

Total Maximum **Daily Load** for Dioxin in the Houston Ship Channel

September 19, 2006

Focus

qRMA2-WASP modeling
updateqLoad allocation
spreadsheet model

RMA2-WASP modeling update

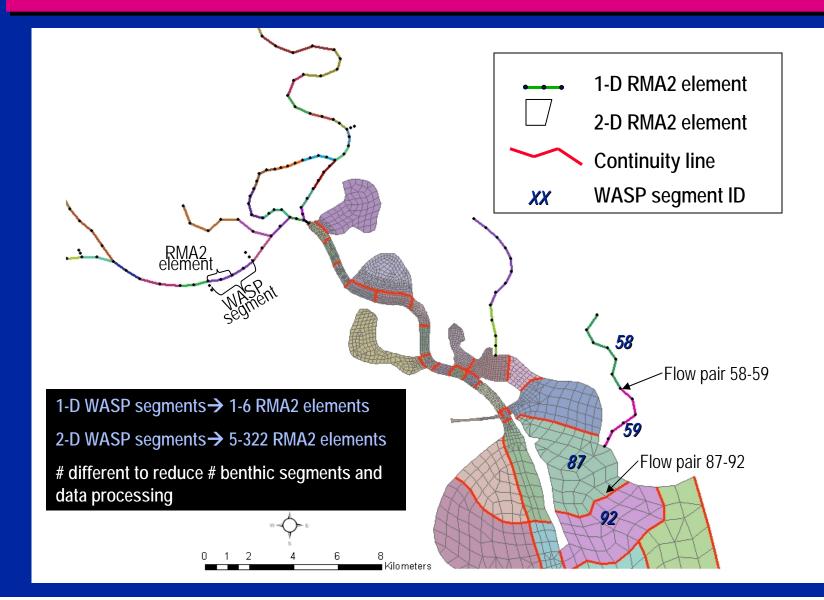
It all started with Larry's comments:

"What got me on this was the "average flow" of 42.8 cms out of Burnett Bay - that's a LOT of water, there is no river into the bay..."

and

"[You] could crosscheck volume calculations by comparing different calculations."

RMA2-WASP segmentation



RMA2-WASP modeling process

RMA2 Geometry File Node coordinates, bottom elevations, nodes comprising each element

RMA2 Output

Depth, Vx, Vy, flow for 1-D nodes Depth, Vx,Vy for 2-D nodes Flows across "continuity lines"

Junction File Flow pairs (1-D and 2-D elements) RMA2 elements composing each WASP segment

<u>WASP</u>

Uses volume data to calculate concentrations and flows to move contaminant between segments. Velocities used for re-aeration rates (not applicable to dioxin modeling)

Interface (HSCREAD)

Calculates average depth for each WASP segment on a time-step basis

Calculates volume for each WASP element for each time step by aggregating volumes of RMA elements (1-D: average cross-section*length, 2-D: surface area (also calculated by interface)*depth)

Calculates average velocities at each segment as the resultant of all the velocity vectors

Calculates flows in and out of a 1-D WASP segment as cross-section area*velocity

Reads flows across a continuity line from RMA output and assign them to the WASP 2-D pairs

Creates a hydrodynamic file in a format that can be read by WASP

Hydrodynamic model issues

$_{\rm q}$ $\,$ Net flow out of side bays too high

- Mass-balance not preserved for individual RMA2 elements (model checks balance for the entire system only). RMA2 calculates water surface elevations on a node-basis using continuity equation and velocities using conservation of momentum
- gModel output flows for continuity lines with only 2nodes inaccurate
- gWetting/drying of upstream reaches caused model to
crash (long-term run)
- ^q There were water losses between some 1-D elements

Mass-balance issue

For a given WASP element it should be true that $V_{t+1} = V_t + (Q_{in_t} - Q_{out_t}) \cdot dt$

and

$$V_{t+1} = \overline{A}_{x \sec t+1} \cdot length$$

The RMA2 Manual recommends that the difference between change in storage and change in volume not be greater than 3% for a given time step, otherwise there could be problems in the water quality simulation

How were the issues addressed?

- q1-D element geometry at some junctions wasmodified to eliminate water leaks
- ^q Upstream reaches with bottom elevations
 higher than -0.5 m above sea level were
 eliminated
- g2-D grid was refined to minimize mass-
balance problems
- Continuity lines were specified so that at least two RMA2 elements were on each side of the interface (5 nodes)

RMA2 mesh refinement

 The RMA2 model segmentation was refined so that for each of the WASP segments, the difference in volumes calculated using the two different methods was not greater than 3% of the volume at any time step.

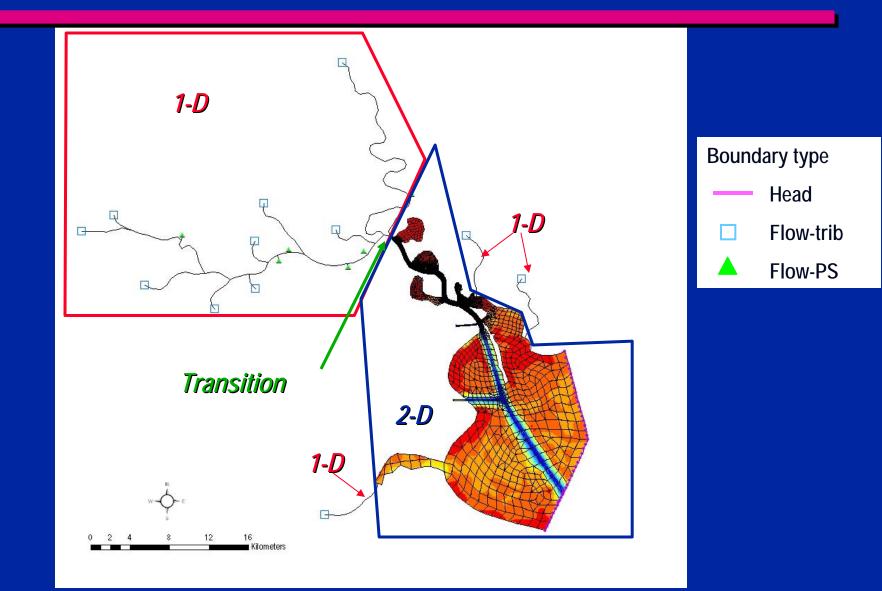
^q Iterative process.

^q There are still a few WASP segments that have errors greater than the criterion for a number of time steps.

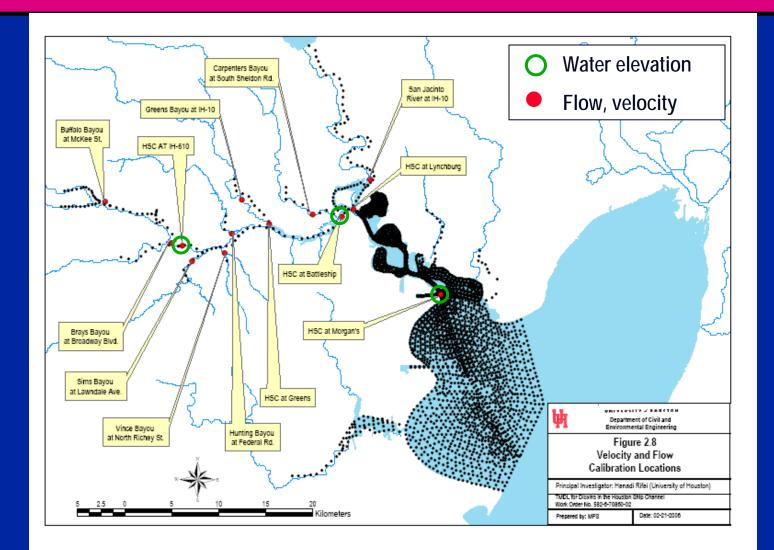
RMA2 model for the HSC and UGB

- 108 linear elements (including major tributaries), 3228 2-D elements, 16 junction elements, 4 transition elements
- $_{\rm q}$ Calibration period: March 20 to April 21, 2005
- ^q Time step: 6 minutes
- ^q Spin-up time: 48 hours (480 time steps)
- **g** Boundaries
 - g *d/s: tide data from NOAA*
 - *u/s freshwater inflows from major tributaries (USGS hourly data) plus point source self-reported flows (includes runoff)*
 - ^q inflows from PS discharging to the channel (@ five locations)
- g Meteorology: wind, rainfall, and evaporation data

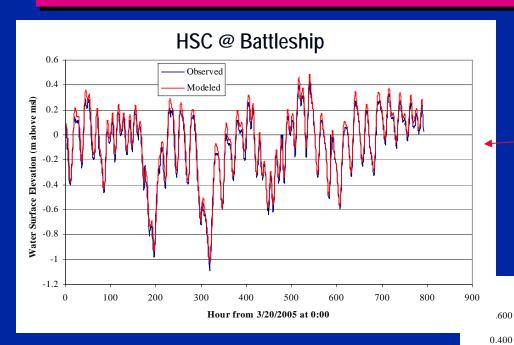
RMA2 current model segmentation



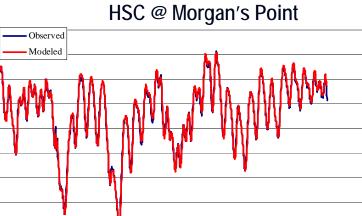
RMA2 model – calibration locations



RMA2 model – WSE calibration







300

Water Surface Elevation (m above msl)

0.200 0.000 -0.200 -0.400 -0.600 -0.800 -1.000 -1.2000

100

200

Hour from 3/20/2005 at 0:00

400

500

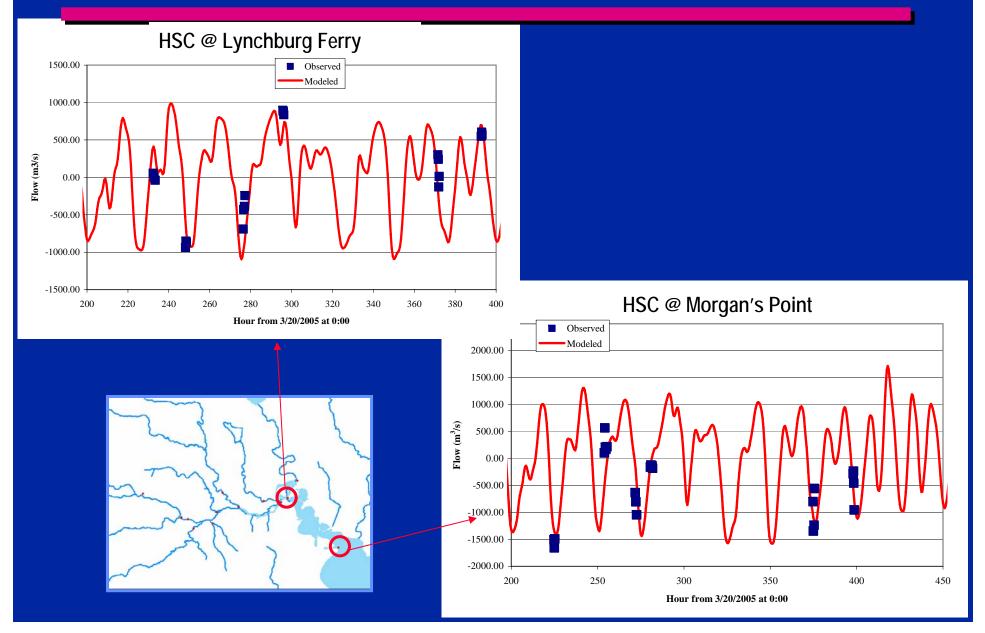
600

700

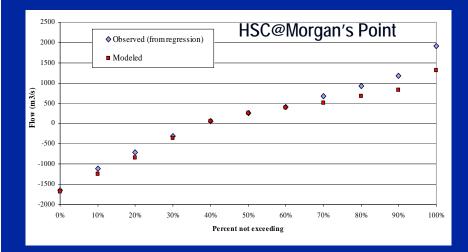
800

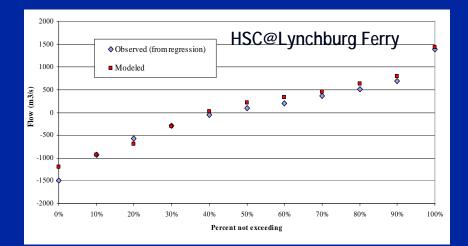
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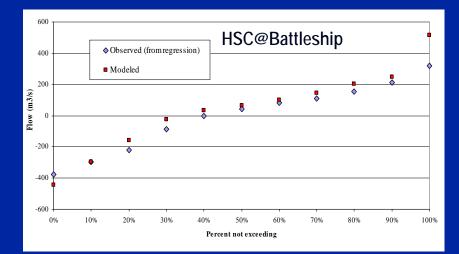
RMA2 model – Flow calibration

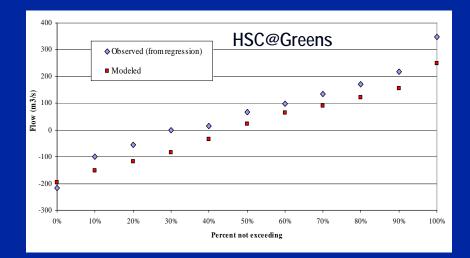


RMA2 model – goodness-of-fit for flow





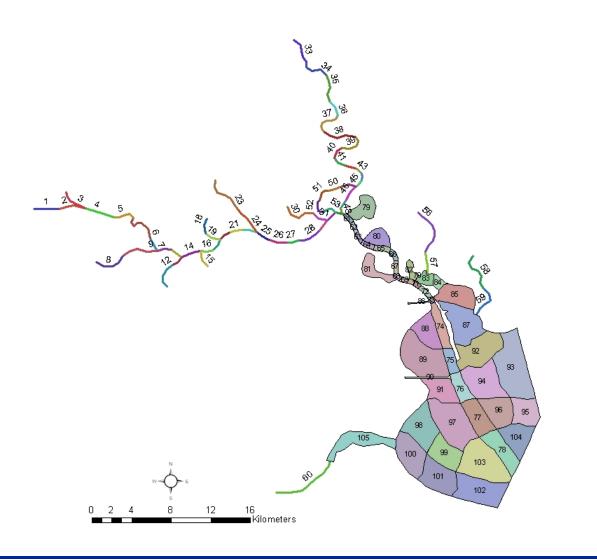




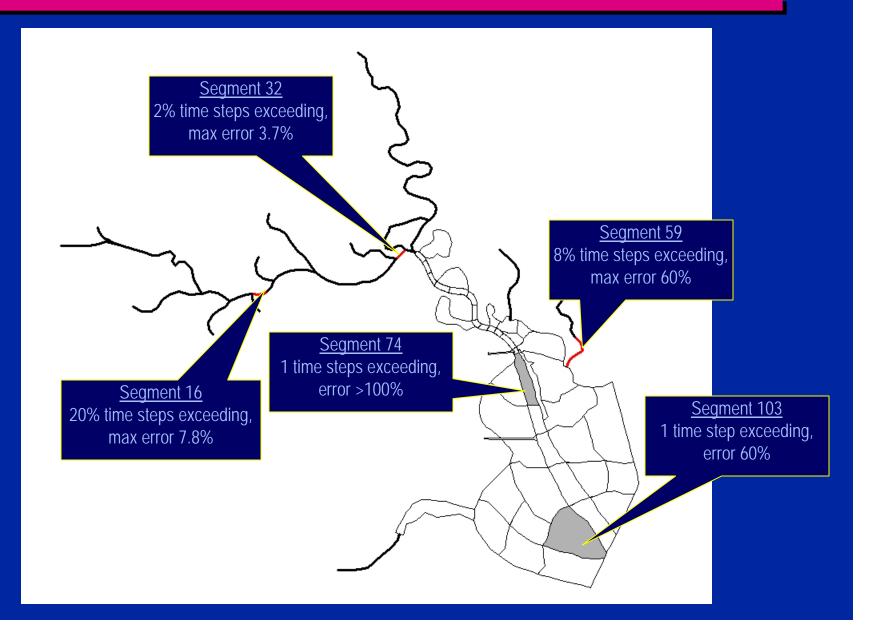
WASP salinity model for HSC and UGB

- g 60 1-D water surface elements, 45 2-D water surface elements, 105 benthic elements
- $_{\rm q}$ Calibration period for salinity: March 20 to April 21, 2005
- ^q Time step: 6 minutes
- **g** Boundaries
 - g *d/s: salinity data for Eagle Point (TCOON)*
 - qu/s: average salinity concentration for major tributaries fromhistorical TRACS data

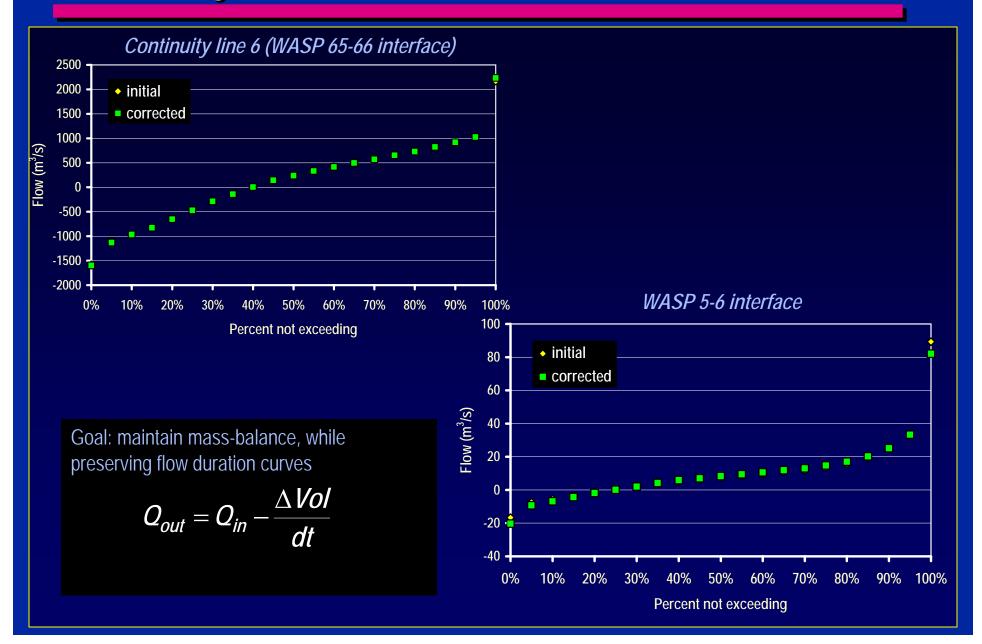
WASP current model segmentation



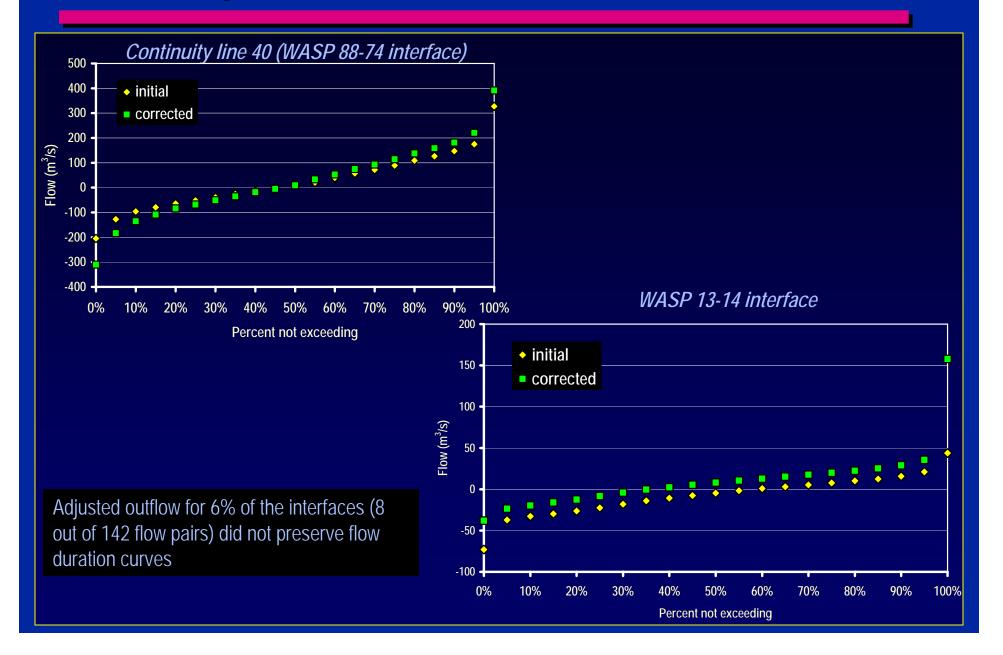
WASP elements with remaining issues



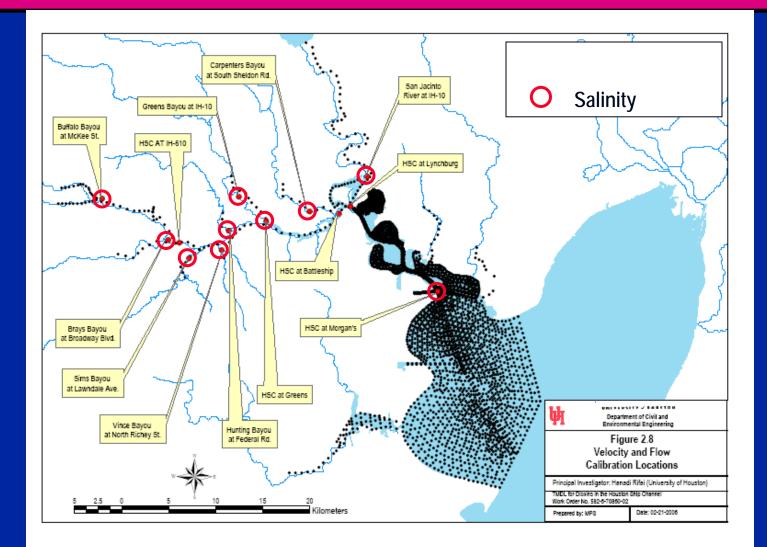
Flow adjustment



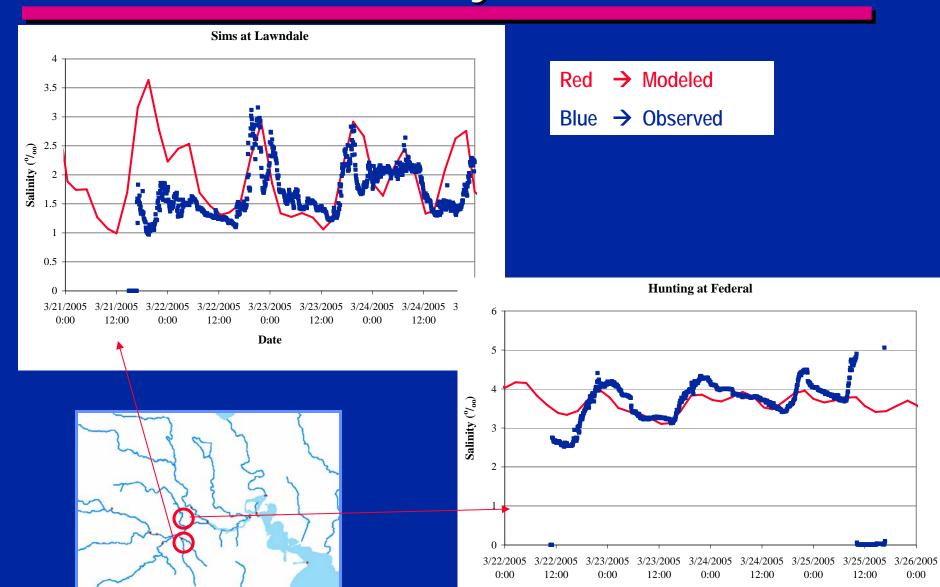
Flow adjustment – cont'd



Salinity model – calibration locations



WASP model – salinity calibration



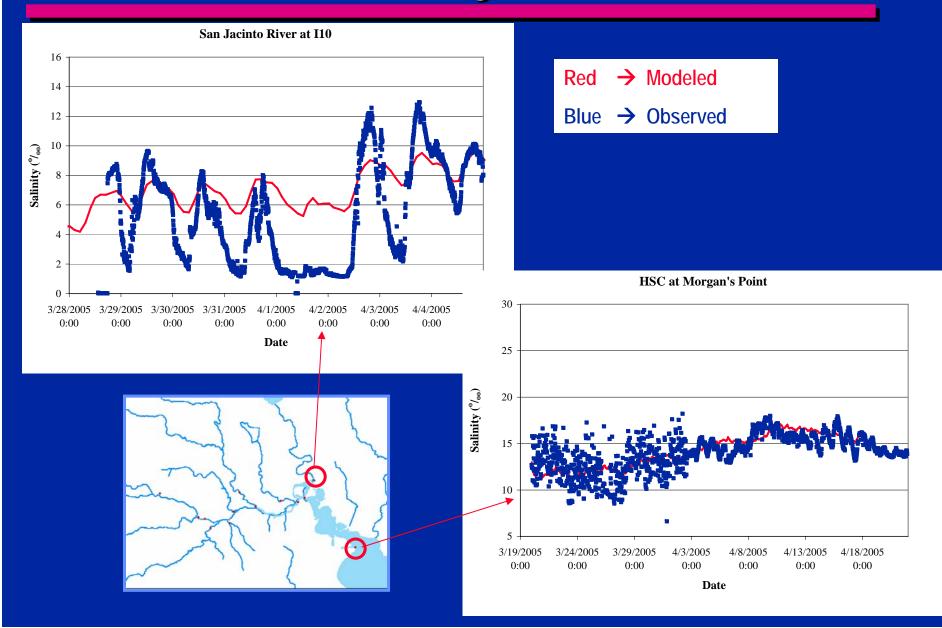
0:00

Date

12:00

0:00

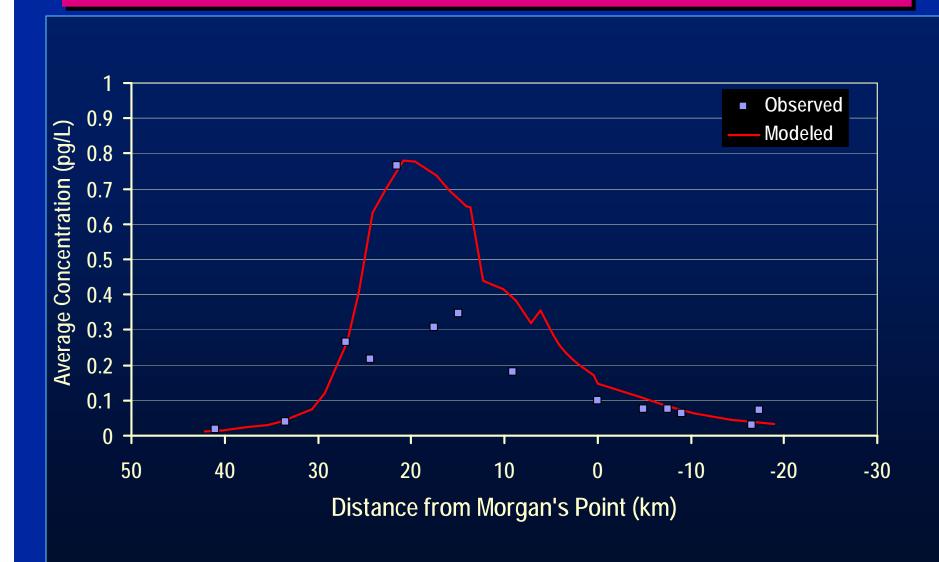
WASP model – salinity calibration (cont'd)



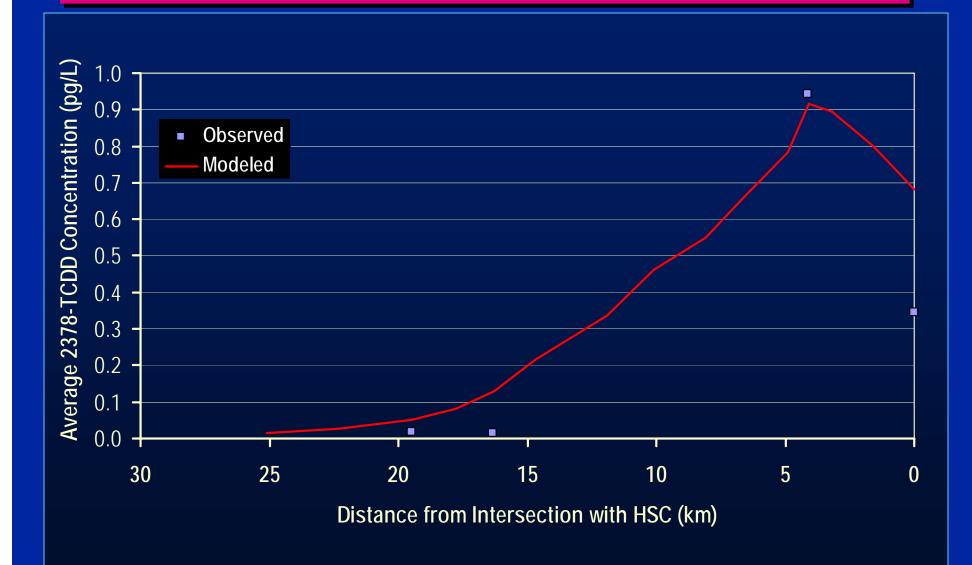
WASP 2378-TCDD model (2005)

- gPS loads for direct discharges to WASP segments:QQSelf-reported
- $_{\rm q}$ NPS loads and PS discharging u/s model segments: ${\rm Q}_{\rm USGS\ gage}^* {\rm Concentration}$
 - **Runoff concentration for rainy days**
 - $_{\rm q}$ Tributary concentration for dry days
- **^q** Direct deposition: deposition flux*area
- ^q March-April 2005 was generally a dry period so NPS are lower than expected for rest of year
- gLong-term simulation will account for dry and wetperiods

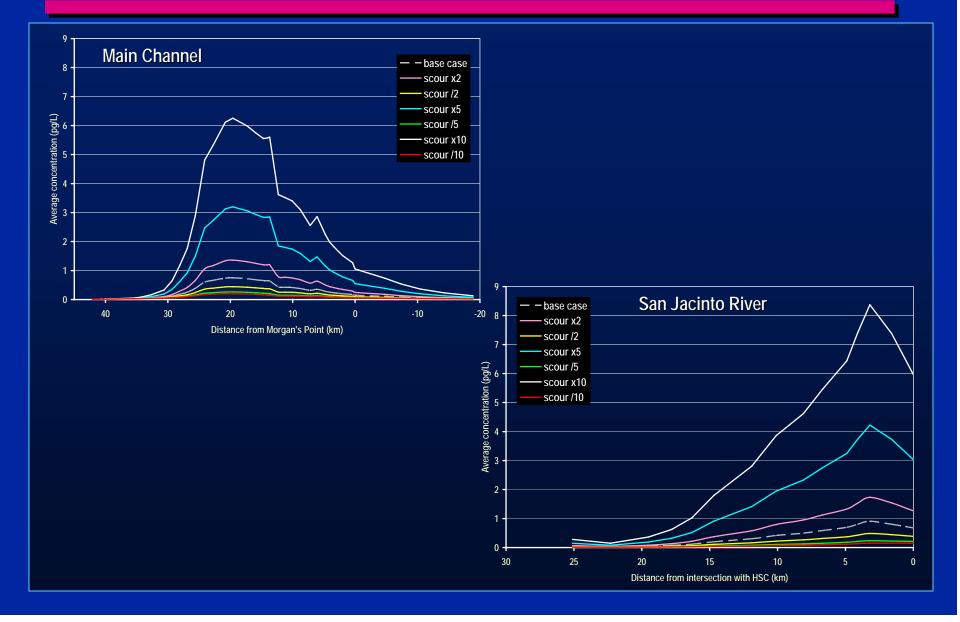
WASP 2378-TCDD model (2005) – main channel



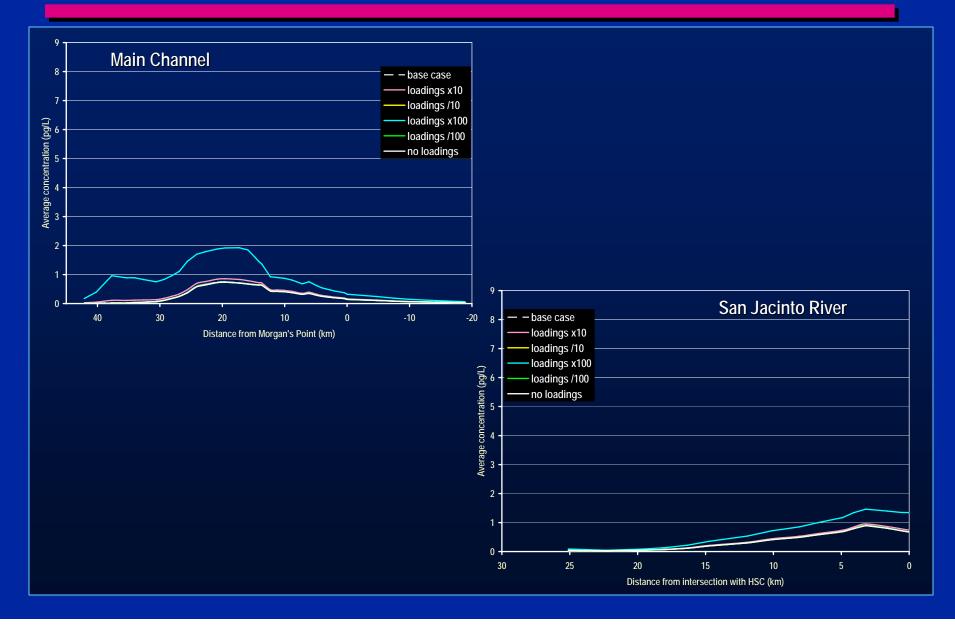
WASP 2378-TCDD model (2005) – San Jac



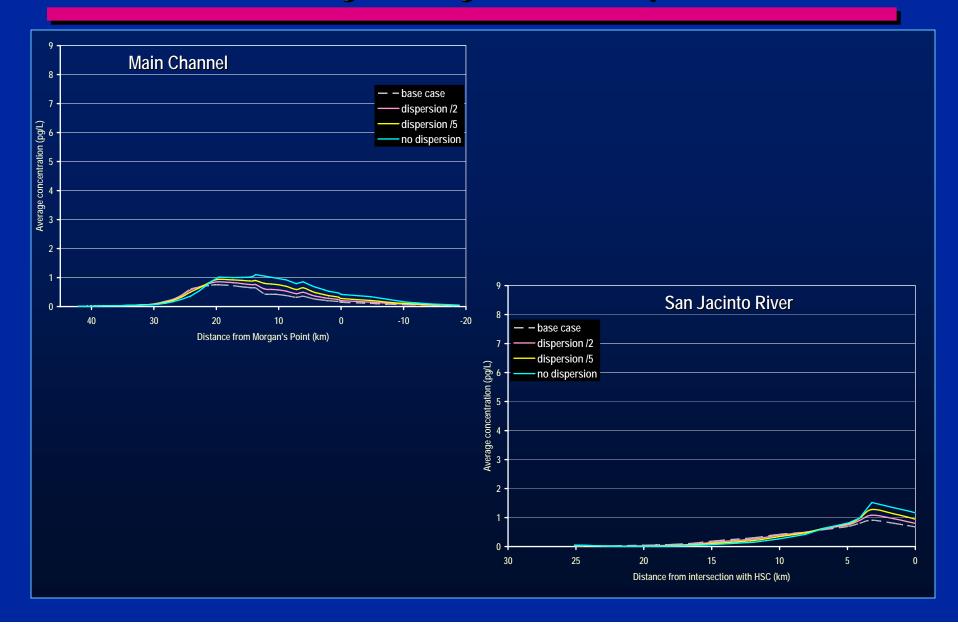
WASP sensitivity analysis – scour velocity



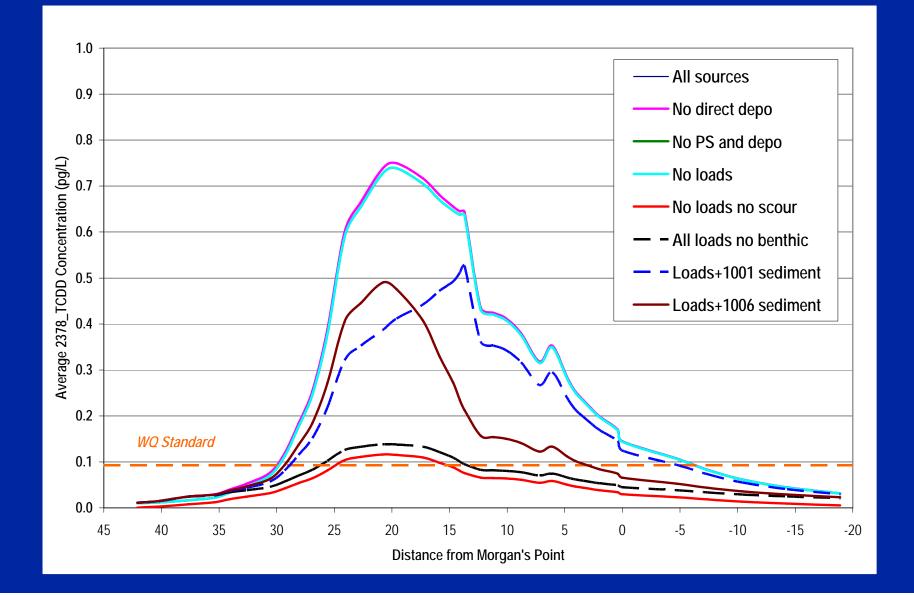
WASP sensitivity analysis – loading



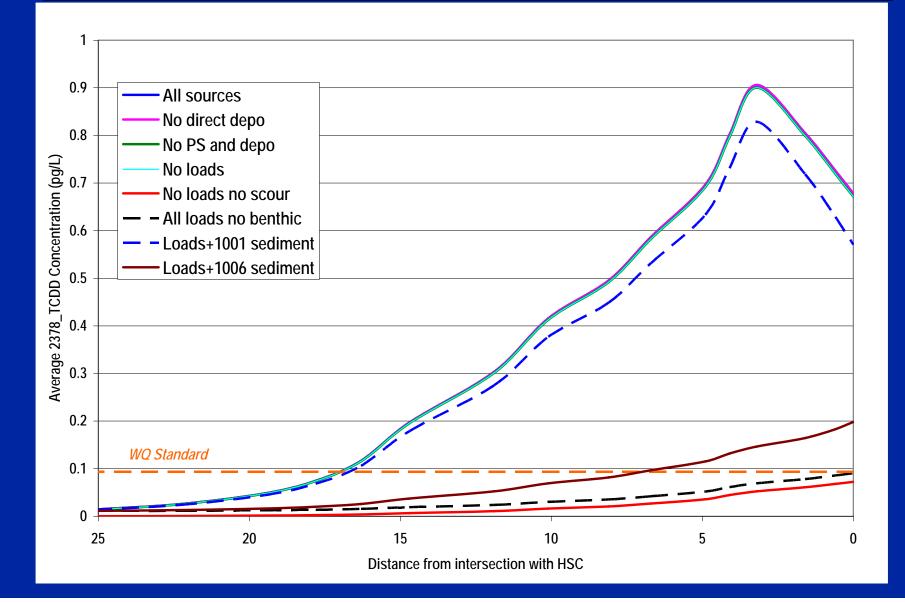
WASP sensitivity analysis – dispersion



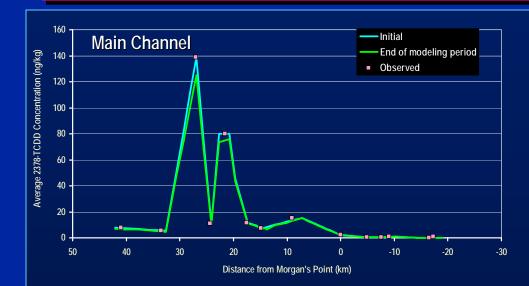
WASP load scenarios – main channel



WASP load scenarios – main channel

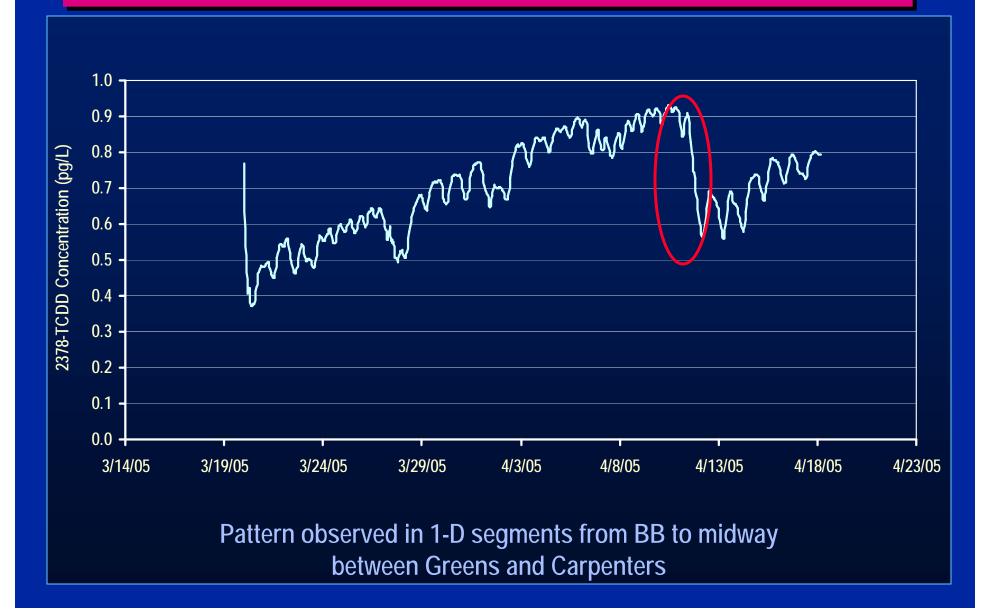


WASP benthic segments





WASP issues



Load allocation spreadsheet model

Point source load estimates

$_{\rm q}~$ 2378-TCDD and TEQ

g 5-year average of self-reported flows

^q Dioxin concentrations

- g If effluent sampled in 2003, measured concentration
- If only sludge measured in 2002, used sludgeeffluent regression
- gIf PS not sampled, average concentration for SICcode

Runoff load estimates

g 2378-TCDD and TEQ

- Flows determined using SCS curve method and average daily precipitation for year 2002 (total yearly precipitation divided by 365)
- Dioxin concentrations in runoff measured in 2003 and 2005 assigned by proximity to watersheds

Direct deposition load estimates

$_{\rm q}$ 2378-TCDD and TEQ

- ^q Deposition fluxes measured in this project (100% non-detects for 2378-TCDD)
 - Wet: 0.6 pg/m²/day for 2378-TCDD and 10 pg/m²/day for TEQ
 - g Dry: 0.4 pg/m²/day for 2378-TCDD and 2.4 pg/m²/day for TEQ
- qFluxes multiplied by surface area of the
water quality segments
- $_{\rm q}~$ Non-detects assumed as ½ MDL

In-stream load estimates

$_{\rm q}$ 2378-TCDD and TEQ

- Net flow out of each segment (average of flows simulated for the period March-April 2005 at downstream end of segments)
- Average water concentrations at locations
 where flow was measured
- Load for a given segment is load out of the segment minus load from upstream segments

Load spreadsheet – preliminary mass balance

Segment	In-stream load ^a	Source Loads (ng/day)					
		Point Sources	Stormwater Runoff	Direct deposition	Sediment? ^b		
1014+1017	7,867	15,953	35,127	291	-43,504		
1007	555,615	43,941	34,607	4,566	472,501		
1016	27,736	7,434	30,633	581	-10,912		
1006	1,430,737	19,275	4,673	6,259	1,400,529		
1001 upper	145,107	4,071	47,568	4,644	88,824		
1001 lower	7,185,593		15,856	1,548	7,168,189		
2430	640		379	5,525	-5,264		
Old River	1,170			516	654		
2429	2,985		344	3,859	-1,219		
2428	207		207	3,178	-3,177		
2427	6,234	1,696	310	5,339	-1,111		
2426	75,048	593	517	10,394	63,545		
2436	465	50		268	147		
1005	1,063,016	4,876	1,557	13,319	1,043,264		
2438	4,807			677	4,130		

^a Average concentration measured in 2002-2004 times net flow out of segment

^b Difference between in-stream load and the sum of loads from

Non-detects assumed equal to 1/2MDL for load calculations

Load spreadsheet – preliminary overall reduction

Segment	Net Flow ^a (m ³ /s)	Allowable Load (ng/day)	In-stream Load (ng/day)	% Overall Reduction
1014+1017	10.3	8,341	7,867	0%
1007	24.7	19,944	555,615	96%
1016	5.4	4,357	27,736	84%
1006	30.8	24,847	1,430,737	98%
1001 upper	90.3	72,787	145,107	50%
1001 lower	90.1	72,637	7,185,593	99%
2430	0.05	38	640	94%
Old River	0.7	575	1,170	51%
2429	0.2	148	2,985	95%
2428	0.07	55	207	74%
2427	0.21	170	6,234	97%
2426	5.1	4,114	75,048	95%
2436	0.01	12	465	98%
1005	124.8	100,588	1,063,016	91%
2438	3.2	2,577	4,807	46%

^a Average of simulated flows out of segment for period March-April 2005

Load spreadsheet – load reductions example1

WWTP Reduction	70%
Stormwater Runoff Reduction	70%
Direct Depo Reduction	70%
Sediment Load Reduction	90%

	Net Flow ^a (m ³ /s)	Allowable Load (ng/day)	Reduced Loads				
Segment			Point Sources		Non-point Sources		Meet
			WWTPs	Stormwater runoff (MS4s)	Direct deposition	Sediment?	Allowable Load?
1014+1017	10.3	8,341	4,786	10,538	87	0	No
1007	24.7	19,944	13,182	10,382	1,370	47,250	No
1016	5.4	4,357	2,230	9,190	174	0	No
1006	30.8	24,847	5,783	1,402	1,878	140,053	No
1001 upper	90.3	72,787	1,221	14,270	1,393	8,882	Yes
1001 lower	90.1	72,637	0	4,757	464	716,819	No
2430	0.05	38	0	114	1,658	0	No
Old River	0.7	575	0	0	155	65	Yes
2429	0.2	148	0	103	1,158	0	No
2428	0.07	55	0	62	953	0	No
2427	0.21	170	509	93	1,602	0	No
2426	5.1	4,114	178	155	3,118	6,355	No
2436	0.01	12	15	0	80	15	No
1005	124.8	100,588	1,463	467	3,996	104,326	No
2438	3.2	2,577	0	0	203	413	Yes

^a Average of simulated flows out of segment for period March-April 2005

Load spreadsheet – load reductions example2

WWTP Reduction	0%
Stormwater Runoff Reduction	0%
Direct Depo Reduction	0%
Sediment Load Reduction	9 5%

	Net Flow ^a (m ³ /s)	Allowable Load (ng/day)	Reduced Loads				
Segment			Point Sources		Non-point Sources		Meet
			WWTPs	Stormwater Runoff (MS4s)	Direct deposition	Sediment?	Allowable Load?
1014+1017	10.3	8,341	15,953	35,127	291	0	No
1007	24.7	19,944	43,941	34,607	4,566	23,625	No
1016	5.4	4,357	7,434	30,633	581	0	No
1006	30.8	24,847	19,275	4,673	6,259	70,026	No
1001 upper	90.3	72,787	4,071	47,568	4,644	4,441	Yes
1001 lower	90.1	72,637	0	15,856	1,548	358,409	No
2430	0.05	38	0	379	5,525	0	No
Old River	0.7	575	0	0	516	33	Yes
2429	0.2	148	0	344	3,859	0	No
2428	0.07	55	0	207	3,178	0	No
2427	0.21	170	1,696	310	5,339	0	No
2426	5.1	4,114	593	517	10,394	3,177	No
2436	0.01	12	50	0	268	7	No
1005	124.8	100,588	4,876	1,557	13,319	52,163	Yes
2438	3.2	2,577	0	0	677	206	Yes

^a Average of simulated flows out of segment for period March-April 2005

Summary

- Hydrodynamic model finished (6 min timestep), a few mass-balance issues remain
- qRunoff loads entered to WASP as the measured
concentrations (dry and runoff) and USGS flows
- gWASP model very sensitive to sediment-relatedparameters
- Preliminary dioxin results indicate source in 1001 affects dioxin levels in 1006
- qPreliminary load calculationsand model resultsindicated major contribution from sediment

Next steps

- **Address remaining RMA2/WASP issues**
- Calibrate dioxin model to concentrations
 measured between 2002 and 2004 (long-term run)
- $_{\rm q}~$ Refine scour and settling models
- **Run load reduction scenarios**
- gUpdate load spreadsheet model and defineTMDL