D3212
Pipe Stiffness:	ASTM D2412, $F/\Delta Y = 115 \text{ psi}$
Installation:	ASTM D2321
Tensile Strength:	7000 psi
Modulus of Elasticity:	400,000 psi
Nominal Inside Diameter:	5.793 inches
Average Outside Diameter:	6.28 inches
Wall Thickness:	0.241 inches
Approximate Weight:	3.11 lbs/ft

2. Pipe Bedding Class:

The pipe bedding class must comply with ASTM D2321 class IA, IB, II, or III for materials and densification. No sand bedding will be allowed. Class III material is assumed to be used, since it has the most conservative value for the Modulus of Soil Reaction, E' .

From Table 7.3, Pg. 207 of the UNI-BELL Handbook of PVC PIPE, 3rd Edition
Coarse-grained Soils with Fines (Bedding Class III) and 85% to 95% Compaction

$$E_b = 1000 \text{ psi}$$

3. Manholes:

Manholes and/or cleanouts are provided at all changes in size, grade and alignment of pipe at all feasible locations. The maximum distance between manholes is less than the maximum spacing requirement of 500 feet allowed for all pipe sizes less than 15 inches in diameter. All manholes will be coated per the City of Austin standard specifications. Item number WW-511 from the City's Standard Products list includes several products which will achieve the design life and corrosion protection required. The site plan process took into account the existing topography, trees, and the natural aesthetics of the site, thus attempting to limit the use of large construction equipment in areas with trees and other natural areas. There are no vertical grade breaks proposed at the bend locations.

4. Buckling Analysis:

a) Allowable buckling pressure

$$R_w = 1 - 0.33 * (h_w / h)$$

$$B' = \frac{1}{1 + 4 * e^{-0.065H}}$$

$$I = (t^3 / 12) * (\text{inches}^4 / \text{Linch})$$

$$q_a = 0.4 * \sqrt[3]{32 * R_w * B' * E_b * (E * I / D^3)}$$

- q_a = allowable buckling pressure, pounds per square inch (psi)
 h = height of soil surface above top of pipe in inches (in)
 h_w = height of water surface above top of pipe in inches (in) (groundwater elevation)
 R_w = Water buoyancy factor. If $h_w = 0$, $R_w = 1$. If $0 \leq h_w \leq h$ (groundwater elevation is between the top of the pipe and the ground surface), calculate R_w with Equation 2
 H = Depth of burial in feet (ft) from ground surface to crown of pipe.
 B' = Empirical coefficient of elastic support
 E_b = modulus of soil reaction for the bedding material (psi)
 E = modulus of elasticity of the pipe material (psi)
 I = moment of inertia of the pipe wall cross section per linear inch of pipe, $\text{inch}^4 / \text{lineal inch} = \text{inch}^3$. For solid wall pipe, I can be calculated with equation 4. If the pipe used is not solid wall pipe (for example a pipe with a ribbed cross section), the proper moment of inertia formula must be obtained from the manufacturer.
 t = pipe structural wall thickness (in)
 D = mean pipe diameter (in)

6" SDR-26 ASTM D3034 PVC

H = Deepest bury depth will not exceed 15' and is thus assumed for this calculation.

$h_w = 0$ (no ground water)

$$I = t^3 / 12 = (0.241)^3 / 12 = 0.00117 \text{ in}^4 / \text{in}$$

$$R_w = 1 + 0.33(h_w / h) = 1$$

$$B' = 1 / (1 + 4e^{-0.065H}) = 1 / (1 + 4e^{-0.065(15)}) = 0.399$$

$$q_a = 0.4 * \sqrt[3]{32 * 1 * 0.399 * 1000 * (400000 * 0.00117 / 5.793^3)}^{0.5}$$

$$\mathbf{q_a = 69.99 \text{ psi}}$$

b) Calculate pressure applied to pipe under installed conditions:

$$W_c = \gamma_s * H * (D + t) / 144$$

$$q_p = \gamma_w * h_w + R_w * (W_c / D) + L_l$$

q_p = pressure applied to pipe under installed conditions (psi)

$\gamma_w = 0.0361$ pounds per cubic inch (pci), specific weight of water

γ_s = specific weight of soil in pounds per cubic foot (pcf)

W_c = vertical soil load on the pipe per unit length in pounds per linear inch (lb/in)

L_l = Live load = 0 (All bury depths are greater than 3 feet)

6" SDR-26 ASTM D3034 PVC

$$W_c = 120 * 15 * (5.793 + 0.241) / 144 = 75.43 \text{ lb/in}$$

$$q_p = (0.0361 * 0) + (1 * (75.43 / 5.793)) + (0)$$

$$q_p = 13.0200 \text{ psi}$$

$$q_p < q_a \\ 13.02 < 69.99 ,$$

The buckling pressure under installed conditions is less than the allowable buckling pressure of the specified pipe.

5. Wall Crushing:

Wall crushing due to compressive stress can be calculated from the compressive stress formula, as referenced in Plastic Pipe Design Manual published by Vylon Pipe, Page 14.

6" SDR-26 ASTM D3034 PVC

D_o = outside pipe diameter, in. = 6.28 in.

P_c = compressive stress, lb/in² , = T / A , for typical PVC pipe assume 4,000 psi.

A = surface area of the pipe wall, in²/ft = 0.241 in²/ft

γ_s = specific weight of soil in pounds per cubic foot (pcf) = 120 pcf

P_y = vertical soil pressure, lb/in² = $\gamma_s * H / 144$

T = wall thrust = $P_y D_o / 2$

H = Depth of burial in feet(ft) from ground surface to crown of pipe

Substituting the Thrust equation into the compressive strength equation:

$$P_c = P_y D_o / 2A$$

Substitute the equation for P_y shown above

$$P_c = [(\gamma_s * H / 144) * D_o] / 2A$$

Solving for H , the equation becomes:

$$H = (288 * P_c * A) / (\gamma_s * D_o)$$

$$H = (288) * (4000) * (.241) / (120 * 6.28)$$

$$H_a = 368.41$$

$$H_p < H_a$$

$$15.00 < 368.41 ,$$

The proposed maximum depth(H_p) is less than the maximum allowable depth (H_a) before wall crushing would occur.

6. Deflection Analysis: Zeta Factor

Leonhard's Zeta Factor can be calculated using Equation 7.37 of the UNI-BELL Handbook of PVC PIPE, 3rd Edition.

$$zeta = \frac{1.44}{f + (1.44 - f) * (E_b / E_{n'})}$$

$$f = \frac{b/d_a - 1}{1.154 + 0.444 * (b/d_a - 1)}$$

- f = pipe/trench width coefficient
 b = trench width = 2.52 ft = 30.3 in for 6" Wastewater Pipe
 d_a = pipe diameter = 6.28 in for 6" Wastewater Pipe
 E_b = modulus of soil reaction for the bedding material (psi) = 1000
 E'_n = modulus of soil reaction for the in-situ soil (psi) = 1000

6" Wastewater Pipe

$$f = (30.3/6.28 - 1) / (1.154 + 0.444 * (30.3/6.28 - 1)) = 1.550$$

$$\text{zeta} = 1.44 / [1.550 + (1.44 - 1.550) * (1000/1000)]$$

$$\text{zeta} = 1.0$$

7. Pipe Stiffness:

Using equation 7.1, from the Uni-Bell Handbook of PVC Pipe, 3rd Edition.

$$P_s = \frac{EI}{0.149 * r^3}$$

P_s = Pipe Stiffness (psi)

- E = modulus of elasticity of the pipe material (psi)
 I = moment of inertia of the pipe wall cross section per linear inch of pipe, inch⁴/lineal inch = inch³. For solid wall pipe, I can be calculated with equation 4. If the pipe used is not solid wall pipe (for example a pipe with a ribbed cross section), the proper moment of inertia formula must be obtained from the manufacturer.
 r = mean radius (in)

$$P_s = (400000 * .00117) / (0.149 * 2.897) = 128.86 \text{ psi (6" SDR-26 ASTM D3034)}$$

8. Pipe Stiffness to Soil Stiffness Factor Ration (P_s / SSF):

The Pipe Stiffness to Soil Stiffness Factor must be greater than 0.15

P_s = Pipe Stiffness (psi)

E_b = modulus of soil reaction for the bedding material = 1,000 psi

zeta = 1.0

SSF = soil stiffness factor (0.061 * zeta * E_b) = 61

$$P_s/SSF = 128.86/61 = 2.11 \text{ (6" SDR-26 ASTM D3034)}$$

9. Predicted Pipe Deflection:

Using equation 7.1, from the Uni-Bell Handbook of PVC Pipe, 3rd Edition.

$$\% \Delta Y = \frac{D_L K P (100)}{}$$

$$D = 0.149P_s + 0.061E'$$

$$P = y_s H / 144$$

$\% \Delta Y/D$ = Predicted % vertical deflection under load

P = Prism Load (psi)

K = Bedding angle constant. Assumed to be 0.1

E' = Modulus of soil reaction = 1000 psi

D_L = Deflection lag factor = 1.5

y_s = Unit weight of soil = 120 pcf

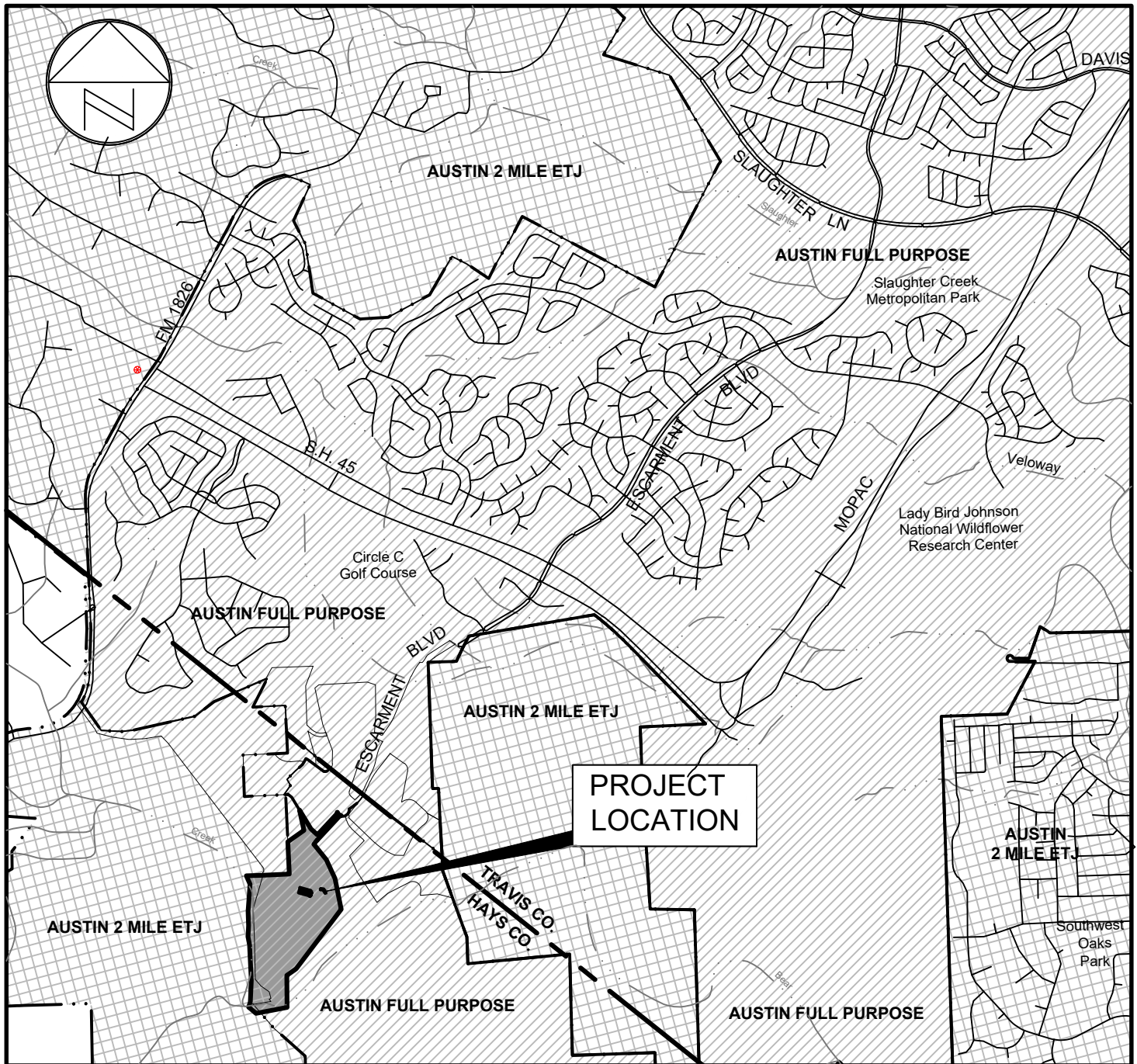
H = Depth of burial (ft) from ground surface to crown of pipe

$$P = 120 \times 15.0 / 144 = 12.50 \text{ psi (6" SDR-26 ASTM D3034 PVC)}$$

6" SDR-26 ASTM D3034 PVC

$$\% \Delta Y/D = (1.5)(0.1)(12.50)(100) / [(.149)(128.86) + (0.061)(1000)] = 2.34\%$$

APPENDIX A - PROJECT LOCATION MAP



(NTS)

GRID NO.: A-13, A-14, WZ13

MAPSCO PAGE: 670V, 670Z, 671N, 671S, 671W

LJA Engineering, Inc.

5316 Highway 290 West
Suite 150
Austin, Texas 78735



Phone 512.439.4700
Fax 512.439.4716
FRN - F-1386

Veritas Academy
Gym Addition and Campus Improvements
Road Map

13401 Escarpment Boulevard, Austin TX

**FORM 0587,
ATTACHMENT A**

1 OF 3

APPENDIX B - WASTEWATER FLOW CALCULATIONS

Veritas Academy Gym Addition and Campus Improvements (COA SP-2023-0426C)

WASTEWATER CALCULATIONS

Use	LUE Equivalent	Population	LUEs
Prop. Trailer for Portables & Offices (12 Students / Portable & 4 Faculty)	1 LUE / 16.33 Students	52	3.2
Prop. Community Hall	1 LUE / 16.33 Students	70	4.3
Prop. Gym	1 LUE / 16.33 Students	378	23.1
Total Campus Population	1 LUE / 16.33 Students	870	53.3

AVERAGE DRY WEATHER FLOW
F = LUEs x 245/1440 gpm

PEAK FLOW FACTOR (>= 4)
PFF = $[(18 + (0.0206 \times F)^{0.5}) / (4 + (0.0206 \times F)^{0.5})]$

INFLOW / INFILTRATION
(I/I) = 750 gal./day/acre = 0.521 gpm/acre

PEAK DRY WEATHER FLOW (gpm)
Qpdwf = PFF X F

PEAK WET WEATHER FLOW (gpm)
Qpwwf = Qpdwf + I/I

WASTEWATER LINE	STATION FROM	STATION TO	LUEs	AVERAGE DRY WEATHER FLOW F (gpm)	PEAK FLOW FACTOR PFF	PEAK DRY WEATHER FLOW Qpdwf (gpm)	INFLOW / INFILTRATION I/I (gpm)	PEAK WET WEATHER FLOW Qpwwf (gpm)	PIPE SIZE (IN)	PIPE SLOPE (%)	PEAK DRY WEATHER VELOCITY Vpdwf (ft/s)	PEAK DRY WEATHER DEPTH d _{pdwf} (ft)	PEAK WET WEATHER VELOCITY Vpwwf (ft/s)	PEAK WET WEATHER DEPTH d _{pwwf} (ft)
Proposed WWL 'A'	5+62.21	6+19.86	7.5	1.27	4.36	5.55	0.78	6.33	6	1.83	2.01	0.033	2.08	0.042
Proposed WWL 'A'	2+55.31	5+62.21	30.6	5.21	4.24	22.05	1.56	23.61	6	1.50	2.44	0.080	2.49	0.082
Proposed WWL 'A'	1+00.00	2+55.31	53	9.02	4.16	37.51	1.56	39.07	8	1.00	2.39	0.104	2.42	0.107

Existng Veritas Academy 6" WWL 'A' (43.37 LF) (COA SP-2014-0249C)

WASTEWATER CALCULATIONS

Use	LUE Equivalent	Population	LUEs
2 Existing Portables (12 Students / Portable)	1 LUE / 16.33 Students	24 students	1.5

AVERAGE DRY WEATHER FLOW
F = LUEs x 245/1440 gpm

PEAK FLOW FACTOR (>= 4)
PFF = $[(18 + (0.0206 \times F)^{0.5}) / (4 + (0.0206 \times F)^{0.5})]$

INFLOW / INFILTRATION
(I/I) = 750 gal./day/acre = 0.521 gpm/acre

PEAK DRY WEATHER FLOW (gpm)
Qpdwf = PFF X F

PEAK WET WEATHER FLOW (gpm)
Qpwwf = Qpdwf + I/I

WASTEWATER LINE	STATION FROM	STATION TO	LUEs	AVERAGE DRY WEATHER FLOW F (gpm)	PEAK FLOW FACTOR PFF	PEAK DRY WEATHER FLOW Qpdwf (gpm)	INFLOW / INFILTRATION I/I (gpm)	PEAK WET WEATHER FLOW Qpwwf (gpm)	PIPE SIZE (IN)	PIPE SLOPE (%)	PEAK DRY WEATHER VELOCITY Vpdwf (ft/s)	PEAK DRY WEATHER DEPTH d _{pdwf} (ft)	PEAK WET WEATHER VELOCITY Vpwwf (ft/s)	PEAK WET WEATHER DEPTH d _{pwwf} (ft)
Existing WWL 'A'	9+14.30	8+70.93	1.5	0.26	4.44	1.13	1.56	2.70	6	3.18	1.31	0.02	1.67	0.03

Note: Existing WWL 'A' and Proposed WWL 'A' are entirely different wastewater lines.



APPENDIX C - TCEQ SCS GENERAL CONSTRUCTION NOTES

**Texas Commission on Environmental Quality
Organized Sewage Collection System
General Construction Notes**

Edwards Aquifer Protection Program Construction Notes – Legal Disclaimer

The following/listed “construction notes” are intended to be advisory in nature only and do not constitute an approval or conditional approval by the Executive Director, nor do they constitute a comprehensive listing of rules or conditions to be followed during construction. Further actions may be required to achieve compliance with TCEQ regulations found in Title 30, Texas Administrative Code, Chapters 213 and 217, as well as local ordinances and regulations providing for the protection of water quality. Additionally, nothing contained in the following/listed “construction notes” restricts the powers of the Executive Director, the commission or any other governmental entity to prevent, correct, or curtail activities that result or may result in pollution of the Edwards Aquifer or hydrologically connected surface waters. The holder of any Edwards Aquifer Protection Plan containing “construction notes” is still responsible for compliance with Title 30, Texas Administrative Code, Chapters 213 or any other applicable TCEQ regulation, as well as all conditions of an Edwards Aquifer Protection Plan through all phases of plan implementation. Failure to comply with any condition of the Executive Director’s approval, whether or not in contradiction of any “construction notes,” is a violation of TCEQ regulations and any violation is subject to administrative rules, orders, and penalties as provided under Title 30, Texas Administrative Code § 213.10 (relating to Enforcement). Such violations may also be subject to civil penalties and injunction. The following/listed “construction notes” in no way represent an approved exception by the Executive Director to any part of Title 30 Texas Administrative Code, Chapters 213 and 217, or any other TCEQ applicable regulation.

1. This Organized Sewage Collection System (SCS) must be constructed in accordance with 30 Texas Administrative Code (TAC) §213.5(c), the Texas Commission on Environmental Quality’s (TCEQ) Edwards Aquifer Rules and any local government standard specifications.
2. All contractors conducting regulated activities associated with this proposed regulated project must be provided with copies of the SCS plan and the TCEQ letter indicating the specific conditions of its approval. During the course of these regulated activities, the contractors must be required to keep on-site copies of the plan and the approval letter.
3. A written notice of construction must be submitted to the presiding TCEQ regional office at least 48 hours prior to the start of any regulated activities. This notice must include:
 - the name of the approved project;
 - the activity start date; and
 - the contact information of the prime contractor.
4. Any modification to the activities described in the referenced SCS application following the date of approval may require the submittal of an SCS application to modify this approval, including the payment of appropriate fees and all information necessary for its review and approval.
5. Prior to beginning any construction activity, all temporary erosion and sedimentation (E&S) control measures must be properly installed and maintained in accordance with the manufacturers specifications. These controls must remain in place until the disturbed areas have been permanently stabilized.
6. If any sensitive features are discovered during the wastewater line trenching activities, all regulated activities near the sensitive feature must be suspended immediately. The applicant must immediately notify the appropriate regional office of the TCEQ of the feature discovered. A geologist’s assessment of the location and extent of the feature discovered must be reported to that regional office in writing and the applicant must submit a plan for ensuring the structural integrity of the sewer line or for modifying the proposed collection system alignment around the feature. The regulated activities near the sensitive feature may not proceed until the

executive director has reviewed and approved the methods proposed to protect the sensitive feature and the Edwards Aquifer from any potentially adverse impacts to water quality while maintaining the structural integrity of the line.

7. Sewer lines located within or crossing the 5-year floodplain of a drainage way will be protected from inundation and stream velocities which could cause erosion and scouring of backfill. The trench must be capped with concrete to prevent scouring of backfill, or the sewer lines must be encased in concrete. All concrete shall have a minimum thickness of 6 inches.
8. Blasting procedures for protection of existing sewer lines and other utilities will be in accordance with the National Fire Protection Association criteria. Sand is not allowed as bedding or backfill in trenches that have been blasted. If any existing sewer lines are damaged, the lines must be repaired and retested.
9. All manholes constructed or rehabilitated on this project must have watertight size on size resilient connectors allowing for differential settlement. If manholes are constructed within the 100-year floodplain, the cover must have a gasket and be bolted to the ring. Where gasketed manhole covers are required for more than three manholes in sequence or for more than 1500 feet, alternate means of venting will be provided. Bricks are not an acceptable construction material for any portion of the manhole.

The diameter of the manholes must be a minimum of four feet and the manhole for entry must have a minimum clear opening diameter of 30 inches. These dimensions and other details showing compliance with the commission's rules concerning manholes and sewer line/manhole inverts described in 30 TAC §217.55 are included on Plan Sheet 32 of 40.

It is suggested that entrance into manholes in excess of four feet deep be accomplished by means of a portable ladder. The inclusion of steps in a manhole is prohibited.

10. Where water lines and new sewer line are installed with a separation distance closer than nine feet (i.e., water lines crossing wastewater lines, water lines paralleling wastewater lines, or water lines next to manholes) the installation must meet the requirements of 30 TAC §217.53(d) (Pipe Design) and 30 TAC §290.44(e) (Water Distribution).
11. Where sewers lines deviate from straight alignment and uniform grade all curvature of sewer pipe must be achieved by the following procedure which is recommended by the pipe manufacturer: No curvature of sewer lines is proposed.

If pipe flexure is proposed, the following method of preventing deflection of the joint must be used: No flexure is proposed within the Sewage Collection System.

Specific care must be taken to ensure that the joint is placed in the center of the trench and properly bedded in accordance with 30 TAC §217.54.

12. New sewage collection system lines must be constructed with stub outs for the connection of anticipated extensions. The location of such stub outs must be marked on the ground such that their location can be easily determined at the time of connection of the extensions. Such stub outs must be manufactured wyes or tees that are compatible in size and material with both the sewer line and the extension. At the time of original construction, new stub-outs must be constructed sufficiently to extend beyond the end of the street pavement. All stub-outs must be sealed with a manufactured cap to prevent leakage. Extensions that were not anticipated at the time of original construction or that are to be connected to an existing sewer line not furnished with stub outs must be connected using a manufactured saddle and in accordance with accepted plumbing techniques.

If no stub-out is present an alternate method of joining laterals is shown in the detail on Plan Sheet ___ of ___. (For potential future laterals). There are no stub-outs proposed for future lateral connections.

The private service lateral stub-outs must be installed as shown on the plan and profile sheets on Plan Sheet 31 of 40 and marked after backfilling as shown in the detail on Plan Sheet 31 of 40.

13. Trenching, bedding and backfill must conform with 30 TAC §217.54. The bedding and backfill for flexible pipe must comply with the standards of ASTM D-2321, Classes IA, IB, II or III. Rigid pipe bedding must comply with the requirements of ASTM C 12 (ANSI A 106.2) classes A, B or C.
14. Sewer lines must be tested from manhole to manhole. When a new sewer line is connected to an existing stub or clean-out, it must be tested from existing manhole to new manhole. If a stub or clean-out is used at the end of the proposed sewer line, no private service attachments may be connected between the last manhole and the cleanout unless it can be certified as conforming with the provisions of 30 TAC §213.5(c)(3)(E).
15. All sewer lines must be tested in accordance with 30 TAC §217.57. The engineer must retain copies of all test results which must be made available to the executive director upon request. The engineer must certify in writing that all wastewater lines have passed all required testing to the appropriate regional office within 30 days of test completion and prior to use of the new collection system. Testing method will be:
 - (a) For a collection system pipe that will transport wastewater by gravity flow, the design must specify an infiltration and exfiltration test or a low-pressure air test. A test must conform to the following requirements:
 - (1) *Low Pressure Air Test.*
 - (A) A low pressure air test must follow the procedures described in American Society For Testing And Materials (ASTM) C-828, ASTM C-924, or ASTM F-1417 or other procedure approved by the executive director, except as to testing times as required in Table C.3 in subparagraph (C) of this paragraph or Equation C.3 in subparagraph (B)(ii) of this paragraph.
 - (B) For sections of collection system pipe less than 36 inch average inside diameter, the following procedure must apply, unless a pipe is to be tested as required by paragraph (2) of this subsection.
 - (i) A pipe must be pressurized to 3.5 pounds per square inch (psi) greater than the pressure exerted by groundwater above the pipe.
 - (ii) Once the pressure is stabilized, the minimum time allowable for the pressure to drop from 3.5 psi gauge to 2.5 psi gauge is computed from the following equation:

Equation C.3

$$T = \frac{0.085 \times D \times K}{Q}$$

Where:

- T = time for pressure to drop 1.0 pound per square inch gauge in seconds
K = 0.000419 X D X L, but not less than 1.0
D = average inside pipe diameter in inches

L = length of line of same size being tested, in feet
 Q = rate of loss, 0.0015 cubic feet per minute per square foot internal surface

- (C) Since a K value of less than 1.0 may not be used, the minimum testing time for each pipe diameter is shown in the following Table C.3:

Pipe Diameter (inches)	Minimum Time (seconds)	Maximum Length for Minimum Time (feet)	Time for Longer Length (seconds/foot)
6	340	398	0.855
8	454	298	1.520
10	567	239	2.374
12	680	199	3.419
15	850	159	5.342
18	1020	133	7.693
21	1190	114	10.471
24	1360	100	13.676
27	1530	88	17.309
30	1700	80	21.369
33	1870	72	25.856

- (D) An owner may stop a test if no pressure loss has occurred during the first 25% of the calculated testing time.
- (E) If any pressure loss or leakage has occurred during the first 25% of a testing period, then the test must continue for the entire test duration as outlined above or until failure.
- (F) Wastewater collection system pipes with a 27 inch or larger average inside diameter may be air tested at each joint instead of following the procedure outlined in this section.
- (G) A testing procedure for pipe with an inside diameter greater than 33 inches must be approved by the executive director.
- (2) ***Infiltration/Exfiltration Test.***
- (A) The total exfiltration, as determined by a hydrostatic head test, must not exceed 50 gallons per inch of diameter per mile of pipe per 24 hours at a minimum test head of 2.0 feet above the crown of a pipe at an upstream manhole.
- (B) An owner shall use an infiltration test in lieu of an exfiltration test when pipes are installed below the groundwater level.
- (C) The total exfiltration, as determined by a hydrostatic head test, must not exceed 50 gallons per inch diameter per mile of pipe per 24 hours at a minimum test head of two feet above the crown of a pipe at an upstream manhole, or at least two feet above existing groundwater level, whichever is greater.
- (D) For construction within a 25-year flood plain, the infiltration or exfiltration must not exceed 10 gallons per inch diameter per mile of pipe per 24 hours at the same minimum test head as in subparagraph (C) of this paragraph.
- (E) If the quantity of infiltration or exfiltration exceeds the maximum quantity specified, an owner shall undertake remedial action in order to reduce

the infiltration or exfiltration to an amount within the limits specified. An owner shall retest a pipe following a remediation action.

- (b) If a gravity collection pipe is composed of flexible pipe, deflection testing is also required. The following procedures must be followed:

- (1) For a collection pipe with inside diameter less than 27 inches, deflection measurement requires a rigid mandrel.

(A) *Mandrel Sizing.*

- (i) A rigid mandrel must have an outside diameter (OD) not less than 95% of the base inside diameter (ID) or average ID of a pipe, as specified in the appropriate standard by the ASTMs, American Water Works Association, UNI-BELL, or American National Standards Institute, or any related appendix.
- (ii) If a mandrel sizing diameter is not specified in the appropriate standard, the mandrel must have an OD equal to 95% of the ID of a pipe. In this case, the ID of the pipe, for the purpose of determining the OD of the mandrel, must equal be the average outside diameter minus two minimum wall thicknesses for OD controlled pipe and the average inside diameter for ID controlled pipe.
- (iii) All dimensions must meet the appropriate standard.

(B) *Mandrel Design.*

- (i) A rigid mandrel must be constructed of a metal or a rigid plastic material that can withstand 200 psi without being deformed.
- (ii) A mandrel must have nine or more odd number of runners or legs.
- (iii) A barrel section length must equal at least 75% of the inside diameter of a pipe.
- (iv) Each size mandrel must use a separate proving ring.

(C) *Method Options.*

- (i) An adjustable or flexible mandrel is prohibited.
- (ii) A test may not use television inspection as a substitute for a deflection test.
- (iii) If requested, the executive director may approve the use of a deflectometer or a mandrel with removable legs or runners on a case-by-case basis.

- (2) For a gravity collection system pipe with an inside diameter 27 inches and greater, other test methods may be used to determine vertical deflection.
- (3) A deflection test method must be accurate to within plus or minus 0.2% deflection.
- (4) An owner shall not conduct a deflection test until at least 30 days after the final backfill.
- (5) Gravity collection system pipe deflection must not exceed five percent (5%).
- (6) If a pipe section fails a deflection test, an owner shall correct the problem and conduct a second test after the final backfill has been in place at least 30 days.

16. All manholes must be tested to meet or exceed the requirements of 30 TAC §217.58.

- (a) All manholes must pass a leakage test.
- (b) An owner shall test each manhole (after assembly and backfilling) for leakage, separate and independent of the collection system pipes, by hydrostatic exfiltration testing, vacuum testing, or other method approved by the executive director.
 - (1) Hydrostatic Testing.

- (A) The maximum leakage for hydrostatic testing or any alternative test methods is 0.025 gallons per foot diameter per foot of manhole depth per hour.
- (B) To perform a hydrostatic exfiltration test, an owner shall seal all wastewater pipes coming into a manhole with an internal pipe plug, fill the manhole with water, and maintain the test for at least one hour.
- (C) A test for concrete manholes may use a 24-hour wetting period before testing to allow saturation of the concrete.

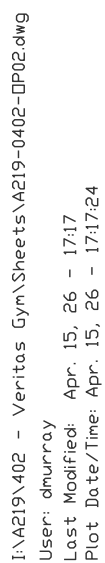
(2) Vacuum Testing.

- (A) To perform a vacuum test, an owner shall plug all lift holes and exterior joints with a non-shrink grout and plug all pipes entering a manhole.
- (B) No grout must be placed in horizontal joints before testing.
- (C) Stub-outs, manhole boots, and pipe plugs must be secured to prevent movement while a vacuum is drawn.
- (D) An owner shall use a minimum 60 inch/lb torque wrench to tighten the external clamps that secure a test cover to the top of a manhole.
- (E) A test head must be placed at the inside of the top of a cone section, and the seal inflated in accordance with the manufacturer's recommendations.
- (F) There must be a vacuum of 10 inches of mercury inside a manhole to perform a valid test.
- (G) A test does not begin until after the vacuum pump is off.
- (H) A manhole passes the test if after 2.0 minutes and with all valves closed, the vacuum is at least 9.0 inches of mercury.

17. All private service laterals must be inspected and certified in accordance with 30 TAC §213.5(c)(3)(I). After installation of and, prior to covering and connecting a private service lateral to an existing organized sewage collection system, a Texas Licensed Professional Engineer, Texas Registered Sanitarian, or appropriate city inspector must visually inspect the private service lateral and the connection to the sewage collection system, and certify that it is constructed in conformity with the applicable provisions of this section. The owner of the collection system must maintain such certifications for five years and forward copies to the appropriate regional office upon request. Connections may only be made to an approved sewage collection system.

Austin Regional Office 12100 Park 35 Circle, Building A Austin, Texas 78753-1808 Phone (512) 339-2929 Fax (512) 339-3795	San Antonio Regional Office 14250 Judson Road San Antonio, Texas 78233-4480 Phone (210) 490-3096 Fax (210) 545-4329
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THESE GENERAL CONSTRUCTION NOTES MUST BE INCLUDED ON THE CONSTRUCTION PLANS PROVIDED TO THE CONTRACTOR AND ALL SUBCONTRACTORS.



ATLAS 14 UNPAVED 16.1345 PAVED 20.3282

Time of Concentration Calculations - Developed Conditions												
Drainage Area	SHEET					SCF				CHANNEL FLOW		
	L(ft)	Slope	2-Year P	n	Tc (Min.)	L(ft)	Paved?	Slope	Tc (Min.)	L(ft)	Velocity (ft/s)	Tc (Min.)
D1	100	5.00%	4.14	0.24	8.7	1533	NO	4.40%	7.5	337	1.07	5.2
D2	5 Minutes Assumed											
D3	5 Minutes Assumed											
											Total Tc (Min.)	Lag Tc (Min.)
											5.0	3.0
											5.0	3.0

HEC-HMS

Drainage Area Calculations

COA Full Purpose, Travis County, Texas

NOAA ATLAS 14 24Hr Rainfall Distribution Precipitation Data

Developed Ground Cover	CN
Pasture, Good Condition (Type D Soils)	80
Impervious Pavement, Buildings, Sidewalks	98

Developed Conditions Drainage Calculations (Per HEC-HMS Drainage Model)

Type D												
AREA NAME	AC.	AREA MI. ²	IMPERVIOUS COVER %	Ac.	IMPERVIOUS COVER MI. ²	Pasture (CN: 80) Ac.	ML. ²	Composite CN	Tc (Lag) min.	Q2 CFS	Q10 CFS	Q25 CFS
D1	43.47	0.06792188	0.00%	0.000	0.000000	43.470	0.0679	80	12.90	105.0	187.3	243.7
D2	8.98	0.01403125	38.94%	3.497	0.005464	5.483	0.0086	87	3.00	38.8	64.9	82.3
D3	4.46	0.00696875	57.33%	2.557	0.003995	1.903	0.0030	90	3.00	20.1	33.2	41.8
DETENTION POND DISCHARGE										6.7	8.2	11.2
STUDY POINT										111.3	194.9	252.1
										397.8		

DETENTION POND			
STORM EVENT	INFLOW (CFS)	OUTFLOW (CFS)	WS ELEV.
2YR	51.1	6.7	850.7
10YR	88.1	8.2	852.3
25YR	112.7	11.2	853.4
100YR	152.8	68.0	854.0

STUDY POINT		
STORM	Q(EX) CFS	Q(DEV) CFS
2YR	113.0	111.3
10YR	218.1	194.9
25YR	289.3	252.1
100YR	404.8	397.8

Veritas Academy Gym Dam Safety Calculations

Maximum Water Surface Elevations in Spillway (ft., msl.)									
1 Hr. PMP	2 Hr. PMP	3 Hr. PMP	6 Hr. PMP	12 Hr. PMP	24 Hr. PMP	48 Hr. PMP	72 Hr. PMP		
854.5	854.6	854.5	854.3	854.1	854.0	853.7	853.6		

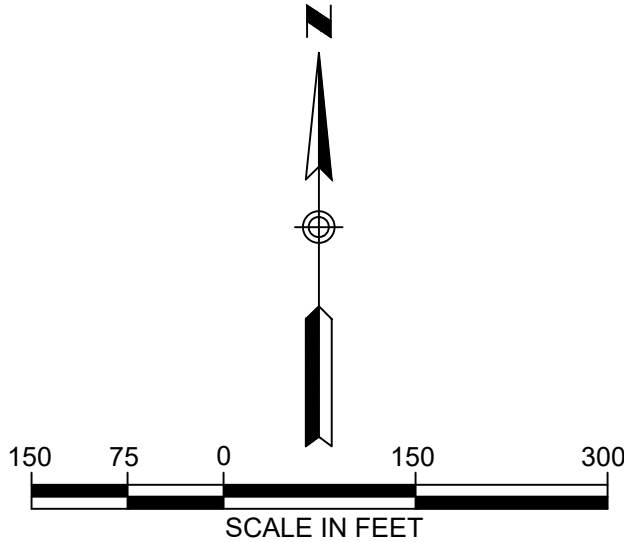
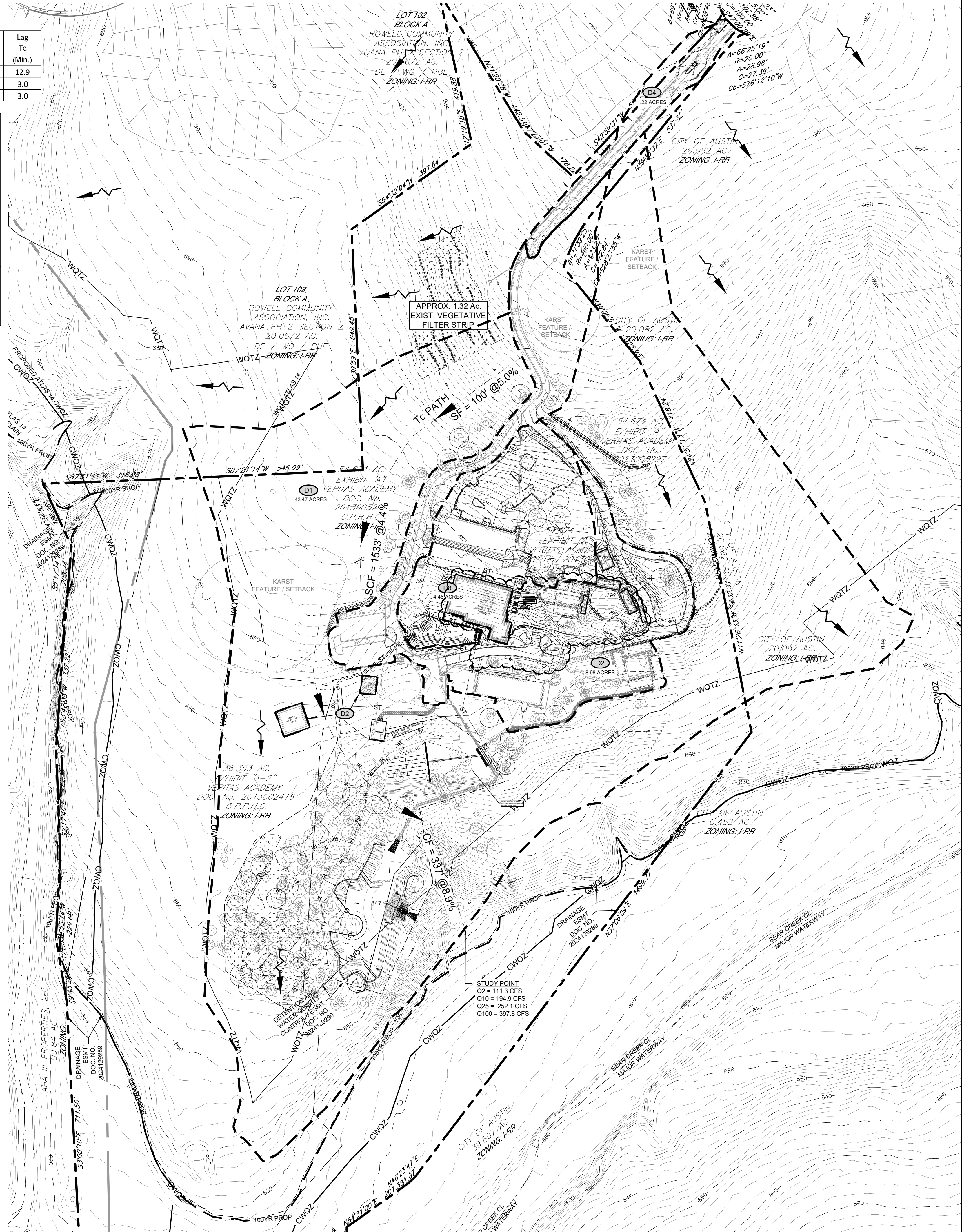
Detention Pond Stage - Storage - Discharge									
Elevation	Area (sf)	Volume (cf)	Cum. Volume (cf)	Volume (ac-ft)	Weir 1 (cfs)	Orifice (cfs)	Total (cfs)		
847.0	-	0	-	0.000	0.00	0.00	0.00		
848.0	4,622	2311	2,311	0.053	0.00	2.67	2.67		
849.0	17,710	11166	13,477	0.309	0.00	4.63	4.63		
850.0	32,708	25209	38,686	0.888	0.00	5.98	5.98		
851.0	40,683	36696	75,382	1.731	0.00	7.07	7.07		
852.0	42,500	41592	116,973	2.685	0.00	8.02	8.02		
853.0	43,879	43190	160,163	3.677	0.00	8.87	8.87		
854.0	45,284	44582	204,744	4.700	60.01	9.64	69.65		
855.0	46,900	46092	250,836	5.758	249.73	10.36	260.08		
		40	Weir 1 Length	853.37	Weir 1 Elevation				

Weir Discha $C_w \cdot L \cdot h^{3/2}$
Orifice Disch $C_d \cdot A \cdot (2 \cdot g \cdot h)^{1/2}$

Where C_w is assumed to be 3.
0.6 C_d

PIPE OR ORIFICE? (PIPE, OR)
12 Pipe Diameter or Orifice Height (in)
0.79 Area of Pipe
1.00 Number of Pipes / Orifices
0.79 Total Area

	Design Capacity		Proposed	
	Total AC.	ICAC.	Total AC.	ICAC.
Existing Water Quality Pond	8.98	3.50	8.98	3.20
Proposed Water Quality Pond	4.46	2.56	4.46	2.25



LEGEND

- 2.59 ACRES
- DRAINAGE AREA NUMBER AND ACREAGE
- DRAINAGE AREA
- PROPERTY LINE
- TC PATH
- SF
- SCF
- SHEET FLOW
- SHALLOW CONCENTRATED FLOW
- Q25 = 16.64 cfs
- Q100 = 23.62 cfs
- Q25
- Q100
- FLOW DIRECTION ARROW

- NOTE:
1. DRAINAGE CALCULATIONS ASSUME MAXIMUM ALLOWABLE IMPERVIOUS COVER (6.6 ACRES). ONLY 6.09 ACRES ARE PROPOSED WITH THIS SITE PLAN. (SEE SITE CALCS)
 2. DRAINAGE AREA D1 IS UNDEVELOPED AND SHALL DRAIN OFFSITE. WATER QUALITY SHALL BE PROVIDED FOR DRAINAGE AREA D2 VIA THE EXISTING RET-IRR POND. WATER QUALITY SHALL BE PROVIDED FOR DRAINAGE AREA D3 VIA THE PROPOSED RET-IRR POND. WATER QUALITY SHALL BE PROVIDED FOR DRAINAGE AREA D4 VIA THE EXISTING VFS.

VERITAS ACADEMY GYM ADDITION & CAMPUS IMPROVEMENTS PROPOSED CONDITIONS DRAINAGE AREA MAP

13401 ESCARPMENT BLVD., AUSTIN TEXAS

REVISONS		DATE	BY
DESCRIPTION			
1		01/14/2026	DSM

JOB NUMBER:	A219-0402
DATE:	11/14/20
DESIGNED BY:	DSM
DRAWN BY:	JLK
CHECKED BY:	RBH
DRAWING NAME:	A219-0402-DM02



01/14/2026

LJA Engineering, Inc.
7500 Riata Boulevard
Building II, Suite 100
Austin, Texas 78735
Phone 512.439.4700
Fax 512.439.4716
FRN-F-1386

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FRN-F-1386

JOB NUMBER:
A219-0402

DM02

SHEET NO.

7

OF 52 SHEETS

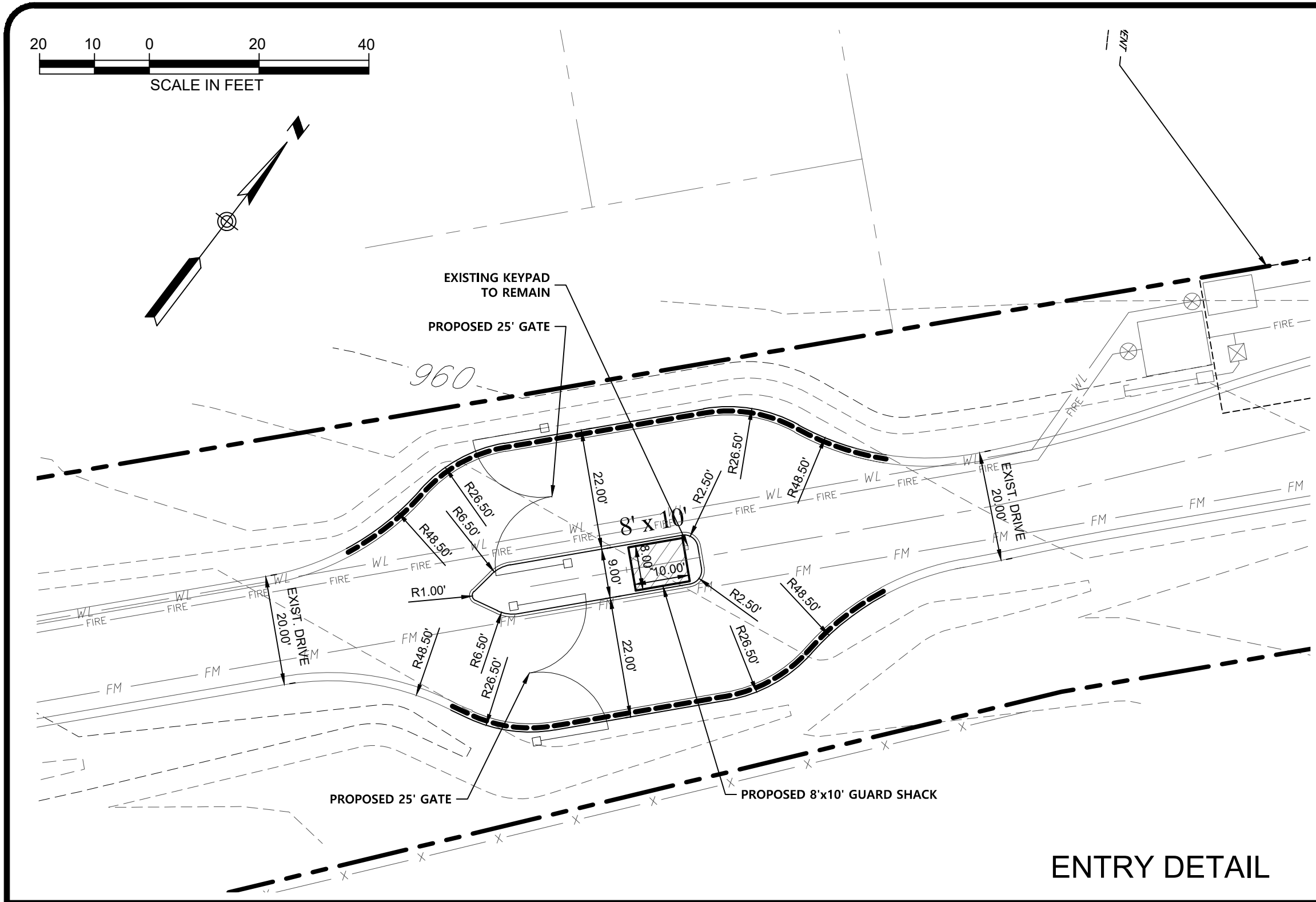
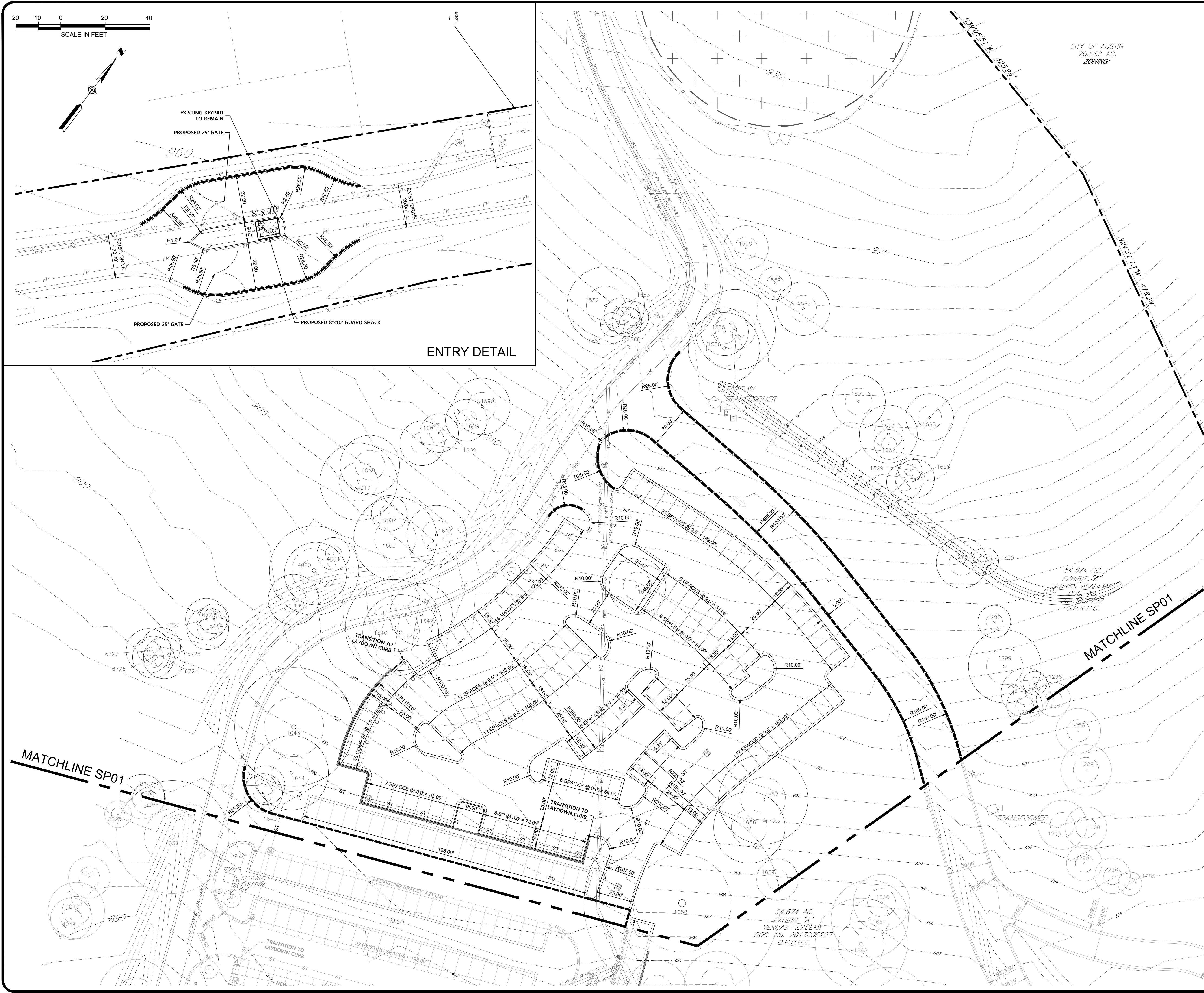
LOCATION OF EXISTING UNDERGROUND AND OVERHEAD UTILITIES ARE APPROXIMATE LOCATIONS ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES PRIOR TO BEGINNING WORK AND SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT OCCUR.



SP-2023-0426C

C1 REPLACEMENT SHEET

1/4/2019/1402 - Veritas Gym Addition/2018-0402-SP01.dwg
User: jwrtac
Last Modified: Oct. 16, 23 - 1943
Plot Date/Time: Nov. 13, 23 - 10:55:28



LEGEND	
PROPOSED	EXISTING

- NOTES:
1. ALL DIMENSIONS ARE TO FACE OF CURB UNLESS OTHERWISE NOTED.
 2. ALL RADII ARE 3' UNLESS OTHERWISE NOTED.
 3. ALL SIDEWALKS ADJACENT TO BACK OF CURB SHALL BE 5' WIDE. ALL OTHER SIDEWALKS SHALL BE 4' WIDE UNLESS OTHERWISE NOTED.
 4. ALL PAVEMENT SHALL BE CONCRETE WITH CURB AND GUTTER.

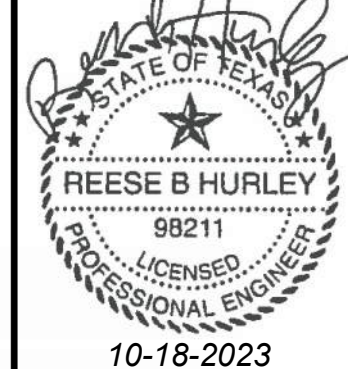
LOCATION OF EXISTING UNDERGROUND AND OVERHEAD UTILITIES ARE APPROXIMATE LOCATIONS ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES PRIOR TO BEGINNING WORK AND SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT OCCUR.



VERITAS ACADEMY
GYM ADDITION & CAMPUS IMPROVEMENTS
SITE PLAN 1

REVISIONS		DATE
NO	DESCRIPTION	BY

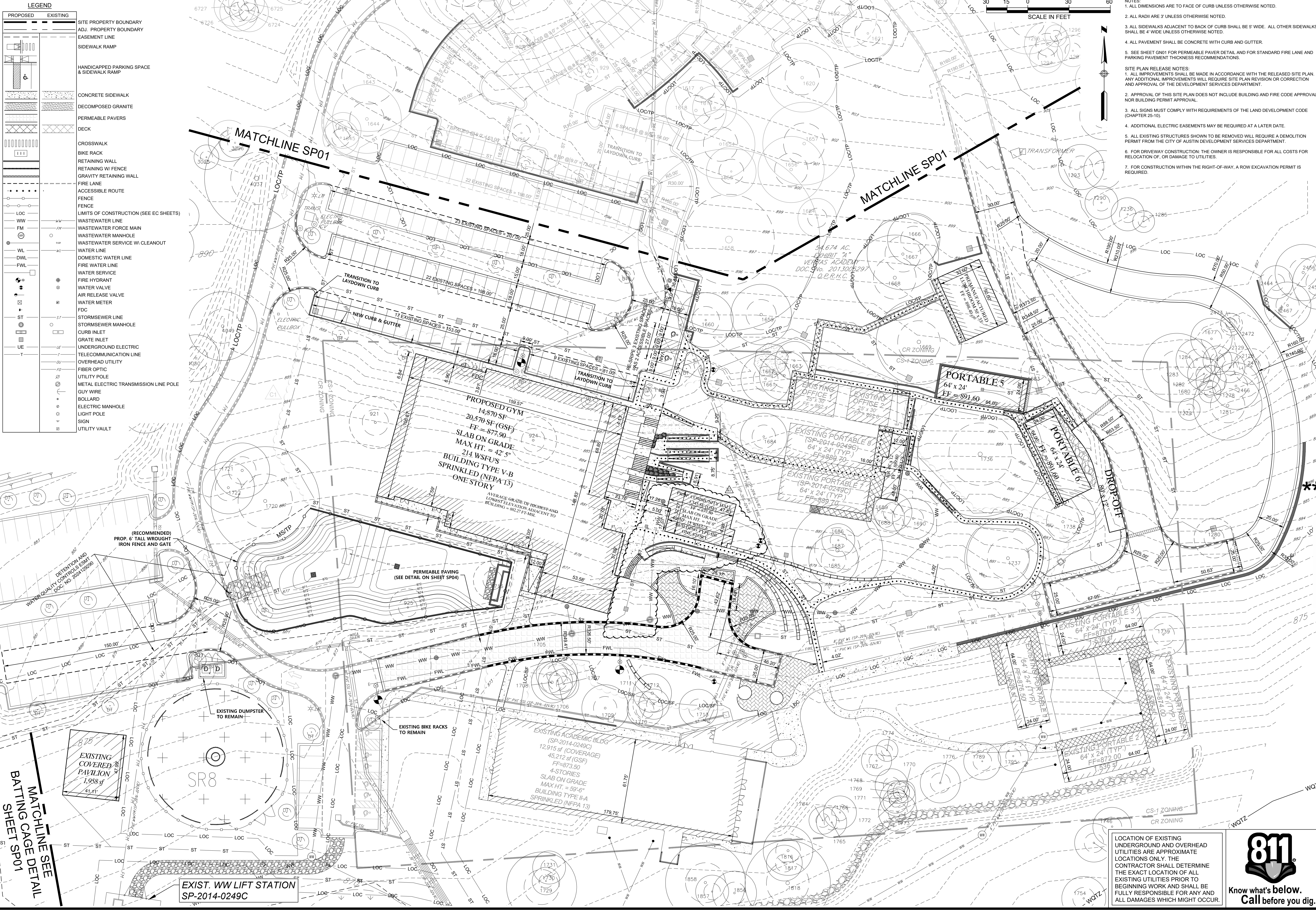
JOB NUMBER:	A210-0402
DATE:	11/13/23
DESIGNED BY:	DSM
DRAWN BY:	JKK
CHECKED BY:	RBH
DRAWING NAME:	A210-0402-SP01.DWG



LJA Engineering, Inc.
7500 Riata Boulevard
Building II, Suite 100
Austin, Texas 78735
Phone 512.439.4700
Fax 512.439.4716
FRN-F-1386

JOB NUMBER:	A210-0402
SP01	
SHEET NO.	17
OF 40 SHEETS	

PROPOSED	EXISTING	
		SITE PROPERTY BOUNDARY
		ADJ. PROPERTY BOUNDARY
		EASEMENT LINE
		SIDEWALK RAMP
		HANDICAPPED PARKING SPACE & SIDEWALK RAMP
		CONCRETE SIDEWALK
		DECOMPOSED GRANITE
		PERMEABLE PAVERS
		DECK
		CROSSWALK
		BIKE RACK
		RETAINING WALL
		RETAINING W/ FENCE
		GRAVITY RETAINING WALL
		FIRE LANE
		ACCESSIBLE ROUTE
		FENCE
		LIMITS OF CONSTRUCTION (SEE EC SHEETS)
		WASTEWATER LINE
		WASTEWATER FORCE MAIN
		WASTEWATER MANHOLE
		WASTEWATER SERVICE W/ CLEANOUT
		WATER LINE
		DOMESTIC WATER LINE
		FIRE WATER LINE
		WATER SERVICE
		FIRE HYDRANT
		WATER VALVE
		AIR RELEASE VALVE
		WATER METER
		FDC
		STORMSEWER LINE
		STORMSEWER MANHOLE
		CURB INLET
		GRATE INLET
		UNDERGROUND ELECTRIC
		TELECOMMUNICATION LINE
		OVERHEAD UTILITY
		FIBER OPTIC
		UTILITY POLE
		METAL ELECTRIC TRANSMISSION LINE POLE
		GUY WIRE
		BOLLARD
		ELECTRIC MANHOLE
		LIGHT POLE
		SIGN
		UTILITY VAULT



SLAT STORMWATER LOAD ANALYSIS TOOL 2.1

Quick Guide

1. Enable Macros in the worksheet.
2. Click "Restore Defaults" button to the right.
3. Fill all yellow cells with project specifics, moving from top to bottom.
4. Click "View Full Results" button.
5. Project Passes If Green "COMPLIANT" button appears.

Find the Full User Manual at SLAT@AustinTexas.gov

Click Here To
Restore Defaults
For a New Analysis

Click Here To
View Results

KEY Required User Input Key User Input Internal Calculation Error Calculator Output Does Not Apply

Step 1: Input site characteristics in yellow highlighted cells

Veritas 02	Dylan Murray	8/7/2023	SLAT 2.1 - 08/2022
Is your site within the Barton Springs Zone (BSZ)?	Yes		
How many drainage areas, n_{max} does your site have?	2		
Drainage area to the control, A_c (acres)	4.46	8.98	
Base impervious cover of the drainage area, I_{cB} (%)	0.0	0.0	
Developed impervious cover of the drainage area, I_{cD} (%)	57.3	38.9	

Step 2: Input SCM characteristics in yellow highlighted cells

	Drainage Area A	Drainage Area B
SCM 1 (First in Series)	SCM A1	SCM B1
SCM Type	Retention Basin	Retention Basin
Is SCM 1 off-line?	Yes (Off-Line)	Yes (Off-Line)
What is the Water Quality Volume, WQV (inches) [aka Capture Depth]	1.01	1.55
Minimum water quality volume allowed (in)	0.87	0.69
SCM 1 Actual Volume (ft ³)	1652	5026
Do you know the drawdown time or the flow rate?	Drawdown Time	Drawdown Time
Drawdown Time, DDT (hrs) [tot. time to empty full SCM]	60	60
Flow Rate (gpm) [only for "alternative" controls]		
Treatment Rate, Q (in/hr)	0.017	0.026
Do you already know the runoff capture efficiency?	No	No
User Entered Runoff Capture Efficiency, RCE (%)		
Runoff Capture Efficiency, RCE (%)	92.3%	97.3%
Conveyance		
How is effluent from SCM 1 discharged?	Pumped	Pumped
Delay after end of rainfall before discharging SCM 1 (hrs)	12	12
SCM 2 (Second in Series)	SCM A2	SCM B2
SCM Type	Infiltration Field	Infiltration Field
Do you know the infiltrated or reused water quantity?	No; infiltrate all routed water	No; infiltrate all routed water
User-entered infiltr. water quality volume, WQV _{inf} (in)		
-OR- Percent of yearly runoff infiltrated, RCE _{inf} (%)		
Soil infiltration rate (in/hr)	0.2	0.2
Ratio of drawdown time / irrigation time, for any zone	2	2
Approximate Minimum Field Area (Ac)	0.75	2.32

Note: VFS previously approved with SP-2014-0249C

APPENDIX R-9 VEGETATIVE FILTER STRIP POND CALCULATIONS FOR DEVELOPMENT PERMITS

DRAINAGE AREA AND WATER QUALITY VOLUME DATA:

Drainage Area (DA)	14,670 ac.	
Drainage Area Impervious Cover	44.31 %	6,500 ac
Capture Depth (CD)	1.12 in.	
Total Site Required Water Quality Volume (WQV=CD*DA*3630)	59,642 cf	

VEGETATIVE FILTER STRIP CALCULATIONS:

Drainage Area to Proposed Vegetative Filter Strip	1,222 ac	
Impervious Cover of area treated by Vegetative Filter Strip (Treated IC)	0.446 ac	
Impervious Cover of area treated by Vegetative Filter Strip (Treated IC)	36.50 %	
Soil Type (Type A, B, C, D, Amended C, or Amended D)	Amended D	

	Required	Provided
Size of Vegetative Filter Strip per ECM 1.6.7(B) - Figure B-1 = 0.92*DA	1,124 ac	1,320 ac
Width of Vegetative Filter Strip (VFS _{WIDTH})		320 ft
Hydraulic Loading Rate (HLR _{VFS} =Q _{PEAK} /VFS _{WIDTH})		0.0381 cfs/ft

WATER QUALITY CREDIT:

Impervious Area Factor (IAF = Treated IC/IC)	maximum 1.0	0.069
Percent Infiltration Provided by VFS (I _{VFS}) per ECM 1.6.7.5(B) - Figure B-2		54 %
BMP Design Factor (BMPDF)		
For HLF<0.05 cfs/ft: BMPDF=I _{VFS} /65	maximum 1.0	0.83
For HLF>0.05 and <0.15 cfs/ft: BMPDF = (I _{VFS} /65)*(0.05/HLR _{VFS})	maximum 1.0	N/A
Water Quality Credit (WQC = IAF*BMPDF)	maximum 1.0	0.057004
Water Quality Volume Reduction (WQV/WQC)		3,400 cf

Site Name: Veritas 02 By: Dylan Murray Date: 45145
RESULTS: COMPLIANCE TABLE SLAT 2.1 - 08/2022

POLLUTANT	DEVELOPED LOAD, WITH CONTROLS					EXISTING	LOAD EQUIV.	COMPLIES?	
	Drainage Area A	Drainage Area B	Drainage Area C	Drainage Area D	TOTAL LOAD,	LOAD	FACTOR, LEF		
CO ₂	lbs/yr	5.56E+01	1.99E+01	0.00E+00	0.00E+00	7.56E+01	1.54E+02	0.49	YES
E. coli	10 ⁶ MPN/yr	7.77E+04	2.85E+04	0.00E+00	0.00E+00	1.06E+05	1.51E+05	0.71	YES
Pb	lbs/yr	1.14E-02	4.11E-03	0.00E+00	0.00E+00	1.55E-02	1.70E-02	0.91	YES
TN	lbs/yr	2.07E+00	8.47E-01	0.00E+00	0.00E+00	2.92E+00	4.72E+00	0.62	YES
TP	lbs/yr	3.51E-01	1.29E-01	0.00E+00	0.00E+00	4.80E-01	4.92E-01	0.99	YES
TSS	lbs/yr	8.04E+01	2.29E+01	0.00E+00	0.00E+00	1.03E+02	6.59E+02	0.16	YES
Zn	lbs/yr	6.70E-02	2.49E-02	0.00E+00	0.00E+00	9.19E-02	9.35E-02	0.98	YES

Results subject to review and approval by CMA Development Services Department

SUMMARY OF INPUTS

Site Location	Drainage Area A	Drainage Area B	Drainage Area C	Drainage Area D	TOTALS
Within Barton Springs Zone - Compare to Existing Loads	4.46	8.98	N/A	N/A	13.44
Developed IC, I _{cD} (%)	57.3	38.9	N/A	N/A	45%
SCM 1	Retention Basin	Retention Basin	N/A	N/A	-
Water Qual. Vol, WQV (in)	1.01	1.55	N/A	N/A	-
Actual Volume (ft ³)	1652	5026	N/A	N/A	66878
Drawdown Time, DDT (hrs)	60	60	N/A	N/A	-
Flowrate (gpm)	33.58	105	N/A	N/A	-
SCM 2	Infiltration Field	Infiltration Field	N/A	N/A	-
Infiltration Rate (in/hr)	0.20	0.20	N/A	N/A	-
Appx. Min. Infiltr. Field Area (Ac)	0.75	2.32	N/A	N/A	3.07
Irrigation Rate (gpm)	34.9	105.0	N/A	N/A	-
Error with Input Values?	NO	NO	NO	NO	-

APPENDIX R-5 RETENTION/IRRIGATION POND CALCULATIONS FOR DEVELOPMENT PERMITS VERITAS ACADEMY EXISTING WQ POND

DRAINAGE AREA DATA:

DRAINAGE AREA TO CONTROL (DA)	8.98 ac.	
DRAINAGE AREA IMPERVIOUS COVER (IC)	38.94 %	3,497 ac.
CAPTURE DEPTH (CD) = PER SLAT TOOL	1.55 in.	

WATER QUALITY CONTROL CALCULATIONS: REQUIRED PROVIDED

THE WATER QUALITY CONTROL IS TO BE		
RETENTION IRRIGATION		
25-YR PEAK FLOW RATE TO CONTROL (Q25)	72.5 cfs	
100-YR PEAK FLOW RATE TO CONTROL (Q100)	99.1 cfs	

WATER QUALITY VOLUME (WQV = CD*DA*3630)	50,526 cf	57,464 cf
RETENTION POND VOLUME	50,526 cf	88,347 cf

WATER QUALITY ELEVATION	866.79 ft. msl	
ELEVATION OF SPLITTER/OVERFLOW WEIR	866.79 ft. msl	867.00 ft. msl

LENGTH OF SPLITTER WEIR		53.00 ft
REQUIRED HEAD TO PASS Q100		0.73 ft
POND FREEBOARD PROVIDED TO PASS Q100		0.27 ft

Volume Calculations	Pond Hydraulic Calculations
ELEV. (SF)	AREA (SF)
864	8169
865	16065
866	24159
867	26310
868	35457
	30884
	88347

WATER QUALITY CREDIT: 0.057004

WATER QUALITY VOLUME REDUCTION (WQV/WQC): 3,400 cf

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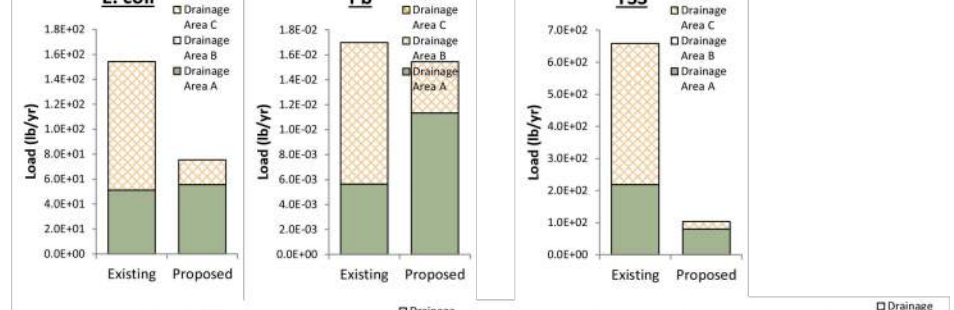
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Site Name: Veritas 02 By: Dylan Murray Date: 45145
RESULTS: PLOTS SLAT 2.1 - 08/2022



Results subject to review and approval by CMA Development Services Department

SUMMARY OF INPUTS

Site Location	Drainage Area A	Drainage Area B	Drainage Area C	Drainage Area D	TOTALS
Within Barton Springs Zone - Compare to Existing Loads	4.46	8.98	N/A	N/A	13.44
Developed IC, I _{cD} (%)	57.3	38.9	N/A	N/A	45%
SCM 1	Retention Basin	Retention Basin	N/A	N/A	-
Water Qual. Vol, WQV (in)	1.01	1.55	N/A	N/A	-
Actual Volume (ft ³)	1652	5026	N/A	N/A	66878
Drawdown Time, DDT (hrs)	60	60	N/A	N/A	-
Flowrate (gpm)	33.58	105	N/A	N/A	-
SCM 2	Infiltration Field	Infiltration Field	N/A	N/A	-
Infiltration Rate (in/hr)	0.20	0.20	N/A	N/A	-
Appx. Min. Infiltr. Field Area (Ac)	0.75	2.32	N/A	N/A	3.07
Irrigation Rate (gpm)	34.9	105.0	N/A	N/A	-
Error with Input Values?	NO	NO	NO	NO	-

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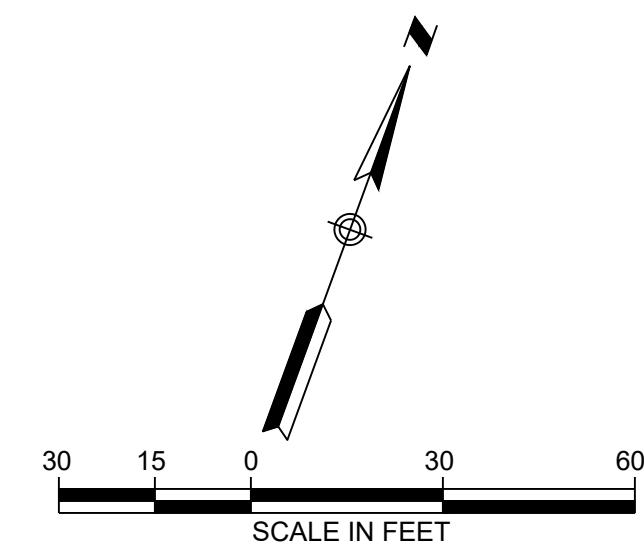
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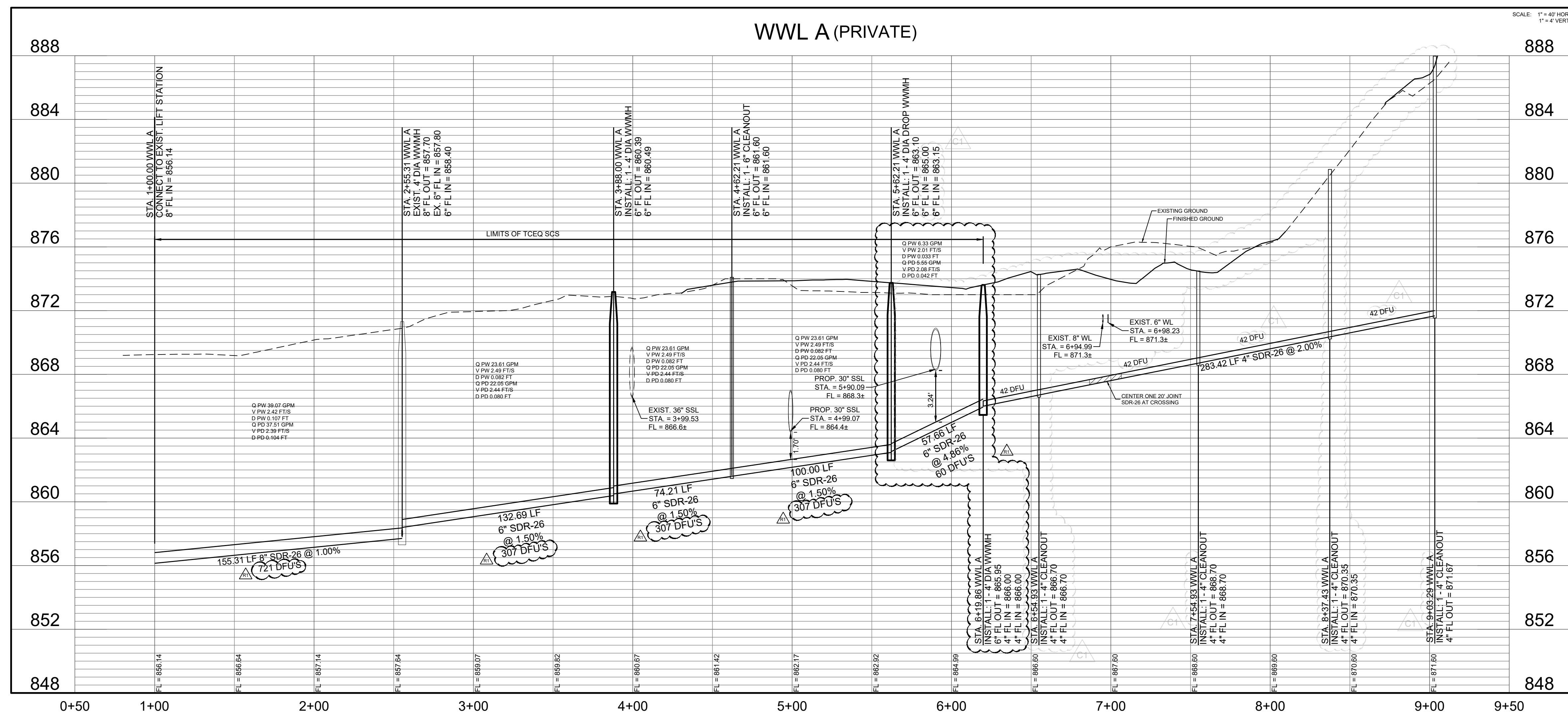
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LEGEND		
PROPOSED	EXISTING	
		SITE PROPERTY BOUNDARY
		ADJ. PROPERTY BOUNDARY
		EASEMENT LINE
		CONCRETE SIDEWALK
		DECOMPOSED GRANITE
		PAVER/BRICK SIDEWALK
		GRASSCRETE
		DECK
		RETAINING WALL
		RETAINING W/ FENCE
		GRAVITY RETAINING WALL
		FENCE
		SURVEYED CONTOUR LINE
		DRAINAGE SWALE
		WATER QUALITY TRANSITION ZONE
		WATER QUALITY BUFFER ZONE
		100 YEAR FLOODPLAIN
		WASTEWATER LINE
		WASTEWATER FORCE MAIN
		WASTEWATER SERVICE W/ CLEANOUT
		WATER LINE
		FIRE WATER LINE
		WATER SERVICE
		FIRE HYDRANT
		WATER VALVE
		WATER METER
		FDC
		STORMSEWER LINE
		STORMSEWER MANHOLE
		CURB INLET
		GRATE INLET
		UNDERGROUND ELECTRIC
		TELECOMMUNICATION LINE
		OVERHEAD UTILITY
		FIBER OPTIC
		TREE TO REMAIN
		METAL POLE
		UTILITY ELECTRIC TRANSMISSION LINE POLE
		GUY WIRE
		BOLLARD
		ELECTRIC MANHOLE
		LIGHT POLE
		SIGN
		UTILITY VAULT



WASTEWATER NOTES:

1. CONTRACTOR TO FIELD VERIFY EXACT LOCATION OF ALL EXISTING UTILITIES HORIZONTALLY AND VERTICALLY PRIOR TO CONSTRUCTION.
2. MANDREL TESTING WILL BE REQUIRED ON ALL FLEXIBLE GRAVITY WASTEWATER PIPE AS PER TCEQ RULES.
3. ALL GRAVITY WASTEWATER LINES SHALL BE SDR-26 WWL ASTM D3034 UNLESS OTHERWISE NOTED.
4. CONTRACTOR TO FILL AND COMPACT TO 96% DENSITY IN FIELD SECTIONS OVER WASTEWATER LINES. 2.0'(MIN) COVER OVER WASTEWATER PRIOR TO CONSTRUCTION.
5. ALL SERVICES SHALL TERMINATE WITH A 2-WAY CLEANOUT.
6. ALL ON-SITE UTILITIES SHALL BE LOCATED UNDERGROUND UNLESS REQUIRED BY THE UTILITY TO BE OTHERWISE LOCATED (SECTION 25-2.1125).
7. SERVICE LINE SHALL HAVE A MIN. 1% SLOPE.
8. ALL SERVICES SHALL BE 4" MIN. UNLESS OTHERWISE SHOWN.

LOCATION OF EXISTING UNDERGROUND AND OVERHEAD UTILITIES ARE APPROXIMATE LOCATIONS ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES PRIOR TO BEGINNING WORK AND SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT OCCUR.



Know what's below.
Call before you dig.

SP-2023-0426C

 REPLACEMENT SHEET

VERITAS ACADEMY
GYM ADDITION & CAMPUS IMPROVEMENTS

WASTEWATER LINE LAYOUT

13401 ESCARPMENT BLVD., AUSTIN TEXAS

[illegible]

JOB NUMBER: A21B-0402
DATE: 4/15/26
DESIGNED BY: DSM
DRAWN BY: JLK
CHECKED BY: RBH
WASTEWATER LINE
DRAWING NAME: LAYOUT DWG



The seal is circular with a five-pointed star in the center. The words "STATE OF TEXAS" are written in a circle around the star. Below the star, the name "REESE B HURLEY" is printed in a bold, sans-serif font. Underneath the name is the license number "98211". At the bottom of the seal, the words "LICENSED PROFESSIONAL ENGINEER" are written in a circular path.



04/15/2026



Phone 512.439.4700
Fax 512.439.4716
FRN-F-1386

LJA Engineering, Inc.
7500 Rialto Boulevard

LJA Engine
7500 Rialto Boulevard
Building II, Suite 100
Austin, Texas 78735

JOB NUMBER:
A219-0402

WW01

SHEET NO.

33

OF 53 SHEET

Temporary Stormwater Section

Texas Commission on Environmental Quality

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

To ensure that the application is administratively complete, confirm that all fields in the form are complete, verify that all requested information is provided, consistently reference the same site and contact person in all forms in the application, and ensure forms are signed by the appropriate party.

Note: Including all the information requested in the form and attachments contributes to more streamlined technical reviews.

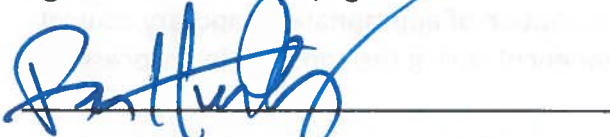
Signature

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **Temporary Stormwater Section** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Print Name of Customer/Agent: Reese Hurley, PE

Date: 4/16/2026

Signature of Customer/Agent:



Regulated Entity Name: Veritas Academy

Project Information

Potential Sources of Contamination

Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.

1. Fuels for construction equipment and hazardous substances which will be used during construction:

☐ The following fuels and/or hazardous substances will be stored on the site: _____

These fuels and/or hazardous substances will be stored in:

- ☐ Aboveground storage tanks with a cumulative storage capacity of less than 250 gallons will be stored on the site for less than one (1) year.

- ☐ Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year.
- ☐ Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project.
- ☒ Fuels and hazardous substances will not be stored on the site.
- 2. ☒ **Attachment A - Spill Response Actions.** A site specific description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is attached.
- 3. ☒ Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
- 4. ☒ **Attachment B - Potential Sources of Contamination.** A description of any activities or processes which may be a potential source of contamination affecting surface water quality is attached.

Sequence of Construction

- 5. ☒ **Attachment C - Sequence of Major Activities.** A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is attached.
 - ☒ For each activity described, an estimate (in acres) of the total area of the site to be disturbed by each activity is given.
 - ☒ For each activity described, include a description of appropriate temporary control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
- 6. ☒ Name the receiving water(s) at or near the site which will be disturbed or which will receive discharges from disturbed areas of the project: Bear Creek

Temporary Best Management Practices (TBMPs)

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

- 7. ☒ **Attachment D – Temporary Best Management Practices and Measures.** TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information is attached:

- ☒ A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.
 - ☒ A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.
 - ☒ A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer.
 - ☒ A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.
8. ☒ The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
- ☐ **Attachment E - Request to Temporarily Seal a Feature.** A request to temporarily seal a feature is attached. The request includes justification as to why no reasonable and practicable alternative exists for each feature.
- ☒ There will be no temporary sealing of naturally-occurring sensitive features on the site.
9. ☒ **Attachment F - Structural Practices.** A description of the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site is attached. Placement of structural practices in floodplains has been avoided.
10. ☒ **Attachment G - Drainage Area Map.** A drainage area map supporting the following requirements is attached:
- ☒ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided.
 - ☐ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be used.
 - ☐ For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area.
 - ☐ There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area.

- ☐ There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. Erosion and sediment controls other than sediment basins or sediment traps within each disturbed drainage area will be used.
11. ☒ **Attachment H - Temporary Sediment Pond(s) Plans and Calculations.** Temporary sediment pond or basin construction plans and design calculations for a proposed temporary BMP or measure have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information must be signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed temporary BMPs and measures are attached.
- ☐ N/A
12. ☒ **Attachment I - Inspection and Maintenance for BMPs.** A plan for the inspection of each temporary BMP(s) and measure(s) and for their timely maintenance, repairs, and, if necessary, retrofit is attached. A description of the documentation procedures, recordkeeping practices, and inspection frequency are included in the plan and are specific to the site and/or BMP.
13. ☒ All control measures must be properly selected, installed, and maintained in accordance with the manufacturer's specifications and good engineering practices. If periodic inspections by the applicant or the executive director, or other information indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations.
14. ☒ If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain).
15. ☒ Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50%. A permanent stake will be provided that can indicate when the sediment occupies 50% of the basin volume.
16. ☒ Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from becoming a pollutant source for stormwater discharges (e.g., screening outfalls, picked up daily).

Soil Stabilization Practices

Examples: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

17. ☒ **Attachment J - Schedule of Interim and Permanent Soil Stabilization Practices.** A schedule of the interim and permanent soil stabilization practices for the site is attached.

18. ☒ Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
19. ☒ Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

Administrative Information

20. ☒ All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
21. ☒ If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
22. ☒ Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

ATTACHMENTS TO FORM 0602

ATTACHMENT A - Spill Response Actions

Possible sources of a hydrocarbon or other hazardous substance spill would be from a construction vehicle leaking fuel, lubricants, coolants, etc. Potential leakage is not likely to be significant and any soil that appears to be contaminated will be removed and disposed of in a TCEQ certified landfill. If it is determined that a reportable spill has occurred as defined in 30 TAC Chapter 327, the TCEQ shall be notified by phone at the regional office (512) 339-2929 or at the State Emergency Response Center (800) 832-8224 as soon as possible. Requirements under 30 TAC Chapter 327 will be followed to ensure that the spill is contained and disposed of in an expedient and thorough manner and that proper authorities are kept informed throughout the process.

ATTACHMENT B – Potential Sources of Contamination

Other potential sources of contamination could come from the use of asphaltic products during the construction of the site driveways and parking areas, thus tracking offsite onto area streets during wet weather periods and stockpiling solid waste. During the construction of the site, silt fencing will remain in place and collect any contaminants should they migrate due to an unforeseen rain event. The stabilized construction entrances will remain in place to minimize vehicle tracking and wheel washing will occur should the contractor wish to continue construction during wet site conditions. Solid waste will be placed in on-site containers and will be disposed of in permitted dump sites.

ATTACHMENT C – Sequence of Major Activities

The sequencing of construction will generally take place in the following manner. Most of the activities following rough grading will not further disturb the site.

1. Assign an environmental project manager who will be onsite greater than 90% of the time during construction activity and be responsible for the activities list under section 1.2.2.3 of the City of Austin environmental criteria manual. Call City Public Works Department (PWD) Environmental Inspection at 512-974-2278, 48 hours prior to beginning any work. Call the TESS at 1-800-344-8377 for utility locations and obtain permit for any work within City right-of-way.
2. Install temporary erosion controls and tree/natural area protection fencing as indicated on the approved plans prior to preconstruction meeting and any site clearing and grubbing. ($\pm$ 15.39 Ac)
3. The Environmental Project Manager must contact the Watershed Protection and Development Review Department to schedule a preconstruction coordination meeting to be held on site. The Environmental Project Manager must also notify COA Environmental Inspection at 974-2278 and Project Engineer at 439-4700 for preconstruction meeting at least 3 days prior to the meeting. The owner/developer shall notify the City Arborist (Naomi Rotramel) 3 days prior to the preconstruction meeting.
4. Erosion controls will be revised, if needed, to comply with City of Austin inspectors' directives and revised construction schedule relative to the water quality plan

requirements and the erosion/sedimentation control and tree protection plan. (Typically, only minor localized soil disturbances would be required for this activity.)

5. Rough cut water quality pond prior to rough grading site. Either the permanent outlet structure or a temporary outlet will be constructed prior to development of any embankment or excavation that leads to ponding conditions. The outlet system must consist of a low-level outlet and an emergency overflow meeting the requirements of the City of Austin's Drainage Criteria Manual (Section 8.3) and/or the City of Austin's Environmental Criteria Manual (Section 1.4.2.K) as required. The outlet system shall be protected from erosion and shall be maintained throughout the course of construction until final restoration is achieved. Water Quality Pond ($\pm$ 0.25 acres).
6. Temporary Controls to be inspected and maintained weekly and prior to anticipated rainfall events, and after rainfall events, as needed. (Typically, only minor localized soil disturbances would be required for this activity.)
7. Rough grade site. ($\pm$ 5.50 Ac)
8. Trench and install all utilities to be located under the proposed pavement. ($\pm$ 2.00 Ac)
9. Environmental Project Manager will schedule a mid-construction conference to coordinate changes in the construction schedule and evaluate effectiveness of the erosion/sedimentation control and tree protection plan after possible construction alterations to the site. Participants shall include the City Inspector, Project Engineer, General Contractor and Environmental Project Manager. The anticipated completion date and final construction sequence and inspection schedule will be coordinated with the appropriate City Inspector. A mid-construction conference is required for each phase, if phasing is proposed, with the watershed protection and development review department staff, which will be coordinated based upon completion of buildings, drainage facilities, water quality controls and temporary erosion controls by phase.
10. Complete all underground utility installations, including irrigation system. (Some additional soil disturbance is anticipated during this activity to install the irrigation system, $<$ 2.00 acres). All other trenching activities will have already been completed; thus minor localized disturbances may be required for adjustments.)
11. Re-grade site to sub-grade. ($\pm$ 5.50 Ac)
12. Ensure that all underground utility crossings are completed. This task is only a check on the already installed utility lines. (No additional soil disturbance is anticipated during this activity as trenching activities will have already been completed.)
13. Lay first course base material on parking areas and driveways. (No additional soil disturbance is anticipated during this activity because site grading activities will have already disturbed this area down to sub-grade.)
14. Install curb and gutter. (No additional soil disturbance is anticipated during this activity as the rough grading will have already disturbed this area down to sub-grade.)

15. Lay final base course on all parking areas and driveways. (No additional soil disturbance is anticipated during this activity because site grading activities will have already disturbed this area down to sub-grade.)
16. Lay asphalt. (This activity will not require additional soil disturbance.)
17. Install site signs. (< 200 SF)
18. Complete construction of permanent structural controls, including irrigation system. Permanent controls will be cleaned out and filter media will be installed prior to/concurrently with revegetation of site.
19. Start revegetation of the site and installation of any required landscaping. ($\pm$ 2.0 Ac).
20. Upon completion of the site construction and revegetation of the project site, the Design Engineer shall submit an Engineer's Letter of Concurrence to the Planning and Development Review Department indicating that construction, including revegetation, is complete and in substantial conformity with the approved plans. After receiving this letter, a final inspection will be scheduled by the appropriate City Inspector.
21. Upon completion of landscape installation of a project site, the Landscape Architect shall submit a Letter of Concurrence to the Planning and Development Review Department indicating that the required landscaping is complete and in substantial conformity with the approved plans. After receiving this letter, a final inspection will be scheduled by the appropriate City Inspector.
22. After a final inspection has been conducted by the City Inspector and with approval from the City inspector, remove the temporary erosion and sedimentation controls and complete any necessary final revegetation resulting from removal of the controls. Conduct any maintenance and rehabilitation of the water quality ponds or controls.

ATTACHMENT D – Temporary Best Management Practices and Measures

- a. The site will not receive appreciable up-gradient stormwater from offsite. Upstream runoff from the Avana Subdivision development is captured within its boundary and routed via drainage improvements to the respective water quality and detention facilities. A small amount of contributing offsite drainage from the northwest will flow along the edge of the site, but not flow into or across the limits of construction.
- b. Silt fence is used throughout the project to prevent pollution of stormwater runoff for areas that exhibit sheet flow characteristics. Before construction begins, silt fence and tree protection will be in place. The principal potential pollutant on site is sediment caused by disturbance during construction. The controls installed will be monitored on a regular basis and after any significant rainfall to ensure effective operation. Throughout construction, inspection forms will be used to record the condition of the controls after rainfall events. Rock berms and energy dissipaters are installed at discharge locations to prevent erosion and transport of sediment from the site and also decrease flow velocities.

- c. Runoff from the site (± 15.39 acres) will be captured by proposed inlets and existing inlets and conveyed by storm sewer to the proposed retention/irrigation water quality pond and existing retention/irrigation water quality pond. The inlets will be protected during construction with sediment traps. Discharge from the water quality ponds will be retained until discharged via sprinklers onto the designated irrigation area. Any runoff from the irrigation area will flow directly into Bear Creek. Larger storm events will flow over the splitter box weir into the detention pond before being released over a 24 hour period. The detention pond outlet will be protected until final restoration of the site has been achieved. The detention pond also discharges into Bear Creek and not into any identified sensitive features.
- d. As described above, there are no naturally occurring sensitive features identified by the geological assessment that will receive developed discharge from the proposed site. Setbacks for identified sensitive features were delineated and approved by the City of Austin as part of the overall Avana Subdivision preliminary plan. At that time, the TCEQ agreed with the City of Austin's determination of the required/needed setbacks. The setbacks allow undeveloped runoff to flow into these features.

ATTACHMENT E – Request to Temporarily Seal a Feature

No request is anticipated for this project.

ATTACHMENT F – Structural Practices

Contractor will construct and maintain silt fence, rock berms, a stabilized construction entrance, and other temporary and permanent erosion and sedimentation controls as appropriate to prevent pollutants from exiting the site during construction.

ATTACHMENT G – Drainage Area Map

The onsite drainage area map is included with the attached set of plans.

ATTACHMENT H – Temporary Sediment Pond(s) Plans and Calculations

The proposed retention/irrigation water quality pond and existing retention/irrigation water quality pond will act as sediment traps should any of the other erosion controls fail. Since ± 13.44 acres will drain to the water quality ponds, using 3,600 cf per acre of drainage, 48,384 cf of volume is required to be provided during construction. The water quality ponds will provide a total volume of 75,666 cf, which is sufficient to provide temporary controls. The sequence of construction indicates that the proposed water quality pond shall be rough cut to provide full volume prior to site grading.

ATTACHMENT I – Inspection and Maintenance for BMPs.

Inspection and maintenance for Best Management Practices is taken from the TCEQ Manual, "Complying with the Edwards Aquifer Rules: Technical Guidance on Best Management Practices", dated July 2005.

Silt Fence:

1. Inspect all fencing weekly, and after any rainfall.
2. Remove sediment when buildup reaches 6 inches.
3. Replace any torn fabric or install a second line of fencing parallel to the torn section.
4. Replace or repair any section crushed or collapsed in the course of construction activity. If a section of fence is obstructing vehicular access, consider relocating it to a spot where it will provide equal protection, but will not obstruct vehicles. A triangular filter dike may be preferable to a silt fence at common vehicular access points.
5. When construction is complete, the sediment should be disposed of in a manner that will not cause additional siltation and the prior location of the silt fence should be revegetated. The fence itself should be disposed of in a approved landfill.

Rock Berms:

1. Inspection should be made weekly and after each rainfall by the responsible party. For installations in streambeds, additional daily inspections should be made.
2. Remove sediment and other debris when buildup reaches 6 inches and dispose of the accumulated silt in an approved manner that will not cause any additional siltation.
3. Repair any loose wire sheathing.
4. The berm should be reshaped as needed during inspection.
5. The berm should be replaced when the structure ceases to function as intended due to silt accumulation among the rocks, washout, construction traffic damage, etc.
6. The rock berm should be left in place until all upstream areas are stabilized and accumulated silt is removed.

Stabilized Construction Entrance:

1. The entrance should be maintained in a condition, which will prevent tracking or flowing of sediment onto public rights-of-way. This may require periodic top dressing with additional stone as conditions demand and repair and/or cleanout of any measures used to trap sediment.
2. All sediment spilled, dropped, washed or tracked onto public rights-of-way should be removed immediately by contractor.
3. When necessary, wheels should be cleaned to remove sediment prior to entrance onto public right-of-way.
4. When washing is required, it should be done on an area stabilized with crushed stone that drains into an approved sediment trap or sediment basin.

5. All sediment should be prevented from entering any storm drain, ditch or water course by using approved methods.

Concrete Washout Area:

1. Routine inspection in accordance with section 1.4.18 of TCEQ Manual: RG-348 of the area to ensure that sufficient quantity and volume remain to contain all liquid and concrete waste generated by washout operations.
2. Locate washout area at least 50 feet from sensitive features, storm drains, open ditches, or water bodies. Do not allow runoff from this area by constructing a temporary pit or bermed area large enough for liquid and solid waste.
3. Plastic lining material should be a minimum of 10 mil in polyethylene sheeting and should be free of holes, tears, or other defects that compromise the impermeability of the material.
4. When temporary concrete washout facilities are no longer required for the work, the hardened concrete should be removed and disposed of. Materials used to construct temporary concrete washout facilities should be removed from the site of the work and disposed of. Holes, depressions, or other ground disturbance caused by the removal of the temporary concrete washout facilities should be backfilled and repaired.

ATTACHMENT J – Schedule of Interim and Permanent Soil Stabilization Practices

The following are the proposed stabilization (temporary and permanent) practices:

Temporary Vegetative Stabilization:

1. From September 15 to March 1, seeding shall be with cool season cover crops (Wheat at 0.5 pounds per 1000 SF, Oats at 0.5 pounds per 1000 SF, Cereal Rye Grain at 0.5 pounds per 1000 SF) with a total rate of 1.5 pounds per 1000 SF. Cool season cover crops are not permanent erosion control.
2. From March 2 through September 14, seeding shall be with hulled Bermuda at a rate of 1 pound per 1000 SF.
 - A. Fertilizer shall be water soluble with an analysis of 15-15-15 to be applied once at planting and once during the period of establishment at a rate of 1/2 pound per 1000 SF.
 - B. Hydro-mulch shall comply with Table 1, below.

Material	Description	Longevity	Typical Applications	Application Rates
100 % or any blend of wood, cellulose, straw, and/or cotton plant material (except no mulch shall exceed 30% paper)	70% or greater Wood/Straw 30% or less Paper or Natural Fibers	0-3 months	Moderate slopes; from flat to 3:1	1500 to 2000 lbs per acre

- C. Temporary erosion control shall be acceptable when the grass has grown at least 1-1/2 inches high with 95% coverage, provided no bare spots larger than 16 square feet exist.
 - D. When required, native grass seeding shall comply with requirements of the City of Austin Environmental Criteria Manual.

Permanent Vegetative Stabilization:

1. From September 15 to March 1, seeding is considered to be temporary stabilization only. If cool season cover crops exist where permanent vegetative stabilization is desired, the grasses shall be mowed to a height of less than one-half (1/2) inch and the area shall be re-seeded in accordance with 2, below.
2. From March 2 to September 14, seeding shall be with hulled Bermuda at a rate of 1 pound per 1,000 square feet with a purity of 95% with 85% germination. Bermuda grass is a warm season grass and is considered permanent erosion control.
 - A. Fertilizer shall be water soluble with an analysis of 15-15-15 to be applied once at planting and once during the period of establishment at a rate of 1/2 pound per 1000 SF.

B. Hydro-mulch shall comply with Table 2, below.

Material	Description	Longevity	Typical Applications	Application Rates
Bonded Fiber Matrix (BFM)	80% Organic defibrated fibers 10% Tackifier	6 months	On slopes up to 2:1 and erosive soil conditions	2500 to 4000 lbs per acre (see manufacturers recommendations)
Fiber Reinforced Matrix (FRM)	65% Organic defibrated fibers 25% Reinforcing Fibers or less 10% Tackifier	Up to 12 months	On slopes up to 1:1 and erosive soil conditions	3000 to 4500 lbs per acre (see manufacturers recommendations)

- C. The planted area shall be irrigated or sprinkled in a manner that will not erode the topsoil, but will sufficiently soak the soil to a depth of six inches. The irrigation shall occur at daily intervals (minimum) during the first two months. Rainfall occurrences of 1/2 inch or more shall postpone the watering schedule for one week.
- D. Permanent erosion control shall be acceptable when the grass has grown at least 1-1/2 inches high with 95% coverage, provided no bare spots larger than 16 square feet exist.
- E. When required, native grass seeding shall comply with requirements of the City of Austin Environmental Criteria Manual.

TPDES Construction Inspection and Maintenance Report Form

Project Name: Veritas Academy Gym Addition and Campus Improvements

Permit Number: _____

Facility Operators: _____

Inspector's Name: _____
 (attach qualifications summary for each inspector)

Date of Last Rainfall: _____

Amount of Last Rainfall: _____

Date of Inspection: _____

Inspection Notes

Condition Code*	Area Inspected	Changes Required (if any)
☐	Stabilized Construction Entrance(s)	_____ _____ _____
☐	Silt fencing and rock berms downstream of improvements	_____ _____ _____
☐	Silt fencing downstream of Temporary Spoils/ Construction Staging Areas	_____ _____ _____
☐	Areas temporarily and/or finally stabilized (inspect at least once every month)	_____ _____ _____
☐	Concrete Washout Area	_____ _____ _____
☐		_____ _____ _____
☐		_____ _____ _____
☐		_____ _____ _____

***Condition Codes**

- 01 - In compliance with the storm water pollution prevention plan and perm
- 02 - To be repaired or replaced within 24 hours
- 03 - To be repaired or replaced within 48 hours
- 04 - To be repaired or replaced within 7 days

Please note major construction activities taking place. Include dates when major grading activities and/or disturbances occur, dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated. Major observations should include: The locations of discharges of sediment or other pollutants from the site; locations of controls that need to be maintained; locations of controls that failed to operate as designed or proved inadequate for a particular location; and locations where additional controls are needed. (Attach additional pages as required and/or attach daily construction reports.)

I certify under penalty of law that this document was prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Signature: _____ Date: _____

Signature: _____ Date: _____

**Change in Responsibility for Maintenance
on Permanent Best Management Practices and Measures**

The applicant is no longer responsible for maintaining the permanent best management practice (BMP) and other measures. The project information and the new entity responsible for maintenance is listed below.

Customer: _____

Regulated Entity Name: _____

Site Address: _____

City, Texas, Zip: _____

County: _____

Approval Letter Date: _____

BMPs for the project: _____

New Responsible Party: _____

Name of contact: _____

Mailing Address: _____

City, State: _____ Zip: _____

Telephone: _____ FAX: _____

Signature of New Responsible Party Date

I acknowledge and understand that I am assuming full responsibility for maintaining all permanent best management practices and measures approved by the TCEQ for the site, until another entity assumes such obligations in writing or ownership is transferred.

If you have questions on how to fill out this form or about the Edwards Aquifer protection program, please contact us at 210/490-3096 for projects located in the San Antonio Region or 512/339-2929 for projects located in the Austin Region.

Individuals are entitled to request and review their personal information that the agency gathers on its forms. They may also have any errors in their information corrected. To review such information, contact us at 512/239-3282.

Agent Authorization Form
For Required Signature
Edwards Aquifer Protection Program
Relating to 30 TAC Chapter 213
Effective June 1, 1999

I Scott Rhodes,
Print Name
Chief Business Officer,
Title - Owner/President/Other
of Veritas Academy,
Corporation/Partnership/Entity Name
have authorized Reese Hurley, PE,
Print Name of Agent/Engineer
of LJA Engineering, Inc.,
Print Name of Firm

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:

SA Rhodes
Applicant's Signature

4/14/2026
Date

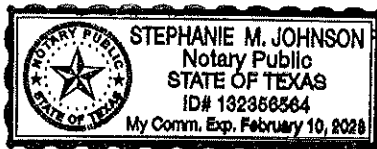
THE STATE OF TEXAS §

County of HAYS §

BEFORE ME, the undersigned authority, on this day personally appeared Scott Rhodes known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that (s)he executed same for the purpose and consideration therein expressed.

GIVEN under my hand and seal of office on this 14 day of April, 2026.

Stephanie M. Johnson
NOTARY PUBLIC



Stephanie M. Johnson
Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 02/10/2028

Application Fee Form

Texas Commission on Environmental Quality

Name of Proposed Regulated Entity: Veritas Academy

Regulated Entity Location: 13401 Escarpment Blvd., Austin, TX 78739

Name of Customer: Veritas Academy

Contact Person: Cameron Cook

Phone: 512-891-1674

Customer Reference Number (if issued): CN 604817940

Regulated Entity Reference Number (if issued): RN 108353129

Austin Regional Office (3373)

☒ Hays

☐ Travis

☐ Williamson

San Antonio Regional Office (3362)

☐ Bexar

☐ Medina

☐ Uvalde

☐ Comal

☐ Kinney

Application fees must be paid by check, certified check, or money order, payable to the **Texas Commission on Environmental Quality**. Your canceled check will serve as your receipt. **This form must be submitted with your fee payment.** This payment is being submitted to:

☒ Austin Regional Office

☐ San Antonio Regional Office

☐ Mailed to: TCEQ - Cashier

☐ Overnight Delivery to: TCEQ - Cashier

Revenues Section

Mail Code 214

P.O. Box 13088

Austin, TX 78711-3088

12100 Park 35 Circle

Building A, 3rd Floor

Austin, TX 78753

(512)239-0357

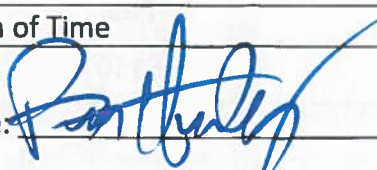
Site Location (Check All That Apply):

☒ Recharge Zone

☐ Contributing Zone

☐ Transition Zone

Type of Plan	Size	Fee Due
Water Pollution Abatement Plan, Contributing Zone Plan: One Single Family Residential Dwelling	Acres	\$
Water Pollution Abatement Plan, Contributing Zone Plan: Multiple Single Family Residential and Parks	Acres	\$
Water Pollution Abatement Plan, Contributing Zone Plan: Non-residential	Acres	\$
Sewage Collection System	520 L.F.	\$ 650
Lift Stations without sewer lines	Acres	\$
Underground or Aboveground Storage Tank Facility	Tanks	\$
Piping System(s)(only)	Each	\$
Exception	Each	\$
Extension of Time	Each	\$

Signature: 

Date: 04/16/2026

Application Fee Schedule

Texas Commission on Environmental Quality

Edwards Aquifer Protection Program 30 TAC Chapter 213 (effective 05/01/2008)

Water Pollution Abatement Plans and Modifications

Contributing Zone Plans and Modifications

Project	Project Area in Acres	Fee
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5	\$1,500
	5 < 10	\$3,000
	10 < 40	\$4,000
	40 < 100	\$6,500
	100 < 500	\$8,000
	≥ 500	\$10,000
Non-residential (Commercial, industrial, institutional, multi-family residential, schools, and other sites where regulated activities will occur)	< 1	\$3,000
	1 < 5	\$4,000
	5 < 10	\$5,000
	10 < 40	\$6,500
	40 < 100	\$8,000
	≥ 100	\$10,000

Organized Sewage Collection Systems and Modifications

Project	Cost per Linear Foot	Minimum Fee- Maximum Fee
Sewage Collection Systems	\$0.50	\$650 - \$6,500

Underground and Aboveground Storage Tank System Facility Plans and Modifications

Project	Cost per Tank or Piping System	Minimum Fee- Maximum Fee
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500

Exception Requests

Project	Fee
Exception Request	\$500

Extension of Time Requests

Project	Fee
Extension of Time Request	\$150



TCEQ Core Data Form

For detailed instructions on completing this form, please read the Core Data Form Instructions or call 512-239-5175.

SECTION I: General Information

1. Reason for Submission (If other is checked please describe in space provided.)		
☐ New Permit, Registration or Authorization (Core Data Form should be submitted with the program application.)		
☐ Renewal (Core Data Form should be submitted with the renewal form)		☒ Other WPAP & SCS Modification
2. Customer Reference Number (if issued)	Follow this link to search for CN or RN numbers in Central Registry**	3. Regulated Entity Reference Number (if issued)
CN 604817940		RN 108353129

SECTION II: Customer Information

4. General Customer Information		5. Effective Date for Customer Information Updates (mm/dd/yyyy)		
☐ New Customer ☒ Update to Customer Information ☐ Change in Regulated Entity Ownership				
☐ Change in Legal Name (Verifiable with the Texas Secretary of State or Texas Comptroller of Public Accounts)				
<i>The Customer Name submitted here may be updated automatically based on what is current and active with the Texas Secretary of State (SOS) or Texas Comptroller of Public Accounts (CPA).</i>				
6. Customer Legal Name (If an individual, print last name first: eg: Doe, John)			<i>If new Customer, enter previous Customer below:</i>	
Veritas Academy				
7. TX SOS/CPA Filing Number	8. TX State Tax ID (11 digits)	9. Federal Tax ID (9 digits)	10. DUNS Number (if applicable)	
08003383672	12016327459	20-1632745	606124035	
11. Type of Customer:	☐ Corporation	☐ Individual	Partnership: ☐ General ☐ Limited	
Government: ☐ City ☐ County ☐ Federal ☐ Local ☐ State ☐ Other		☐ Sole Proprietorship	☒ Other: Private School	
12. Number of Employees		13. Independently Owned and Operated?		
☐ 0-20 ☐ 21-100 ☒ 101-250 ☐ 251-500 ☐ 501 and higher		☒ Yes ☐ No		
14. Customer Role (Proposed or Actual) – as it relates to the Regulated Entity listed on this form. Please check one of the following				
☐ Owner ☐ Operator ☒ Owner & Operator ☐ Other:				
☐ Occupational Licensee ☐ Responsible Party ☐ VCP/BSA Applicant				
15. Mailing Address:	13401 Escarpment Blvd			
	City	Austin	State	TX
		ZIP	78739	ZIP + 4
16. Country Mailing Information (if outside USA)			17. E-Mail Address (if applicable)	
			cameron.cook@veritasacademy.net	

18. Telephone Number (512) 891-1674	19. Extension or Code	20. Fax Number (if applicable) (512) 891-1693
---	------------------------------	---

SECTION III: Regulated Entity Information

21. General Regulated Entity Information (If 'New Regulated Entity' is selected, a new permit application is also required.) ☐ New Regulated Entity ☐ Update to Regulated Entity Name ☐ Update to Regulated Entity Information							
<i>The Regulated Entity Name submitted may be updated, in order to meet TCEQ Core Data Standards (removal of organizational endings such as Inc, LP, or LLC).</i>							
22. Regulated Entity Name (Enter name of the site where the regulated action is taking place.) Veritas Academy							
23. Street Address of the Regulated Entity: (No PO Boxes)	13401 Escarpment						
	City	Austin	State	TX	ZIP	78739	ZIP + 4
24. County	HAYS						

If no Street Address is provided, fields 25-28 are required.

25. Description to Physical Location:							
26. Nearest City				State		Nearest ZIP Code	
<i>Latitude/Longitude are required and may be added/updated to meet TCEQ Core Data Standards. (Geocoding of the Physical Address may be used to supply coordinates where none have been provided or to gain accuracy).</i>							
27. Latitude (N) In Decimal:			28. Longitude (W) In Decimal:				
Degrees	Minutes	Seconds	Degrees	Minutes	Seconds		
29. Primary SIC Code (4 digits)		30. Secondary SIC Code (4 digits)		31. Primary NAICS Code (5 or 6 digits)		32. Secondary NAICS Code (5 or 6 digits)	
8211				611110			
33. What is the Primary Business of this entity? (Do not repeat the SIC or NAICS description.) Private Educational Facility							
34. Mailing Address:	13401 Escarpment Blvd						
	City	Austin	State	TX	ZIP	78739	ZIP + 4
35. E-Mail Address:							
36. Telephone Number		37. Extension or Code		38. Fax Number (if applicable)			
(512) 891-1673				(512) 891-1693			

39. TCEQ Programs and ID Numbers Check all Programs and write in the permits/registration numbers that will be affected by the updates submitted on this form. See the Core Data Form instructions for additional guidance.

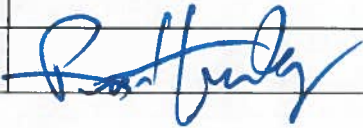
☐ Dam Safety	☐ Districts	☒ Edwards Aquifer	☐ Emissions Inventory Air	☐ Industrial Hazardous Waste
☐ Municipal Solid Waste	☐ New Source Review Air	☐ OSSF	☐ Petroleum Storage Tank	☐ PWS
☐ Sludge	☐ Storm Water	☐ Title V Air	☐ Tires	☐ Used Oil
☐ Voluntary Cleanup	☐ Wastewater	☐ Wastewater Agriculture	☐ Water Rights	☐ Other:

SECTION IV: Preparer Information

40. Name:	Reese Hurley, PE		41. Title:	Senior Project Manager
42. Telephone Number	43. Ext./Code	44. Fax Number	45. E-Mail Address	
(512) 439-4700		(512) 439-4716	rhurley@lja.com	

SECTION V: Authorized Signature

46. By my signature below, I certify, to the best of my knowledge, that the information provided in this form is true and complete, and that I have signature authority to submit this form on behalf of the entity specified in Section II, Field 6 and/or as required for the updates to the ID numbers identified in field 39.

Company:	LJA Engineering, Inc.	Job Title:	Senior Project Manager	
Name (In Print):	Reese Hurley, PE	Phone:	(512) 439- 4700	
Signature:			Date:	4/10/2026

TSS Removal Calculations for Vegetative Filter Strip 3-25-2024

Project Name: **Veritas Academy**Date Prepared: **4/14/2026**

Additional information is provided for cells with a red triangle in the upper right corner. Place the cursor over the cell.

Text shown in blue indicate location of instructions in the Technical Guidance Manual - RG-348.

Characters shown in red are data entry fields.

Characters shown in black (Bold) are calculated fields. Changes to these fields will remove the equations used in the spreadsheet.

1. The Required Load Reduction for the total project:

Calculations from RG-348

Pages 3-27 to 3-30

Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$

where:

 L_M TOTAL PROJECT = Required TSS removal resulting from the proposed development = 80% of increased load A_N = Net increase in impervious area for the project P = Average annual precipitation, inches

Site Data: Determine Required Load Removal Based on the Entire Project

County =	Hays	
Total project area included in plan =	96.60	acres
Predevelopment impervious area within the limits of the plan =	0.00	acres
Total post-development impervious area within the limits of the plan =	6.23	acres
Total post-development impervious cover fraction =	0.06	
P =	33	inches
L_M TOTAL PROJECT =	5592	lbs.
Number of drainage basins / outfalls areas leaving the plan area =	3	

2. Drainage Basin Parameters (This information should be provided for each basin):

Drainage Basin/Outfall Area No. =	D4	
Total drainage basin/outfall area =	1.22	acres
Predevelopment impervious area within drainage basin/outfall area =	0.00	acres
Post-development impervious area within drainage basin/outfall area =	0.45	acres
Post-development impervious fraction within drainage basin/outfall area =	0.37	
L_M THIS BASIN =	400	lbs.

3. Indicate the proposed BMP Code for this basin.Proposed BMP = **Vegetated Filter Strips****4. Calculate Maximum TSS Load Removed (L_R) for this Drainage Basin by the selected BMP Type.**RG-348 Page 3-33 Equation 3.7: $L_R = (\text{BMP efficiency}) \times P \times (A_i \times 34.6 + A_p \times 0.54)$

where:

 A_C = Total On-Site drainage area in the BMP catchment area A_i = Impervious area proposed in the BMP catchment area A_p = Pervious area remaining in the BMP catchment area L_R = TSS Load removed from this catchment area by the proposed BMP A_C = **1.22** acres A_i = **0.45** acres A_p = **0.77** acres L_R = **445** lbs**5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area**Desired L_M THIS BASIN = **400** lbs. F = **0.90****5. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.**

Calculations from RG-348

Pages 3-34 to 3-36

Rainfall Depth =	1.70	inches
Post Development Runoff Coefficient =	0.29	
On-site Water Quality Volume =	2186	cubic feet
Off-site area draining to BMP =	0.00	acres
Off-site Impervious cover draining to BMP =	0.00	acres
Impervious fraction of off-site area =	0	
Off-site Runoff Coefficient =	0.00	
Off-site Water Quality Volume =	0	cubic feet
Storage for Sediment =	437	
Total Capture Volume (required water quality volume(s) x 1.20) =	2624	cubic feet

Designed as Required in RG-348

Pages 3-55 to 3-57

16. Vegetated Filter Strips

There are no calculations required for determining the load or size of vegetative filter strips.

The 80% removal is provided when the contributing drainage area does not exceed 72 feet (direction of flow) and the sheet flow leaving the impervious cover is directed across 15 feet of engineered filter strips with maximum slope of 20% or across 50 feet of natural vegetation with a maximum slope of 10%. There can be a break in grade as long as no slope exceeds 20%. If vegetative filter strips are proposed for an interim permanent BMP, they may be sized as described on Page 3-56 of RG-348.



Additional Information is provided for cells with a red triangle in the upper right corner. Place the cursor over the cell.
Text shown in blue indicate location of instructions in the Technical Guidance Manual - RG-348.
Characters shown in red are data entry fields.
Characters shown in black (Bold) are calculated fields. Changes to these fields will remove the equations used in the spreadsheet.

1. The Required Load Reduction for the total project:

Calculations from RG-348

Pages 3-27 to 3-30

where:

 $L_{\text{M TOTAL PROJECT}} = \text{Required TSS removal resulting from the proposed development} = 80\% \text{ of increased load}$ $A_{\text{N}} = \text{Net Increase in Impervious area for the project}$ $P = \text{Average annual precipitation, inches}$

Site Data: Determine Required Load Removal Based on the Entire Project

County = **Hays**Total project area included in plan = **96.00** acresPredominant impervious area within the limits of the plan = **0.00** acresTotal post-development impervious area within the limits of the plan = **6.23** acresTotal post-development impervious cover fraction = **0.06** $P =$ **33** inches $L_{\text{M TOTAL PROJECT}} =$ **5592** lbs.Number of drainage basins / outfalls areas leaving the plan area = **3**

The RI pond will provide compensatory water quality for Area D2 (See DM02)

2. Drainage Basin Parameters (This information should be provided for each basin):Drainage Basin/Outfall Area No. = **D2**Total drainage basin/outfall area = **6.96** acresPredominant impervious area within drainage basin/outfall area = **0.00** acresPost-development impervious area within drainage basin/outfall area = **3.23** acresPost-development impervious fraction within drainage basin/outfall area = **0.36** $L_{\text{M THIS BASIN}} =$ **2897** lbs.**3. Indicate the proposed BMP Code for this basin.**Proposed BMP = **Retention / Irrigation****4. Calculate Maximum TSS Load Removed (L_{R}) for this Drainage Basin by the selected BMP Type.**RG-348 Page 3-33 Equation 3.7 $L_{\text{R}} = (\text{BMP efficiency}) \times P \times (A_{\text{I}} \times 34.6 + A_{\text{P}} \times 0.54)$ $A_{\text{C}} = \text{Total On-Site drainage area in the BMP catchment area}$ $A_{\text{I}} = \text{Impervious area proposed in the BMP catchment area}$ $A_{\text{P}} = \text{Pervious area remaining in the BMP catchment area}$ $L_{\text{R}} = \text{TSS Load removed from this catchment area by the proposed BMP}$ $A_{\text{C}} =$ **6.96** acres $A_{\text{I}} =$ **3.23** acres $A_{\text{P}} =$ **5.75** acres $L_{\text{R}} =$ **3787** lbs. $L_{\text{M THIS BASIN}} =$ **3139** lbs. $F =$ **0.83**Rainfall Depth = **1.20** inchesPost Development Runoff Coefficient = **0.29**On-site Water Quality Volume = **11243** cubic feet

Calculations from RG-348

Pages 3-36 to 3-37

Off-site area draining to BMP = **0.00** acresOff-site impervious cover draining to BMP = **0.00** acresImpervious fraction of off-site area = **0**Off-site Runoff Coefficient = **0.00**Off-site Water Quality Volume = **0** cubic feetStorage for Sediment = **2249** cubic feetTotal Capture Volume (required water quality volume(s) x 1.20) = **13492** cubic feet

The following sections are used to calculate the required water quality volume(s) for the selected BMP.

7. Retention/Irrigation SystemRequired Water Quality Volume for retention basin = **13492** cubic feetProvided Water Quality Volume for retention basin = **57494** cubic feet

Irrigation Area Calculations:

Soil Infiltration/permeability rate = **0.1** in/hrIrrigation area = **53967** square feetProvided Irrigation area = **1.24** acresSoil Infiltration/permeability rate = **0.1** in/hrIrrigation area = **53967** square feetProvided Irrigation area = **1.24** acresSoil Infiltration/permeability rate = **0.1** in/hrIrrigation area = **53967** square feetProvided Irrigation area = **1.24** acresSoil Infiltration/permeability rate = **0.1** in/hrIrrigation area = **53967** square feetProvided Irrigation area = **1.24** acresSoil Infiltration/permeability rate = **0.1** in/hrIrrigation area = **53967** square feetProvided Irrigation area = **1.24** acresSoil Infiltration/permeability rate = **0.1** in/hrIrrigation area = **53967** square feetProvided Irrigation area = **1.24** acresSoil Infiltration/permeability rate = **0.1** in/hrIrrigation area = **53967** square feetProvided Irrigation area = **1.24** acres

